DATE 11/01/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000023799

APPLICANT LEVY SAPP PHONE 386.754.5882

ADDRESS 524 NW CARR CT LAKE CITY FL 32055

OWNER MORRELL'S, INC. PHONE 752.3910

ADDRESS 461 SW DEPUTY J. DAVIS LN LAKE CITY FL 32024

CONTRACTOR LEVY SAPP PHONE 754.5882

LOCATION OF PROPERTY 90-W TO W ON J. DEPUTY DAVIS LN, BACK SIDE OF MORRELL'S

TYPE DEVELOPMENT METAL BLDG ESTIMATED COST OF CONSTRUCTION 49500.00

HEATED FLOOR AREA _____ TOTAL AREA 3000.00 HEIGHT 15.80 STORIES 1

FOUNDATION CONC WALLS STEEL ROOF PITCH 1'12 FLOOR CONC

LAND USE & ZONING CHI MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 20.00 REAR 15.00 SIDE 5.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO. _____

PARCEL ID 34-3S-16-02475-001 SUBDIVISION _____

LOT _____ BLOCK _____ PHASE _____ UNIT _____ TOTAL ACRES 5.56

_____	_____	CGC046566	Barry Sapp	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
EXISTING	X-05-0272	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: SDP 05-04. NOC ON FILE

Check # or Cash 3306

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 250.00 CERTIFICATION FEE $ 15.00 SURCHARGE FEE $ 15.00

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $ _____

FLOOD DEVELOPMENT FEE $ _____ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ _____ **TOTAL FEE** 355.00

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

P.O. Box 1625 • Lake City, FL 32056-1625
6919 Distribution Avenue S., Unit #5 • Jacksonville, FL 32257

Tel. (386) 755-3633 • Fax (386) 752-5456
Tel. (904) 262-4046 • Fax (904) 262-4047

JOB NO.: 05-515
DATE TESTED: 11-14-2005
DATE REPORTED: 11-15-2005

REPORT OF IN-PLACE DENSITY TEST 23799

PROJECT:	Proposed Replacement Bldg. & Building Addition
CLIENT:	L & L Construction 524 NW Carr Court, Lake City, Fl 32055
GENERAL CONTRACTOR:	L & L Construction
EARTHWORK CONTRACTOR:	L & L Construction
INSPECTOR:	P Geiger

ASTM METHOD	**SOIL USE**
(D-2922) Nuclear	BUILDING FILL

SPECIFICATION REQUIREMENTS: 95%

TEST NO.	TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft^3)	MOISTURE PERCENT	DRY DENSITY (lb/ft^3)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
7	20'S X 5'E OF NW CORNER	0-12"	106.3	4.1	102.1	2	104.9	97.3%
8	18'W X 10'N OF SE CORNER	0-12"	104.2	3.7	100.5	2	104.9	95.8%

REMARKS: The Above Tests Meet Specification Requirements.

PROCTORS

PROCTOR NO.	SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft^3)	OPT. MOIST.	TYPE
2	Dark Greyish Tan Sand	104.9	11.2	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer

Linda M. Creamer
President - CEO
smw

Reviewed By:

Date: 11/15/05
Florida Registration No: 52612

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

"Excellence in Engineering & Geoscience"

Columbia County Building Permit Application

For Office Use Only Application # 0509-83 Date Received 9/8/05 By G Permit # 23799

Application Approved by - Zoning Official BLK Date 9.12.05 Plans Examiner OK JTH Date 11-1-05

Flood Zone X Development Permit N/A Zoning CHI Land Use Plan Map Category Highway Interch.

Comments - SDP 05-04 -

Applicants Name L & L CONSTRUCTION, LLC Phone 386 754-5882

Address 524 NW CARE CT. LAKE CITY, FL 32055

Owners Name MORRELL'S Phone

911 Address 461 SW DEPUTY J. DAVIS LANE LAKE CITY, FL 32024

Contractors Name LEVY SAPP Phone 386 754-5882

Address 524 NW CARE CT. LAKE CITY, FL 32055

Fee Simple Owner Name & Address MORRELL'S 461 NW DEPUTY J. DAVIS LANE L.C. 32024

Bonding Co. Name & Address NONE

Architect/Engineer Name & Address GTC DESIGN GROUP 130 W HOWARD ST LIVE OAK 32064

Mortgage Lenders Name & Address NONE

Property ID Number 34-3S-16-02475-001 Estimated Cost of Construction $49,500.00

Subdivision Name NONE Lot ____ Block ____ Unit ____ Phase ____

Driving Directions WEST ON US 90 - WEST ON DEPUTY J. DAVIS LANE BACK SIDE OF MORRELL'S

Type of Construction METAL BUILDING Number of Existing Dwellings on Property ____

Total Acreage 5.56 Lot Size ____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 522' Side 700' Side 362' Rear 230'

Total Building Height 15'-8" Number of Stories 1 ~~Heated~~ UNHEATED Floor Area 3,000SF Roof Pitch 1:12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

L & L CONSTRUCTION, LLC
Owner Builder or Agent (Including Contractor)

Levy Sapp
Contractor Signature
Contractors License Number CG-[illegible]66
Competency Card Number

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me
this 28 day of September 20 05.
Personally known ___ or Produced Identification ___

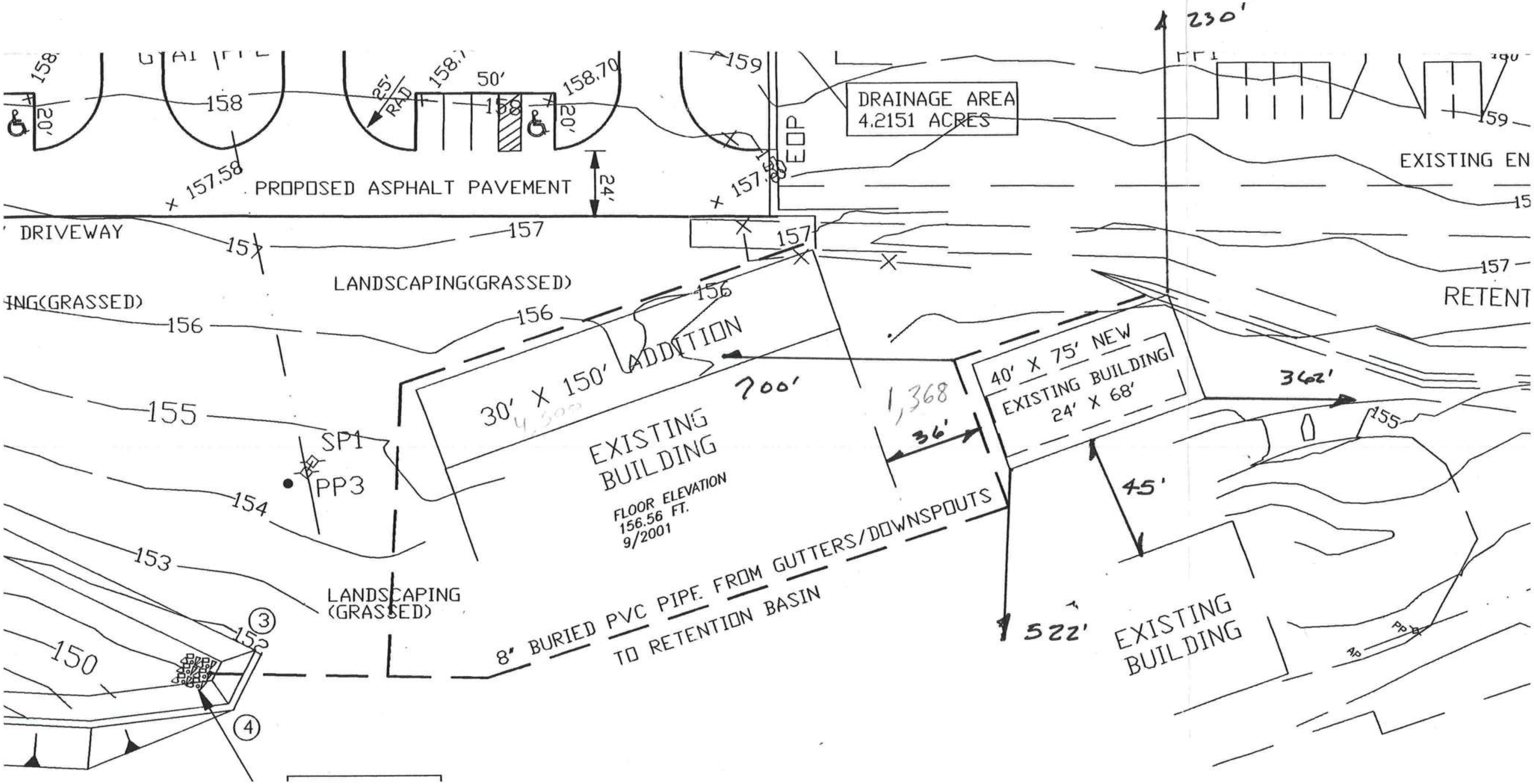
NOTARY STAMP/SEAL

Amber Meads

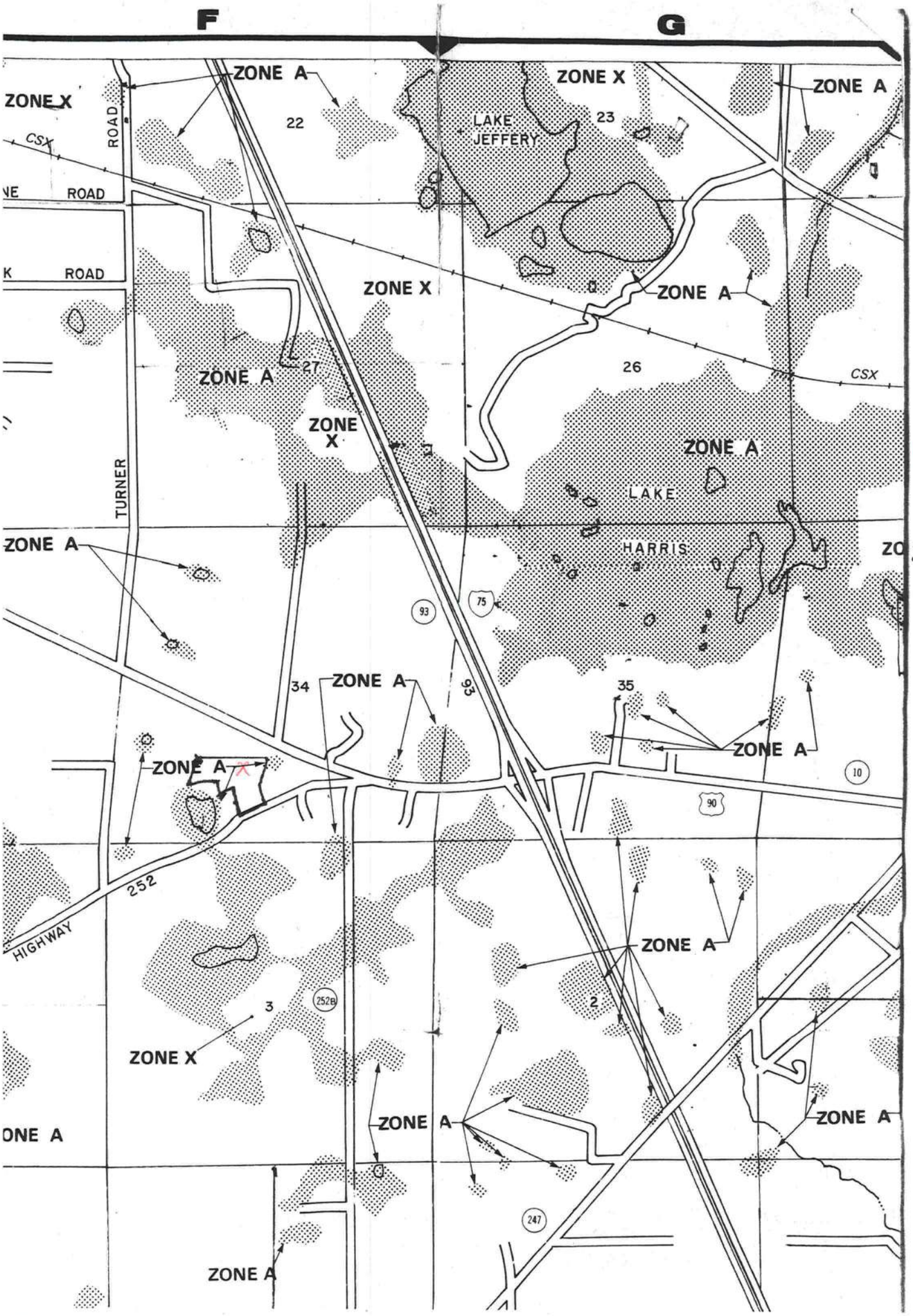
Amber Meads
MY COMMISSION EXPIRES March 25, 2007
#DD245720
Bonded thru Troy Fain-Insurance
NOTARY PUBLIC, STATE OF FLORIDA

Paid 359.00

0509-83



230'
DRAINAGE AREA 4.2151 ACRES
EDP
25' RAD
50'
158.70
158
20'
24'
+ 157.58
PROPOSED ASPHALT PAVEMENT
DRIVEWAY
157
LANDSCAPING(GRASSED)
ING(GRASSED)
156
155
154
153
150
SP1
PP3
30' X 150' ADDITION
EXISTING BUILDING
FLOOR ELEVATION 156.56 FT. 9/2001
700'
1,368
36'
40' X 75' NEW
EXISTING BUILDING 24' X 68'
362'
45'
522'
EXISTING BUILDING
LANDSCAPING (GRASSED)
8" BURIED PVC PIPE FROM GUTTERS/DOWNSPOUTS TO RETENTION BASIN
EXISTING EN
RETENT
159
3
4



057-83

SDP05-4

SUWANNEE
RIVER
WATER
MANAGEMENT
DISTRICT
9225 CR 49
LIVE OAK, FLORIDA 32060
TELEPHONE: (386) 362-1001
TELEPHONE: 800-226-1066
FAX (386) 362-1056

GENERAL PERMIT

PERMITTEE:
MORRELLS, INC.
461 SOUTHWEST DEPUTY J. DAVIS LANE
LAKE CITY, FL 32024

PERMIT NUMBER: ERP89-0162M2
DATE ISSUED: 10/24/2005
DATE EXPIRES: 10/24/2008
COUNTY: COLUMBIA
TRS: S34/T3S/R16E

Duplicate

PROJECT: MORRELL'S INC. MODIFICATION

Approved entity to whom operation and maintenance may be transferred pursuant to rule 40B-4.1130, Florida Administrative Code (F.A.C.):

DAVID MORRELL
MORRELLS, INC.
461 SOUTHWEST DEPUTY J. DAVIS LANE
LAKE CITY, FL 32024

Based on information provided, the Suwannee River Water Management District's (District) rules have been adhered to and an environmental resource general permit is in effect for the permitted activity description below:

Previous permit issued for 1.41 acres of impervious surface on 29.00 acres. Modification consists of construction and operation of a surfacewater management system serving 2.82 acres of impervious surface on a total project area of 29.00 acres in a manner consistent with the application package submitted by Curtis Keen of Keen Engineering & Surveying, Inc. certified on September 30, 2005.

It is your responsibility to ensure that adverse off-site impacts do not occur either during or after construction. Any additional construction or alterations not authorized by this permit may result in flood control or water quality problems both on and off site and will be a violation of District rule.

You or any other substantially affected persons are entitled to request an administrative hearing pursuant to ss.120.57(1), Florida Statutes (F.S.), and s.40B-1.511, F.A.C., if they object to the District's actions. Failure to request a hearing within 14 days will constitute a waiver of your right

to request such a hearing. In addition, the District will presume that permittee waives Chapter 120, F.S., rights to object or appeal the action upon commencement of construction authorized by the permit.

This permit is issued under the provisions of chapter 373, F.S., chapter 40B-4, and chapter 40B-400, F.A.C. A general permit authorizes the construction, operation, maintenance, alteration, abandonment, or removal of certain minor surface water management systems. This permit authorizes the permittee to perform the work necessary to construct, operate, and maintain the surface water management system shown on the application and other documents included in the application. This is to notify you of District's agency action concerning Notice Of Intent. This action is taken pursuant to rule 40B-4 and 40B-400, F.A.C.

Standard Conditions for All General Permits:

1. The permittee shall perform all construction authorized in a manner so as to minimize adverse impacts to fish, wildlife, natural environmental values, and water quality. The permittee shall institute necessary measures during construction including riprap, reinforcement, or compaction of any fill materials placed around newly installed structures, to minimize erosion, turbidity, nutrient loading, and sedimentation in the receiving waters.

2. Water quality data representative of the water discharged from the permitted system, including, but not limited to, the parameters in chapter 62-302, F.A.C., shall be submitted to the District as required. If water quality data are required, the permittee shall provide data as required on the volume and rate of discharge including the total volume discharged during the sampling period. All water quality data shall be in accordance with and reference the specific method of analysis in "Standard Methods for the Examination of Water and Wastewater" by the American Public Health Association or "Methods for Chemical Analysis of Water and Wastes" by the U.S. Environmental Protection Agency.

3. The operational and maintenance phase of an environmental resource permit will not become effective until the owner or his authorized agent certifies that all facilities have been constructed in accordance with the design permitted by the District. If required by the District, such as-built certification shall be made by an engineer or surveyor. Within 30 days after the completion of construction of the system, the permittee shall notify the District that the facilities are complete. If appropriate, the permittee shall request transfer of the permit to the responsible entity approved by the District for operation and maintenance. The District may inspect the system and, as necessary, require remedial measures as a condition of transfer of the permit or release for operation and maintenance of the system.

4. Off-site discharges during and after construction shall be made only through the facilities

authorized by the permit. Water discharged from the project shall be through structures suitable for regulating upstream stage if so required by the District. Such discharges may be subject to operating schedules established by the District.

5. The permit does not convey to the permittee any property right nor any rights or privileges other than those specified in the permit and chapter 40B-1, F.A.C.

6. The permittee shall hold and save the District harmless from any and all damages, claims, or liabilities which may arise by reason of the construction, operation, maintenance, alteration, abandonment, or development in a Works of the District which is authorized by the permit.

7. The permit is issued based on the information submitted by the applicant which reasonably demonstrates that adverse off-site water resource impacts will not be caused by the permitted activity. It is the responsibility of the permittee to insure that such adverse impacts do not in fact occur either during or after construction.

8. It is the responsibility of the permittee to obtain all other clearances, permits, or authorizations required by any unit of local, state, or federal government.

9. The surfacewater management system shall be constructed prior to or concurrent with the development that the system is intended to serve and the system shall be completed within 30 days of substantial completion of the development which the system is intended to serve.

10. Except for General Permits After Notice or permits issued to a unit of government, or unless a different schedule is specified in the permit, the system shall be inspected at least once every third year after transfer of a permit to operation and maintenance by the permittee or his agent to ascertain that the system is being operated and maintained in a manner consistent with the permit. A report of inspection is to be sent to the District within 30 days of the inspection date. If required by chapter 471, F.S., such inspection and report shall be made by an engineer.

11. The permittee shall allow reasonable access to District personnel or agents for the purpose of inspecting the system to insure compliance with the permit. The permittee shall allow the District, at its expense, to install equipment or devices to monitor performance of the system authorized by their permit.

12. The surfacewater management system shall be operated and maintained in a manner which is consistent with the conditions of the permit and chapter 40B-4.2040, F.A.C.

13. The permittee is responsible for the perpetual operation and maintenance of the system unless the operation and maintenance is transferred pursuant to chapter 40B-4.1130, F.A.C., or the permit

is modified to authorize a new operation and maintenance entity pursuant to chapter 40B-4.1110, F.A.C.

14. All activities shall be implemented as set forth in the plans, specifications and performance criteria as approved by this permit. Any deviation from the permitted activity and the conditions for undertaking that activity shall constitute a violation of this permit.

