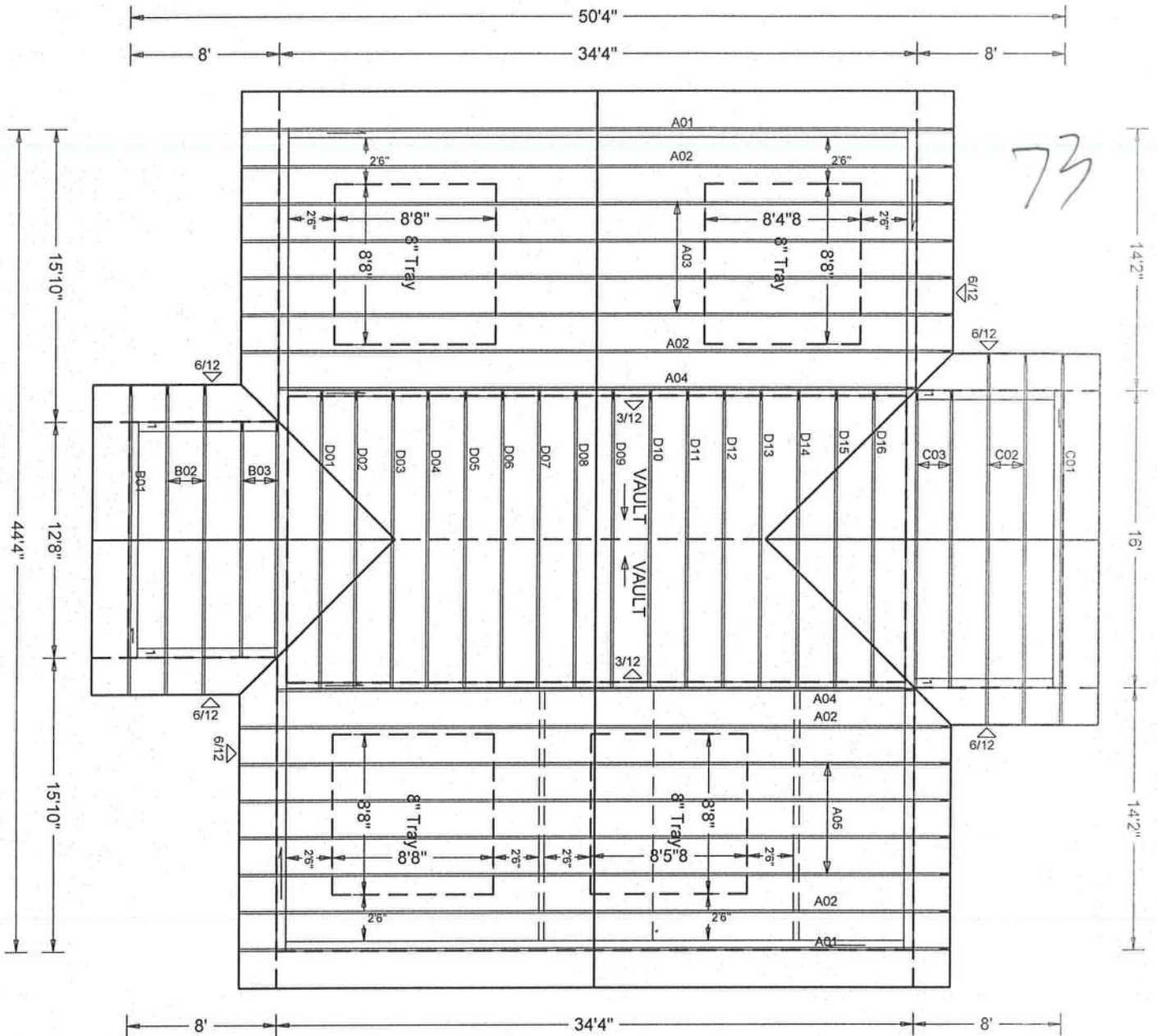




ALL BEAMS BY OTHERS

ROOF PITCH: 6/12
OVERHANG: 2'
CEILING: 10'
EXT. WALLS: 6"
LOADING: 40psf
WIND LOAD: 130mph
EXPOSURE: C

DATE: 9/20/23

W.B. Howland Truss Co.
610 11th St. SW
Live Oak, FL 32064
(386) 362-1235
(386) 362-7124 (Fax)
howlandtruss@gmail.com



JOB #: 23-9993

Job Name: Kyle McLeod- Office
Customer: OWNER BUILDER
Designer: Kelly Caudill
ADDRESS:
Salesman: HOUSE
: 09-21-2023

JOB NO:

23-9993

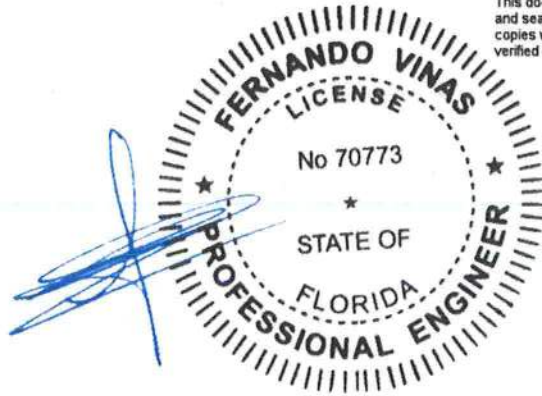
PAGE NO:

1 OF 1

This document has been electronically signed
and sealed using a Digital Signature. Printed
copies without an original signature mark are
verified using the original electronic version.



Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com



09/20/2023

COA#0-278

Florida Certificate of Product Approval #FL1999



Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 23-9993
Job Description: Kyle McLeod- Office	
Address: LAKE CITY, FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 22.02.00
	JRef #: 1XT92150006
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 27 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	263.23.1525.41610	A01
3	263.23.1525.45607	A03
5	263.23.1526.01477	A05
7	263.23.1526.23207	B02
9	263.23.1526.41090	C01
11	263.23.1527.05770	C03
13	263.23.1528.33813	D02
15	263.23.1528.36743	D04
17	263.23.1528.40730	D06
19	263.23.1529.13467	D08
21	263.23.1529.38977	D10
23	263.23.1530.03890	D12
25	263.23.1530.43483	D14
27	263.23.1530.56710	D16
29	BRCLBSUB0119	
31	160TL	

Item	Drawing Number	Truss
2	263.23.1525.43210	A02
4	263.23.1525.56230	A04
6	263.23.1526.09490	B01
8	263.23.1526.28437	B03
10	263.23.1526.46290	C02
12	263.23.1531.04410	D01
14	263.23.1528.35420	D03
16	263.23.1528.39367	D05
18	263.23.1528.49073	D07
20	263.23.1529.28053	D09
22	263.23.1529.55760	D11
24	263.23.1530.34187	D13
26	263.23.1530.45357	D15
28	A14015ENC160118	
30	GBLLETIN0118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

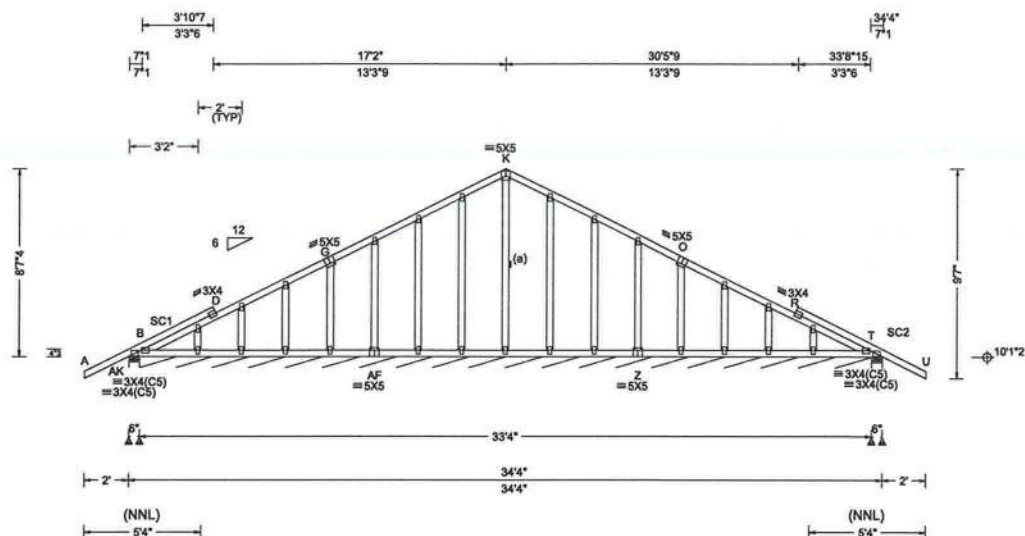
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 724234 FROM: CDM	GABL	Ply: 1 Qty: 2	Job Number: 23-9993 Kyle McLeod- Office Truss Label: A01	Cust: R 215 JRef: 1XT92150006 T2 DrwNo: 263.23.1525.41610 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.43 ft Loc. from endwall: Any GCpt: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 D 999 240 VERT(CL): 0.004 D 999 180 HORZ(LL): 0.005 R - - HORZ(TL): 0.007 R - - Creep Factor: 2.0 Max TC CSI: 0.396 Max BC CSI: 0.040 Max Web CSI: 0.147 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity AK 348 /- /- /199 /63 /274 B* 72 /- /- /39 /13 /- T 348 /- /- /240 /51 /- Wind reactions based on MWFRS AK Brg Wid = 6.0 Min Req = 1.5 (Truss) B Brg Wid = 400 Min Req = - T Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings AK, B, & T are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.

The overall height of this truss excluding overhang is 8-7-4.



COA#0-278
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09/20/2023

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

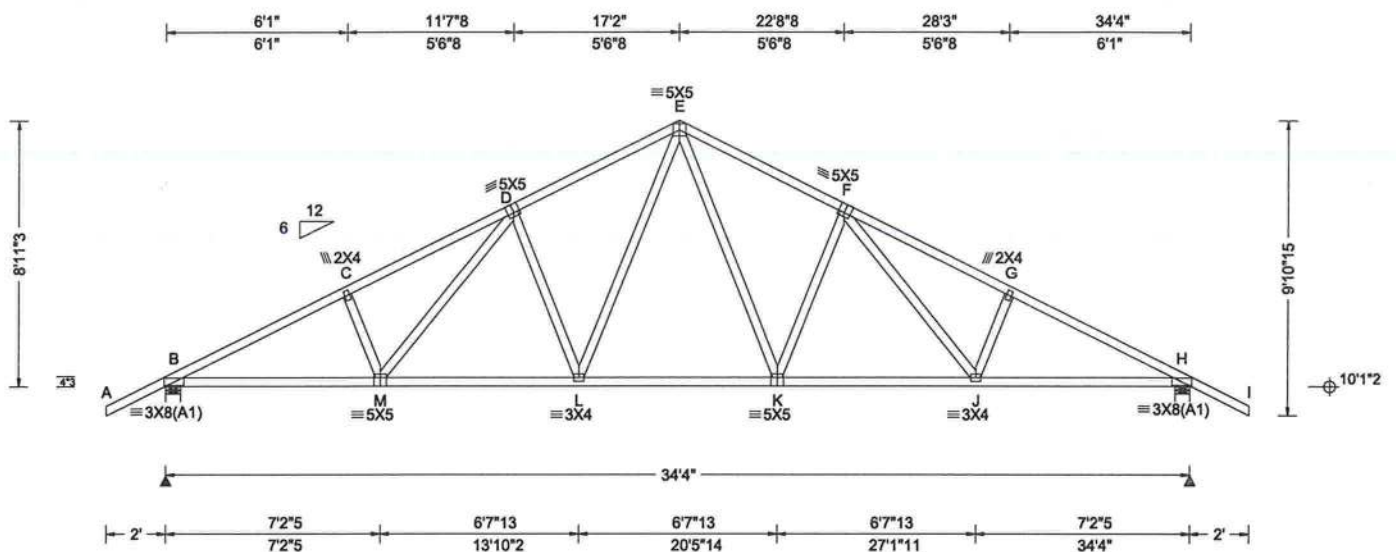
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724236 FROM: CDM	COMN	Ply: 1 Qty: 4	Job Number: 23-9993 Kyle McLeod- Office Truss Label: A02	Cust: R 215 JRef: 1XT92150006 T1 DrwNo: 263.23.1525.43210 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.43 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.168 L 999 240 VERT(CL): 0.311 L 999 180 HORZ(LL): 0.065 H - - HORZ(TL): 0.121 H - - Creep Factor: 2.0 Max TC CSI: 0.496 Max BC CSI: 0.792 Max Web CSI: 0.531 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1702 - / - / - / 939 / 277 / 276 H 1702 - / - / - / 939 / 277 / - Wind reactions based on MWFRS B Brg Wid = 6.0 Min Req = 2.0 (Truss) H Brg Wid = 6.0 Min Req = 2.0 (Truss) Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1033 -2902 E - F 970 -2218 C - D 1085 -2775 F - G 1084 -2776 D - E 970 -2220 G - H 1032 -2903

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8-11-3.



