DATE 01/30/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000024096

APPLICANT RON RIPPLE PHONE 352 351-4251

ADDRESS 4390 NE 34TH COURT OCALA FL 34478

OWNER FREDRICK & SALLY SCHUENEMAN PHONE

ADDRESS 182 SW HONEYBEE COURT FT. WHITE FL 32038

CONTRACTOR STANTON VAN CONNER/AM.HOME PL PHONE 352 351-4251

LOCATION OF PROPERTY 41S, TR ON TOMMY LITES, TR ON TUSTENUGGEE, TL ON SASSAFRAS, TR ON HONEY BEE, 1ST LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 110150.00

HEATED FLOOR AREA 2203.00 TOTAL AREA 3063.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 7/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 30

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X PS DEVELOPMENT PERMIT NO.

PARCEL ID 07-6S-17-09621-211 SUBDIVISION TUSTENUGGEE OAKS UNREC

LOT 11 BLOCK PHASE UNIT TOTAL ACRES

000000958 | | | Applicant/Owner/Contractor

Culvert Permit No. | Culvert Waiver | Contractor's License Number

CULVERT | 05-1231-N | BK | JH | | |

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 4599

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by | Foundation date/app. by | Monolithic date/app. by

Under slab rough-in plumbing date/app. by | Slab date/app. by | Sheathing/Nailing date/app. by

Framing date/app. by | Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by | Heat & Air Duct date/app. by | Peri. beam (Lintel) date/app. by

Permanent power date/app. by | C.O. Finall date/app. by | Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by | Pool date/app. by

Reconnection date/app. by | Pump pole date/app. by | Utility Pole date/app. by

M/H Pole date/app. by | Travel Trailer date/app. by | Re-roof date/app. by

BUILDING PERMIT FEE $ 555.00 CERTIFICATION FEE $ 15.31 SURCHARGE FEE $ 15.31

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 685.62

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Rec
CC

THIS INSTRUMENT PREPARED BY
AND RETURN TO:
TITLE OFFICES, LLC
13795 SW 36TH AVE RD, STE.6
MARION OAKS PROFESSIONAL BLDG
OCALA, FL 34473

Parcel I.D. #: *07-65-17-09621-211*
Owner(s) SS#'s:

DAVID R. ELLSPERMANN, CLERK OF COURT MARION COUNTY
DATE: 08/04/2005 11:15:48 AM
FILE #: 2005137348 OR BK 04127 PGS 0481-0482

RECORDING FEES 18.50

Inst:2005022257 Date:09/12/2005 Time:16:39
DC,P.DeWitt Cason,Columbia County B:1058 P: 9

SPACE ABOVE THIS LINE FOR PROCESSING DATA

This instrument is being rerecorded to record in the correct county

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713.13, Florida Statutes, the following information is provided in this Notice of Commencement. This Notice shall be void and of no force and effect if construction is not commenced within ninety (90) days after recordation.

1. Description of property: (Legal description of property, and street address if available)

SEE EXHIBIT "A" ATTACHED HERETO AND MADE A PART HEREOF

2. General description of improvement: **construction of single family dwelling**

3. Owner information:
   a. Name and address:
   FREDRICK W. SCHUENEMAN and SALLY E. SCHUENEMAN
   2601 15th AVENUE NORTH, ST PETERSBURG, FL 33713
   b. Interest in property: **Fee Simple**
   c. Name and Address of Fee Simple Titleholder (if other than owner):

4. Contractor: (Name and Address)
America's Home Place/ Stanton Van Conner
2144 Hilton Drive, Gainesville, FL
Telephone Number: _____

5. Surety (if any):
   a. Name and Address:
   Telephone Number: _____
   b. Amount of Bond $_____

6. Lender: (Name and Address)
SUNTRUST BANK
350 N. LAKE DESTINY ROAD, MAITLAND, FL 32751
Telephone Number: **407-667-7537**

7. Persons within the State of Florida designated by Owner upon whom notice or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: (Name and Address)
N/A

8. In addition to himself, Owner designates the following person(s) to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes: (Name and Address)
SUNTRUST BANK
350 N. LAKE DESTINY ROAD, MAITLAND, FL 32751
Telephone Number: **407-667-7537**

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____.

Fredrick W Schueneman (SEAL)
FREDRICK W. SCHUENEMAN by Sally E Schueneman P.O.A.

Sally E Schueneman (SEAL)
SALLY E. SCHUENEMAN

Sworn to and subscribed before me this day of July 29, 2005, by ~~**FREDRICK W. SCHUENEMAN**~~ and **SALLY E. SCHUENEMAN**, who are personally known to me or who have produced FL DR Lic as identification.

Annabell L Bailey
Notary Public
My Commission Expires: _____

ANNABELL L. BAILEY
MY COMMISSION # DD 292769
EXPIRES: February 22, 2008
Bonded Thru Notary Public Underwriters

Inst:2005022257 Date:09/12/2005 Time:16:39
DC,P.DeWitt Cason,Columbia County B:1058 P:10

PARCEL NO. 11

Commence at the Northeast corner of Section 7, Township 6 South, Range 17 East, Columbia County, Florida and run thence S 89°54'54" W along the North line of said Section 7, 40.00 feet to the West right-of-way line of County Road No. C-131; thence S 00°20'48" W along said West right-of-way line of County Road No. C-131, 638.44 to the Northeast corner of Lot 1 of Tustenuggee Ridge, a subdivision according to plat thereof recorded in Plat Book 6, Page 212 of the Public Records of Columbia County, Florida; thence N 89°38'15" W along the North line of said Lot 1, 683.44 feet to the Northwest corner of said Lot 1; thence S 00°20'48" W along the West line of said Subdivision, 671.46 feet; thence N 89°38'15" W, 650.04 feet to the POINT OF BEGINNING; thence continue S 89°38'15" W, 650.04 feet; thence S 00°20'48" W, 671.46 feet; thence S 89°38'15" E, 650.04 feet; thence N 00°20'48" E, 671.46 feet to the POINT OF BEGINNING. Said lands being subject to an easement for ingress and egress as follows: the South 30 feet and the East 30 feet and that portion of a 50-foot radius cul-de-sac in the Northeast corner thereof. Containing 10.02 acres, more or less.

TOGETHER WITH:

60-FT. ROAD EASEMENT FOR TUSTENUGGEE OAKS

A strip of land 60 feet in width being 30 feet each side of a centerline described as follows: Commence at the Northwest corner of the NE 1/4 of Section 12, Township 6 South, Range 16 East, Columbia County, Florida and run thence S 00°03'14" E along the West line of the East ½ of said Section 12, 22.91 feet to the South line of Ichetucknee Road (a county maintained graded road); thence N 89°36'50" E along said South line of Ichetucknee Road, 581.07 feet, thence N 89°05'20" E still along said South line of Ichetucknee Road, 785.95 feet to the POINT OF BEGINNING; thence S 00°20'48" W, 1892.66 feet; thence S 89°38'15" E, 2600.79 feet to Reference Point "E"; thence continue S 89°38'15" E, 1300.06 feet to Reference Point "F"; thence continue S 89°38'15" E, 1300.07 feet to Reference Point "G"; thence continue S 89°38'15" E, 1333.48 feet to the West right-of-way line of County Road No. C-131 and the POINT OF TERMINATION. Also BEGIN at Reference Point "E" and run thence N 00°20'48" E, 671.46 feet to the centerpoint of a cul-de-sac having a radius of 50 feet and to the POINT OF TERMINATION. Also begin at Reference Point "E" and run thence S 00°20'48" W, 671.46 feet to the centerpoint of a cul-de-sac having a radius of 50 feet and to the POINT OF TERMINATION. Also begin at Reference Point "F", and run thence N 00°20'48" E, 671.46 feet to the centerpoint of a cul-de-sac having a radius of 50 feet and to the POINT OF TERMINATION. Also begin at Reference Point "F" and run thence S 00°20'48" W, 671.46 feet to the POINT OF TERMINATION. Also begin at Reference Point "G" and run thence N 00°20'48" E, 671.46 feet to the centerpoint of a cul-de-sac having a radius of 50 feet and to the POINT OF TERMINATION. Also begin at Reference Point "G" and run thence S 00°20'48" W, 671.46 feet to the centerpoint of a cul-de-sac having a radius of 50 feet and to the POINT OF TERMINATION. Said easement being a part of the NE 1/4 of Section 12 and the NW 1/4 and the NE 1/4 of Section 7, Township 6 South, Range 17 East.

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	A01	ROOF TRUSS	11	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:39 2005 Page 1

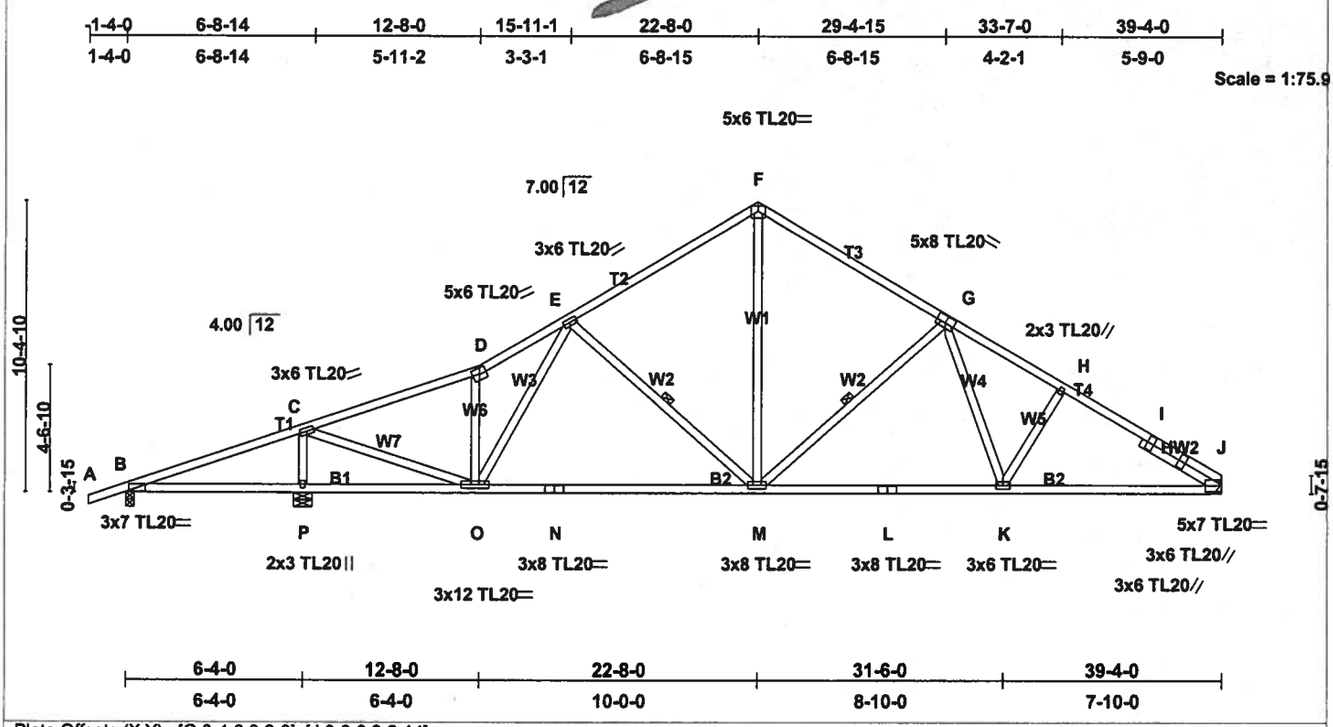


Plate Offsets (X,Y): [G:0-4-0,0-3-0], [J:0-0-0,0-2-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.20	M-O	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.57	M-O	>696	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.06	J	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 215 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 4 SYP No.3 3-3-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt E-M, G-M

REACTIONS (lb/size) B=126/0-3-8, J=1279/Mechanical, P=1820/0-8-0
Max Horz B=462(load case 4)
Max Uplift B=-352(load case 3), J=-409(load case 6), P=-731(load case 5)
Max Grav B=163(load case 9), J=1279(load case 1), P=1820(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/24, B-C=-261/567, C-D=-1501/440, D-E=-1635/571, E-F=-1316/548, F-G=-1319/518, G-H=-1844/672, H-I=-1891/648, I-J=-2008/627
BOT CHORD B-P=-470/239, O-P=-470/239, N-O=-363/1313, M-N=-363/1313, L-M=-280/1454, K-L=-280/1454, J-K=-424/1633
WEBS C-O=-517/1935, D-O=-592/322, E-O=-213/207, E-M=-400/317, F-M=-261/793, G-M=-559/393, G-K=-101/371, H-K=-167/219, C-P=-1679/682

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 352 lb uplift at joint B, 409 lb uplift at joint J and 731 lb uplift at joint P.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.
7) H26R: Right end may be attached to 2x8 So.Pine or larger BC of two or more ply girder truss with Simpson HUS26 or eq. Follow Simpson instructions for installation.

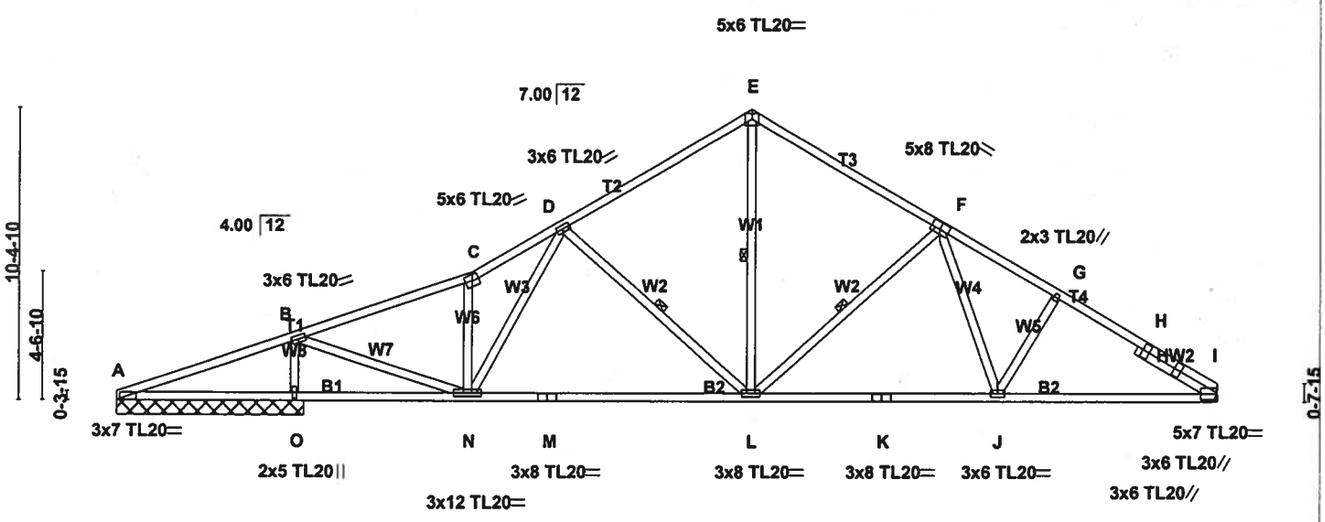
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	A02	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:40 2005 Page 1

6-8-14	12-8-0	15-11-1	22-8-0	29-4-15	33-7-0	39-4-0
6-8-14	5-11-2	3-3-1	6-8-15	6-8-15	4-2-1	5-9-0

Scale = 1:75.6



6-4-0	6-8-0	12-8-0	22-8-0	31-6-0	39-4-0
6-4-0	0-4-0	6-0-0	10-0-0	8-10-0	7-10-0

Plate Offsets (X,Y): [A:0-1-6,0-0-2], [F:0-4-0,0-3-0], [I:0-0-0,0-2-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.40	L-N	>996	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.57	L-N	>694	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.98	Horz(TL)	0.06	I	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 213 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 4 SYP No.3 3-3-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt D-L, E-L, F-L

REACTIONS (lb/size) A=35/6-8-0, I=1278/Mechanical, O=1834/6-8-0
Max Horz A=447(load case 4)
Max UpliftA=-37(load case 3), I=-785(load case 5), O=-1222(load case 5)
Max Grav A=85(load case 5), I=1278(load case 1), O=1834(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-471/562, B-C=-1497/1004, C-D=-1629/1220, D-E=-1315/957, E-F=-1318/989, F-G=-1843/1259, G-H=-1890/1235, H-I=-2007/1202
BOT CHORD A-O=-464/332, N-O=-464/332, M-N=-842/1311, L-M=-842/1311, K-L=-814/1453, J-K=-814/1453, I-J=-958/1632
WEBS B-N=-1320/1924, C-N=-586/458, D-N=-416/205, D-L=-399/413, E-L=-705/792, F-L=-559/434, F-J=-313/371, G-J=-167/219, B-O=-1682/1198

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint A, 785 lb uplift at joint I and 1222 lb uplift at joint O.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.
7) H26R: Right end may be attached to 2x8 So.Pine or larger BC of two or more ply girder truss with Simpson HUS26 or eq. Follow Simpson instructions for installation.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	A03	ROOF TRUSS	2	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:41 2005 Page 1

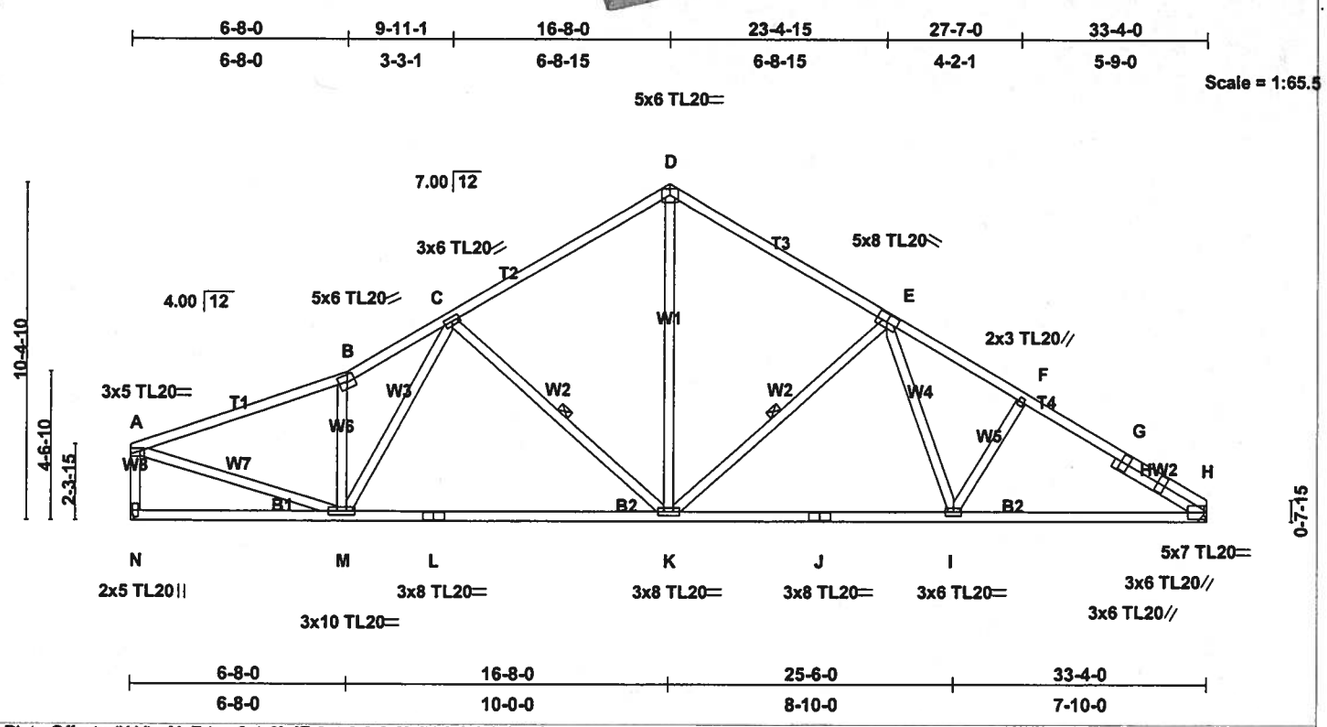


Plate Offsets (X,Y): [A:Edge,0-1-0], [E:0-4-0,0-3-0], [H:0-0-0,0-2-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.85	Vert(LL)	-0.20	K-M	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.57	K-M	>703	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.08	H	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 4 SYP No.3 3-3-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-3-15 oc bracing.
WEBS 1 Row at midpt C-K, E-K

REACTIONS (lb/size) N=1328/Mechanical, H=1328/Mechanical
Max Horz N=356(load case 4)
Max UpliftN=-421(load case 5), H=-416(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-1835/560, B-C=-2014/743, C-D=-1410/561, D-E=-1413/558, E-F=-1933/685, F-G=-2023/661, G-H=-2097/640, A-N=-1269/441
BOT CHORD M-N=-408/195, L-M=-444/1500, K-L=-444/1500, J-K=-314/1533, I-J=-314/1533, H-I=-434/1708
WEBS B-M=-701/385, C-M=-228/427, C-K=-520/378, D-K=-273/886, E-K=-558/392, E-I=-101/368, F-I=-163/218, A-M=-394/1616

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 421 lb uplift at joint N and 416 lb uplift at joint H.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.
7) H26E: End(s) may be attached to 2x8 So.Pine or larger BC of two or more ply girder truss with Simpson HUS26 or eq. Follow Simpson instructions for installation.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	A04	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:42 2005 Page 1

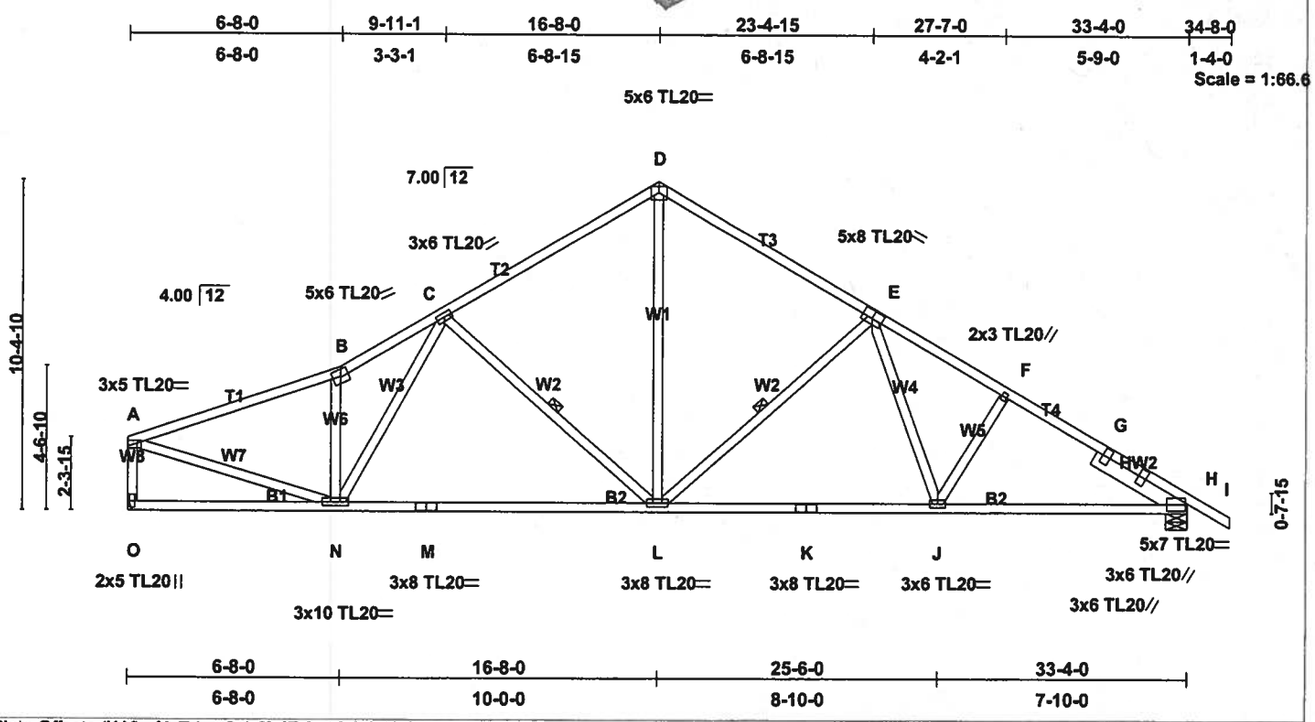


Plate Offsets (X,Y): [A:Edge,0-1-0], [E:0-4-0,0-3-0], [H:0-0-10,0-2-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.85	Vert(LL)	-0.20	L-N	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.56	L-N	>705	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.08	H	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 200 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.2 3-3-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-5-11 oc bracing.
WEBS 1 Row at midpt C-L, E-L

REACTIONS (lb/size) O=1326/Mechanical, H=1409/0-8-0
Max Horz O=347(load case 4)
Max UpliftO=-420(load case 5), H=-504(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-1832/559, B-C=-2011/742, C-D=-1407/557, D-E=-1410/557, E-F=-1921/670, F-G=-2010/646, G-H=-2086/627, H-I=0/25, A-O=-1268/441
BOT CHORD N-O=-399/228, M-N=-430/1497, L-M=-430/1497, K-L=-299/1527, J-K=-299/1527, H-J=-384/1694
WEBS B-N=-700/384, C-N=-228/427, C-L=-520/378, D-L=-270/884, E-L=-553/387, E-J=-88/364, F-J=-155/207, A-N=-392/1613

