DATE 01/3	0/2008			uilding Permit on Premises During Co		PERMIT
A DDI TO A NET	DDENID (e Prominently Posted			000026687
APPLICANT	BRENDA			PHONE	755-8699	. FI 22056
ADDRESS OWNER	MIKE RO	PO BOX 3535	*	LAKE CITY PHONE	755 0476	FL 32056
ADDRESS		SW CHESTERFIELI	D CIRCLE	LAKE CITY	755-9476	FL 32024
CONTRACTO	309 WII	LIAM WOOD	DCIRCLE	PHONE	755-8699	FL 32024
LOCATION O			242 P APPOWHEAT	D, FOLLOW ONTO CAN		D
LOCATION O	FROFER	-		EN 4TH LOT ON LEFT	NON CREEK D	K,
TYPE DEVEL	OPMENT	SFD,UTILITY		TIMATED COST OF CO	ONSTRUCTION	106850.00
HEATED FLO	OR AREA	1495.00	TOTAL ARI	EA2137.00	HEIGHT 1	8.20 STORIES <u>1</u>
FOUNDATIO	CONC	CRETE WALL	S FRAMED F	ROOF PITCH 6/12	FL	LOOR SLAB
LAND USE &	ZONING	RSF-2		MAX	K. HEIGHT	35
Minimum Set I	Back Requir	ments: STREET-F	FRONT 25.00	REAR	15.00	SIDE 10.00
NO. EX.D.U.	0	FLOOD ZONE	XPP	DEVELOPMENT PER	MIT NO.	
PARCEL ID	24-4S-16-	03117-118	SUBDIVISIO	N CROSSWINDS		
LOT 18	BLOCK	PHASE _	UNIT		AL ACRES0.	.52
000001537			CBC058182	Kno	11/10	2 1000
Culvert Permit	No.	Culvert Waiver Co	ontractor's License Nur	nber	Applicant/Owner	/Contractor
PERMIT		07-1004	BK		JH	И
Driveway Conr	nection	Septic Tank Number	LU & Zoni	ng checked by Ap	proved for Issuance	ce New Resident
COMMENTS:	FLOOR C	NE FOOT ABOVE TH	HE ROAD, NOC ON F	II F		
				IDL		
					Check # or C	ash 1254
			ILDING & ZONIN			
Temporary Pov	ver		933 0 703	NG DEPARTMENT	ONLY	fash 1254 (footer/Slab)
Temporary Pov	ver		933 0 703		ONLY	
Temporary Pov	100-000	FOR BU date/app. by sing	Foundation Slab _	IG DEPARTMENT	ONLY Monolithic	(footer/Slab)
Under slab rou	gh-in plumb	FOR BU date/app. by ing date/app	Foundation Slab _	NG DEPARTMENT	ONLY Monolithic	(footer/Slab) date/app. by
	gh-in plumb	FOR BU date/app. by ing date/app	Foundation Slab o. by	NG DEPARTMENT date/app. by	Monolithic Sheathing/	(footer/Slab) date/app. by /Nailing date/app. by
Under slab rou	gh-in plumb date/ap	FOR BU date/app. by ing date/app	Foundation Slab o. by Rough-in plumbing al	date/app. by	Monolithic Sheathing/	(footer/Slab) date/app. by /Nailing date/app. by date/app. by
Under slab roug	gh-in plumb date/ap	FOR BU date/app. by ing date/app	Foundation Slab o. by	date/app. by	Monolithic Sheathing/	(footer/Slab) date/app. by /Nailing date/app. by date/app. by
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Under slab roug Framing Electrical roug Permanent pow	date/aph-in date/aph-in date/ap	date/app. by date/app date/app date/app te/app. by	Foundation Slab D. by Rough-in plumbing al Heat & Air Duct C.O. Final	date/app. by date/app. by date/app. by	Monolithic Sheathing/ d floor Peri. beam (Linter Culvert	(footer/Slab) date/app. by /Nailing date/app. by date/app. by
Under slab roug Framing Electrical roug Permanent pow	date/aph-in date/aph-in date/ap	date/app. by ing date/app date/app p. by date/app. by	Foundation Slab D. by Rough-in plumbing al Heat & Air Duct C.O. Final	date/app. by date/app. by ove slab and below woo date/app. by	Monolithic Sheathing/ d floor Peri. beam (Linte	(footer/Slab) date/app. by /Nailing date/app. by date/app. by el) date/app. by date/app. by
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Under slab roug Framing Electrical roug Permanent pow M/H tie downs, Reconnection M/H Pole	date/aph-in date/aph-in da	date/app. by date/app p. by date/app. by te/app. by lectricity and plumbing	Foundation Slab D. by Rough-in plumbing al Heat & Air Duct C.O. Final date/app Pump pole date //el Trailer	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by Utility Po	Monolithic Sheathing/ d floor Peri. beam (Linte Culvert Pool date/app. by	(footer/Slab) date/app. by /Nailing date/app. by date/app. by date/app. by date/app. by date/app. by
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Under slab roug Framing Electrical roug Permanent pow M/H tie downs, Reconnection M/H Pole dat	date/aph-in plumb date/aph-in er da blocking, el de/app. by	date/app. by ing date/app date/app p. by date/app. by te/app. by lectricity and plumbing date/app. by Trav	Foundation Slab Slab Do. by Rough-in plumbing al Heat & Air Duct C.O. Final date/app Pump pole date //el Trailer	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by Utility Po /app. by ate/app. by E \$ 10.69	Monolithic Sheathing/ d floor Peri. beam (Linte Culvert Pool ole date/app. by Re-roof	(footer/Slab) date/app. by /Nailing
Under slab roug Framing Electrical roug Permanent pow M/H tie downs, Reconnection M/H Pole dat BUILDING PE MISC. FEES \$	date/aph-in plumb date/aph-in er da blocking, el de/app. by RMIT FEE 0.00 LOPMENT	date/app. by ing date/app date/app p. by date/app. by te/app. by lectricity and plumbing date/app. by Trav \$ 535.00 ZONING O	Foundation Slab D. by Rough-in plumbing al Heat & Air Duct C.O. Final date/app Pump pole date CERTIFICATION FE CERT. FEE \$ 50.00 DD ZONE FEE \$ 25.0	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by Utility Po /app. by ate/app. by E \$ 10.69	Monolithic Sheathing/ d floor Peri. beam (Linte Culvert Pool date/app. by Re-roof SURCHARGE WAST	(footer/Slab) date/app. by /Nailing

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

1254

Columbia County Building Permit Application

For Office Use Only Application # 0712-76 Date Received 12 26 07 By Permit # 15 37/ 26687
Zoning Official Date 03.01.08 Flood Zone FEMA Map # Zoning RSF-2
Land Use KES Low Delevation N/A MFE about River N/A Plans Examiner OF JTH Date 1-8-08
Comments
NOC (EH □ Deed or PA Site Plan □ State Road Info □ Parent Parcel # □ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Septic Permit No
Name Authorized Person Signing Permit Brenda Terry Phone 755-8699
Address VO Box 3535 LAKE City, FL 32056
Owners Name Mike Roberts Phone 755-9476
911 Address 309 SW Chasterfield CIR LAKE City, Fl320
Contractors Name William Wood Phone 755 - 8699
Address POBOX 3535 LAKE City FL' 32056
Fee Simple Owner Name & Address 1/2
Bonding Co. Name & Address 1
Architect/Engineer Name & Address Mark Disobway PE
Mortgage Lenders Name & Address / / / / /
Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy
Property ID Number 24-45-16-03117-118 Estimated Cost of Construction 120,000
Subdivision Name Cross Winds Lot 18 Block Unit Phase 1
Driving Directions SR 47 South to CR 242 turn Right
So to Arrowhead Rd turn Right Goto CANNON Creek
)R. turn into Chesterfield Circle turn left 4th 10 ton left
Construction of Single Fumily Dwelling Total Acreage
Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 18 2
Actual Distance of Structure from Property Lines - Front_ 36 Side, 36 _ Side, 36 _ Side, 20 _ Rear_150 _
Number of Stories Heated Floor Area 1495 Total Floor Area 2137 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Page 1 of 2 (Both Pages must be submitted together.)

Revised 11-30-07

IN SCROKE WITH BRENDA 1.9.08

Columbia County Building Permit Application

<u>WARNING TO OWNER:</u> YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

<u>YOU ARE HEREBY NOTIFIED</u> as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's License Number

Competency Card Number

Competency Card Number

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 2b day of December 20 0.7

Personally known or Produced Identification

SEAL:

Brenda Terry

State of Florida Notary Signature (For the Contractor)

My Commission DD293888 Expires February 24, 2008

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001537

DATE 01/30	0/2008 PARCEL ID #	24-4S-16-03117-118	
APPLICANT	BRENDA TERRY	PHONE 755-8	599
ADDRESS _	PO BOX 3535	LAKE CITY	FL 32056
OWNER MI	KE ROBERTS	PHONE 755-94	76
ADDRESS 30	99 SW CHESTERFIELD CIRCLE	LAKE CITY	FL 32024
CONTRACTO	R WILLIAM WOOD	PHONE 755-86	599
LOCATION OF	F PROPERTY SR 47 S, R 242, R ARROW	HEAD, FOLLOW ONTO CANNON	CREEK DR,
L CHESTERFIELD	O CIRCLE, THEN 4TH LOT ON LEFT		
SUBDIVISION	LOT/BLOCK/PHASE/UNIT CROSSW	INDS	18
SIGNATURE	Brenda Le	us	
	INSTALLATION REQUIREMENT	rs O	
X	Culvert size will be 18 inches in diamet driving surface. Both ends will be miter thick reinforced concrete slab.	— er with a total lenght of 32 feet,	leaving 24 feet of poured with a 4 inch
	INSTALLATION NOTE: Turnouts will I a) a majority of the current and existi b) the driveway to be served will be p Turnouts shall be concrete or pave concrete or paved driveway, whiche current and existing paved or concr	ng driveway turnouts are paved aved or formed with concrete. d a minimum of 12 feet wide or ever is greater. The width shall	the width of the
	Culvert installation shall conform to the	e approved site plan standards.	
	Department of Transportation Permit i	nstallation approved standards.	
	Other		
		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
		nipre (1870) no contrato de la contrato del contrato del contrato de la contrato del contrato de la contrato del contrato de la contrato del contrato de la contrato del contrato de la co	
ALL PROPER SA	FETY REQUIREMENTS SHOULD BE FOLI	LOWED	A SIL SO

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: mike roberts-1495	Builder: owner
Address:	Permitting Office: (dumbia
City, State:	Permit Number: 2 66 87
Owner:	Jurisdiction Number: 22000
Climate Zone: North	
	1
1. New construction or existing New	12. Cooling systems
2. Single family or multi-family Single family	a. Central Unit Cap: 28.0 kBtu/hr SEER: 13.00
3. Number of units, if multi-family	Washington and the second of t
4. Number of Bedrooms 3 5. Is this a worst case? Yes	b. N/A
5. Is this a worst case? Yes 6. Conditioned floor area (ft²) 1495 ft²	c. N/A
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)	C. NA
a. U-factor: Description Area	13. Heating systems
(or Single or Double DEFAULT) 7a. (Dble Default) 146.0 ft ²	a. Electric Heat Pump Cap: 30.0 kBtu/hr
b. SHGC:	HSPF: 8.00
(or Clear or Tint DEFAULT) 7b. (Clear) 146.0 ft ²	b. N/A
8. Floor types	CONTROL STATE OF THE STATE OF T
a. Slab-On-Grade Edge Insulation R=0.0, 189.0(p) ft	c. N/A
b. N/A	
c. N/A	14. Hot water systems
9. Wall types	a. Electric Resistance Cap: 50.0 gallons
a. Frame, Wood, Adjacent R=13.0, 290.0 ft ²	EF: 0.90
b. Frame, Wood, Exterior R=13.0, 1100.0 ft ²	b. N/A
c. N/A	
d. N/A	c. Conservation credits
e. N/A	(HR-Heat recovery, Solar
10. Ceiling types	DHP-Dedicated heat pump)
a. Under Attic R=30.0, 1495.0 ft ²	15. HVAC credits
b. Under Attic R=19.0, 200.0 ft ²	(CF-Ceiling fan, CV-Cross ventilation,
c. N/A	HF-Whole house fan,
11. Ducts	PT-Programmable Thermostat,
a. Sup: Unc. Ret: Unc. AH: Garage Sup. R=6.0, 123.0 ft	MZ-C-Multizone cooling,
b. N/A	MZ-H-Multizone heating)
_	,
T-1-1 1- 26	
Glass/Floor Area: 0.10	
Total base	points: 22002
I hereby certify that the plans and specifications covered by	Review of the plans and
this calculation are in compliance with the Florida Energy	specifications covered by this
Code.	calculation indicates compliance
PREPARED BY: Surcoust lusulators	with the Florida Energy Code.
DATE: 6-20-07	Before construction is completed this building will be inspected for
	compliance with Section 553.908
I hereby certify that this building, as designed, is in	Florida Statutes.
compliance with the Florida Energy Code.	OD WE TROOP
OWNER/AGENT:	BUILDING OFFICIAL:
DATE:	DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS:,,,	PERMIT#:

BASE		AS-BUILT								
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area	Type/SC Or	Overhang nt Len Hgt	Area X	SPM X	SOF	= Points				
.18 1495.0 18.59 5003.0	1.Double, Clear	E 2.0 5.0	59.0	42.06	0.80	1977.0				
		W 2.0 5.0	77.0	38.52	0.80	2371.0				
	3.Double, Clear	S 2.0 5.0	4.0	35.87	0.72	103.0				
	4.Double, Clear	N 2.0 5.0	6.0	19.20	0.87	100.0				
	As-Built Total:		146.0			4551.0				
WALL TYPES Area X BSPM = Poi	ts Type	R-Value	Area	X SPM	1 =	Points				
Adjacent 290.0 0.70 20	.0 1. Frame, Wood, Adjacent	13.0	290.0	0.60		174.0				
Exterior 1100.0 1.70 187	.0 2. Frame, Wood, Exterior	13.0	1100.0	1.50		1650.0				
Base Total: 1390.0 207	.0 As-Built Total:		1390.0			1824.0				
DOOR TYPES Area X BSPM = Poi	ts Type		Area	X SPM	1 =	Points				
Adjacent 18.0 2.40 4	.2 1.Exterior Insulated		18.0	4.10		73.8				
Exterior 18.0 6.10 10	.8 2.Adjacent Insulated		18.0	1.60		28.8				
Base Total: 36.0 15	.0 As-Built Total:		36.0			102.6				
CEILING TYPES Area X BSPM = Poi	ts Type	R-Value	Area X SI	PM X SC	CM =	Points				
Under Attic 1495.0 1.73 258	.4 1. Under Attic	30.0	1495.0 1	.73 X 1.00		2586.4				
Southeadardarder same control control control	2. Under Attic	19.0	200.0 2	2.34 X 1.00		468.0				
Base Total: 1495.0 258	.4 As-Built Total:		1695.0		COLUMN TO SERVICE DE LA COLUMN TO SERVICE DESTRUCTURA DE LA COLUMN TO SERVICE	3054.4				
FLOOR TYPES Area X BSPM = Poi	ts Type	R-Value	Area	X SPM	1 =	Points				
Slab 189.0(p) -37.0 -699	.0 1. Slab-On-Grade Edge Insulation	0.0	189.0(p	-41.20		-7786.8				
Raised 0.0 0.00	.0									
Base Total: -699	.0 As-Built Total:		189.0	Contro South		-7786.8				
INFILTRATION Area X BSPM = Poi	ts	kalanus kalanga kalanga	Area	X SPM	1 =	Points				
1495.0 10.21 1526	.0		1495.0	10.21		15264.0				

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS:,,,	PERMIT#:

	BASE		AS-BUILT							
Summer Ba	se Points:	18086.3	Summer As-		17009.1					
Total Summer Points	X System Multiplier	= Cooling Points	Total X Component (System - Points	Cap Ratio	X Duct X Multiplier (DM x DSM x AHU)	System X Multiplier	Credit Multiplier	= Cooling Points		
18086.3	0.3250	5878.0	(sys 1: Central Unit 28 17009 17009.1	1.00 1.00	SEER/EFF(13.0) Ducts (1.09 x 1.147 x 1.00) 1.250		9ar(AH),R6.0(IN 1.000 1.000	5529.0 5 529.0		

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS:,,,
PERMIT#:

BASE	BASE AS-BUILT						
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Overhal Type/SC Ornt Le	ng en Hgt Area X WPM X WOF = Po					
.18 1495.0 20.17 5428.0	1.Double, Clear E 2.0						
	2.Double, Clear W 2.0						
	3.Double, Clear S 2.0						
	4.Double, Clear N 2.0	0 5.0 6.0 24.58 1.01 14					
	As-Built Total:	146.0 311					
WALL TYPES Area X BWPM = Poin	Type F	R-Value Area X WPM = Point					
Adjacent 290.0 3.60 1044	1. Frame, Wood, Adjacent	13.0 290.0 3.30 95					
Exterior 1100.0 3.70 4070	2. Frame, Wood, Exterior	13.0 1100.0 3.40 374					
Base Total: 1390.0 5114	As-Built Total:	1390.0 469					
DOOR TYPES Area X BWPM = Poin	Туре	Area X WPM = Point					
Adjacent 18.0 11.50 207	1.Exterior Insulated	18.0 8.40 15					
Exterior 18.0 12.30 221	2.Adjacent Insulated	18.0 8.00 14					
Base Total: 36.0 428	As-Built Total:	36.0 29					
CEILING TYPES Area X BWPM = Poin	Type R-Val	ue Area X WPM X WCM = Point					
Under Attic 1495.0 2.05 3064	1. Under Attic	30.0 1495.0 2.05 X 1.00 306					
6	2. Under Attic	19.0 200.0 2.70 X 1.00 54					
Base Total: 1495.0 3064	As-Built Total:	1695.0 360					
FLOOR TYPES Area X BWPM = Poin	Type F	R-Value Area X WPM = Point					
Slab 189.0(p) 8.9 1682	Slab-On-Grade Edge Insulation	0.0 189.0(p 18.80 355					
Raised 0.0 0.00 0		37 					
Base Total: 1682	As-Built Total:	189.0 355					
INFILTRATION Area X BWPM = Poin		Area X WPM = Point					
1495.0 -0.59 -882		1495.0 -0.59 -882					

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

	the state of the s
ADDRESS:,,,	PERMIT #:

	BASE	AS-BUILT											
Winter Base	Points:	Winter As-Built Points:								5.2 Winter As-Built Points:		14381.1	
Total Winter X Points	Heating Points	Total Component (System - P		Cap Ratio						Credit = Heating Multiplier Points			
14835.2	0.5540	8218.7	(sys 1: Electric 14381.1 14381.1	Hea	1.000 1.000			9 x 1) Ducts:Unc(.00) 0.426 0.426		Jnc(R),Gar(A 1.000 1.000	2/6	86.0 7660.4 '660.4

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE					AS-BUILT							
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X	Credit Multipli	
3		2635.00		7905.0	50.0 As-Built To	0.90 otal:	3	(ALIENSE)	1.00	2693.56	1.00	8080.7 8080.7

	CODE COMPLIANCE STATUS												
BASE					AS-BUILT								
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
5878		8219		7905		22002	5529		7660		8081		21270

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS:,,,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK		
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.			
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.			
Floors 606.1.ABC.1.2.2		Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.			
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.			
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.			
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.			
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.			

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.2

The higher the score, the more efficient the home.