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09/20/2023

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCE) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

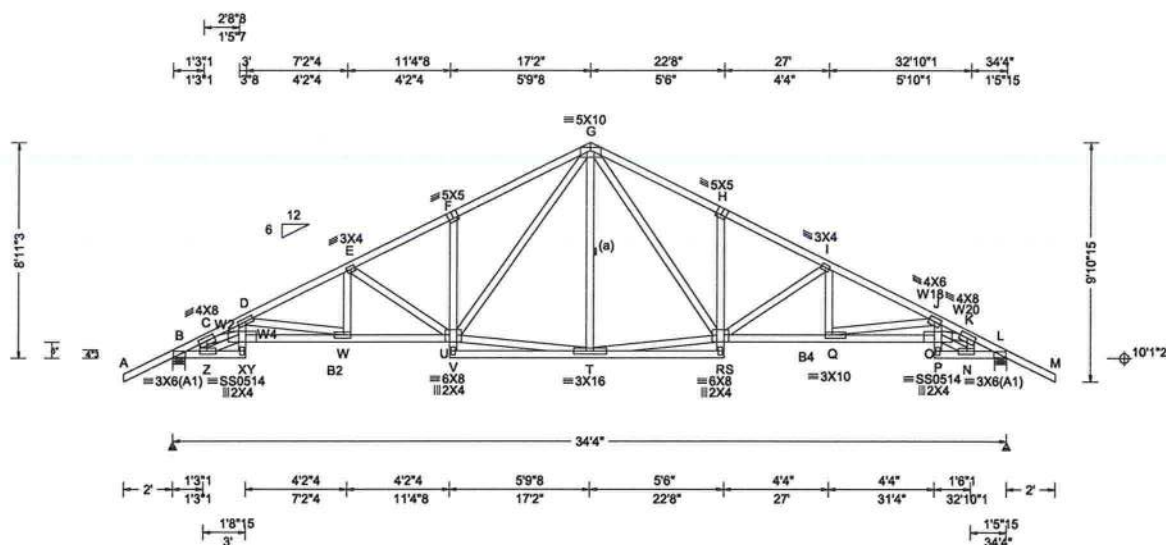
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCE: sbcecomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724231 FROM: CDM	COMN	Ply: 1 Qty: 4	Job Number: 23-9993 Kyle McLeod- Office Truss Label: A03	Cust: R 215 JRef: 1XT92150006 T19 DrwNo: 263.23.1525.45607 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.43 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, 18SS	PP Deflection in loc L/defl L/# VERT(LL): 0.255 F 999 240 VERT(CL): 0.515 F 793 180 HORZ(LL): 0.163 L - - HORZ(TL): 0.328 L - - Creep Factor: 2.0 Max TC CSI: 0.611 Max BC CSI: 0.569 Max Web CSI: 0.960 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1548 - / - / - / 939 / 277 / 276 L 1548 - / - / - / 939 / 277 / - Non-Gravity Wind reactions based on MWFRS B Brg Wid = 6.0 Min Req = 1.8 (Truss) L Brg Wid = 6.0 Min Req = 1.8 (Truss) Bearings B & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 407 -2232 G - H 661 -2384 C - D 974 -5232 H - I 557 -2395 D - E 604 -3005 I - J 603 -2985 E - F 563 -2432 J - K 995 -5300 F - G 670 -2430 K - L 422 -2281

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B2, B4 2x4 SP M-31;
Webs: 2x4 SP #3; W2, W4, W18, W20 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X8 except as noted.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 8-11-3.



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09/20/2023

Chords	Tens.Comp.	Chords	Tens. Comp.
B - Z	1898 -278	R - Q	2617 -405
X - W	4884 -837	Q - O	4923 -858
W - U	2637 -402	N - L	1946 -298

Webs	Tens.Comp.	Webs	Tens. Comp.
C - Z	178 -1033	G - R	1194 -312
C - X	2852 -536	T - R	1324 -86
Z - X	1711 -237	R - I	156 -639
X - D	1341 -176	Q - J	462 -2326
D - W	445 -2269	J - O	1368 -182
E - U	145 -620	O - N	1882 -276
U - G	1213 -317	O - K	2870 -538
U - T	1315 -86	N - K	217 -1248

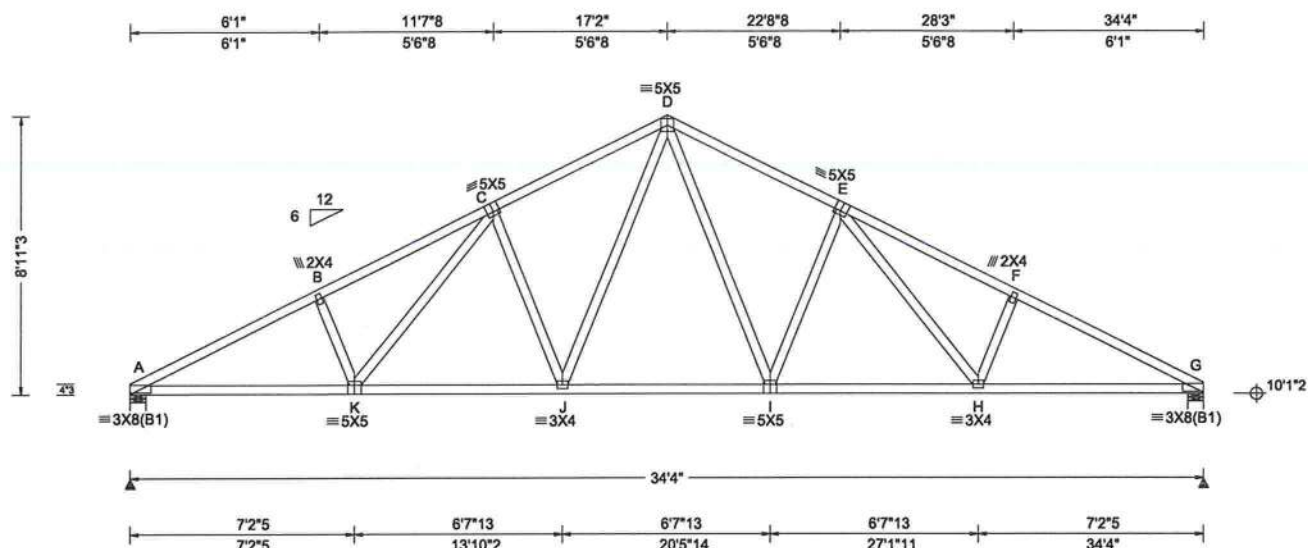
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724227 FROM: CDM	COMN Ply: 1 Qty: 2	Job Number: 23-9993 Kyle McLeod- Office Truss Label: A04	Cust: R 215 JRef: 1XT92150006 T4 DrwNo: 263.23.1525.56230 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity				
	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.166 J 999 240	Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	
	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.309 J 999 180	A	1569	/-	/-	/825	/24	/225
	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.065 G - -	G	1569	/-	/-	/825	/24	/-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.121 G - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	A Brg Wid = 6.0 Min Req = 1.9 (Truss)						
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.394	G Brg Wid = 6.0 Min Req = 1.9 (Truss)						
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.791	Bearings A & G are a rigid surface.						
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.430	Members not listed have forces less than 375#						
	C&C Dist a: 3.43 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)						
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Chords	Tens.Comp.	Chords	Tens.	Comp.		
	GCpi: 0.18	WAVE	VIEW Ver: 22.02.00.0914.12	A - B	580 -2960	D - E	564	-2241		
	Wind Duration: 1.60			B - C	625 -2834	E - F	624	-2836		
Lumber				C - D	564 -2243	F - G	580	-2961		

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8-11-3.



COA#0-278

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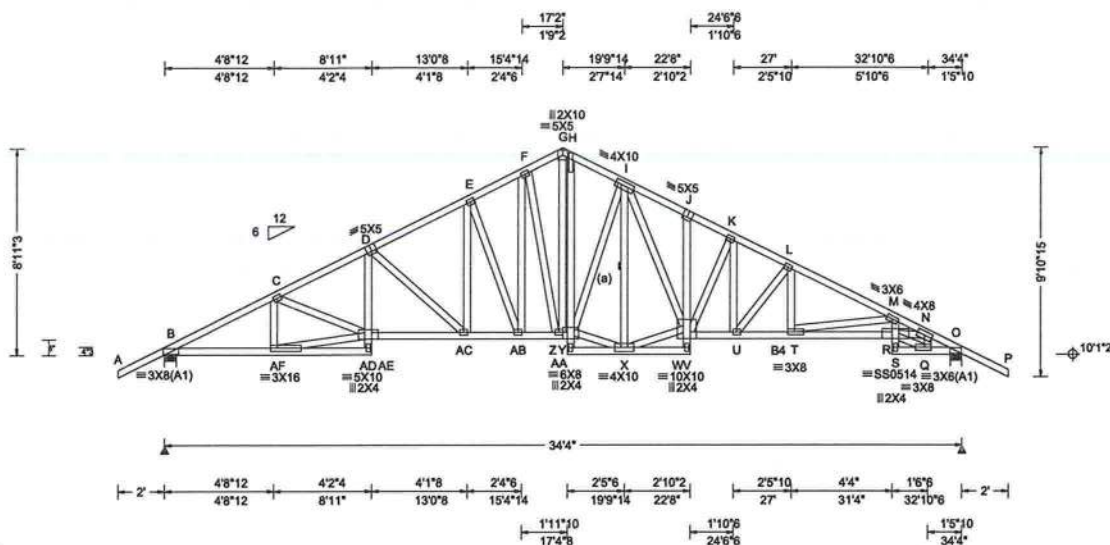
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724220 FROM: CDM	COMN	Ply: 1 Qty: 4	Job Number: 23-9993 Kyle McLeod- Office Truss Label: A05	Cust: R 215 JRef: 1XT92150006 T30 DrwNo: 263.23.1526.01477 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Def/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.43 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	PP Deflection in loc L/defl L/# VERT(LL): 0.232 J 999 240 VERT(CL): 0.468 J 872 180 HORZ(LL): 0.129 O - - HORZ(TL): 0.260 O - - Creep Factor: 2.0 Max TC CSI: 0.506 Max BC CSI: 0.987 Max Web CSI: 0.943 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ /R- /Rh /Rw /U /RL B 1548 -/- /- /939 /277 /276 O 1548 -/- /- /939 /277 -/ Non-Gravity Wind reactions based on MWFRS B Brg Wid = 6.0 Min Req = 1.8 (Truss) O Brg Wid = 6.0 Min Req = 1.8 (Truss) Bearings B & O are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 521 -2588 I - J 616 -2384 C - D 597 -2800 J - K 572 -2400 D - E 537 -2192 K - L 586 -2570 E - F 533 -1894 L - M 605 -2976 F - G 516 -1709 M - N 912 -4821 G - H 457 -1325 N - O 420 -2272 H - I 519 -1807

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B4 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

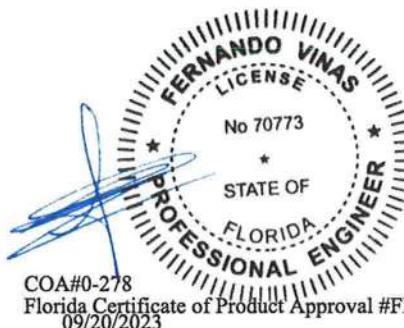
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8-11.3.