15. This permit or a copy thereof, complete with all conditions, attachments, exhibits, and modifications, shall be kept at the work site of the permitted activity. The complete permit shall be available for review at the work site upon request by District staff. The permittee shall require the contractor to review the complete permit prior to commencement of the activity authorized by this permit.

16. Activities approved by this permit shall be conducted in a manner which do not cause violations of state water quality standards.

17. Prior to and during construction, the permittee shall implement and maintain all erosion and sediment control measures (best management practices) required to retain sediment on-site and to prevent violations of state water quality standards. All practices must be in accordance with the guidelines and specifications in the Florida Stormwater, Erosion, and Sedimentation Control Inspector's Manual unless a project specific erosion and sediment control plan is approved as part of the permit, in which case the practices must be in accordance with the plan. If site-specific conditions require additional measures during any phase of construction or operation to prevent erosion or control sediment, beyond those specified in the erosion and sediment control plan, the permittee shall implement additional best management practices as necessary, in accordance with the Florida Stormwater, Erosion, and Sedimentation Control Inspector's Manual. The permittee shall correct any erosion or shoaling that causes adverse impacts to the water resources.

18. Stabilization measures shall be initiated for erosion and sediment control on disturbed areas as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, but in no case more than seven days after the construction activity in that portion of the site has temporarily or permanently ceased.

19. At least 48 hours prior to commencement of activity authorized by this permit, the permittee shall submit to the District a Construction Commencement Notice Form No. 40B-1.901(14) indicating the actual start date and the expected completion date.

20. When the duration of construction will exceed one year, the permittee shall submit construction status reports to the District on an annual basis utilizing an Annual Status Report Form No. 40B-1.901(15). These forms shall be submitted during June of each following year.

21. For those systems which will be operated or maintained by an entity requiring an easement or deed restriction in order to provide that entity with the authority necessary to operate or maintain the system, such easement or deed restriction, together with any other final operation or maintenance documents as are required by Paragraph 40B-4.2030(2)(g), F.A.C., and Rule 40B-4.2035, F.A.C., must be submitted to the District for approval. Documents meeting the requirements set forth in these subsections of District rules will be approved. Deed restrictions, easements and other operation and maintenance documents which require recordation either with the Secretary of State or Clerk of the Circuit Court must be so recorded prior to lot or unit sales within the project served by the system, or upon completion of construction of the system, whichever occurs first. For those systems which are proposed to be maintained by county or municipal entities, final operation and maintenance documents must be received by the District when maintenance and operation of the system is accepted by the local governmental entity. Failure to submit the appropriate final documents referenced in this paragraph will result in the permittee remaining liable for carrying out maintenance and operation of the permitted system.

22. Each phase or independent portion of the permitted system must be completed in accordance with the permitted plans and permit conditions prior to the initiation of the permitted use of site infrastructure located within the area served by that portion or phase of the system. Each phase or independent portion of the system must be completed in accordance with the permitted plans and permit conditions prior to transfer of responsibility for operation and maintenance of that phase or portion of the system to a local government or other responsible entity.

23. Within 30 days after completion of construction of the permitted system, or independent portion of the system, the permittee shall submit a written statement of completion and certification by a registered professional engineer or other appropriate individual as authorized by law, using the supplied As-Built Certification Form No. 40B-1.901(16) incorporated by reference in Subsection 40B-1.901(16), F.A.C. When the completed system differs substantially from the permitted plans, any substantial deviations shall be noted and explained and two copies of as-built drawings submitted to the District. Submittal of the completed form shall serve to notify the District that the system is ready for inspection. The statement of completion and certification shall be based on on-site observation of construction (conducted by the registered professional engineer, or other appropriate individual as authorized by law, or under his or her direct supervision) or review of as-built drawings for the purpose of determining if the work was completed in compliance with approved plans and specifications. As-built drawings shall be the permitted drawings revised to reflect any changes made during construction. Both the original and any revised specifications must be clearly shown. The plans must be clearly labeled as "as-built" or "record" drawing. All surveyed dimensions and elevations shall be certified by a registered surveyor. The following information, at a minimum, shall be verified on the as-built drawings:

a. Dimensions and elevations of all discharge structures including all weirs, slots, gates, pumps,

pipes, and oil and grease skimmers;

b. Locations, dimensions, and elevations of all filter, exfiltration, or underdrain systems including cleanouts, pipes, connections to control structures, and points of discharge to the receiving waters;

c. Dimensions, elevations, contours, or cross-sections of all treatment storage areas sufficient to determine stage-storage relationships of the storage area and the permanent pool depth and volume below the control elevation for normally wet systems, when appropriate;

d. Dimensions, elevations, contours, final grades, or cross-sections of the system to determine flow directions and conveyance of runoff to the treatment system;

e. Dimensions, elevations, contours, final grades, or cross-sections of all conveyance systems utilized to convey off-site runoff around the system;

f. Existing water elevation(s) and the date determined; and

g. Elevation and location of benchmark(s) for the survey.

24. The operation phase of this permit shall not become effective until the permittee has complied with the requirements of the condition in paragraph 23 above, the District determines the system to be in compliance with the permitted plans, and the entity approved by the District in accordance with Rule 40B-4.2035, F.A.C., accepts responsibility for operation and maintenance of the system. The permit may not be transferred to such approved operation and maintenance entity until the operation phase of the permit becomes effective. Following inspection and approval of the permitted system by the District, the permittee shall request transfer of the permit to the approved responsible operation and maintenance operating entity if different from the permittee. Until the permit is transferred pursuant to Rule 40B-4.1130, F.A.C., the permittee shall be liable for compliance with the terms of the permit.

25. Should any other regulatory agency require changes to the permitted system, the permittee shall provide written notification to the District of the changes prior to implementation so that a determination can be made whether a permit modification is required.

26. This permit does not eliminate the necessity to obtain any required federal, state, local and special District authorizations prior to the start of any activity approved by this permit. This permit does not convey to the permittee or create in the permittee any property right, or any interest in real property, nor does it authorize any entrance upon or activities on property which is not owned or controlled by the permittee, or convey any rights or privileges other than those specified in the permit and in this chapter and Chapter 40B-4, F.A.C.

27. The permittee is hereby advised that Section 253.77, F.S., states that a person may not commence any excavation, construction, or other activity involving the use of sovereign or other lands of the state, the title to which is vested in the Board of Trustees of the Internal Improvement Trust Fund without obtaining the required lease, license, easement, or other form of consent authorizing the proposed use. Therefore, the permittee is responsible for obtaining any necessary authorizations from the Board of Trustees prior to commencing activity on sovereignty lands or other state-owned lands.

28. Any delineation of the extent of a wetland or other surface water submitted as part of the permit application, including plans or other supporting documentation, shall not be considered specifically approved unless a specific condition of this permit or a formal determination under 40B-400.046, F.A.C., provides otherwise.

29. The permittee shall notify the District in writing within 30 days of any sale, conveyance, or other transfer of ownership or control of the permitted system or the real property at which the permitted system is located. All transfers of ownership or transfers of a permit are subject to the requirements of Rule 40B-4.1130, F.A.C. The permittee transferring the permit shall remain liable for any corrective actions that may be required as a result of any permit violations prior to such sale, conveyance or other transfer.

30. If historical or archaeological artifacts are discovered at any time on the project site, the permittee shall immediately notify the District.

31. The permittee shall immediately notify the District in writing of any previously submitted information that is later discovered to be inaccurate.

WITHIN 30 DAYS AFTER COMPLETION OF THE PROJECT, THE PERMITTEE SHALL NOTIFY THE DISTRICT, IN WRITING, THAT THE FACILITIES ARE COMPLETE.

Approved by _____ Date Approved 10-24-05
District Staff

_____ _____
Clerk Executive Director

OTICE OF COMMENCEMENT FORM
OLUMBIA COUNTY, FLORIDA

******THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.******

HE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance ith Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

ax Parcel ID Number 34-3S-16-02475-001

. **Description of property: (legal description of the property and street address or 911 address)**
PARCEL 34-3S-16-02475-001 BEGIN AT NW COR OF SE 1/4 OF SW 1/4 RUN E 1148.46 FT, SW 324.32 FT, SE 330.58 FT TO N

. **General description of improvement:** 40' X 75' METAL BUILDING

. **Owner Name & Address** MORRIS'S 461 SW DEPUTY J. DAWN LANE LAKE CITY, FL 32024 **Interest in Property** ~~FEE~~ SIMPLE

. **Name & Address of Fee Simple Owner (if other than owner):** N/A

. **Contractor Name** L&L CONSTRUCTION LLC - LEVY SAPP **Phone Number** 386 754-5882
Address 524 NW. CALE CT. LAKE CITY, FL 32055

. **Surety Holders Name** NONE **Phone Number**
Address
Amount of Bond

Inst:2005024024 Date:09/28/2005 Time:13:53
DC,P.DeWitt Cason,Columbia County B:1059 P:2741

. **Lender Name** NONE
Address

. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be erved as provided by section 718.13 (1)(a) 7; Florida Statutes:**
Name NONE **Phone Number**
Address

). In addition to himself/herself the owner designates N/A **of** **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) –**
(a) 7. Phone Number of the designee

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)

OTICE AS PER CHAPTER 713, Florida Statutes:
he owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Signature of Owner

Sworn to (or affirmed) and subscribed before day of 9-28 **20** 05

NOTARY STAMP/SEAL

Amber Meads
MY COMMISSION EXPIRES March 25, 2007
#DD245720
Bonded thru Troy Fain-Insurance
NOTARY PUBLIC, STATE OF FLORIDA

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Reference to: Build permit application Number: **0509-83 L & L Construction Owner Morrell's @ 461 SW Deputy J. Davis Lane**

On the date of September 30, 2005 application 0509-83 and plans for construction of an group S2 occupancy building for warehouse usage were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0509-83 when making reference to this application.

1. Furnish two copies of foundation plans and provide a details design which complies with chapter 18 of the Florida Building Code.

2. Please furnish two copies of the findings from a subsurface investigation of the soils done by a geotechnical testing company to determine the soils supporting stability at each of the corners which the building requires a supporting or load bearing foundation.

3. Please submit a site plan which will locate the 3,000 sq.ft. building to be used as a group S2 occupancy warehouse. Show the distance of this proposed building as it will be located within the property boundaries and all other existing building on this property.

4. Show on a floor plan the exits doors as required of the Florida Building Code. Section 1027: Storage building.

5. Please provide approval of products by The Florida Building Commission for materials to be used in the sheer walls or roof systems for the following categories of products: (1) Panel Walls;

(2) Exterior Doors; (3) Roofing Products; (4) Skylights; (5) Windows; (6) Shutters; and (7) Structural components.

None 6. Provide an electrical plan to include the following electrical information: Electrical wiring, services, feeders and branch circuits, over-current protection, grounding, wiring methods, materials and lighting requirements.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

COLUMBIA COUNTY BUILDING DEPARTMENT

COMMERCIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 WITH AMENDMENTS

ALL REQUIREMENTS LISTED ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INCLUDE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 WITH AMENDMENTS BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SIGNATURE AND SEAL OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA. THE FOLLOWING BASIC WIND SPEED AS PER SECTION 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ---------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ---------- 110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: **Two (2) complete sets of plans containing a floor plan, site plan, foundation plan, floor/roof framing plan or truss layout, wall sections and all exterior elevations with the following criteria and documents:**

Applicant	Plans Examiner	
☑	☐	**All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.**
☑	☐	**Designers name and signature on document (FBC 104.2.1) If licensed architect or engineer, official seal shall be affixed.**
☐	☐	**Two (2) Copies of Approved Site Plan**
☐	☐	**Minimum Type Construction (FBC Table 500)**
☑	☐	**Wind Load Engineering Summary, calculations and any details required:** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC 1. Basic wind speed (MPH) 2. Wind importance factor (I) and building category 3. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated 4. The applicable internal pressure coefficient 5. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☐	☐	**Fire Resistant Construction Requirements shall include:**
☐	☐	a) Fire resistant separations (listed system)
☐	☐	b) Fire resistant protection for type of construction
☐	☐	c) Protection of openings and penetrations of rated walls (listed systems)
☐	☐	d) Fire blocking and draft-stopping
☐	☐	e) Calculated fire resistance

Fire Suppression Systems shall include: (To be reviewed by Fire Department)

- ☐ ☐ a) Fire sprinklers
- ☐ ☐ b) Fire alarm system (early warning) with name of licensed installer. If not shown on plans or not known at time of permitting, a separate permit shall be required by the licensed installer
- ☐ ☐ **c)** Smoke evacuation system schematic
- ☐ ☐ **d)** Stand-pipes
  - Pre-engineered system
  - Riser diagram

Life Safety Systems shall include: (To be reviewed by Fire Department)

- ☐ ☐ a) Occupancy load and egress capacity
- ☐ ☐ b) Early warning
- ☐ ☐ c) Smoke control
- ☐ ☐ d) Stair pressurization
- ☐ ☐ e) Systems schematic

Occupancy Load/Egress Requirements shall include:

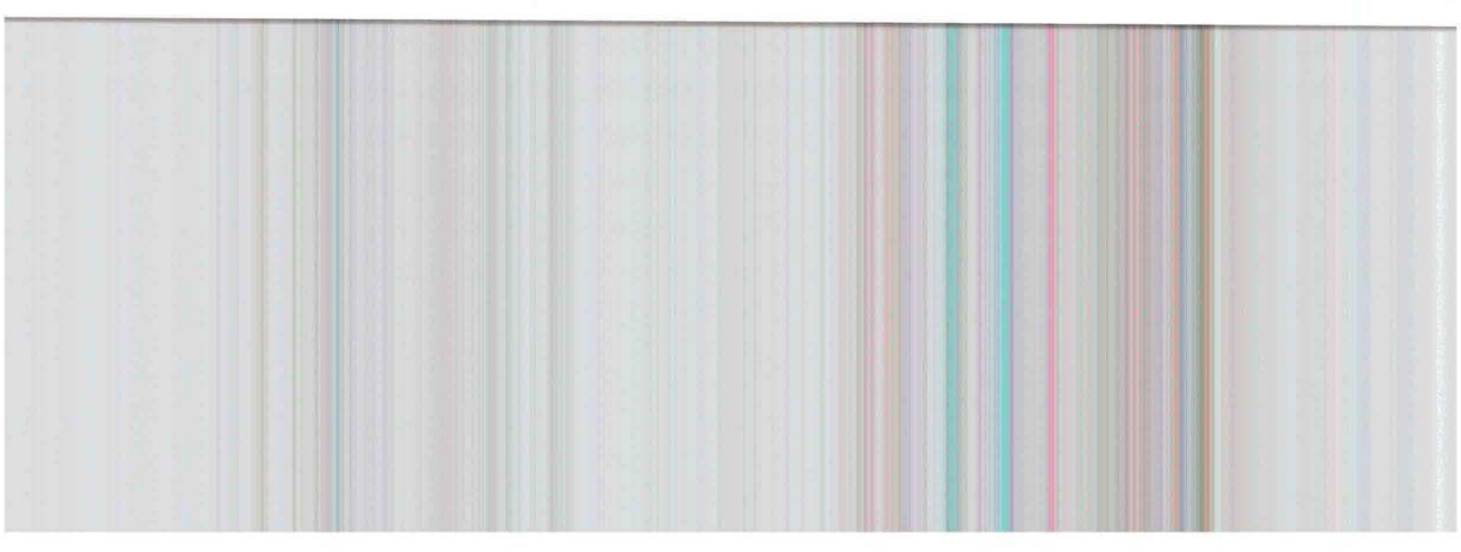
- ☐ ☐ a) Occupancy load (gross and net)
- ☐ ☐ b) Means of egress
  - exit access, exit and exit discharge
- ☐ ☐ c) Stair construction/geometry and protection
- ☐ ☐ d) Doors
- ☐ ☐ e) Emergency lighting and exit signs
- ☐ ☐ f) Specific occupancy requirements
  1. Construction requirements
  2. Horizontal exits/exit passageways

Structural Requirements shall include:

- ☐ ☐ a) Soil conditions/analysis
- ☐ ☐ b) Show type of termite treatment (termicide or alternative method)
- ☐ ☐ c) Design loads
- ☐ ☐ d) Wind requirements
- ☐ ☐ e) Building envelope
- ☐ ☐ f) Structural calculations
- ☑ ☐ g) Foundations
- ☐ ☐ h) Wall systems
- ☐ ☐ i) Floor systems
- ☐ ☐ j) Roof systems
- ☐ ☐ k) Threshold inspection plan (if applicable)
- ☐ ☐ l) Stair systems

Materials shall include:

- ☐ ☐ a) Wood
- ☑ ☐ b) Steel
- ☐ ☐ c) Aluminum
- ☑ ☐ d) Concrete
- ☐ ☐ e) Plastic
- ☐ ☐ f) Glass (mfg. Listing for wind zone including details for installation and attachments
- ☐ ☐ g) Masonry
- ☐ ☐ h) Gypsum board and plaster
- ☐ ☐ i) Insulating (mechanical)
- ☑ ☐ j) Roofing (mfg. Listed system for wind zone with installation and attachments)
- ☑ ☐ k) Insulation



Accessibility Requirements shall include:

- ☐ ☐ a) Site requirements
- ☐ ☐ b) Accessible route
- ☐ ☐ c) Vertical accessibility
- ☐ ☐ d) Toilet and bathing facilities
- ☐ ☐ e) Drinking fountains
- ☐ ☐ f) Equipment
- ☐ ☐ g) Special occupancy requirements
- ☐ ☐ h) Fair housing requirements

Interior Requirements shall include:

- ☐ ☐ a) Interior finishes (flame spread/smoke develop)
- ☐ ☐ b) Light and ventilation
- ☐ ☐ c) Sanitation

Special Systems shall include:

- ☐ ☐ a) Elevators
- ☐ ☐ b) Escalators
- ☐ ☐ c) Lifts

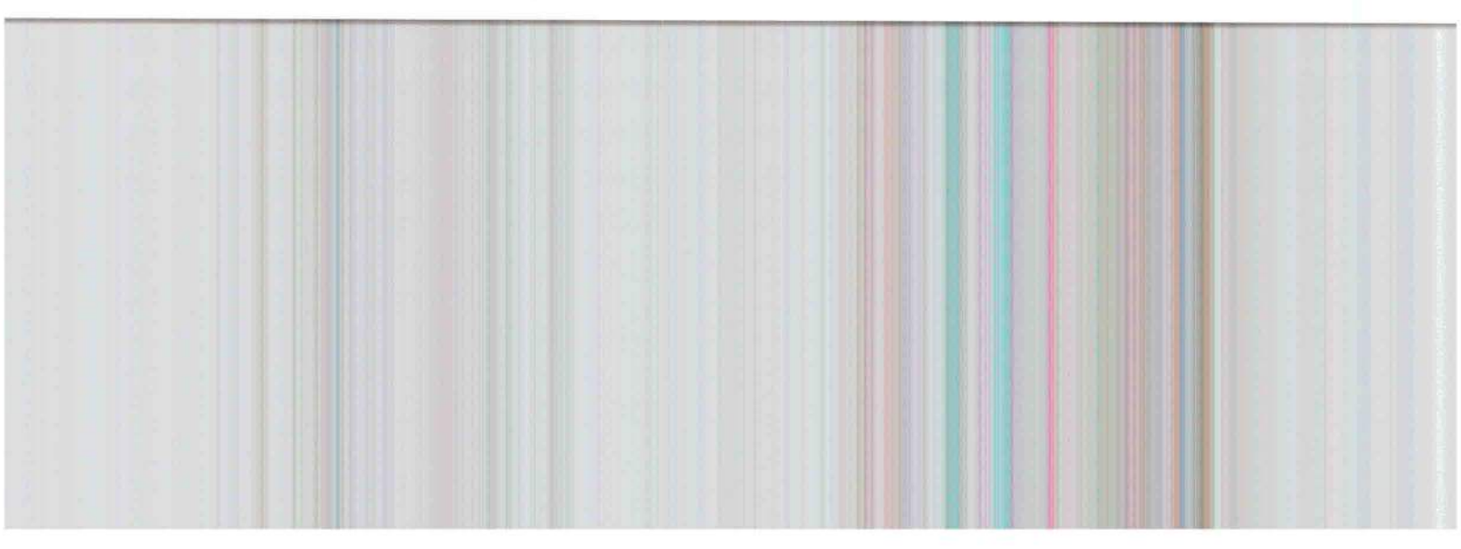
Swimming Pools – Commercial – Plans shall be signed and sealed by a Professional Engineer registered in the State of Florida and approved by the Department of Business and Professional Regulation/Health Department Indicating compliance with the Florida Administrative Code, Chapter 64E-9 And Section 424 of the Florida Building Code