1.	New construction or existing	New	_		Cooling systems		
2.	Single family or multi-family	Single family		a.	Central Unit	Cap: 28.0 kBtu/hr	_
3.	Number of units, if multi-family	1	-			SEER: 13.00	_
4.	Number of Bedrooms	3		Ь.	N/A		_
5.	Is this a worst case?	Yes					_
6.	Conditioned floor area (ft²)	1495 ft²		c.	N/A		
7.	Glass type 1 and area: (Label reqd.	by 13-104.4.5 if not default)					_
a.	U-factor:	Description Area		13.	Heating systems		
	(or Single or Double DEFAULT)	7a. (Dble Default) 146.0 ft ²	_	a.	Electric Heat Pump	Cap: 30.0 kBtu/hr	_
b.	SHGC:					HSPF: 8.00	_
	(or Clear or Tint DEFAULT)	7b. (Clear) 146.0 ft ²	-	b.	N/A		
8.	Floor types	0.7					
a.	Slab-On-Grade Edge Insulation	R=0.0, 189.0(p) ft	_	C.	N/A		
ь.	N/A	3					
c.	N/A		_	14.	Hot water systems		
9.	Wall types			a.	Electric Resistance	Cap: 50.0 gallons	-
a.	Frame, Wood, Adjacent	R=13.0, 290.0 ft ²	_			EF: 0.90	-
b.	Frame, Wood, Exterior	R=13.0, 1100.0 ft ²	_	b.	N/A		
C.	N/A						
d.	N/A			C.	Conservation credits		
e,	N/A				(HR-Heat recovery, Solar		
10.	Ceiling types				DHP-Dedicated heat pump)		
a.	Under Attic	R=30.0, 1495.0 ft ²	1.	15.	HVAC credits		_
b.	Under Attic	R=19.0, 200.0 ft ²			(CF-Ceiling fan, CV-Cross ventilation,		
c.	N/A		_		HF-Whole house fan,		
11.	Ducts				PT-Programmable Thermostat,	單	
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 123.0 ft	-		MZ-C-Multizone cooling,		
b.	N/A				MZ-H-Multizone heating)		
	rtify that this home has comp					THE STATE	
	struction through the above e					30	B
	his home before final inspection		Displa	ay Ca	ard will be completed		115
base	ed on installed Code complian	nt features.				S Line San	
Buil	lder Signature:		Date:			is the	
Add	lress of New Home:		City/	FL Z	ip:	GOD WE THUS	A STATE OF THE STA
*N/	OTF: The home's estimated en	perav performance score is	s only	avail	able through the FLA/RES comp	uter program	

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStd^M designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCSB v4.5)

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave Lake City, FL. 32025 Phone 386-752-6677 Fax 386-752-1477

Building Permit # Owner's Name	
Well DepthFt. Casing DepthFt. Wa	ater LevelFt.
Casing Size 4 inch Steel Pump Installation: Deep	#
Pump Make Clernotor Pump Model 53	10-100 HP 1
System Pressure (PSI) On 30 Off 50	Average Pressure 40
Pumping System GPM at average pressure and pumping le	
Tank Installation: Rladder/Galvanized Make Chamber Model C2-44 Size 81	Menger)
Tank Draw-down per cycle at system pressure	5./ gallons
I HEREBY VERTIFY THAT THIS WATER WELL S INSTALLED AS PER THE ABOVE INFORMATION	
Signature Newcomb	Linda Newcomb Print Name
2609	B
License Number	Date

Inst. Number: 200712028129 Book: 1139 Page: 344 Date:	12/21/2007 Time: 12:21:00 PM Page 1 of 2
	ta.
When recorded, mail to:	
Name: <u>PETTA OMEGA PROPUNTES INC.</u> Address: <u>3454 SW CR 242</u>	
Address: 3454 SW CR 242	
	15
City/State/Zip Code: LAKE CITY, FLA. 32024	Inst:200712028129 Date:12/21/2007 Time:12:21 PM Doc Stamp-Deed:0.70 DC,P.DeWitt Cason,Columbia County Page 1 of 1
	Space above this line for Recorder's use
MANOW ALL MEN BY THESE BRESENTS	IM DEED
That I (wa) DEGTA OMEGA PROGRAMUS	James 1000 Smither Valuable considerations, do
the undersigned, for the consideration of Ten Dol	lars (\$10.00), and other valuable considerations, do
hereby release, remise, and forever quitclaim unto	THILE POSSES
all right, title and interest in that certain Property s State of <u>Funda</u> , and describe	d as follows:
according to the plat thereof 1 79-82, public records, Columbia	County, Florida.
SUBJECT TO: Restrictions, ease mineral rights of record, if an current year.	ements and outstanding ny, and taxes for the
	· · · · · · · · · · · · · · · · · · ·
N WITNESS WHEREOF, I (we) have hereunto s	et my(our) hand(s) and seal this 21 day of
Printed Name of Releasor MIKE W ROBOWS Printed Name of Releasor	Signature of Releasor
Printed Name of Witness (If required by State Laws)	Signature of Witness (If required by State Laws)

Inst. Number: 200712028129 Book: 1139 Page: 344a Date: 12/21/2007 Time: 12:21:00 PM .Page 2 of 2



STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DEWILL GASON, CLERK OF COURTS

By OX	dist	add	
-		Deputy Clerk	
Date	10	ale 0?	

State of FLORIDA	Date 10 ale 07
State of FLORIDA County of Cocumpia	-)
	ES THEM Sm, THEY
known to me to be the Individual(s) who exec	uted the foregoing instrument and acknowledged the same
to be his(her)(their) free act and deed. My Commission Expires: 2420	08 Shela Darlere Kaemmer Notary Public
If acknowledged in the State of Florida, com	plete section(s) below:
(Releasor) Personally Known (or) Produce	EVDIDES to all ages
(Co-Releasor) Personally Known (or) DF If applicable, Type of Identification Produce	
	NOWLEDGMENT tate Of California)
State of California)) 8S.
County of	
	, before me,
	, the undersigned Notary Public, personally appeared,
name(s) is(are) subscribed to the attache	the basis of satisfactory evidence) to be the person(s) whose d instrument and acknowledged to me that he(she)(they) ized capacity(ies), and that by his(her)(their) signature(s) on y upon behalf of which the person(s) acted, executed the
Notary Public	EXPIRES: June 24, 2008 Bonded Thru Budget Notary Services
Motery Fubilio	

PREPARED BY AND RETURN TO: TERRY MCDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328

Property Appraiser's Parcel Identification No.: R-03117-000

Inst:2007009361 Date:04/25/2007 Time:16:05 DC,P.DeWitt Cason,Columbia County B:1117 P:1483 Doc Stamp-Deed :

_ . .

File No: 07-156

WARRANTY DEED

THIS INDENTURE, made this 2 day of April, 2007 between DELTA OMEGA PROPERTIES, INC., a corporation existing under the laws of the State of Florida, whose post office address is 3454 SW CR 242, Lake City, FL 32024, and having its principal place of business in the County of Columbia, State of Florida, party of the first part, and MICHAEL W. ROBERTS, whose post office address is 657 SW Catherine Lane, Lake City, Florida, of the County of Columbia, State of Florida, parties of the second part,

WITNESSETH: that the said party of the first part, for and in consideration of the sum of Ten Dollars (\$10.00), to it in hand paid, the receipt whereof is hereby acknowledged, has granted, bargained, sold, aliened, remised, released, conveyed and confirmed, and by these presents doth grant, bargain, sell, alien, remise, release, convey and confirm unto the said party of the second part, and its heirs and assigns forever, all that certain parcel of land lying and being in the County of Columbia and State of Florida, more particularly described as follows:

Lot 12, 25 and 45, Crosswinds, Phase One, a subdivision according to the plat thereof recorded in Plat Book 8, Page 79-82, public records, Columbia County, Florida.

SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year.

TOGETHER with all the tenements, hereditaments and appurtenances, with every privilege, right, title, interest and estate, reversion, remainder and easement thereto belong or in anywise appertaining:

TO HAVE AND TO HOLD the same in fee simple forever.

And the said party of the first part doth covenant with said party of the second part that it is lawfully seized of said premises; that they are free of all encumbrances, and that it has good right and lawful authority to sell the same; and the said party of the first part does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

IN WITNESS WHEREOF, the party of the first part has caused these presents to be signed in its name by its President, and its corporate seal to be affixed, the day and year above written.

Signed, sealed and delivered in our presence:

DELTA OMEGA PROPERTIES, INC.

TETETY MADAVIO

TOREN N. Ou 5 56

KAREN N. Drught

"Witnesses"

James R. Smithey, President

Inst:2007009361 Date:04/25/2007 Time:16:05

Doc Stamp-Deed: 882.00

DC,P.DeWitt Cason,Columbia County B:1117 P:1484

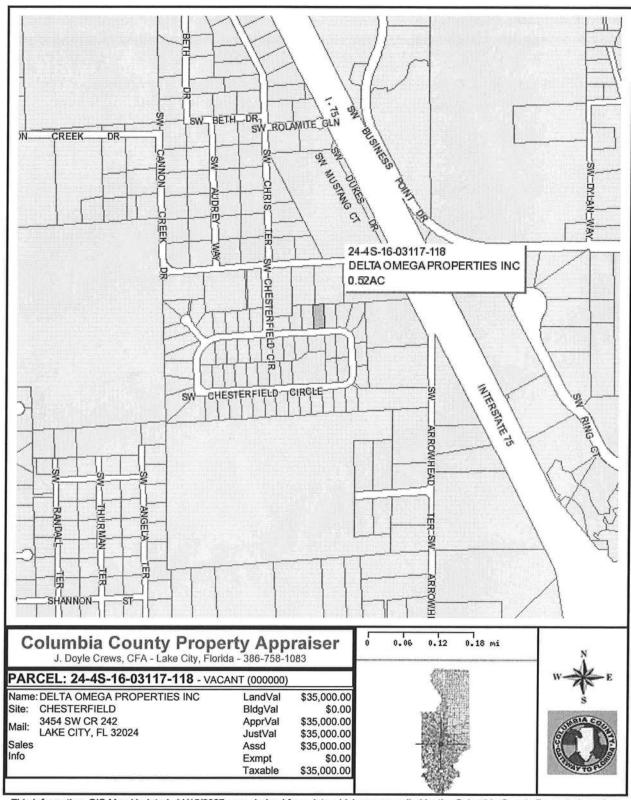
STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this day of April, 2007, by James R. Smithey, President of Delta Omega Properties, Inc., a State of Florida corporation, on behalf of the corporation. He is personally known to me and did not take an oath.

TERRY MCDAVID
MY COMMISSION # DD 500788
EXPIRES: January 16, 2010
Bondlert Thru Notery Public Underwriters

Notary Public

My Commission Expires:



This information, GIS Map Updated: 11/15/2007, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

COLUMBIA COUNTY 9-1-1 ADDRESSING / GIS DEPARTMENT

P. O. Box 1787, Lake City, FL 32056-1787 Telephone: (386) 758-1125 * Fax: (386) 758-1365 * E-mail: ron_croft@columbiacountyfla.com

ADDRESS ASSIGNMENT DATA

The Columbia County Board of County Commissioners has passed Ordinance 2001-9, which provides for a uniform numbering system. A copy of this ordinance is available in the Clerk of Court records, located in the courthouse. This new numbering system will increase the efficiency of POLICE, FIRE AND EMERGENCY MEDICAL vehicles responding to calls within Columbia County by immediately identifying the location of the caller.

<u>Residential or Other Structure on Parcel Number:</u> 24-4S-16-03117-118

Address Assignment: 309 SW CHESTERFIELD CIR, LAKE CITY, FL, 32024

Any questions concerning this information should be referred to the Columbia County 9-1-1 Addressing / GIS Department at the address or telephone number above.

NOTICE OF COMMENCEMENT
Tax Parcel Identification Number 24-45-16-03117-118
THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT .
1. Description of property (legal description): 10+18 Crosswinds 8/0 Phase 1 a) Street (job) Address: 309 Sw ChesterfieldCir, Lake City, FL 3202
2. General description of improvements: Single Family Dwelling
a) Name and address: Mike Roberts 657 sw Catherine Lane b) Name and address of fee simple titleholder (if other than owner) c) Interest in property
4 Contractor Information
a) Name and address: William & Wood Po Box 3535 LAKE City, FL 32056 b) Telephone No.: 755-8699 Fax No. (Opt.)
5. Surety Information
a) Name and address: N/A
b) Amount of Bond: A Fax No. (Opt.)
6 Lender
a) Name and address: N/A b) Phone No.
7. Identity of person within the State of Florida designated by owner upon whom notic a) Name and address DA Inst: 200712028327 Date: 12/26/2007 Time: 3:05 PM DC,P.DeWitt Cason, Columbia County Page 1
b) Telephone No.:Fax No. (Opt.)
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
a) Name and address: Name and address: Name and address: Name and address:
a) Name and address: b) Telephone No.: Fax No. (Opt.)
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified):
WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT. STATE OF FLORIDA COUNTY OF COLUMBIA COUNTY OF COLUMBIA The foregoing instrument was acknowledged before me, a Florida Notary, this day of December, 20 0 7, by:
The foregoing instrument was acknowledged before me, a Florida Notary, this day of JECENTOE, 20 0 1, by: Mike Roberts as Ounce (type of authority, e.g. officer, trustee, attorney)
fact) for
Personally Known OR Produced Identification Type
Notary Signature Bull de Dely Notary Stamp or Seal:
11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the
facts stated in it are true to the best of my knowledge and belief.
Signature of Natural Person Signing (in line #10 above.)



STATE OF FLORIDA DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number <u>67-1004</u>

		——— PART II - SITE PL	AN	
Scale: Each block	represents 5 feet and 1 in	ch = 50 feet.		
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	PHI CROSS		45-16-03117	1-118
Site Plan submitte	ed by: Zolust w	Dock he Signature		Title
Plan Approved V		Not Approved		Date 12-27-07
ву	s Zona	ely	Columbia	County Health Department
	LI CHANGES MUST	SE ADDROVED BY THE	COUNTY HEALTH D	EDADTMENT



STATE OF FLORIDA DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number ___ - PART II - SITE PLAN - cale: Each block represents 5 feet and 1 inch = 50 feet. DRY ReTENTION SW CHESTERFIELD e Plan submitted by: ın Approved ____ Not Approved Date

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

County Health Department



COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in

accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03117-118

Building permit No. 000026687

Fire:

77.00

Use Classification SFD, UTILITY

Waste: 201.00

Owner of Building MIKE ROBERTS

Permit Holder WILLIAM WOOD

Total: 278.00

Location: 309 SW CHESTERFIELD CIRCLE, LAKE CITY, FL

Date: 10/06/2008

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)



Project Information for:

L264517

Address:

309 Southwest Chesterfield Road

Lake City, FL

County:

Columbia

Truss Count:

28

Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002
Truss Design Load Information:
Gravity: Wind:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Engineer of Record: Unknown at time of Seal Date Address: Unknown at time of Seal Date

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

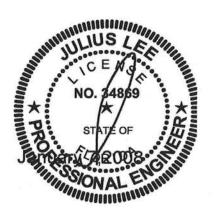
Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

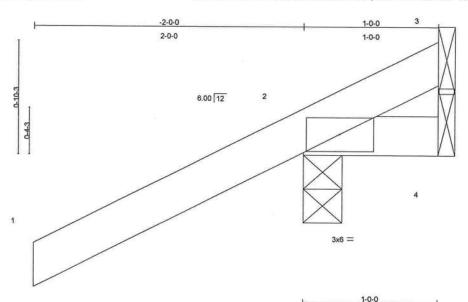
No.	Drwg. #	Truss ID	Seal Date
1	J1921219	CJ1	1/7/08
2	J1921220	CJ3	1/7/08
3	J1921221	CJ5	1/7/08
4	J1921222	EJ7	1/7/08
5	J1921223	EJ7A	1/7/08
6	J1921224	EJ7B	1/7/08
7	J1921225	HJ3	1/7/08
8	J1921226	HJ7	1/7/08
9	J1921227	HJ9	1/7/08
10	J1921228	T01	1/7/08
11	J1921229	T02	1/7/08
12	J1921230	T03	1/7/08
13	J1921231	T04	1/7/08
14	J1921232	T05	1/7/08
15	J1921233	T06	1/7/08
16	J1921234	T07	1/7/08
17	J1921235	T08	1/7/08
18	J1921236	T09	1/7/08
19	J1921237	T10	1/7/08
20	J1921238	T11	1/7/08
21	J1921239	T12	1/7/08
22	J1921240	T13	1/7/08
23	J1921241	T14	1/7/08
24	J1921242	T15	1/7/08
25	J1921243	T16	1/7/08
26	J1921244	T17	1/7/08
27	J1921245	T18	1/7/08
28	J1921246	T19	1/7/08



Job Truss Truss Type Ply LOT 18 Qty J1921219 L264517 CJ1 JACK 16 1 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:00 2007 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	- BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)	handed Nessale			1100000	>2/6/72/	Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

1-0-0

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracina.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=87(load case 6)

Max Uplift 2=-286(load case 6), 4=-9(load case 4), 3=-90(load case 1)

Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-69/75

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 Ib uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3. Continued on page 2

January 7,2008

Scale: 1.5"=1"

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	CJ1	JACK	16	1		J1921219
			1,55,55		Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:00 2007 Page 2

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Florida PE No. 24869 1100 Ceastal Bay Blvd

January 7,2008



Job Truss Truss Type Ply **LOT 18** Qty J1921220 L264517 CJ3 **JACK** 14 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:00 2007 Page 1 -2-0-0 3-0-0 2-0-0 3-0-0 Scale = 1:12.5 6.00 12 2 043 3x6 = 3-0-0 3-0-0 LOADING (psf) **SPACING** 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.29 Vert(LL) 0.01 >999 2-4 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.08 Vert(TL) -0.01>999 2-4 240 BCLL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 3 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 13 lb LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=132(load case 6)

Max Uplift 3=-28(load case 7), 2=-238(load case 6), 4=-27(load case 4)

Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-57/7

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4. Continued on page 2

January 7,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conner Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	CJ3	JACK	14	1		J1921220
					Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:00 2007 Page 2

LOAD CASE(S) Standard

January 7,2008



Job Truss Truss Type LOT 18 Qty Ply J1921221 L264517 CJ5 **JACK** 14 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:00 2007 Page 1 -2-0-0 5-0-0 2-0-0 5-0-0 Scale = 1:16.9 6.00 12 5-0-0 5-0-0 LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP in TCLL 20.0 Plates Increase 1.25 TC 0.29 Vert(LL) 0.09 2-4 >663 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.24 Vert(TL) -0.052-4 >999 240 BCLL 10.0 * Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 3 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 19 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 5-0-0 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-87(load case 6), 2=-260(load case 6), 4=-46(load case 4) Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-88/36

BOT CHORD

2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4. Continued on page 2

January 7,2008

🔬 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI/TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	CJ5	JACK	14	1		J1921221
			1000000	1	Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:01 2007 Page 2

LOAD CASE(S) Standard

Julius Lem Truss Cesign Engineer Floada PE No. 24866 1109 Cestel Bay Blyd Boynton Besch, FL 33435

January 7,2008



Job	Truss	Truss T	уре	Qty	Ply	L	OT 18			
L264517	EJ7	JACK		30		1				J1921222
Buildere EiretCo	ource, Lake City, FI 3205					Jo	b Referen	ce (option	al)	
Dulluers Firston	dice, Lake City, Fr 3200	55	6.3	500 S Apr 19 200	o Milek I	naust	tries, Inc.	Mon Jan (07 08:13:52 2008	Page 1
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									4	
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		I		6-11					7-β -0	
				6-11	-4				0-0-12	_
LOADING (psf	SPACING	2-0-0	CSI	DEFL		(loc)		L/d	PLATES	GRIP
TCLL 20.0 TCDL 7.0		1.25 1.25	TC 0.50 BC 0.45	Vert(LL)	0.32	2-4		360	MT20	244/190
BCLL 10.0		YES	WB 0.00	Vert(TL) Horz(TL)	-0.16	2-4		240 n/a		
					- (5/200/35/)		1.01.76		Weight: 26 lb	
BCDL 5.0	Code FBC2004/	1712002	(Matrix)						vveignt. 26 it)

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=154/Mechanical, 2=352/0-4-0, 4=44/Mechanical

Max Horz 2=161(load case 6)

Max Uplift 3=-94(load case 6), 2=-225(load case 6), 4=-64(load case 5) Max Grav 3=154(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-131/54

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.57

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0pst; Category II, LAP J., enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip

 This trues is designed for C-C for members and forces, and for MWFRS for reactions 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B;
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3, 225 lb uplift at joint 2 and 64 lb uplift at joint 4.

LOAD CASE(S) Standard

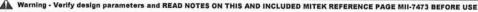
January 7,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Julius Les Truss Cesian Endineer Florida PE No. 34868 Florida PE No. 34868 Goynlon Besch, FL 33436 Boynlon Besch, FL 33436

January 7,2008



Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job Truss Truss Type Qty Ply **LOT 18** J1921223 L264517 EJ7A COMMON 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:01 2007 Page 1 -2-0-0 6-7-0 7-0-0 2-0-0 6-7-0 0-5-0 Scale: 1/2"=1" 4x6 = 6.00 12 043 6 5 3x6 = 2x4 || 7-0-0 7-0-0 LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.37 Vert(LL) 0.10 2-6 >812 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.29 Vert(TL) -0.17>473 240 2-6 **BCLL** 10.0 Rep Stress Incr YES WB 0.05 Horz(TL) 0.00 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 30 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 Structural wood sheathing directly applied or TOP CHORD BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins.