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - AF	2246 -363	V - U	2261 -316
AD-AC	2469 -362	U - T	2595 -402
AC-AB	1885 -215	T - R	4447 -775
AB-AA	1635 -142	Q - O	1936 -295
AA- Y	1426 -70		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - AF	136 -410	X - I	151 -1098
AF-AD	2181 -350	X - V	1631 -169
AD-D	526 -56	I - V	1319 -256
D-AC	195 -769	U - L	148 -587
AC-E	542 -84	L - T	377 -17
E-AB	202 -695	T - M	378 -1859
AB-F	508 -140	M - R	1027 -120
F-AA	189 -474	R - Q	1850 -270
AA-G	449 -236	R - N	2461 -469
Y-H	954 -158	Q - N	201 -1141
Y-X	1660 -173		

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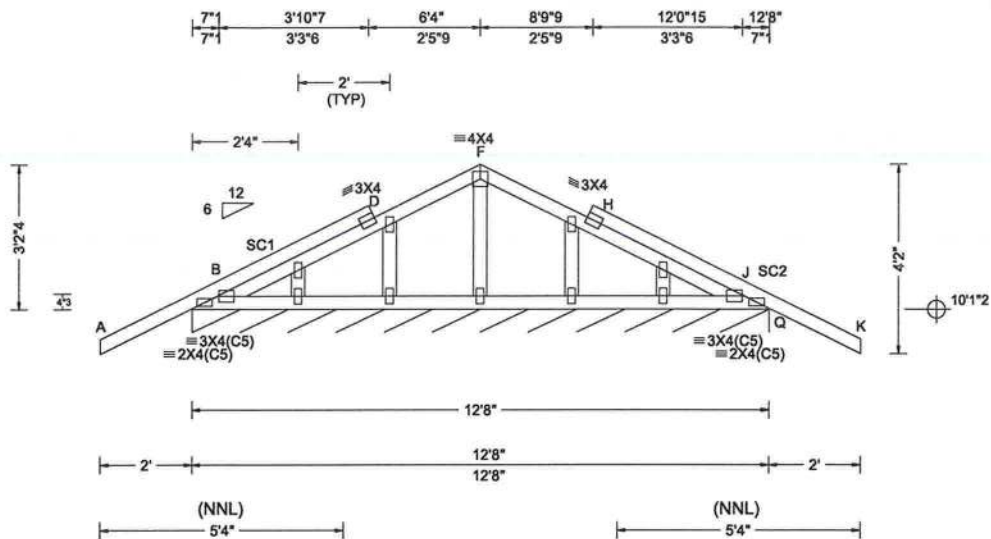
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724181 FROM: CDM	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: B01	Cust: R 215 JRef: 1XT92150006 T8 DrwNo: 263.23.1526.09490 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Def/CSI Criteria	Δ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): -0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.411 Max BC CSI: 0.026 Max Web CSI: 0.055 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Q* 103 /- /- /51 /19 /10 Wind reactions based on MWFRS Q Brg Wid = 152 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 3-2-4.



COA#0-278

Florida Certificate of Product Approval #FL1999
09/20/2023

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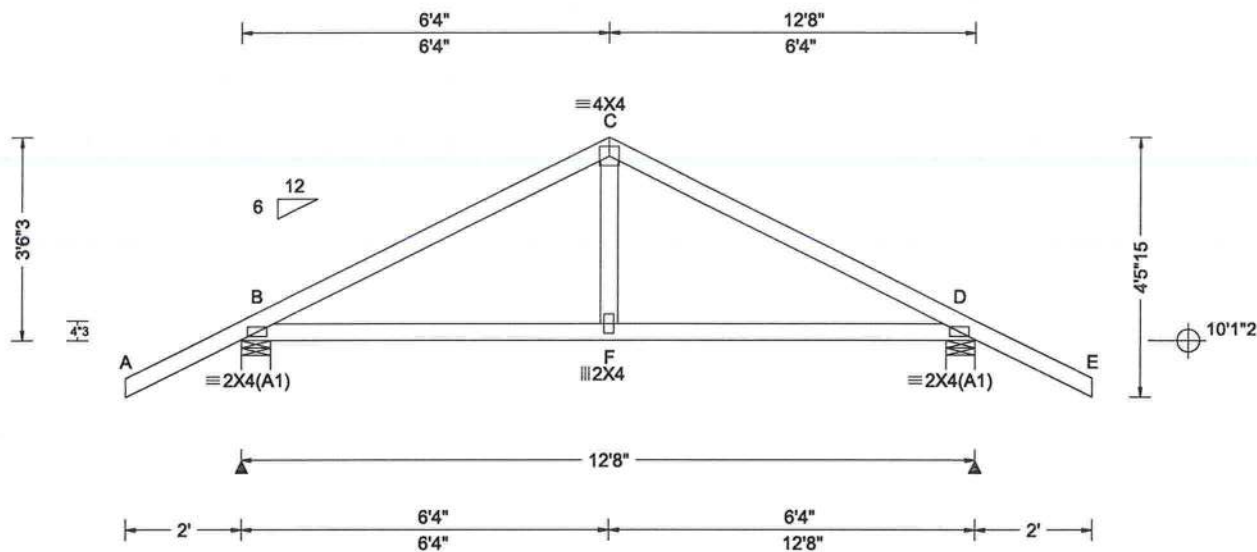
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ALPINE
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724183 FROM: CDM	COMN Qty: 2	Ply: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: B02	Cust: R 215 JRef: 1XT92150006 T5 DrwNo: 263.23.1526.23207 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Δ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.011 F 999 240 VERT(CL): 0.022 F 999 180 HORZ(LL): 0.005 D - - HORZ(TL): 0.009 D - - Creep Factor: 2.0 Max TC CSI: 0.336 Max BC CSI: 0.377 Max Web CSI: 0.103 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 655 /- /- /420 /123 /132 D 655 /- /- /420 /123 /- Wind reactions based on MWFRS B Brg Wid = 6.0 Min Req = 1.5 (Truss) D Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 365 -662 C - D 366 -662

Lumber	Maximum Bot Chord Forces Per Ply (lbs)
Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;	Chords Tens.Comp. Chords Tens. Comp. B - F 522 -121 F - D 522 -121

Wind
Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.

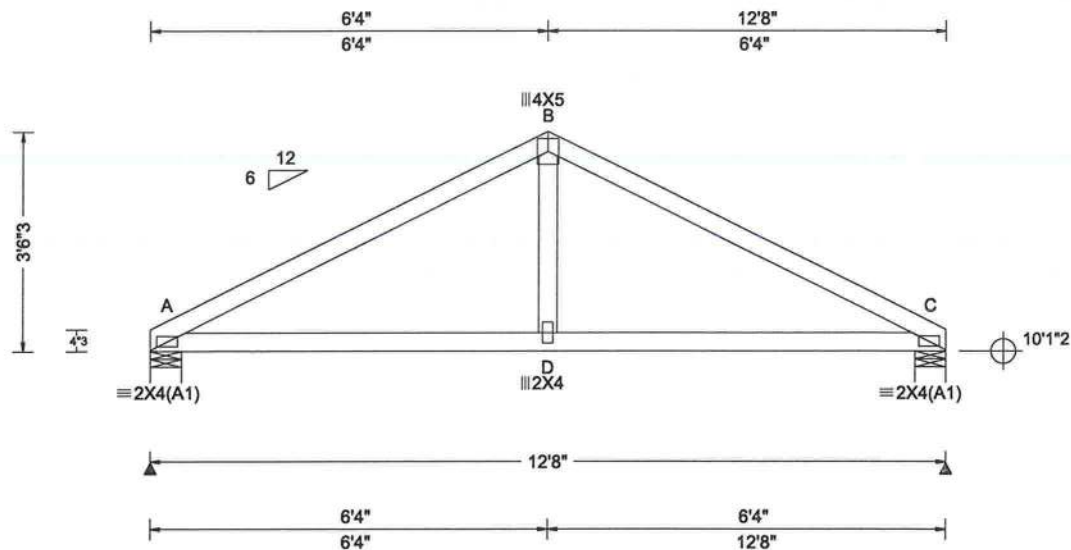
Additional Notes
The overall height of this truss excluding overhang is 3-6-3.



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SEQN: 724185 FROM: CDM	COMN Qty: 2	Ply: 1 Job Number: 23-9993 Kyle McLeod- Office Truss Label: B03	Cust: R 215 JRef: 1XT92150006 T7 DrwNo: 263.23.1526.28437 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 D 999 240 VERT(CL): 0.019 D 999 180 HORZ(LL): 0.006 A - - HORZ(TL): 0.011 A - - Creep Factor: 2.0 Max TC CSI: 0.403 Max BC CSI: 0.410 Max Web CSI: 0.108 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 522 /- /- /301 /88 /81 C 522 /- /- /301 /88 /- Wind reactions based on MWFRS A Brg Wid = 6.0 Min Req = 1.5 (Truss) C Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings A & C are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 350 -731 B - C 349 -731

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3'-6"-3".



COA#0-278
Florida Certificate of Product Approval #FL1999
09/20/2023

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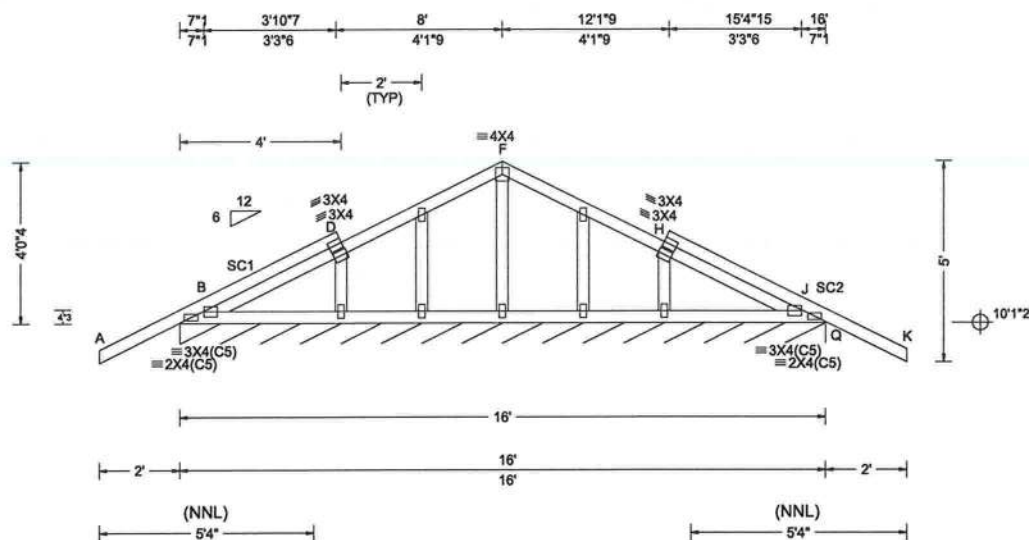
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724187 FROM: CDM	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: C01	Cust: R 215 JRef: 1XT92150006 T9 DrwNo: 263.23.1526.41090 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 J 999 240 VERT(CL): 0.003 J 999 180 HORZ(LL): -0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.392 Max BC CSI: 0.064 Max Web CSI: 0.047 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ /R- /Rh /Rw /U /RL Non-Gravity Q* 99 /- /- /50 /18 /9 Wind reactions based on MWFRS Q Brg Wid = 192 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 4-0-4.



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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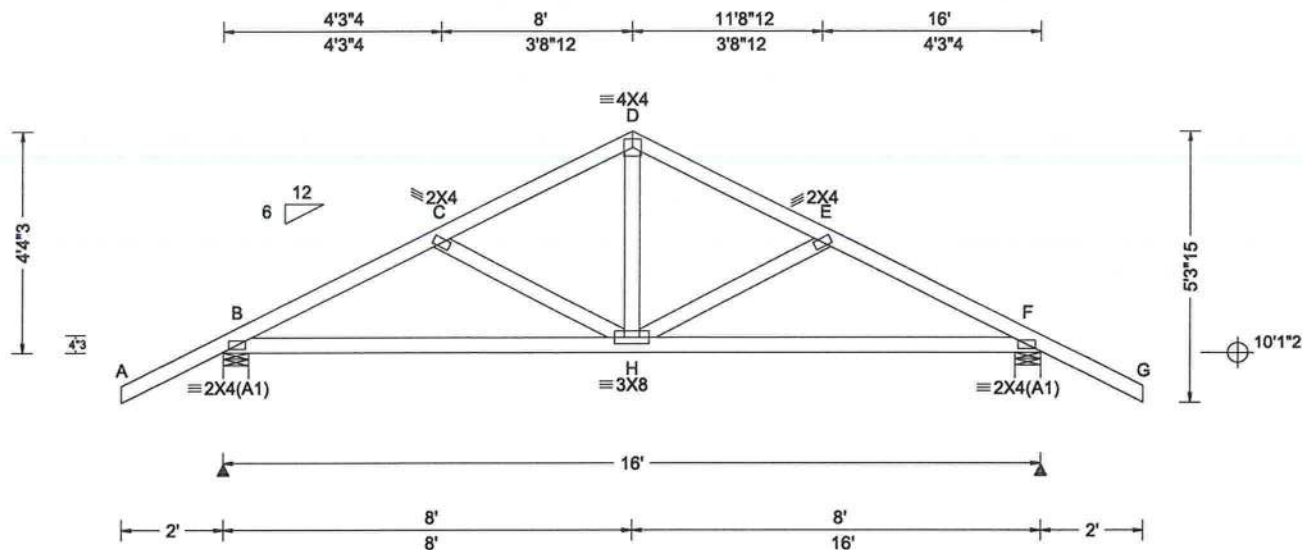
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724189 FROM: CDM	COMN	Ply: 1 Qty: 2	Job Number: 23-9993 Kyle McLeod- Office Truss Label: C02	Cust: R 215 JRef: 1XT92150006 T8 DrwNo: 263.23.1526.46290 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.025 H 999 240 VERT(CL): 0.050 H 999 180 HORZ(LL): 0.011 F - - HORZ(TL): 0.021 F - - Creep Factor: 2.0 Max TC CSI: 0.292 Max BC CSI: 0.566 Max Web CSI: 0.169 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh Non-Gravity / Rw / U / RL B 793 /- /- /497 /146 /153 F 793 /- /- /497 /146 /- Wind reactions based on MWFRS B Brg Wid = 6.0 Min Req = 1.5 (Truss) F Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 540 -992 D - E 424 -755 C - D 424 -755 E - F 540 -992

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-4-3.