Electrical:

- ☐ ☐ a) Electrical wiring, services, feeders and branch circuits, over-current protection, grounding, wiring methods and materials, GFCIs
- ☐ ☐ b) Equipment
- ☐ ☐ c) Special Occupancies
- ☐ ☐ d) Emergency Systems
- ☐ ☐ e) Communication Systems
- ☐ ☐ f) Low Voltage
- ☐ ☐ g) Load calculations
- ☐ ☐ h) Riser diagram

Plumbing:

- ☐ ☐ a) Minimum plumbing facilities
- ☐ ☐ b) Fixture requirements
- ☐ ☐ c) Water supply piping
- ☐ ☐ d) Sanitary drainage
- ☐ ☐ e) Water heaters
- ☐ ☐ f) Vents
- ☐ ☐ g) Roof drainage
- ☐ ☐ h) Back flow prevention
- ☐ ☐ i) Irrigation
- ☐ ☐ j) Location of water supply
- ☐ ☐ k) Grease traps
- ☐ ☐ l) Environmental requirements
- ☐ ☐ m) Plumbing riser



Mechanical:

- ☐ ☐ a) Energy calculation (signed and sealed by Architect or Engineer, registered in the State of Florida)
- ☐ ☐ b) Exhaust systems (clothes dryer exhaust, kitchen equipment exhaust, Specialty equipment exhaust)
- ☐ ☐ c) Equipment
- ☐ ☐ d) Equipment location
- ☐ ☐ e) Make-up air
- ☐ ☐ f) Roof mounted equipment
- ☐ ☐ g) Duct systems
- ☐ ☐ h) Ventilation
- ☐ ☐ i) Combustion air
- ☐ ☐ j) Chimneys, fireplaces and vents
- ☐ ☐ k) Appliances
- ☐ ☐ l) Boilers
- ☐ ☐ m) Refrigeration
- ☐ ☐ n) Bathroom ventilation
- ☐ ☐ o) Laboratory

Gas:

- ☐ ☐ a) Gas piping
- ☐ ☐ b) Venting
- ☐ ☐ c) Combustion air
- ☐ ☐ d) Chimney's and vents
- ☐ ☐ e) Appliances
- ☐ ☐ f) Type of gas
- ☐ ☐ g) Fireplaces
- ☐ ☐ h) LP tank locations
- ☐ ☐ i) Riser diagram/shut offs

☐ ☐ **Disclosure Statement for Owner Builders**

☐ ☐ ***Notice of Commencement Required Before Any Inspections will be Done**

☐ ☐ **Private Potable Water:**

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS:

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all construction projects; If you were required to have a Site and Development Plan Approval, list SDP number.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser is required. A copy of property deed is also requested. (386) 758-1084

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic tank approval or sewer tap is required

4. **City Approval:** If the project is located within the city limits of the Town of Fort White prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) **has been** established shall meet the requirements of section 8.8 of the Columbia County Land Development Regulations. Any project that is located within a flood zone where the base flood elevation (100 year flood) **has not been** established shall meet the requirements of section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
   A development permit will also be required. **The development permit cost is $50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit must be made **($25.00)**. Culvert installation for commercial, industrial and other uses shall **conform to the approved site plan or to the specifications of a registered engineer. Joint use culverts will comply with Florida Department of Transportation specifications**. If the project is to be located on a F.D.O.T. maintained road, then an F.D.O.T. access permit is required.

7. **Suwannee River Water Management District Approval:** All commercial projects must have an SRWMD permit issued or an exemption letter, before a building will be issued.

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS – PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

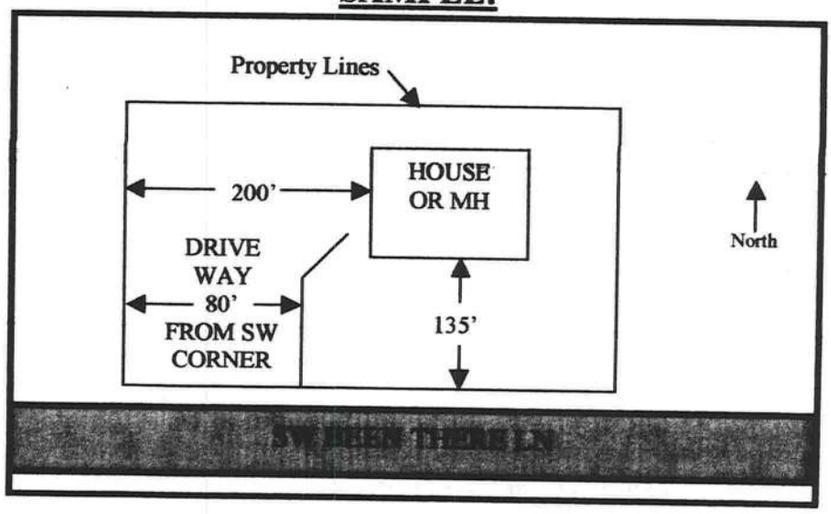
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123) FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
   a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
   b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
   c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

BSX **BUILDING SYSTEMS EXPRESS**
Division of Metal Buildings, Inc.

Mailing Address:
P.O. Box 860
Thomasville, GA 31799-0860

Factory location:
115 Fairbanks Ave.
Thomasville, GA 31792

Telephone:
1-800-279-9455
Fax (229) 226-6874
E-mail: mbi@rose.net

Effective: January 3, 2005

BSX building packages incorporate one or more of the following products:

Vendor & Florida Approval Information

Steel Roof & Wall Panels

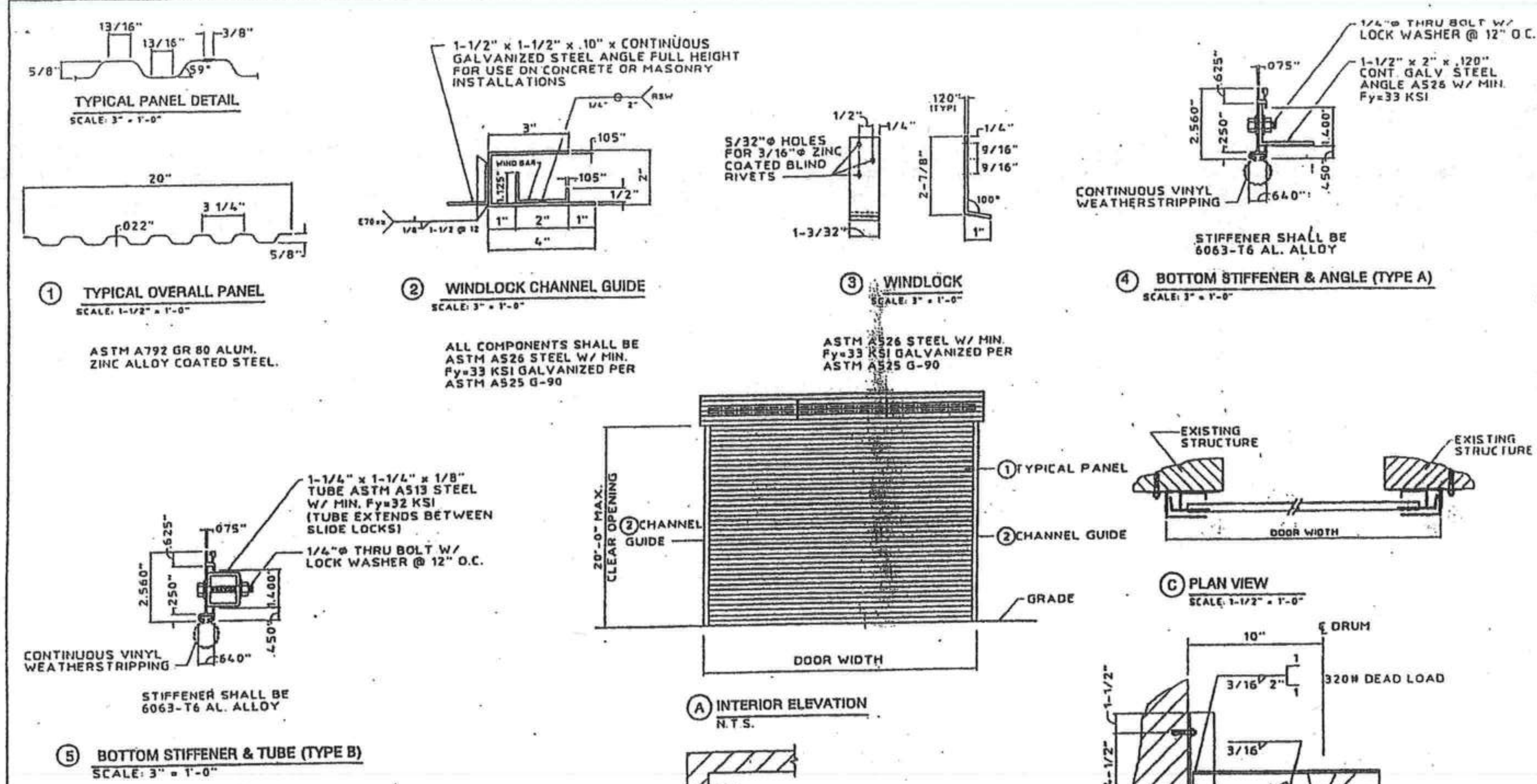
Whirlwind Building Systems
FL 1845.3 Superspan & Superspan-X Roof Panel
FL 2489.1 Superspan Wall Panel

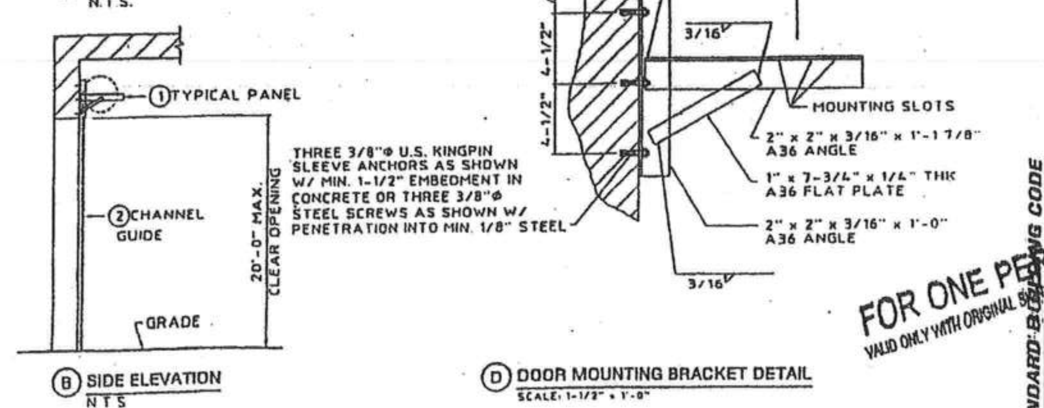
Steel Walkdoors

Premier Products, Inc
FL 3661.2 3070 Reversible Door

Skylites/Wall Lights - Light Transmitting Panels

Paltough Ltd/Suntuf Inc
MIAMI-DADE NOA 00-1226.02 Polycarbonate Sheets





GENERAL NOTES:

1. THIS ROLL-UP DOOR IS DESIGNED IN ACCORDANCE WITH THE STANDARD BUILDING CODE, 1991, 1994 & 1997 EDITION, INCLUDING THE 1995 PALM BEACH COUNTY AMENDMENTS.
2. THIS ROLL-UP DOOR HAS BEEN TESTED IN ACCORDANCE WITH THE STANDARD BUILDING CODE AND ASTM E-330 TO SAFELY RESIST A POSITIVE OR NEGATIVE WIND LOAD AS NOTED IN THE DOOR SCHEDULE. A TEST LOAD OF 1.5 x DESIGN LOAD HAS BEEN USED.
3. WIND LOADS FOR BUILDING OPENINGS SHALL BE DETERMINED BY A PROFESSIONAL ENGINEER USING APPROPRIATE WIND SPEED AND DESIGN CRITERIA. THIS DOOR MAY BE USED WHERE THE DESIGN LOAD MEETS OR EXCEEDS THE DESIGN LOAD FOR THE BUILDING OPENING.
4. SUPERIMPOSED LOADS ON THE JAMBS FROM THIS DOOR ARE DESIGNATED AS Vx AND Vy HEREIN. CONTRACTORS SHALL HAVE BUILDING ENGINEER VERIFY ADEQUACY OF BUILDING STRUCTURE TO RESIST SUPERIMPOSED LOADS Vx, Vy AND BRACKET LOADS SHOWN.
5. ALL WELDING SHALL BE PERFORMED BY QUALIFIED WELDERS IN ACCORDANCE WITH A.W.S. SPECIFICATIONS, LATEST EDITION. ALL WELDING ELECTRODES SHALL CONFORM TO A.W.S. A5.1 GRADE E-70.
6. DOORS SHALL BE PROVIDED WITH SLIDE LOCK MECHANISMS AT EACH SIDE OF BOTTOM OF DOOR ENGAGING CHANNEL GUIDES IN THE LOCKED POSITION.

STANDARD BUILDING CODE

KNEZEVICH & ASSOCIATES, INC.
CONSULTING ENGINEERS • PRODUCT TESTING
1280 N. UNIVERSITY DRIVE, SUITE 180 • FORT LAUDERDALE, FL 33322

ROLL - UP DOOR

revisions

98-

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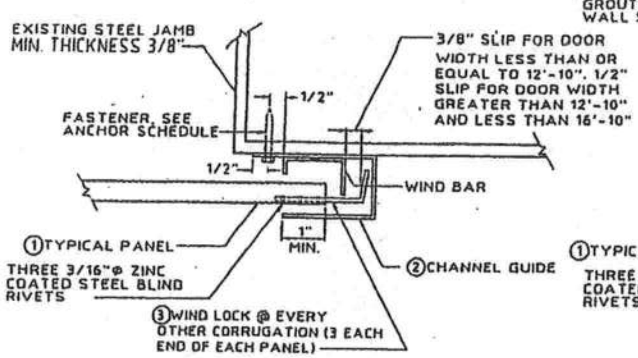
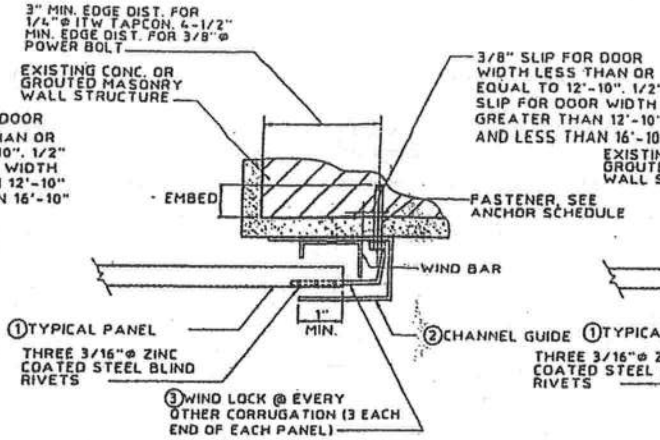
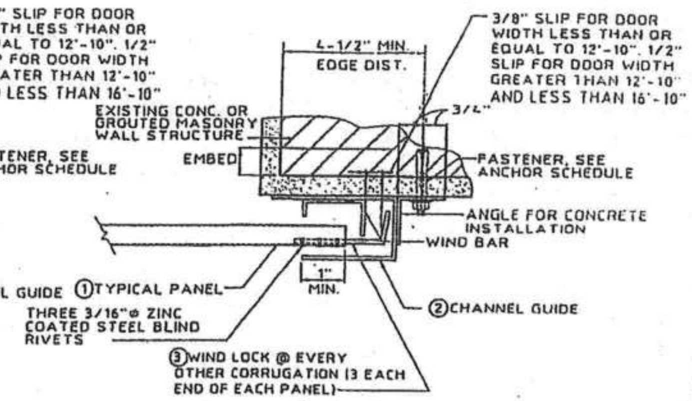


(E1) WINDLOCK GUIDE CONNECTION TO STEEL JAMB DETAIL
SCALE: 3" = 1'-0"

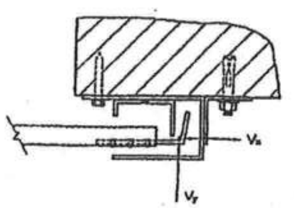


(E2) WINDLOCK GUIDE CONNECTION TO CONCRETE OR GROUTED MASONRY DETAIL (INSIDE GUIDE)
SCALE: 3" = 1'-0"



(E3) WINDLOCK GUIDE CONNECTION TO CONCRETE OR GROUTED MASONRY DETAIL (OUTSIDE GUIDE)
SCALE: 3" = 1'-0"

DOOR SCHEDULE				ANCHOR SCHEDULE - (FASTENER MAXIMUM SPACING)						
				DETAIL (E1)	DETAIL (E2)		DETAIL (E2)		DETAIL (E3)	
MAX. DOOR WIDTH	MAX. DESIGN PRESSURE (P.S.F.)	Vx (P.L.F.)	Vy (P.L.F.)	3/8"ø -12 x 1" HEX WASHER HEAD TYPE B THREAD FORMING ZINC PLATED STEEL SCREW PLUS A 3/8"ø FLAT WASHER	3/8"ø POWER BOLT ANCHOR WITH MINIMUM 2" EMBEDMENT		1/4"ø ITW TAPCON ANCHOR WITH MINIMUM 1-3/4" EMBEDMENT		3/8"ø U.S. KINGPIN SLEEVE ANCHOR WITH MINIMUM 1-7/8" EMBEDMENT	
				STEEL STRUCTURE	CONCRETE STRUCTURE	GROUTED MASONRY STRUCTURE (SEE NOTE No. 2)	CONCRETE STRUCTURE	GROUTED MASONRY STRUCTURE (SEE NOTE No. 2)	CONCRETE STRUCTURE	GROUTED MASONRY STRUCTURE (SEE NOTE No. 2)
≤ 12'-10"	± 35.0	810	223	12"	12"	12"	6"	4"	12"	7"
	± 26.0	478	165	14"	14"	14"	11"	6"	14"	11"
> 12'-10"	± 30.3	1169	255	8"	12"	9"	4"	N/A	8"	5"
≤ 16'-10"	± 26.0	962	218	8"	14"	11"	5"	3"	10"	6"



(E) SUPERIMPOSED LOAD DIAGRAM
SCALE: 3" = 1'-0"

ANCHOR NOTES:

1. EMBEDMENT LENGTH DOES NOT INCLUDE STUCCO FINISH.
2. FOR HOLLOW MASONRY, FILL ALL CELLS WITHIN 8" OF THE ANCHOR W/ 2500 PSI GROUT.
3. ANCHORS SHALL BE INSTALLED IN ACCORDANCE W/ MANUFACTURERS SPECIFICATIONS.