BOT CHORD

bracing.

REACTIONS

WEBS

(lb/size) 2=351/0-3-8, 5=202/Mechanical

Max Horz 2=147(load case 6)

Max Uplift 2=-146(load case 6), 5=-69(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-95/52, 3-4=0/10

BOT CHORD

2-6=0/0, 5-6=0/0

WEBS

3-6=-171/217

2 X 4 SYP No.3

JOINT STRESS INDEX

2 = 0.56, 3 = 0.09 and 6 = 0.12

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 2 and 69 lb uplift at joint 5. Continued on page 2

Truss Les Truss Design Engineer Florida PE No. 24 Bes 1 109 Coestal Bay Blvd Boynton Besch, FL 99436

Rigid ceiling directly applied or 10-0-0 oc

January 7,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	LOT 18	1 a Thirth agus agus agus an channais
L264517	EJ7A	COMMON	1	1		J1921223
			· · · · · · · · · · · · · · · · · · ·	1000	Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

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LOAD CASE(S) Standard

January 7,2008



Job Ply Truss Truss Type LOT 18 Qty J1921224 L264517 EJ7B MONO HIP 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:02 2007 Page 1 2-0-0 5-7-0 7-0-0 2-0-0 5-7-0 1-5-0 Scale = 1:21.5 5x8 = 2x4 II 4 6.00 12 04-3 Simpson HTU26 6 5 3x6 2x4 || 3x6 = 5-7-0 7-0-0 5-7-0 1-5-0 LOADING (psf) SPACING 2-0-0 CSI DEFL in L/d **PLATES** GRIP (loc) I/defl TCLL 20.0 Plates Increase 1.25 TC 0.27 Vert(LL) -0.022-6 >999 360 MT20 244/190 TCDL 7.0 BC Lumber Increase 1.25 0.16 Vert(TL) -0.032-6 >999 240 BCLL 10.0 * Rep Stress Incr NO WB 0.11 0.00 Horz(TL) 5 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 37 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins. WEBS 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=407/0-3-8, 5=450/Mechanical

Max Horz 2=140(load case 5)

Max Uplift 2=-170(load case 5), 5=-151(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-325/55, 3-4=-1/0

BOT CHORD 2-6=-73/227, 5-6=-70/212

WEBS 3-6=-69/297, 3-5=-518/171, 4-5=-37/33

JOINT STRESS INDEX

2 = 0.26, 3 = 0.23, 4 = 0.02, 5 = 0.22 and 6 = 0.21

NOTES

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 Ib uplift at joint 2 and 151 lb uplift at joint 5. Continued on page 2

January 7,2008

🔬 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	LOT 18	***********
L264517	EJ7B	MONO HIP	1	1		J1921224
The second second	10 2 1 AND 18 10 17 10 10 10 10 10 10 10 10 10 10 10 10 10				Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:02 2007 Page 2

NOTES

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

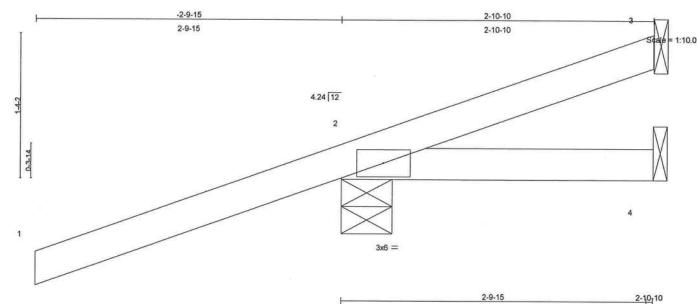
1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-54, 3-4=-98(F=-44), 2-6=-10, 5-6=-18(F=-8) Concentrated Loads (lb) Vert: 6=-243(F)

> Julius Lee Truss Design Engineer Florida PE No. 3-1869 1100 Ceastel Bay Blyd Boynton Beach, Ft. 23-435

> > January 7,2008



Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	НЈЗ	MONO TRUSS	1	1	J1921225
Buildoro FiretCourse	Lake City, FI 32055	0.000	10 00001		Job Reference (optional) Justries, Inc. Mon Jan 07 08:15:38 2008 Page 1



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.00	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mati					711.00	100	Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD

Structural wood sheathing directly applied or

2045

2-10-10 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size)

(lb/size) 3=-63/Mechanical, 2=286/0-5-11, 4=6/Mechanical

Max Horz 2=78(load case 3)

Max Uplift 3=-63(load case 1), 2=-289(load case 3)

Max Grav 3=89(load case 3), 2=286(load case 1), 4=32(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=-43/32

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.12

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 3 and 289 lb uplift at joint 2.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Julius Less Truss Design Engineer Florida PE No. 24868 1100 Coastal Bay Blvd Boynton Beach, FL 22425

January 7,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.

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Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	нлз	MONO TRUSS	1	1		J1921225
		(Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 07 08:15:38 2008 Page 2

LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-54
 Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-3=-39(F=8, B=8), 2=0(F=5, B=5)-to-4=-7(F=1, B=1)

Julius Lee Truss Design Engineer Florida PE No. 34868 1100 Costal Bay Blyri



Job Truss Truss Type LOT 18 Qty Ply J1921226 L264517 HJ7 MONO TRUSS 1 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:03 2007 Page 1 -2-9-15 7-10-12 2-9-15 7-10-12 Scale 4.24 12 0-3-14 7-10-1 7-10-12 7-10-1 0-0-11 LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL Plates Increase 20.0 1.25 TC 0.62 Vert(LL) >733 -0.122-4 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.37 Vert(TL) -0.242-4 >386 240 BCLL 10.0 * Rep Stress Incr NO WB 0.00 Horz(TL) -0.00 3 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 29 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 7-10-12 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=237/Mechanical, 2=367/0-5-11, 4=54/Mechanical

Max Horz 2=194(load case 3)

Max Uplift 3=-190(load case 3), 2=-257(load case 3)

Max Grav 3=237(load case 1), 2=367(load case 1), 4=115(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=-88/55

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.49

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 3 and 257 lb uplift at joint 2.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

Julius Less Truss Cession Engineer Flonda PE No. 24868 1109 Cessial Bay Blvd Boynton Beach, FL 22425

January 7,2008

A Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LOT 18	V/0.00 2 000 0 000 000
L264517	HJ7	MONO TRUSS	1	1	11	J1921226
					Job Reference (optional)	

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-3=-107(F=-26, B=-26), 2=-0(F=5, B=5)-to-4=-20(F=-5, B=-5)



Job Truss Truss Type Ply LOT 18 Qty J1921227 L264517 HJ9 MONO TRUSS 6 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:03 2007 Page 1 -2-9-15 9-10-13 2-9-15 4-3-0 5-7-13 4.24 12 3x6 = 0-3-14 7 2x4 || 3x6 = 4-3-0 9-10-1 9-10-13 4-3-0 5-7-1 LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defi L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.61 Vert(LL) 0.10 6-7 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.40 Vert(TL) -0.12>984 6-7 240 BCLL 10.0 * Rep Stress Incr NO WB 0.34 Horz(TL) 0.01 5 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 45 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 Structural wood sheathing directly applied or TOP CHORD BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins. WEBS 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 7-11-9 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=456/0-5-11, 5=218/Mechanical

Max Horz 2=269(load case 3)

Max Uplift 4=-233(load case 3), 2=-401(load case 3), 5=-181(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=-647/363, 3-4=-105/65

BOT CHORD 2-7=-535/599, 6-7=-535/599, 5-6=0/0

WEBS 3-7=-94/190, 3-6=-624/558

JOINT STRESS INDEX

2 = 0.77, 3 = 0.22, 6 = 0.17 and 7 = 0.13

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 401 lb uplift at joint 2 and 181 lb uplift at joint 5.

Truss Design Engineer Florida FE No. 24869 1 109 Ceastal Bay Blvd Boynton Beach, FL 93435

Continued on page 2

January 7,2008

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Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	НЈ9	MONO TRUSS	6	1		J1921227
			ľ		Job Reference (optional)	

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NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

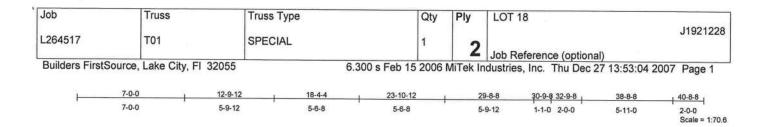
Vert: 1-2=-54

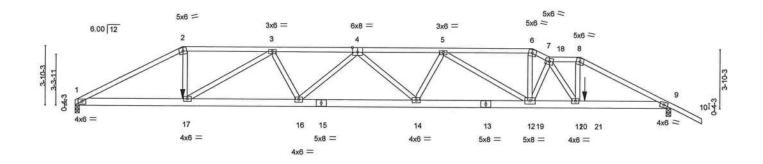
Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee Trues Design Engineer Florida PE No. 34869 1109 Coastel Bay Blyri







	7-0	0-0 7-	-6-7		7-7-10		7-6	6-7	3-	1-0 0-4-0	5-7-0	
Plate O	ffsets (X,Y): [4:0-4-0,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.30	14-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.57	14-16	>803	240	MINES:	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)			- 5	0.075:000		Weight: 440 lb	55

22-2-1

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TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-8-7 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

32-9-8 33-1-8

38-8-8

bracing.

29-8-8

REACTIONS (lb/size) 1=2560/0-3-8, 9=2766/0-3-8

Max Horz 1=-104(load case 6)

Max Uplift 1=-793(load case 4), 9=-803(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-5345/1716, 2-3=-4782/1587, 3-4=-7187/2350, 4-5=-7467/2394,

5-6=-5653/1782, 6-18=-5980/1858, 7-18=-6013/1855, 7-8=-4883/1486,

8-9=-5429/1605, 9-10=0/51

BOT CHORD 1-17=-1489/4704, 16-17=-2231/6881, 15-16=-2477/7681, 14-15=-2477/7681,

13-14=-2299/7293, 12-13=-2299/7293, 12-19=-1708/5846, 19-20=-1708/5846,

11-20=-1708/5846, 11-21=-1371/4782, 9-21=-1371/4782

WEBS 2-17=-551/1849, 3-17=-2559/897, 3-16=-114/714, 4-16=-694/297, 4-14=-345/215,

5-14=-43/435, 5-12=-1999/738, 6-12=-456/1854, 7-12=-702/174, 7-11=-1806/629,

8-11=-626/2031

JOINT STRESS INDEX

1 = 0.80, 2 = 0.53, 3 = 0.44, 4 = 0.27, 5 = 0.44, 6 = 0.62, 7 = 0.56, 8 = 0.49, 9 = 0.81, 11 = 0.45, 12 = 0.29, 13 = 0.75, 14 = 0.80, 12 = 0.80, 13 = 0.80, 13 = 0.80, 14 = 0.0.30, 15 = 0.77, 16 = 0.30 and 17 = 0.41

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T01	SPECIAL	1		0	J1921228
				2	Job Reference (optional)	

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NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

5) Provide adequate drainage to prevent water ponding.

6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 793 lb uplift at joint 1 and 803 lb uplift at joint 9.
- 9) Girder carries tie-in span(s): 7-0-0 from 30-1-8 to 32-1-8

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

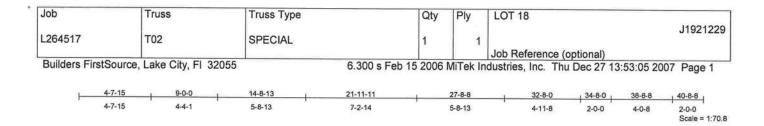
Vert: 1-2=-54, 2-6=-118(F=-64), 6-18=-118(F=-64), 7-18=-54, 7-8=-54, 8-10=-54, 1-17=-10, 17-19=-22(F=-12),

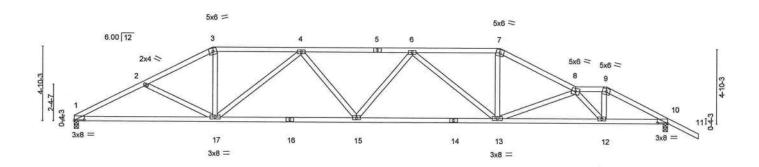
19-20=-85(F=-75), 9-20=-10

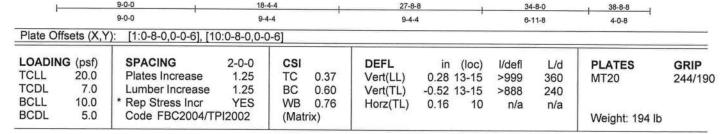
Concentrated Loads (lb)

Vert: 17=-411(F) 21=-450(F)









LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

3-7-4 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 5-4-2 oc bracing.

REACTIONS (lb/size) 1=1226/0-3-8, 10=1348/0-3-8

Max Horz 1=-102(load case 7)

Max Uplift 1=-237(load case 5), 10=-314(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2376/1296, 2-3=-2145/1165, 3-4=-1893/1103, 4-5=-2551/1420,

5-6=-2551/1420, 6-7=-2142/1224, 7-8=-2405/1290, 8-9=-2095/1096.

9-10=-2354/1166, 10-11=0/47

BOT CHORD 1-17=-1005/2071, 16-17=-1111/2468, 15-16=-1111/2468, 14-15=-1155/2561,

13-14=-1155/2561, 12-13=-1392/2907, 10-12=-880/2029

WEBS 2-17=-227/248, 3-17=-318/668, 4-17=-819/410, 4-15=0/229, 6-15=-68/126,

6-13=-652/311, 7-13=-350/754, 8-13=-851/527, 8-12=-1215/717, 9-12=-473/937

JOINT STRESS INDEX

1 = 0.70, 2 = 0.33, 3 = 0.42, 4 = 0.38, 5 = 0.60, 6 = 0.38, 7 = 0.51, 8 = 0.51, 9 = 0.41, 10 = 0.72, 12 = 0.59, 13 = 0.56, 14 = 0.88, 15 = 0.38, 16 = 0.87 and 17 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

Julius Les Truse Cesign Engineer Florida PE No. 24888 1 100 Coestal Bay Blvd Boynton Beach, FL 22425

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January 7,2008

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Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T02	SPECIAL	1	1		J1921229
					Job Reference (optional)	

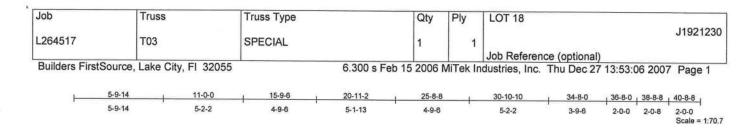
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:05 2007 Page 2

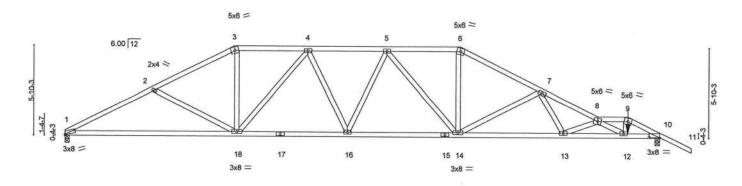
NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 1 and 314 lb uplift at joint 10.

LOAD CASE(S) Standard







—	11-0-0	18-4-4	25-8-8	32-5-0	36-8-0	38-8-8
	11-0-0	7-4-4	7-4-4	6-8-8	4-3-0	2-0-8

Plate Offsets (X,Y): [1:0-0-10,Edge], [10:0-8-0,0-0-6]
--

TCLL TCDL BCLL	IG (psf) 20.0 7.0 10.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr	2-0-0 1.25 1.25 NO	TC BC WB	0.46 0.85 0.51	Vert(LL) Vert(TL) Horz(TL)	in -0.33 -0.66 0.15	(loc) 1-18 1-18	I/defl >999 >703 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)					100	Weight: 206 lb	

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TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-3-15 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 4-7-0 oc

bracing.

REACTIONS (lb/size) 1=1227/0-3-8, 10=1360/0-3-8

Max Horz 1=-114(load case 7)

Max Uplift 1=-218(load case 6), 10=-337(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2338/1298, 2-3=-2029/1131, 3-4=-1769/1075, 4-5=-2100/1242,

5-6=-1923/1157, 6-7=-2194/1218, 7-8=-3189/1691, 8-9=-2065/1010.

9-10=-2254/1083, 10-11=0/47

BOT CHORD 1-18=-994/2037, 17-18=-877/2061, 16-17=-877/2061, 15-16=-905/2114,

14-15=-905/2114, 13-14=-1237/2587, 12-13=-1880/3832, 10-12=-810/1931

2-18=-325/324, 3-18=-293/609, 4-18=-555/253, 4-16=-20/160, 5-16=-95/64,

5-14=-429/170, 6-14=-324/669, 7-14=-770/507, 7-13=-249/590, 8-13=-1096/593,

8-12=-2039/1158, 9-12=-494/984

Julius Lee Truss Design Engineer Florida FE No. 34866 1400 Casstel Ray Blyd Boynton Besch, FL 23435

JOINT STRESS INDEX

1 = 0.87, 2 = 0.33, 3 = 0.42, 4 = 0.45, 5 = 0.45, 6 = 0.46, 7 = 0.42, 8 = 0.64, 9 = 0.42, 10 = 0.70, 12 = 0.62, 13 = 0.46, 14 = 0.56, 15 = 0.71, 16 = 0.45, 17 = 0.66 and 18 = 0.56

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design. Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	Т03	SPECIAL	1	1		J1921230
	12/10/14/11	10 CO			Job Reference (optional)	

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NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint 1 and 337 lb uplift at joint 10.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

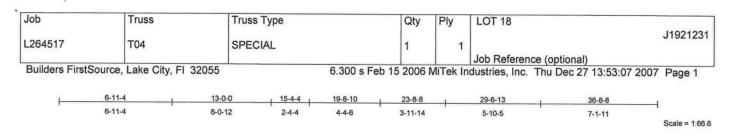
1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

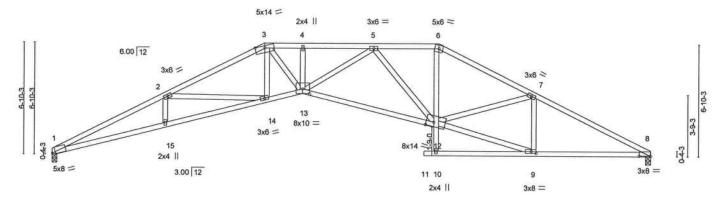
Vert: 1-3=-54, 3-6=-54, 6-8=-54, 8-9=-54, 9-11=-54, 1-10=-10

Concentrated Loads (lb)

Vert: 12=-12(B)







Plata Of	fsets (X, Y		.00400	61 10.0	20046	7-5-12	0-7-0	0.0.5.0	100		0-9-4	
Flate Of	isels (A, I	'): [1:0-2-6,Edge], [8	.0-0-4,0-0-	·o], [9:0-	3-8,0-1-8	3], [12:0-5-8,0	-3-2], [1	3:0-5-0	,0-3-8]			
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.50	13	>878	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.84	12-13	>523	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.50	8	n/a	n/a		
BCDL	5.0	Code FBC2004/T	PI2002	(Mat	rix)	, ,	17.004.5	8		14.00	Weight: 189 lb	

22-10-0

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TOP CHORD 2 X 4 SYP No.2

6-11-4

BOT CHORD 2 X 4 SYP No.2 *Except*

6-10 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

36-8-8

2-6-11 oc purlins.

29-11-4

. . .

BOT CHORD

23-5-0

Rigid ceiling directly applied or 4-4-13 oc

bracing.