COA#0-278
Florida Certificate of Product Approval #FL1999
09/20/2023

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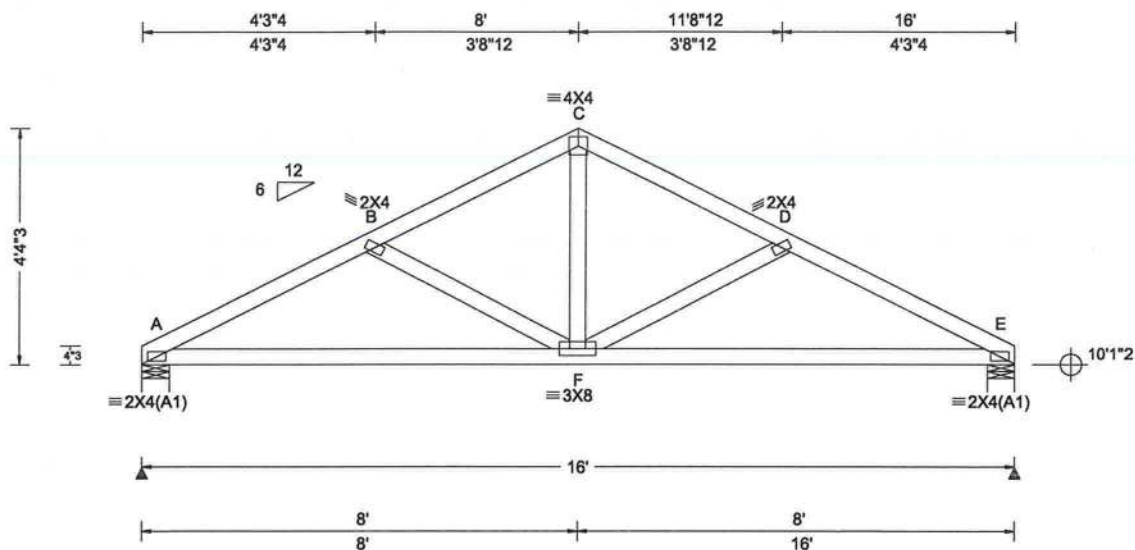
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724191 FROM: CDM	COMN Qty: 2	Ply: 1 Job Number: 23-9993 Kyle McLeod- Office Truss Label: C03	Cust: R 215 JRef: 1XT92150006 T12 DrwNo: 263.23.1527.05770 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.024 F 999 240 VERT(CL): 0.049 F 999 180 HORZ(LL): 0.011 E - - HORZ(TL): 0.022 E - - Creep Factor: 2.0 Max TC CSI: 0.192 Max BC CSI: 0.591 Max Web CSI: 0.179 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh Non-Gravity / Rw / U / RL A 659 /- /- /380 /111 /103 E 659 /- /- /380 /111 /- Wind reactions based on MWFRS A Brg Wid = 6.0 Min Req = 1.5 (Truss) E Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 443 -1067 C - D 346 -806 B - C 347 -806 D - E 444 -1067

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-4-3.



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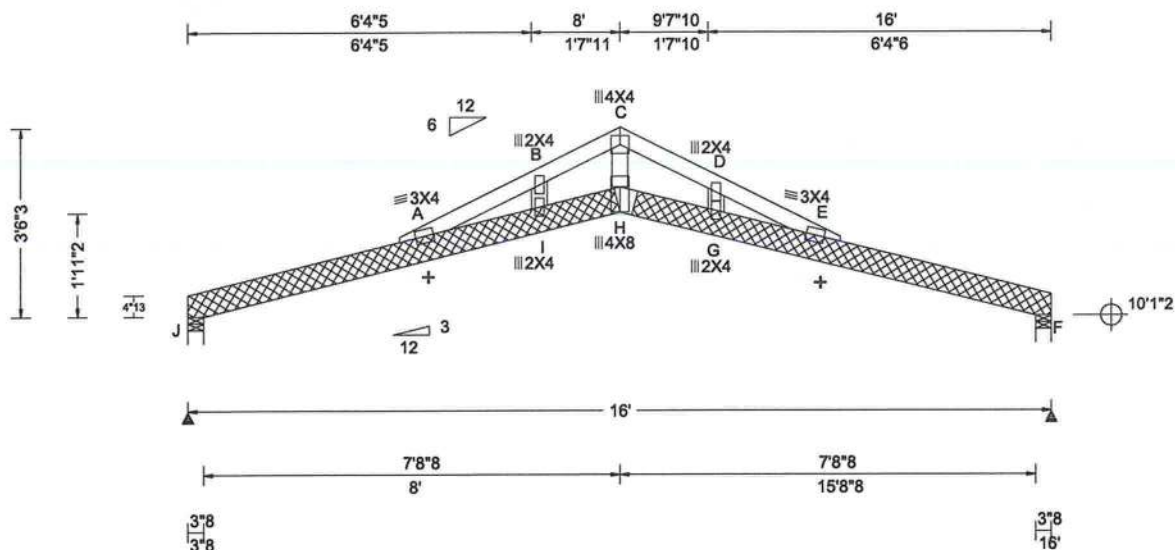
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724153 FROM: CDM	MONO Qty: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: D01	Cust: R 215 JRef: 1XT92150006 T18 DrwNo: 263.23.1531.04410 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	R-	/ Rh	/ Rw	/ U	/ RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.291 H 659 240	J	657	-	-	/257	/11	/50
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.598 H 321 180	F	657	-	-	/257	/11	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.152 F - -	Wind reactions based on MWFRS						
	EXP: C Kzt: NA		HORZ(TL): 0.312 F - -	J Brg Wid = 3.5 Min Req = 1.5 (Truss)						
Des Ld: 40.00	Mean Height: 15.00 ft		Creep Factor: 2.0	F Brg Wid = 3.5 Min Req = 1.5 (Truss)						
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Max TC CSI: 0.266	Bearings J & F are a rigid surface.						
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max BC CSI: 0.391	Members not listed have forces less than 375#						
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max Web CSI: 0.759	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes		Chords Tens.Comp.		Chords Tens. Comp.				
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		A - B	629	-2615	C - D	658	-2480	
	GCpi: 0.18	Plate Type(s):	VIEW Ver: 22.02.00.0914.12	B - C	656	-2475	D - E	644	-2624	
	Wind Duration: 1.60	WAVE								

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Tray Scab(s)

(2) 2x6x8-2-12 x SP 2400f-2.0E scabs at left end.
Attach one scab to each outer face of chord with:
0.131"x3", min. nails @ 8" oc, Plus additional nail
clusters at: BRG.: (2), heel: (4), 1st panel point: (3).
(2) 2x6x8-0-1 x SP 2400f-2.0E scabs at right end.
Attach one scab to each outer face of chord with:
0.131"x3", min. nails @ 8" oc, plus additional nail
clusters at: BRG.: (2), heel: (4), 1st panel point: (3).

Wind

Wind loads based on MWFRS with additional C&C
member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is
3-6-3.

+ Member to be laterally braced for horizontal
wind load. Bracing system to be designed and
furnished by others.

See DWGS A16015ENC101014 & GBLLETIN0118
for gable wind bracing and other requirements.



COA#0-278
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09/20/2023

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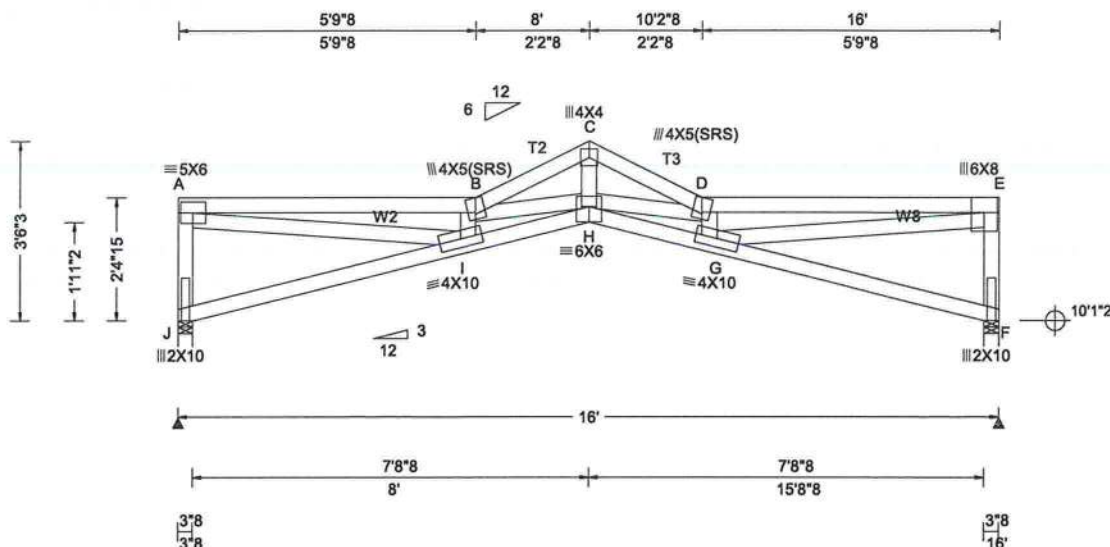
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724155 FROM: CDM	MONO Qty: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: D02	Cust: R 215 JRef: 1XT92150006 T24 DrwNo: 263.23.1528.33813 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.259 H 742 240 VERT(CL): 0.537 H 357 180 HORZ(LL): 0.144 F - - HORZ(TL): 0.298 F - - Creep Factor: 2.0 Max TC CSI: 0.365 Max BC CSI: 0.853 Max Web CSI: 0.979 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 664 /- /- /332 /119 /28 F 664 /- /- /332 /119 /- Wind reactions based on MWFRS J Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings J & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 1863 -2877 C - D 1496 -2363 B - C 1488 -2363 D - E 1846 -2877

Lumber

Top chord: 2x4 SP M-31; T2,T3 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W2,W8 2x4 SP #2;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-6-3.



COA#0-278

Florida Certificate of Product Approval #FL1999
09/20/2023

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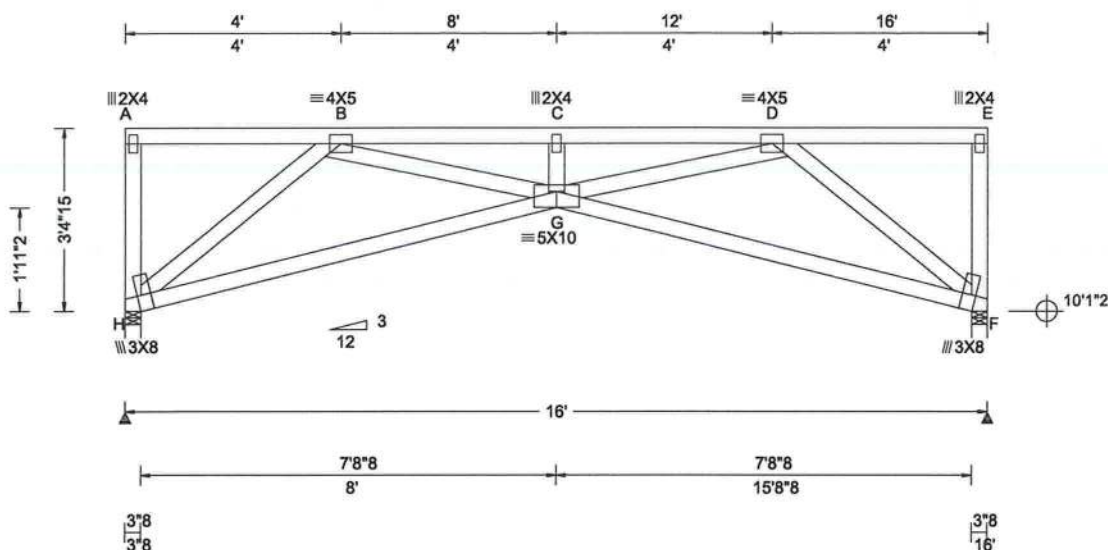
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724157 FROM: CDM	MONO Ply: 1 Qty: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: D03	Cust: R 215 JRef: 1XT92150006 T17 DrwNo: 263.23.1528.35420 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 10.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.099 C 999 240 VERT(CL): 0.200 C 957 180 HORZ(LL): 0.062 F - - HORZ(TL): 0.126 F - - Creep Factor: 2.0 Max TC CSI: 0.286 Max BC CSI: 0.665 Max Web CSI: 0.545 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 645 /- /- /327 /124 /- F 645 /- /- /327 /124 /- Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1224 -2258 C - D 1224 -2258

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

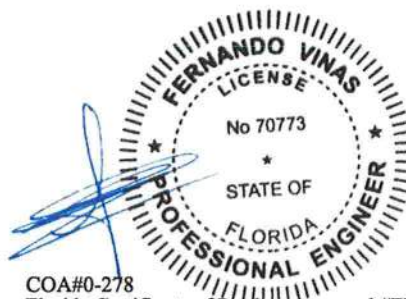
Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Additional Notes

The overall height of this truss excluding overhang is 3-4-15.