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 420 lb uplift at joint O and 504 lb uplift at joint H.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.
7) H26L: Left end may be attached to 2x8 So.Pine or larger BC of two or more ply girder truss with Simpson HUS26 or eq. Follow Simpson instructions for installation.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	B01	ROOF TRUSS	6	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:42 2005 Page 1

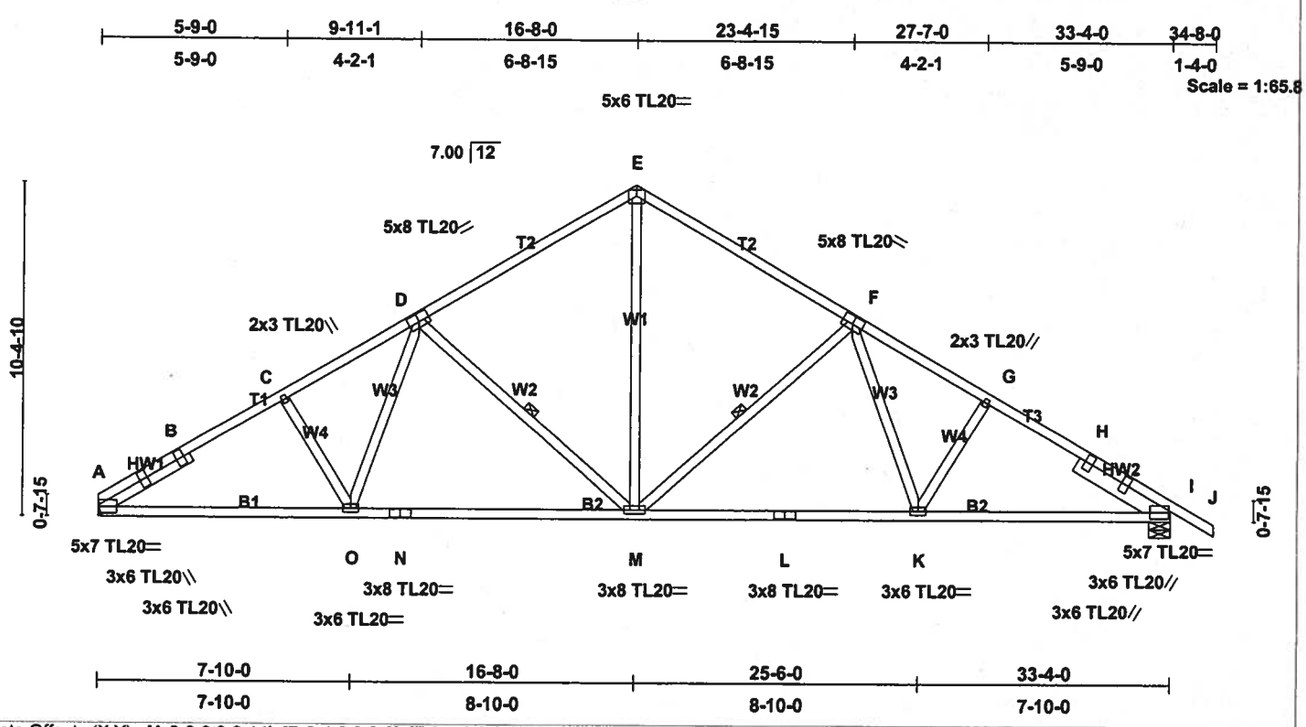


Plate Offsets (X,Y): [A:0-0-0,0-2-14], [D:0-4-0,0-3-0], [F:0-4-0,0-3-0], [I:0-0-10,0-2-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.11	K-M	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.33	K-M	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.10	I	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 3-3-4, Right 2 X 6 SYP No.2 3-3-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-0-10 oc bracing.
WEBS 1 Row at midpt D-M, F-M

REACTIONS (lb/size) A=1332/Mechanical, I=1415/0-8-0
Max Horz A=-428(load case 3)
Max UpliftA=-419(load case 5), I=-508(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-2107/645, B-C=-2032/666, C-D=-1943/690, D-E=-1417/567, E-F=-1417/569, F-G=-1934/676, G-H=-2023/652, H-I=-2098/633, I-J=0/25
BOT CHORD A-O=-593/1715, N-O=-452/1540, M-N=-452/1540, L-M=-308/1536, K-L=-308/1536, I-K=-389/1704
WEBS C-O=-162/218, D-O=-98/378, D-M=-562/390, E-M=-282/894, F-M=-557/385, F-K=-85/374, G-K=-154/207

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 419 lb uplift at joint A and 508 lb uplift at joint I.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.
7) H26L: Left end may be attached to 2x8 So.Pine or larger BC of two or more ply girder truss with Simpson HUS26 or eq. Follow Simpson instructions for installation.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	B01G	ROOF TRUSS	2	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:44 2005 Page 1

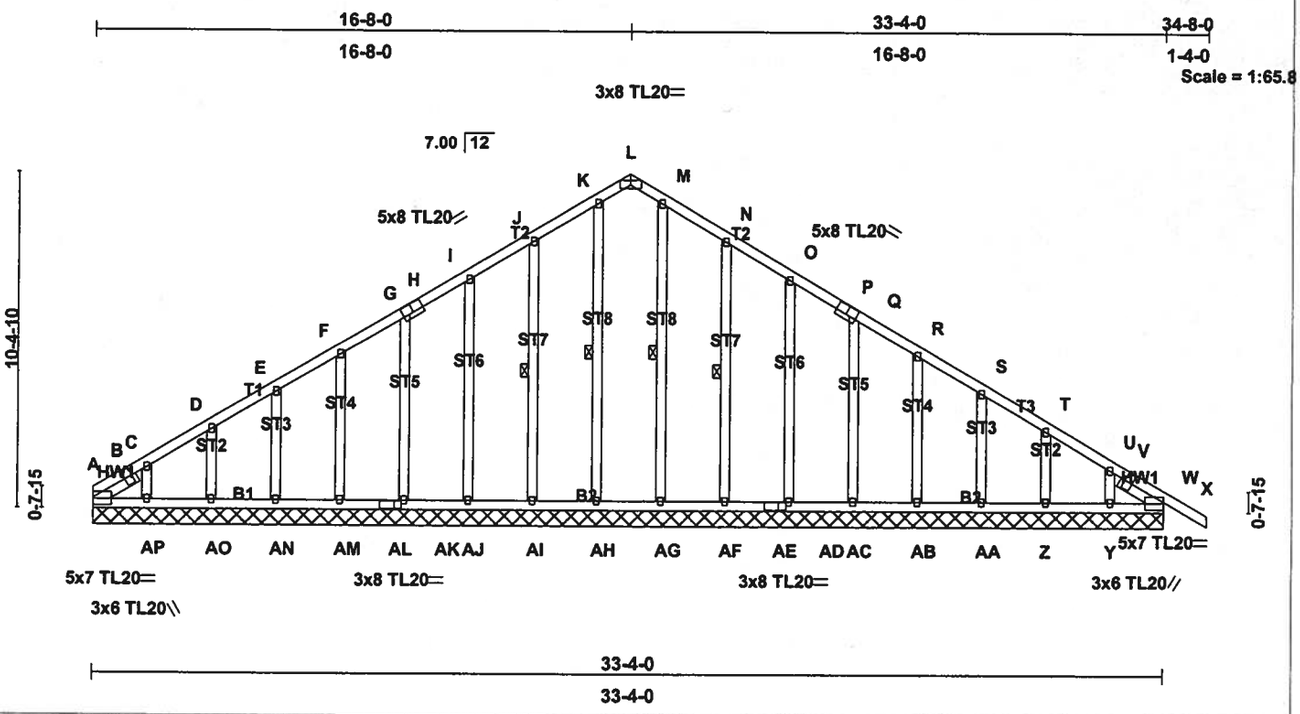


Plate Offsets (X,Y): [A:0-0-0,0-2-14], [H:0-4-0,0-3-4], [L:0-4-0,Edge], [P:0-4-0,0-3-4], [W:0-0-0,0-2-14], [AE:0-2-12,0-1-8], [AL:0-2-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	X	n/r	120	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	X	n/r	90		
BCLL	0.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.01	W	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 237 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 1-6-6, Right 2 X 4 SYP No.3 1-6-6

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt K-AH, J-AI, M-AG, N-AF

REACTIONS (lb/size) A=66/33-4-0, W=183/33-4-0, AH=148/33-4-0, AI=162/33-4-0, AJ=160/33-4-0, AK=160/33-4-0, AM=160/33-4-0, AN=160/33-4-0, AO=162/33-4-0, AP=156/33-4-0, AG=148/33-4-0, AF=162/33-4-0, AD=160/33-4-0, AC=160/33-4-0, AB=160/33-4-0, AA=158/33-4-0, Z=168/33-4-0, Y=115/33-4-0
Max Horz A=-428(load case 3)
Max Uplift A=-137(load case 3), W=-58(load case 4), AH=-20(load case 4), AI=-124(load case 5), AJ=-96(load case 5), AK=-100(load case 5), AM=-99(load case 5), AN=-100(load case 5), AO=-95(load case 5), AP=-172(load case 5), AF=-130(load case 6), AD=-95(load case 6), AC=-100(load case 6), AB=-99(load case 6), AA=-98(load case 6), Z=-102(load case 6), Y=-110(load case 6)
Max Grav A=251(load case 4), W=183(load case 1), AH=149(load case 3), AI=164(load case 9), AJ=160(load case 1), AK=160(load case 9), AM=160(load case 1), AN=160(load case 9), AO=162(load case 1), AP=157(load case 9), AG=148(load case 1), AF=164(load case 10), AD=160(load case 1), AC=160(load case 10), AB=160(load case 1), AA=158(load case 10), Z=168(load case 1), Y=116(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-412/251, B-C=-395/250, C-D=-320/222, D-E=-272/217, E-F=-223/210, F-G=-173/204, G-H=-123/186, H-I=-122/198, I-J=-74/235, J-K=-52/300, K-L=-46/251, L-M=-46/247, M-N=-52/278, N-O=-51/190, O-P=-45/122, P-Q=-51/110, Q-R=-51/70, R-S=-81/69, S-T=-130/75, T-U=-192/81, U-V=-264/98, V-W=-278/94, W-X=0/25
BOT CHORD A-AP=-65/298, AO-AP=-65/298, AN-AO=-65/298, AM-AN=-65/298, AL-AM=-65/298, AK-AL=-65/298, AJ-AK=-65/298, AI-AJ=-65/298, AH-AI=-65/298, AG-AH=-65/298, AF-AG=-65/298, AE-AF=-65/298, AD-AE=-65/298, AC-AD=-65/298, AB-AC=-65/298, AA-AB=-65/298, Z-AA=-65/298, Y-Z=-65/298, W-Y=-65/298
WEBS K-AH=-125/44, J-AI=-124/148, I-AJ=-120/120, G-AK=-120/123, F-AM=-120/123, E-AN=-119/123, D-AO=-122/121, C-AP=-115/184, M-AG=-108/0, N-AF=-124/154, O-AD=-120/119, Q-AC=-120/124, R-AB=-120/123, S-AA=-119/123, T-Z=-125/126, U-Y=-87/138

NOTES (10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) Truss designed for wind loads in the plane of the truss only.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x3 TL20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint A, 58 lb uplift at joint W, 20 lb uplift at joint AH, 124 lb uplift at joint AI, 96 lb uplift at joint AJ, 100 lb uplift at joint AK, 99 lb uplift at joint AM, 100 lb uplift at joint AN, 95 lb uplift at joint AO, 172 lb uplift at joint AP, 130 lb uplift at joint AF, 95 lb uplift at joint AD, 100 lb uplift at joint AC, 99 lb uplift at joint AB, 98 lb uplift at joint AA, 102 lb uplift at joint Z and 110 lb uplift at joint Y.
10) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	B02	ROOF TRUSS	3	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:45 2005 Page 1

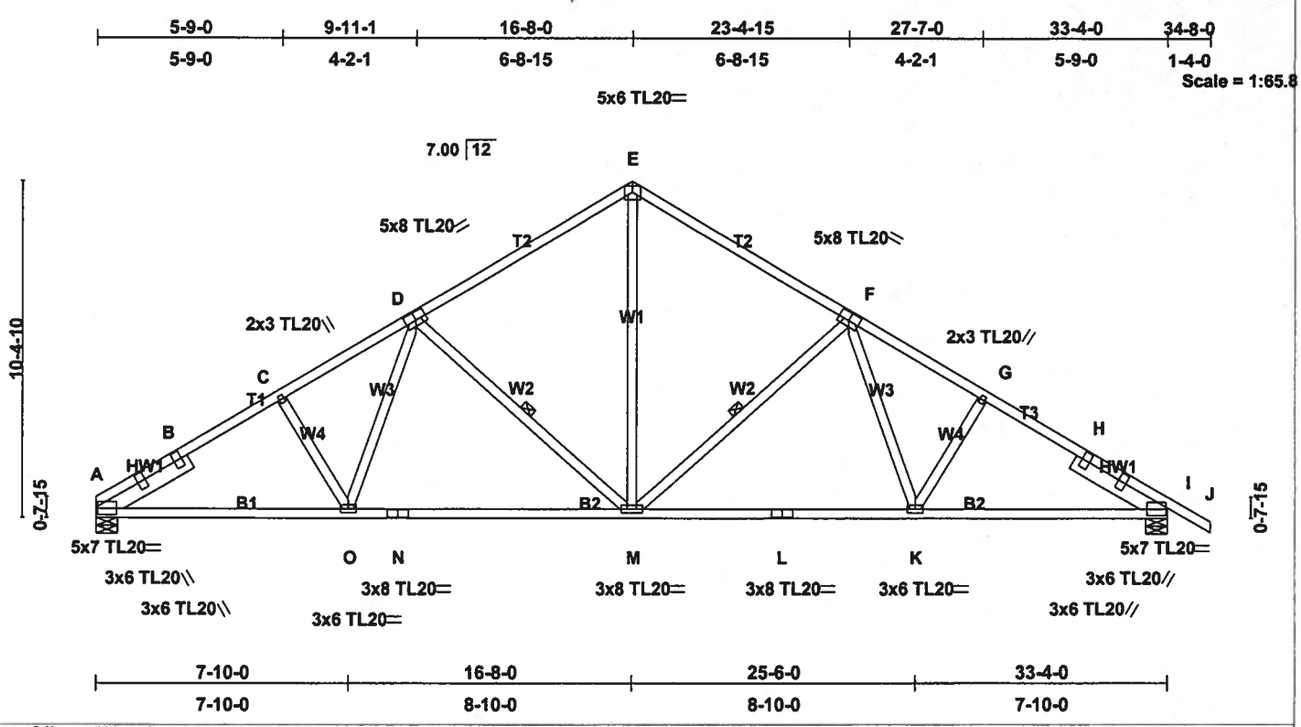


Plate Offsets (X,Y): [A:0-0-10,0-2-14], [D:0-4-0,0-3-0], [F:0-4-0,0-3-0], [I:0-0-10,0-2-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.11	M-O	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.33	K-M	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.10	I	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.2 3-3-4, Right 2 X 6 SYP No.2 3-3-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-0-12 oc bracing.
WEBS 1 Row at midpt D-M, F-M

REACTIONS (lb/size) A=1332/0-8-0, I=1415/0-8-0
Max Horz A=-428(load case 3)
Max UpliftA=-419(load case 5), I=-508(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-2107/644, B-C=-2032/665, C-D=-1942/689, D-E=-1417/567, E-F=-1417/569, F-G=-1933/676, G-H=-2023/652, H-I=-2098/633, I-J=0/25
BOT CHORD A-O=-593/1716, N-O=-452/1540, M-N=-452/1540, L-M=-308/1536, K-L=-308/1536, I-K=-389/1704
WEBS C-O=-163/218, D-O=-97/377, D-M=-562/390, E-M=-282/894, F-M=-557/385, F-K=-85/374, G-K=-154/207

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 419 lb uplift at joint A and 508 lb uplift at joint I.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	B03	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:46 2005 Page 1

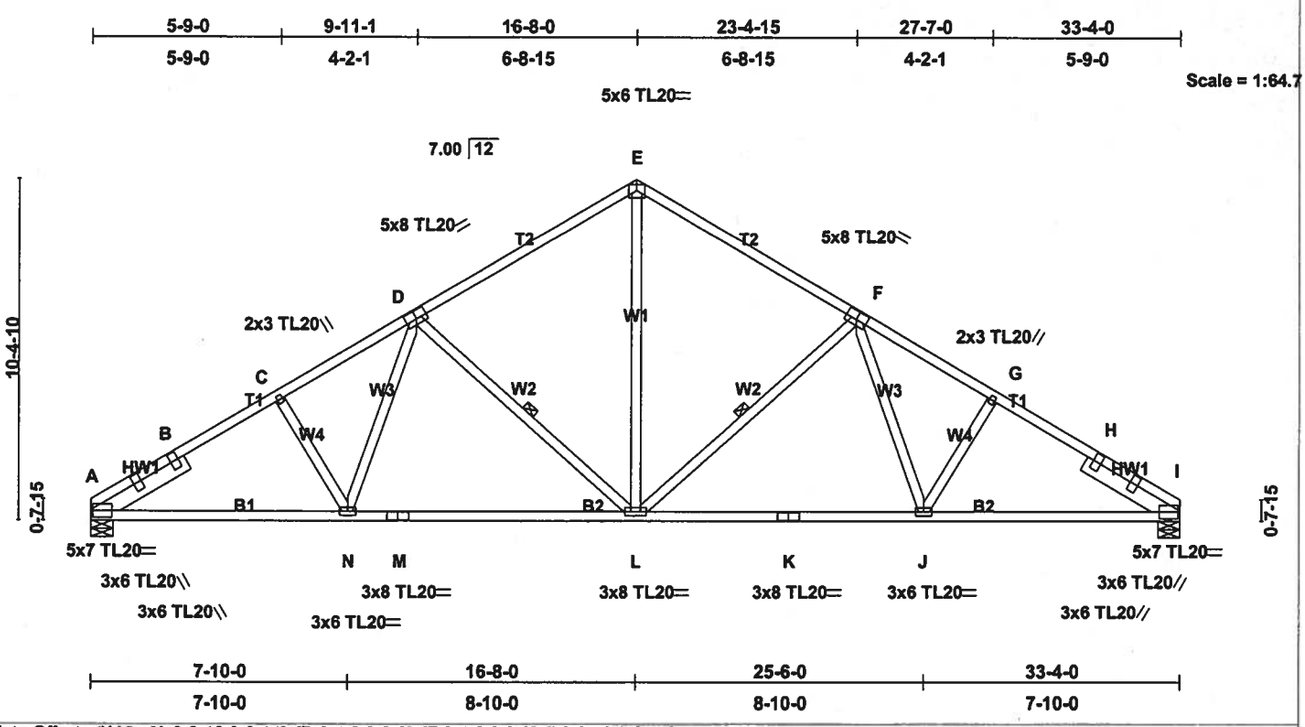


Plate Offsets (X,Y): [A:0-0-10,0-2-14], [D:0-4-0,0-3-0], [F:0-4-0,0-3-0], [I:0-0-10,0-2-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.11	L-N	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.33	L-N	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.10	I	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 194 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.2 3-3-4, Right 2 X 6 SYP No.2 3-3-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-11 oc bracing.
WEBS 1 Row at midpt D-L, F-L

REACTIONS (lb/size) A=1333/0-8-0, I=1333/0-8-0
Max Horz A=416(load case 4)
Max UpliftA=-420(load case 5), I=-420(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-2110/645, B-C=-2035/666, C-D=-1945/690, D-E=-1420/570, E-F=-1420/570, F-G=-1945/691, G-H=-2035/667, H-I=-2110/645
BOT CHORD A-N=-607/1718, M-N=-466/1542, L-M=-466/1542, K-L=-323/1542, J-K=-323/1542, I-J=-439/1718
WEBS C-N=-163/218, D-N=-97/377, D-L=-562/390, E-L=-283/896, F-L=-562/390, F-J=-97/377, G-J=-163/218

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 420 lb uplift at joint A and 420 lb uplift at joint I.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	B04	ROOF TRUSS	11	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:47 2005 Page 1

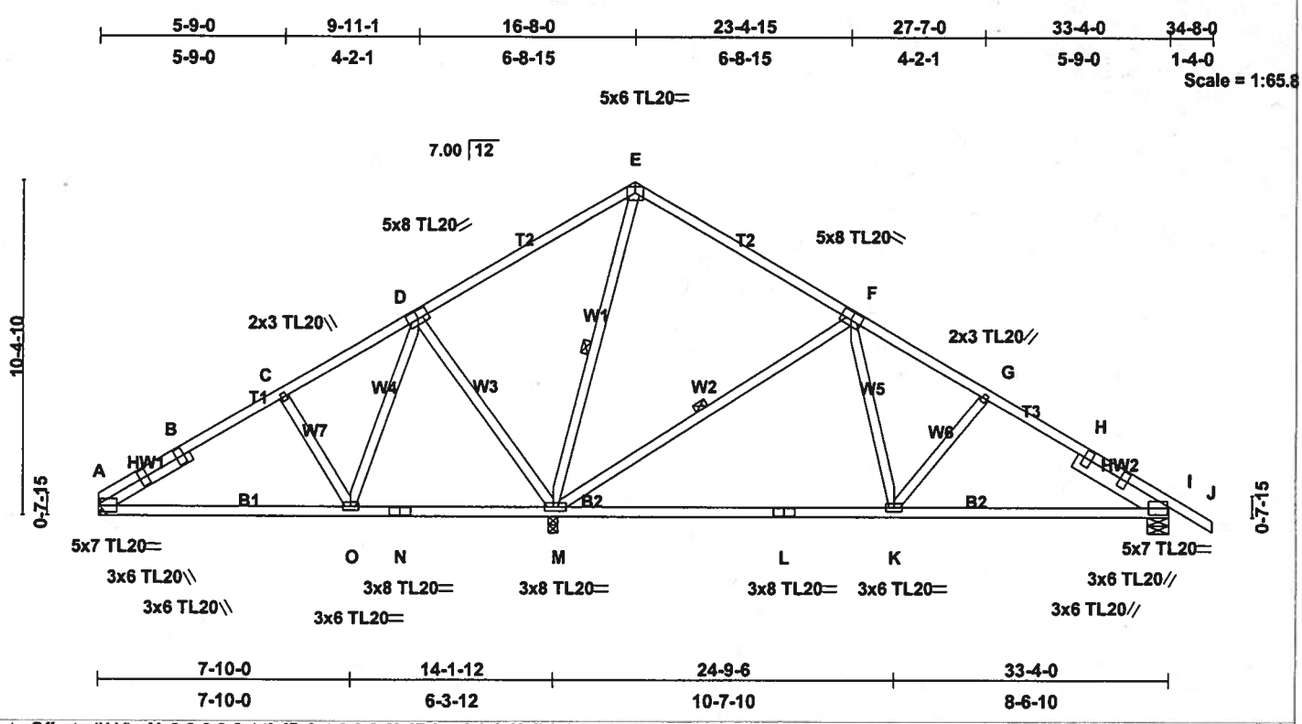


Plate Offsets (X,Y): [A:0-0-0,0-2-14], [D:0-4-0,0-3-0], [F:0-4-0,0-3-0], [I:0-0-10,0-2-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.20	K-M	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.51	K-M	>446	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.02	I	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 3-3-4, Right 2 X 6 SYP No.2 3-3-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: M-O.
WEBS 1 Row at midpt E-M, F-M

REACTIONS (lb/size) A=429/Mechanical, M=1579/0-3-8, I=739/0-8-0
Max Horz A=-428(load case 3)
Max UpliftA=-159(load case 5), M=-456(load case 5), I=-348(load case 6)
Max Grav A=474(load case 9), M=1579(load case 1), I=745(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-539/181, B-C=-464/200, C-D=-374/223, D-E=-10/374, E-F=-10/203, F-G=-685/356, G-H=-788/363, H-I=-879/345, I-J=0/25
BOT CHORD A-O=-343/434, N-O=-256/345, M-N=-256/345, L-M=0/470, K-L=0/470, I-K=-147/681
WEBS C-O=-256/243, D-O=-152/378, D-M=-546/404, E-M=-636/134, F-M=-660/393, G-K=-221/227, F-K=-62/487

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint A, 456 lb uplift at joint M and 348 lb uplift at joint I.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.
7) H26L: Left end may be attached to 2x8 So.Pine or larger BC of two or more ply girder truss with Simpson HUS26 or eq. Follow Simpson instructions for installation.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	C01	ROOF TRUSS	3	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:48 2005 Page 1

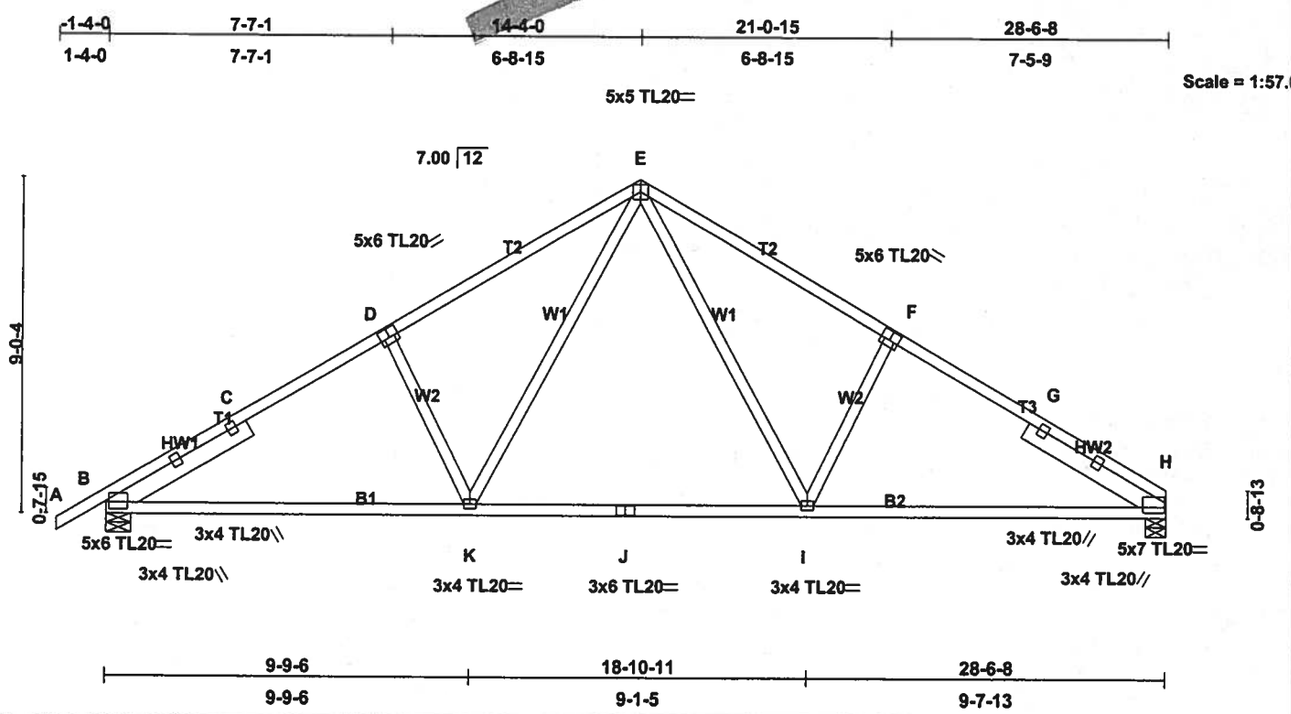


Plate Offsets (X,Y): [B:0-0-14,0-2-10], [D:0-3-0,0-3-0], [F:0-3-0,0-3-0], [H:0-0-6,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.13	B-K	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.36	B-K	>944	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.06	H	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 158 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.2 4-4-0, Right 2 X 6 SYP No.2 4-3-10

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-0-13 oc bracing.