REACTIONS (lb/size) 1=1168/0-3-8, 8=1170/0-3-8

Max Horz 1=85(load case 5)

Max Uplift 1=-224(load case 6), 8=-222(load case 7)

R-D-12

244

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-4123/2169, 2-3=-3345/1715, 3-4=-3724/1929, 4-5=-3723/1929, TOP CHORD

5-6=-2072/1210, 6-7=-2357/1282, 7-8=-2225/1197

BOT CHORD 1-15=-1891/3705, 14-15=-1889/3704, 13-14=-1306/3025, 12-13=-1247/2839,

10-12=0/102, 6-12=-374/762, 10-11=0/0, 9-10=-21/29, 8-9=-965/1906

2-15=0/208, 2-14=-683/566, 3-14=-160/250, 3-13=-504/1222, 4-13=-156/81,

5-13=-462/1154, 5-12=-1086/527, 9-12=-983/1956, 7-9=-460/296, 7-12=-77/290

JOINT STRESS INDEX

1 = 0.77, 2 = 0.39, 3 = 0.72, 4 = 0.33, 5 = 0.65, 6 = 0.43, 7 = 0.39, 8 = 0.72, 9 = 0.75, 10 = 0.70, 12 = 0.47, 13 = 0.56, 14 = 0.470.37 and 15 = 0.33

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LOT 18	5 2
L264517	T04	SPECIAL	1	1		J1921231
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:07 2007 Page 2

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

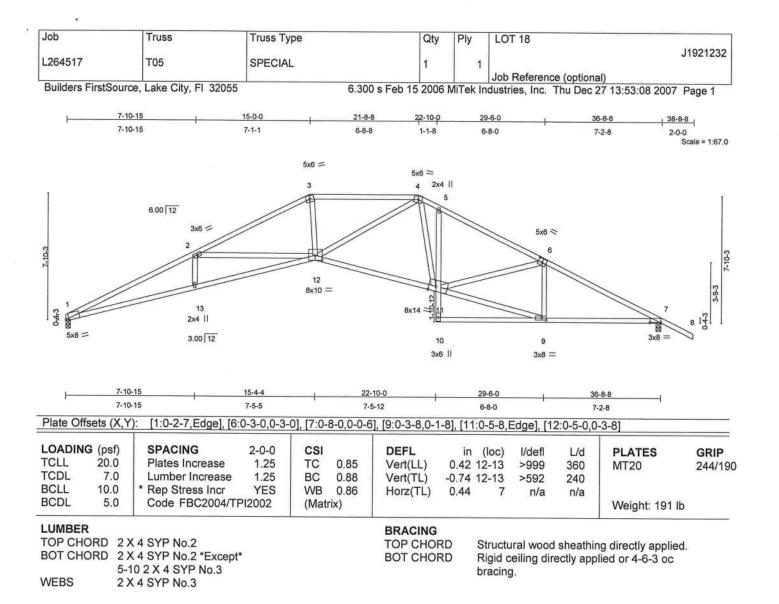
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 1 and 222 lb uplift at joint 8.

LOAD CASE(S) Standard





REACTIONS

(lb/size) 1=1162/0-3-8, 7=1285/0-3-8

Max Horz 1=-137(load case 7)

Max Uplift 1=-237(load case 6), 7=-331(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4060/2070, 2-3=-3083/1485, 3-4=-2769/1436, 4-5=-2216/1319,

5-6=-2309/1239, 6-7=-2179/1159, 7-8=0/47

BOT CHORD 1-13=-1711/3647, 12-13=-1707/3643, 11-12=-697/1998, 10-11=0/90,

5-11=-151/194, 9-10=-14/34, 7-9=-845/1862

WEBS 2-13=0/237, 2-12=-869/712, 3-12=-375/967, 4-12=-352/976, 4-11=-273/289,

9-11=-873/1915, 6-11=-57/242, 6-9=-460/292

JOINT STRESS INDEX

1 = 0.75, 2 = 0.39, 3 = 0.73, 4 = 0.51, 5 = 0.59, 6 = 0.73, 7 = 0.67, 9 = 0.73, 10 = 0.28, 11 = 0.30, 12 = 0.67 and 13 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This Continued designed for C-C for members and forces, and for MWFRS for reactions specified.

January 7,2008

🚵 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T05	SPECIAL	1	1		J1921232
	1100 50				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:08 2007 Page 2

NOTES

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

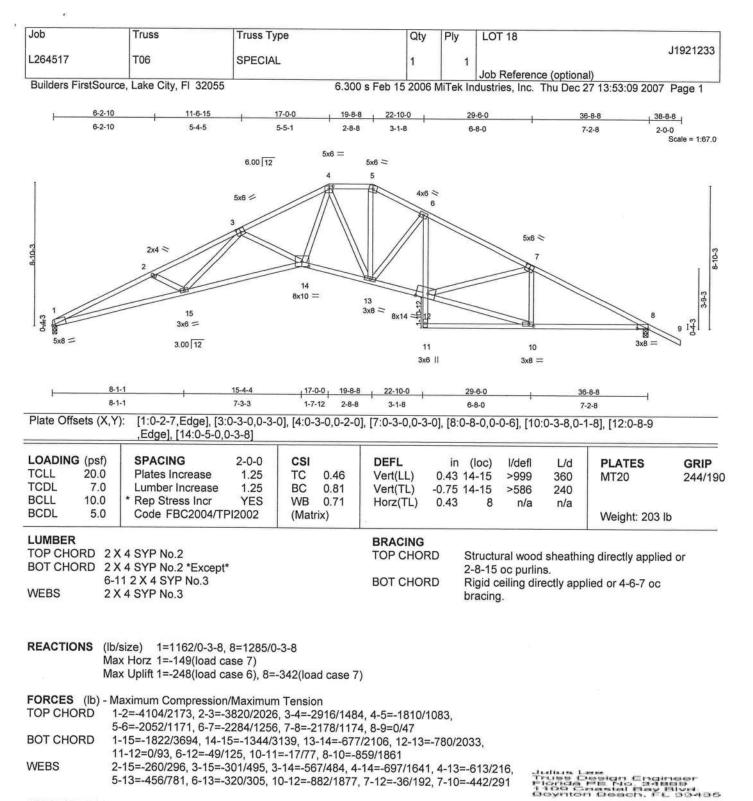
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 1 and 331 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34869 1100 Crestal Bay Blvd Boynton Beach, FL 33436





JOINT STRESS INDEX

1 = 0.76, 2 = 0.33, 3 = 0.58, 4 = 0.83, 5 = 0.36, 6 = 0.34, 7 = 0.72, 8 = 0.67, 10 = 0.71, 11 = 0.32, 12 = 0.54, 13 = 0.62, 14 = 0.58 and 15 = 0.37

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T06	SPECIAL	1	1		J1921233
L					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:09 2007 Page 2

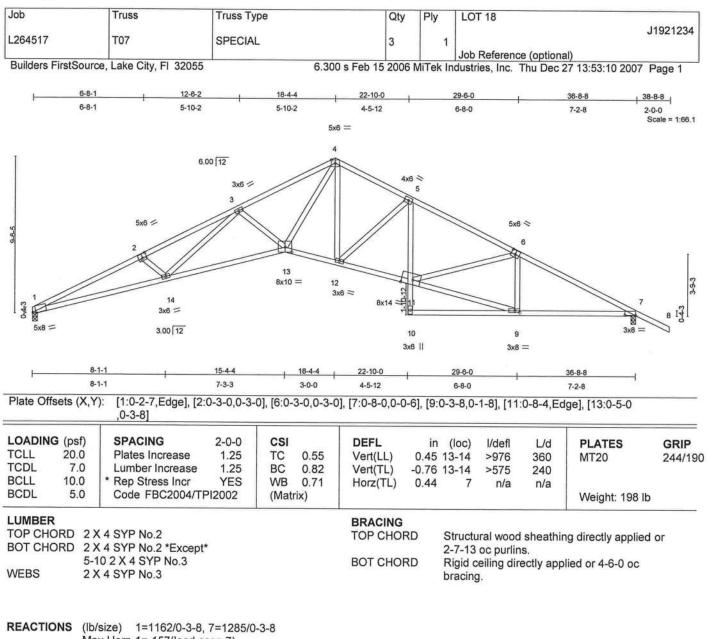
NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 1 and 342 lb uplift at joint 8.

LOAD CASE(S) Standard

as lesion Engineer PE No. 34869 eastal Bay Blvd. eastal Bay Blvd.





Max Horz 1=-157(load case 7)

Max Uplift 1=-254(load case 6), 7=-348(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4095/2172, 2-3=-3848/2094, 3-4=-2957/1551, 4-5=-1994/1138,

5-6=-2286/1271, 6-7=-2178/1185, 7-8=0/47

BOT CHORD 1-14=-1816/3685, 13-14=-1285/3055, 12-13=-533/1783, 11-12=-794/2037,

10-11=0/92, 5-11=-46/151, 9-10=-18/65, 7-9=-868/1861

2-14=-267/295, 3-14=-417/619, 3-13=-521/453, 4-13=-799/1767, 4-12=-271/385, **WEBS**

5-12=-390/321, 9-11=-891/1883, 6-11=-35/170, 6-9=-443/293

JOINT STRESS INDEX

1 = 0.76, 2 = 0.67, 3 = 0.39, 4 = 0.76, 5 = 0.29, 6 = 0.72, 7 = 0.67, 9 = 0.72, 10 = 0.31, 11 = 0.53, 12 = 0.38, 13 = 0.56 and 14 = 0.58= 0.38

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T07	SPECIAL	3	1		J1921234
					Job Reference (optional)	

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NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 1 and 348 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34869 1199 Crestal Bay Blvd Boynton Besch, FL 93435



Job Truss Truss Type Qty Ply **LOT 18** J1921235 L264517 T08 SPECIAL 4 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:11 2007 Page 1 -2-0-0 6-8-1 12-6-2 24-4-10 30-5-0 36-8-8 38-8-8 2-0-0 6-8-1 5-10-2 5-10-2 6-0-6 6-0-6 6-3-8 2-0-0 Scale = 1:70.2 5x6 = 5 6.00 12 3x6 = 2x4 || 5x6 = 5x6 > 12 8x10 = 3x6 = 5x8 = 3x8 = 3.00 12 10 5x6 = 15-4-4 24-4-10 30-6-11 8-1-1 7-3-3 9-0-6 6-0-6 0-1-11 6-1-13 Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [12:0-5-0,0-3-8] LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** in GRIP TCLL 20.0 Plates Increase 1.25 TC 0.39 Vert(LL) 0.23 12-13 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.59 -0.43 12-13 Vert(TL) >836 240 BCLL 10.0 Rep Stress Incr YES WB 0.54 Horz(TL) 0.26 10 n/a n/a BCDL Code FBC2004/TPI2002 5.0 (Matrix) Weight: 184 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 3-4-5 oc purlins. 2 X 4 SYP No.3 WEBS **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. **WEBS** T-Brace: 2 X 4 SYP No.3 -5-11 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length. REACTIONS (lb/size) 2=1021/0-3-8, 10=1543/0-3-8 Max Horz 2=-144(load case 7)

Max Uplift 2=-311(load case 6), 10=-588(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/46, 2-3=-2959/1272, 3-4=-2708/1194, 4-5=-1805/680, 5-6=-837/332, TOP CHORD

6-7=-842/199, 7-8=-846/714, 8-9=0/47

BOT CHORD 2-13=-983/2639, 12-13=-479/1999, 11-12=-101/988, 10-11=-616/954,

8-10=-565/896

WEBS 3-13=-266/293, 4-13=-387/600, 4-12=-520/441, 5-12=-392/1281, 5-11=-427/201,

6-11=-334/326, 7-11=-758/1320, 7-10=-1321/922

JOINT STRESS INDEX

2 = 0.68, 3 = 0.54, 4 = 0.39, 5 = 0.56, 6 = 0.33, 7 = 0.73, 8 = 0.44, 10 = 0.63, 11 = 0.55, 12 = 0.42 and 13 = 0.37

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	тов	SPECIAL	4	1		J1921235
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:11 2007 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

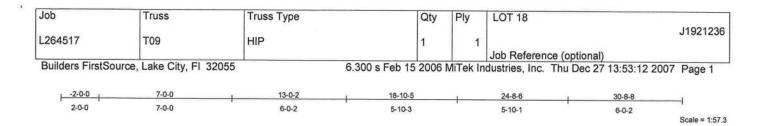
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

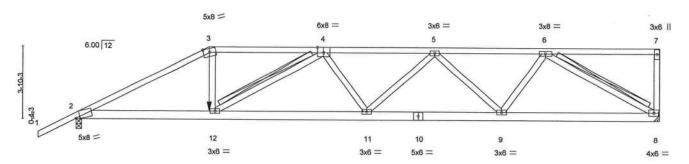
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 588 lb uplift at joint 10.

LOAD CASE(S) Standard







15-3-10

Simpson HTU26

		7-0-0		8-3-10			7-1-6				8-3-8	
Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-15,Edge]												
LOADIN	NG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.26	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.50	11-12	>734	240	10.101.000.000	
BCLL	10.0	* Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)				(10)1000		Weight: 176 lb	

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TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D WEBS 2 X 4 SYP No.3

7-0-0

BRACING

TOP CHORD

BOT CHORD

2-5-3 oc purlins, except end verticals. Rigid ceiling directly applied or 6-6-5 oc

Structural wood sheathing directly applied or

30-8-8

bracing.

22-5-0

WEBS

T-Brace:

2 X 4 SYP No.3 -

4-12, 6-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=2151/Mechanical, 2=2096/0-3-8

Max Horz 2=165(load case 5)

Max Uplift 8=-741(load case 4), 2=-654(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/51, 2-3=-4025/1311, 3-4=-3578/1218, 4-5=-4750/1603, 5-6=-3673/1225,

6-7=-120/42, 7-8=-330/167

BOT CHORD 2-12=-1204/3525, 11-12=-1673/4758, 10-11=-1592/4557, 9-10=-1592/4557.

8-9=-1070/3012

WEBS 3-12=-357/1234, 4-12=-1372/577, 4-11=-14/170, 5-11=-22/269, 5-9=-1236/513,

6-9=-282/1202, 6-8=-3362/1195

JOINT STRESS INDEX

2 = 0.76, 3 = 0.73, 4 = 0.46, 5 = 0.37, 6 = 0.85, 7 = 0.71, 8 = 0.85, 9 = 0.85, 10 = 0.95, 11 = 0.40 and 12 = 0.78 Continued on page 2

January 7,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	Т09	HIP	1	1		J1921236
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:12 2007 Page 2

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 741 lb uplift at joint 8 and 654 lb uplift at joint 2.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

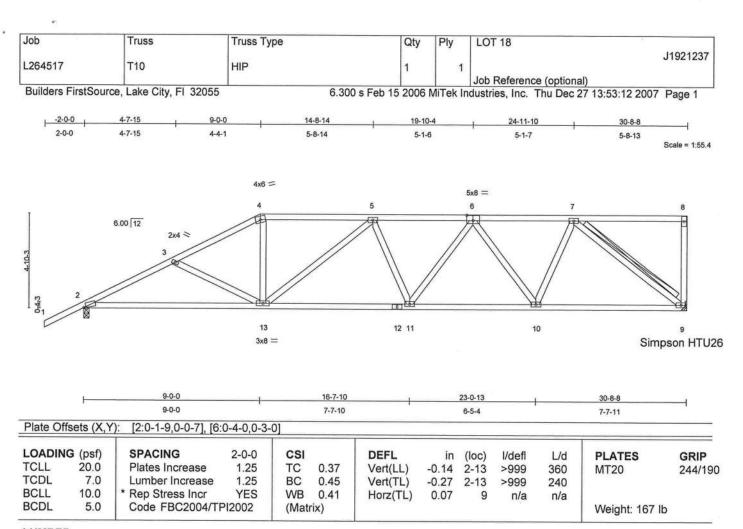
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-10, 8-12=-22(F=-12)

Concentrated Loads (lb) Vert: 12=-411(F)





LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 6-4-15 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 - 7-9 Fasten T and I braces to narrow edge of web

with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 9=969/Mechanical, 2=1093/0-3-8

Max Horz 2=195(load case 6)

Max Uplift 9=-265(load case 5), 2=-262(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1801/904, 3-4=-1570/802, 4-5=-1374/777, 5-6=-1598/863,

6-7=-1198/630, 7-8=-32/12, 8-9=-142/100

BOT CHORD 2-13=-952/1544, 12-13=-906/1633, 11-12=-906/1633, 10-11=-813/1490.

> 9-10=-543/997 3-13=-204/199, 4-13=-131/412, 5-13=-335/166, 5-11=-93/112, 6-11=-86/192,

6-10=-507/317, 7-10=-227/521, 7-9=-1245/685

JOINT STRESS INDEX

WEBS

2 = 0.86, 3 = 0.33, 4 = 0.60, 5 = 0.46, 6 = 0.26, 7 = 0.46, 8 = 0.26, 9 = 0.46, 10 = 0.46, 11 = 0.46, 12 = 0.54 and 13 = 0.56

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T10	HIP	1	1	1)	J1921237
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:13 2007 Page 2

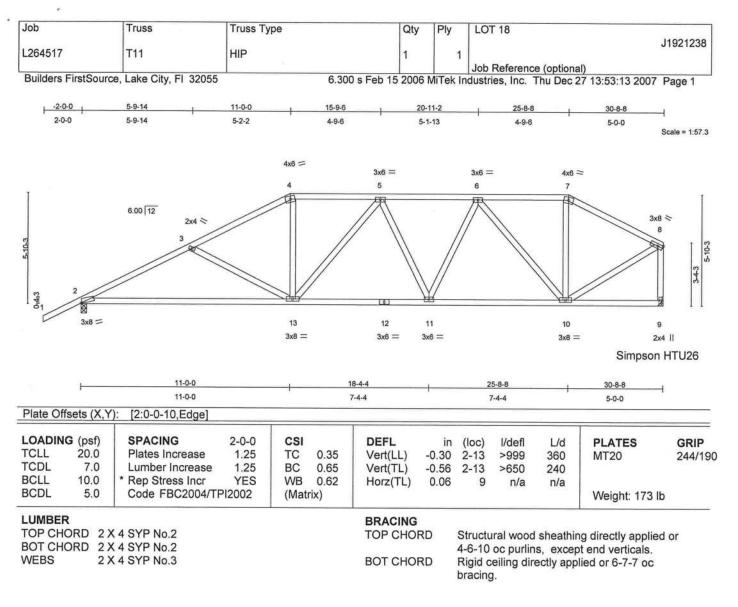
NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 265 lb uplift at joint 9 and 262 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda PE No. 34869 1109 Chastal Bay Blvd





REACTIONS (lb/size) 2=1093/0-3-8, 9=969/Mechanical

Max Horz 2=177(load case 6)

Max Uplift 2=-280(load case 6), 9=-182(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/47, 2-3=-1772/936, 3-4=-1455/788, 4-5=-1250/768, 5-6=-1264/764, TOP CHORD

6-7=-752/497, 7-8=-893/493, 8-9=-944/530

BOT CHORD 2-13=-897/1516, 12-13=-695/1337, 11-12=-695/1337, 10-11=-600/1160,

9-10=-29/33

WEBS 3-13=-311/290, 4-13=-132/385, 5-13=-252/125, 5-11=-189/132, 6-11=-93/261,

6-10=-674/354, 7-10=-19/193, 8-10=-405/843

JOINT STRESS INDEX

2 = 0.89, 3 = 0.33, 4 = 0.59, 5 = 0.45, 6 = 0.45, 7 = 0.51, 8 = 0.94, 9 = 0.42, 10 = 0.78, 11 = 0.45, 12 = 0.44 and 13 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

& Principle adequate drainage to prevent water ponding.