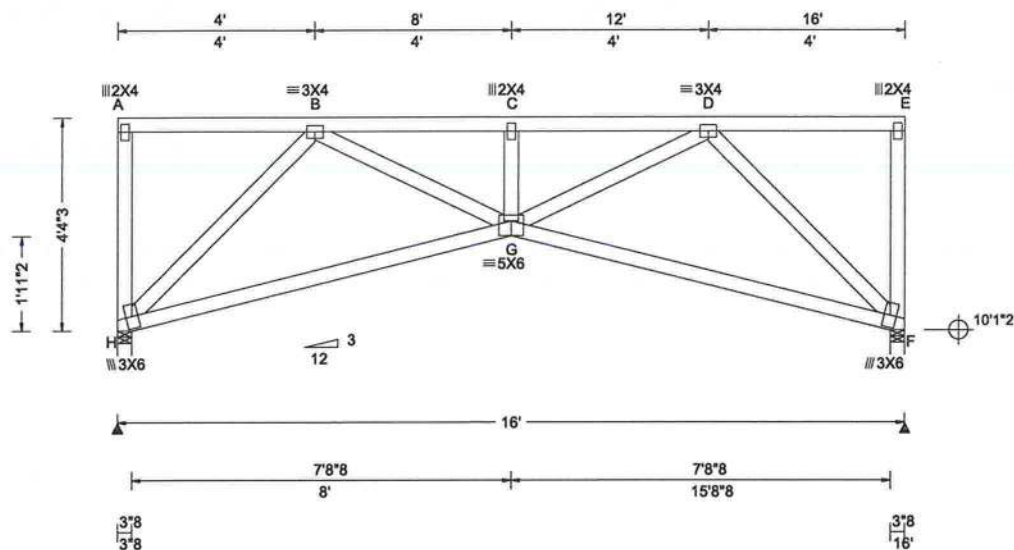


COA#0-278
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09/20/2023

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724159 FROM: CDM	MONO Qty: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: D04	Cust: R 215 JRef: 1XT92150006 T20 DrwNo: 263.23.1528.36743 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 10.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/ VERT(LL): 0.039 C 999 240 VERT(CL): 0.078 C 999 180 HORZ(LL): 0.028 F - - HORZ(TL): 0.056 F - - Creep Factor: 2.0 Max TC CSI: 0.227 Max BC CSI: 0.792 Max Web CSI: 0.542 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 645 /- /- /327 /124 /- F 645 /- /- /327 /124 /- Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 699 -1299 C - D 699 -1299

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Additional Notes

The overall height of this truss excluding overhang is 4-4-3.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
H - G	662 -448	G - F	662 -448

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
H - B	614 -890	G - D	770 -370
B - G	770 -370	D - F	614 -890



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09/20/2023

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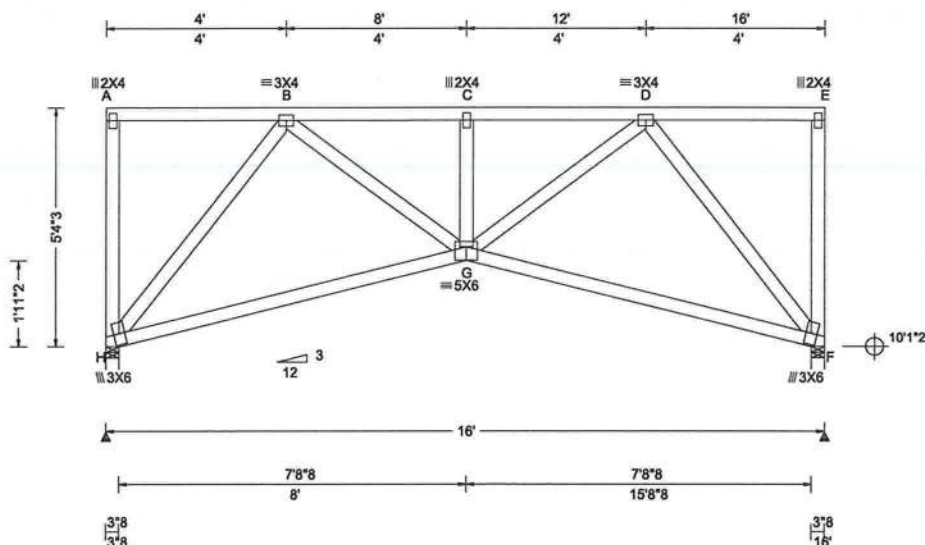
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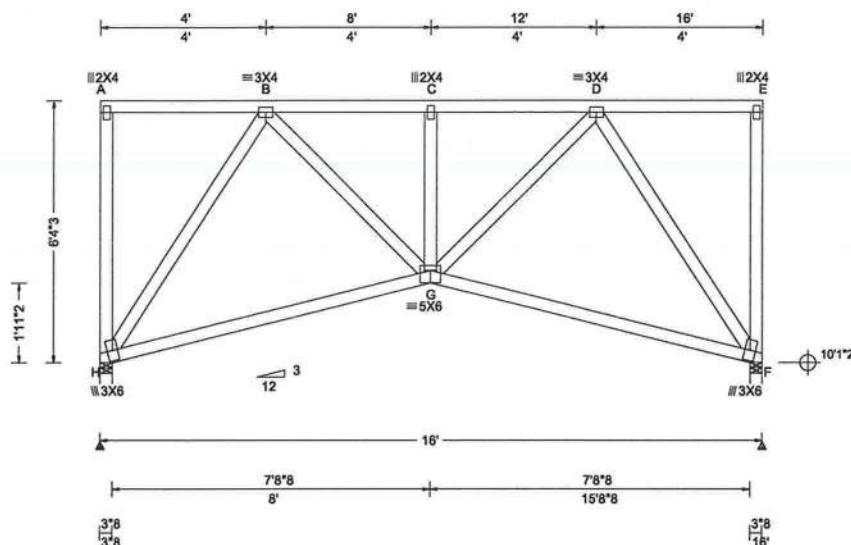
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.023 C 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.046 C 999 180	H 645 /- /- /327 /127 /-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.018 F - -	F 645 /- /- /327 /127 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.036 F - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.44 ft	Building Code:	Creep Factor: 2.0	H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Soffit: 2.00	TCDL: 5.0 psf	IBC 7th Ed. 2020 Res.	Max TC CSI: 0.226	F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.780	Bearings H & F are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.594	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 10.50 ft	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE	VIEW Ver: 22.02.00.0914.12	B - C 488 -889 C - D 488 -889
	Wind Duration: 1.60			

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SEQN: 724163 FROM: CDM	MONO Qty: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: D06	Cust: R 215 JRef: 1XT92150006 T14 DrwNo: 263.23.1528.40730 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)							
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity					
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.017 C 999 240	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.034 C 999 180	H 645	-	-	-	/327	/139	-	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.014 F - -	F 645	-	-	-	/327	/139	-	
	EXP: C Kzt: NA		HORZ(TL): 0.027 F - -	Wind reactions based on MWFRS							
Des Ld: 40.00	Mean Height: 16.44 ft		Creep Factor: 2.0	H Brg Wid = 3.5 Min Req = 1.5 (Truss)							
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Max TC CSI: 0.227	F Brg Wid = 3.5 Min Req = 1.5 (Truss)							
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max BC CSI: 0.770	Bearings H & F are a rigid surface.							
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max Web CSI: 0.684	Members not listed have forces less than 375#							
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes		Maximum Top Chord Forces Per Ply (lbs)							
	Loc. from endwall: not in 21.00 ft	FT/RT: 20(0)/10(0)		Chords		Tens.Comp.		Chords		Tens. Comp.	
	GCpi: 0.18	Plate Type(s):		B - C		387 -674		C - D		387 -674	
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.00.0914.12								

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Additional Notes

The overall height of this truss excluding overhang is 6-4-3.

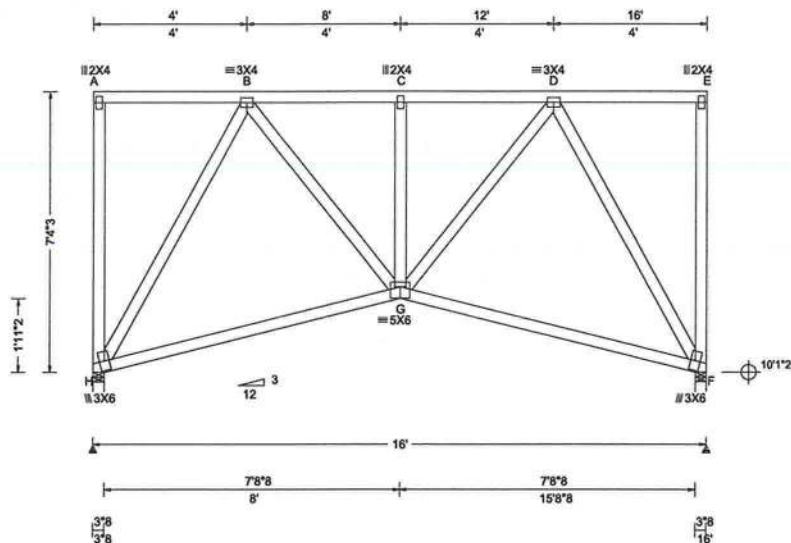


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SEQN: 724165 FROM: CDM	MONO Qty: 1	Ply: 1 Job Number: 23-9993 Kyle McLeod- Office Truss Label: D07	Cust: R 215 JRef: 1XT92150006 T13 DrwNo: 263.23.1528.49073 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.44 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 12.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.014 C 999 240 VERT(CL): 0.029 C 999 180 HORZ(LL): 0.011 F - - HORZ(TL): 0.023 F - - Creep Factor: 2.0 Max TC CSI: 0.228 Max BC CSI: 0.762 Max Web CSI: 0.802 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 645 /- /- /327 /150 /- F 645 /- /- /327 /150 /- Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 325 -543 C - D 325 -543

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Additional Notes

The overall height of this truss excluding overhang is 7-4-3.

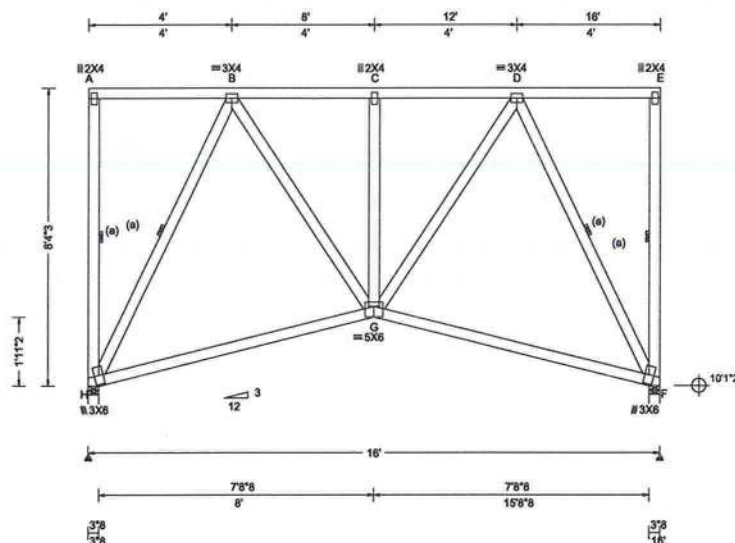


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SEQN: 724167 FROM: CDM	MONO Qty: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: D08	Cust: R 215 JRef: 1XT92150006 T16 DrwNo: 263.23.1529.13467 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.44 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 25.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.013 C 999 240 VERT(CL): 0.027 C 999 180 HORZ(LL): 0.010 F - - HORZ(TL): 0.020 F - - Creep Factor: 2.0 Max TC CSI: 0.229 Max BC CSI: 0.756 Max Web CSI: 0.263 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 645 /- /- /327 /161 /- F 645 /- /- /327 /161 /- Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 282 -455 C - D 282 -455

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Additional Notes

The overall height of this truss excluding overhang is 8-4-3.