FOR ONE PERMIT ONLY
VALID ONLY WITH ORIGINAL SIGNATURE & IMPRESSION
JAN 10 20

STANDARD BUILDING CODE

KNEZEVICH & ASSOCIATES, INC.
CONSULTING ENGINEERS • PRODUCT TESTING
1280 N. UNIVERSITY DRIVE, SUITE 140 • FORT LAUDERDALE, FL 33322
TEL: (954) 382-2800 • FAX: (954) 382-2580 • FLORIDA COA #3905
WEBSITE: WWW.KNEZEVICH.COM • E-MAIL: KA@KNEZEVICH.COM

ROLL - UP DOOR

DBCI
DOORS & BUILDING COMPONENTS, INC.

revisions			
no	date	by	description
1	03/17/99	DWM	PREV. DRAWING No. 95-438
2	06/23/99	JP	GENERAL REVISIONS

V.J. KNEZEVICH
PROFESSIONAL ENG.
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date 07/17
scale AS NOTED
design by MG
drawing no. 98-176
sheet 2 of

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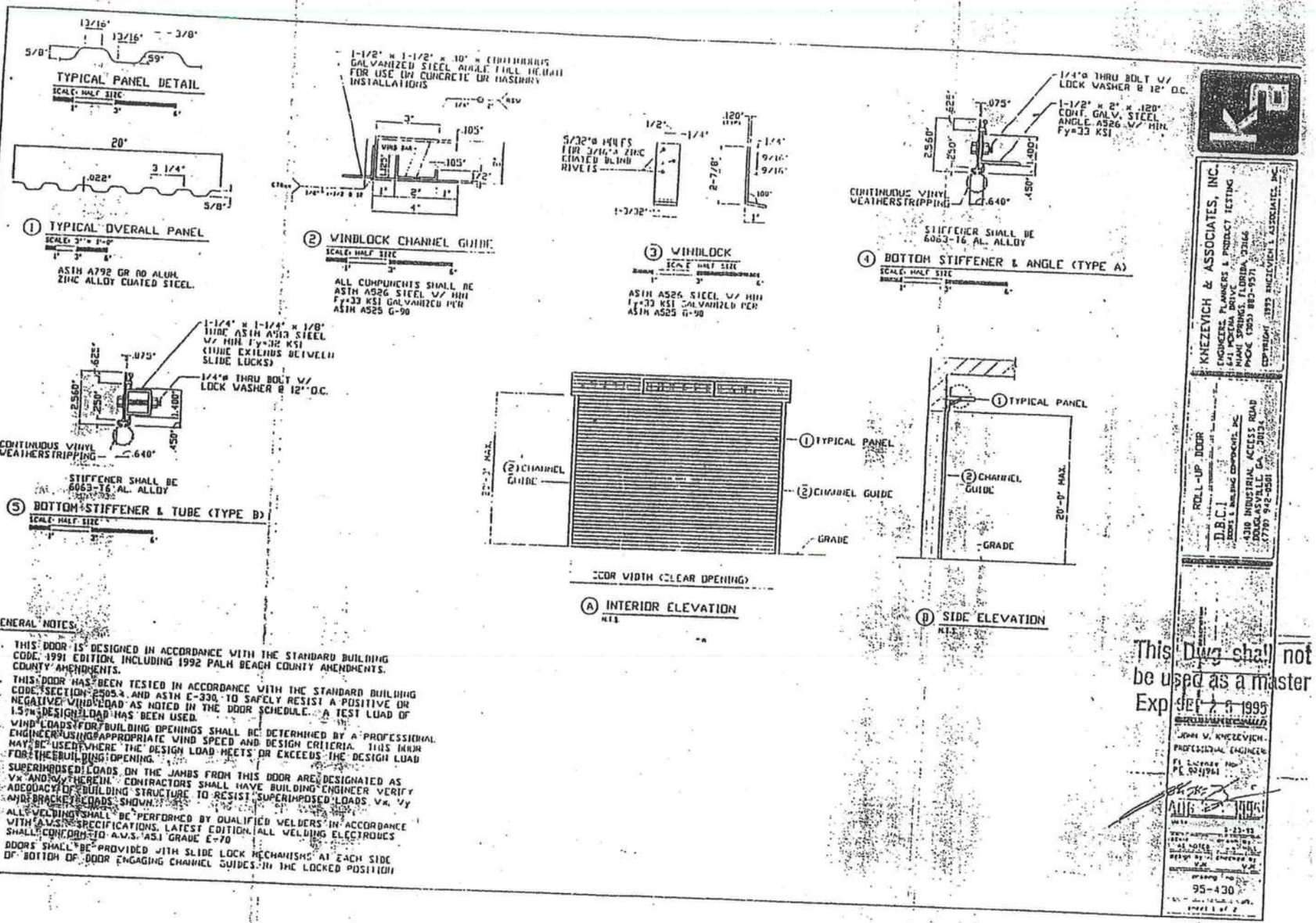
TYPICAL PANEL DETAIL
SCALE: HALF SIZE
1" 3" 6"
13/16"
13/16"
3/8"
5/8"
59°
20"
3 1/4"
.022"
5/8"
① TYPICAL OVERALL PANEL
SCALE: 3"=1'-0"
ASTM A792 GR 80 ALUM. ZINC ALLOY COATED STEEL.
1-1/2" x 1-1/2" x 10" x CONTINUOUS GALVANIZED STEEL ANGLE FULL HEIGHT FOR USE ON CONCRETE OR MASONRY INSTALLATIONS
WIND BAR
.105"
.125"
1/2"
1"
2"
4"
② WINDLOCK CHANNEL GUIDE
SCALE: HALF SIZE
ALL COMPONENTS SHALL BE ASTM A526 STEEL W/ MIN Fy=33 KSI GALVANIZED PER ASTM A525 G-90
5/32"ø HOLES FOR 3/16"ø ZINC COATED BLIND RIVETS
1/2"
1/4"
.120"
2-7/8"
1"
1-3/32"
③ WINDLOCK
SCALE: HALF SIZE
ASTM A526 STEEL W/ MIN Fy=33 KSI GALVANIZED PER ASTM A525 G-90
1/4"ø THRU BOLT W/ LOCK WASHER @ 12" O.C.
.075"
1-1/2" x 2" x .120" CONT. GALV. STEEL ANGLE A526 W/ MIN. Fy=33 KSI
2.560"
.250"
.400"
.450"
.640"
CONTINUOUS VINYL WEATHERSTRIPPING
STIFFENER SHALL BE 6063-T6 AL. ALLOY
④ BOTTOM STIFFENER & ANGLE (TYPE A)
SCALE: HALF SIZE
1-1/4" x 1-1/4" x 1/8" TUBE ASTM A501 STEEL W/ MIN Fy=36 KSI (TUBE EXTENDS BETWEEN SLIDE LOCKS)
1/4"ø THRU BOLT W/ LOCK WASHER @ 12" O.C.
.075"
.625"
2.560"
.250"
.400"
.450"
.640"
CONTINUOUS VINYL WEATHERSTRIPPING
STIFFENER SHALL BE 6063-T6 AL. ALLOY
⑤ BOTTOM STIFFENER & TUBE (TYPE B)
SCALE: HALF SIZE
1" 3" 6"
22'-3" MAX.
② CHANNEL GUIDE
① TYPICAL PANEL
② CHANNEL GUIDE
GRADE
DOOR WIDTH (CLEAR OPENING)
Ⓐ INTERIOR ELEVATION
NTS
① TYPICAL PANEL
② CHANNEL GUIDE
20'-0" MAX.
GRADE
Ⓑ SIDE ELEVATION
NTS
GENERAL NOTES:
1. THIS DOOR IS DESIGNED IN ACCORDANCE WITH THE STANDARD BUILDING CODE, 1991 EDITION, INCLUDING 1992 PALM BEACH COUNTY AMENDMENTS.
2. THIS DOOR HAS BEEN TESTED IN ACCORDANCE WITH THE STANDARD BUILDING CODE, SECTION 2505.4 AND ASTM E-330, TO SAFELY RESIST A POSITIVE OR NEGATIVE WIND LOAD AS NOTED IN THE DOOR SCHEDULE. A TEST LOAD OF 1.5 x DESIGN LOAD HAS BEEN USED.
3. WIND LOADS FOR BUILDING OPENINGS SHALL BE DETERMINED BY A PROFESSIONAL ENGINEER USING APPROPRIATE WIND SPEED AND DESIGN CRITERIA. THIS DOOR MAY BE USED WHERE THE DESIGN LOAD MEETS OR EXCEEDS THE DESIGN LOAD FOR THE BUILDING OPENING.
4. SUPERIMPOSED LOADS ON THE JAMBS FROM THIS DOOR ARE DESIGNATED AS Vx AND Vy THEREIN. CONTRACTORS SHALL HAVE BUILDING ENGINEER VERIFY ADEQUACY OF BUILDING STRUCTURE TO RESIST SUPERIMPOSED LOADS Vx, Vy AND BRACKET LOADS SHOWN.
5. ALL WELDING SHALL BE PERFORMED BY QUALIFIED WELDERS IN ACCORDANCE WITH A.W.S. SPECIFICATIONS, LATEST EDITION. ALL WELDING ELECTRODES SHALL CONFORM TO A.W.S. A5.1 GRADE E-70
6. DOORS SHALL BE PROVIDED WITH SLIDE LOCK MECHANISMS AT EACH SIDE OR BOTTOM OF DOOR ENGAGING CHANNEL GUIDES IN THE LOCKED POSITION
KNEZEVICH & ASSOCIATES, INC.
ENGINEERS, PLANNERS & PRODUCT TESTING
641 POINCIANA DRIVE
MAIKE SPRINGS, FLORIDA 32046
PHONE (305) 883-9571
COPYRIGHT 1993 KNEZEVICH & ASSOCIATES, INC.
ROLL-UP DOOR
D.B.C.I.
DOORS & BUILDING COMPONENTS, INC.
4310 INDUSTRIAL ACCESS ROAD
DOUGLASVILLE, GA. 30134
(770) 942-8581
This Dwg shall not be used as a master
Exp Dec 31 1995
JOHN V. KNEZEVICH
PROFESSIONAL ENGINEER
FL License No. PE 0041961
AUG 21 1995
95-430

CONSTRUCTION TESTING CORPORATION
13873 N.W. 19th Ave. Miami, Florida 33054
Phone: (305) 685-6657 Fax: (305) 685-6659

Report No: 95-029

23 August 1995

Test Date: 17 August 1995

TESTS ON ROLL DOWN DOORS

Client:

Door & Building Components, Inc.

4310 Industrial Access Road
Douglasville, Georgia 30134

General : Uniform Static Air Pressure Loading, per ASTM E-330

Testing witnessed by:

John W. Knezevich, P.E. Knezevich & Ass.
Don Mills, Product Engineer for D.B.C.I.
Bill Mathews, President J. B. Mathews
George Dotzler, CTC Test Engineer

Statement of Conformance : This is a general statement and does not supersede the specific product descriptions in this report. The specimens are in conformance with drawings provided by the manufacturer, labeled:

ROLL - UP DOOR

D.B.C.I.

Door & Building Components, Inc.
4310 Industrial Access Road
Douglasville, Georgia 30134

Date : 8 - 23 - 95 Drawing # 95 - 430

Description of Test Specimen: The specimen was a roll down door manufactured by Door & Building Components, Inc. . This door was installed covering a nominal opening 16'-0" wide by 16'-0" high. The door was constructed of painted galvanized steel sheet (mic'd @ 0.0240" w/ galvanized, w/o paint). The specimens channel guides were secured to the steel jamb (1/4" steel plate) of the test chamber with 9/16" hex head self threading 3/8" x 1" screws at 4" on center. These channel guides were as shown in detail 2 "Windlock Channel Guide" of the manufacturers supplied drawings. The left guide fastenings were secured through the 1-1/2" x 1-1/2" angle typically used for concrete installations. The right channel guide did not include this feature (the 1-1/2" x 1-1/2" angle) and the fastenings were secured through the center of the 1" protruding flange as shown detail 2. The door's bottom bar was as shown in detail 5 "Bottom Stiffener & Angle (Type B)" in the manufacturers drawings. Before testing this door was fully functional.

CONSTRUCTION TESTING CORPORATION
13873 N.W. 19th Ave. Miami, Florida 33054
Phone: (305) 685-6657 Fax: (305) 685-6659

Static Wind Loading / Manner of Testing:

Loads applied to the specimen (10 seconds durations in loading cycles greater than 40 seconds) were at levels specified by the client's Consulting Engineer. Polyethylene film (2 mil) and tape were used to seal air leakage during loads. The film and tape were used in a manner that did not influence the results. Deflection gauges were mounted at each jamb to record deflections along the center line of the door. The deflection readings are as follows:

		Left		Center		Right		Net @ Center Line		
Load	Load	Delta	Delta	Delta	Delta	Delta	Delta	Delta	Delta	Percent
PSF	In. H2O	.@ Load	.@ Rec'y	.@ Load	.@ Rec'y	.@ Load	.@ Rec'y	.@ Load	.@ Rec'y	Recovery
0.0	0.0	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	NA
15.6	3.0	0.500	0.063	11.438	0.188	0.500	0.016	10.938	0.148	98.6
20.8	4.0	0.531	0.063	12.063	0.125	0.516	0.156	11.539	0.016	99.9
26.0	5.0	0.531	0.031	13.000	0.063	0.563	0.031	12.453	0.031	99.7
31.2	6.0	0.563	0.031	13.750	0.125	0.625	0.063	13.156	0.078	99.4
38.5	7.4	0.625	0.125	14.938	0.313	0.750	0.063	14.250	0.219	98.5
45.5	8.8	0.688	0.156	16.250	0.750	0.875	0.063	15.469	0.641	95.9
52.5	10.1	NR	NR	NR	NR	NR	NR	NR	NR	NR

As loading was initiated it momentarily rose to approximately 55 PSF then imediately reduced to the desired level.

The correct load was held for approximately 4 seconds when the windlocks failed at the center of the right jamb.

SUMMARY

One roll down door specimen manufactured by DBCI was wind loaded in accordance with **ASTM** E-330 under the supervision of the clients consulting engineer. Loads were chosen to prove the adequacy of the product to sustain a design load of 25.5 PSF. In fact the sustained test load of 45.5 PSF proved the product to a design load of 30.3 PSF.

Respectfully submitted,

CONSTRUCTION TESTING CORPORATION.
(Dade County Certification # 95-0419.02)

Report by George Dotzler : _____

Test witnessed & report reviewed by John W. Knezevich, P.E. : _____

13873 N.W. 19th Ave. Miami, Florida 33054
Phone: (305) 685-6657 Fax: (305) 685-6659

Static Wind Loading / Manner of Testing:

Loads applied to the specimen (10 seconds durations in loading cycles greater than 40 seconds) were at levels specified by the client's Consulting Engineer. Polyethylene film (2 mil) and tape were used to seal air leakage during loads. The film and tape were used in a manner that did not influence the results. Deflection gauges were mounted at each jamb to record deflections along the center line of the door. The deflection readings are as follows:

		Left		Center		Right		Net @ Center Line		
Load	Load	Delta	Delta	Delta	Delta	Delta	Delta	Delta	Delta	Percent
PSF	In. H2O	@ Load	@ Rec'y	@ Load	@ Rec'y	@ Load	@ Rec'y	@ Load	@ Rec'y	Recovery
0.0	0.0	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	NA
15.6	3.0	0.313	0.000	7.625	0.094	0.406	0.000	7.266	0.094	98.7
20.8	4.0	0.313	0.016	8.063	0.094	0.438	0.000	7.688	0.086	98.9
26.0	5.0	0.313	0.016	8.625	0.188	0.500	0.000	8.219	0.180	97.8
31.2	6.0	0.375	0.016	9.125	0.219	0.531	0.000	8.672	0.211	97.6
38.5	7.4	0.375	0.016	9.750	0.125	0.594	0.031	9.266	0.102	98.9
45.5	8.8	0.406	0.016	10.531	0.172	0.688	0.063	9.984	0.133	98.7
52.5	10.1	0.469	0.031	11.266	0.563	0.813	0.188	10.625	0.453	95.7
61.1	11.8	0.938	NR	14.875	NR	1.250	NR	13.781	NR	NR

Load was held for 9 seconds at this level when the wind locks failed at the left center jamb.

13675 N.W. 19th Ave. Miami, Florida 33054
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Impact Manner of Testing: In accordance with Dade County Test Protocol PA 201-94 (rev 2) IMPACT TEST PROCEDURE.

This testing was performed as an experiment for the research and development of this product for Dade County Product Approval. One door assembly was tested, this was installed as previously described. It was impacted twice with a **9.0 lb. 2x4 of No. 2 Southern Pine** in locations as indicated in the document "Answers to questions most frequently asked about the new impact test" (by Jaime Gascon of DCPC) and a third time in a location specified by the clients engineer.

Impact Test Results

Shot	Impact Location	Impact Coordinates Rt(in), Up(in)	Firing Pressure in Hg	Impact Velocity Ft / Sec	Results
1	Right Bottom Corner	140, 12	9.88	49.6	No Penetration
2	Panel center @ Midspan	72, 33.5	10.00	50.2	No Penetration
3	Panel seam @ Midspan	73, 42.5	10.00	49.5	No Penetration

SUMMARY

One roll down door specimen manufactured by DBCI was wind loaded in accordance with **ASTM E-330** under the supervision of the clients consulting engineer. Loads were chosen to prove the adequacy of the product to sustain a design load of 25.5 PSF. In fact the product sustained a test load of 52.5 PSF adequate to prove a design load of 35 PSF.

Following wind loading undamaged portions of the specimen were subjected to three impacts in accordance with **Dade County Test Protocol PA 201-94 (ver 2.0)**. None of these impacts resulted in the penetration of the specimen.

Respectfully submitted,

CONSTRUCTION TESTING CORPORATION.
(Dade County Certification # 95-0419.02)

Report by George Dotzler : _____ 8-23-95

Test witnessed & report reviewed by John W. Knezevich, P.E. : _____

BSX - BUILDING SYSTEMS EXPRESS
Division of Metal Buildings, Inc.
115 Fairbanks Ave.
P.O. Box 860
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Engineered Assessment Solutions
15 Williamsburg Avenue, Thomasville, Georgia 31757

LETTER OF DESIGN CERTIFICATION

Date: 08/17/2005

Customer: Mr. Lee Sapp
L & L Construction
524 NE Carr Court
Lake City, FL

RE: 40' W x 75' L x 14' EH Building
Columbia County, FL
BSX Job No. XF50827

To whom it may concern:

This is to certify that the pre-fabricated metal building system, described and detailed on the documents listed below, has been designed utilizing the criteria listed on pages 1 and 2 of the attached "General Notes / Specifications". This certification is limited to the structural design of the steel framing and cladding provided and / or manufactured by BSX. This specifically does NOT include any determination of the suitability of any existing structure or foundations to resist any additional loadings as may be imposed by the new BSX structure. Design of the interaction of the new BSX structure with any existing structures is the responsibility of the owner or the owner's design professional (i.e., NOT the undersigned, nor BSX). The supplying of erection drawings and / or engineering data for the pre-fabricated metal building system does not imply or constitute an agreement that BSX or the undersigned is acting as the engineer-of-record or prime professional for the construction project. It is the responsibility of the owner or the owner's design professional to insure that his or her project specifications, and other other aspects of overall project design, are adequate for the intended building use and are in compliance with all applicable requirements of any governing authorities. When the governing building code or any governing authority requires preparation of quality assurance plans and/or special inspections for wind and seismic force resisting systems, the preparation and implementation of such plans and inspections shall be the responsibility of the owner or the owner's design professional (i.e., NOT the undersigned, nor BSX). The following is a list of the pre-fabricated metal building system construction / design documents issued by BSX for general use on this project:

General Notes / Specifications, pages 1 and 2

Reactions for Foundation Design, pages 1 thru 4

Anchor Bolt Location Drawings AB1 & AB2

Erection Drawings E1 THRU E4

When formally requested, two copies of the above documents, along with design calculations and shop fabrication drawings, will be furnished directly to the engineer-of-record or prime professional for his or her review. If this has been requested, one set of these documents shall be returned to BSX noted with comments for any required actions. If additional copies of these documents are desired, they can be furnished on a fee basis.

If there are any questions or should any additional information be needed regarding this project, please contact BSX. Business hours are from 8:00 a.m. to 5:00 p.m., Monday through Thursday, and from 8:00 a.m. to 4:00 p.m. on Fridays.