REACTIONS (lb/size) B=1224/0-8-0, H=1140/0-6-8
Max Horz B=372(load case 4)
Max UpliftB=-448(load case 5), H=-358(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/25, B-C=-1696/512, C-D=-1583/538, D-E=-1511/611, E-F=-1503/616, F-G=-1570/543, G-H=-1686/514
BOT CHORD B-K=-448/1368, J-K=-158/954, I-J=-158/954, H-I=-307/1356
WEBS D-K=-373/384, E-K=-270/599, E-I=-273/587, F-I=-365/385

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 448 lb uplift at joint B and 358 lb uplift at joint H.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	C01G	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:49 2005 Page 1

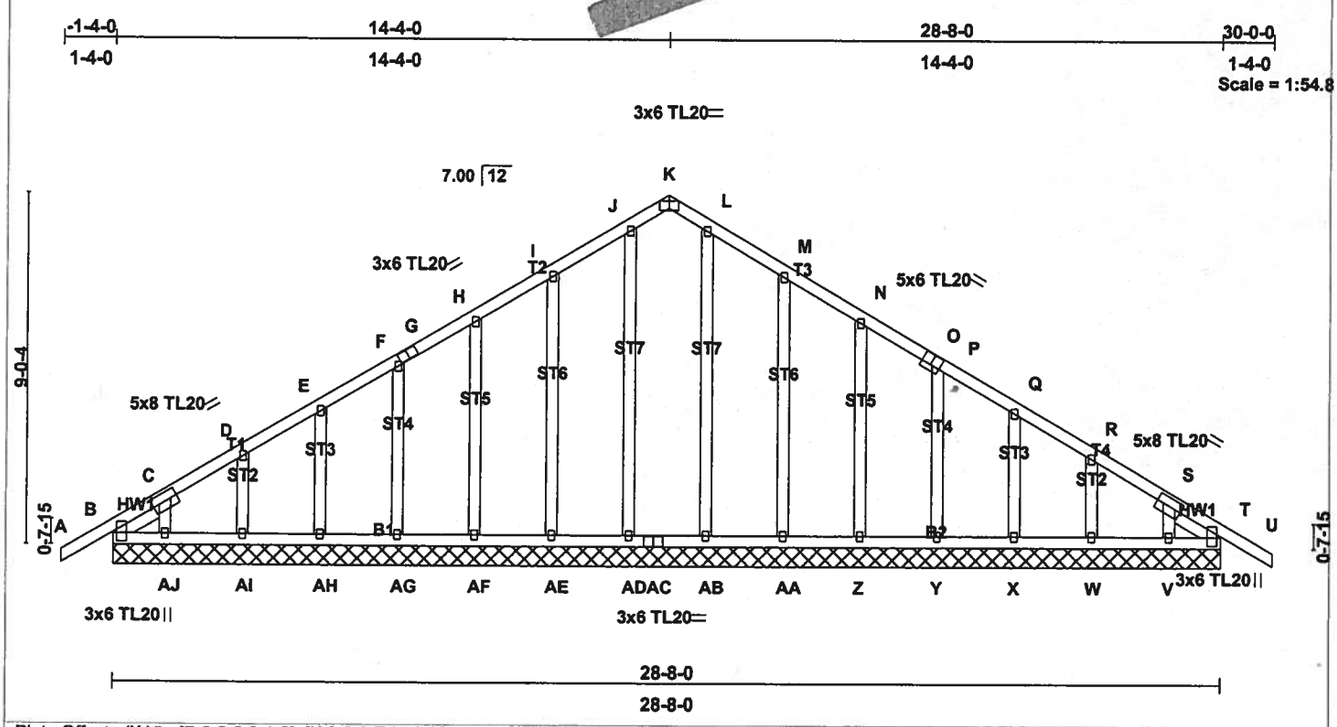


Plate Offsets (X,Y): [B:0-2-8,0-1-2], [K:0-3-0,Edge], [O:0-3-0,0-3-4], [T:0-2-8,0-2-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.00	U	n/r	120	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	U	n/r	90		
BCLL	0.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.01	T	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 1-4-11, Right 2 X 4 SYP No.3 1-4-11

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) B=174/28-8-0, T=174/28-8-0, AD=148/28-8-0, AE=162/28-8-0, AF=160/28-8-0, AG=160/28-8-0, AH=159/28-8-0, AI=166/28-8-0, AJ=98/28-8-0, AB=148/28-8-0, AA=162/28-8-0, Z=161/28-8-0, Y=161/28-8-0, X=158/28-8-0, W=166/28-8-0, V=98/28-8-0
Max Horz B=-363(load case 3)
Max Uplift B=-147(load case 3), T=-59(load case 4), AD=-23(load case 4), AE=-122(load case 5), AF=-96(load case 5), AG=-100(load case 5), AH=-98(load case 5), AI=-104(load case 5), AJ=-109(load case 4), AA=-127(load case 6), Z=-96(load case 6), Y=-101(load case 6), X=-97(load case 6), W=-104(load case 6), V=-95(load case 6)
Max Grav B=197(load case 4), T=174(load case 1), AD=148(load case 1), AE=164(load case 9), AF=160(load case 1), AG=160(load case 9), AH=159(load case 1), AI=166(load case 9), AJ=109(load case 3), AB=148(load case 1), AA=163(load case 10), Z=161(load case 1), Y=161(load case 10), X=158(load case 1), W=166(load case 10), V=100(load case 10)

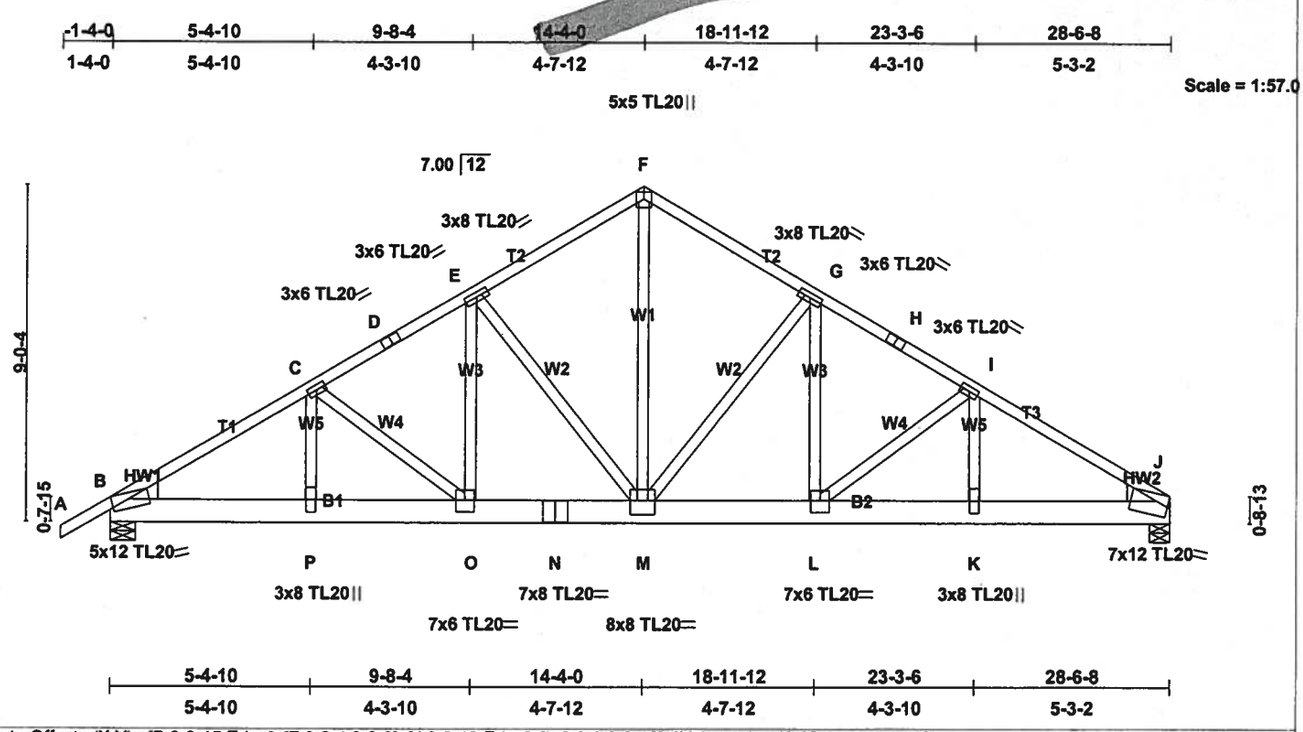
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/25, B-C=-362/226, C-D=-278/191, D-E=-227/184, E-F=-178/178, F-G=-128/161, G-H=-122/172, H-I=-79/186, I-J=-52/249, J-K=-46/214, K-L=-46/211, L-M=-52/228, M-N=-51/141, N-O=-47/81, O-P=-51/69, P-Q=-58/57, Q-R=-107/64, R-S=-169/70, S-T=-250/91, T-U=0/25
BOT CHORD B-AJ=-54/266, AI-AJ=-54/266, AH-AI=-54/266, AG-AH=-54/266, AF-AG=-54/266, AE-AF=-54/266, AD-AE=-54/266, AC-AD=-54/266, AB-AC=-54/266, AA-AB=-54/266, Z-AA=-54/266, Y-Z=-54/266, X-Y=-54/266, W-X=-54/266, V-W=-54/266, T-V=-54/266
WEBS J-AD=-111/47, I-AE=-124/146, H-AF=-120/120, F-AG=-120/124, E-AH=-119/122, D-AI=-123/127, C-AJ=-82/139, L-AB=-108/0, M-AA=-123/151, N-Z=-121/120, P-Y=-121/125, Q-X=-119/122, R-W=-123/127, S-V=-76/127

NOTES (10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Truss designed for wind loads in the plane of the truss only.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x3 TL20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint B, 59 lb uplift at joint T, 23 lb uplift at joint AD, 122 lb uplift at joint AE, 96 lb uplift at joint AF, 100 lb uplift at joint AG, 98 lb uplift at joint AH, 104 lb uplift at joint AI, 109 lb uplift at joint AJ, 127 lb uplift at joint AA, 96 lb uplift at joint Z, 101 lb uplift at joint Y, 97 lb uplift at joint X, 104 lb uplift at joint W and 95 lb uplift at joint V.
10) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	C02	ROOF TRUSS	1	2	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:50 2005 Page 1



5-4-10	9-8-4	14-4-0	18-11-12	23-3-6	28-6-8
5-4-10	4-3-10	4-7-12	4-7-12	4-3-10	5-3-2

Plate Offsets (X,Y): [B:0-0-15,Edge], [F:0-2-4,0-2-8], [J:0-3-13,Edge], [L:0-3-0,0-3-12], [M:0-4-0,0-4-8], [O:0-3-0,0-3-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	0.28	M-O	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-0.50	M-O	>667	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.84	Horz(TL)	0.13	J	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 432 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2
WEDGE
Left: 2 X 6 SYP No.2, Right: 2 X 6 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 9-0-11 oc bracing.

REACTIONS (lb/size) B=9788/0-8-0, J=9682/0-6-8
Max Horz B=369(load case 4)
Max UpliftB=-3920(load case 5), J=-3806(load case 6)

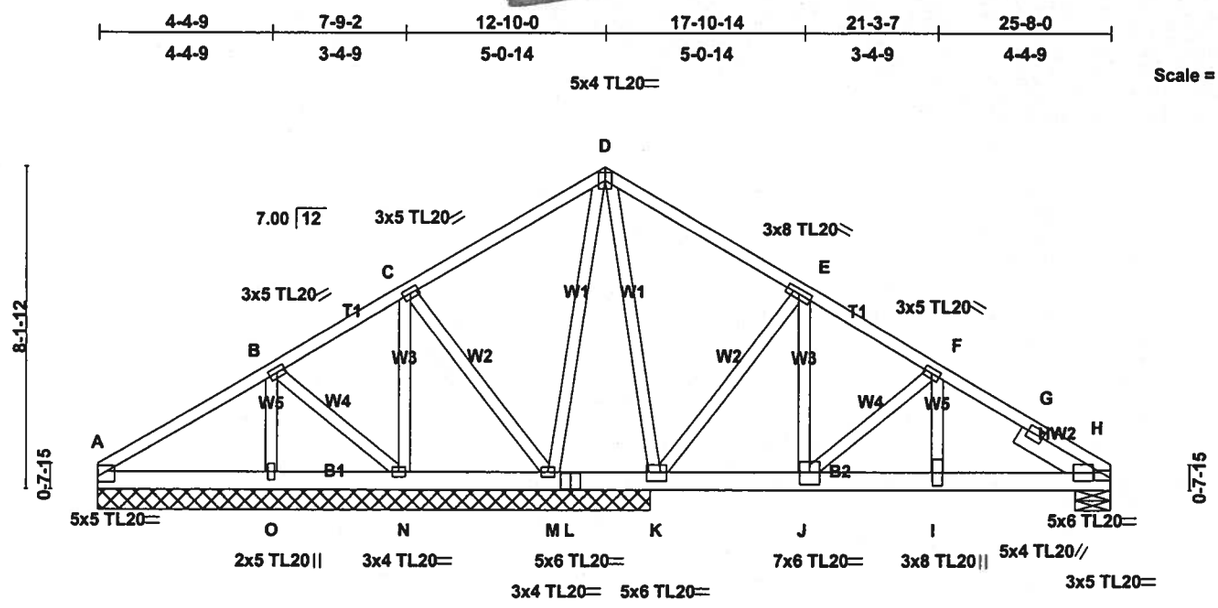
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/33, B-C=-14604/5705, C-D=-12108/4790, D-E=-12037/4803, E-F=-9408/3827, F-G=-9408/3822, G-H=-12003/4799, H-I=-12075/4786, I-J=-14536/5706
BOT CHORD B-P=-4801/12144, O-P=-4801/12144, N-O=-4026/10397, M-N=-4026/10397, L-M=-3925/10368, K-L=-4642/12044, J-K=-4642/12044
WEBS E-O=-1588/4031, E-M=-3714/1606, F-M=-3641/9078, G-M=-3668/1593, G-L=-1573/3978, C-P=-1128/2990, C-O=-2227/981, I-K=-1131/2984, I-L=-2137/965

NOTES (9)
1) 2-ply truss to be connected together with 10d Common(.148"x3") Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3920 lb uplift at joint B and 3806 lb uplift at joint J.
8) Girder carries tie-in span(s): 33-4-0 from 0-0-0 to 28-6-8
9) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: A-F=-60, F-J=-60, B-J=-633(F=-613)

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	D01	ROOF TRUSS	1	2	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:51 2005 Page 1



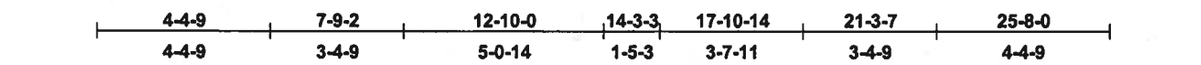


Plate Offsets (X,Y): [A:0-0-0,0-1-11], [H:0-2-7,0-2-8], [H:Edge,0-2-5], [J:0-3-0,0-3-12], [K:0-1-12,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.04	I-J	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.07	I-J	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.68	Horz(TL)	0.01	H	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 364 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.2 2-3-13

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) A=-55/14-0-0, H=3775/0-10-11, O=-119/14-0-0, N=97/14-0-0, M=369/14-0-0, K=6109/14-0-0
Max Horz A=-320(load case 3)
Max UpliftA=-154(load case 10), H=-1499(load case 6), O=-269(load case 10), N=-77(load case 5), M=-136(load case 5), K=-2438(load case 6)
Max Grav A=137(load case 4), H=3778(load case 10), O=293(load case 3), N=153(load case 3), M=369(load case 1), K=6109(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-300/446, B-C=-372/798, C-D=-430/1177, D-E=-427/1326, E-F=-1591/694, F-G=-3868/1562, G-H=-3905/1528
BOT CHORD A-O=-354/351, N-O=-354/351, M-N=-672/464, L-M=-892/590, K-L=-892/590, J-K=-453/1365, I-J=-1227/3280, H-I=-1227/3280
WEBS B-O=-219/347, B-N=-428/258, C-N=-244/437, C-M=-492/289, E-K=-3960/1708, E-J=-1669/4223, F-J=-2499/1084, F-I=-1036/2681, D-M=-369/124, D-K=-1151/423

NOTES (9)
1) 2-ply truss to be connected together with 10d Common(.148"x3") Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint A, 1499 lb uplift at joint H, 269 lb uplift at joint O, 77 lb uplift at joint N, 136 lb uplift at joint M and 2438 lb uplift at joint K.
8) Girder carries tie-in span(s): 39-4-0 from 14-0-0 to 25-8-0
9) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: A-D=-60, D-H=-60, A-K=-20, H-K=-749(F=-729)

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	D01G	DUAL RIDGE GABLE	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:52 2005 Page 1

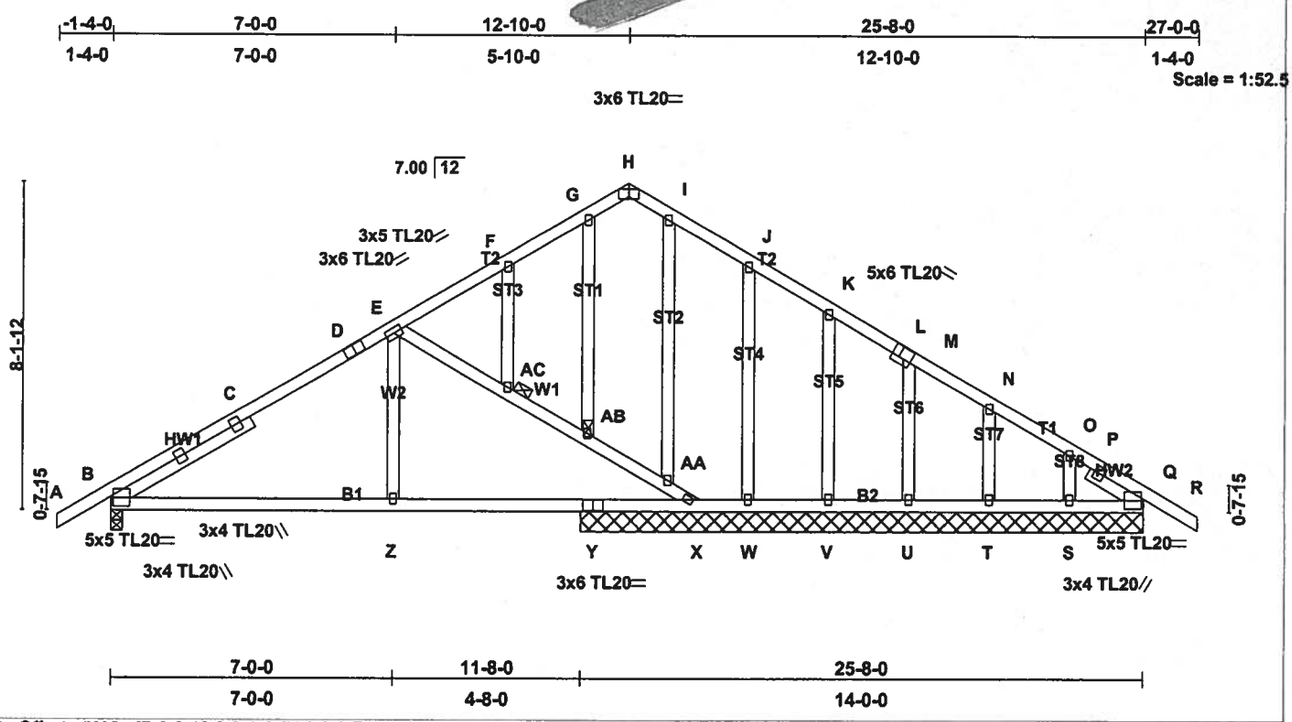


Plate Offsets (X,Y): [B:0-0-10,0-2-14], [H:0-3-0,Edge], [L:0-2-4,0-3-4], [Q:0-0-10,0-2-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.05	X-Z	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.12	X-Z	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.02	Q	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 3-11-13, Right 2 X 4 SYP No.3 1-6-5

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): AB, AC

REACTIONS (lb/size) B=766/0-3-8, X=443/14-0-0, W=88/14-0-0, V=161/14-0-0, U=156/14-0-0, T=173/14-0-0, S=78/14-0-0, Q=348/14-0-0
Max Horz B=326(load case 4)
Max Uplift B=-315(load case 5), X=-18(load case 5), W=-181(load case 6), V=-89(load case 6), U=-101(load case 6), T=-101(load case 6), S=-107(load case 6), Q=-94(load case 4)
Max Grav B=766(load case 1), X=562(load case 2), W=88(load case 1), V=164(load case 10), U=156(load case 1), T=173(load case 1), S=104(load case 10), Q=348(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/25, B-C=-931/258, C-D=-807/261, D-E=-729/282, E-F=-347/165, F-G=-274/192, G-H=-212/198, H-I=-230/209, I-J=-295/230, J-K=-281/175, K-L=-283/127, L-M=-289/115, M-N=-288/134, N-O=-284/139, O-P=-310/161, P-Q=-326/159, Q-R=0/25
BOT CHORD B-Z=-270/697, Y-Z=-270/697, X-Y=-270/697, W-X=-116/293, V-W=-116/293, U-V=-116/293, T-U=-116/293, S-T=-116/293, Q-S=-116/293
WEBS E-AC=-544/327, AB-AC=-561/344, AA-AB=-561/342, X-AA=-550/314, I-AA=-59/40, J-W=-147/138, G-AB=-17/19, K-V=-108/123, M-U=-121/123, N-T=-128/125, O-S=-75/134, F-AC=-37/32, E-Z=0/307

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 2x3 TL20 unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 315 lb uplift at joint B, 18 lb uplift at joint X, 181 lb uplift at joint W, 89 lb uplift at joint V, 101 lb uplift at joint U, 101 lb uplift at joint T, 107 lb uplift at joint S and 94 lb uplift at joint Q.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