January 7,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T11	HIP	1	1		J1921238
		,			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:13 2007 Page 2

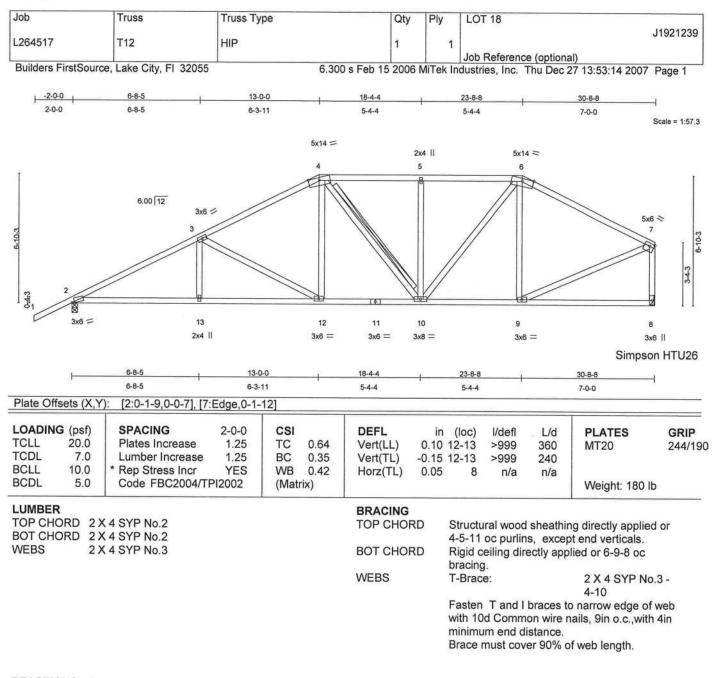
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2 and 182 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida ME No. 34868 1109 Ceastel Bay Blvd Boynton Beach, FL 38436





REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical

Max Horz 2=189(load case 6)

Max Uplift 2=-292(load case 6), 8=-158(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1782/908, 3-4=-1326/762, 4-5=-1095/721, 5-6=-1095/721,

6-7=-1004/566, 7-8=-928/547

2-13=-866/1513, 12-13=-866/1513, 11-12=-576/1122, 10-11=-576/1122, **BOT CHORD**

9-10=-404/819, 8-9=-57/74

3-13=0/210, 3-12=-452/330, 4-12=-120/328, 4-10=-75/116, 5-10=-287/160,

6-10=-223/505, 6-9=-277/204, 7-9=-385/822

WEBS

2 = 0.77, 3 = 0.39, 4 = 0.78, 5 = 0.33, 6 = 0.94, 7 = 0.69, 8 = 0.30, 9 = 0.46, 10 = 0.56, 11 = 0.38, 12 = 0.34 and 13 = 0.33

January 7,2008 Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and/ or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T12	HIP	1	1		J1921239
	Andrew C				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:14 2007 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

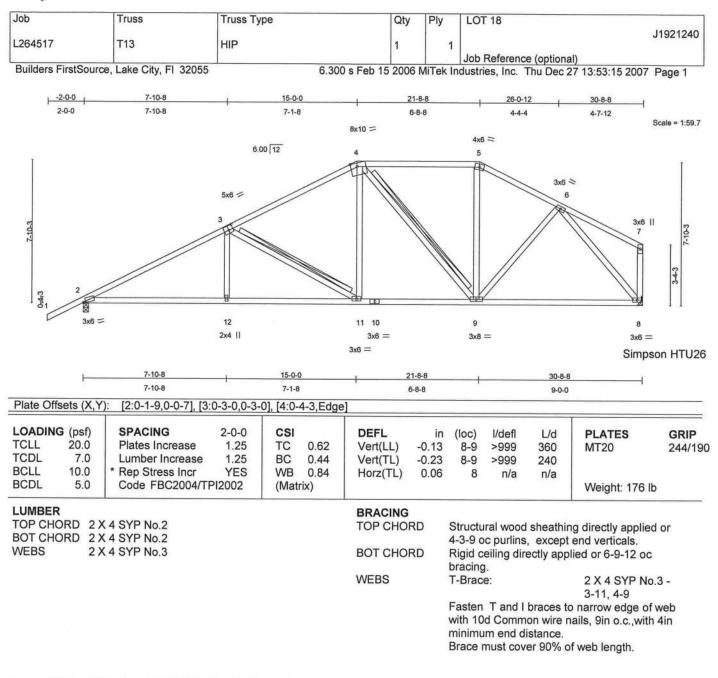
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 158 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda PE No. 34869 1100 Cestal Bay Blvd Boviton Beach, FL 33435





REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical

Max Horz 2=201(load case 6)

Max Uplift 2=-302(load case 6), 8=-172(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1742/902, 3-4=-1205/723, 4-5=-837/617, 5-6=-979/631, 6-7=-109/79

7-8=-147/118

BOT CHORD 2-12=-846/1468, 11-12=-846/1467, 10-11=-498/1004, 9-10=-498/1004,

rida PE No. 34999 OG Creastal Pay Blyd

8-9=-382/691

3-12=0/250, 3-11=-537/400, 4-11=-157/369, 4-9=-329/164, 5-9=-63/207,

6-9=-51/286, 6-8=-1008/582

JOINT STRESS INDEX

2 = 0.76, 3 = 0.82, 4 = 0.60, 5 = 0.66, 6 = 0.37, 7 = 0.23, 8 = 0.77, 9 = 0.56, 10 = 0.33, 11 = 0.34 and 12 = 0.33

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T13	HIP	1	1		J1921240
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:15 2007 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

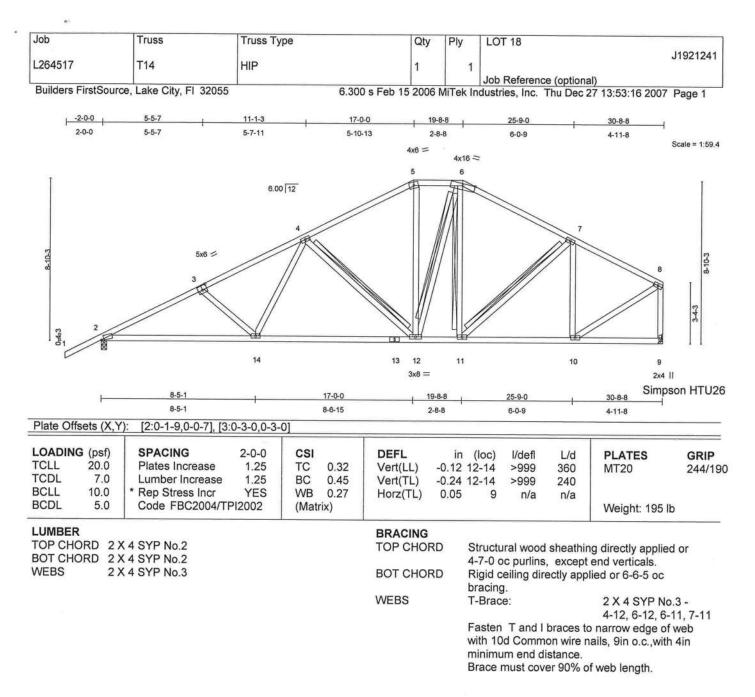
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 2 and 172 lb uplift at joint 8.

LOAD CASE(S) Standard





REACTIONS (lb/size) 2=1093/0-3-8, 9=969/Mechanical

Max Horz 2=213(load case 6)

Max Uplift 2=-310(load case 6), 9=-185(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/47, 2-3=-1798/964, 3-4=-1580/900, 4-5=-1039/689, 5-6=-864/679, TOP CHORD

6-7=-978/654, 7-8=-880/516, 8-9=-944/557

BOT CHORD 2-14=-927/1538, 13-14=-697/1231, 12-13=-697/1231, 11-12=-372/806,

10-11=-402/743, 9-10=-21/28

3-14=-253/246, 4-14=-115/361, 4-12=-504/405, 5-12=-140/242, 6-12=-104/322,

6-11=-78/52, 7-11=-39/167, 7-10=-409/290, 8-10=-454/851

JOINT STRESS INDEX

WEBS

NT STRESS INDEX
2 = 0.77, 3 = 0.39, 4 = 0.40, 5 = 0.53, 6 = 0.80, 7 = 0.39, 8 = 0.69, 9 = 0.41, 10 = 0.48, 11 = 0.35, 12 = 0.66, 13 = 0.40 and 14

January 7,2008 Continuedon page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T14	HIP	1	1		J1921241
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:16 2007 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

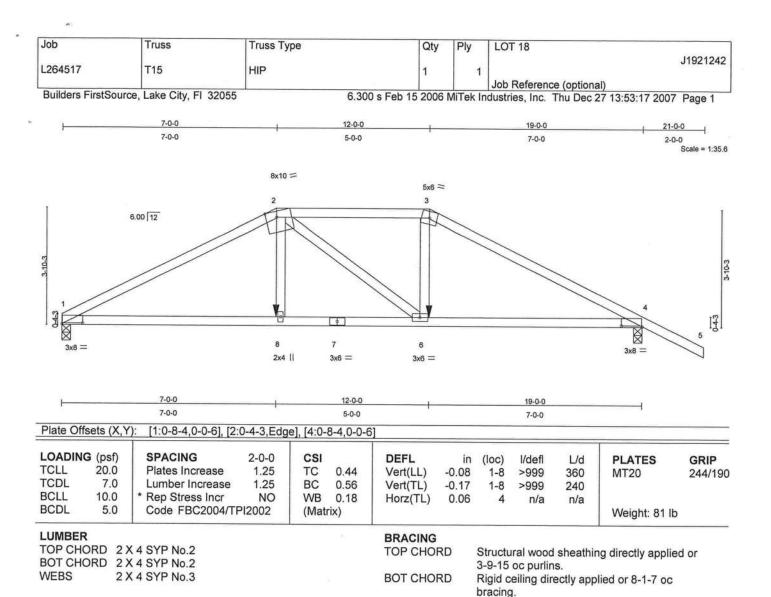
5) All plates are 3x6 MT20 unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 310 lb uplift at joint 2 and 185 lb uplift at joint 9.

LOAD CASE(S) Standard





REACTIONS (lb/size) 1=1189/0-3-8, 4=1318/0-3-8

Max Horz 1=-90(load case 6)

Max Uplift 1=-358(load case 5), 4=-456(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2278/694, 2-3=-1957/639, 3-4=-2260/676, 4-5=0/47

BOT CHORD 1-8=-580/1957, 7-8=-585/1978, 6-7=-585/1978, 4-6=-535/1936

WEBS 2-8=-126/524, 2-6=-149/118, 3-6=-145/576

JOINT STRESS INDEX

1 = 0.73, 2 = 0.73, 3 = 0.76, 4 = 0.75, 6 = 0.37, 7 = 0.65 and 8 = 0.37

NOTES

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truss Design Engineer Florida PE No. 34869 1199 Casstal Eay Blvd Boynton Beach, Ft. 20405

January 7,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Medison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T15	HIP	1	1		J1921242
		1.300			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:17 2007 Page 2

NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 358 lb uplift at joint 1 and 456 lb uplift at joint 4.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

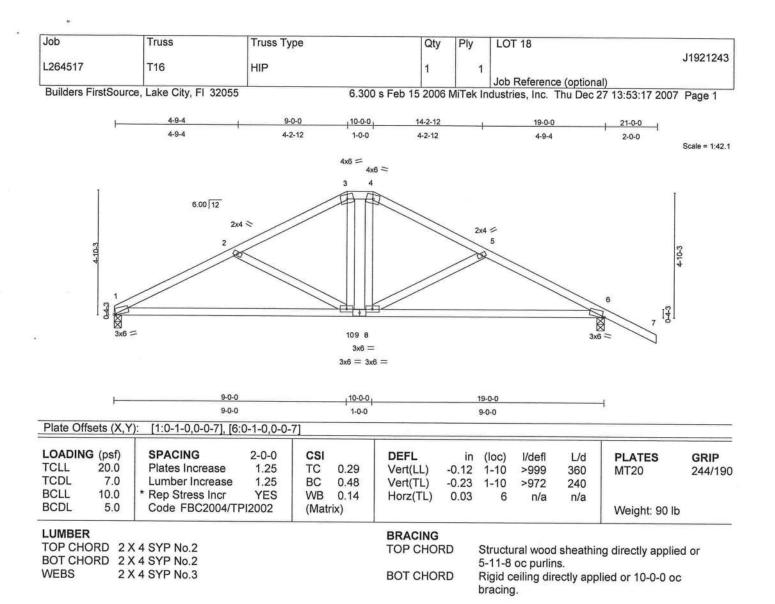
Vert: 1-2=-54, 2-3=-118(F=-64), 3-5=-54, 1-8=-10, 6-8=-22(F=-12), 4-6=-10

Concentrated Loads (lb)

Vert: 8=-411(F) 6=-411(F)

Julius Lee Truss Design Engineer Florida FE No. 24888 1100 Coastal Bay Blyd





REACTIONS (lb/size) 1=592/0-3-8, 6=721/0-3-8

Max Horz 1=-102(load case 7)

Max Uplift 1=-127(load case 6), 6=-225(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1021/590, 2-3=-756/444, 3-4=-630/445, 4-5=-753/439, 5-6=-1008/550.

6-7=0/47

BOT CHORD 1-10=-380/875, 9-10=-136/630, 8-9=-136/630, 6-8=-329/844

WEBS 2-10=-302/280, 3-10=-126/211, 4-8=-39/197, 5-8=-268/219

JOINT STRESS INDEX

1 = 0.85, 2 = 0.33, 3 = 0.45, 4 = 0.45, 5 = 0.33, 6 = 0.85, 8 = 0.34, 9 = 0.74 and 10 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Coliva leads page 2

Truse Les Truse Design Engineer Florida FE No. 24868 1 100 Gnastal Bay Blvd Boynton Beach, FL 33435

January 7,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HiB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T16	HIP	1	1	4	J1921243
					Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:18 2007 Page 2

NOTES

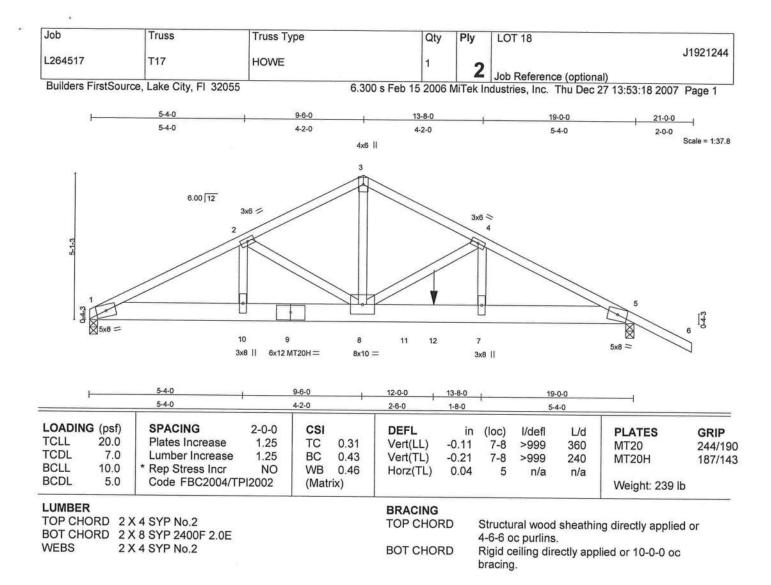
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 1 and 225 lb

LOAD CASE(S) Standard

January 7,2008





REACTIONS (lb/size) 1=4884/0-3-8, 5=3516/0-3-8

Max Horz 1=-111(load case 6)

Max Uplift 1=-1317(load case 5), 5=-1001(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-8132/2180, 2-3=-5888

1-2=-8132/2180, 2-3=-5888/1594, 3-4=-5876/1603, 4-5=-7289/1908, 5-6=0/53

BOT CHORD 1-10=-1947/7255, 9-10=-1947/7255, 8-9=-1947/7255, 8-11=-1661/6476,

11-12=-1661/6476, 7-12=-1661/6476, 5-7=-1661/6476

WEBS 2-10=-545/1961, 2-8=-2393/709, 3-8=-1342/4963, 4-8=-1479/428, 4-7=-278/1151

JOINT STRESS INDEX

1 = 0.82, 2 = 0.71, 3 = 0.55, 4 = 0.71, 5 = 0.82, 7 = 0.31, 8 = 0.45, 9 = 0.71 and 10 = 0.31

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

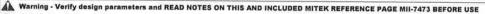
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

 Unbalanced roof live loads have been considered for this design. Continued on page 2 Julius Les Truss Design Engineer Florida PE No. 24868 1109 Ceastel Bay Blvd Boynton Besch, Ft. 22426

January 7,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building does. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T17	HOWE	1	_		J1921244
		Company Article Program (Committee Program (Committ		2	Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

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NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1317 lb uplift at joint 1 and 1001 lb uplift at joint 5.
- 9) Girder carries tie-in span(s): 30-8-8 from 0-0-0 to 11-0-0

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

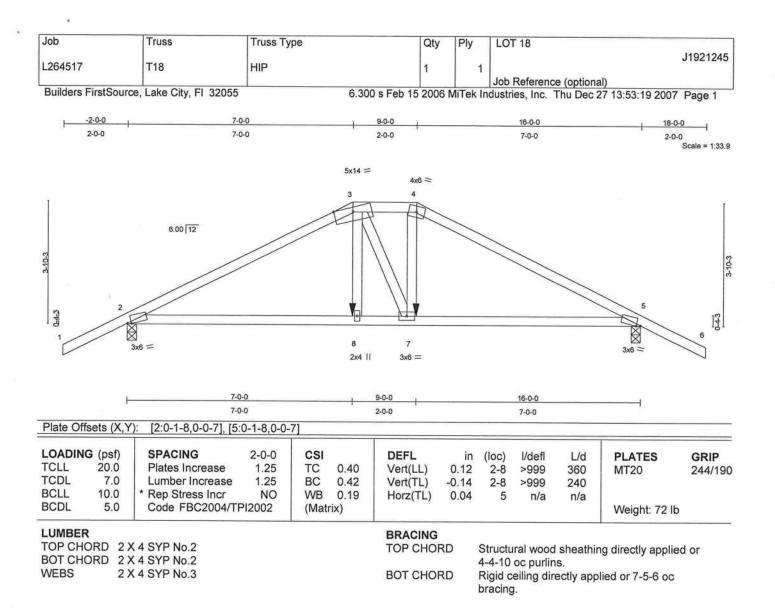
Vert: 1-3=-54, 3-6=-54, 1-11=-465(B=-455), 5-11=-10

Concentrated Loads (lb) Vert: 12=-2151(F)

> dullus Lee Trues Design Engineer Florida FE No. 34868 1100 Ceastel Bay Blyd

> > January 7,2008





REACTIONS (lb/size) 2=1103/0-3-8, 5=1103/0-3-8

Max Horz 2=77(load case 5)

Max Uplift 2=-595(load case 5), 5=-595(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1778/804, 3-4=-1526/770, 4-5=-1781/805, 5-6=0/47

2-8=-674/1503, 7-8=-684/1523, 5-7=-658/1506

BOT CHORD WEBS

3-8=-262/480, 3-7=-146/159, 4-7=-303/592

JOINT STRESS INDEX

2 = 0.77, 3 = 0.87, 4 = 0.76, 5 = 0.77, 7 = 0.38 and 8 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 7,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	LOT 18	20.00
L264517	T18	HIP	1	1		J1921245
	Lac manual	9			Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:19 2007 Page 2

NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 2 and 595 lb uplift at joint 5.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-10, 7-8=-22(F=-12), 5-7=-10

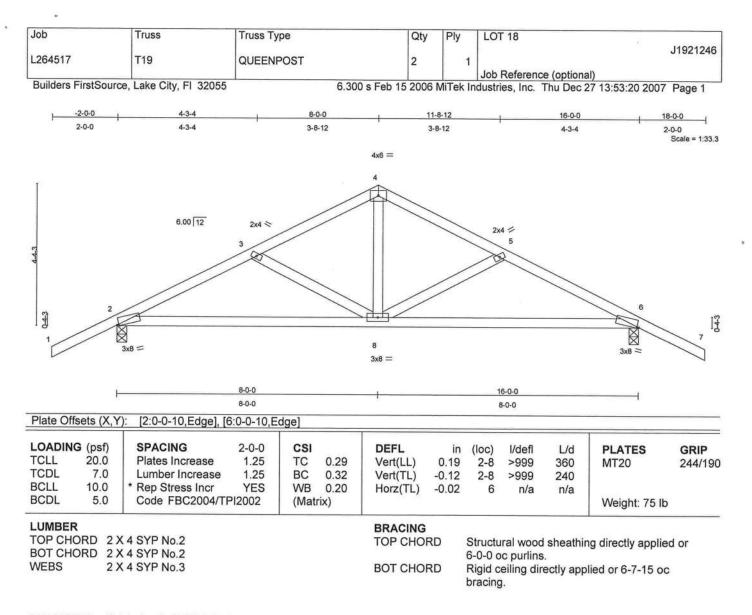
Concentrated Loads (lb)

Vert: 8=-411(F) 7=-411(F)

Julius Lee Truss Design Engineer Florida FE No. 34869 1100 Ceastal Bay Blvd

January 7,2008





REACTIONS

(lb/size) 2=619/0-3-8, 6=619/0-3-8

Max Horz 2=83(load case 6)

Max Uplift 2=-404(load case 6), 6=-404(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-802/1064, 3-4=-590/942, 4-5=-590/942, 5-6=-802/1064, 6-7=0/47

BOT CHORD

2-8=-810/663, 6-8=-810/663

WEBS

3-8=-217/243, 4-8=-670/331, 5-8=-217/243

JOINT STRESS INDEX

2 = 0.74, 3 = 0.13, 4 = 0.32, 5 = 0.13, 6 = 0.74 and 8 = 0.17

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Chall hearings age assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Law Truss Design Engineer Flonda PE No. 34869 1109 Ceastel Bay Blvd Boynton Beach, FL 33435

January 7,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Job	Truss	Truss Type	Qty	Ply	LOT 18	
L264517	T19	QUEENPOST	2	1		J1921246
					Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:53:20 2007 Page 2

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2 and 404 lb uplift at joint 6.