Respectfully Yours,

Richard A. Powell, P.E., FL Reg. No. 40676 08/19/05
Engineered Assessment Solutions, Inc.

General Notes / Specifications for PRE-FABRICATED METAL BUILDING SYSTEM

Project: 40' W x 75' L x 14' EH Building
Customer: L & L Construction
Job No: XF50827

1. DESIGN REFERENCES AND STANDARDS

1.1 2001 Florida Building Code

1.2 ASCE 7-98 "Minimum Design Loads for Buildings and Other Structures"

1.3 MBMA "Low Rise Building Systems Manual" 2002 Edition (Loadings to be determined from Reference 1.1 above)

1.4 AISC "Specification for Structural Steel Buildings" Allowable Stress Design and Plastic Design, June 1, 1989 with Commentary, and with Supplement No. 1, December 17, 2001

1.5 AISC "Seismic Provisions for Structural Steel Buildings", April 15, 1997 with Supplement No. 2, November 10, 2000

1.6 RCSC "Specification for Structural Joints Using ASTM A325 or A490 Bolts" Allowable Stress Design, June 23, 2000

1.7 AISI "North American Specification for the Design of Cold-Formed Steel Structural Members", 2001 Edition

1.8 SDI "Diaphragm Design Manual", First and Second Editions

1.9 ANSI / AWS D1.1-2000 "Structural Welding Code - Steel"

1.10 ANSI / AWS D1.3-1989 "Structural Welding Code - Sheet Steel"

2. DESIGN LOADS

2.1 Roof dead load taken as the weight of the framing and cladding (1.75 psf + rafter weight).

2.2 No collateral load (i.e., additional dead load) has been specified in the order documents for this building.
No special concentrated loads have been specified or included for the design of the roof system (i.e., roof top HVAC units, hoisting systems, etc.).

2.3 Design for roof live load is based on the provisions of section 1604.6 of reference 1.1 above. Roof live loads are as follows:

20.00 psf for members supporting up to 200 sf
16.00 for members supporting 201 to 600 sf
12.00 psf for members supporting over 600 sf

2.4 Per section 1605 of reference 1.1, all buildings and structures are exempt from snow load provisions.

Ground snow load, Pg, is NA
Flat-roof snow load, Pf, is NA
Snow exposure factor, Ce, is NA
Thermal factor, Ct, is NA
Roof slope factor, Cs, is NA
Snow load importance factor, Is, is NA

2.5 Design for wind load is based on the lowrise provisions of section 6.0 of reference 1.2 above. Wind load design data is as follows:

Basic wind speed, V, is 110 mph (3-sec gust)
Wind importance factor, Iw, is 1.00
Wind exposure category is B
Internal pressure coefficients, GCpi, are +0.18; -0.18

2.6 Per reference 1.1, there are no seismic provisions to be applied to this structure.

Seismic use group is NA
Seismic importance factor, Ie, is NA
Seismic design category is NA
Short period spectral response coefficient, SDS, is NA
1-second spectral response coefficient, SD1, is NA
Site class or soil profile is NA

The basic seismic-force-resisting systems used and their corresponding response modification coefficients, R, and coefficients Cs, are as follows:

R	Cs	Description of System:
NA	NA	Rigid gable frame parallel to endwalls (OMF)
NA	NA	Rigid portal frame parallel to sidewalls (OMF)
NA	NA	Concentric braced frame parallel to endwalls (OCBF)
NA	NA	Concentric braced frame parallel to sidewalls (OCBF)
NA	NA	Cantilevered fixed base bracing column parallel to sidewalls
NA	NA	Cold-formed post & beam shear wall frame parallel to endwalls
NA	NA	Metal panel shear wall diaphragm parallel to endwalls
NA	NA	Metal panel shear wall diaphragm parallel to sidewalls

The seismic base shear is equal to Cs x W, where W is the effective seismic weight of the structure. The following weights for roof and exterior wall assemblies have been used in the determination of seismic forces:

NA psf for roof (includes frames, purlins, panels & specified collateral load)
NA psf for wall at End 1
NA psf for wall at End 2
NA psf for wall at Side 1
NA psf for wall at Side 2

2.7 The load combinations used for design are those specified in section 1609.4 of reference 1.1, and section 2.4 of reference 1.2.

3. STRUCTURAL STEEL FRAMING

3.1 The steel framing members are designed to meet the standards, loadings and criteria established in sections 1 and 2 of these general notes. The framing system is a combination of Type 1 and Type 2 construction as defined in section A2 of reference 1.4. The system has been designed to resist the stresses determined from a first-order elastic analysis. Except where noted otherwise on the drawings or where otherwise indicated by the reactions furnished for foundation design, column bases have been treated as frictionless hinges in the analysis. Lateral stability for the system as a whole is provided by the following:

Yes	Rigid frame action of clearspan gable frames parallel to endwalls
No	Rigid frame action of portal frames parallel to sidewalls
No	Concentric braced frame parallel to endwalls
No	Concentric braced frame parallel to sidewalls
No	Cantilevered fixed base bracing column parallel to sidewalls
Yes	Metal panel shear wall diaphragm parallel to endwalls
Yes	Metal panel shear wall diaphragm parallel to sidewalls

Roof panels and/or bridging provide lateral stability for roof purlins, which in turn provide lateral stability for their supporting rafters. Where present, wall panels and/or bridging provide lateral stability for wall girts, which in turn provide lateral stability for their supporting columns.

3.2 Fabrication shall be in accordance with references 1.3 and 1.4 above. Unless noted otherwise in the order documents, all hot-rolled steel members shall be cleaned in accordance with SSPC - SP2 or SP7, and shall receive one shop coat of standard red primer of not less than 1.0 mil dry film thickness. Painted cold-formed components (i.e., cees, zees, etc.) shall receive one coat of polyester-based red primer, not less than 0.5 mil dry film thickness, oven baked prior to cold-forming by the component manufacturer. After cutting and welding operations, damaged areas of the oven baked primer on cold-formed members will be cleaned in accordance with SSPC-SP2 and touched up with same primer as applied to hot-rolled members.

Note: The primer shop coat applied to the structural members is designed to provide minimal short term protection of the steel during shipping and erection. If a greater level of protection is needed or desired, such protection shall be designed and provided by others, not by the building manufacturer.

3.3 Erection shall be in accordance with these plans and Section IV of reference 1.3 above. All A325 bolts need only be tightened to the snug-tight condition, as defined in reference 1.6 above, unless noted otherwise on the drawings. All field welding shall be performed by certified welders using low hydrogen electrodes.

Erection shall be performed by a qualified erector using proper tools and equipment. It shall be the responsibility of the erector to comply with all applicable laws and safety regulations. It shall be the responsibility of the erector to determine and provide any and all temporary bracing and/or securing of the system as required for stability during the entire erection process. One recognized reference on the design of erection bracing for low-rise steel buildings is AISC Design Guide No. 10.

3.4 Materials

W Shapes	ASTM A992 or A572-50 Fy = 50 ksi
Round Tubing & Pipe	ASTM A53, Gr. B Fy = 35 ksi
Square & Rectangular Tubing	ASTM A500, Gr. B Fy = 42 ksi
Channels, angles, plates & bars	ASTM A36 Fy = 36 ksi
Cold-Formed Cees, Zees & Angles (Painted)	ASTM A1011, Gr.55 Fy = 55 ksi
Cold-Formed Cees, Zees & Angles (Galvanized	ASTM A653, Gr. 50 Fy = 50 ksi

Structural bolts for primary framing shall be 5/8 inch diameter or larger, as noted on the drawings, and shall meet ASTM A325.
Structural bolts for secondary framing (e.g., purlins and girts) shall be 1/2 inch diameter, and shall meet ASTM A307 or ASTM A325 as noted on the drawings.

Welding electrodes for shop and field welds shall conform to one of the following:

SMAW process	AWS A5.1, E70XX
GMAW process	AWS A5.18, E70S-X
FCAW process	AWS A5.20, E7XT-X

Galvanized cable used for X-bracing shall conform to ASTM A475, Extra High Strength. Cable brace assemblies shall have the following minimum breaking strengths:

Strand Size	Eye Bolt Dia.	Minimum Assembly Breaking Strength (lbs)
1/4"	1/2"	6,650
5/16"	5/8"	11,200
3/8"	5/8"	15,400
7/16"	3/4"	20,800
1/2"	7/8"	26,900

4. METAL ROOF AND WALL COVERING AND TRIM

4.1 Design

The metal roof and wall systems are designed to meet the loadings, standards and criteria listed in sections 1 and 2 of these general notes. Where metal roof and wall systems are used as diaphragms, they have been designed in accordance with the general procedure outlined in reference 1.8, modified as needed to comply with the more recent requirements of references 1.2 and 1.7.

Note: When section 2.6 and/or section 3.1 of these general notes indicate that metal wall panels will be used as shear wall diaphragms, it is absolutely essential that the panels are properly fastened to their supports and to each other in order to provide the strength and stiffness required of the diaphragm assembly.

The minimum standards for fastening of metal wall panels for this project are given in sections 4.4 and 4.6 of theses general notes.

4.2 Materials

Unless noted otherwise on the drawings, the exposed surfaces of all metal roof and wall panels and trim shall be either clear acrylic coated or factory painted GALVALUME*. Acrylic coated GALVALUME* shall have a Coating Class AZ55. Factory painted GALVALUME* shall have a minimum Coating Class AZ50. GALVALUME* coated steel for roof and wall panels shall conform to ASTM A792 Grade 80. GALVALUME* coated steel for flashing and trim shall conform to ASTM A792 Grade 50. Unless noted otherwise in the order documents, factory painted GALVALUME* panels and trim shall have a baked-on silicone polyester finish coat applied to one side and a baked-on straight polyester wash coat applied to the other side. Including the primer coat, thickness of the finish coat and the wash coat shall be a nominal 1.0 and 0.5 mils respectively.

Note: GALVALUME* is a registered trademark of BIEC International, Inc.

4.3 Roof Panels

Roof panels shall be "Super Span" or "Super Span-X" panel as manufactured by WHIRLWIND BUILDING COMPONENTS or an essentially equivalent panel produced by a number of other component manufacturers and generally referred to as "R" or "PBR" panel. Unless noted otherwise on the drawings, roof panels shall be secured to framing members as follows:

Panel to framing connections at interior lines of support to be 12 inches o.c. beginning 2-1/2 to 3 inches from centerline of major panel rib. This results in one fastener for each major panel rib.

Panel to framing connections at panel ends to be in a 5 - 7 - 5 - 7 inch pattern beginning 2-1/2 to 3 inches from centerline of major panel rib. This results in two fasteners for each major panel rib.

Panel to panel connections (i.e., side laps) shall be approximately 20 inches o.c. with one fastener located in line with the panel to framing fasteners.

The first panel run at each end of the building shall be fastened to the endwall rafter, rake angle or other shear collecting member at approximately 20 inches o.c.

Where called for in the order documents and indicated on the drawings, a fiberglass insulation blanket not more than 6 inches thick may be installed between the panels and supporting framing.

4.4 Wall Panels

When provided, wall panels shall be "Super Span" panel as manufactured by WHIRLWIND BUILDING COMPONENTS or an essentially equivalent panel produced by a number of other component manufacturers and generally refered to as "R" panel. Unless noted otherwise on the drawings, wall panels shall be secured to framing members as follows:

Panel to framing connections at interior lines of support to be 12 inches o.c. beginning 2-1/2 to 3 inches from centerline of major panel rib. This results in one fastener for each major panel rib.

Panel to framing connections at panel ends to be in a 5 - 7 - 5 - 7 inch pattern beginning 2-1/2 to 3 inches from centerline of major panel rib. This results in two fasteners for each major panel rib.

Panel to panel connections (i.e., side laps) shall be approximately 16 inches o.c. with one fastener located in line with the panel to framing fasteners.

Panel runs at each end of the building and each side of openings shall be fastened to columns, door jambs or other shear collecting members at approximately 16 inches o.c.

Where called for in the order documents and indicated on the drawings, a fiberglass insulation blanket not more than 6 inches thick may be installed between the panels and supporting framing.

4.5 Roof Panel Fasteners

Roof panel to framing fasteners shall be No. 12-14 x 1-1/4 inch self-drilling, self-tapping, hex head, plated steel screw with a separate 5/8 inch O.D. formed steel washer and a neoprene sealing washer.

OR

When called for in the order documents, roof panel to framing fasteners shall be No. 12-14 x 1-1/4 inch self-drilling, self-tapping plated screw with an EPDM sealing washer housed in an undercut head (i.e., ZAC screw as manufactured by Construction Fasteners or equivalent long-life screw).

Roof panel sidelap fasteners shall be No. 14 x 7/8 inch screws of the same type as roof panel to framing fastener.

Where an insulation blanket is to be installed between the panels and supporting framing, greater length panel-to-framing screws will be furnished as needed based on the thickness of the insulation blanket.

4.6 Wall Panel Fasteners

Wall panel to framing fasteners shall be No. 12-14 x 1 inch self-drilling, self-tapping, hex head, plated steel screws (no washer) with heads painted to approximately match color of wall panels.

OR

When called for in the order documents, wall panel to framing fasteners shall be No. 12-14 x 1 inch self-drilling, self-tapping, hex head, plated steel screw with a separate 5/8 inch O.D. formed steel washer and a neoprene sealing washer.

Wall panel sidelap fasteners shall be No. 14 x 7/8 inch screws of the same type as wall panel to framing fastener.

Where an insulation blanket is to be installed between the panels and supporting framing, greater length panel-to-framing screws will be furnished as needed based on the thickness of the insulation blanket.

5. FOUNDATIONS AND BUILDING ANCHORAGE

5.1 Foundations

Unless noted otherwise in the order documents, the building manufacturer is not responsible for the design, materials and workmanship of the foundation. The anchor bolt plans prepared by the building manufacturer are intended to show only location, diameter and projection of anchor rods required to attach the building to the foundation. The manufacturer shall be responsible for providing the loads imposed on the foundation by the metal building structure. The manufacturer shall meet this requirement by furnishing the service level reactions on a load case basis; thereby allowing the foundation designer to generate as many different load combinations as needed using the principle of superpositioning. The furnishing of service level reactions on a load case basis, as opposed to a load combination basis, will also allow the foundation designer to apply the appropriate load factors for each load case in a given combination if load factored procedures will be used to design the foundation. If requested, the manufacturer will provide reactions for the allowable stress design combinations specified in section 2.7 of these notes.

5.2 Anchor Rods

Unless noted otherwise in the order documents, anchor rods shall be furnished by others, not by the building manufacturer.

The minimum anchor rod diameters indicated on the anchor bolt plans have been determined using either the shear friction approach of ACI 349 or the trilinear equation given in the AISC-ASD Specification Table J3.3 for threaded parts. Unless noted otherwise on the drawings, determination of required anchor rod diameters has assumed the use of material meeting ASTM A307, A36 or A1554 Grade 36.

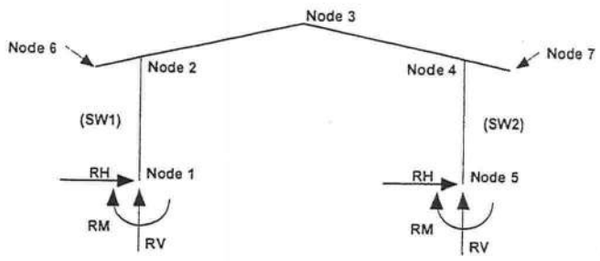
Only the interaction of shear and tension as governed by the steel strength of the anchor material has been considered in the determination of the indicated anchor diameters. Other considerations that may affect anchorage strength (i.e., concrete breakout strength of an anchor or group of anchors in shear, tension or combined shear and tension, etc.) are an integral part of concrete pedestal / footing design, and therefore must be investigated by the foundation designer, not the building manufacturer. The foundation designer may refer to section 1912 of IBC-2000 for allowable stress design of anchorage to concrete. However, this procedure is very restrictive due to the short embedment lengths listed for use in Table 1912.2. Therefore, the foundation designer may wish to use the strength design method of IBC-2000, section 1913, to obtain more economical results (Note: FBC-2001 does not include a specific method or procedure for anchorage design).

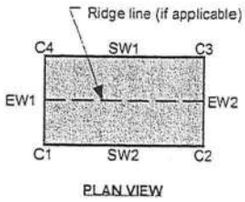
Note: If the foundation designer determines that larger diameter anchors than those indicated on the manufacturer's drawings are necessary, the purchaser shall notify the building manufacturer of the required changes, prior to returning a signed authorization to proceed with fabrication, so that column base plate design and details can be revised to accomodate the larger anchors.

REACTIONS FOR FOUNDATION DESIGN

Project: 40' W x 75' L x 14' EH Building
Customer: L & L Construction
Job No: XF50827

FRAME REACTIONS BY LOAD CASE (based on loading at Line 2):





PLAN VIEW

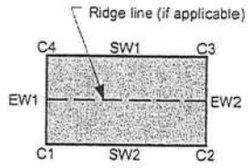
NOTE: Positive reactions act in the direction indicated above.

Load Case No.	DESCRIPTION	Node 1 RH (kips)	Node 1 RV (kips)	Node 1 RM (kip-ft)	Node 5 RH (kips)	Node 5 RV (kips)	Node 5 RM (kip-ft)	Equilibrium Check Sum VF (kips)	Sum HF (kips)	Sum M (kip-ft)
1	Dead Load	0.49	1.58	0.00	-0.49	1.58	0.00	0	0	0
2	Collateral Load									
3	Live Load	2.38	6.61	0.00	-2.38	6.61	0.00	0	0	0
4	Wind Load A, Transverse, w/ I.P. (SW1 windward)	-4.49	-8.89	0.00	-0.14	-5.52	0.00	0	0	0
5	Wind Load A, Transverse, w/ I.P. (SW2 windward)	0.14	-5.52	0.00	4.49	-8.89	0.00	0	0	0
6	Wind Load A, Transverse, w/ I.S. (SW1 windward)	-4.37	-5.24	0.00	-0.26	-1.87	0.00	0	0	0
7	Wind Load A, Transverse, w/ I.S. (SW2 windward)	0.26	-1.87	0.00	4.37	-5.24	0.00	0	0	0
8	Wind Load B, Transverse, w/ I.P. (SW1 windward)	-5.26	-11.13	0.00	0.48	-8.07	0.00	0	0	0
9	Wind Load B, Transverse, w/ I.P. (SW2 windward)	-0.48	-8.07	0.00	5.26	-11.13	0.00	0	0	0
10	Wind Load B, Transverse, w/ I.S. (SW1 windward)	-5.14	-7.47	0.00	0.36	-4.42	0.00	0	0	0
11	Wind Load B, Transverse, w/ I.S. (SW2 windward)	-0.36	-4.42	0.00	5.14	-7.47	0.00	0	0	0
12	Wind Load B, Longitudinal, w/ I.P. (SW1 windward)	-0.36	-7.97	0.00	0.63	-6.44	0.00	0	0	0
13	Wind Load B, Longitudinal, w/ I.P. (SW2 windward)	-0.63	-6.44	0.00	0.36	-7.97	0.00	0	0	0
14	Wind Load B, Longitudinal, w/ I.S. (SW1 windward)	-0.24	-4.32	0.00	0.51	-2.79	0.00	0	0	0
15	Wind Load B, Longitudinal, w/ I.S. (SW2 windward)	-0.51	-2.79	0.00	0.24	-4.32	0.00	0	0	0
16	Wind Load A, Longitudinal, w/ I.P. (SW1 windward)	-3.22	-10.21	0.00	3.35	-8.99	0.00	0	0	0
17	Wind Load A, Longitudinal, w/ I.P. (SW2 windward)	-3.35	-8.99	0.00	3.22	-10.21	0.00	0	0	0
18										
19										
20										
21										
22										
23										
24										

REACTIONS FOR FOUNDATION DESIGN

Project: 40' W x 75' L x 14' EH Building
Customer: L & L Construction
Job No: XF50827

COLUMN REACTIONS FOR ENDWALL 1 (By Load Case):



PLAN VIEW

Standard Load Cases

No.	Description
1	Dead load, D
2	Collateral load, C
3	Roof live load, Lr
4	Snow load, S
5	Roof wind load pressure, Wp
6	Roof wind load suction, Ws
7	Wall load pressure
8	Wall load suction
9	Vertical seismic, 0.2*SDS*(D+C)
10	Wall seismic, Max(0.40*Ie*SDS*wall weight, 0.10*wall weight)

NOTES:

1. Positive axial reactions indicate a compressive force at the column base.
2. Negative axial reactions indicate an up-lift force at the column base.
3. Column 1 is at Sidewall 1 (i.e., SW1) with other column numbers assigned in sequential order towards Sidewall 2 (i.e., SW2).