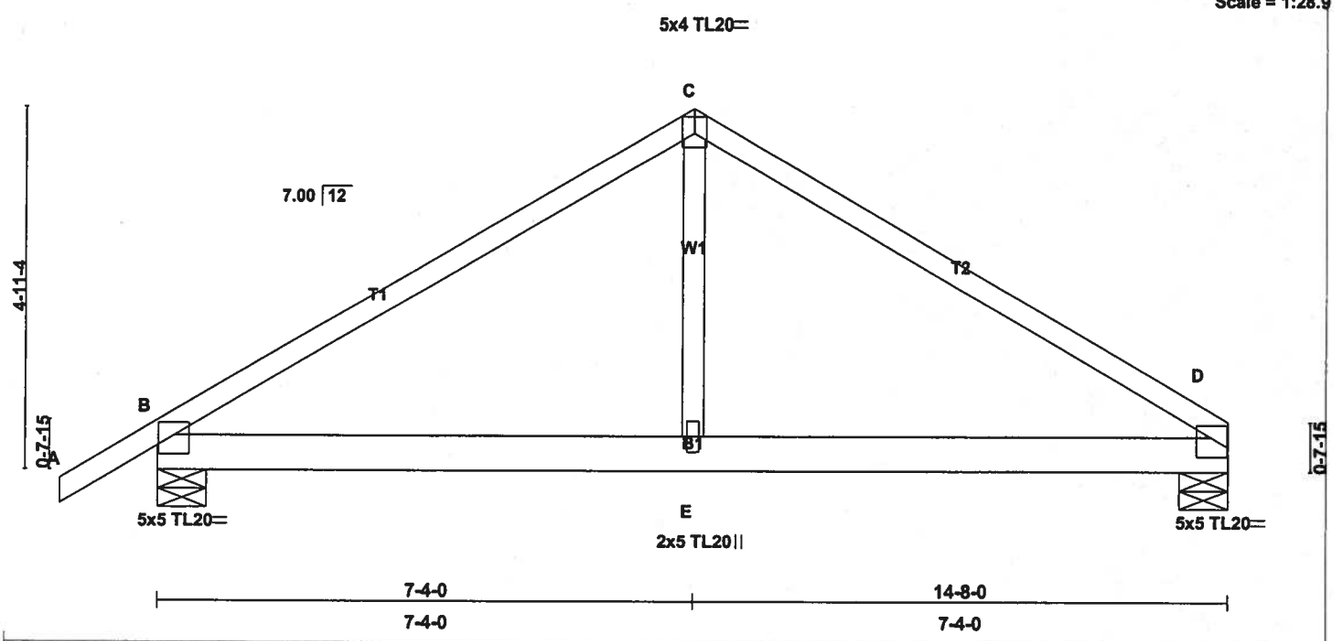
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	E01	ROOF TRUSS	2	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:53 2005 Page 1

-1-4-0 / 1-4-0 — 7-4-0 / 7-4-0 — 14-8-0 / 7-4-0

Scale = 1:28.9



7-4-0 / 7-4-0 — 14-8-0 / 7-4-0

Plate Offsets (X,Y): [B:0-0-2,0-1-7], [C:0-2-0,0-2-12], [D:0-0-2,0-1-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.04	D-E	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.06	D-E	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.01	D	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 68 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) D=554/0-8-0, B=666/0-8-0
Max Horz B=202(load case 4)
Max UpliftD=-173(load case 6), B=-290(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/29, B-C=-728/243, C-D=-727/229
BOT CHORD B-E=-93/520, D-E=-93/520
WEBS C-E=0/355

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 173 lb uplift at joint D and 290 lb uplift at joint B.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	E01G	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 8.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:53 2005 Page 1

-1-4-0 / 1-4-0 — 7-4-0 / 7-4-0 — 14-8-0 / 7-4-0

Scale = 1:29.1

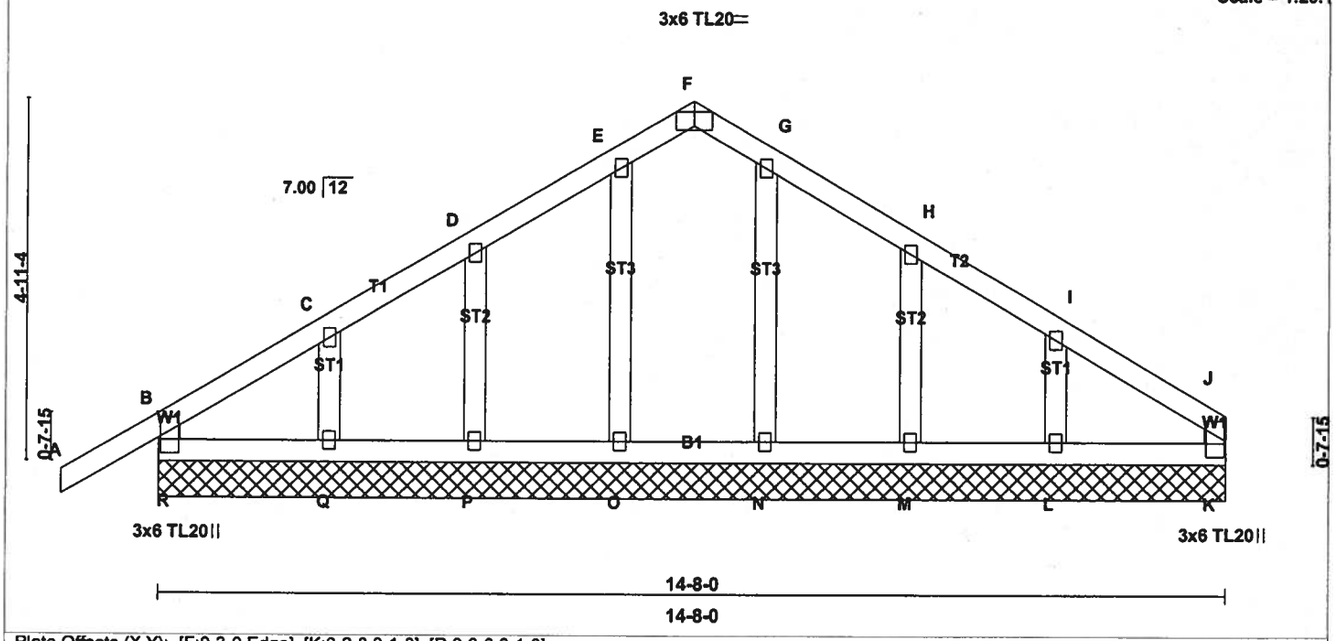


Plate Offsets (X,Y): [F:0-3-0,Edge], [K:0-2-8,0-1-8], [R:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	A	n/r	120	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	A	n/r	90		
BCLL	0.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	K	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 74 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) R=208/14-8-0, K=81/14-8-0, O=149/14-8-0, P=168/14-8-0, Q=138/14-8-0, N=153/14-8-0, M=154/14-8-0, L=187/14-8-0
Max Horz R=197(load case 4)
Max Uplift R=-79(load case 5), K=-7(load case 4), O=-29(load case 4), P=-112(load case 5), Q=-102(load case 5), N=-7(load case 3), M=-103(load case 6), L=-147(load case 6)
Max Grav R=208(load case 1), K=84(load case 10), O=149(load case 1), P=168(load case 1), Q=141(load case 9), N=153(load case 1), M=157(load case 10), L=187(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD B-R=-178/99, A-B=0/45, B-C=-138/95, C-D=-79/78, D-E=-47/115, E-F=-42/117, F-G=-42/114, G-H=-50/92, H-I=-50/41, I-J=-102/54, J-K=-68/11
BOT CHORD Q-R=-40/100, P-Q=-40/100, O-P=-40/100, N-O=-40/100, M-N=-40/100, L-M=-40/100, K-L=-40/100
WEBS E-O=-110/51, D-P=-126/138, C-Q=-108/119, G-N=-112/29, H-M=-120/133, I-L=-136/149

NOTES (11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) Truss designed for wind loads in the plane of the truss only.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x3 TL20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
9) Gable studs spaced at 2-0-0 oc.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint R, 7 lb uplift at joint K, 29 lb uplift at joint O, 112 lb uplift at joint P, 102 lb uplift at joint Q, 7 lb uplift at joint N, 103 lb uplift at joint M and 147 lb uplift at joint L.
11) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	E02	ROOF TRUSS	1	2	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:54 2005 Page 1

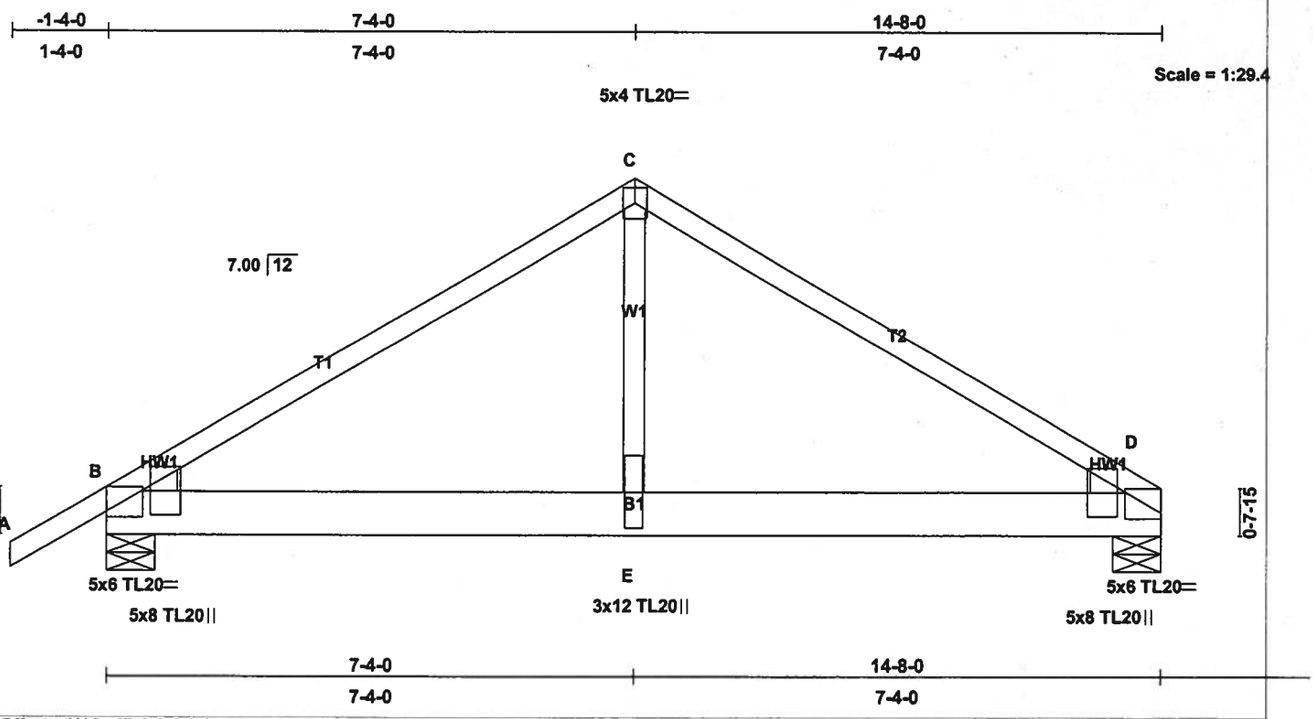


Plate Offsets (X,Y): [B:0-0-0,0-0-15], [D:0-0-0,0-0-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.11	D-E	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.19	D-E	>886	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.03	D	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 161 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.2, Right: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) B=4959/0-8-0, D=4847/0-8-0
Max Horz B=201(load case 4)
Max UpliftB=-2023(load case 5), D=-1905(load case 6)

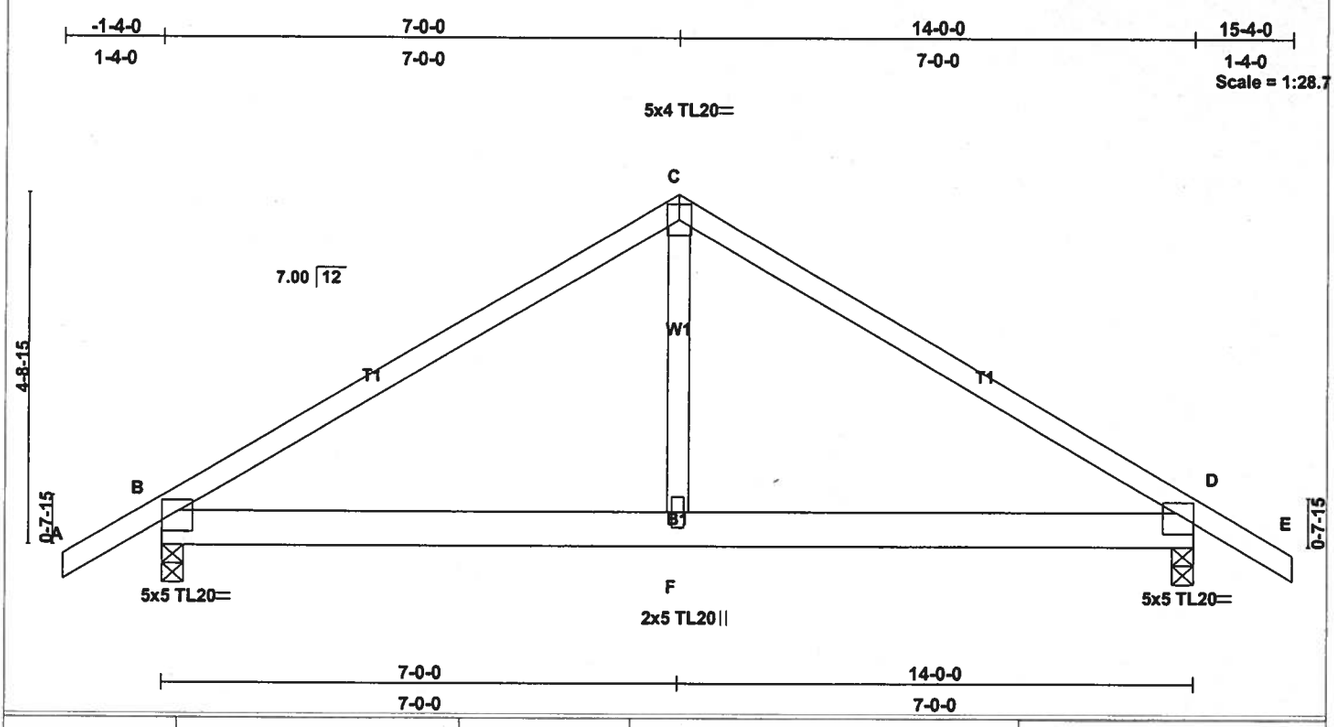
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/33, B-C=-5529/2188, C-D=-5529/2174
BOT CHORD B-E=-1742/4608, D-E=-1742/4608
WEBS C-E=-2040/5359

NOTES (9)
1) 2-ply truss to be connected together with 10d Common(.148"x3") Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2023 lb uplift at joint B and 1905 lb uplift at joint D.
8) Girder carries tie-in span(s): 33-4-0 from 0-0-0 to 14-8-0
9) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: A-C=-60, C-D=-60, B-D=-633(F=-613)

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	F01	ROOF TRUSS	4	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:55 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.03	B-F	>999	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.05	B-F	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.01	D	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 68 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) B=637/0-3-8, D=637/0-3-8
Max Horz B=-183(load case 3)
Max UpliftB=-272(load case 5), D=-272(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/29, B-C=-710/224, C-D=-710/223, D-E=0/29
BOT CHORD B-F=-77/511, D-F=-77/511
WEBS C-F=0/344

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 272 lb uplift at joint B and 272 lb uplift at joint D.
6) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

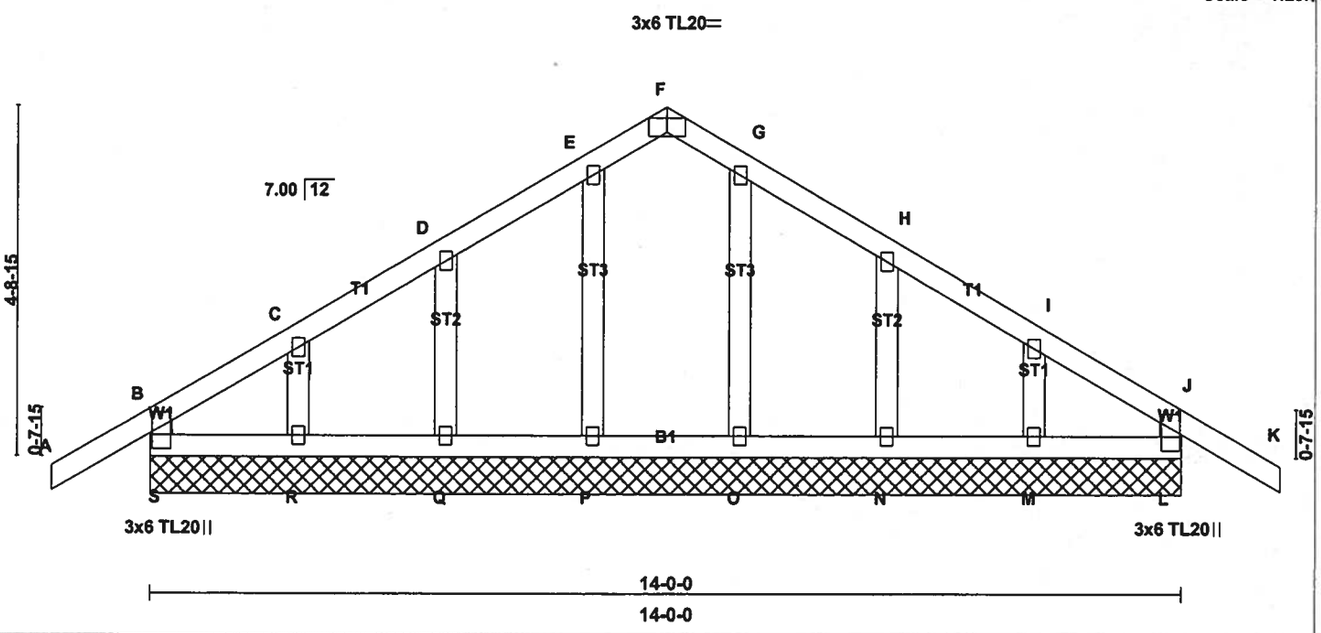
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	F01G	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:55 2005 Page 1

-1-4-0 | 7-0-0 | 14-0-0 | 15-4-0
1-4-0 | 7-0-0 | 7-0-0 | 1-4-0

Scale = 1:28.7



14-0-0
14-0-0

Plate Offsets (X,Y): [F:0-3-0,Edge], [L:0-2-8,0-1-8], [S:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.01	K	n/r	120	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.02	K	n/r	90		
BCLL	0.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	L	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) S=184/14-0-0, L=184/14-0-0, P=163/14-0-0, Q=168/14-0-0, R=122/14-0-0, O=163/14-0-0, N=168/14-0-0, M=122/14-0-0
Max Horz S=-171(load case 3)
Max Uplift S=-75(load case 5), L=-91(load case 6), P=-29(load case 4), Q=-115(load case 5), R=-89(load case 5), O=-13(load case 6), N=-119(load case 6), M=-81(load case 6)
Max Grav S=187(load case 9), L=187(load case 10), P=163(load case 1), Q=171(load case 9), R=122(load case 1), O=163(load case 1), N=171(load case 10), M=122(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD B-S=-159/92, A-B=0/45, B-C=-134/92, C-D=-81/75, D-E=-30/96, E-F=-24/104, F-G=-24/100, G-H=-26/75, H-I=-46/42, I-J=-97/54, J-K=0/45, J-L=-159/106
BOT CHORD R-S=-20/168, Q-R=-20/168, P-Q=-20/168, O-P=-20/168, N-O=-20/168, M-N=-20/168, L-M=-20/168
WEBS E-P=-124/51, D-Q=-128/141, C-R=-94/104, G-O=-124/37, H-N=-128/144, I-M=-94/100

NOTES (11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) Truss designed for wind loads in the plane of the truss only.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x3 TL20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
9) Gable studs spaced at 2-0-0 oc.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint S, 91 lb uplift at joint L, 29 lb uplift at joint P, 115 lb uplift at joint Q, 89 lb uplift at joint R, 13 lb uplift at joint O, 119 lb uplift at joint N and 81 lb uplift at joint M.
11) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	M01	ROOF TRUSS	3	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:56 2005 Page 1

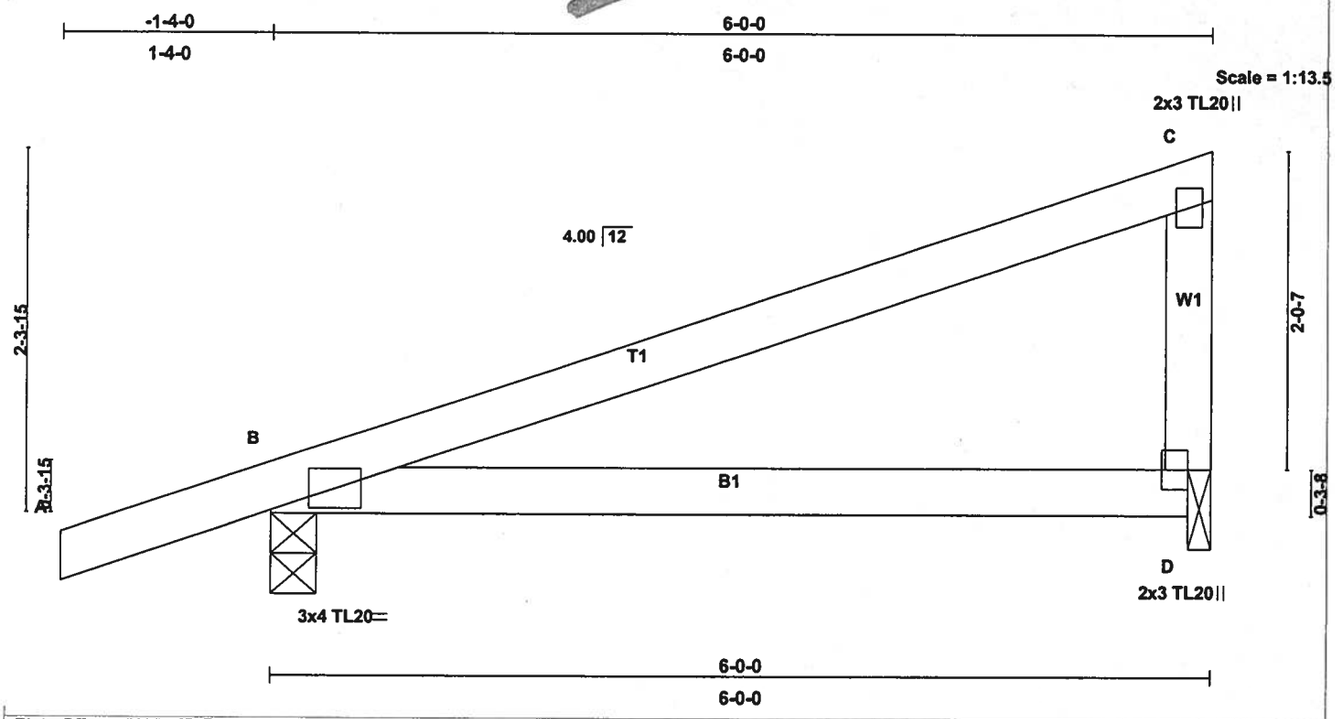


Plate Offsets (X,Y): [D:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.08	B-D	>824	240	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.14	B-D	>492	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00		n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) D=217/0-1-12, B=329/0-3-8
Max Horz B=141(load case 3)
Max UpliftD=-179(load case 3), B=-264(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/24, B-C=-64/41, C-D=-160/128
BOT CHORD B-D=0/0

NOTES (7)
1) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint D and 264 lb uplift at joint B.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V01	ROOF TRUSS 3x4 TL20	3	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:56 2005 Page 1