LOAD CASE(S) Standard

January 7,2008

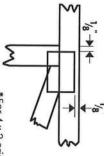


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise securely seat. plates to both sides of truss and Dimensions are in inches. Apply



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



This symbol indicates the required direction of slots in connector plates

PLATE SIZE

4 × 4

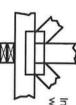
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



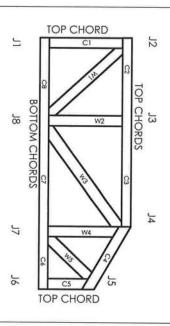
continuous lateral bracing. Indicates location of required

BEARING



which bearings (supports) occur. Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

ICBO

BOCA

96-31, 96-67

9667, 9432A 3907, 4922

WISC/DILHR

SBCCI

思

561

960022-W, 970036-N



MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)

4

- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Camber is a non-structural consideration and Unless expressly noted, this design is not preservative treated lumber. applicable for use with fire retardant or

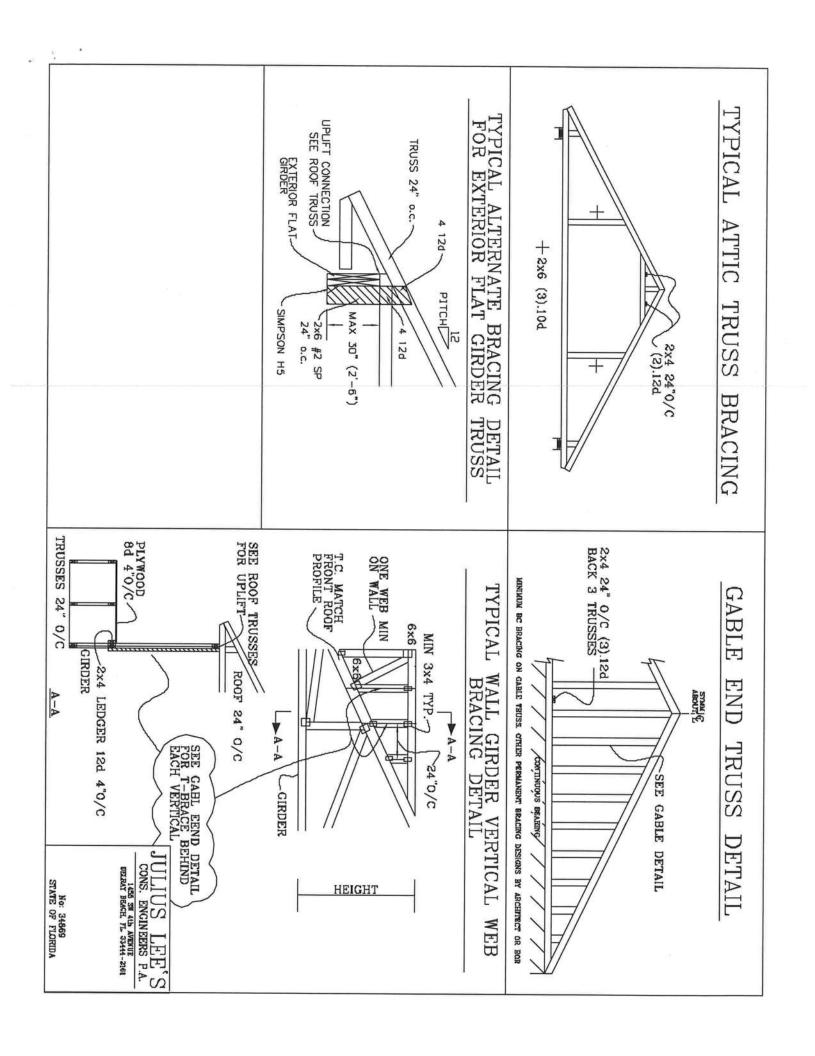
6

5

- 00 is the responsibility of truss fabricator. General Plate type, size and location dimensions practice is to camber for dead load deflection
- shown indicate minimum plating requirements
- 9 Lumber shall be of the species and size, and grade specified in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed unless otherwise noted
- Anchorage and / or load transferring others unless shown. connections to trusses are the responsibility of
- 13. Do not overload roof or floor trusses with stacks of construction materials
- Do not cut or after truss member or plate engineer, without prior approval of a professional
- Care should be exercised in handling erection and installation of trusses.
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DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED, CONNECT DIAGONAL BRACE FOR SAG AT EACH END. MAX WEB TOTAL LENGTH IS 14". MAX **GABLE VERTICAL** LENGTH VERTICAL LENGTH IN TABLE ABOVE. SPACING SPECIES 24" O.C. O.C. O.C. CONNECT DIAGONAL AT GABLE VERTICAL SPF SPF SPF DFL DFL DFL SP SP SP H H ASCE NAOHS \$1 / #2 \$3 STANDARD \$1 / #2 \$3 STUD STANDARD \$1 #2 #3 STANDARD STANDARD STANDARD GRADE STANDARD #2 #2 STUD STUD WEB 古書古 BRACE 7-02: ***MARDIGG** TRUSSES REDUIRE EXTRENE CARE IN FARRICATING, MANDLING, SIPPING, INSTALLING AND BRACING. RETER IO BESS 1-4G (BUILDING COPPINGN) FOR FETTY INFORMATION, PUBLISED BY TPI (TRUSS PLATE INSTITUTE, 383 DINGTRED BY, SUITE 200, MANDLING, VET 237159 AND VITA (AVOID TRUSS COLUCTIO OF MICRICA, 6300 ENTERPRISE UN, MANDISH, VI 33719) FOR SAFETY PARCIFICES PRIZE TO PERFORMING THICKE CANCILING. WILCES OF INFERVISE WINDLING CONTROL OF COLUMN AND PROPERLY ATTACHED STRUCTURAL PANELS AND BUTTON CHORD SMILL HAVE A PROPERLY ATTACHED RIGHT CELLING. 2# GABLE TRUSS BRACES 130 ZX4 SF #2N, DF-L #2, SFF #1/#2, DR BETTER DIAGONAL BRACE, SINGLE OR DOUBLE CUT (AS SHIPM) AT GROUP A Ξ MPH IX4 "L" BRACE . UPPER END. GROUP H o, 6, 2 8 0 6. 6 6 6. WIND (1) 2X4 "L" BRACE . GROUP A SPEED, GROUP B REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH SHADORY STANKES 15 18 (2) 2X4 "L" GROUP A MEAN CONTINUOUS BEARING EX4 MEN OR BETTER GROUP B BRACE ** 10' 2" 0 HEIGHT, CONS. 12 5 13 8 13 8 GROUP A (1) 2X6 DELRAY BEACH, FL 33444-2161 12' 5" 12' 4" 10' 10' 12, 3, 10' 7" 10' 4" No: 34869 STATE OF FLORIDA JUS LEE'S s. ENGINEERS P.A. 0 ď ENCLOSED, GROUP B 10' 7' 13' 5" 12' 6" 12' 6" BRACE . 13 14 8 8 0 12' 4" 10' 4" 12 GROUP A (Z) ZXB 12 11 S 12, TA. 0. r. Н MAX. MAX. GROUP B BRACE II 13' 11" 13' 7" 13' 7" 12' 11" 13 3 12' 0" TOT. SPACING 1.00, Б ATTACH EACH 'L' BRACE WITH 104 NAIS. \$ FOR (1) 'L' BRACE: SPACE NAIS AT 2" O.C. \$ FOR (2) 'L' BRACES: SPACE NAIS AT 3" O.C. \$ FOR (2) 'L' BRACES: SPACE NAIS AT 3" O.C. IN 18" EMD ZONES AND 6" O.C. BETWEEN ZONES. CABLE END SUPPORTS LOAD FROM 4: 0" PROVIDE UPLATT CONNECTIONS FOR 136 FLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). LIVE LOAD DEPLECTION CRITERIA IS L/240. MEMBER LENGTH. L' BRACING MUST BE A MINIMUM OF 80% OF WEB DOUGLAS FIR-LARCH #3 STUD STANDARD SPRUCE-PINE-FIR #1 / #2 STANDARD #3 STUD PLYWOOD OVERHANG. BRACING GROUP SPECIES CABLE TRUSS DETAIL NOTES: EXPOSURE 60 SOUTHERN 12 GREATER THAN 4' D', BUT 24.0" PEAK, SPLICE, AND HEEL PLATES. PERTICAL LENGTH GABLE VERTICAL PLATE SIZES PSF PINE REF DRWG MITEK STD CABLE 15 E HT DATE CROUP HEM-PIR H & BIR GROUP 0 DOUGLAS FIR-LARCE 11/26/03 ASCE7-02-CAB13015 Ä SOUTHERN PINE A: 3 2 NO SPLICE STANDARD AND HEM-PIR STANDARD 2.5X4 200 GRADES: GUIS

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WIRN DIAGONAL BRACE IS USED. CONNECT BRACE IS USED. MAY WEB AT EACH END. MAY WEB TOTAL LENGTH IS 14*. **GABLE** VERTICAL MAX LENGTH VERTICAL LENGTH IN TABLE ABOVE. SPACING SPECIES 24" O.C. 16 O.C. O.C. MIDPOINT OF VERTICAL WEB GABLE VERTICAL SPF DFL SPF SPF DFL SP SP SP H H E ASCE NAOHS STANDARD \$1 \$TUD \$TANDARD \$1 \$1 #2 #3 \$TUD STANDARD STANDARD STANDARD STANDARD GRADE #2 #2 STUD STUD 7-02: BRACE RAVARNOCIM TRASSIS REBURE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BRACHG. REPRINT DEST 1-43 ENDLING COMPRET SAFETY (REDAKTION), PUBLISHED BY TPI CIRLISS PAIR INSTITUIC, 383 DYNOCROED BX, SUITE 201, MINISON, LV CATION AND LYCH, AVIDDI TRUSS COLOCIL OF ANEREY, 6300 ENTERPRISS, LW, MIGISON, LV CATION FOR SAFETY PAIR, TO PERCONNING IN-SIE FUNCTIONS, UNICLESS OTHERWISES UNICLESS OF MERCATED TO CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PAIR CAPETY PAIR AND SOFTEN ATTACHED STRUCTURAL PAIR CAPETY PAIR AND SOFTEN ATTACHED STRUCTURAL PAIR PROPERTY ATTACHED REGID CELLING. 5# CABLE TRUSS BRACES 4 4 4 4 0 0 0 130 ZX4 SP OR DIT-L #2 OR BETTER DIAGONAL BRACE; SINGLE GROUP A (1) 1X4 "L" BRACE • (1) 2X4 "L" BRACE • (2) 2X4 "L" BRACE •• CUT (AS SHOWN) AT UPPER END OR DOUBLE MPH GROUP B 4.8 WIND 4. 9. (0) GROUP A 5 10 SPEED, GROUP B REFER TO 30' THE PRINTE GROUP A 9' 10" 9, 10, 7' 10" CHART ABOVE FOR MAX GABLE VERTICAL LENGTH MEAN 10,0 EX4 MEN OR BETTER CONTINUOUS BEARING GROUP B 9, 10, 8, 10, 10, 1 10' 7' 10' 4' 7' 9' 9' B 9 5 7 7 3 8 11 ° HEIGHT, 10, 10 • ULIUS LEI cons. Engineers (1) 2X6 12. 10. DELRAY BEACH, FL 33444-2161 12' 11" 12' 11" GROUP A GROUP B 10' 3" 10 10 No: 34868 STATE OF FLORIDA 1 r; ENCLOSED, 12 BRACE • (2) 2XB 12' B" 12' B" NA. GROUP A 10, 10, 12' 3" 14' 0" ō ď. MAX. MAX. GROUP BRACE 12, 8, 11 14" 0" 13' 2' 12' 3" 10' 7" 10, 10, TOT. SPACING 1.00, E ATTACH EACH 'L' BRACE WITH 104 WAILS. \$ FOR (1) 'L' BRACE: SPACE NAILS AT 2" O.C. \$ FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. IN 18" EYD ZONES AND 6" O.C. BETWEEN ZONES. IN 18" EYD ZONES AND 6" O.C. BETWEEN ZONES. CABLE END SUPPORTS LOAD FROM 4. 0" OUTLOOKERS WITH 2. 0" OVERHANG, OR 12" PROVIDE UPLAT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LIVE LOAD DEPLECTION CHITERIA IS L/240. MEMBER LENGTH. DOUGLAS FIR-LARCH #3 STUD STANDARD PLYWOOD OVERHANG. BRACING GROUP SPECIES USENTAL LENGTH 128S THAN 4.0° 1 GREATER THAN 4.0° 1 LESS THAN 11' 6" BRACING MUST BE A MINIMUM OF 80% OF WEB \$PRUCE-PINE-IVE \$1 / \$2 STANDARD \$3 STUD CABLE TRUSS EXPOSURE 60 SOUTHERN PINE 24.0 PEAK, SPLICE, AND HEEL PLATES. CABLE VERTICAL PLATE SIZES PSF REF DATE DWG MITEK STD GABLE 30' E HT GROUP HEM-FIR FI & BIR GROUP DETAIL NOTES: 0 11/26/03 DOUGLAS FIR-LARCH Ä ASCE7-02-GAB13030 SOUTHERN PINE A. 2 2 NO SPLICE 2.5%4 AND STANDARD HEM-PIR 274 STANDARD GRADES:



TOP CHORD 2X4 2X4 2X4 #2 OR BETTER #2 OR BETTER #3 OR BETTER

PIGGYBACK DETAIL

TYPE

30,

34

88 5

52

SPANS ПÞ

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OF THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENCINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HCT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TO DL-5 PSF, WIND BC DL-5 PSF 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C. WIND TO DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C. WIND TC DL=5 PSF, WIND HC DL=5 PSF

M

8 584

OR 3X6 TRULOX AT 4'

20

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5X5

5X5

1.5X3

1.5X4

1.5X4

1.5X4 5X6

4X8 284

5X6

5X8

5X6 3X5

2.5X4

2.5X4

FRONT FACE (5,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. ACCEPTABLE OCATION IS MAX C 20' FLAT TOP CHORD MAX SPAN TYP. W 8 MAX SIZE OF ZXIZ C-TYP. D-SPLICE

INFOR	EQUAL
MATION.	PER F
J. KEP	ACE PER
EK TO I	R PLY.
RAWING	(8) 0.12 (4) NAIL
160	ILS IN I
TL FOR T	ACH M
TRULOX	MBER 1
	ರಸ

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4" OC OR LESS. * PIGGYBACK SPECIAL PLATE

N

C

n C

8 1/4"

				*ATTACH
		THESE FUNCTIONS. UNLESS OF REMINER HE 13/19 HE SALL HAVE A PROPERT A FIRCHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERTY A FIRCHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERTY A FIRCHED STRUCTURAL PARELS.	PACING REFER TO BOX INDICATED BY THE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BACING, REFER TO BOX I HOLD GUILDING COMPONENT SAMELY INFORMATION), PLEISTED BY THE CRIESS COLUMNIC THREE MEDICAL COLUMNIA COLUMNIA CARE INSTALLING AND INCIDENCE AND	*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.
STATE OF FLORIDA		DELEAY BEACH, FL 33444-2161	JULIUS LEE'S cons. ENGINEERS P.A.	THIS DRAW
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	PSF	MAX LOADING 55 PSF AT	ING REPLACES DRAWINGS
		-ENG JL PIGGY	REF PIGCYBACK DATE 09/12/07	THIS DRAWING REPLACES DRAWINGS 634,016 834,017 & 847,045

VALLEYTRUSS DETAIL

TOP CHORD BOT CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
2X4 SP #3 OR BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d ASCE 7-02 130 MPH WIND. 15' MEAN HEICHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. FOR

EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9". UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING,

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

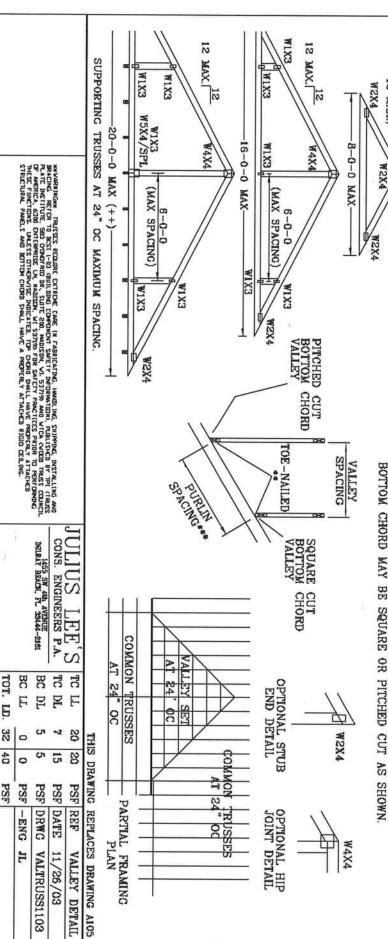
CUT FROM 2X6 OR LARGER AS REQ'D

4-0-0 MAX

12 MAX.

+ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



No: 34869 STATE OF FLORIDA

SPACING

24" 1.25

DUR.FAC. 1.25

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

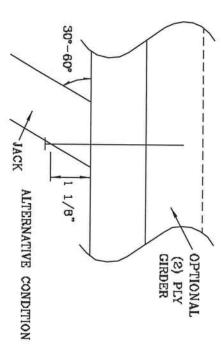
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

ALL VALUE	5	.4	ယ	N	I OE-NAILS	NUMBER OF	
'S MAY BE	493#	394#	296#	187#	1 PLY	SOUTHE	
ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.	639#	511#	383#	256#	2 PLIES	SOUTHERN PINE	
D BY APP	452#	361#	271#	181#	1 PLY	DOUGLAS	
ROPRIATE	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH	
DURATION	390#	312#	234#	156#	1 PLY		
OF LOAD F	507#	406#	304#	203#	2 PLIES	HEM-FIR	
ACTOR	384#	307#	230#	154#	1 PLY	SPRUCE	
	496#	397#	298#	189#	2 PLIES	SPRUCE PINE FIR	

1 1/8 GIRDER

JACK 30°



THIS DRAWING REPLACES DRAWING 784040

			STRUCTURAL PANELS A	DE ANERICA, 6300 ENTER	BRACING. REFER TO B	
			STRUCTURAL PANELS AND BUTTON CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CCILING	. UNLESS CITERAUSE INDICATED, TOP CHORD SHALL HAVE REPORTLY ATTACHED.	REDUJRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, SI 1-03 CIVILIDNG COMPONENT SAFETY (NEURNATION), PUBLISHE	READOUR COLOR
STATE OF FLORIDA	No: 34868			DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	1.00	PSF	PSF	PSF	PSF	PSF
			-ENG JL	DRWG	DATE	REF
			JL	CNTONAIL1103	09/12/07	TOE-NAIL

DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL T0GRAIN.

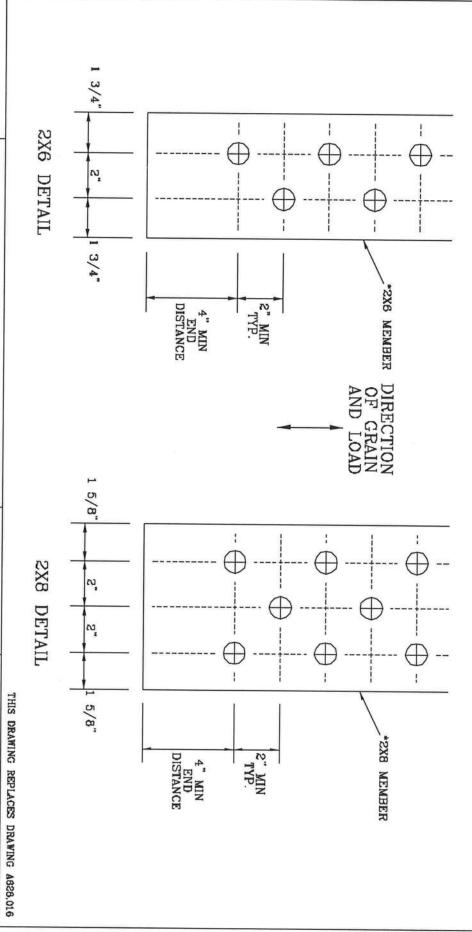
* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



c CONS. DELRAY BEACH, FL 33444-2161 No: 34869 STATE OF FLORIDA IUS LEE'S TC LL BC DL BC LL TOT. SPACING DUR. FAC. U PSF PSF PSF PSF PSF REF DATE DRWG 11/26/03 T CNBOLTSP1103 BOLT SPACING

TRULOX CONNECTION DETAIL

II GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (+).