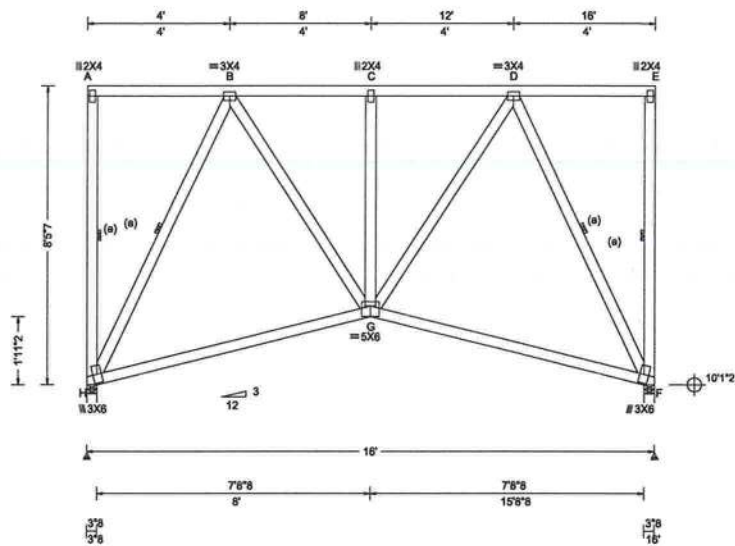


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SEQN: 724169 FROM: CDM	MONO Qty: 1	Ply: 1 Job Number: 23-9993 Kyle McLeod- Office Truss Label: D09	Cust: R 215 JRef: 1XT92150006 T25 DrwNo: 263.23.1529.28053 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.54 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 25.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.013 C 999 240 VERT(CL): 0.027 C 999 180 HORZ(LL): 0.010 F - - HORZ(TL): 0.020 F - - Creep Factor: 2.0 Max TC CSI: 0.229 Max BC CSI: 0.756 Max Web CSI: 0.267 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 645 -/- /- /327 /162 -/ F 645 -/- /- /327 /162 -/ Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 278 -447 C - D 278 -447

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Additional Notes

The overall height of this truss excluding overhang is 8'-5-7.



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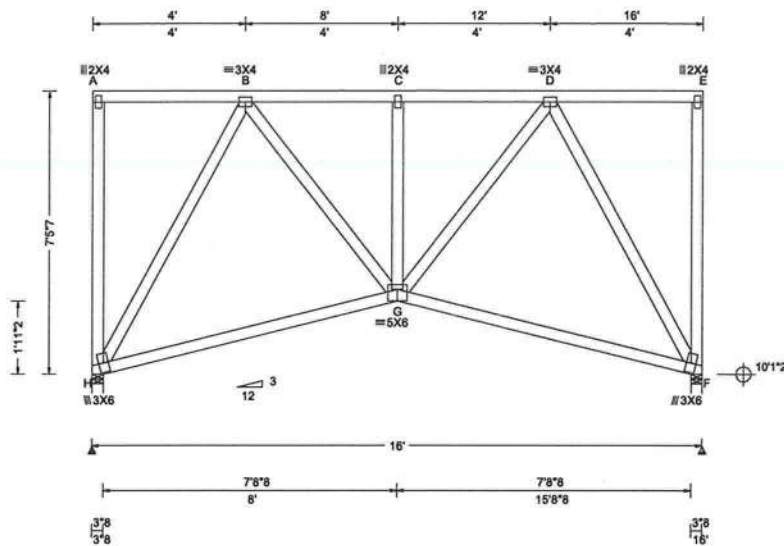
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SEQN: 724171 FROM: CDM	MONO Qty: 1	Ply: 1 Job Number: 23-9993 Kyle McLeod- Office Truss Label: D10	Cust: R 215 JRef: 1XT92150006 T26 DrwNo: 263.23.1529.38977 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.54 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 12.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.014 C 999 240 VERT(CL): 0.029 C 999 180 HORZ(LL): 0.011 F - - HORZ(TL): 0.022 F - - Creep Factor: 2.0 Max TC CSI: 0.228 Max BC CSI: 0.761 Max Web CSI: 0.816 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 645 /- /- /327 /151 /- F 645 /- /- /327 /151 /- Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 319 -532 C - D 319 -532

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.

Additional Notes

The overall height of this truss excluding overhang is 7'-5 7/8".

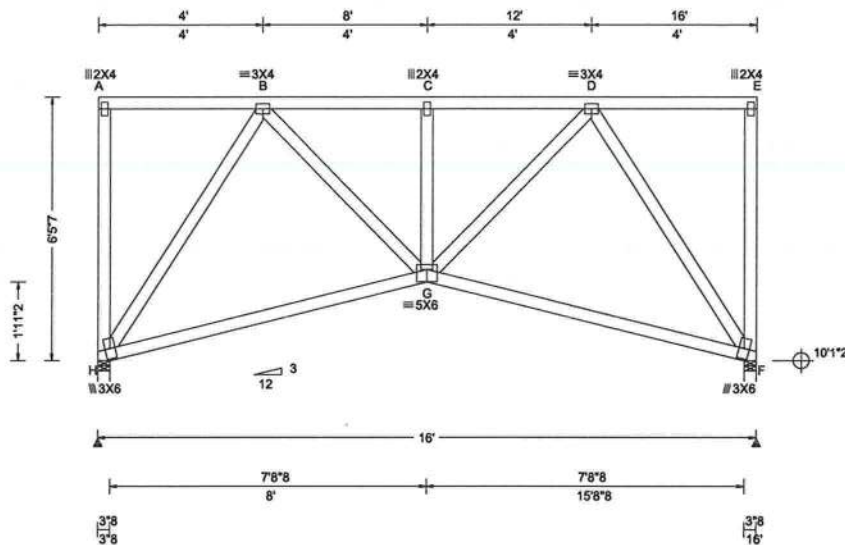


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Glenview, IL 60025

SEQN: 724173 FROM: CDM	MONO Qty: 1	Ply: 1 Job Number: 23-9993 Kyle McLeod- Office Truss Label: D11	Cust: R 215 JRef: 1XT92150006 T27 DrwNo: 263.23.1529.55760 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)							
				Gravity			Non-Gravity				
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.017 C 999 240	H 645	-/-	-/-	-/-	/327	/140	-/-	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.034 C 999 180	F 645	-/-	-/-	-/-	/327	/140	-/-	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.013 F - -	Wind reactions based on MWFRS							
Des Ld: 40.00	EXP: C Kzt: NA	Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.027 F - -	H Brg Wid = 3.5 Min Req = 1.5 (Truss)							
NCBCLL: 10.00	Mean Height: 16.54 ft		Creep Factor: 2.0	F Brg Wid = 3.5 Min Req = 1.5 (Truss)							
Soffit: 2.00	TCDL: 5.0 psf		Max TC CSI: 0.227	Bearings H & F are a rigid surface.							
Load Duration: 1.25	BCDL: 5.0 psf		Max BC CSI: 0.769	Members not listed have forces less than 375#							
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h		Max Web CSI: 0.695	Maximum Top Chord Forces Per Ply (lbs)							
	C&C Dist a: 3.00 ft			Chords		Tens.Comp.		Chords		Tens. Comp.	
	Loc. from endwall: not in 21.00 ft			B - C		379 -658		C - D		379 -658	
	GCpi: 0.18										
	Wind Duration: 1.60		VIEW Ver: 22.02.00.0914.12								

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

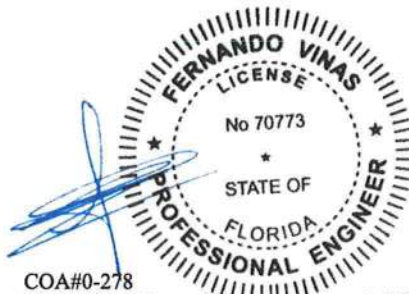
Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Additional Notes

The overall height of this truss excluding overhang is 6-5-7.



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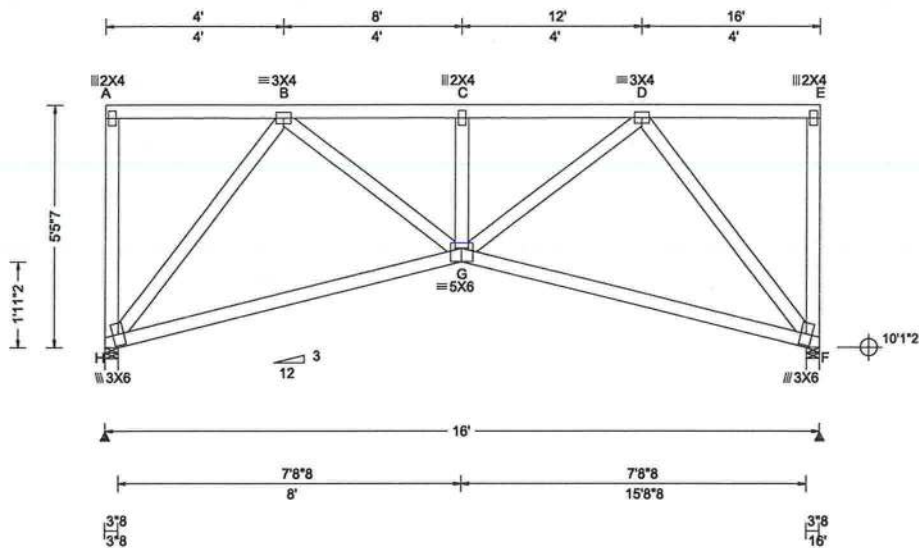
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724177 FROM: CDM	MONO Qty: 1	Ply: 1 Job Number: 23-9993 Kyle McLeod- Office Truss Label: D12	Cust: R 215 JRef: 1XT92150006 T28 DrwNo: 263.23.1530.03890 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.54 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 10.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.022 C 999 240 VERT(CL): 0.044 C 999 180 HORZ(LL): 0.017 F - - HORZ(TL): 0.035 F - - Creep Factor: 2.0 Max TC CSI: 0.226 Max BC CSI: 0.779 Max Web CSI: 0.602 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 645 -/- /- /327 /128 -/ F 645 -/- /- /327 /128 -/ Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 475 -860 C - D 475 -860

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.

Additional Notes

The overall height of this truss excluding overhang is 5-5-7.

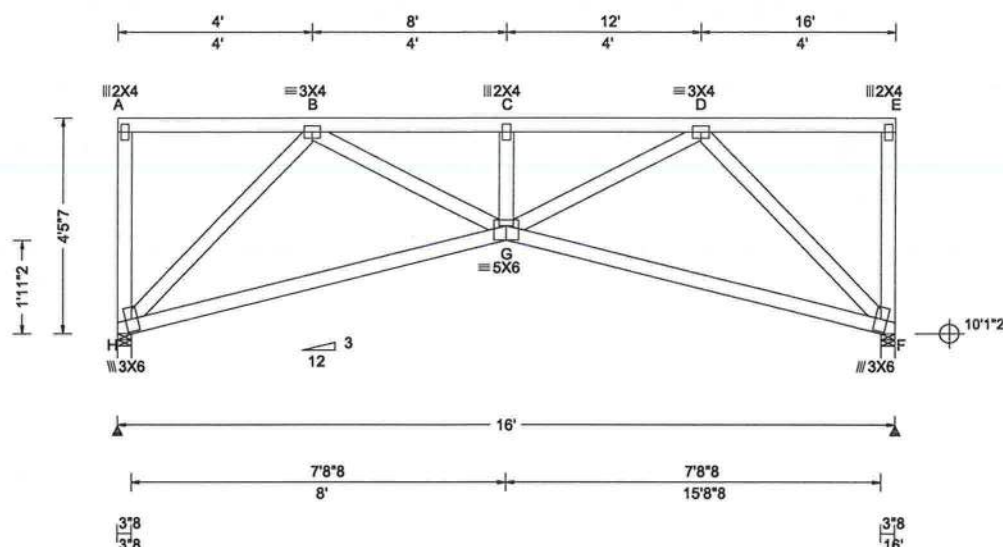


COA#0-278
Florida Certificate of Product Approval #FL1999
09/20/2023

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724179 FROM: CDM	MONO Qty: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: D13	Cust: R 215 JRef: 1XT92150006 T29 DrwNo: 263.23.1530.34187 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 10.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.036 C 999 240 VERT(CL): 0.073 C 999 180 HORZ(LL): 0.026 F - - HORZ(TL): 0.053 F - - Creep Factor: 2.0 Max TC CSI: 0.227 Max BC CSI: 0.791 Max Web CSI: 0.545 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 645 - / - / 327 / 124 - F 645 - / - / 327 / 124 - Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 667 -1240 C - D 667 -1240

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Additional Notes

The overall height of this truss excluding overhang is 4-5-7.