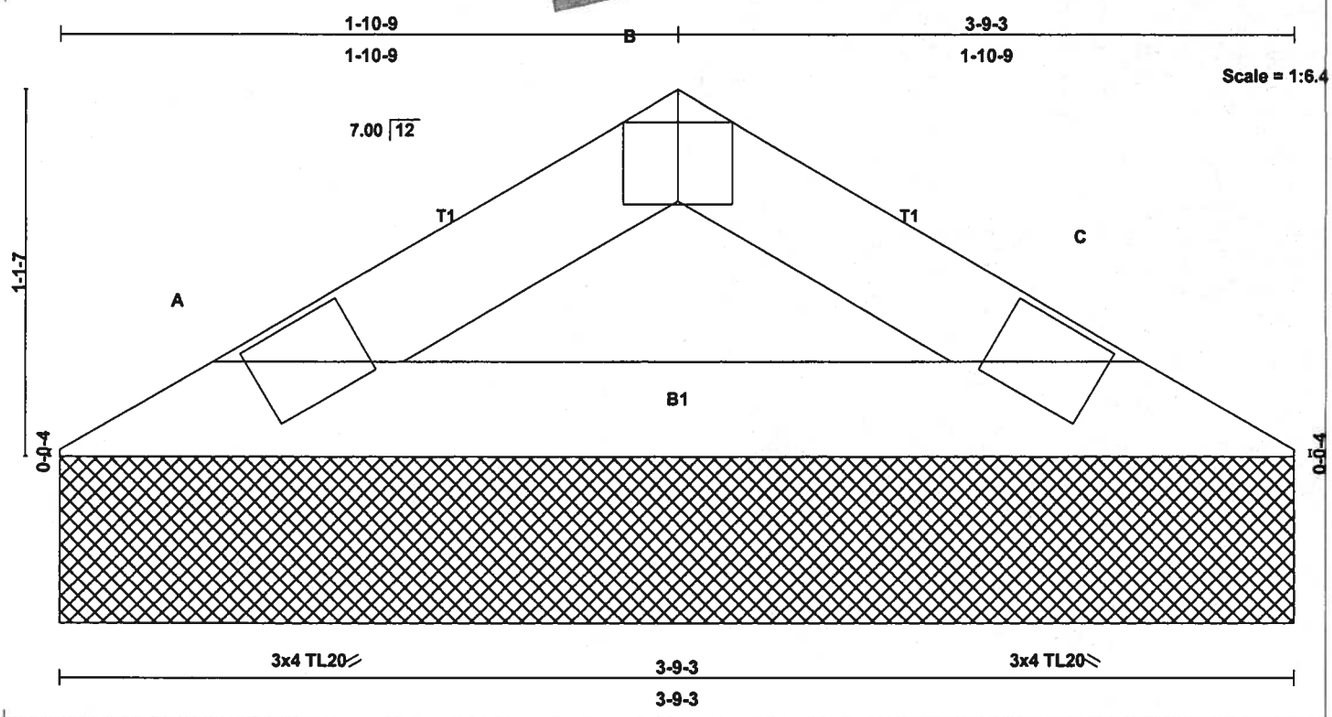


Plate Offsets (X,Y): [B:0-2-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.03	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	C	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 10 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=110/3-9-3, C=110/3-9-3
Max Horz A=-33(load case 3)
Max UpliftA=-35(load case 5), C=-35(load case 6)

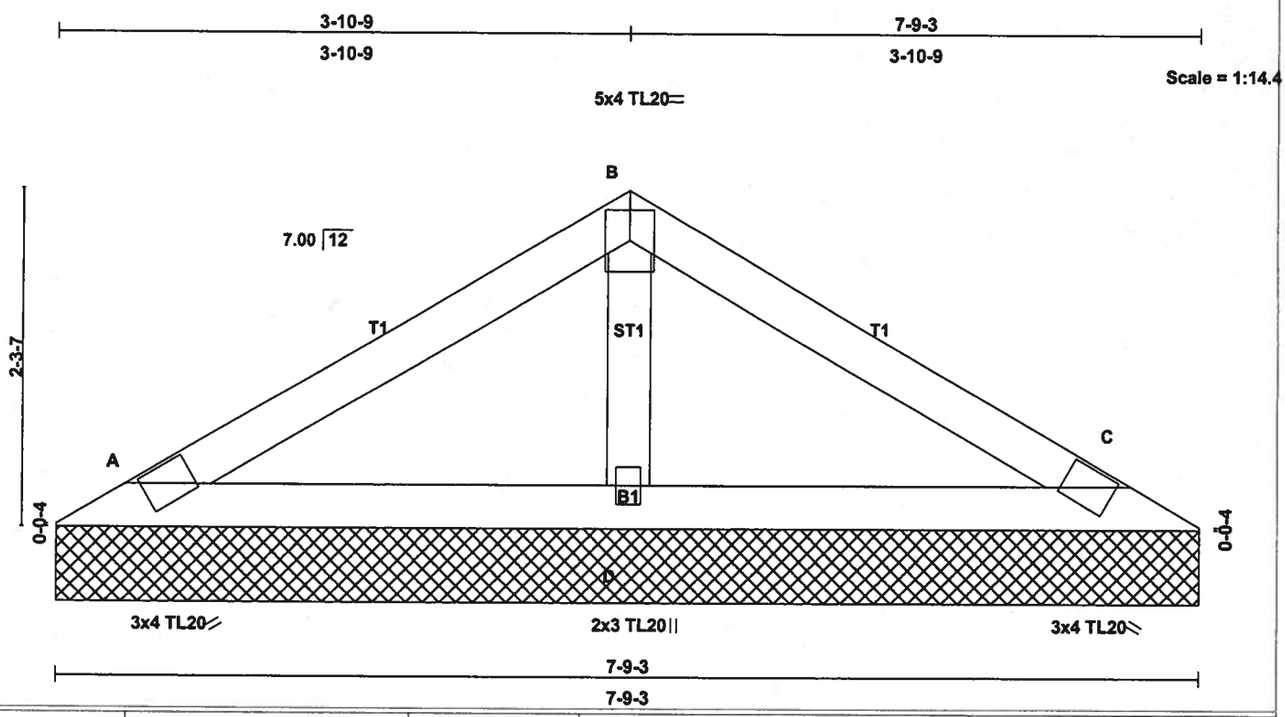
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-99/54, B-C=-99/54
BOT CHORD A-C=-29/71

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint A and 35 lb uplift at joint C.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V02	ROOF TRUSS	3	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:57 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	C	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=142/7-9-3, C=142/7-9-3, D=257/7-9-3
Max Horz A=-81(load case 3)
Max UpliftA=-67(load case 5), C=-73(load case 6), D=-37(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-71/59, B-C=-71/46
BOT CHORD A-D=-18/36, C-D=-18/36
WEBS B-D=-172/88

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint A, 73 lb uplift at joint C and 37 lb uplift at joint D.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V03	ROOF TRUSS	3	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:57 2005 Page 1

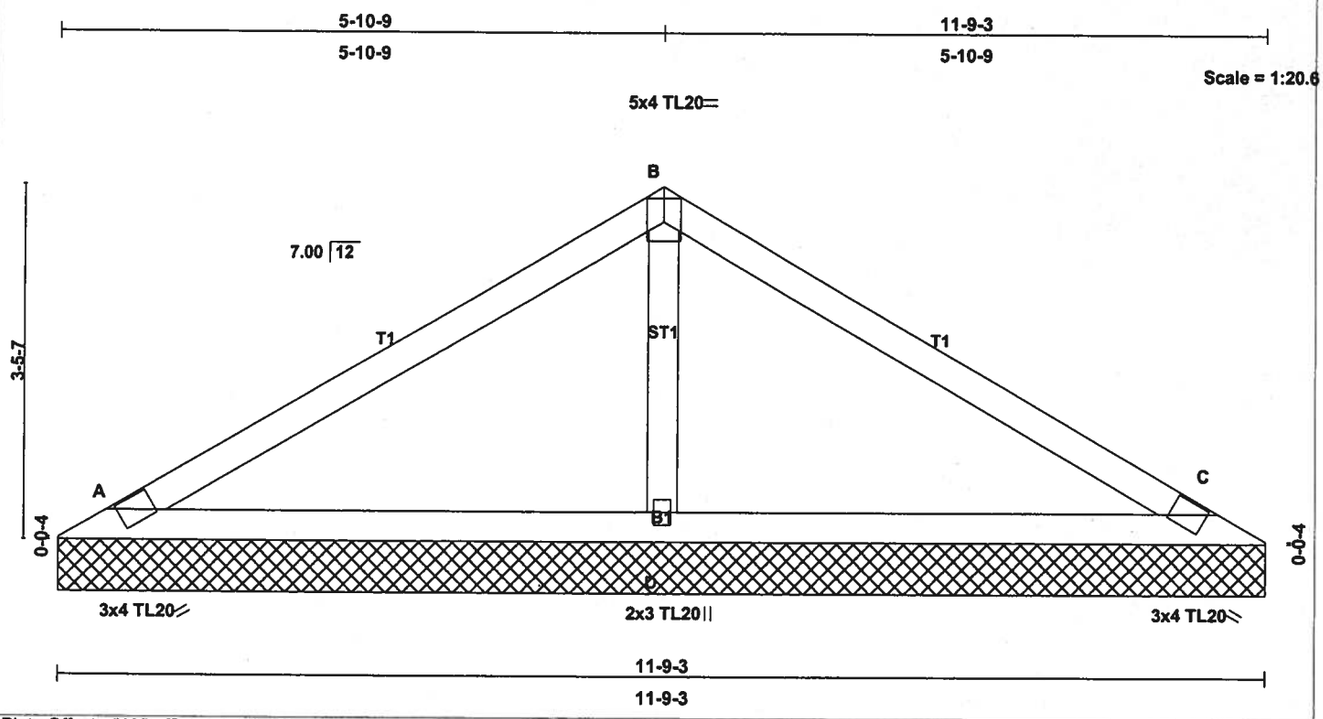


Plate Offsets (X,Y): [B:0-2-0,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.00	C	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 40 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=193/11-9-3, C=193/11-9-3, D=475/11-9-3
Max Horz A=-130(load case 3)
Max UpliftA=-76(load case 5), C=-86(load case 6), D=-120(load case 5)
Max Grav A=196(load case 9), C=196(load case 10), D=475(load case 1)

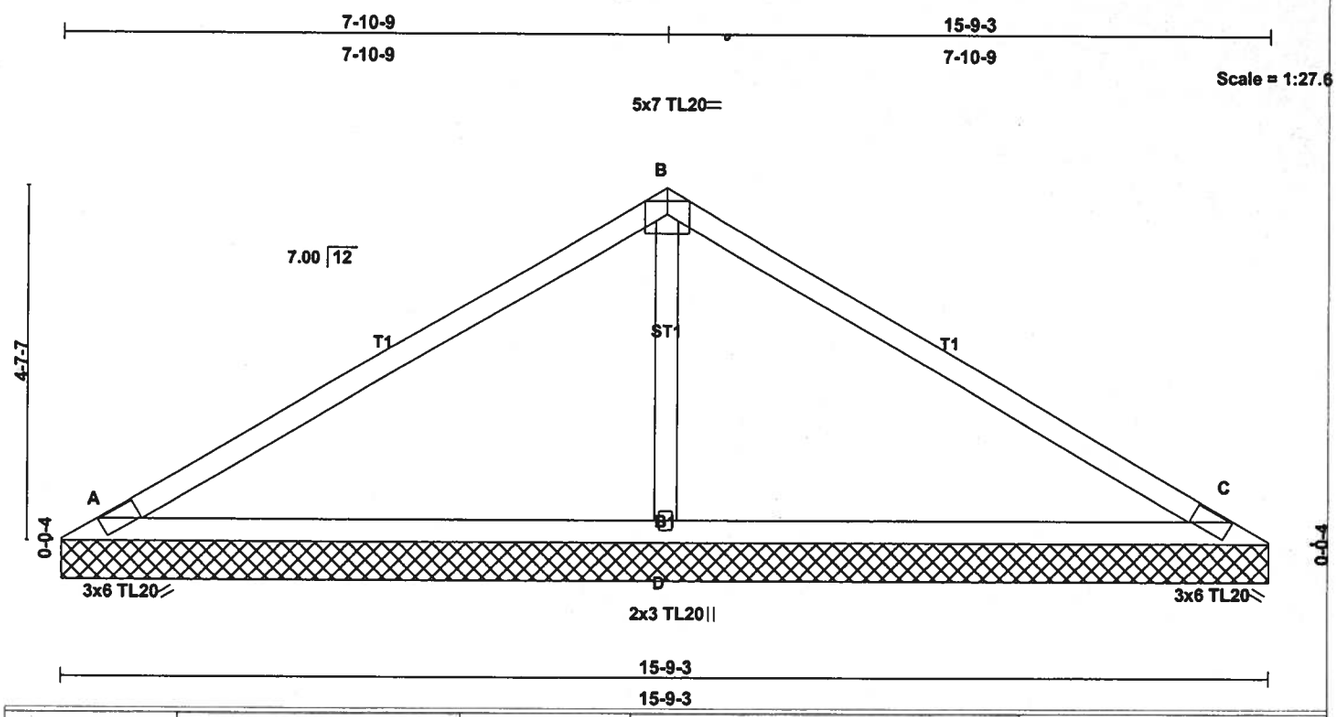
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-131/96, B-C=-131/76
BOT CHORD A-D=-29/58, C-D=-29/58
WEBS B-D=-309/157

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint A, 86 lb uplift at joint C and 120 lb uplift at joint D.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V04	ROOF TRUSS	3	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:58 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.00	C	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 55 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=264/15-9-3, C=264/15-9-3, D=652/15-9-3
Max Horz A=178(load case 4)
Max UpliftA=-104(load case 5), C=-118(load case 6), D=-164(load case 5)
Max Grav A=269(load case 9), C=269(load case 10), D=652(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-180/132, B-C=-180/104
BOT CHORD A-D=-40/79, C-D=-40/79
WEBS B-D=-425/215

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint A, 118 lb uplift at joint C and 164 lb uplift at joint D.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

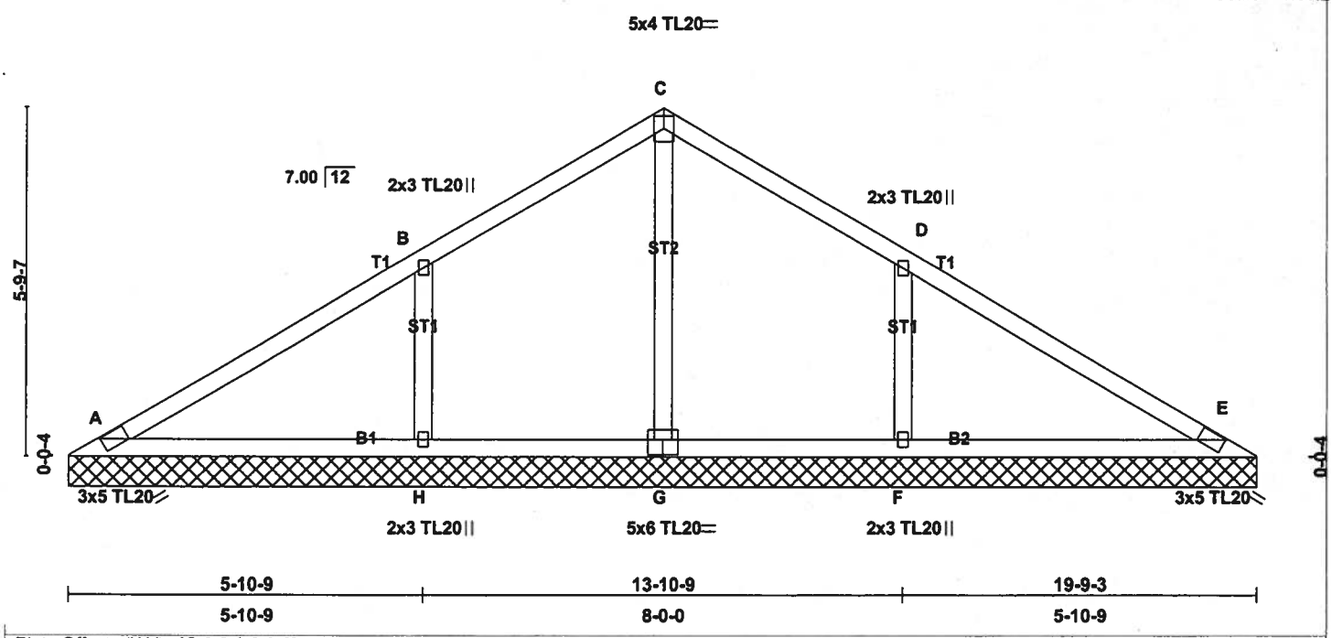
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V05	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:59 2005 Page 1

5-10-9	9-10-9	13-10-9	19-9-3
5-10-9	4-0-0	4-0-0	5-10-9

Scale = 1:35.2



5-10-9	13-10-9	19-9-3
5-10-9	8-0-0	5-10-9

Plate Offsets (X,Y): [G:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.00	E	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 78 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=188/19-9-3, E=188/19-9-3, G=205/19-9-3, H=460/19-9-3, F=460/19-9-3
Max Horz A=-226(load case 3)
Max Uplift A=-46(load case 6), E=-41(load case 6), H=-286(load case 5), F=-286(load case 6)
Max Grav A=188(load case 1), E=188(load case 1), G=205(load case 1), H=464(load case 9), F=464(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-163/150, B-C=-101/181, C-D=-101/166, D-E=-101/71
BOT CHORD A-H=-44/114, G-H=-44/114, F-G=-44/114, E-F=-44/114
WEBS C-G=-161/13, B-H=-329/319, D-F=-329/319

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint A, 41 lb uplift at joint E, 286 lb uplift at joint H and 286 lb uplift at joint F.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V06	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 08:59:59 2005 Page 1

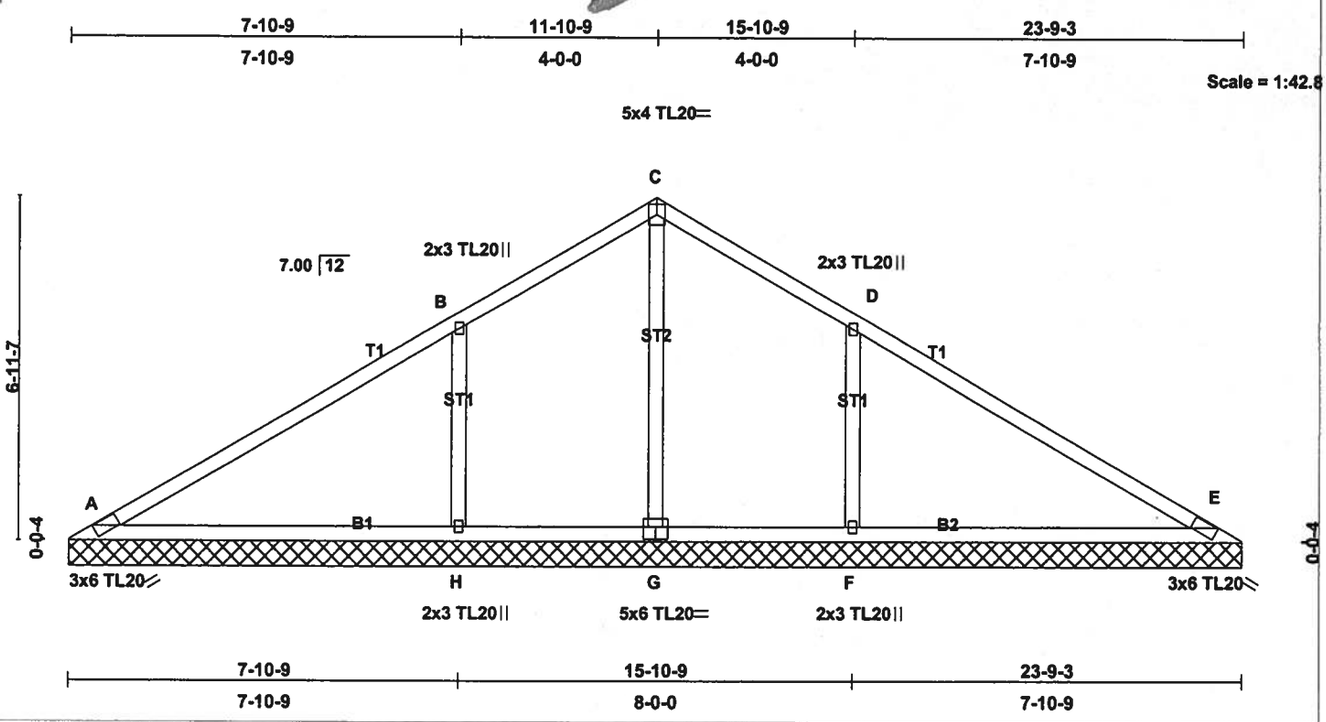


Plate Offsets (X,Y): [G:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.01	E	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=245/23-9-3, E=245/23-9-3, G=77/23-9-3, H=627/23-9-3, F=627/23-9-3
Max Horz A=-274(load case 3)
Max UpliftA=-56(load case 6), E=-59(load case 6), H=-385(load case 5), F=-385(load case 6)
Max Grav A=245(load case 1), E=245(load case 1), G=201(load case 6), H=627(load case 1), F=627(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-191/179, B-C=-108/221, C-D=-108/211, D-E=-113/114
BOT CHORD A-H=-50/153, G-H=-50/153, F-G=-50/153, E-F=-50/153
WEBS C-G=-143/2, B-H=-433/418, D-F=-433/418

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint A, 59 lb uplift at joint E, 385 lb uplift at joint H and 385 lb uplift at joint F.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

qpwe

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V07	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 09:00:00 2005 Page 1

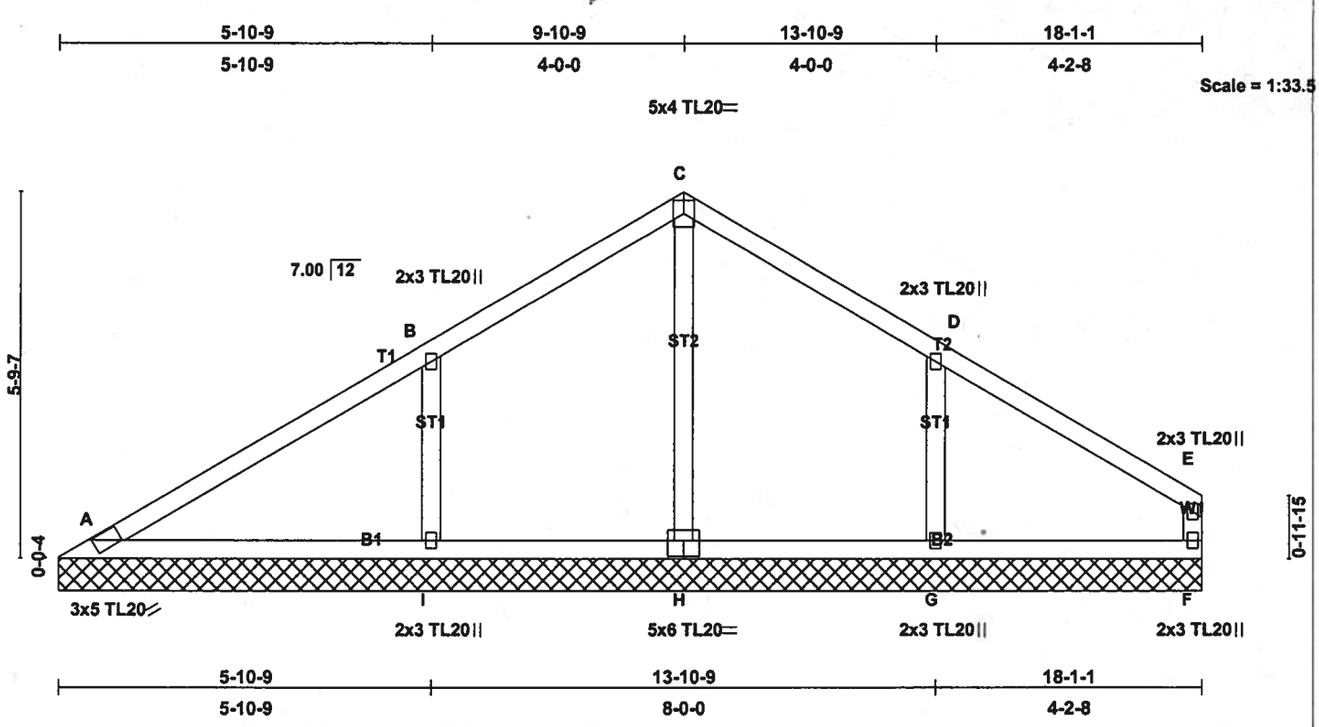


Plate Offsets (X,Y): [H:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	F	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=221/18-1-1, F=193/18-1-1, H=177/18-1-1, I=451/18-1-1, G=352/18-1-1
Max Horz A=222(load case 4)
Max UpliftA=-89(load case 6), F=-79(load case 6), H=-10(load case 4), I=-284(load case 5), G=-234(load case 6)
Max Grav A=221(load case 1), F=193(load case 1), H=177(load case 1), I=460(load case 9), G=358(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-213/214, B-C=-163/262, C-D=-157/242, D-E=-149/80, E-F=-159/110
BOT CHORD A-I=-8/80, H-I=-8/80, G-H=-8/80, F-G=-8/80
WEBS C-H=-130/57, B-I=-327/316, D-G=-267/273

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint A, 79 lb uplift at joint F, 10 lb uplift at joint H, 284 lb uplift at joint I and 234 lb uplift at joint G.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V08	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 09:00:00 2005 Page 1