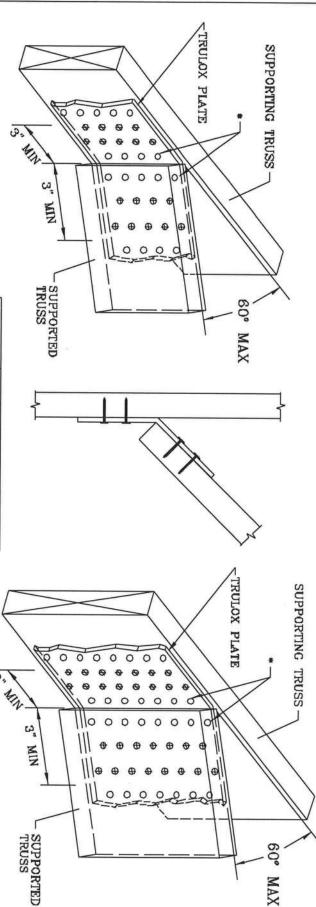
* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-F

THIS DETAIL MAY BE USED WITH SO. PINE. DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE

REQUIRED NAILS PER TRUSS

MAXIMUM LOAD UP OR DOWN

MINIMUM 5X6 TRULOX PLATE

3X6

15

350#

ULIUS LEE'S cons. Engineers P.A. 1455 SW 444, AVENUE DELEAN BOOK, PL. 33444-2161

THIS DRAWING REPLACES DRAWINGS 1.158,989 1.158,989/R 1,154,844 1.152,217 1,152,017 1,159,154 & 1,151,524

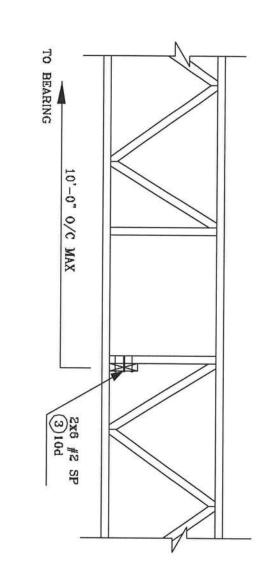
DATE

TRULOX 11/26/03

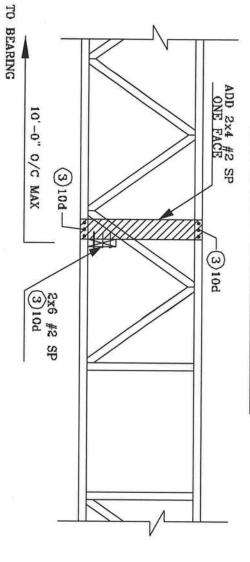
DRWG CNTRULOX1103

No: 34869 STATE OF FLORIDA TRUSSES REQUIRE EXTROME CARE IN FABRICATING, HANDLING, SHIPPIN TO INCID 19-01 (BUILDING COMPUNCYT SAFETY DAFBRATION, PUBLIS TI, SAS DYDNITHI DR. SUITE ROW, MAISTON, V.T. 32759 AND VITOA N. DID ONLING PRICE SHIPPING WAS UNCES CHERRYIES WILDIATER, TIP DATORD SHALL HAVE PROPER SHIPPING MAISTON WELS DIRECTION CHORD SHALL HAVE PROPERLY ATTACHED RIGID WELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED RIGID.

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS

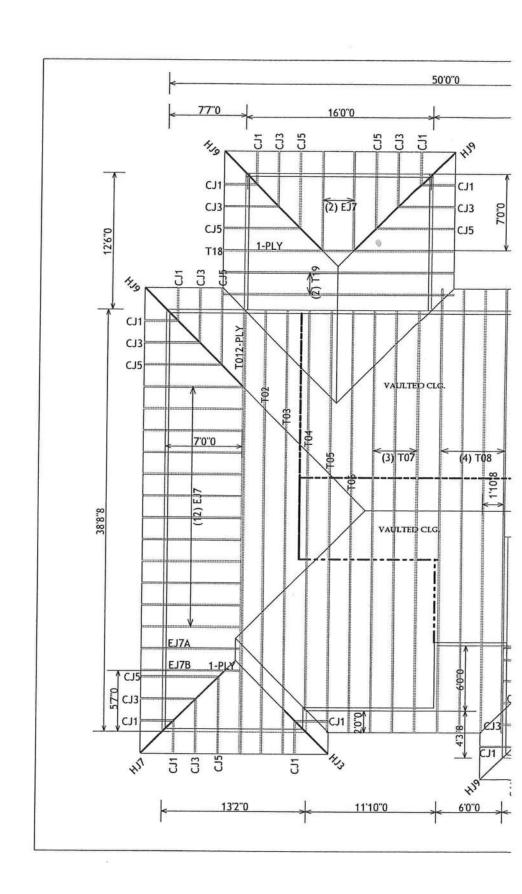


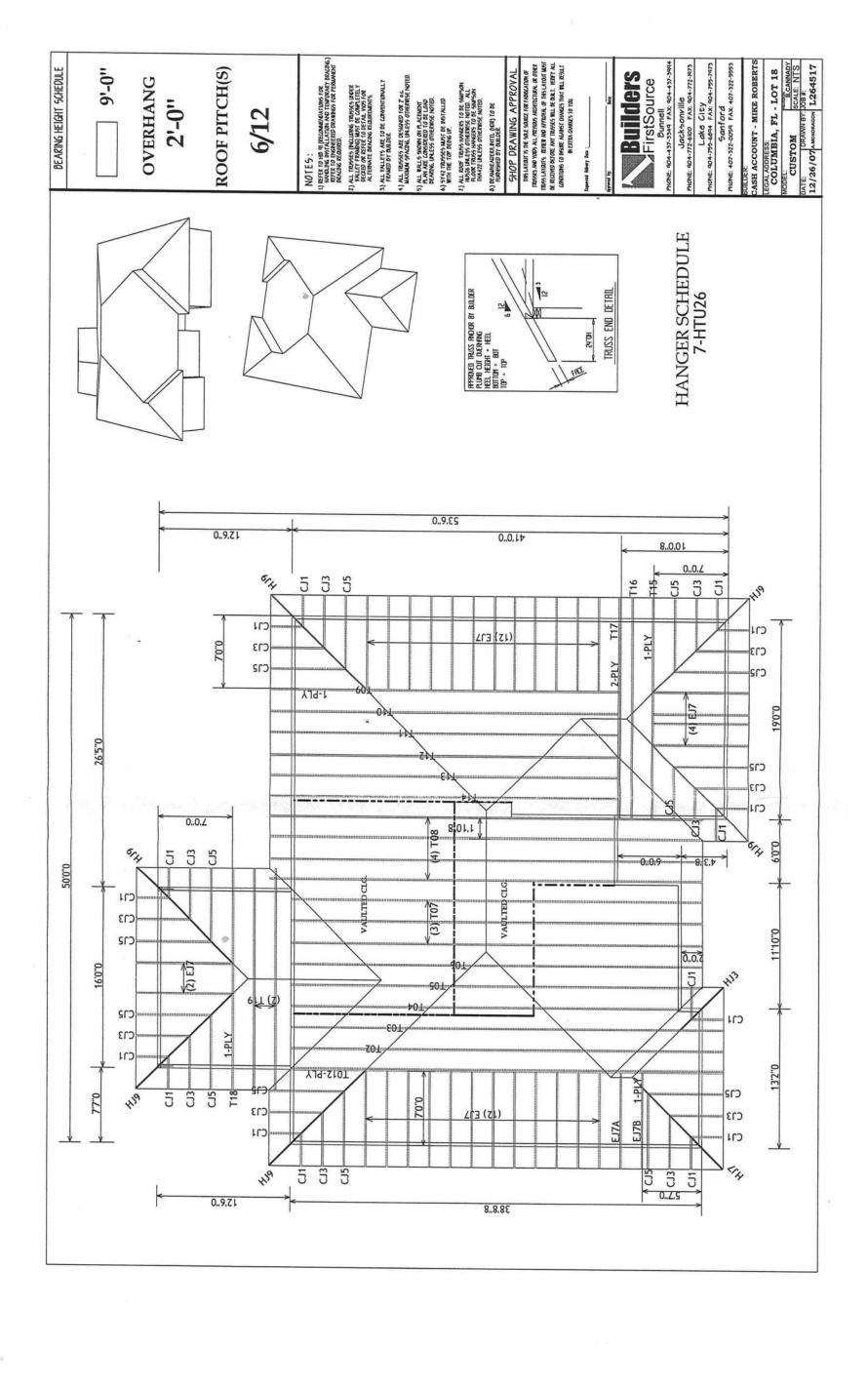
ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



3	D ₈
1456 SW 411	JLII CONS.
SW 4	ENCIN
TH AVENUE	222
AJINA	LE]
0.000000	P.F.

No: 34869 STATE OF FLORIDA













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Product Approval Menu > Product or Application Search > Application List > Application Detail

NG & COMMUNITY OPMENT

FL#

Application Type Code Version

Application Status

Comments

Archived

FL4645 New

2004

Approved

Product Manufacturer Address/Phone/Email

C.H.I. Overhead Doors 1485 Sunrise Drive Arthur, IL 61911 (217) 543-2135 ext 4309

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Authorized Signature

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Address/Phone/Email

Jerod Price

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Arthur, IL 61911 (217) 543-2135 jprice@chiohd.com

Category

Exterior Doors

Subcategory

Sectional Exterior Door Assemblies

Compliance Method

Evaluation Report from a Florida Registered Architect or a

Licensed Florida Professional Engineer Evaluation Report - Hardcopy Received

Florida Engineer or Architect Name

who developed the Evaluation Report Florida License

Quality Assurance Entity

Validated By

John E. Scates, P.E.

PE- 51737

Architectural Testing, Inc. Gordon Thomas, P.E.

Certificate of Independence

Referenced Standard and Year (of Standard)

Standard

Year

ANSI/DASMA 108-2002	2002
ASTM D 1929	2001
ASTM D 2843	1999
ASTM E 330-02	2002

Equivalence of Product Standards Certified By

Sections from the Code

Method 1 Option D
06/09/2005
08/01/2005
06/20/2005
08/05/2005

	PTID 464	25 T Z5-1007-03000s.pdf 25 T Z5-1607-04000s.pdf 25 T Z5-1807-02000s.pdf 25 T Z5-1807-03000s.pdf 25 T Z6-0907-04000s.pdf 25 T Z6-1007-02000s.pdf 25 T Z6-1607-04000.pdf 25 T Z6-1807-02000s.pdf 25 T Z7-0907-04000.pdf 25 T Z7-1007-01000s.pdf 25 T Z7-1007-01000s.pdf 26 T Z7-1007-02000s.pdf 27 T Z7-1007-03000s.pdf 28 T Z7-1007-03000s.pdf 29 T Z7-1607-04000.pdf
impact rated Design	Other) in HVHZ: outside HVHZ:	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation Installation Instructions Verified By: Evaluation Reports
impact rated Design	Other) in HVHZ: outside HVHZ:	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation Installation Instructions Verified By: Evaluation Reports
impact rated Design	Other) in HVHZ: outside HVHZ:	26 ga. ext. min. 27 ga. int. min with foamed in place polyurethane insulation Installation Instructions Verified By: Evaluation Reports
impact rated Design	Other) in HVHZ: outside HVHZ:	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation Installation Instructions Verified By: Evaluation Reports
Limits of Use (See Approved for use Approved for use Impact Resistant Design Pressure: Other: C.H.I. Draw impact rated Design	in HVHZ: outside HVHZ:	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation Installation Instructions Verified By: Evaluation Reports

4645.7	was a second of the second of	Marca Control of the
1043.7	Model: 2216, 2217, 4216 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See		Installation Instructions
Approved for use Approved for use		Verified By:
Impact Resistant		Evaluation Reports
Design Pressure		
Other: C.H.I. Drav	wing: Z5-1807-03000 Non	
	n Load: +25.9 / -28.8 Test	
Load: +38.9 / -43.	2 16'-1" thru 18'-0" wide	
4645.8	Model: 2216, 2217, 4216 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See	Other)	Installation Instructions
Approved for use		Verified By:
Approved for use		Evaluation Reports
Impact Resistant Design Pressure:		11
	wing: Z4-1807-03000 Non	
	n Load: +22.0 / -24.5 Test	
	8 16'-1" thru 18'-0" wide	
4645.9	Model: 2216, 2217, 4216	26 ga. min. ext. and 27 ga.min. int. with foamed
1043.9	and 5216	in place polyurethane insulation
Limits of Use (See		Installation Instructions
Approved for use		Verified By:
Approved for use		Evaluation Reports
Impact Resistant		•
Design Pressure:		
	wing: Z1-1807-03000 Non	
	n load: +12.4 / -13.8 Test ' 16'-1" thru 18'-0" wide	
1044. 110.07 20.7	10 1 till 10 0 wide	
4645.10	Model: 2216, 2217, 4216 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See		Installation Instructions
Approved for use		Verified By:
Approved for use	outside HVHZ:	Evaluation Reports
Impact Decictant	•	
Impact Resistant Design Pressure:		19
Design Pressure:	: +/-	2
Design Pressure: Other: C.H.I. Draw impact rated Design	+/- ving: Z2-1807-03000 Non n load: +15.3 / -17.0 Test	2
Design Pressure: Other: C.H.I. Draw impact rated Design	: +/- ving: Z2-1807-03000 Non	
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5	+/- ving: Z2-1807-03000 Non n load: +15.3 / -17.0 Test i 16'-1" thru 18'-0" wide	26 ga. ext. min. 27 ga. int. min. with foamed in
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5	he +/- ving: Z2-1807-03000 Non n load: +15.3 / -17.0 Test i 16'-1" thru 18'-0" wide Model: 2216, 2217, 4216 and 5216	place polyurethane insulation
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5	Model: 2216, 2217, 4216 and 5216	place polyurethane insulation Installation Instructions
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use	model: 2216, 2217, 4216 and 5216 c Other) in hydronic in hydronic	place polyurethane insulation
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use Impact Resistant	ming: Z2-1807-03000 Non in load: +15.3 / -17.0 Test is 16'-1" thru 18'-0" wide Model: 2216, 2217, 4216 and 5216 Other) in HVHZ: coutside HVHZ:	place polyurethane insulation Installation Instructions Verified By:
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use Impact Resistant Design Pressure:	H/- ving: Z2-1807-03000 Non load: +15.3 / -17.0 Test 16'-1" thru 18'-0" wide Model: 2216, 2217, 4216 and 5216 Other) in HVHZ: coutside HVHZ: ith H/-	place polyurethane insulation Installation Instructions Verified By:
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Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use Approved Resistant Design Pressure: Other: C.H.I. Draw impact rated Design	model: 2216, 2217, 4216 and 5216 c Other) c in HVHZ: c outside HVHZ: t +/- ving: Z3-1807-03000 Non n load: +18.5 / -20.7 Test	place polyurethane insulation Installation Instructions Verified By:
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use Impact Resistant Design Pressure: Other: C.H.I. Draw impact rated Design load: +27.8 / -31.1	# +/- ving: Z2-1807-03000 Non in load: +15.3 / -17.0 Test is 16'-1" thru 18'-0" wide Model: 2216, 2217, 4216 and 5216 Other) in HVHZ: in outside HVHZ: i:	place polyurethane insulation Installation Instructions Verified By: Evaluation Reports
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use Approved Resistant Design Pressure: Other: C.H.I. Draw impact rated Design	model: 2216, 2217, 4216 and 5216 c Other) c in HVHZ: c outside HVHZ: t +/- ving: Z3-1807-03000 Non n load: +18.5 / -20.7 Test	place polyurethane insulation Installation Instructions Verified By:
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use Impact Resistant Design Pressure: Other: C.H.I. Draw impact rated Design load: +27.8 / -31.1 4645.12 Limits of Use (See	# +/- ving: Z2-1807-03000 Non in load: +15.3 / -17.0 Test is 16'-1" thru 18'-0" wide Model: 2216, 2217, 4216 and 5216 Other) in HVHZ: outside HVHZ: iiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiii	place polyurethane insulation Installation Instructions Verified By: Evaluation Reports Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation Installation Instructions
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use Impact Resistant Design Pressure: Other: C.H.I. Draw impact rated Design load: +27.8 / -31.1 4645.12 Limits of Use (See Approved for use	# +/- ving: Z2-1807-03000 Non in load: +15.3 / -17.0 Test is 16'-1" thru 18'-0" wide Model: 2216, 2217, 4216 and 5216 Other) in HVHZ: outside HVHZ: it is	Installation Instructions Verified By: Evaluation Reports Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation Installation Instructions Verified By:
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Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use Impact Resistant Design Pressure: Other: C.H.I. Draw impact rated Design load: +27.8 / -31.1 4645.12 Limits of Use (See Approved for use Impact Resistant Design Pressure: Other: C.H.I.Draw impact rated Design	model: 2216, 2217, 4216 and 5216 c Other) in load: +18.5 / -20.7 Test outside HVHZ: in HVHZ:	Installation Instructions Verified By: Evaluation Reports Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation Installation Instructions Verified By:
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use Impact Resistant Design Pressure: Other: C.H.I. Draw impact rated Design load: +27.8 / -31.1 4645.12 Limits of Use (See Approved for use Approved for use Impact Resistant Cother: C.H.I.Draw impact Resistant Design Pressure: Other: C.H.I.Draw impact rated Design Load: +40.4 / -46.2	model: 2216, 2217, 4216 and 5216 c Other) in load: +18.5 / -20.7 Test in HVHZ: coutside HVHZ: in HVHZ: in HVHZ: coutside HVHZ: in	Installation Instructions Verified By: Evaluation Reports Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation Installation Instructions Verified By: Evaluation Reports
Design Pressure: Other: C.H.I. Draw impact rated Design load: +23.0 / -25.5 4645.11 Limits of Use (See Approved for use Approved for use Impact Resistant Design Pressure: Other: C.H.I. Draw impact rated Design load: +27.8 / -31.1 4645.12 Limits of Use (See Approved for use Impact Resistant Design Pressure: Other: C.H.I.Draw impact rated Design	model: 2216, 2217, 4216 and 5216 c Other) in load: +18.5 / -20.7 Test in HVHZ: coutside HVHZ: HV	Installation Instructions Verified By: Evaluation Reports Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation Installation Instructions Verified By:

Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: C.H.I. Drawing: Z1-1007-01000 Non impact rated Design load: +12.8 / -14.8 Test load: +19.2 / -22.2 9'-1" thru 10'-0" wide	Installation Instructions Verified By: Evaluation Reports
Model: 2250, 2251, 2240 and 2241 Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: C.H.I. Drawing: Z2-1007-01000 Non impact rated Design load: +15.9 / -18.2 Test load: +23.9 / -27.3 9'-1" thru 10'-0" wide	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation Installation Instructions Verified By: Evaluation Reports
A645.15 Model: 2250, 2251, 2240 and 2241 Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: C.H.I. Drawing: Z4-1007-01000 Non impact rated Design load: +22.9 / -26.3 Test load: +34.4 / -39.5 9'-1" thru 10'-0" wide	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation Installation Instructions Verified By: Evaluation Reports
Model: 2250, 2251, 2240 and 2241 Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: C.H.I. Drawing: Z7-1007-01000 Non impact rated Design load: +35.7 / -41.0 Test load: +53.6 / -61.5 9'-1" thru 10'-0" wide	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation Installation Instructions Verified By: Evaluation Reports
Model: 2250, 2251, 4250, 4251, 2240, 2241, 4240, 4241, 5240 and 5241 Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: C.H.I. Drawing: Z1-1807-01000 Non impact rated Design Load: +12.4 / -13.8 Test Load: +18.6 / -13.8 16'-1" thru 18'-0" wide	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation Installation Instructions Verified By: Evaluation Reports
Model: 2250, 2251, 4250, 4251, 2240, 2241, 4240, 4241, 5240 and 5241 Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: C.H.I. Drawing: Z5-0907-01000 Non impact rated Design Load: +26.9 / -30.8 Test Load: +40.4 / -46.2 Thru 9'-0" wide	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation Installation Instructions Verified By: Evaluation Reports
Model: 2283, 2284, 2285 and 2286	27 ga. int. min. 27 ga. ext. min. with polystyrene insulation