COA#0-278

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09/20/2023

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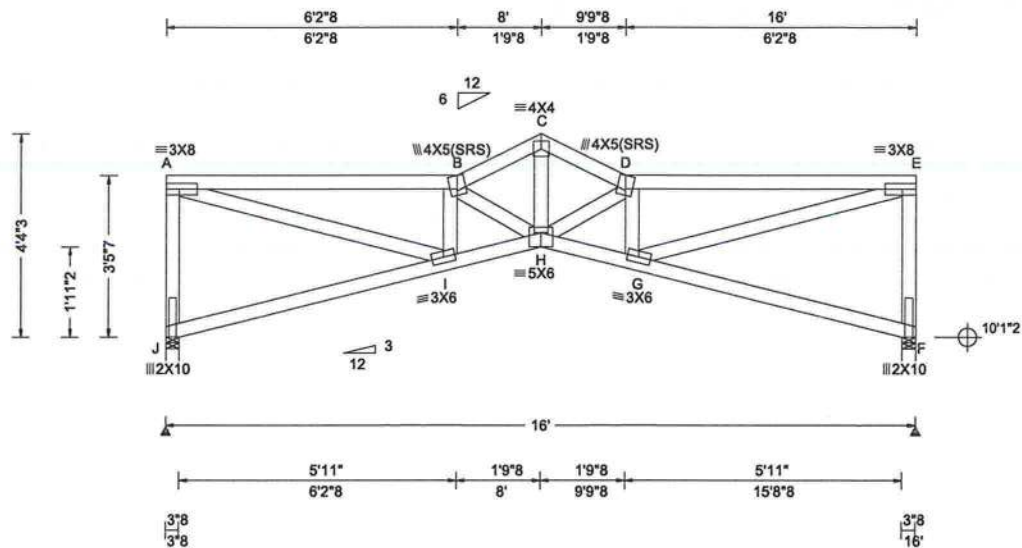
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.083 H 999 240	Loc	R+	/R-	/Rh	/Rw	/U	/RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.172 H 999 180	J	664	/-	/-	/333	/95	/22
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.047 F - -	F	664	/-	/-	/333	/95	/-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.098 F - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	J	Brg	Wid = 3.5	Min	Req = 1.5	(Truss)	
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.709	F	Brg	Wid = 3.5	Min	Req = 1.5	(Truss)	
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.457	Bearings J & F are a rigid surface.						
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.743	Members not listed have forces less than 375#						
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)						
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Chords	Tens.	Comp.	Chords	Tens.	Comp.	
	GCpi: 0.18		VIEW Ver: 22.02.00.0914.12	A - B	1013	- 1500	C - D	991	- 1450	
	Wind Duration: 1.60	WAVE		B - C	985	- 1450	D - E	1012	- 1500	

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-4-3.



COA#0-278

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09/20/2023

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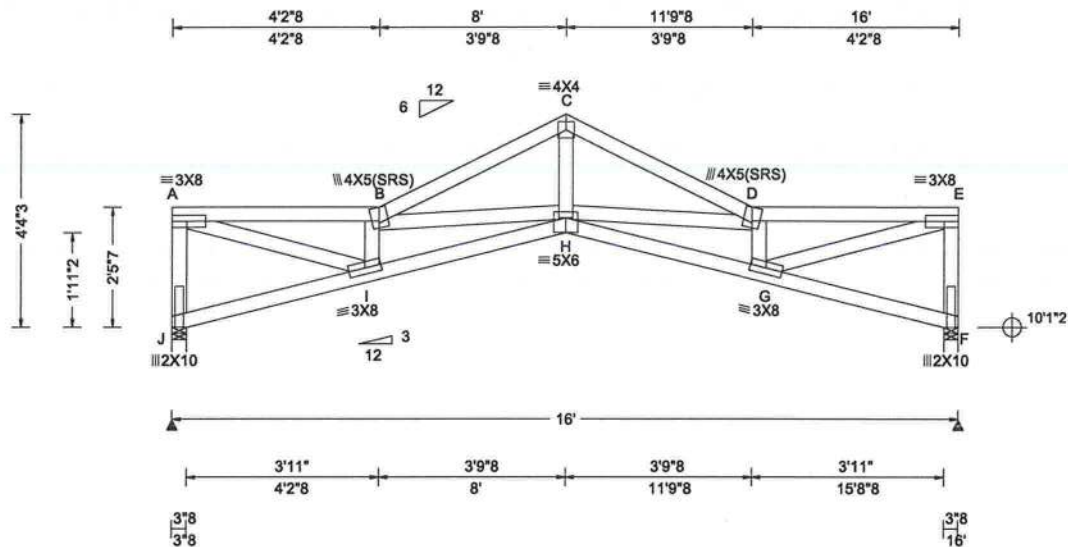
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724142 FROM: CDM	MONO Qty: 1	Job Number: 23-9993 Kyle McLeod- Office Truss Label: D15	Cust: R 215 JRef: 1XT92150006 T23 DrwNo: 263.23.1530.45357 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.086 H 999 240 VERT(CL): 0.179 H 999 180 HORZ(LL): 0.056 F - - HORZ(TL): 0.115 F - - Creep Factor: 2.0 Max TC CSI: 0.352 Max BC CSI: 0.479 Max Web CSI: 0.633 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 664 /- /- /336 /117 /48 F 664 /- /- /336 /117 /- Wind reactions based on MWFRS J Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings J & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 883 -1597 C - D 778 -1505 B - C 766 -1505 D - E 868 -1597 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. I - H 1845 -1063 H - G 1845 -1026 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - J 426 -620 H - D 385 -476 A - I 1661 -915 D - G 590 -891 I - B 599 -891 G - E 1661 -900 B - H 383 -476 E - F 423 -620 C - H 1046 -522

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-4-3.



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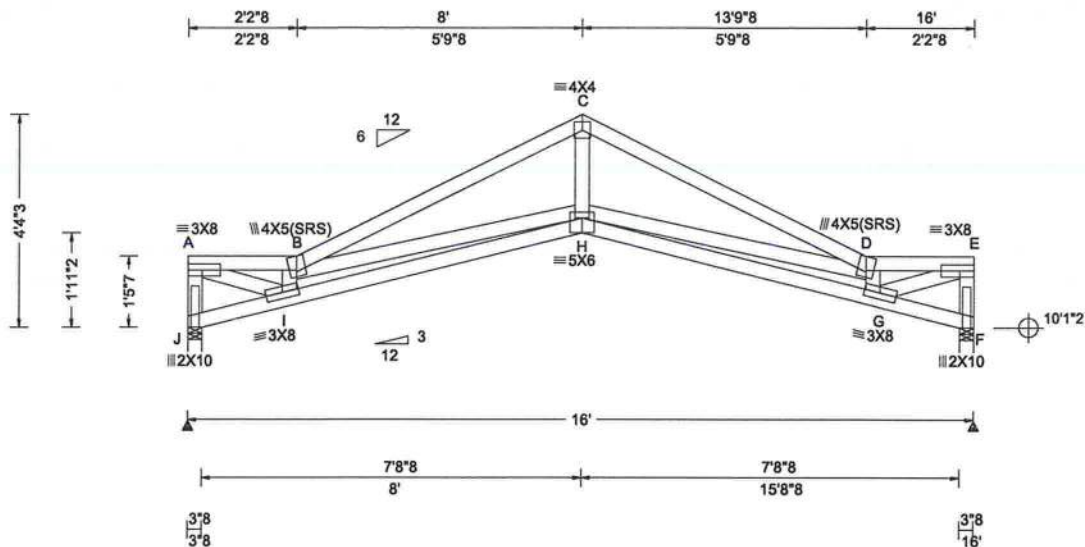
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 724140 FROM: CDM	MONO Qty: 1	Ply: 1 Job Number: 23-9993 Kyle McLeod- Office Truss Label: D16	Cust: R 215 JRef: 1XT92150006 T22 DrwNo: 263.23.1530.56710 AK / FV 09/20/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.086 H 999 240 VERT(CL): 0.178 H 999 180 HORZ(LL): 0.062 F - - HORZ(TL): 0.130 F - - Creep Factor: 2.0 Max TC CSI: 0.420 Max BC CSI: 0.613 Max Web CSI: 0.619 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 664 /- /- /351 /115 /74 F 664 /- /- /351 /115 /- Wind reactions based on MWFRS J Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings J & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 666 -1526 C - D 655 -1624 B - C 640 -1624 D - E 656 -1526

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-4-3/8.



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
I - H	1955 -915	H - G	1955 -882

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - J	306 -623	H - D	375 -490
A - I	1626 -708	D - G	576 -1063
I - B	585 -1063	G - E	1626 -697
B - H	371 -490	E - F	304 -623
C - H	971 -289		

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, $K_{zt} = 1.00$

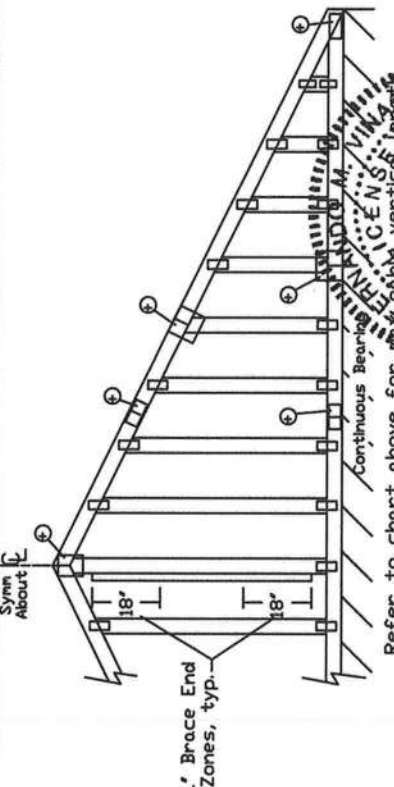
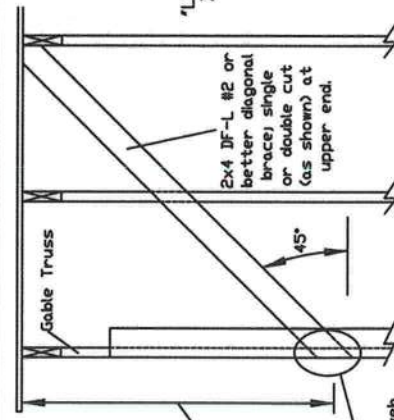
Dr1 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Dr1 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
 Dr1 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

2x4 Gable Vertical Spacing		Brace		No Braces	(1) 1x4 "L" Brace ■ (2) 2x4 "L" Brace ■■ (3) 2x6 "L" Brace ■■■ (4) 2x6 "L" Brace ■■■■ (5) 2x6 "L" Brace ■■■■ (6) 2x6 "L" Brace ■■■■ (7) 2x6 "L" Brace ■■■■ (8) 2x6 "L" Brace ■■■■ (9) 2x6 "L" Brace ■■■■ (10) 2x6 "L" Brace ■■■■															
		Species	Grade		Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
Max Gable Vertical Length	12" o.c.	SPF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			Stud	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		HF	Standard	4' 1"	6' 0"	6' 4"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 3"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	16" o.c.	SP	#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	12" o.c.	SPF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		HF	Stud	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SP	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL	#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" o.c.	SPF	Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	HF	#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" o.c.	SP	Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	

Diagonal brace options: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450' at each end. Max web total length is 14'.

Vertical length shown
in table above.

Connect diagonal at midpoint of vertical web



Refer to chart above for maximum vertical length

==WARNING== READ AND FOLLOW ALL NOTES ON THIS DRAWING
==IMPORTANT== FURNISH THE DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BESS Guiding Component Safety Information, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BESS. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chords shall have permanent lateral bracing. Trusses are designed as a minimum for permanent lateral restraint of webs under full design loading conditions. BESS sections B-7 through B-10 specify additional requirements for truss installation. BESS sections B-11 through B-16 specify additional requirements for each type of truss and position as shown above on the sheet details. Unless noted otherwise, all dimensions shall have been inspected and approved by the manufacturer.

Refer to drawings 160A-Z for standard plate positions.