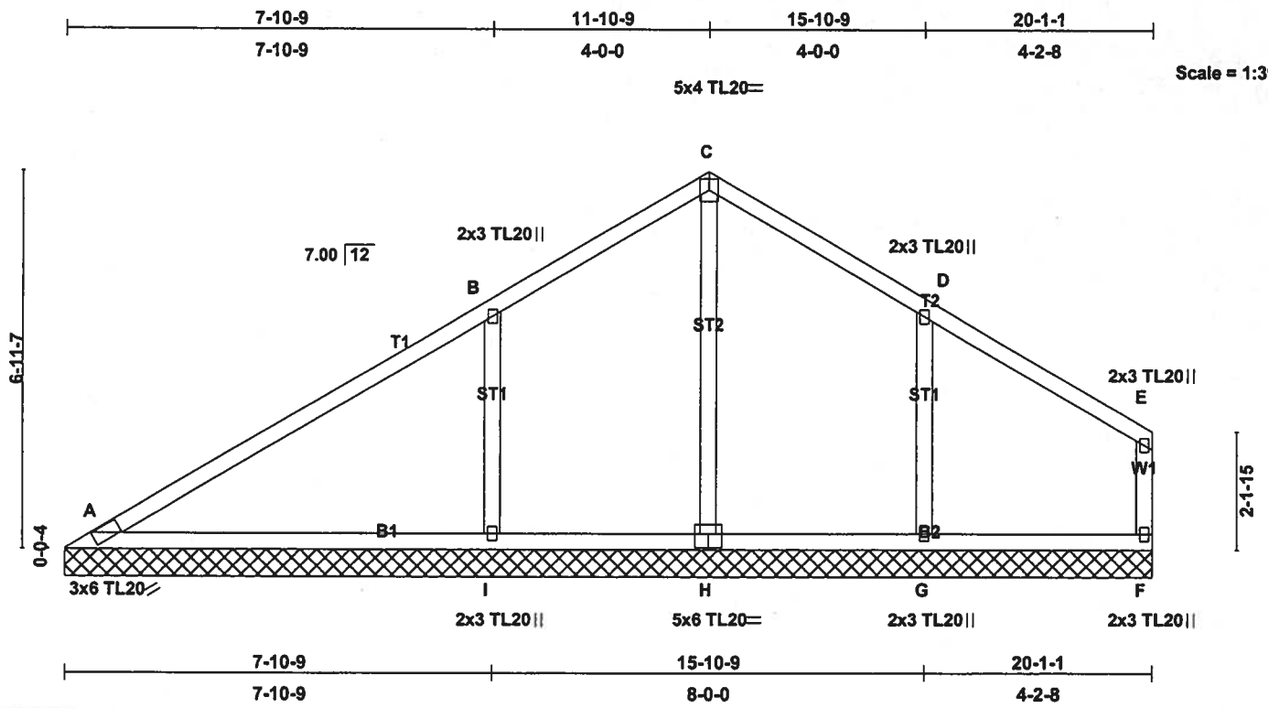


Plate Offsets (X,Y): [H:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.00	F	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 88 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=263/20-1-1, F=170/20-1-1, H=153/20-1-1, I=604/20-1-1, G=365/20-1-1
Max Horz A=265(load case 4)
Max UpliftA=-93(load case 6), F=-94(load case 6), I=-377(load case 5), G=-230(load case 6)
Max Grav A=263(load case 1), F=171(load case 10), H=205(load case 6), I=612(load case 9), G=365(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-249/253, B-C=-135/288, C-D=-114/260, D-E=-101/95, E-F=-133/118
BOT CHORD A-I=-13/39, H-I=-13/39, G-H=-13/39, F-G=-13/39
WEBS C-H=-156/39, B-I=-427/411, D-G=-271/278

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint A, 94 lb uplift at joint F, 377 lb uplift at joint I and 230 lb uplift at joint G.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V09	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 09:00:01 2005 Page 1

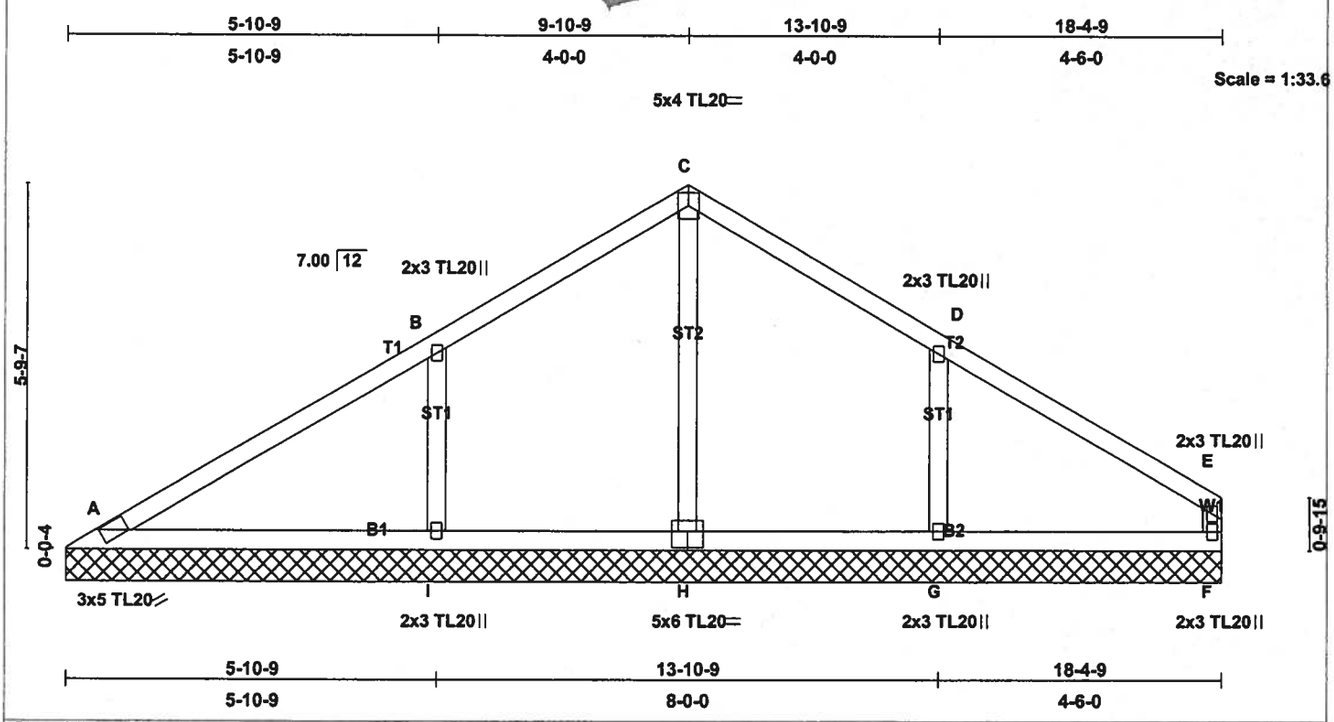


Plate Offsets (X,Y): [H:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	F	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=231/18-4-9, F=211/18-4-9, H=152/18-4-9, I=453/18-4-9, G=372/18-4-9
Max Horz A=223(load case 4)
Max UpliftA=-91(load case 6), F=-82(load case 6), H=-13(load case 4), I=-284(load case 5), G=-246(load case 6)
Max Grav A=231(load case 1), F=211(load case 1), H=177(load case 6), I=461(load case 9), G=377(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-214/211, B-C=-184/267, C-D=-178/248, D-E=-173/81, E-F=-177/118
BOT CHORD A-I=-6/97, H-I=-6/97, G-H=-6/97, F-G=-6/97
WEBS C-H=-129/60, B-I=-327/317, D-G=-278/283

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint A, 82 lb uplift at joint F, 13 lb uplift at joint H, 284 lb uplift at joint I and 246 lb uplift at joint G.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V10	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 09:00:02 2005 Page 1

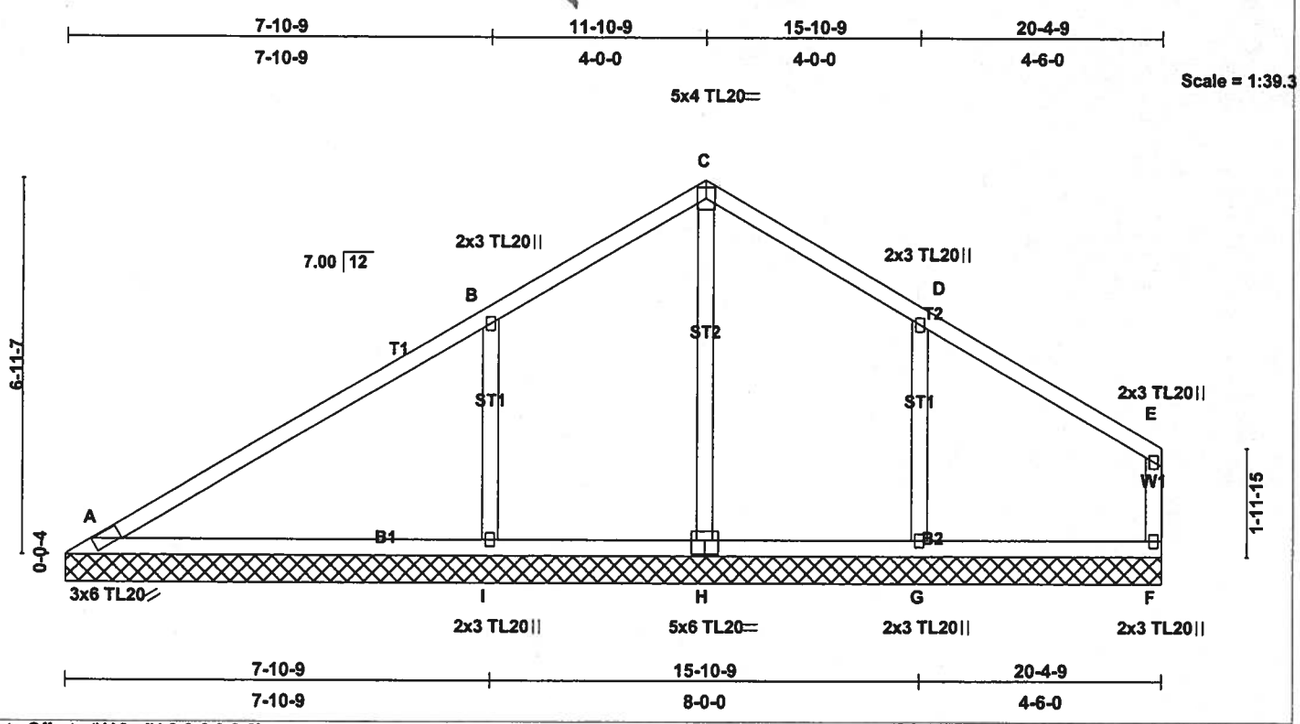


Plate Offsets (X,Y): [H:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.00	F	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 89 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=270/20-4-9, F=191/20-4-9, H=135/20-4-9, I=605/20-4-9, G=377/20-4-9
Max Horz A=266(load case 4)
Max UpliftA=-101(load case 6), F=-105(load case 6), I=-378(load case 5), G=-238(load case 6)
Max Grav A=270(load case 1), F=192(load case 10), H=224(load case 6), I=612(load case 9), G=377(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-254/262, B-C=-149/305, C-D=-130/278, D-E=-120/109, E-F=-151/131
BOT CHORD A-I=-18/51, H-I=-18/51, G-H=-19/51, F-G=-19/51
WEBS C-H=-176/45, B-I=-427/411, D-G=-280/288

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint A, 105 lb uplift at joint F, 378 lb uplift at joint I and 238 lb uplift at joint G.
7) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	416-AMERICASHOMEPLACE-DWH-092805
55023251	V11	ROOF TRUSS	1	1	Job Reference (optional)

Universal Forest Products,Inc., Burlington, NC 27215, Dan Habla — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 28 09:00:03 2005 Page 1

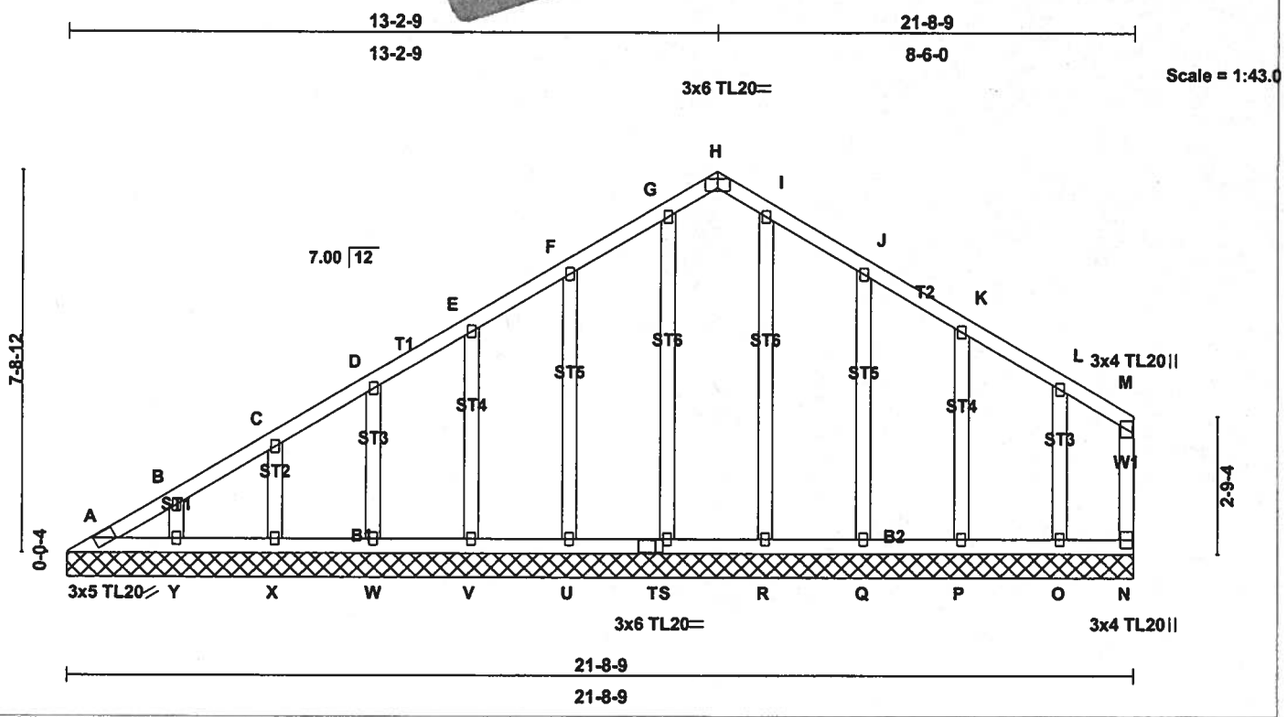


Plate Offsets (X,Y): [H:0-3-0,Edge], [T:0-1-13,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	n/a	-	n/a	999	TL20	245/193
TCDL	10.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.00	N	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 134 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=55/21-8-9, N=52/21-8-9, S=156/21-8-9, U=161/21-8-9, V=160/21-8-9, W=160/21-8-9, X=159/21-8-9, Y=166/21-8-9, R=157/21-8-9, Q=160/21-8-9, P=165/21-8-9, O=136/21-8-9
Max Horz A=295(load case 4)
Max Uplift A=-94(load case 3), N=-7(load case 4), S=-49(load case 4), U=-121(load case 5), V=-95(load case 5), W=-100(load case 5), X=-98(load case 5), Y=-103(load case 5), Q=-130(load case 6), P=-90(load case 6), O=-138(load case 6)
Max Grav A=151(load case 4), N=52(load case 1), S=158(load case 3), U=163(load case 9), V=160(load case 1), W=160(load case 9), X=159(load case 1), Y=166(load case 9), R=157(load case 1), Q=162(load case 10), P=165(load case 1), O=138(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-318/230, B-C=-272/224, C-D=-222/218, D-E=-172/212, E-F=-123/204, F-G=-73/254, G-H=-34/217, H-I=-34/214, I-J=-36/232, J-K=-36/145, K-L=-34/73, L-M=-23/20, M-N=-37/7
BOT CHORD A-Y=-7/18, X-Y=-7/18, W-X=-7/18, V-W=-7/18, U-V=-7/18, T-U=-7/18, S-T=-7/18, R-S=-7/18, Q-R=-7/18, P-Q=-7/18, O-P=-7/18, N-O=-7/18
WEBS G-S=-135/73, F-U=-123/145, E-V=-120/120, D-W=-120/124, C-X=-120/124, B-Y=-119/119, I-R=-117/0, J-Q=-122/153, K-P=-123/118, L-O=-105/134

NOTES (8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=15ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 2x3 TL20 unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Gable requires continuous bottom chord bearing.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint A, 7 lb uplift at joint N, 49 lb uplift at joint S, 121 lb uplift at joint U, 95 lb uplift at joint V, 100 lb uplift at joint W, 98 lb uplift at joint X, 103 lb uplift at joint Y, 130 lb uplift at joint Q, 90 lb uplift at joint P and 138 lb uplift at joint O.
8) Truss shall be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. Provide bracing where indicated and within 4" of interior joints. Bracing indicated is to reduce buckling of individual members only and does not replace erection and permanent bracing. Engineer's certification vaild only when truss is fabricated by a UFPI operated plant. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. The truss designer accepts no responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Any references to job names and locations are for administrative purposes only and are not part of the review or certification of the truss designer.

LOAD CASE(S) Standard

Universal Forest Products,Inc.
5631 South NC 62
Burlington
Phone: 1-800-476-9356 Fax: 1-336-227-0599

To:

AMERICAS HOMEPLACE

Reaction Summary

Job Number:
Page: 1
Date: 09-28-2005 - 9:20:15 AM
Project ID: 55023251

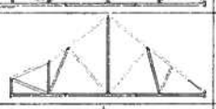
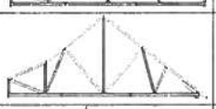
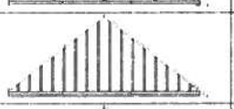
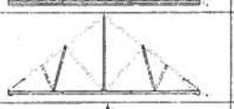
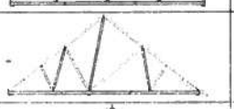
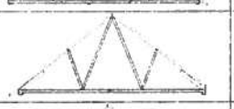
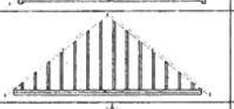
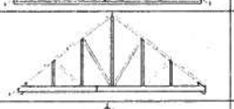
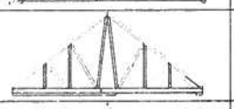
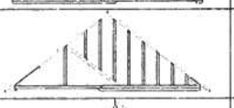
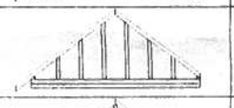
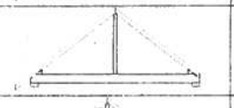
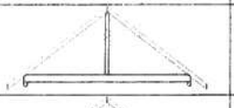
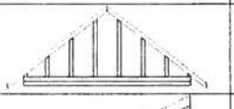
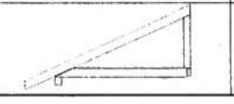
Project: SCHUENEMAN Block No:
Model: Lot No:

Contact: Site: Office:
Name:
Phone:
Fax:

Tentative Delivery Date:

Deliver To:

Account No:
Designer: GLENN HENRY
Salesperson:
Quote Number:

Profile:	Qty:	Truss Id:	Span:	Truss Type:	Slope:	Reactions:
	11	A01	39 - 4 - 0	ROOF TRUSS	4.00 0.00	Joint 2 Joint 16 Joint 10 163 lbs. 1820 lbs. 1279 lbs. -379 lbs. -1097 lbs. -667 lbs.
	1	A02	39 - 4 - 0	ROOF TRUSS	4.00 0.00	Joint 1 Joint 15 Joint 9 85 lbs. 1834 lbs. 1278 lbs. -45 lbs. -1591 lbs. -1043 lbs.
	2	A03	39 - 4 - 0	ROOF TRUSS	4.00 0.00	Joint 14 Joint 8 1328 lbs. 1328 lbs. -688 lbs. -684 lbs.
	1	A04	39 - 4 - 0	ROOF TRUSS	4.00 0.00	Joint 15 Joint 8 1326 lbs. 1409 lbs. -687 lbs. -789 lbs.
	6	B01	33 - 4 - 0	ROOF TRUSS	7.00 0.00	Joint 1 Joint 9 1332 lbs. 1415 lbs. -688 lbs. -794 lbs.
	2	B01G	33 - 4 - 0	ROOF TRUSS	7.00 0.00	Joint 1 Joint 27 Joint 26 Joint 25 Joint 23 251 lbs. 158 lbs. 168 lbs. 116 lbs. 183 lbs. -180 lbs. -126 lbs. -132 lbs. -121 lbs. -124 lbs.
	3	B02	33 - 4 - 0	ROOF TRUSS	7.00 0.00	Joint 1 Joint 9 1332 lbs. 1415 lbs. -688 lbs. -794 lbs.
	1	B03	33 - 4 - 0	ROOF TRUSS	7.00 0.00	Joint 1 Joint 9 1333 lbs. 1333 lbs. -689 lbs. -689 lbs.
	11	B04	33 - 4 - 0	ROOF TRUSS	7.00 0.00	Joint 1 Joint 13 Joint 9 474 lbs. 1579 lbs. 745 lbs. -239 lbs. -787 lbs. -493 lbs.
	3	C01	28 - 6 - 8	ROOF TRUSS	7.00 0.00	Joint 2 Joint 8 1224 lbs. 1140 lbs. -695 lbs. -588 lbs.
	1	C01G	28 - 8 - 0	ROOF TRUSS	7.00 0.00	Joint 2 Joint 24 Joint 23 Joint 22 Joint 20 197 lbs. 158 lbs. 166 lbs. 100 lbs. 174 lbs. -210 lbs. -125 lbs. -134 lbs. -103 lbs. -123 lbs.
	2	(1) 2-Ply C02	28 - 6 - 8 2X4/2X8	ROOF TRUSS	7.00 0.00	Joint 2 Joint 10 **9788 lbs. 9682 lbs.** **-5741 lbs. -5606 lbs.**
	2	(1) 2-Ply D01	25 - 8 - 0 2X4/2X6	ROOF TRUSS	7.00 0.00	Joint 1 Joint 14 Joint 13 Joint 11 Joint 8 149 lbs. 153 lbs. 369 lbs. **6109 lbs.** 3778 lbs. -155 lbs. -96 lbs. -209 lbs. **-3576 lbs.** **-2200 lbs.**
	1	D01G	25 - 8 - 0	DUAL RIDGE GAB	7.00 0.00	Joint 2 Joint 21 Joint 20 Joint 19 Joint 17 766 lbs. 156 lbs. 173 lbs. 104 lbs. 348 lbs. -462 lbs. -126 lbs. -132 lbs. -115 lbs. -175 lbs.
	2	E01	14 - 8 - 0 2X4/2X6	ROOF TRUSS	7.00 0.00	Joint 2 Joint 4 666 lbs. 554 lbs. -425 lbs. -285 lbs.
	1	E01G	14 - 8 - 0	ROOF TRUSS	7.00 0.00	Joint 15 Joint 14 Joint 13 Joint 12 Joint 11 149 lbs. 153 lbs. 157 lbs. 187 lbs. 84 lbs. -69 lbs. -48 lbs. -128 lbs. -176 lbs. -31 lbs.
	2	(1) 2-Ply E02	14 - 8 - 0 2X4/2X8	ROOF TRUSS	7.00 0.00	Joint 2 Joint 4 4959 lbs. 4847 lbs. **-2947 lbs.** **-2807 lbs.**
	4	F01	14 - 0 - 0 2X4/2X6	ROOF TRUSS	7.00 0.00	Joint 2 Joint 4 637 lbs. 637 lbs. -401 lbs. -401 lbs.
	1	F01G	14 - 0 - 0	ROOF TRUSS	7.00 0.00	Joint 16 Joint 15 Joint 14 Joint 13 Joint 12 163 lbs. 163 lbs. 171 lbs. 122 lbs. 187 lbs. -70 lbs. -55 lbs. -146 lbs. -98 lbs. -137 lbs.
	3	M01	6 - 0 - 0	ROOF TRUSS	4.00 0.00	Joint 2 Joint 4 329 lbs. 217 lbs. -334 lbs. -221 lbs.