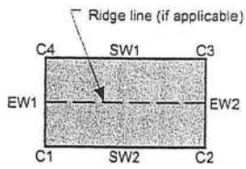
Column No.	CASE 1		CASE 2		CASE 3		CASE 4		CASE 5	
	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)
1	0.00	0.20	NA	NA	0.00	1.38	NA	NA	0.00	0.69
2	0.00	0.49	NA	NA	0.01	3.69	NA	NA	0.01	1.85
3	0.00	0.49	NA	NA	0.01	3.69	NA	NA	0.01	1.85
4	0.00	0.20	NA	NA	0.00	1.38	NA	NA	0.00	0.69
5	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
6	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
7	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
8	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
9	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
10	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
11	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
12	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
13	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
14	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
15	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
16	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
17	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
18	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
19	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
20	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
21	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA

Column No.	CASE 6		CASE 7		CASE 8		CASE 9		CASE 10	
	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)
1	0.00	-1.63	0.76	0.00	0.95	0.00	NA	NA	NA	NA
2	0.01	-4.35	1.56	0.00	1.84	0.00	NA	NA	NA	NA
3	0.01	-4.35	1.56	0.00	1.84	0.00	NA	NA	NA	NA
4	0.00	-1.63	0.76	0.00	0.95	0.00	NA	NA	NA	NA
5	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
6	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
7	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
8	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
9	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
10	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
11	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
12	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
13	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
14	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
15	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
16	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
17	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
18	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
19	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
20	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
21	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA

REACTIONS FOR FOUNDATION DESIGN

Project: 40' W x 75' L x 14' EH Building
Customer: L & L Construction
Job No: XF50827

COLUMN REACTIONS FOR ENDWALL 2 (By Load Case):



PLAN VIEW

Standard Load Cases

No.	Description
1	Dead load, D
2	Collateral load, C
3	Roof live load, Lr
4	Snow load, S
5	Roof wind load pressure, Wp
6	Roof wind load suction, Ws
7	Wall load pressure
8	Wall load suction
9	Vertical seismic, 0.2*SDS*(D+C)
10	Wall seismic, Max(0.40*Ie*SDS*wall weight, 0.10*wall weight)

NOTES:

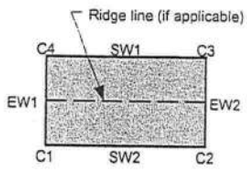
1. Positive axial reactions indicate a compressive force at the column base.
2. Negative axial reactions indicate an up-lift force at the column base.
3. Column 1 is at Sidewall 1 (i.e., SW1) with other column numbers assigned in sequential order towards Sidewall 2 (i.e., SW2).

Column No.	CASE 1		CASE 2		CASE 3		CASE 4		CASE 5	
	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)
1	0.00	0.23	NA	NA	0.01	1.70	NA	NA	0.00	0.85
2	0.00	0.46	NA	NA	0.01	3.38	NA	NA	0.00	1.69
3	0.00	0.46	NA	NA	0.01	3.38	NA	NA	0.00	1.69
4	0.00	0.23	NA	NA	0.01	1.70	NA	NA	0.00	0.85
5	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
6	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
7	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
8	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
9	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
10	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
11	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
12	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
13	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
14	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
15	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
16	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
17	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
18	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
19	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
20	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
21	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA

Column No.	CASE 6		CASE 7		CASE 8		CASE 9		CASE 10	
	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)
1	0.01	-2.00	0.83	0.00	1.01	0.00	NA	NA	NA	NA
2	0.01	-3.98	1.54	0.00	1.87	0.00	NA	NA	NA	NA
3	0.01	-3.98	1.54	0.00	1.87	0.00	NA	NA	NA	NA
4	0.01	-2.00	0.83	0.00	1.01	0.00	NA	NA	NA	NA
5	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
6	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
7	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
8	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
9	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
10	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
11	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
12	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
13	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
14	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
15	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
16	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
17	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
18	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
19	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
20	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
21	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA

REACTIONS FOR FOUNDATION DESIGN

Project: 40' W x 75' L x 14' EH Building
Customer: L & L Construction
Job No: XF50827



PLAN VIEW

METAL PANEL SHEAR WALL DIAPHRAGMS:

Wall Location	Mean Wall Hgt. (feet)	PH Wind (lbs)	PH Seismic (lbs)	Fv Wind (lbs / lf)	Fv Seismic (lbs / lf)	Lmin (Feet)	fv Wind (lbs / lf)	fv Seismic (lbs / lf)	CF Wind (lbs)	CF Seismic (lbs)	Le (Feet)	
EW1	14.83333	2,606	NA	118	NA	22.08	105.6	NA	1,567	NA	24.66667	O.K.
EW2	14.83333	2,606	NA	117	NA	22.27	90.9	NA	1,348	NA	28.66667	O.K.
SW1	14	2,219	NA	117	NA	18.97	29.9	NA	418	NA	74.33333	O.K.
SW2	14	2,219	NA	117	NA	18.97	29.9	NA	418	NA	74.33333	O.K.

Definitions:

PH = Maximum horizontal force, wind or seismic, to be resisted by diaphragm.

Fv = Allowable unit shear for specifed wall panel (factor-of-safety is 2.65 when failure mode is shear buckling; otherwise 2.0 for wind and 2.5 for seismic). It should be recognized that there is no standard allowable unit shear for a given panel profile and gage. The strength of a diaphragm system can be limited by the strength of the connections (i.e., connections at panel edge, panel side laps, or panel ends), local buckling in the panels, or by general plate-like buckling of the entire diaphragm area.

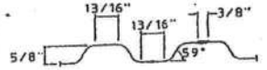
fv = Computed unit shear in diaphragm = PH / Le

Lmin = Minimum effective sheeted length of wall (Le) required to resist PH (Lmin = PH / Fv, but not less than 1/2 of wall height or 6 feet). Values of Lmin indicated above have been rounded up to the nearest foot.

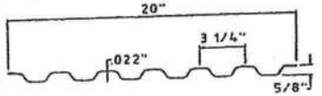
Le = Effective sheeted length of wall. This is generally taken as the full wall length, less the sum of the width of all openings in the wall and all wall segments between openings that are less than 1/2 the wall height or 6 feet in length. Where this approach is taken, diaphragm boundary members (i.e., chord members) are required at each side of each opening; otherwise, diaphragms must be evaluated for stress concentrations that will ocurr at openings.

CF = Axial load (i.e., Chord force) in diaphragm boundary member, for wind or seismic, based on length Le.

NOTE: If openings other than those indicated on the building manufacturer's panel plans are to be installed, the diaphragm must be evaluated for the resulting vertical and horizontal stress concentrations by a qualified design professional. To avoid the possibility of underdesigned diaphragms, the purchaser should provide the building manufacturer with detailed information on the size and exact location of all planned openings so that their effect can be evaluated in the original design of the pre-fabricated steel building system. The introduction of unplanned openings into the building system may, in some cases, result in a loss of column stability under lateral or lateral-torsional buckling, with possible catastrophic results.

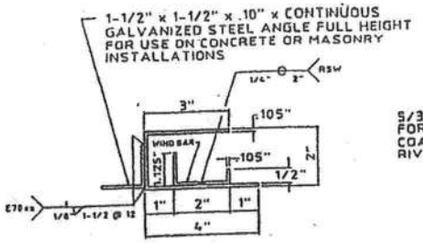


TYPICAL PANEL DETAIL
SCALE: 3" = 1'-0"



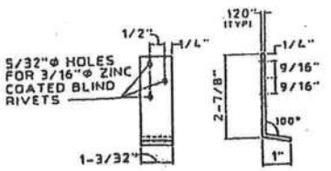
① TYPICAL OVERALL PANEL
SCALE: 1-1/2" = 1'-0"

ASTM A792 GR 80 ALUM. ZINC ALLOY COATED STEEL.



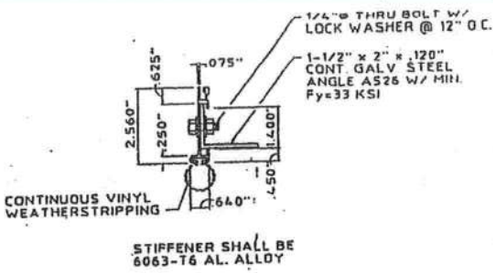
② WINDLOCK CHANNEL GUIDE
SCALE: 3" = 1'-0"

ALL COMPONENTS SHALL BE ASTM A526 STEEL W/ MIN. Fy=33 KSI GALVANIZED PER ASTM A525 G-90



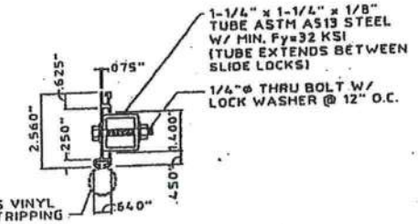
③ WINDLOCK
SCALE: 3" = 1'-0"

ASTM A526 STEEL W/ MIN. Fy=33 KSI GALVANIZED PER ASTM A525 G-90



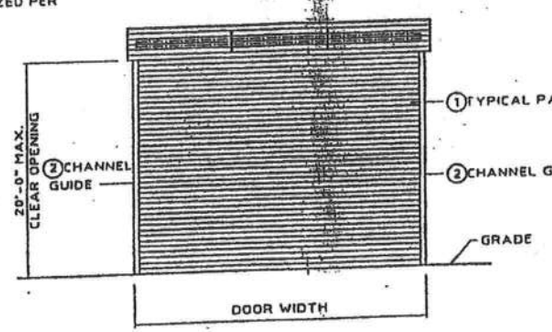
STIFFENER SHALL BE 6063-T6 AL. ALLOY

④ BOTTOM STIFFENER & ANGLE (TYPE A)
SCALE: 3" = 1'-0"

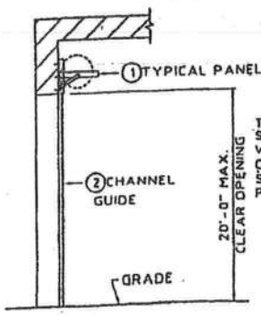


STIFFENER SHALL BE 6063-T6 AL. ALLOY

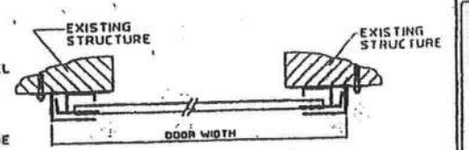
⑤ BOTTOM STIFFENER & TUBE (TYPE B)
SCALE: 3" = 1'-0"



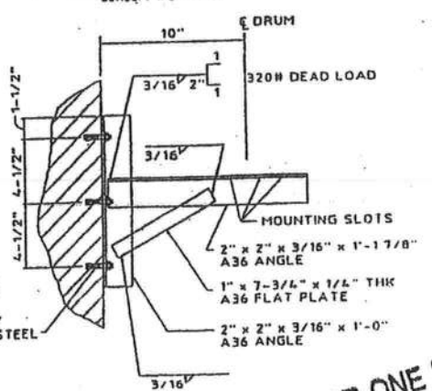
Ⓐ INTERIOR ELEVATION
N.T.S.



Ⓑ SIDE ELEVATION
N.T.S.



Ⓒ PLAN VIEW
SCALE: 1-1/2" = 1'-0"



THREE 3/8"Φ U.S. KINGPIN SLEEVE ANCHORS AS SHOWN W/ MIN. 1-1/2" EMBEDMENT IN CONCRETE OR THREE 3/8"Φ STEEL SCREWS AS SHOWN W/ PENETRATION INTO MIN. 1/8" STEEL

Ⓓ DOOR MOUNTING BRACKET DETAIL
SCALE: 1-1/2" = 1'-0"

GENERAL NOTES:

1. THIS ROLL-UP DOOR IS DESIGNED IN ACCORDANCE WITH THE STANDARD BUILDING CODE, 1991, 1994 & 1997 EDITION, INCLUDING THE 1995 PALM BEACH COUNTY AMENDMENTS.
2. THIS ROLL-UP DOOR HAS BEEN TESTED IN ACCORDANCE WITH THE STANDARD BUILDING CODE AND ASTM E-330 TO SAFELY RESIST A POSITIVE OR NEGATIVE WIND LOAD AS NOTED IN THE DOOR SCHEDULE. A TEST LOAD OF 1.5 x DESIGN LOAD HAS BEEN USED.
3. WIND LOADS FOR BUILDING OPENINGS SHALL BE DETERMINED BY A PROFESSIONAL ENGINEER USING APPROPRIATE WIND SPEED AND DESIGN CRITERIA. THIS DOOR MAY BE USED WHERE THE DESIGN LOAD MEETS OR EXCEEDS THE DESIGN LOAD FOR THE BUILDING OPENING.
4. SUPERIMPOSED LOADS ON THE JAMBS FROM THIS DOOR ARE DESIGNATED AS Vx AND Vy HEREIN. CONTRACTORS SHALL HAVE BUILDING ENGINEER VERIFY ADEQUACY OF BUILDING STRUCTURE TO RESIST SUPERIMPOSED LOADS Vx, Vy AND BRACKET LOADS SHOWN.
5. ALL WELDING SHALL BE PERFORMED BY QUALIFIED WELDERS IN ACCORDANCE WITH A.W.S SPECIFICATIONS, LATEST EDITION. ALL WELDING ELECTRODES SHALL CONFORM TO A.W.S. A5.1 GRADE E-70.
6. DOORS SHALL BE PROVIDED WITH SLIDE LOCK MECHANISMS AT EACH SIDE OF BOTTOM OF DOOR ENGAGING CHANNEL GUIDES IN THE LOCKED POSITION.

KNEZEVICH & ASSOCIATES, INC.
CONSULTING ENGINEERS • PRODUCT TESTING
1200 N. UNIVERSITY DRIVE, SUITE 100 • FORT LAUDERDALE, FL. 33322

ROLL - UP DOOR

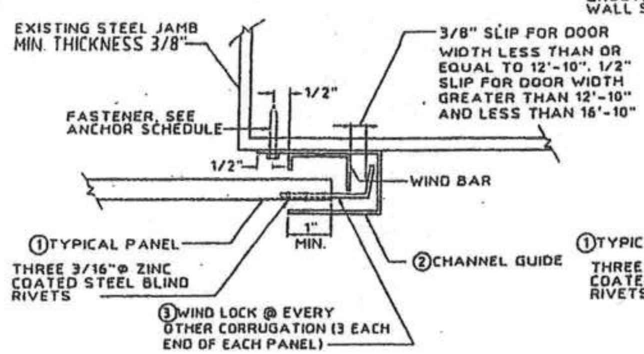
STANDARD BUILDING CODE

FOR ONE PERMIT
VALID ONLY WITH ORIGINAL SIGNATURE

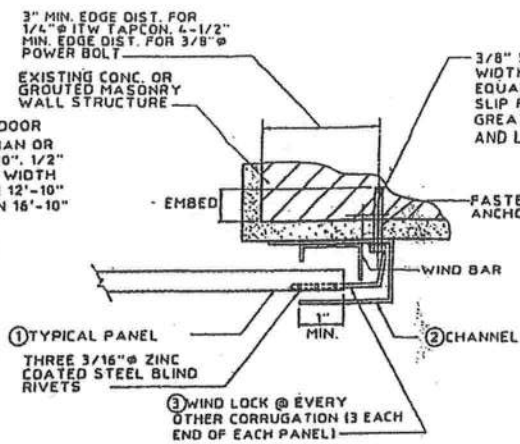
JAN 1

98-

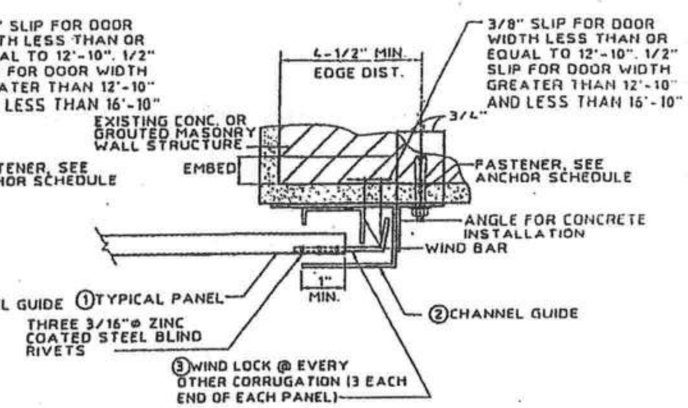
08/31/99 09:35



E1 WINDLOCK GUIDE CONNECTION TO STEEL JAMB DETAIL
SCALE: 3" = 1'-0"

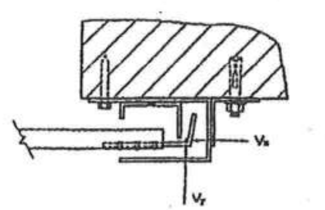


E2 WINDLOCK GUIDE CONNECTION TO CONCRETE OR GROUTED MASONRY DETAIL (INSIDE GUIDE)
SCALE: 3" = 1'-0"



E3 WINDLOCK GUIDE CONNECTION TO CONCRETE OR GROUTED MASONRY DETAIL (OUTSIDE GUIDE)
SCALE: 3" = 1'-0"

DOOR SCHEDULE				ANCHOR SCHEDULE - (FASTENER MAXIMUM SPACING)						
				DETAIL E1	DETAIL E2		DETAIL E2		DETAIL E3	
MAX. DOOR WIDTH	MAX. DESIGN PRESSURE (P.S.F.)	Vx (P.L.F.)	Vy (P.L.F.)	3/8"φ -12 x 1" HEX WASHER HEAD TYPE B THREAD FORMING ZINC PLATED STEEL SCREW PLUS A 3/8"φ FLAT WASHER	3/8"φ POWER BOLT ANCHOR WITH MINIMUM 2" EMBEDMENT		1/4"φ ITW TAPCON ANCHOR WITH MINIMUM 1-3/4" EMBEDMENT		3/8"φ U.S. KINGPIN SLEEVE ANCHOR WITH MINIMUM 1-7/8" EMBEDMENT	
				STEEL STRUCTURE	CONCRETE STRUCTURE	GROUTED MASONRY STRUCTURE (SEE NOTE No. 2)	CONCRETE STRUCTURE	GROUTED MASONRY STRUCTURE (SEE NOTE No. 2)	CONCRETE STRUCTURE	GROUTED MASONRY STRUCTURE (SEE NOTE No. 2)
≤ 12'-10"	± 35.0	810	223	12"	12"	12"	6"	4"	12"	7"
	± 26.0	478	165	14"	14"	14"	11"	6"	14"	11"
> 12'-10"	± 30.3	1169	255	8"	12"	9"	4"	N/A	8"	5"
≤ 16'-10"	± 26.0	962	218	8"	14"	11"	5"	3"	10"	6"



E SUPERIMPOSED LOAD DIAGRAM
SCALE: 3" = 1'-0"

ANCHOR NOTES:

1. EMBEDMENT LENGTH DOES NOT INCLUDE STUCCO FINISH.
2. FOR HOLLOW MASONRY, FILL ALL CELLS WITHIN 8" OF THE ANCHOR W/ 2500 PSI GROUT.
3. ANCHORS SHALL BE INSTALLED IN ACCORDANCE W/ MANUFACTURERS SPECIFICATIONS.