Alpha, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:

For more information see this job's general notes page and these Web sites:
ALPINE: www.alpineitw.com TPI: www.tpiest.com SRC: www.scr.com

Group A:			
Spruce-Pine-Fir #1 / #2 Standard #3 Stud	Hem-Fir #2 Stud #3 Standard	Douglas Fir-Larch #3 Stud Standard	Southern Pine #3 Stud Standard

Group B:	
Hem-Fir #1 & 3tr #1	Southern Pine #1 Stud #2 Standard

1x4 Braces shall be SRB (Stress-Rated Board).
 For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Cable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

* For (1) "L" brace: space nails at 2' o.c.

in 18" end zones and 4" o.c. between zones.
**For (2) "L" braces: space nalls at 3" o.c.

in 18" end zones and 6" o.c. between zones.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

REF	ASCE7-16-GABI4015
DATE	01/26/2018
DRWG	A14015ENC160118

MAX. TOT. LD. 60 PSF
09/20/2023

MAX. SPACING 24.0"

COA#0-278

CLR Reinforcing

Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

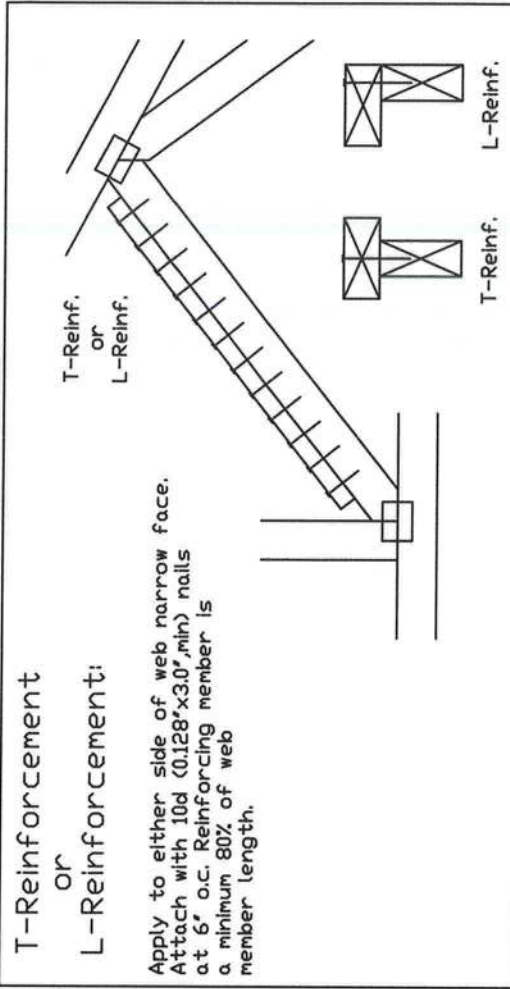
Notes:

- This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.
- Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.
- Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(Ø)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(Ø)

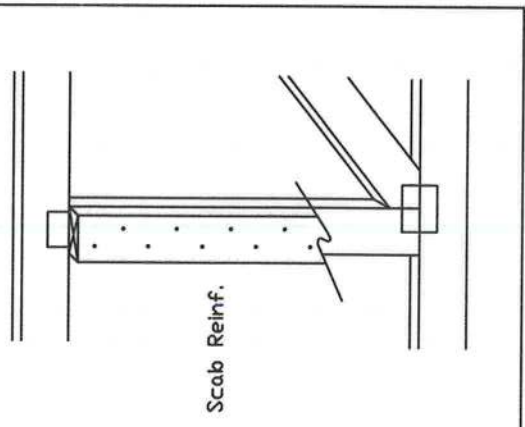
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

Ø Center scab on wide face of web. Apply (1) scab to each face of web.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Guiding Component Safety Information, by TPI and SCSA for safety instructions to properly perform these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, all trusses shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply bracing to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings 160A-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineer. The authority and use of this design shall remain the responsibility of the professional engineer. The liability of the professional engineer for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec2.

For more information, see this job's general notes page and these web sites:
ALPINE: www.alphinet.com TPI: www.tpi.org SCSA: www.scsa.com ICD: www.icdusa.com

REINFORCEMENT M. VINAS

No 70773

STATE OF FLORIDA

PROFESSIONAL ENGINEER

COA#0-27X

TC LL

TC DL

BC DL

BC LL

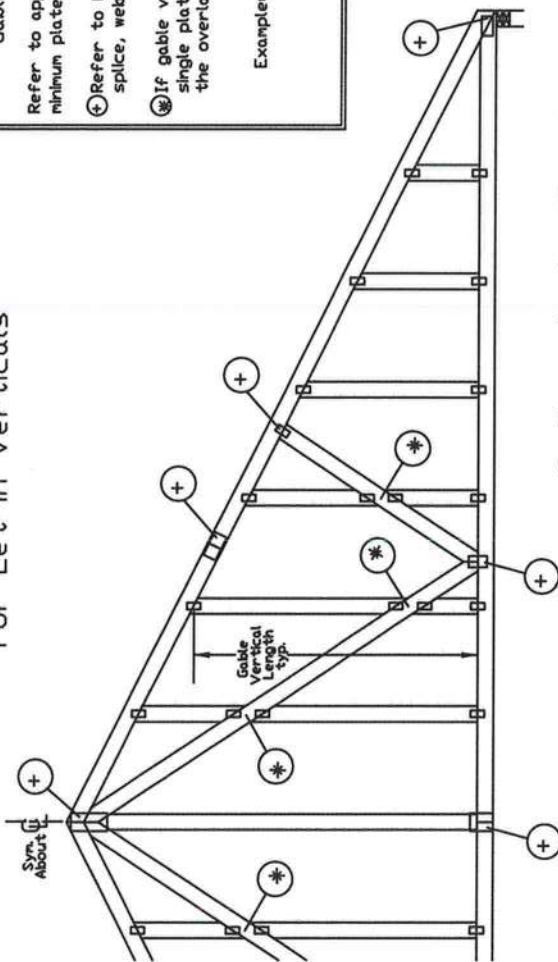
TOT. LD.

10/18/2019

SPACING

PSF	REF	CLR Subst.
PSF	DATE	01/02/19
PSF	DRWG	BRCLBSUB0119
PSF		
PSF		

Gable Detail For Let-in Verticals



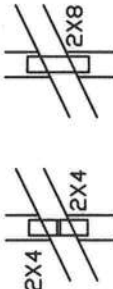
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



'T' Reinforcement Attachment Detail

'T' Reinforcing Member

'T' Reinforcing Member



Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:
10d Common (0.148" x 3", min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

Toenail Nails:

10d Common (0.148" x 3", min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

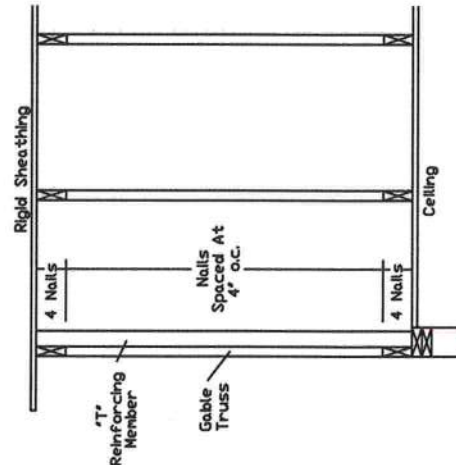
ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A22015ENC100118, A24015ENC100118, A26015ENC100118, A28015ENC100118, A30015ENC100118, A32015ENC100118, A34015ENC100118, A36015ENC100118, A38015ENC100118, A40015ENC100118, A42015ENC100118, A44015ENC100118, A46015ENC100118, A48015ENC100118, A50015ENC100118, A52015ENC100118, A54015ENC100118, A56015ENC100118, A58015ENC100118, A60015ENC100118, A62015ENC100118, A64015ENC100118, A66015ENC100118, A68015ENC100118, A70015ENC100118, A72015ENC100118, A74015ENC100118, A76015ENC100118, A78015ENC100118, A80015ENC100118, A82015ENC100118, A84015ENC100118, A86015ENC100118, A88015ENC100118, A90015ENC100118, A92015ENC100118, A94015ENC100118, A96015ENC100118, A98015ENC100118, A100015ENC100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (From Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length

1.30 x 8' 7" = 11' 2"

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING.

INSTALLERS MUST FURNISH TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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Refer to drawings 160A-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, or bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The authority and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpinealpine.com TPI: www.tpi.com SBCA: www.sbcasolutions.com ITW: www.itw.com

REF LET-IN VERT
DATE 01/02/2018
DRWG GBLLETTIN0118

MAX. TOT. LD. 60 PSF
DUE TO E/W.C. ANY
MAX. SPACING 24.0'

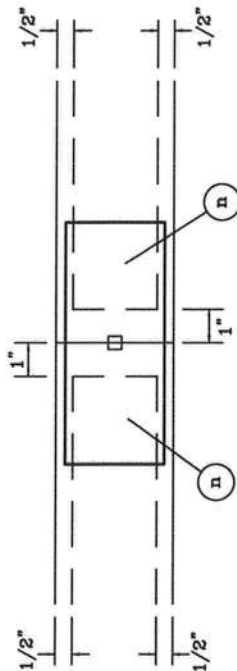


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AN ITW COMPANY

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Glenview, IL 60025

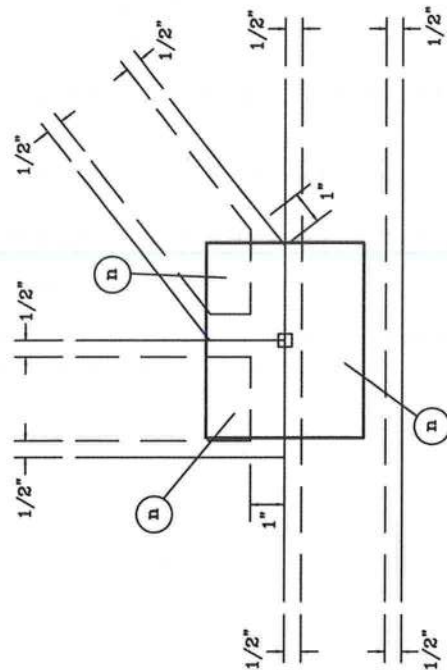
TRULOX INFORMATION DETAIL

TYPICAL OFF PANEL SPLICE



DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.
NAILS MUST NOT SPLIT LUMBER.

TYPICAL PANEL POINT WITHOUT SPLICE



TYPICAL FILLER

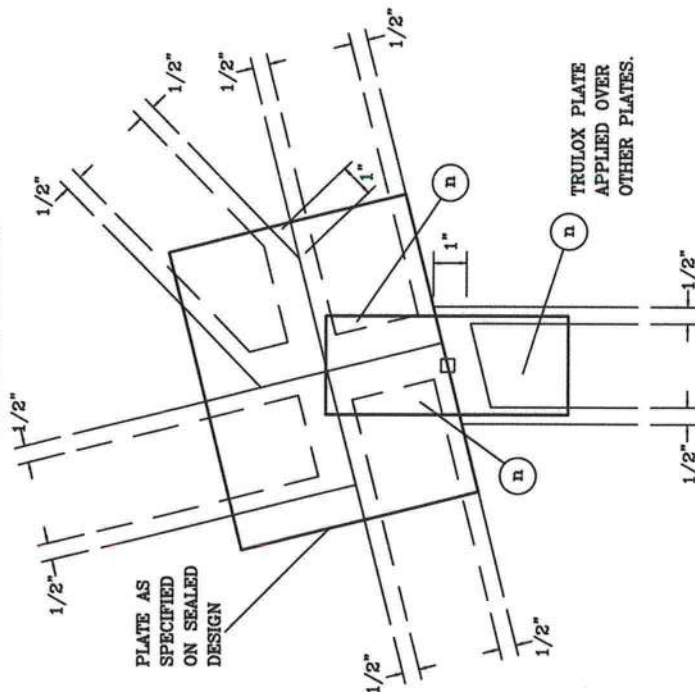
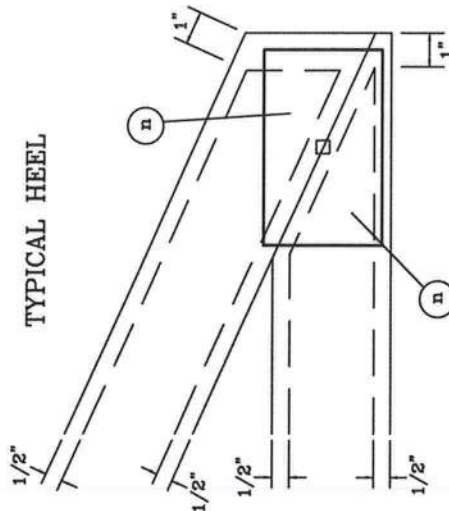


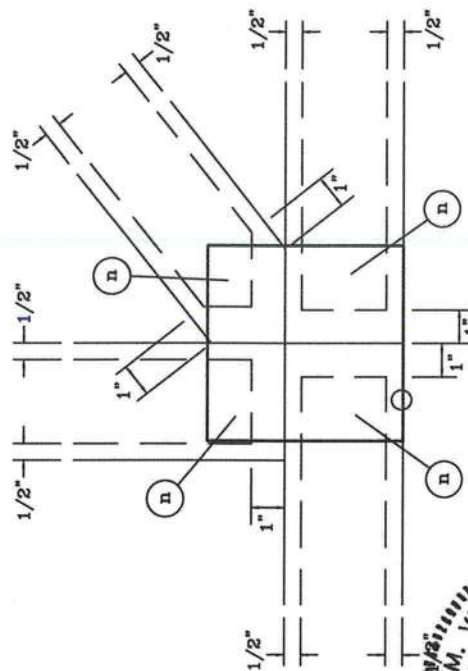
PLATE AS SPECIFIED ON SEALED DESIGN

TRULOX PLATE APPLIED OVER OTHER PLATES.

TYPICAL HEEL



TYPICAL PANEL POINT SPLICE



NOTES:

(a) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.

○ LOCATES PLATE CORNER OR FLUSH EDGE.
□ LOCATES PLATE CENTER.



TRULOX PLATING

160
TL

PAGE 1 OF 1
DATE 10/01/14