Universal Forest Products,Inc.
5631 South NC 62
Burlington
Phone: 1-800-476-9356 Fax: 1-336-227-0599

To:

AMERICAS HOMEPLACE

Reaction Summary

Job Number:
Page: 2
Date: 09-28-2005 - 9:20:15 AM
Project ID: 55023251

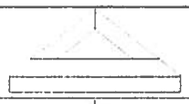
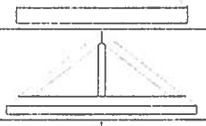
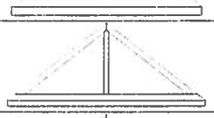
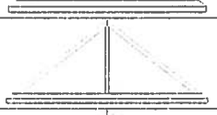
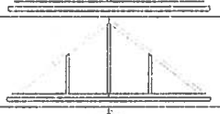
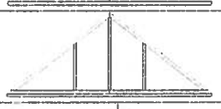
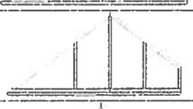
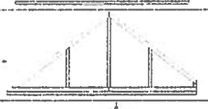
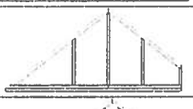
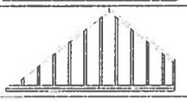
Project: SCHUENEMAN Block No:
Model: Lot No:

Contact: Site: Office:
Name:
Phone:
Fax:

Tentative Delivery Date:

Deliver To:

Account No:
Designer: GLENN HENRY
Salesperson:
Quote Number:

Profile:	Qty:	Truss Id:	Span:	Truss Type:	Slope:	Reactions:
	3	V01	3 - 9 - 3	ROOF TRUSS	7.00 0.00	Joint 1 / 110 lbs. / -58 lbs. Joint 3 / 110 lbs. / -58 lbs.
	3	V02	7 - 9 - 3	ROOF TRUSS	7.00 0.00	Joint 1 / 142 lbs. / -90 lbs. Joint 4 / 257 lbs. / -103 lbs. Joint 3 / 142 lbs. / -96 lbs.
	3	V03	11 - 9 - 3	ROOF TRUSS	7.00 0.00	Joint 1 / 196 lbs. / -112 lbs. Joint 4 / 475 lbs. / -225 lbs. Joint 3 / 196 lbs. / -122 lbs.
	3	V04	15 - 9 - 3	ROOF TRUSS	7.00 0.00	Joint 1 / 269 lbs. / -153 lbs. Joint 4 / 652 lbs. / -308 lbs. Joint 3 / 269 lbs. / -167 lbs.
	1	V05	19 - 9 - 3	ROOF TRUSS	7.00 0.00	Joint 1 / 188 lbs. / -87 lbs. Joint 8 / 464 lbs. / -365 lbs. Joint 7 / 205 lbs. / -41 lbs. Joint 6 / 464 lbs. / -364 lbs. Joint 5 / 188 lbs. / -82 lbs.
	1	V06	23 - 9 - 3	ROOF TRUSS	7.00 0.00	Joint 1 / 245 lbs. / -110 lbs. Joint 8 / 627 lbs. / -491 lbs. Joint 7 / 201 lbs. / -31 lbs. Joint 6 / 627 lbs. / -491 lbs. Joint 5 / 245 lbs. / -112 lbs.
	1	V07	18 - 1 - 1	ROOF TRUSS	7.00 0.00	Joint 1 / 221 lbs. / -133 lbs. Joint 9 / 460 lbs. / -360 lbs. Joint 8 / 177 lbs. / -81 lbs. Joint 7 / 358 lbs. / -293 lbs. Joint 6 / 193 lbs. / -116 lbs.
	1	V08	20 - 1 - 1	ROOF TRUSS	7.00 0.00	Joint 1 / 263 lbs. / -147 lbs. Joint 9 / 612 lbs. / -480 lbs. Joint 8 / 205 lbs. / -64 lbs. Joint 7 / 365 lbs. / -291 lbs. Joint 6 / 171 lbs. / -124 lbs.
	1	V09	18 - 4 - 9	ROOF TRUSS	7.00 0.00	Joint 1 / 231 lbs. / -137 lbs. Joint 9 / 461 lbs. / -361 lbs. Joint 8 / 177 lbs. / -80 lbs. Joint 7 / 377 lbs. / -308 lbs. Joint 6 / 211 lbs. / -123 lbs.
	1	V10	20 - 4 - 9	ROOF TRUSS	7.00 0.00	Joint 1 / 270 lbs. / -156 lbs. Joint 9 / 612 lbs. / -480 lbs. Joint 8 / 224 lbs. / -70 lbs. Joint 7 / 377 lbs. / -301 lbs. Joint 6 / 192 lbs. / -139 lbs.
	1	V11	21 - 8 - 9	ROOF TRUSS	7.00 0.00	Joint 1 / 151 lbs. / -119 lbs. Joint 17 / 162 lbs. / -153 lbs. Joint 16 / 165 lbs. / -120 lbs. Joint 15 / 138 lbs. / -156 lbs. Joint 14 / 52 lbs. / -23 lbs.

Call 386 758-1163 before you leave
give them check #4599 + file

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0512-22 Date Received 12/8/05 By G Permit # 958/24096

Application Approved by - Zoning Official BLK Date 12.12.05 Plans Examiner OK JTH Date 1-17-06

Flood Zone X per survey Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments

Applicants Name Janine A Nance JNP Permits Phone FAX - 352 351-4251

Address 4390 NE 34th Court Ocala FL 34478

Owners Name Fredrick + Sally Schueneman Phone

911 Address 182 SW Honeybee Ct, Ft. White, FL 32038

Contractors Name Americas HomePlace, Inc. / Stanton Van Conner Phone

Address 3101 SW 34th Ave #902, Ocala, FL 34474 CRC057203

Fee Simple Owner Name & Address Same as above

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address J. Lee Smith Engineering Services 1299 W. Fairbanks Ave Winter Park FL 32789

Mortgage Lenders Name & Address Suntrust Bank 350 N. Lake Destiny Road, Maitland FL

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 07-6S-17-09621-211 Estimated Cost of Construction 165,900

Subdivision Name Tustenuggee Oaks Unrec Lot parcel 11 Block Unit Phase

Driving Directions From exit 414 on I-75, travel (S) on SR 441 to Right on Tomo Lites, Turn Right on Tustenuggee, left on Sassafras (R) on Honey bee, property on left - 1st lot on left

Type of Construction sitebuilt SFR Number of Existing Dwellings on Property 0

Total Acreage 10.02 Lot Size 436,150 sq ft Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 182' Side 203.99 Side 428' Rear 395'

Total Building Height 30' Number of Stories 1 Heated Floor Area 2203 Roof Pitch 7/12

Porches 316 Garage 544 Living 2203 Total 3063

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Stanton Van Conner
Owner Builder or Agent (Including Contractor)

Stanton Van Conner
Contractor Signature
Contractors License Number CRC057203
Competency Card Number ~~1556 marion~~
NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this 25 day of January 2006.

Personally known ____ or (Produced Identification) GADL

Jose M. Vidal
MY COMMISSION # DD194237 EXPIRES
March 17, 2007
BONDED THRU TROY FAIN INSURANCE, INC.

Notary Signature

NEED ORIGINAL APPLICATION

Bring in original when picking up permit

LIMITED POWER OF ATTORNEY

January 12, 2006
DATE

I hereby name and appoint Ron Ripple
of America's Home Place to be my lawful attorney
Marion Co., Levy Co. Alachua Co
in fact to act for me and apply to Sumter Co., Putnam Co., Citrus Co for
Columbia Co.
a Building permit for work to be performed
at a location desired as: Section _____ Township _____
Range _____ Lot _____ Block _____ Subdivision _____

(Address of Job) For AHP Ocala office Jobs and Locations.

(Owner of Property and Address)

And to sign my name and do all things necessary to this appointment.

Stanton Van Conner #CR-C057203
Type or Print Name of Certified Contractor, License #

Stanton Van Conner
Signature of Certified Contractor

State of Florida County: Marion
Acknowledged:

Sworn to and subscribed before me this
12th Day of January AD 2006 By Stanton Van Conner
Know personally Notary Public, State of Florida

(Seal)

My Commission Expires: 11-9-2007

Terry B Cummings
Terry B Cummings

TERRY B. CUMMINGS
Comm# DD0265747
Expires 11/9/2007
Bonded thru (800)432-4254
Florida Notary Assn., Inc

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000958

DATE 01/30/2006 PARCEL ID # 07-6S-17-09621-211

APPLICANT RON RIPPLE PHONE 352 351-4251

ADDRESS 4390 NE 34TH COURT OCALA FL 34478

OWNER FREDRICK & SALLY SCHUENEMAN PHONE

ADDRESS 182 SW HONEYBEE COURT FT. WHITE FL 32038

CONTRACTOR AMERICA HOME PLACE/STANTON VAN CONNER PHONE 352 351-4251

LOCATION OF PROPERTY 41S, TR ON TOMMY LITES, TR ON TUSTENUGGEE, TL ON SASSAFRAS,
TR ON HONEY BEE, 1ST LOT ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT TUSTENUGGEE OAKS UNR 11

SIGNATURE

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY BOARD OF COUNTY COMMISSIONERS FLORIDA

CK# 004599

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0512-22 Date Received 12/8/05 By G Permit # 958/24096

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Comments ~~Affidavit ... Plat ...~~

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Owners Name Fredrick + Sally Schueneman Phone

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Owner Builder or Agent (Including Contractor)

Contractor Signature
Contractors License Number____________
Competency Card Number____________
NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this __________ day of ________________ 20____.

Personally known_____ or Produced Identification_____

2nd time 1.17.06.

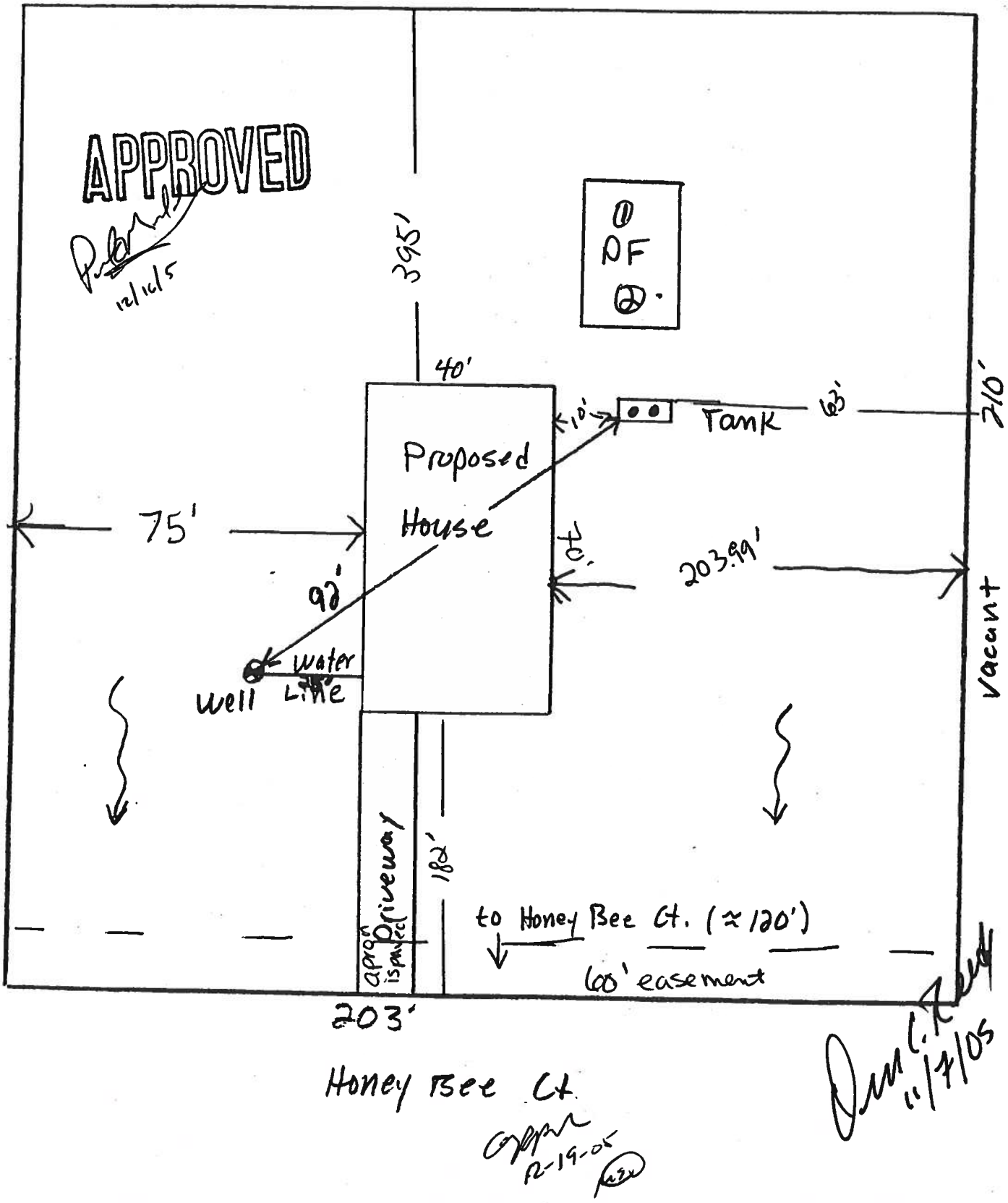
Notary Signature

Bringing original when picking up permit (NEED ORIGINAL APPLICATION)

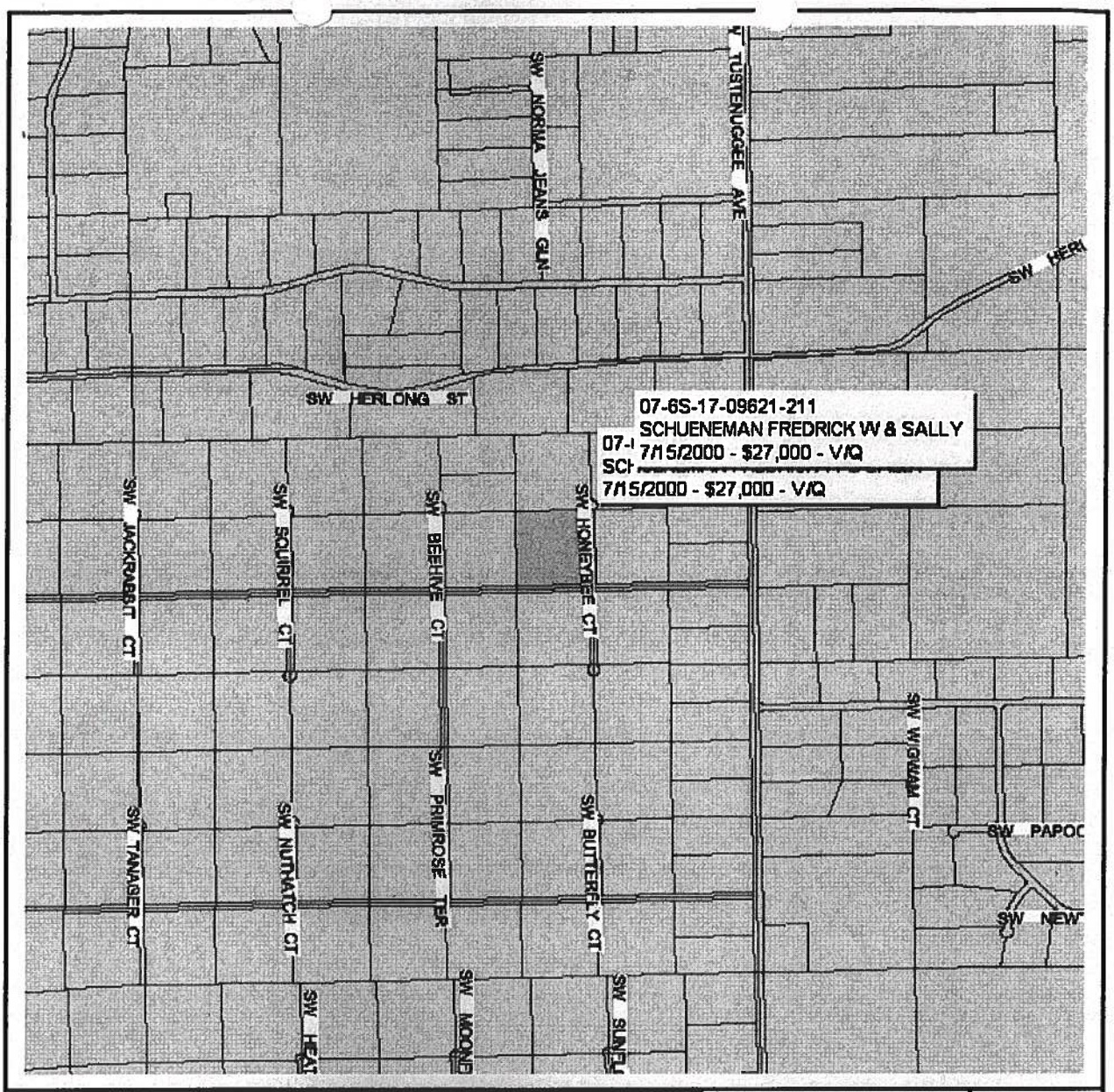
Site Plan
Inset
Fred Schueneman
Property

N
1" = 30'

DSTDS: 05-123IN



LINE OF ICHETUCKNEE ROAD, 785.95 FEET TO THE POINT OF BEGINNING, THENCE S 00°30'48"W, 1892.66 FEET; THENCE S 89°38'15" E, 2600.39 FEET TO REFERENCE POINT



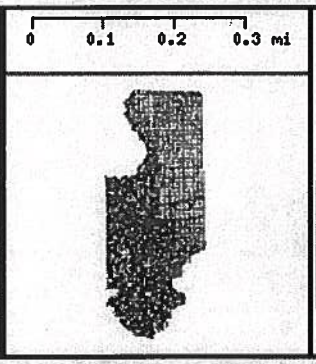
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 07-6S-17-09621-211 - NO AG ACRE (009900)

COMM NE COR, RUN W 40 FT TO W R/W CR-131, S ALONG R/W 638.44 FT, W 683.44 FT, S

Name:	SCHUENEMAN FREDRICK W & SALLY	LandVal	$32,064.00
Site:	TUSTENUGGEE OAKS UNREC	BldgVal	$0.00
Mail:	2601 15TH AVE N ST PETERSBURG, FL 33713	ApprVal	$32,064.00
		JustVal	$32,064.00
Sales Info	7/15/2000 $27,000.00 V / Q	Assd	$32,064.00
		Exmpt	$0.00
		Taxable	$32,064.00



This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE ISSUED: November 14, 2005

ENHANCED 9-1-1 ADDRESS:

182 SW HONEYBEE CT (FORT WHITE, FL 32038)

Addressed Location 911 Phone Number: NOT AVAIL.

OCCUPANT NAME: NOT AVAIL.

OCCUPANT CURRENT MAILING ADDRESS:

PROPERTY APPRAISER PARCEL NUMBER: 07-6S-17-09621-211

Other Contact Phone Number (If any):

Building Permit Number (If known):

Remarks: LOT 11 TUSTENUGGEE OAKS UNREC S/D

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

This Warranty Deed *Made and executed the* 15th *of* July *A. D.* 2000 *by*

COLUMBIA PLANTATION COMPANY

a corporation existing under the laws of STATE OF FLORIDA *, and having its principal place of business at* 8345 Ortega Blvd., Ste. #7, Jacksonville, Fl. 32210 *hereinafter called the grantor, to*

FREDRECK W. SCHUENEMAN and wife, SALLY E. SCHUENEMAN

whose postoffice address is P.O. Box 684, Biglerville, PA 17307

hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth: *That the grantor, for and in consideration of the sum of* $10.00 *and other valuable considerations, receipt whereof is hereby acknowledged, by these presents does grant, bargain, sell, alien, remise, release, convey and confirm unto the grantee, all that certain land situate in* ALACHUA *County, Florida, viz:*

SEE ATTACHED EXHIBIT "A"

Together *with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.*

To Have and to Hold, *the same in fee simple forever.*

And *the grantor hereby covenants with said grantee that it is lawfully seized of said land in fee simple; that it has good right and lawful authority to sell and convey said land; that it hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances* except taxes prior to December 31, 20

(CORPORATE SEAL)

In Witness Whereof *the grantor has caused these presents to be executed in its name, and its corporate seal to be hereunto affixed, by its proper officers thereunto duly authorized, the day and year first above written.*

ATTEST:..... Secretary

COLUMBIA PLANTATION COMPANY

Signed, sealed and delivered in the presence of:

.....

.....

By.....
LEE D. WEDEKIND, JR. President

STATE OF FLORIDA
COUNTY OF DUVAL }

I HEREBY CERTIFY that on this day, before me, an officer duly authorized in the State and County aforesaid to take acknowledgments, personally appeared LEE D. WEDEKIND, JR.

Schuremān

BK 0867 PG 819

EXHIBIT "A" OFFICIAL RECORDS

PARCEL NO. 11

Commence at the Northeast corner of Section 7, Township 6 South, Range 17 East, Columbia County, Florida and run thence S 89°54'34" W along the North line of said Section 7, 40.00 feet to the West right-of-way line of County Road No. C-131; thence S 00°20'48" W along said West right-of-way line of County Road No. C-131, 638.44 to the Northeast corner of Lot 1 of Tustenuggee Ridge, a subdivision according to plat thereof recorded in Plat Book 6, Page 212 of the Public Records of Columbia County, Florida; thence N 89°38'15" W along the North line of said Lot 1, 653.44 feet to the Northwest corner of said Lot 1; thence S 00°20'48" W along the West line of said Subdivision, 671.46 feet; thence N 89°38'15" W, 650.04 feet to the POINT OF BEGINNING; thence continue S 89°38'15" W, 650.04 feet; thence S 00°20'48" W, 671.46 feet; thence S 89°38'15" E, 650.04 feet; thence N 00°20'48" E, 671.46 feet to the POINT OF BEGINNING. Said lands being subject to an easement for ingress and egress as follows: the South 30 feet and the East 30 feet and that portion of a 50-foot radius cul-de-sac in the Northeast corner thereof. Containing 10.02 acres, more or less.

TOGETHER WITH:

60-FT. ROAD EASEMENT FOR TUSTENUGGEE OAKS

A strip of land 60 feet in width being 30 feet each side of a centerline described as follows: Commence at the Northwest corner of the NE 1/4 of Section 12, Township 6 South, Range 16 East, Columbia County, Florida and run thence S 00°03'14" E along the West line of the East ½ of said Section 12, 22.91 feet to the South line of Ichetucknee Road (a county maintained graded road); thence N 89°26'30" E along said South line of Ichetucknee Road, 361.07 feet, thence N 89°05'20" E, still along said South line of Ichetucknee Road, 785.95 feet to the POINT OF BEGINNING; thence S 00°20'48" W, 1892.66 feet; thence S 89°38' 15" E, 2600.49 feet to Reference Point "B"; thence continue S 89°38'15" E, 1300.06 feet to Reference Point "F"; thence continue S 89°38'15" E, 1300.07 feet to Reference Point "G"; thence continue S 89°38'15" E, 1353.48 feet to the West right-of-way line of County Road No. C-131 and the POINT OF TERMINATION. Also BEGIN at Reference Point "E" and run thence N 00°20'48" E, 671.46 feet to the centerpoint of a cul-de-sac having a radius of 50 feet and to the POINT OF TERMINATION. Also begin at Reference Point "B" and run thence S 00°20'48" W, 671.46 feet to the centerpoint of a cul-de-sac having a radius of 50 feet and to the POINT OF TERMINATION. Also begin at Reference Point "F" and run thence N 00°20'48" E, 671.46 feet to the centerpoint of a cul-de-sac having a radius of 50 feet and to the POINT OF TERMINATION. Also begin at Reference Point "F" and run thence S 00°20'48" W, 671.46 feet to the POINT OF TERMINATION. Also begin at Reference Point "G" and run thence N 00°20'48" E, 671.46 feet to the centerpoint of a cul-de-sac having a radius of 50 feet and to the POINT OF TERMINATION. Also begin at Reference Point "G" and run thence S 00°20'48" W, 671.46 feet to the centerpoint of a cul-de-sac having a radius of 50 feet and to the POINT OF TERMINATION. Said easement being a part of the NE 1/4 of Section 12 and the NW 1/4 and the NE 1/4 of Section 7, Township 6 South, Range 17 East.

Columbia County Tax Collector

Site Provided by... governmax.com T1.2

Tax Record

print

Account Number 1 of 1

Details
Tax Record
» Print View
Shopping Cart
License Renewal
Property Info

Searches
Account Number
Owner Name
Mailing Address

Site Functions
Welcome
Tax Search
Occupational Lic.
Contact Us
Online Help
Home

DATA VIEW AS OF: 9/26/2005 11:01:52 AM ET

Ad Valorem Taxes and Non-Ad Valorem Assessments

The information contained herein does not constitute a title search and should not be relied on as such.