Installation Instructions Limits of Use (See Other) Approved for use in HVHZ: Verified By: **Evaluation Reports** Approved for use outside HVHZ: **Impact Resistant:** Design Pressure: +/-Other: C.H.I. Drawing: Z2-1007-02000 Non impact rated Design load: +15.9 / -18.2 Test load: +23.9 / -27.3 9'-1" thru 10'-0" wide 4645.20 Model: 2283, 2284, 2285 27 ga. int. min. 27 ga.ext. min. with polystyrene and 2286 insulation Limits of Use (See Other) Installation Instructions Approved for use in HVHZ: Verified By: Approved for use outside HVHZ: **Evaluation Reports Impact Resistant:** Design Pressure: +/-Other: C.H.I. Drawing: Z3-1007-02000 Non impact rated Design load: +19.2 / -22.0 Test load: +28.8 / -33.0 9'-1" thru 10'-0" wide 0 O Page 1 / 3 O O Go to Page

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DCA Administration

Department of Community Affairs Florida Building Code Online Codes and Standards 2555 Shumard Oak Boulevard

Tallahassee, Florida 32399-2100 (850) 487-1824, Suncom 277-1824, Fax (850) 414-8436 © 2000-2005 The State of Florida. All rights reserved. Copyright and Disclaimer

Product Approval Accepts:











Job:

Date: May 16, 2007

By:

P O Box 1945, Alachua, FI 32616 Phone: 352-225-1308 Fax: 386-418-0549

Project Information

For:

Mike Roberts

Design Information							
	Htg	Clg		Infiltration			
Outside db (°F)	33	92	Method	Simplified			
Inside db (°F)	70	75	Construction quality	Average			
Design TD (°F)	37	17	Fireplaces	1 (Average)			
Daily range	-	M					
Inside humidity (%)	-	50					
Moisture difference (gr/lb)	-	52		*			

HEATING EQUIPMENT

COOLING EQUIPMENT

Make Trade Model	York Guarden HP030X1321			Make Trade Cond Coil	York Guarden HP030X1321 G2FD036S17+	ITV0701	
Efficien	cv	8 HSPF		Efficiency	021 0000011	13 SEER	
Heating				Sensible c	ooling	19600	Btuh
Heating		30000	Btuh @ 47°F	Latent coo		8400	Btuh
	ature rise	29	°F	Total cooli	ng .	28000	Btuh
Actual a		933	cfm	Actual air f	low	933	cfm
Air flow		0.035	cfm/Btuh	Air flow fac	ctor	0.048	cfm/Btuh
Static p		0.10	in H2O	Static pres	sure	0.10	in H2O
	hermostat			Load sens	ible heat ratio	0.69	19

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
Dining rm	131	3391	1820	119	87
Kitchen	108	1408	2284	50	109
Bath	52	975	495	34	24
Bedrm 2	182	5143	3325	181	159
Bedrm 3	186	3195	2343	112	112
Fam rm	357	4398	4219	155	202
Master bedrm	219	3379	3010	119	144
Master bath	78	2926	1128	103	54
W.I.c.	48	1531	569	54	27
core	132	189	314	7 1	15

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Entire House Other equip loads Equip. @ 0.97 RSM Latent cooling	1494	26536 5651	19507 2596 21440 9979	933	933
TOTALS	1494	32187	31419	933	933

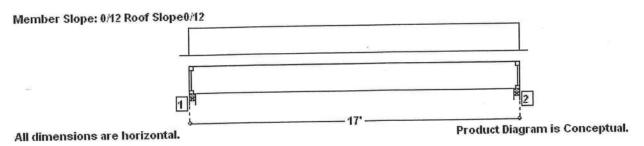


GARAGE DOOR BEAM

2 Pcs of 1 3/4" x 11 7/8" 1.9E Microllam® LVL

User: 2 6/20/2007 8:11:53 AM Page 1 Engine Version: 6.25.71

THIS PRODUCT MEETS OR EXCEEDS THE SET DESIGN CONTROLS FOR THE APPLICATION AND LOADS LISTED



LOADS:

Analysis is for a Drop Beam Member. Tributary Load Width: 1'

Primary Load Group - Roof (psf): 20.0 Live at 125 % duration, 15.0 Dead

Vertical Loads:

Class Type Roof(1.25) 99.0 Uniform(plf)

Dead 40.0

Application Location Adds To 0 To 17'

Comment

EJ3 LOADS

SUPPORTS:

3011 01110.		Input Width		Vertical Reactions (lbs) Live/Dead/Uplift/Total	Detail	Other
1	Stud wall	ele-batana	1.50"	1012 / 565 / 0 / 1577	L1: Blocking	1 Ply 1 3/4" x 11 7/8" 1.9E Microllam® LVL
2	Stud wall	3.50"	1.50"	1012 / 565 / 0 / 1577	L1: Blocking	1 Ply 1 3/4" x 11 7/8" 1.9E Microllam® LVL

⁻See TJ SPECIFIER'S / BUILDERS GUIDE for detail(s): L1: Blocking

DESIGN CONTROLS:

DESIGN CONTROL	Maximum	Design	Control	Control	Location
Shear (lbs)	1546	-1339	9871	Passed (14%)	Rt. end Span 1 under Roof loading
Moment (Ft-Lbs)	6440	6440	22310	Passed (29%)	MID Span 1 under Roof loading
Live Load Defl (in)		0.235	0.556	Passed (L/852)	MID Span 1 under Roof loading
Total Load Defl (in)		0.366	0.833	Passed (L/547)	MID Span 1 under Roof loading

⁻Deflection Criteria: Specified(LL:L/360,TL:L/240).

ADDITIONAL NOTES:

Operator Notes:

GDB

PROJECT INFORMATION:

L229281 MIKE ROBERTS GARAGE DOOR BEAM

OPERATOR INFORMATION:

Kimber Holloway **Builders FirstSource** 2525 East Duval Street Lake City,, FL 32055 Phone: 386-755-6894

: 386-755-7973 Fax

kim.holloway@buildersfirstsource.com

⁻Bracing(Lu): All compression edges (top and bottom) must be braced at 17' o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

 ⁻Design assumes adequate continuous lateral support of the compression edge.

⁻IMPORTANT! The analysis presented is output from software developed by Trus Joist (TJ). TJ warrants the sizing of its products by this software will be accomplished in accordance with TJ product design criteria and code accepted design values. The specific product application, input design loads, and stated dimensions have been provided by the software user. This output has not been reviewed by a TJ Associate.

⁻Not all products are readily available. Check with your supplier or TJ technical representative for product availability.

⁻THIS ANALYSIS FOR TRUS JOIST PRODUCTS ONLY! PRODUCT SUBSTITUTION VOIDS THIS ANALYSIS.

⁻Allowable Stress Design methodology was used for Building Code UBC analyzing the TJ Distribution product listed above.

⁻Note: See TJ SPECIFIER'S / BUILDER'S GUIDES for multiple ply connection.

Community Affairs





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Product Approval
USER: Public User

<u>Product Approval Menu</u> > <u>Product or Application Search</u> > <u>Application List</u> > **Application Detail**

FCOMMUNITY PLANNING

PHOUSING & COMMUNITY DEVELOPMENT

PEMERGENCY

MANAGEMENT

▶ OFFICE OF THE

FL#

Application Type

Code Version

Application Status

Comments

Archived

FL1214-R1

Revision

2004

Approved

Product Manufacturer

Address/Phone/Email

Alenco

615 Carson

Bryan, TX 77802

(979) 779-7770 ext 343 mkoppers@alenco.com

Authorized Signature

Martin Koppers

mkoppers@alenco.com

Technical Representative

Address/Phone/Email

Martin Koppers

615 Carson St.

Bryan, TX 77802

mkoppers@alenco.com

Quality Assurance Representative

Address/Phone/Email

Category

Subcategory

Windows

Single Hung

Compliance Method

Certification Mark or Listing

Certification Agency

National Accreditation & Management Institute,

Referenced Standard and Year (of

Standard

Standard)

AAMA/NWWDA 101/I.S.2

Equivalence of Product Standards Certified By

Sections from the Code

1707.4.2.1

Product Approval Method

Method 1 Option A

Date Submitted

06/08/2005

Date Validated

08/04/2005

Date Pending FBC Approval

06/18/2005

Date Approved

08/05/2005

Summary of Produ	ıcts	
FL#	Model, Number or Name	Description
1214.1	1111	Vinyl Tilt Single Hung
annealed,44X72 R(e in HVHZ: e outside HVHZ: t:	Certification Agency Certificate Installation Instructions PTID 1214 R1 I FL INSTALLATION INSTRUCTIONS - Aluminum B.pdf PTID 1214 R1 I INSTALLATION INSTRUCTIONS - Vinyl B.pdf Verified By:
1214.2	3753	Aluminum Tilt Single Hung
Limits of Use (See Approved for use Approved for use Impact Resistant Design Pressure: Other: 3753:44X73 with DS annealed.For glass to comply with	in HVHZ: outside HVHZ: : +/- 2 R(40) Tested with Tested or smaller window sizes.	Certification Agency Certificate Installation Instructions Verified By:
	4710F	Aluminum Single Hung
Limits of Use (See Approved for use Approved for use Impact Resistant: Design Pressure: Other: 4710F:48X7 DS annealed glass.For glass to comply with	in HVHZ: outside HVHZ: +/- 22 R(40)/DP(50), Tested with or smaller window sizes.	Certification Agency Certificate Installation Instructions Verified By:

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Department of Community Affairs Florida Building Code Online Codes and Standards

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Product Approval Accepts:





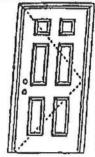






WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Hole:

Units of other sizes are covered by this report as long as the panel used does not exceed 3 0" x 6'8".

Single Door Design Pressure

+66.0/-66.0

Large Missils Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED

AT IN THE PARTY OF MAIN PROPERTY OF A PARTY OF A PARTY OF THE PARTY OF THE REAL PARTY OF THE PAR

MINIMUM ASSEMBLY DETAIL:

Compliance recurses that minimum assembly paralle have been forcibled - see MAID AN MACCON ID

MINIMUM INSTALLATION DETAIL:

Compliance recurred that minimum legislay on one its Asia, been to to see in see Mid. As. Military 127.

APPROVED DOOR STYLES.





























WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold

PRODUCT COMPLIANCE LABELING:

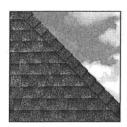
TESTED IN ACCORDANCE WITH MIAMI-DADE BCCO FACOL PAZOL & PAZOL

BRAK TRASECO C D 3'1:1

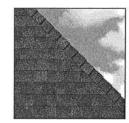
To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Frolessional Engineer Kurt Baltnazor, P.E. - License Number 56533





PRESTIQUE® HIGH DEFINITION®



RAISED PROFILE®

Prestique Plus High Definition and Prestique Gallery Collection™

13%'x 39%" Product size Exposure _ F5/2 Pieces/Bundle Bundles/Square ___ _4/98.5 sq.ft. Squares/Pallet 11

50-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 110 mph*** Product size 13%'x 38%" Exposure . FI/E Pieces/Bundle 22 Bundles/Square __ _3/100 sq.ft. Squares/Pallet 16

Raised Profile

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 70 mph.

Prestique I High Definition

Product size	13% x 39%
Exposure	5%°
Pieces/Bundle .	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 90 mph***

HIP AND RIDGE SHINGLES Seal-A-Ridge® w/FLX™

Size: 12"x 12" Exposure: 6%" Pieces/Bundle: 45 Coverage: 4 Bundles = 100 linear feet Vented RidgeCrest™ w/FLX™

Size: 13"x13%" Exposure: 91/4" Pieces/Box: 26 Coverage: 5 boxes = 100 linear feet

Prestique High Definition

Product size 131/4'x 381/4" Exposure _ 5% Pieces/Bundle _ 22 Bundles/Square __ 3/100 sq.ft. Squares/Pallet 16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph.

Elk Starter Strip 52 Bundles/Pallet 18 Pallets/Truck 936 Bundles/Truck 19 Pieces/Bundle

1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood, Forest Green, Wedgewood, Birchwood, Sandalwood. Gallery Collection: Balsam Forest*, Weathered Sage*, Sienna Sunset*.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard* treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL* Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBO, and Texas Department of Insurance.

"See actual limited warranty for conditions and limitations.
"Effective January 1,2004, the seven year non-prorated Umbrella Coverage Period applies only when a full Elk Roof System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for such products. A full Elik roof system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all rake and eave edges, an Elk ventilation system, and Elk All-Climat Self-Adhering Underlayment in all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment in all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment is required along the rake and eave edges of the roof in and north of the states of VA, KY, MO, KS, CO, UT, NV, & OR.
""For a limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grandé, at least six (6) properly placed NAILS and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.

SPECIFICATIONS

Scope: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula FLX.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

PREPARATION OF ROOF DECK: Roof deck to be dry, wellseasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/6" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard, Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

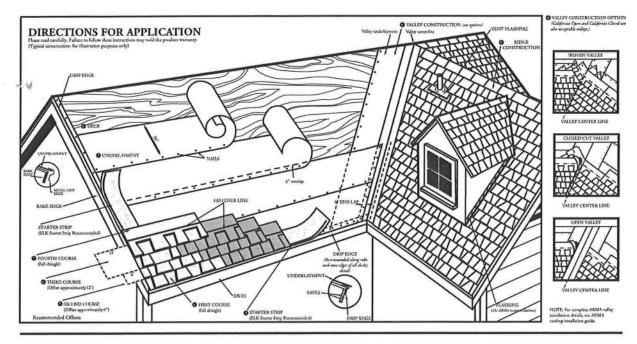
Materials: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater; apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For Low slopes [4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)], use two plies of underlayment overlapped a minimum of 19°. Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula FLX with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.





DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet EIK's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

O DECK PREPARATION

Roof decks should be dry, well-seasoned 1° x 6° boards or exterior grade plywood minimum 3/8° thick and conform to the specifications of the American Plywood Association or 7/16° oriented strandboard, or 7/16° chipboard.

@ UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Elk Versashield® or self adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope(2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19°. Begin by fastening a 19° wide strip of underlayment placed along the eaves. Place a full 36° wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24' beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24' beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

❸ STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3' trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/2' to 3/4'. Fasten 2' from the lower edge and 1' from each side.

O FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof

SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6°. Other offsets are approved if greater than 4°.

THIRD COURSE

Offset the next course by 5° with respect to the second course, or consistent with the original offset.

6 FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

O VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 35° wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6° of valley center.

O RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" Z*Ridge or Seal-A-Ridge* with formula FLX" or RidgeCrest* with FLX (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness common bond area. For shingles without a fastener line, nails or staples must be placed between and/or in the called the ships of the same staples.

NAILS: Corrosive resistant, 3/8' head, minimum 12-gauge roofing nails. Elk recommends 1-1/4' for new roofs and 1-1/2' for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4' ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1' ring shank nails allowed for re-roof. STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16'. Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4* deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

MANSARD APPLICATIONS

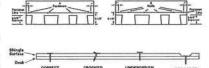
Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1° from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile^{to} shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique, shingles must be applied with 5 properly placed NAILS pershingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more the 3/4 of an inch.

HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the 'fastener line' or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle allowment.



Repair Note 1

FEFAIR Note 1

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Drive another rail nearby. Seel place comerc.

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Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.
All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these

instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.











Affairs



Product Approval Menu > Product or Application Search > Application List > Application Detail

COMMUNITY PLANNING
 HOUSING & COMMUNITY DEVELOPMENT
 EMERGENCY MANAGEMENT
 OFFICE OF THE SECRETARY

FL # FL4090
Application Type New
Code Version 2004
Application Status Approved

Comments Archived

AICHIVE

Product Manufacturer Address/Phone/Email

General American Door 5050 Baseline Rd Montgomery, IL 60538 (630) 859-3000 ext 175 j.campbell@hoermann-gadco.com

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Authorized Signature

James Campbell

j.campbell@hoermann-gadco.com

Technical Representative Address/Phone/Email

Quality Assurance Representative Address/Phone/Email

Category Subcategory **Exterior Doors**

Sectional Exterior Door Assemblies

Compliance Method

Evaluation Report from a Florida Registered Architect or a Licensed Florida Professional Engineer

☑ Evaluation Report - Hardcopy Received

Florida Engineer or Architect Name who developed the Evaluation Report

me Naser R. Keyvan

Florida License

PE- 53774

Quality Assurance Entity

Intertek Testing Services NA Inc

Validated By

John E. Scates, PE

Certificate of Independence

Referenced Standard and Year (of

Standard)

Standard
ANSI / DASMA 108-2002

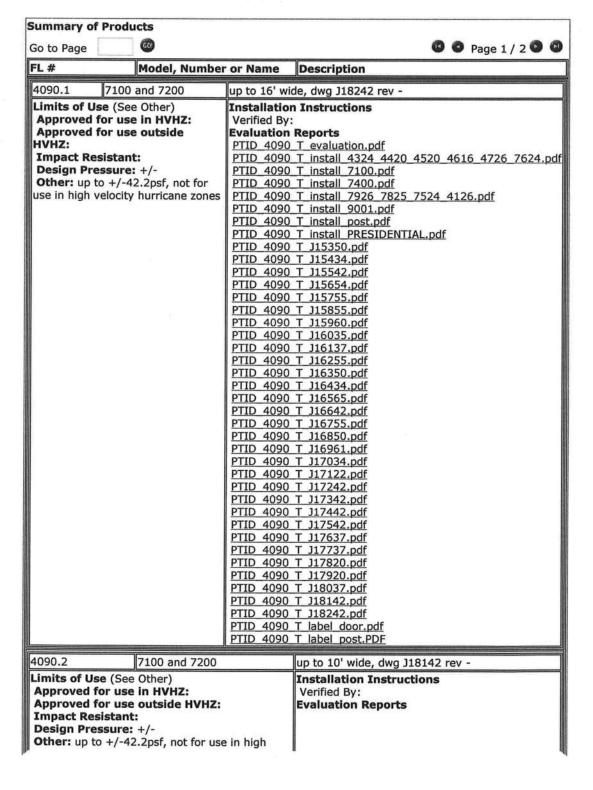
Year 2002

Equivalence of Product Standards Certified By

Sections from the Code

1707.4

Product Approval Method	Method 1 Option D
Date Submitted	02/21/2005
Date Validated	03/03/2005
Date Pending FBC Approval	03/07/2005
Date Approved	03/16/2005



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4090.3	7100 and 7200	up to 10' wide, dwg J18037 rev -
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Impact Resistan	nt:	
Other: up to +/-3	37psf, not for use in high	
velocity hurricane		
4090.4	7100 and 7200	up to 16' wide, dwg J17920 rev -
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Design Pressure		
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4090.5	7825, 7624, 7524, 7400,	up to 16' wide, dwg J17034 rev -
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Other: up to +55,	/-57psf, not for use in high	
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4090.7	7825, 7624, 7524, 7400,	up to 9' wide, dwg J15654 rev -
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	up to 10' wide, dwg J17342 rev -
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Other: up to +/-42.2psf, not for use in high	
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4090.11 Freedom	up to 8' wide, dwg J17242 rev -
Limits of Use (See Other)	Installation Instructions
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Approved for use outside HVHZ:	Evaluation Reports
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Other: up to +/-42.2psf, not for use in high	
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4090.12 Freedom	up to 10' wide, dwg J17637 rev -
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4090.13 Freedom	up to 9' wide dwg 117727 roy -
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4090.18	Presidential	up to 10' wide, dwg J15960 rev -
Impact Resist Design Pressu	use in HVHZ: use outside HVHZ: tant: ure: +/- 60/-64psf, not for use in high	Installation Instructions Verified By: Evaluation Reports
4090.19	Presidential	up to 16' wide, dwg J16255 rev -
Limits of Use (Approved for Approved for Impact Resist	use in HVHZ: use outside HVHZ:	Installation Instructions Verified By: Evaluation Reports
Design Pressu Other: up to +	55/-61psf, not for use in high	
Design Pressu	55/-61psf, not for use in high	up to 16' wide, dwg J15755 rev -

Back Next

DCA Administration

Department of Community Affairs Florida Building Code Online Codes and Standards 2555 Shumard Oak Boulevard

2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436
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Product Approval Accepts:







26687 **Notice of Treatment** Applicator: Florida Pest Control & Chemical Co. (www.flapest.com) Address: BAIA AV Phone City / CROSSWINE Site Location: Subdivision Lot # / 8 Block# Permit # Address Product used **Active Ingredient** % Concentration Premise Imidacloprid 0.1% ☐ Termidor Fipronil 0.12% ☐ Bora-Care Disodium Octaborate Tetrahydrate 23.0% Type treatment: Soil. □ Wood Square feet Gallons Applied Area Treated Linear feet As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval. If this notice is for the final exterior treatment, initial this line Print Technician's Name Covered Ponch NOT TREATED Applicator - White Permit File - Canary Permit Holder - Pink

(C)