FOR ONE PERMIT ONLY
VALID ONLY WITH ORIGINAL SIGNATURE

STANDARD BUILDING CODE

KNEZEVICH & ASSOCIATES, INC.
CONSULTING ENGINEERS • PRODUCT TESTING
1280 N. UNIVERSITY DRIVE, SUITE 180 • FORT LAUDERDALE, FL 33322
TEL: (954) 382-2800 • FAX: (954) 382-2880 • FLORIDA COA #3305
WEBSITE: WWW.KNEZEVICH.COM • E-MAIL: KA@KNEZEVICH.COM

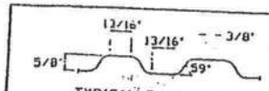
ROLL - UP DOOR

DBCI

revisions

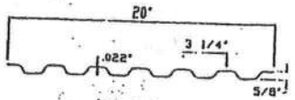
V.J. KNEZEVICH
PROFESSIONAL ENGINEER
JAN 1 0 20
scale AS NOTED
design by
drawing no. 98-176
sheet 2 of

08/31/99 09:36



TYPICAL PANEL DETAIL

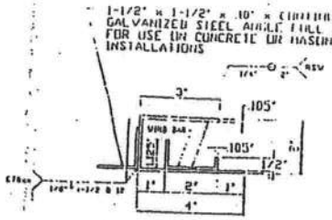
SCALE: HALF SIZE



① TYPICAL OVERALL PANEL

SCALE: 3"=1'-0"

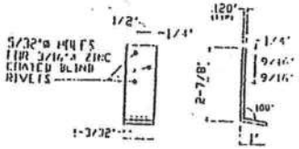
ASTM A792 GR 80 ALUM. ZINC ALLOY COATED STEEL.



② WINDLOCK CHANNEL GUIDE

SCALE: HALF SIZE

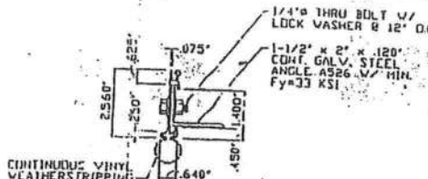
ALL COMPONENTS SHALL BE ASTM A526 STEEL W/ MIN Fy=33 KSI GALVANIZED PER ASTM A525 G-90



③ WINDLOCK

SCALE: HALF SIZE

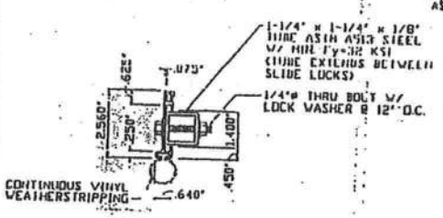
ASTM A526 STEEL W/ MIN Fy=33 KSI GALVANIZED PER ASTM A525 G-90



STIFFENER SHALL BE 6063-T6 AL. ALLOY

④ BOTTOM STIFFENER & ANGLE (TYPE A)

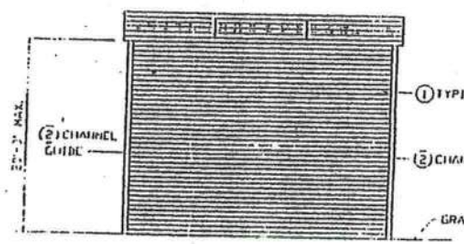
SCALE: HALF SIZE



STIFFENER SHALL BE 6063-T6 AL. ALLOY

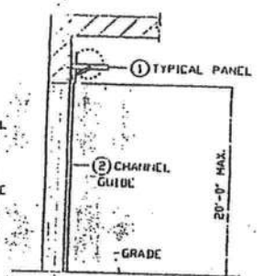
⑤ BOTTOM STIFFENER & TUBE (TYPE B)

SCALE: HALF SIZE



Ⓐ INTERIOR ELEVATION

N.T.S.



Ⓑ SIDE ELEVATION

N.T.S.

GENERAL NOTES:

1. THIS DOOR IS DESIGNED IN ACCORDANCE WITH THE STANDARD BUILDING CODE, 1991 EDITION, INCLUDING 1992 PALM BEACH COUNTY AMENDMENTS.
2. THIS DOOR HAS BEEN TESTED IN ACCORDANCE WITH THE STANDARD BUILDING CODE, SECTION 2505.3 AND ASTM E-330, TO SAFELY RESIST A POSITIVE OR NEGATIVE WIND LOAD AS NOTED IN THE DOOR SCHEDULE. A TEST LOAD OF 1.5 TIMES DESIGN LOAD HAS BEEN USED.
3. WIND LOADS FOR BUILDING OPENINGS SHALL BE DETERMINED BY A PROFESSIONAL ENGINEER USING APPROPRIATE WIND SPEED AND DESIGN CRITERIA. THIS DOOR MAY BE USED WHERE THE DESIGN LOAD MEETS OR EXCEEDS THE DESIGN LOAD FOR THE BUILDING OPENING.
4. SUPERIMPOSED LOADS ON THE JAMBS FROM THIS DOOR ARE DESIGNATED AS Vx AND Vy HEREIN. CONTRACTORS SHALL HAVE BUILDING ENGINEER VERIFY ADEQUACY OF BUILDING STRUCTURE TO RESIST SUPERIMPOSED LOADS Vx, Vy AND BRACKET LOADS SHOWN.
5. ALL WELDING SHALL BE PERFORMED BY QUALIFIED WELDERS IN ACCORDANCE WITH A.W.S. SPECIFICATIONS, LATEST EDITION. ALL WELDING ELECTRODES SHALL CONFORM TO A.W.S. A5.1 GRADE E-70
6. DOORS SHALL BE PROVIDED WITH SLIDE LOCK MECHANISMS AT EACH SIDE OF BOTTOM OF DOOR ENGAGING CHANNEL GUIDES IN THE LOCKED POSITION

KNEZEVICH & ASSOCIATES, INC.
ENGINEERS, PLANNERS & PROJECT TESTING
641 POINCIANA DRIVE
MIAMI SPRINGS, FLORIDA 33166
PHONE (305) 883-9571

ROLL-UP DOOR

D.B.C.I.

4300 INDUSTRIAL ACCESS ROAD
DOUGLASVILLE, GA. 30134
(770) 942-9581

This Dwg shall not be used as a master

Exp DEC 2 5 1995

JOHN V. KNEZEVICH
PROFESSIONAL ENGINEER
FL LICENSE NO.
PE 0041961

AUG 2 1995

95-430

CONSTRUCTION TESTING CORPORATION
13873 N.W. 19th Ave. Miami, Florida 33054
Phone: (305) 685-6657 Fax: (305) 685-6659

Report No: 95-029

23 August 1995

Test Date: 17 August 1995

TESTS ON ROLL DOWN DOORS

Client:

Door & Building Components, Inc.

4310 Industrial Access Road
Douglasville, Georgia 30134

General : Uniform Static Air Pressure Loading, per ASTM E-330

Testing witnessed by:

John W. Knezevich, P.E. Knezevich & Ass.
Don Mills, Product Engineer for D.B.C.I.
Bill Mathews, President J. B. Mathews
George Dotzler, CTC Test Engineer

Statement of Conformance : This is a general statement and does not supersede the specific product descriptions in this report. The specimens are in conformance with drawings provided by the manufacturer, labeled:

ROLL - UP DOOR

D.B.C.I.

Door & Building Components, Inc.
4310 Industrial Access Road
Douglasville, Georgia 30134

Date : 8 - 23 - 95 Drawing # 95 - 430

Description of Test Specimen: The specimen was a roll down door manufactured by Door & Building Components, Inc. . This door was installed covering a nominal opening 16'-0" wide by 16'-0" high. The door was constructed of painted galvanized steel sheet (mic'd @ 0.0240" w/ galvanized, w/o paint). The specimens channel guides were secured to the steel jamb (1/4" steel plate) of the test chamber with 9/16" hex head self threading 3/8" x 1" screws at 4" on center. These channel guides were as shown in detail 2 "Windlock Channel Guide" of the manufacturers supplied drawings. The left guide fastenings were secured through the 1-1/2" x 1-1/2" angle typically used for concrete installations. The right channel guide did not include this feature (the 1-1/2" x 1-1/2" angle) and the fastenings were secured through the center of the 1" protruding flange as shown detail 2. The door's bottom bar was as shown in detail 5 "Bottom Stiffener & Angle (Type B)" in the manufacturers drawings. Before testing this door was fully functional.

CONSTRUCTION TESTING CORPORATION
13873 N.W. 19th Ave. Miami, Florida 33054
Phone: (305) 685-6657 Fax: (305) 685-6659

Static Wind Loading / Manner of Testing:

Loads applied to the specimen (10 seconds durations in loading cycles greater than 40 seconds) were at levels specified by the client's Consulting Engineer. Polyethylene film (2 mil) and tape were used to seal air leakage during loads. The film and tape were used in a manner that did not influence the results. Deflection gauges were mounted at each jamb to record deflections along the center line of the door. The deflection readings are as follows:

		Left		Center		Right		Net @ Center Line		
Load	Load	Delta	Delta	Delta	Delta	Delta	Delta	Delta	Delta	Percent
PSF	In. H2O	.@ Load	.@ Rec'y	.@ Load	.@ Rec'y	.@ Load	.@ Rec'y	.@ Load	.@ Rec'y	Recovery
0.0	0.0	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	NA
15.6	3.0	0.500	0.063	11.438	0.188	0.500	0.016	10.938	0.148	98.6
20.8	4.0	0.531	0.063	12.063	0.125	0.516	0.156	11.539	0.016	99.9
26.0	5.0	0.531	0.031	13.000	0.063	0.563	0.031	12.453	0.031	99.7
31.2	6.0	0.563	0.031	13.750	0.125	0.625	0.063	13.156	0.078	99.4
38.5	7.4	0.625	0.125	14.938	0.313	0.750	0.063	14.250	0.219	98.5
45.5	8.8	0.688	0.156	16.250	0.750	0.875	0.063	15.469	0.641	95.9
52.5	10.1	NR	NR	NR	NR	NR	NR	NR	NR	NR

As loading was initiated it momentarily rose to approximately 55 PSF then imediately reduced to the desired level.

The correct load was held for approximately 4 seconds when the windlocks failed at the center of the right jamb.

SUMMARY

One roll down door specimen manufactured by DBCI was wind loaded in accordance with **ASTM** E-330 under the supervision of the clients consulting engineer. Loads were chosen to prove the adequacy of the product to sustain a design load of 25.5 PSF. In fact the sustained test load of 45.5 PSF proved the product to a design load of 30.3 PSF.

Respectfully submitted,

CONSTRUCTION TESTING CORPORATION.
(Dade County Certification # 95-0419.02)

Report by George Dotzler : _____

Test witnessed & report reviewed by John W. Knezevich, P.E. : _____

13873 N.W. 19th Ave. Miami, Florida 33054
Phone: (305) 685-6657 Fax: (305) 685-6659

Static Wind Loading / Manner of Testing:

Loads applied to the specimen (10 seconds durations in loading cycles greater than 40 seconds) were at levels specified by the client's Consulting Engineer. Polyethylene film (2 mil) and tape were used to seal air leakage during loads. The film and tape were used in a manner that did not influence the results. Deflection gauges were mounted at each jamb to record deflections along the center line of the door. The deflection readings are as follows:

		Left		Center		Right		Net @ Center Line		
Load	Load	Delta	Delta	Delta	Delta	Delta	Delta	Delta	Delta	Percent
PSF	In. H2O	@ Load	@ Rec'y	@ Load	@ Rec'y	@ Load	@ Rec'y	@ Load	@ Rec'y	Recovery
0.0	0.0	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	NA
15.6	3.0	0.313	0.000	7.625	0.094	0.406	0.000	7.266	0.094	98.7
20.8	4.0	0.313	0.016	8.063	0.094	0.438	0.000	7.688	0.086	98.9
26.0	5.0	0.313	0.016	8.625	0.188	0.500	0.000	8.219	0.180	97.8
31.2	6.0	0.375	0.016	9.125	0.219	0.531	0.000	8.672	0.211	97.6
38.5	7.4	0.375	0.016	9.750	0.125	0.594	0.031	9.266	0.102	98.9
45.5	8.8	0.406	0.016	10.531	0.172	0.688	0.063	9.984	0.133	98.7
52.5	10.1	0.469	0.031	11.266	0.563	0.813	0.188	10.625	0.453	95.7
61.1	11.8	0.938	NR	14.875	NR	1.250	NR	13.781	NR	NR

Load was held for 9 seconds at this level when the wind locks failed at the left center jamb.

13675 N.W. 19th Ave. Miami, Florida 33054
Phone: (305) 685-6657 Fax: (305) 685-6659

Impact Manner of Testing: In accordance with Dade County Test Protocol PA 201-94 (rev 2) IMPACT TEST PROCEDURE.

This testing was performed as an experiment for the research and development of this product for Dade County Product Approval. One door assembly was tested, this was installed as previously described. It was impacted twice with a **9.0 lb. 2x4 of No. 2 Southern Pine** in locations as indicated in the document "Answers to questions most frequently asked about the new impact test" (by Jaime Gascon of DCPC) and a third time in a location specified by the clients engineer.

Impact Test Results

Shot	Impact Location	Impact Coordinates Rt(in), Up(in)	Firing Pressure in Hg	Impact Velocity Ft / Sec	Results
1	Right Bottom Corner	140, 12	9.88	49.6	No Penetration
2	Panel center @ Midspan	72, 33.5	10.00	50.2	No Penetration
3	Panel seam @ Midspan	73, 42.5	10.00	49.5	No Penetration

SUMMARY

One roll down door specimen manufactured by DBCI was wind loaded in accordance with **ASTM E-330** under the supervision of the clients consulting engineer. Loads were chosen to prove the adequacy of the product to sustain a design load of 25.5 PSF. In fact the product sustained a test load of 52.5 PSF adequate to prove a design load of 35 PSF.

Following wind loading undamaged portions of the specimen were subjected to three impacts in accordance with **Dade County Test Protocol PA 201-94 (ver 2.0)**. None of these impacts resulted in the penetration of the specimen.

Respectfully submitted,

CONSTRUCTION TESTING CORPORATION.
(Dade County Certification # 95-0419.02)

Report by George Dotzler : _____ 8-23-95

Test witnessed & report reviewed by John W. Knezevich, P.E. : _____

BUILDING SYSTEMS EXPRESS

Division of Metal Buildings, Inc.

Mailing Address:
P.O. Box 860
Thomasville, GA 31799-0860

Factory location:
115 Fairbanks Ave.
Thomasville, GA 31792

Telephone:
1-800-279-9455
Fax (229) 226-6874
E-mail: mbi@rose.net

Effective: January 3, 2005

BSX building packages incorporate one or more of the following products:

Vendor & Florida Approval Information

Steel Roof & Wall Panels

Whirlwind Building Systems
FL 1845.3 Superspan & Superspan-X Roof Panel
FL 2489.1 Superspan Wall Panel

Steel Walkdoors

Premier Products, Inc
FL 3661.2 3070 Reversible Door

Skylites/Wall Lights - Light Transmitting Panels

Paltough Ltd/Suntuf Inc
MIAMI-DADE NOA 00-1226.02 Polycarbonate Sheets

SUWANNEE
RIVER
WATER
MANAGEMENT
DISTRICT
9225 CR 49
LIVE OAK, FLORIDA 32060
TELEPHONE: (386) 362-1001
TELEPHONE: 800-226-1066
FAX (386) 362-1056

GENERAL PERMIT

PERMITTEE:
MORRELLS, INC.
461 SOUTHWEST DEPUTY J. DAVIS LANE
LAKE CITY, FL 32024

PERMIT NUMBER: ERP89-0162M2
DATE ISSUED: 10/24/2005
DATE EXPIRES: 10/24/2008
COUNTY: COLUMBIA
TRS: S34/T3S/R16E

PROJECT: MORRELL'S INC. MODIFICATION

Approved entity to whom operation and maintenance may be transferred pursuant to rule 40B-4.1130, Florida Administrative Code (F.A.C.):

DAVID MORRELL
MORRELLS, INC.
461 SOUTHWEST DEPUTY J. DAVIS LANE
LAKE CITY, FL 32024

Based on information provided, the Suwannee River Water Management District's (District) rules have been adhered to and an environmental resource general permit is in effect for the permitted activity description below:

Previous permit issued for 1.41 acres of impervious surface on 29.00 acres. Modification consists of construction and operation of a surfacewater management system serving 2.82 acres of impervious surface on a total project area of 29.00 acres in a manner consistent with the application package submitted by Curtis Keen of Keen Engineering & Surveying, Inc. certified on September 30, 2005.

It is your responsibility to ensure that adverse off-site impacts do not occur either during or after construction. Any additional construction or alterations not authorized by this permit may result in flood control or water quality problems both on and off site and will be a violation of District rule.

You or any other substantially affected persons are entitled to request an administrative hearing pursuant to ss.120.57(1), Florida Statutes (F.S.), and s.40B-1.511, F.A.C., if they object to the District's actions. Failure to request a hearing within 14 days will constitute a waiver of your right

to request such a hearing. In addition, the District will presume that permittee waives Chapter 120, F.S., rights to object or appeal the action upon commencement of construction authorized by the permit.

This permit is issued under the provisions of chapter 373, F.S., chapter 40B-4, and chapter 40B-400, F.A.C. A general permit authorizes the construction, operation, maintenance, alteration, abandonment, or removal of certain minor surface water management systems. This permit authorizes the permittee to perform the work necessary to construct, operate, and maintain the surface water management system shown on the application and other documents included in the application. This is to notify you of District's agency action concerning Notice Of Intent. This action is taken pursuant to rule 40B-4 and 40B-400, F.A.C.

Standard Conditions for All General Permits:

1. The permittee shall perform all construction authorized in a manner so as to minimize adverse impacts to fish, wildlife, natural environmental values, and water quality. The permittee shall institute necessary measures during construction including riprap, reinforcement, or compaction of any fill materials placed around newly installed structures, to minimize erosion, turbidity, nutrient loading, and sedimentation in the receiving waters.

2. Water quality data representative of the water discharged from the permitted system, including, but not limited to, the parameters in chapter 62-302, F.A.C., shall be submitted to the District as required. If water quality data are required, the permittee shall provide data as required on the volume and rate of discharge including the total volume discharged during the sampling period. All water quality data shall be in accordance with and reference the specific method of analysis in "Standard Methods for the Examination of Water and Wastewater" by the American Public Health Association or "Methods for Chemical Analysis of Water and Wastes" by the U.S. Environmental Protection Agency.

3. The operational and maintenance phase of an environmental resource permit will not become effective until the owner or his authorized agent certifies that all facilities have been constructed in accordance with the design permitted by the District. If required by the District, such as-built certification shall be made by an engineer or surveyor. Within 30 days after the completion of construction of the system, the permittee shall notify the District that the facilities are complete. If appropriate, the permittee shall request transfer of the permit to the responsible entity approved by the District for operation and maintenance. The District may inspect the system and, as necessary, require remedial measures as a condition of transfer of the permit or release for operation and maintenance of the system.

4. Off-site discharges during and after construction shall be made only through the facilities

authorized by the permit. Water discharged from the project shall be through structures suitable for regulating upstream stage if so required by the District. Such discharges may be subject to operating schedules established by the District.

5. The permit does not convey to the permittee any property right nor any rights or privileges other than those specified in the permit and chapter 40B-1, F.A.C.

6. The permittee shall hold and save the District harmless from any and all damages, claims, or liabilities which may arise by reason of the construction, operation, maintenance, alteration, abandonment, or development in a Works of the District which is authorized by the permit.

7. The permit is issued based on the information submitted by the applicant which reasonably demonstrates that adverse off-site water resource impacts will not be caused by the permitted activity. It is the responsibility of the permittee to insure that such adverse impacts do not in fact occur either during or after construction.

