REQUIRED ROOF VENTILATION: AS PER FLORIDA BUILDING CODE 2309.7 RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)
3047 S.F. / 300 x 50% = 5.07 S.F. RIDGE VENT AREA REQUIRED
46.1 FEET OF RIDGE VENT REQUIRED SOFFIT VENT 3047 S.F. / 300 x 50% = 5.07 S.F. SOFFIT VENT AREA REQUIRED 169 FEET OF SOFFIT VENT REQUIRED BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS: 1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT) FRONT ELEVATION DIMENSIONS: ----David Disosway REAR ELEVATION
SCALE: 1/4" = 1'-0"

REVISIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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LIMITATION: This design is valid for one building, at specified location.

Willis Residence

ADDRESS: Lot 79 Emerald Cove S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:: September 28, 2007 DRAWN BY:

FINALS DATE: Sept. 28, 2007

JOB NUMBER: 706276

DRAWING NUMBER A-1



REVISIONS

SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE

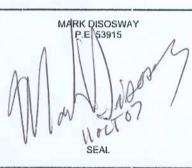
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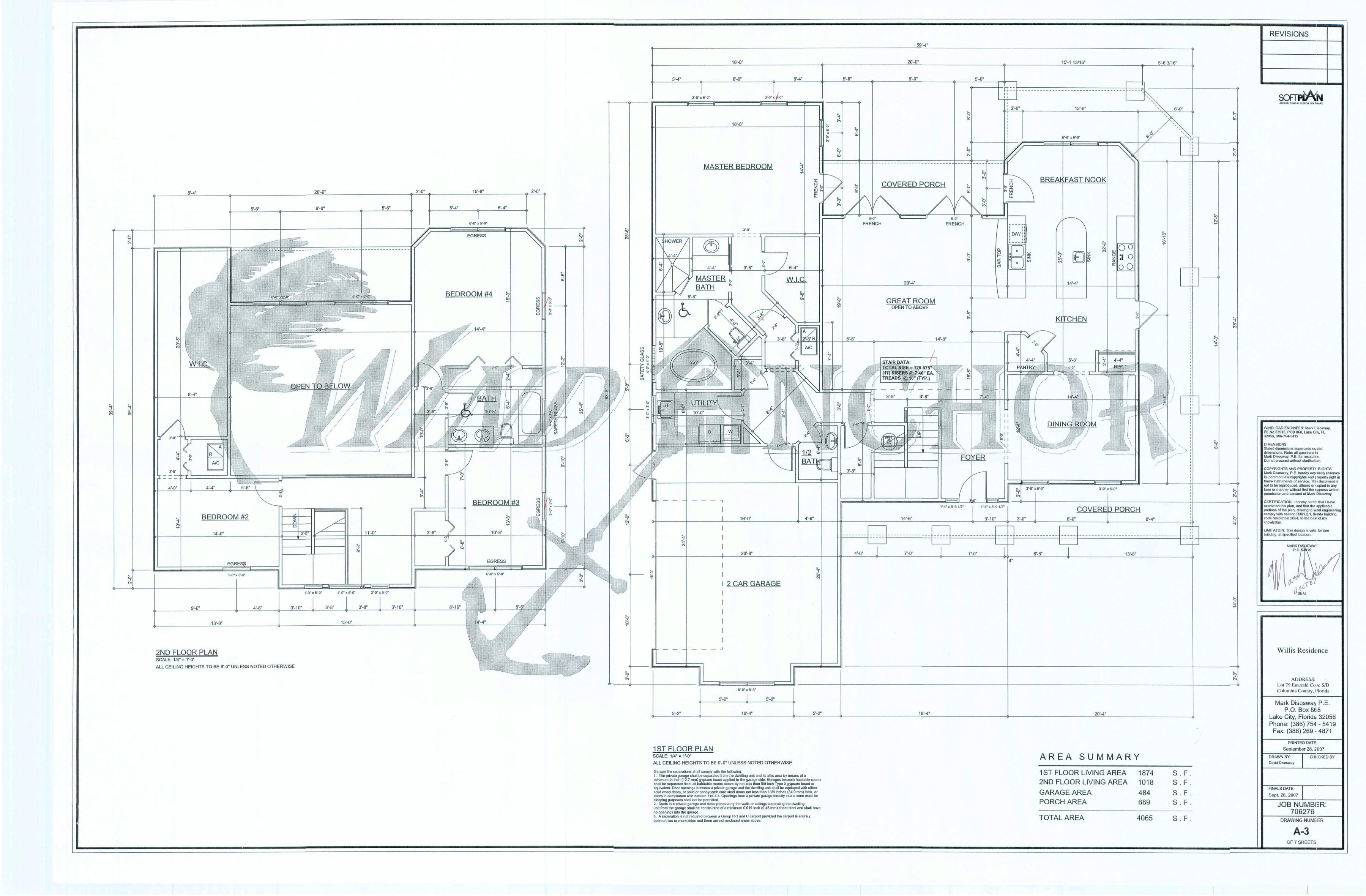
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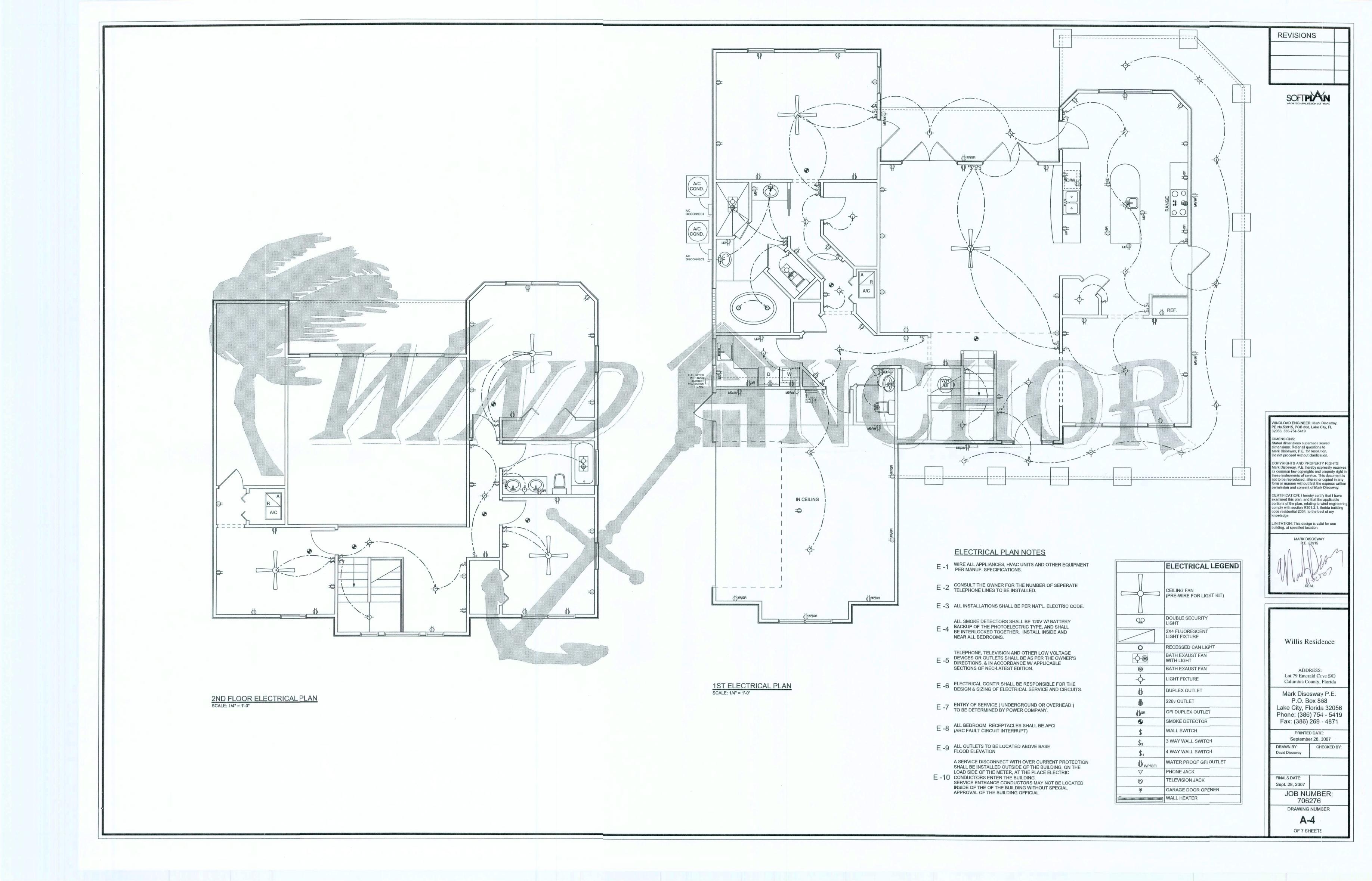
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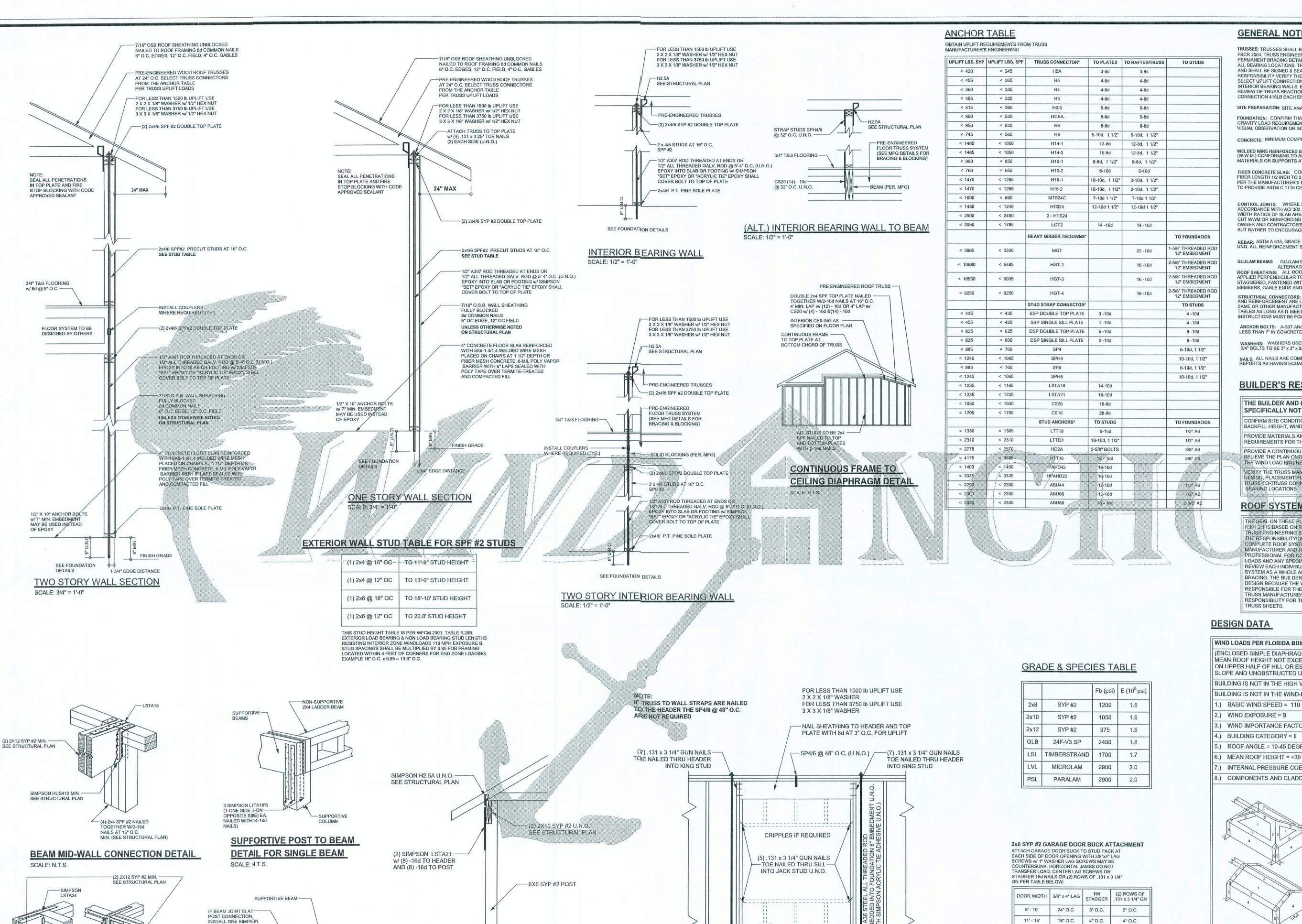
FINALS DATE:

Sept. 28, 2007 JOB NUMBER: 706276

A-2







-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

-SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

ANCHOR BOLT

LSTA18 ON ONE SIDE

(2-ONE SIDE, 2-ON

SUPPORTIVE CENTER POST TO BEAM DETAIL

LSTA18

NAIL THRU 2x4 INTO

BEAM MAY BE ATTACHED IN

BEAM CORNER CONNECTION. DETAIL

-SIMPSON HUS412 MIN.

SCALE: N.T.S.

SEE STRUCTURAL PLAN

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIET AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER, IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" × 4" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO WER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

EBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO, 5UPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

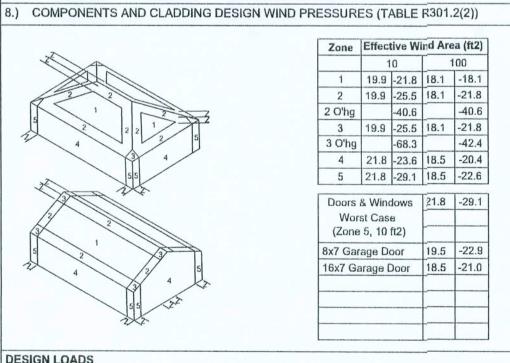
BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION IF YOU RELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, RUSS TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION 301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN SS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

WIND LOADS PER FLORIDA BUILDING CODE 2	004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS) MEAN ROOF HEIGHT NOT EXCEEDING LEAST ON UPPER HALF OF HILL OR ESCARPMENT 60 SLOPE AND UNOBSTRUCTED UPWIND FOR 50	HORIZONTAL DIMENSION OR 60 FT; NOT DET IN EXP. B, 30FT IN EXP. C AND >10%
BUILDING IS NOT IN THE HIGH VELOCITY HUR	RICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS	S REGION
1.) BASIC WIND SPEED = 110 MPH	
2.) WIND EXPOSURE = B	
3.) WIND IMPORTANCE FACTOR = 1.0	
4.) BUILDING CATEGORY = II	
5.) ROOF ANGLE = 10-45 DEGREES	
6.) MEAN ROOF HEIGHT = <30 FT	
7.) INTERNAL PRESSURE COEFFICIENT = N/	A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN V	VIND PRESSURES (TABLE R301.2(2))



DESIGN	LOADS
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BE	ARING CAPACITY 1000PSF
NOT IN	FLOOD ZONE (BUILDER TO VERIFY)

16' - 18'

2x6SYP #2 DOOR BUCK ---

SCALE: N.T.S.

TYPICAL STRAPPING (U.N.O.)

(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O. (1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STORY HEADER STRAPING DETAIL

(SEE STRUCTURAL PLAN)

16" O.C.

GARAGE DOOR BUCK INSTALLATION DETAIL

3" O.C.

3" O.C.

INDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL

REVISIONS

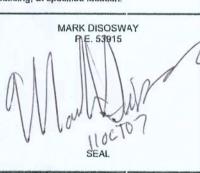
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> > **S-1** OF 7 SHEETS