8. It is the responsibility of the permittee to obtain all other clearances, permits, or authorizations required by any unit of local, state, or federal government.

9. The surfacewater management system shall be constructed prior to or concurrent with the development that the system is intended to serve and the system shall be completed within 30 days of substantial completion of the development which the system is intended to serve.

10. Except for General Permits After Notice or permits issued to a unit of government, or unless a different schedule is specified in the permit, the system shall be inspected at least once every third year after transfer of a permit to operation and maintenance by the permittee or his agent to ascertain that the system is being operated and maintained in a manner consistent with the permit. A report of inspection is to be sent to the District within 30 days of the inspection date. If required by chapter 471, F.S., such inspection and report shall be made by an engineer.

11. The permittee shall allow reasonable access to District personnel or agents for the purpose of inspecting the system to insure compliance with the permit. The permittee shall allow the District, at its expense, to install equipment or devices to monitor performance of the system authorized by their permit.

12. The surfacewater management system shall be operated and maintained in a manner which is consistent with the conditions of the permit and chapter 40B-4.2040, F.A.C.

13. The permittee is responsible for the perpetual operation and maintenance of the system unless the operation and maintenance is transferred pursuant to chapter 40B-4.1130, F.A.C., or the permit

is modified to authorize a new operation and maintenance entity pursuant to chapter 40B-4.1110, F.A.C.

14. All activities shall be implemented as set forth in the plans, specifications and performance criteria as approved by this permit. Any deviation from the permitted activity and the conditions for undertaking that activity shall constitute a violation of this permit.

15. This permit or a copy thereof, complete with all conditions, attachments, exhibits, and modifications, shall be kept at the work site of the permitted activity. The complete permit shall be available for review at the work site upon request by District staff. The permittee shall require the contractor to review the complete permit prior to commencement of the activity authorized by this permit.

16. Activities approved by this permit shall be conducted in a manner which do not cause violations of state water quality standards.

17. Prior to and during construction, the permittee shall implement and maintain all erosion and sediment control measures (best management practices) required to retain sediment on-site and to prevent violations of state water quality standards. All practices must be in accordance with the guidelines and specifications in the Florida Stormwater, Erosion, and Sedimentation Control Inspector's Manual unless a project specific erosion and sediment control plan is approved as part of the permit, in which case the practices must be in accordance with the plan. If site-specific conditions require additional measures during any phase of construction or operation to prevent erosion or control sediment, beyond those specified in the erosion and sediment control plan, the permittee shall implement additional best management practices as necessary, in accordance with the Florida Stormwater, Erosion, and Sedimentation Control Inspector's Manual. The permittee shall correct any erosion or shoaling that causes adverse impacts to the water resources.

18. Stabilization measures shall be initiated for erosion and sediment control on disturbed areas as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, but in no case more than seven days after the construction activity in that portion of the site has temporarily or permanently ceased.

19. At least 48 hours prior to commencement of activity authorized by this permit, the permittee shall submit to the District a Construction Commencement Notice Form No. 40B-1.901(14) indicating the actual start date and the expected completion date.

20. When the duration of construction will exceed one year, the permittee shall submit construction status reports to the District on an annual basis utilizing an Annual Status Report Form No. 40B-1.901(15). These forms shall be submitted during June of each following year.

21. For those systems which will be operated or maintained by an entity requiring an easement or deed restriction in order to provide that entity with the authority necessary to operate or maintain the system, such easement or deed restriction, together with any other final operation or maintenance documents as are required by Paragraph 40B-4.2030(2)(g), F.A.C., and Rule 40B-4.2035, F.A.C., must be submitted to the District for approval. Documents meeting the requirements set forth in these subsections of District rules will be approved. Deed restrictions, easements and other operation and maintenance documents which require recordation either with the Secretary of State or Clerk of the Circuit Court must be so recorded prior to lot or unit sales within the project served by the system, or upon completion of construction of the system, whichever occurs first. For those systems which are proposed to be maintained by county or municipal entities, final operation and maintenance documents must be received by the District when maintenance and operation of the system is accepted by the local governmental entity. Failure to submit the appropriate final documents referenced in this paragraph will result in the permittee remaining liable for carrying out maintenance and operation of the permitted system.

22. Each phase or independent portion of the permitted system must be completed in accordance with the permitted plans and permit conditions prior to the initiation of the permitted use of site infrastructure located within the area served by that portion or phase of the system. Each phase or independent portion of the system must be completed in accordance with the permitted plans and permit conditions prior to transfer of responsibility for operation and maintenance of that phase or portion of the system to a local government or other responsible entity.

23. Within 30 days after completion of construction of the permitted system, or independent portion of the system, the permittee shall submit a written statement of completion and certification by a registered professional engineer or other appropriate individual as authorized by law, using the supplied As-Built Certification Form No. 40B-1.901(16) incorporated by reference in Subsection 40B-1.901(16), F.A.C. When the completed system differs substantially from the permitted plans, any substantial deviations shall be noted and explained and two copies of as-built drawings submitted to the District. Submittal of the completed form shall serve to notify the District that the system is ready for inspection. The statement of completion and certification shall be based on on-site observation of construction (conducted by the registered professional engineer, or other appropriate individual as authorized by law, or under his or her direct supervision) or review of as-built drawings for the purpose of determining if the work was completed in compliance with approved plans and specifications. As-built drawings shall be the permitted drawings revised to reflect any changes made during construction. Both the original and any revised specifications must be clearly shown. The plans must be clearly labeled as "as-built" or "record" drawing. All surveyed dimensions and elevations shall be certified by a registered surveyor. The following information, at a minimum, shall be verified on the as-built drawings:

a. Dimensions and elevations of all discharge structures including all weirs, slots, gates, pumps,

pipes, and oil and grease skimmers;

b. Locations, dimensions, and elevations of all filter, exfiltration, or underdrain systems including cleanouts, pipes, connections to control structures, and points of discharge to the receiving waters;

c. Dimensions, elevations, contours, or cross-sections of all treatment storage areas sufficient to determine stage-storage relationships of the storage area and the permanent pool depth and volume below the control elevation for normally wet systems, when appropriate;

d. Dimensions, elevations, contours, final grades, or cross-sections of the system to determine flow directions and conveyance of runoff to the treatment system;

e. Dimensions, elevations, contours, final grades, or cross-sections of all conveyance systems utilized to convey off-site runoff around the system;

f. Existing water elevation(s) and the date determined; and

g. Elevation and location of benchmark(s) for the survey.

24. The operation phase of this permit shall not become effective until the permittee has complied with the requirements of the condition in paragraph 23 above, the District determines the system to be in compliance with the permitted plans, and the entity approved by the District in accordance with Rule 40B-4.2035, F.A.C., accepts responsibility for operation and maintenance of the system. The permit may not be transferred to such approved operation and maintenance entity until the operation phase of the permit becomes effective. Following inspection and approval of the permitted system by the District, the permittee shall request transfer of the permit to the approved responsible operation and maintenance operating entity if different from the permittee. Until the permit is transferred pursuant to Rule 40B-4.1130, F.A.C., the permittee shall be liable for compliance with the terms of the permit.

25. Should any other regulatory agency require changes to the permitted system, the permittee shall provide written notification to the District of the changes prior to implementation so that a determination can be made whether a permit modification is required.

26. This permit does not eliminate the necessity to obtain any required federal, state, local and special District authorizations prior to the start of any activity approved by this permit. This permit does not convey to the permittee or create in the permittee any property right, or any interest in real property, nor does it authorize any entrance upon or activities on property which is not owned or controlled by the permittee, or convey any rights or privileges other than those specified in the permit and in this chapter and Chapter 40B-4, F.A.C.

27. The permittee is hereby advised that Section 253.77, F.S., states that a person may not commence any excavation, construction, or other activity involving the use of sovereign or other lands of the state, the title to which is vested in the Board of Trustees of the Internal Improvement Trust Fund without obtaining the required lease, license, easement, or other form of consent authorizing the proposed use. Therefore, the permittee is responsible for obtaining any necessary authorizations from the Board of Trustees prior to commencing activity on sovereignty lands or other state-owned lands.

28. Any delineation of the extent of a wetland or other surface water submitted as part of the permit application, including plans or other supporting documentation, shall not be considered specifically approved unless a specific condition of this permit or a formal determination under 40B-400.046, F.A.C., provides otherwise.

29. The permittee shall notify the District in writing within 30 days of any sale, conveyance, or other transfer of ownership or control of the permitted system or the real property at which the permitted system is located. All transfers of ownership or transfers of a permit are subject to the requirements of Rule 40B-4.1130, F.A.C. The permittee transferring the permit shall remain liable for any corrective actions that may be required as a result of any permit violations prior to such sale, conveyance or other transfer.

30. If historical or archaeological artifacts are discovered at any time on the project site, the permittee shall immediately notify the District.

31. The permittee shall immediately notify the District in writing of any previously submitted information that is later discovered to be inaccurate.

WITHIN 30 DAYS AFTER COMPLETION OF THE PROJECT, THE PERMITTEE SHALL NOTIFY THE DISTRICT, IN WRITING, THAT THE FACILITIES ARE COMPLETE.

Approved by John Hutt
District Staff

Date Approved 10-24-05

Timothy J. Sagul
Clerk

Executive Director

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

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October 28, 2005

L and L Construction
524 N. W. Carr Court
Lake City, Florida 32055

Attention: Lee Sapp

Reference: Proposed Replacement Building and Building Addition
Morrell's
Lake City, Florida
Cal-Tech Project No. 05-515

Dear Mr. Sapp,

Cal-Tech Testing, Inc. has completed the subsurface investigation and engineering evaluation of the sites for a proposed building addition and a replacement building to be constructed at Morrell's in Lake City, Florida. Our work was performed in conjunction with and authorized by you.

Introduction

We understand you will construct a single-story, metal building addition with lateral dimensions of approximately 30 feet by 150 feet. This addition will abut the north side of an existing metal building of length 150 feet, approximately. Also, an existing pole building is to be removed and replaced with a metal building having lateral dimensions of approximately 40 feet by 75 feet. Support for the new structures is to be provided by monolithic foundations or by conventional, shallow spread footings. Anticipated foundation loads have not been provided; however, we assume column and wall loads will not exceed 30 kips and 1.5 kips per foot, respectively. Additionally, we assume the finished floor elevation of the addition will match the floor elevation of the existing building. We assume the floor of the replacement building will be located no more than about 1 foot above the average existing surface grade.

The site of the building addition is open, and the ground surface appears to slope very gently south toward the existing building. The ground surface also slopes very gently to the south in the area of the new building. We believe no more than about 1 foot of fill will be required to level each site.

The purposes of our investigation were to determine the general subsurface conditions at the proposed building sites and to provide recommendations for foundation design and construction.

Site Investigation

The subsurface conditions were investigated by performing seven (7) Standard Penetration Test borings advanced to depths of 10 feet. The borings were performed at the approximate locations indicated on the attached Boring Location Plan. These locations were selected and staked by Cal-Tech Testing, Inc. with your assistance.

The Standard Penetration Test (ASTM D-1586) is performed by driving a standard split-barrel sampler into the soil by blows of a 140-pound hammer falling 30 inches. The number of blows required to drive the sampler 1 foot, after seating 6 inches, is designated the penetration resistance, or N-value; this value is an index to soil density or consistency.

Findings

The soil borings encountered two soil strata. The first layer consists of 4 to 10 or more feet of generally very loose to loose, tannish gray, grayish tan or gray and orange sand (SP) or sand with silt (SP/SM). The N-values of this layer range from 2 to 11 blows per foot. The second layer consists of an undetermined thickness of loose to medium dense, generally tannish gray or gray and orange, clayey sand (SC). The N-values of this layer range from 4 to 29 blows per foot.

Groundwater was not encountered at any boring location at the time of our investigation, and we estimate the seasonal high groundwater table will occur at a depth of more than 6 feet. For a more detailed description of the subsurface conditions encountered, please refer to the attached Boring Logs. In the field the transition between soil layers may be gradual and not abrupt as indicated by the logs; therefore, the thickness of soil layers should be considered approximate.

Discussion and Recommendations

From the results of our investigation, it is our opinion the proposed structures can be supported by monolithic foundations or by conventional, shallow spread footings sized to exert a maximum soil bearing pressure of 2,000 pounds per square foot. These foundations or thickened sections should have minimum widths of 18 and 24 inches at wall and column locations, respectively, and the bottoms of foundations generally should be embedded at least 16 inches below the finished surface grade. For the addition the foundations may be placed to match the existing foundations. Due to the generally very loose to loose condition of the near surface soils, we recommend site preparation be particularly thorough.

The existing pole building should be removed. Both building areas should then be stripped of grass, roots and other deleterious materials. Excavation should then be performed as required to establish the proposed foundation and floor bottom grades. Clean, sandy soil should be stockpiled for later use as fill.

The subgrade areas should then be thoroughly proof-rolled with heavy rubber-tired equipment (a large, loaded, front-end loader, for example). Proof rolling helps to compact the bearing soils and to locate zones of especially loose or soft soil that may be present. Such zones should be undercut and back-filled or otherwise treated as directed by the geotechnical engineer.

Following proof-rolling of the sites, the subgrade should be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density to a depth of 2 feet in foundation areas and to a depth of 1 foot in floor slab areas.

The existing metal building should be monitored for movement during proof-rolling and proof-compaction of the addition area. If movement is noted compaction procedures should be temporarily stopped, and the geotechnical engineer should be notified for evaluation of the site and for recommendations of alternative compaction procedures as required.

Fill to raise the sites can be placed as required following proof-rolling and proof-compaction operations. Fill should consist of relatively clean, fine sand containing less than 10% passing the No. 200 sieve. Fill should be placed in maximum 12-inch, loose lifts, and each lift should be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density. Foundation cuts may be placed in the compacted fill if desired. Disturbed fill materials should be recompacted prior to placement of the foundations or floor slabs.

Field density testing should be performed in the compacted subgrade, in each lift of fill, and in foundation excavations to verify the recommended compaction has been achieved.

Our recommendations are based upon our findings as described in this report; however, subsurface conditions may exist that were not encountered in the soil test borings. Cal-Tech Testing, Inc. should be notified immediately if different soil conditions are encountered during construction. It may be necessary to reevaluate these building sites and revise our recommendations.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer
President / C.E.O.

John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer 10/28/05
52612

B-1

Water Table: N/A

Depth (ft)	N-value	Soil Description
0	3	Very Loose, Dark Tannish Grey Sand with Silt (SP/SM)
	3	Very Loose, Tannish Grey Sand (SP)
5	5	Loose, Light Greyish Tan Sand (SP)
	7	Loose, Light Greyish Tan and Orange Sand (SP)
	9	Loose, Light Orangish Tan Sand, Trace Clay (SP)
10	11	Medium Dense, Light Greyish Tan Sand with Clay (SP/SC)

B-2

Water Table: N/A

Depth (ft)	N-value	Soil Description
0	2	Dark Tannish Grey Sand with Silt, Trace Organics (SP/SM)
	2	Very Loose, Tannish Grey Sand with Silt (SP/SM)
5	4	Very Loose, Light Tannish Grey Sand with Silt (SP/SM)
	10	Loose, Light Tannish Grey and Orange, Clayey Sand (SC)
	20	Loose, Light Tannish Grey and Light Orange to Orange, Slightly Clayey Sand (SC)
10	24	Medium Dense, Light Tannish Grey and Light Orange, Slightly Clayey Sand (SC)

B-3

Water Table: N/A

Depth (ft)	N-value	Soil Description
0	3	Dark Tannish Grey, Silty Sand, Trace Organics (SM)
	4	Very Loose, Light Greyish Tan Sand with Silt (SP/SM)
5	4	Loose, Light Tannish Grey to Light Grey Sand (SP)
	14	Loose, Light Orangish Tan, Slightly Clayey Sand (SC)
	20	Medium Dense, Light Grey and Light Orange, Clayey Sand (SC)
10	22	Medium Dense, Light Tannish Grey Sand, Trace Clay (SP)

B-4

Water Table: N/A

Depth (ft)	N-value	Soil Description
0	2	Dark Tannish Grey Sand with Silt (SP/SM)
	4	Very Loose, Light Tannish Grey Sand with Silt (SP/SM)
5	4	Loose, Light Greyish Tan Sand (SP)
	7	Loose, Light Grey Sand (SP)
	16	Loose, Light Tannish Grey and Orange, Clayey Sand (SC)
10	20	Medium Dense, Light Grey and Light Orange, Clayey Sand (SC)
		Medium Dense, Light Tannish Grey, Slightly Clayey Sand (SC)

Boring Logs: Proposed Buildings
Morrell's

B-5

Water Table: N/A

Depth (ft)	N-value	Soil Description
0		Dark Tannish Grey Sand with Silt (SP/SM)
	10	Loose, Tannish Grey Sand with Silt (SP/SM)
	6	Loose, Light Greyish Tan, Fine Sand with Silt (SP/SM)
5	4	Loose, Light Tannish Grey and Orange, Clayey Sand (SC)
	10	Loose, Light Greyish Tan and Orange, Clayey Sand (SC)
	19	Medium Dense, Light Tannish Grey and Orange, Clayey Sand (SC)
	29	
10		

B-6

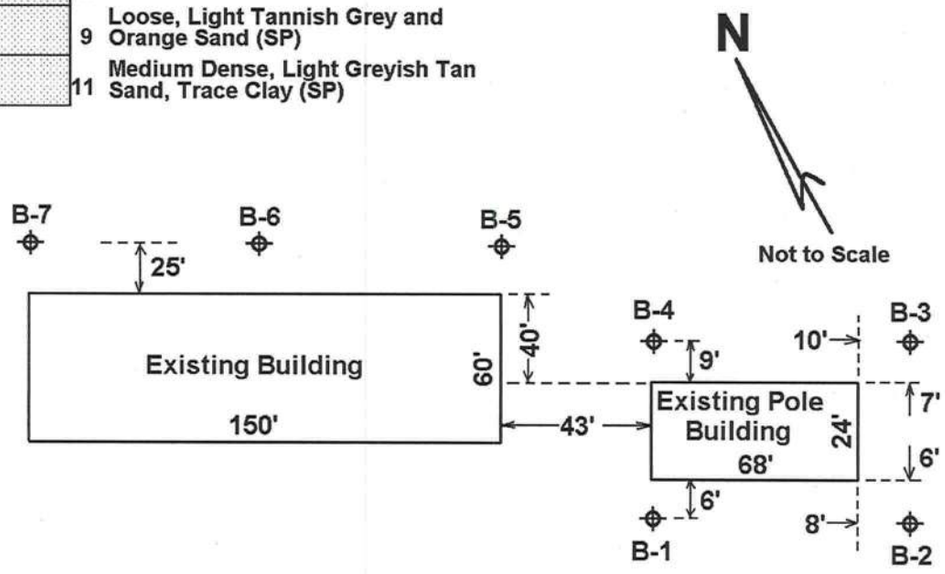
Water Table: N/A

Depth (ft)	N-value	Soil Description
0		Dark Tannish Grey Sand with Silt (SP/SM)
	6	Loose, Tannish Grey Sand with Silt (SP/SM)
	4	
5	6	Loose, Light Tannish Grey Sand (SP)
	10	Loose, Grey and Orange Sand, Trace Clay (SP)
	15	Medium Dense, Light Tannish Grey and Orange, Clayey Sand (SC)
	18	
10		

B-7

Water Table: N/A

Depth (ft)	N-value	Soil Description
0		Dark Tannish Grey Sand with Silt (SP/SM)
	5	Loose, Greyish Tan Sand with Silt (SP/SM)
	5	Loose, Light Greyish Tan Sand with Silt (SP/SM)
5	6	Loose, Light Tannish Grey Sand (SP)
	8	
	9	Loose, Light Tannish Grey and Orange Sand (SP)
	11	Medium Dense, Light Greyish Tan Sand, Trace Clay (SP)
10		



Boring Logs: Proposed Buildings
Morrell's