Account Number	Tax Type	Tax Year
R09621-211	Real Estate	2004

Mailing Address
SCHUENEMAN FREDRICK W & SALLY

2601 N 15TH AVE
ST PETERSBURG FL 33713

Parcel ID
126497.0000

Assessed Value	Exempt Amount	Taxable Value
$25,050.00	$0.00	$25,050.00

Exemption Detail
NO EXEMPTIONS

Millage Rate
003 19.14740

Legal Description
COMM NE COR, RUN W 40 FT TO W R/W CR-131, S ALONG R/W 638.44 FT, W 683.44 FT, S 671.46 FT, W 650.04 FT FOR POB, CONT W 650.04 FT, S 671.46 FT, E 650.04 FT, N 671.46 FT TO POB. (AKA LOT 11 TUSTENUGGEE OAKS S/D UNREC) ORB 907-1614,

Tax Districts Detail

Code	Description	Exemption	Amount
C001	BOARD OF COUNTY COMMISSIONERS	$0.00	$218.59
S002	COLUMBIA COUNTY SCHOOL BOARD	$0.00	$207.72
W SR	SUWANNEE RIVER WATER MGT DIST	$0.00	$12.31
HLSH	LAKE SHORE HOSPITAL AUTH	$0.00	$37.58
IIDA	INDUSTRIAL DEVELOPEMENT AUTH	$0.00	$3.46
FFIR	FIRE ASSESSMENTS	$0.00	$5.22

Total Gross	$484.88
Discount	($19.40)
Total	$465.48

If Paid By	Amount Due
	$0.00

Date Paid	Transaction	Receipt	Amount Paid
11/16/2004	PAYMENT	2300384.0001	$465.48

Prior Year Taxes Due

NO DELINQUENT TAXES

Application for Culvert Permit
Columbia County, Florida

DATE ____________ 20______

TO BOARD OF COUNTY COMMISSIONERS:

Building Permit #____________

Application is hereby made to install one or more culverts on the property owned by

Fredrick & Sally Schueneman
Name of Taxpayer

located outside of any incorporated municipality in said County and described on the Tax Rolls as follows:

SECTION: 7 TOWNSHIP: 16S RANGE: 17

(List tax roll description of property)

(INSTALLER IS TO CONTACT BUILDING INSPECTOR'S OFFICE FOR FINAL INSPECTION)

758-1124

758-1008

______ / ______ Culvert Size Plain/Coated	[signature] Applicant
____________ Culvert Inspector / Inspection Date	P.O. Box 5925 Address: Street, R. R. or P. O. Box
____________ Date of Final Inspection	Ocala FL 34478 City, State, Zip Code

BOARD OF COUNTY COMMISSIONERS
COLUMBIA COUNTY, FLORIDA

PLEASED BE ADVISED
Applicant must notify any appropriate utility company before digging or placement of culvert

____
FEE

____
Building Department

STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION

CONSTRUCTION INDUSTRY LICENSING BOARD (850) 487-1395
1940 NORTH MONROE STREET
TALLAHASSEE FL 32399-0783

CONNER, STANTON VAN
AMERICA'S HOME PLACE INC
PO BX 1316
GAINESVILLE GA 30501

RECEIVED
JUL 19 2004
AMERICA'S HOME PLACE

STATE OF FLORIDA AC# 1479900
DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION

CRC057203 07/08/04 040019767

CERTIFIED RESIDENTIAL CONTRACTOR
CONNER, STANTON VAN
AMERICA'S HOME PLACE INC

IS CERTIFIED under the provisions of Ch.489 FS
Expiration date: AUG 31, 2006 L04070801429

DETACH HERE

AC# 1479905

STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD SEQ# L04070801429

DATE	BATCH NUMBER	LICENSE NBR
07/08/2004	040019767	CRC057203

The RESIDENTIAL CONTRACTOR
Named below IS CERTIFIED
Under the provisions of Chapter 489 FS.
Expiration date: AUG 31, 2006

CONNER, STANTON VAN
AMERICA'S HOME PLACE INC
2144 HILTON DRIVE
GAINSVILLE GA 30501

JEB BUSH
GOVERNOR

DISPLAY AS REQUIRED BY LAW

DIANE CARR
SECRETARY

Marion County Building Department
2631 S.E. 3rd Street
Ocala, Florida 34471-9101
(352) 620-7422

Limited Power of Attorney

I, Stanton Van Conner, hereby appoint ~~Tate~~ Lee Meadors, Janine Nance,
(contractor name) (appointee name)
to be my lawful attorney-in-fact to act for me and apply to the ~~Marion~~ County Building Department for a permit to perform construction, at a location described as: Columbia

Section 7 Township 16S Range 17

Lot _____ Block _____ Subdivision 09621-211

Job Address: 1825 SW Honeybee Ct.

Job Description: SFR

Property Owner: Fred + Sally Scheuneman

and to sign my name, and do all things necessary to this appointment.

Contractor: Stanton Van Conner
printed name

Signature: _____ Date: 11/7/05
contractor signature

Contractor License #: CRC057203

State of Florida)
County of Marion

Sworn to and subscribed before me this 7th day of November, 2005
by Stanton Van Conner (name of person acknowledged) who is
personally ~~known to me~~ or who has produced GA License (identification).

_____
Notary Public
Commission expires: 3/17/07

(seal)

Jose M. Vidal
MY COMMISSION # DD194237 EXPIRES
March 17, 2007
BONDED THRU TROY FAIN INSURANCE, INC.

P-1202 01/03

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**ahp-schue-oxford**	Builder:	
Address:		Permitting Office:	**columbia**
City, State:	,	Permit Number:	24096
Owner:		Jurisdiction Number:	221000
Climate Zone:	**North**		

1.	New construction or existing		New	
2.	Single family or multi-family		Single family	
3.	Number of units, if multi-family		1	
4.	Number of Bedrooms		1	
5.	Is this a worst case?		Yes	
6.	Conditioned floor area (ft²)		2203 ft²	
7.	Glass area & type	Single Pane	Double Pane	
a.	Clear glass, default U-factor	0.0 ft²	170.0 ft²	
b.	Default tint	0.0 ft²	0.0 ft²	
c.	Labeled U or SHGC	0.0 ft²	0.0 ft²	
8.	Floor types			
a.	Slab-On-Grade Edge Insulation		R=0.0, 201.0(p) ft	
b.	N/A			
c.	N/A			
9.	Wall types			
a.	Concrete, Int Insul, Exterior		R=5.0, 1375.0 ft²	
b.	N/A			
c.	N/A			
d.	N/A			
e.	N/A			
10.	Ceiling types			
a.	Under Attic		R=30.0, 2203.0 ft²	
b.	N/A			
c.	N/A			
11.	Ducts			
a.	Sup: Unc. Ret: Unc. AH: Garage		Sup. R=6.0, 15.0 ft	
b.	N/A			

12.	Cooling systems		
a.	Central Unit	Cap: 42.0 kBtu/hr	
		SEER: 10.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 42.0 kBtu/hr	
		HSPF: 7.30	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 50.0 gallons	
		EF: 0.90	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.08

Total as-built points: 24745

Total base points: 25334

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 1-5-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: 12/13/05

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

BUILDING OFFICIAL: _____

DATE: _____

EnergyGauge® (Version: FLRCSB v3.30)

Schuenaman

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.4

The higher the score, the more efficient the home.

1.	New construction or existing		New		12.	Cooling systems
2.	Single family or multi-family		Single family		a.	Central Unit
3.	Number of units, if multi-family		1			
4.	Number of Bedrooms		1		b.	N/A
5.	Is this a worst case?		Yes			
6.	Conditioned floor area (ft²)		2203 ft²		c.	N/A
7.	Glass area & type	Single Pane	Double Pane			
a.	Clear - single pane	0.0 ft²	170.0 ft²		13.	Heating systems
b.	Clear - double pane	0.0 ft²	0.0 ft²		a.	Electric Heat Pump
c.	Tint/other SHGC - single pane	0.0 ft²	0.0 ft²			
d.	Tint/other SHGC - double pane				b.	N/A
8.	Floor types					
a.	Slab-On-Grade Edge Insulation		R=0.0, 201.0(p) ft		c.	N/A
b.	N/A					
c.	N/A				14.	Hot water systems
9.	Wall types				a.	Electric Resistance
a.	Concrete, Int Insul, Exterior		R=5.0, 1375.0 ft²			
b.	N/A				b.	N/A
c.	N/A					
d.	N/A				c.	Conservation credits
e.	N/A					(HR-Heat recovery, Solar
10.	Ceiling types					DHP-Dedicated heat pump)
a.	Under Attic		R=30.0, 2203.0 ft²		15.	HVAC credits
b.	N/A					(CF-Ceiling fan, CV-Cross ventilation,
c.	N/A					HF-Whole house fan,
11.	Ducts					PT-Programmable Thermostat,
a.	Sup: Unc. Ret: Unc. AH: Garage		Sup. R=6.0, 15.0 ft			MZ-C-Multizone cooling,
b.	N/A					MZ-H-Multizone heating)

	Cap: 42.0 kBtu/hr
	SEER: 10.00
	Cap: 42.0 kBtu/hr
	HSPF: 7.30
	Cap: 50.0 gallons
	EF: 0.90

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar℠ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

EnergyGauge® (Version: FLRCSB v3.30)

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	2203.0	12.74	5051.9	Double, Clear	N	1.0	7.0	15.0	24.58	1.00	368.7
				Double, Clear	W	1.0	7.0	15.0	20.73	1.00	312.3
				Double, Clear	E	1.0	7.0	15.0	18.79	1.01	285.0
				Double, Clear	W	1.0	7.0	45.0	20.73	1.00	936.9
				Double, Clear	E	1.0	7.0	30.0	18.79	1.01	570.0
				Double, Clear	S	1.0	7.0	10.0	13.30	1.01	133.8
				Double, Clear	E	1.0	7.0	40.0	18.79	1.01	760.0
				As-Built Total:				**170.0**			**3366.6**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	0.0	0.00	0.0	Concrete, Int Insul, Exterior			5.0	1375.0	5.70		7837.5
Exterior	1375.0	3.70	5087.5								
Base Total:	**1375.0**		**5087.5**	**As-Built Total:**				**1375.0**			**7837.5**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	0.0	0.00	0.0	Exterior Insulated				63.0	8.40		529.2
Exterior	63.0	12.30	774.9								
Base Total:	**63.0**		**774.9**	**As-Built Total:**				**63.0**			**529.2**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X WCM =		Points
Under Attic	2203.0	2.05	4516.1	Under Attic			30.0	2203.0	2.05 X 1.00		4516.1
Base Total:	**2203.0**		**4516.1**	**As-Built Total:**				**2203.0**			**4516.1**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	201.0(p)	8.9	1788.9	Slab-On-Grade Edge Insulation			0.0	201.0(p	18.80		3778.8
Raised	0.0	0.00	0.0								
Base Total:			**1788.9**	**As-Built Total:**				**201.0**			**3778.8**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	2203.0	-0.59	-1299.8					2203.0	-0.59		-1299.8

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**15919.6**	**Winter As-Built Points:**					**18728.5**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
15919.6	**0.6274**	**9988.0**	18728.5 **18728.5**	1.000 **1.00**	(1.069 x 1.169 x 1.00) **1.250**	0.467 **0.467**	1.000 **1.000**	10932.7 **10932.7**

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	2203.0	20.04	7946.7	Double, Clear	N	1.0	7.0	15.0	19.20	0.99	283.8
				Double, Clear	W	1.0	7.0	15.0	38.52	0.98	568.7
				Double, Clear	E	1.0	7.0	15.0	42.06	0.98	620.5
				Double, Clear	W	1.0	7.0	45.0	38.52	0.98	1706.0
				Double, Clear	E	1.0	7.0	30.0	42.06	0.98	1240.9
				Double, Clear	S	1.0	7.0	10.0	35.87	0.97	347.8
				Double, Clear	E	1.0	7.0	40.0	42.06	0.98	1654.6
				As-Built Total:				**170.0**			**6422.3**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	0.0	0.00	0.0	Concrete, Int Insul, Exterior			5.0	1375.0	1.00		1375.0
Exterior	1375.0	1.70	2337.5								
Base Total:	**1375.0**		**2337.5**	**As-Built Total:**				**1375.0**			**1375.0**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	0.0	0.00	0.0	Exterior Insulated				63.0	4.10		258.3
Exterior	63.0	6.10	384.3								
Base Total:	**63.0**		**384.3**	**As-Built Total:**				**63.0**			**258.3**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	2203.0	1.73	3811.2	Under Attic			30.0	2203.0	1.73 X 1.00		3811.2
Base Total:	**2203.0**		**3811.2**	**As-Built Total:**				**2203.0**			**3811.2**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	201.0(p)	-37.0	-7437.0	Slab-On-Grade Edge Insulation			0.0	201.0(p	-41.20		-8281.2
Raised	0.0	0.00	0.0								
Base Total:			**-7437.0**	**As-Built Total:**				**201.0**			**-8281.2**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	2203.0	10.21	22492.6					2203.0	10.21		22492.6

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT						
Summer Base Points:		**29535.3**	**Summer As-Built Points:**						**26078.2**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
29535.3	**0.4266**	**12599.8**	26078.2 **26078.2**	1.000 **1.00**	(1.090 x 1.147 x 1.00) **1.250**	0.341 **0.341**	1.000 **1.000**	11127.7 **11127.7**

HOMETEAM
PEST DEFENSE®

Certificate of

New Construction Subterranean Termite Treatment

This report is submitted for information purposes to the builder on (new) construction cases where treatment for prevention of subterranean termite infestation is required by the Florida Building Code, Section 104.2.6

All contracts for service are between the Pest Control Operator and builder, unless stated otherwise.

Section I: Hometeam Pest Defense
Company Address: 1621 NE 6th Ave City: Ocala State: FL
Zip: 34470 Company Phone No.: 368-3845 Business License No. JB1537

Section 2: Builder Information
Company Name: America's Home Place
Phone No.: _____

Section 3: Property Information
Building Permit No.: 00024096 24096
Location of Structure(s) Treated: 172 SW Honeybee CT H. White, FL

Type of Construction: [X] Slab [ ] Basement [ ] Crawl [ ] Other Stemwall w/ chairblock
Approximate Depth of Footing: Outside:_____ Inside:_____ Type Fill:_____

Section 4: Treatment Information
Date(s) of Treatment(s): 3-6-06 EPA Registration No: 100-1006
Brand Name of Product(s) Used: Probuild TC
Final Mix Solution: .5 Treatment Area Sq. Ft.: 3063.
Linear Ft: 252.7 Linear Ft. of Masonry Voids: _____.
Total Gallons of Termiticide Applied: 122.5

Service Agreement Available	[ ] Yes	[ ] No
Liquid treatment:	[ ] Yes	[ ] No
Liquid Final exterior treatment	[ ] Yes	[ ] No
Borate treatment:	[ ] Yes	[ ] No
Bait in lieu of Pretreat:	[ ] Yes	[ ] No

This building has received a complete treatment for the prevention of subterranean termites. Treatment is in accordance with rules and laws established by the Florida Department of Agriculture and Consumer Services. Initial _____

Note: State law requires service agreement to be issued. This form does not preempt State law.
Attachments (list): _____

Comments: _____

Name of Applicator(s): Shawn Howell Certificate No.: JE 89586

Authorized Signature: _____ Date: 3-6-06

RH-NT9/05

CERTIFICATE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 07-6S-17-09621-211 **Building permit No.** 000024096

Use Classification SFD,UTILITY **Fire:** 16.52

Permit Holder STANTON VAN CONNER/AM.HOME PL **Waste:** 24.50

Owner of Building FREDRICK & SALLY SCHUENEMAN **Total:** 41.02

Location: 182 SW HONEYBEE COURT, FT. WHITE, FL

Date: 08/03/2006

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DIPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. _____
DATE PAID: _____
FEE PAID: _____
RECEIPT #: _____

APPLICATION FOR:
New System [] Existing System [] Holding Tank [] Innovative
Repair [] Abandonment [] Temporary [] _____

APPLICANT: Fred Schueneman (c/o America's Home Place)

AGENT: Janine Nance TELEPHONE: 352/351-4251

MAILING ADDRESS: 4390 NE 34th Ct., Ocala, FL 34479

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. ATTACH BUILDING PLAN AND TO-SCALE SITE PLAN SHOWING PERTINENT FEATURES REQUIRED BY CHAPTER 10D-6, FLORIDA ADMINISTRATIVE CODE.

Approved 12/8/05

PROPERTY INFORMATION [IF LOT IS NOT IN A SUBDIVISION, ATTACH LEGAL DESCRIPTION OR DEED.

LOT: NA BLOCK: NA SUBDIVISION: NA DATE OF SUBDIVISION: Post-1972

PROPERTY ID #: 07-6S-17-09621-211 [Section/Township/Range/Parcel No.] ZONING: Ag

PROPERTY SIZE: ± 10.0 ACRES [Sqft/43560] PROPERTY WATER SUPPLY: [X] PRIVATE [] PUBLIC

PROPERTY STREET ADDRESS: Honey Bee Ct., Ft. White, FL

DIRECTIONS TO PROPERTY: From Exit 414 on I-75, travel South on SR 441 to Right on Tommy Lites, turn Right on Tustenuggee, Left on Sassafras, Right on Honey Bee, property on Left.

BUILDING INFORMATION [X] RESIDENTIAL [ ] COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	# Persons Served	Business Activity For Commercial Only
1	Single Family	3	2,081	2	
2					
3					
4					

[ ] Floor/Equipment Drains [ ] Spas/Hot Tubs [ ] Floor/Equipment Drains
[ ] Ultra-low Volume Flush Toilets [ ] Other (Specify) _____

SIGNATURE: Janine Nance DATE: 11/7/05

RIGHT-J SHORT FORM
Entire House
SUPERIOR A/C & HTG INC.

PO BOX 4491, OCALA, FL 34478 Phone: 352-237-5535 Fax: 352-237-5535

Project Information

For: AMERICAS HOME PLACE
OCALA, FL

Design Information

	Htg	Clg
Outside db (°F)	34	93
Inside db (°F)	70	75
Design TD (°F)	36	18
Daily range	-	M
Inside humidity (%)	-	50
Moisture difference (gr/lb)	-	50

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

HEATING EQUIPMENT

Make Ruud
Trade Ruud UPKA Series
UPKA-043JA

Efficiency	7.3 HSPF	
Heating input		
Heating output	41500	Btuh @ 47°F
Heating temperature rise	29	°F
Actual heating fan	1300	cfm
Heating air flow factor	0.042	cfm/Btuh

Space thermostat

COOLING EQUIPMENT

Make Ruud
Trade Ruud UPKA Series
UPKA-043JA
UBHC-21+RCBA-4882

Efficiency	10.0 SEER	
Sensible cooling	27300	Btuh
Latent cooling	11700	Btuh
Total cooling	39000	Btuh
Actual cooling fan	1300	cfm
Cooling air flow factor	0.055	cfm/Btuh
Load sensible heat ratio	78	%

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
BED 3	210	3518	2537	149	140
HALL BATH	66	972	318	41	17
BED 2	224	3832	1822	162	100
FAMILY	380	5204	3419	220	188
LIV-FOY-DIN	434	6609	4820	279	265
MASTER BED	300	3275	2571	138	141
WIC	55	1255	374	53	21
MASTER BATH	165	1907	1394	81	77
UTILITY	77	170	1456	7	80
HALL	28	-0	0	-0	0
BREAKFAST	96	3809	2012	161	111
KITCHEN	180	225	2914	9	160

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

Entire House Ventilation air Equip. @ 0.98 RSM Latent cooling	d	2215	30776 0	23636 0 23163 6770	1300	1300
TOTALS		2215	30776	29933	1300	1300

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

DUCT SYSTEM SUMMARY
Entire House
SUPERIOR A/C & HTG INC.

PO BOX 4491, OCALA, FL 34478 Phone: 352-237-5535 Fax: 352-237-5535

Project Information

For: AMERICAS HOME PLACE
OCALA, FL

	HEATING	COOLING
External Static Pressure:	0.65 in H2O	0.65 in H2O
Pressure Losses:	0.31 in H2O	0.31 in H2O
Available Static Pressure:	0.34 in H2O	0.34 in H2O
Friction Rate:	0.374 in/100ft	0.374 in/100ft
Actual AVF:	1300 cfm	1300 cfm

Total Effective Length (TEL): 91 ft

Supply Branch Detail Table

Name	Htg (Btuh)	Clg (Btuh)	Htg (cfm)	Clg (cfm)	Dsn FR	Vel (fpm)	Dia (in)	Rect Sz (in)	Duct Matl	Trnk
UTILITY	170	1456	7	80	2.781	587	5	0x 0	VlFx	st1
BREAKFAST	3809	2012	161	111	0.582	602	7	0x 0	VlFx	st2
MASTER BED	3275	2571	138	141	0.715	529	7	0x 0	VlFx	st1
LIV-FOY-DIN	3305	2410	140	133	1.088	522	7	0x 0	VlFx	st2
WIC	1255	374	53	21	0.481	608	4	0x 0	VlFx	st1
FAMILY	5204	3419	220	188	0.533	630	8	0x 0	VlFx	st2
LIV-FOY-DIN-A	3305	2410	140	133	0.611	522	7	0x 0	VlFx	st2
BED 3	3518	2537	149	140	0.374	556	7	0x 0	VlFx	st3
BED 2	3832	1822	162	100	0.556	606	7	0x 0	VlFx	st3
MASTER BATH	1907	1394	81	77	0.611	591	5	0x 0	VlFx	st1
HALL BATH	972	318	41	17	0.417	471	4	0x 0	VlFx	st3
HALL	-0	0	-0	0	0.000	0	0	0x 0	VlFx	st1
KITCHEN	225	2914	9	160	0.782	600	7	0x 0	VlFx	st2

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Vel (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	279	319	584	10	0 x 0	VinlFlx	sp
st2	Peak AVF	1021	981	648	17	0 x 0	VinlFlx	sp
st3	Peak AVF	352	257	645	10	0 x 0	VinlFlx	st2
sp	Peak AVF	1300	1300	660	19	0 x 0	VinlFlx	

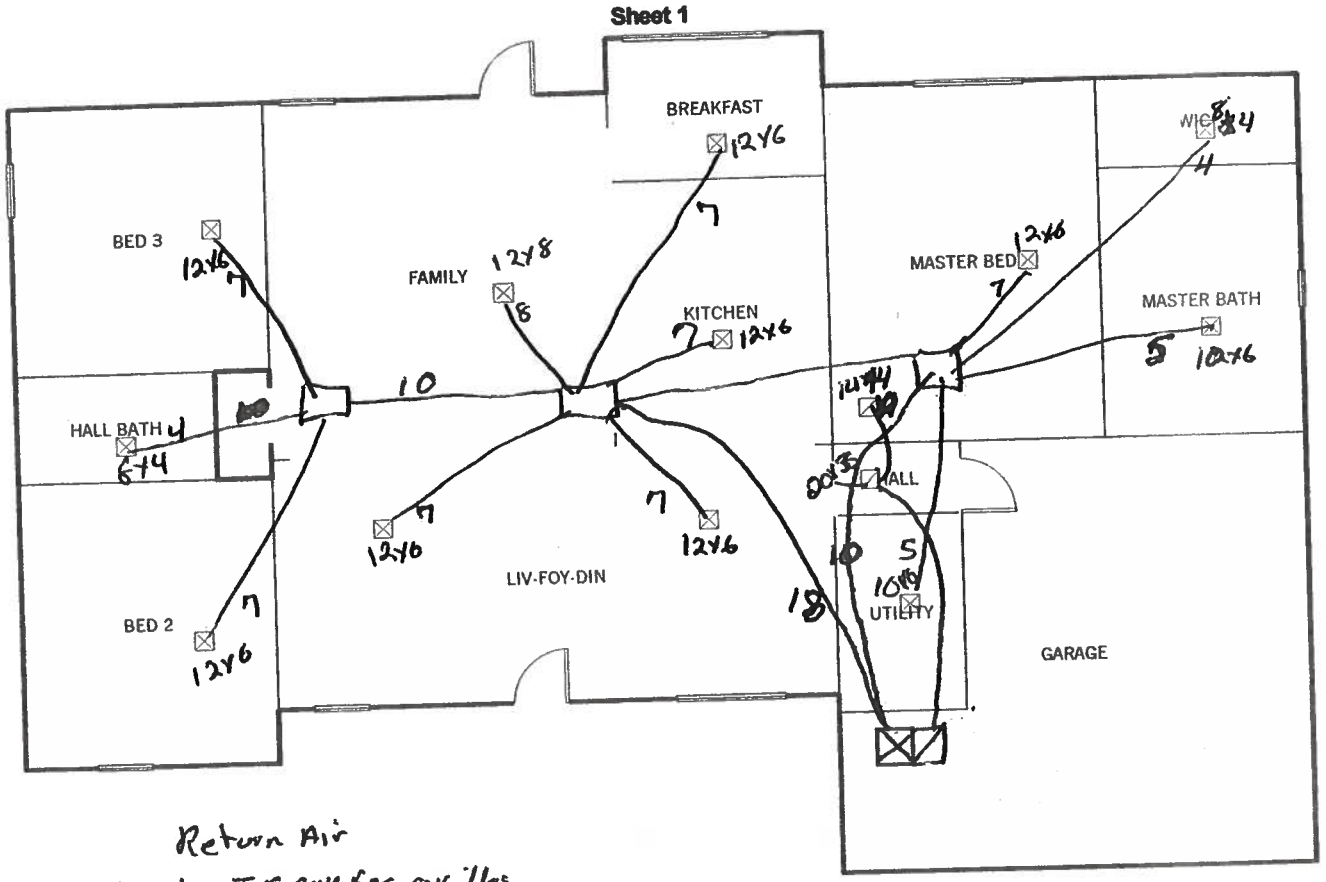
Return Branch Detail Table

Name	Diffus Sz (in)	Design AVF (cfm)	Design (in H2O)	Design FR	Vel (fpm)	Dia (in)	Rect Sz (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb2	14 x 14	319	0.09	0.374	584	10	0 x 0		VlFx	rb1
rb1	22 x 29	1021	0.09	0.374	648	17	0 x 0		VlFx	rt1

Return Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Vel (fpm)	Diam (in)	Rect Duct Sz (in)	Duct Material	Trunk
rt1	Peak AVF	1021	981	648	17	0 x 0	VinlFlx	
rb1	Peak AVF	279	319	584	10	0 x 0	VinlFlx	

Sheet 1



Return Air
14x6 Transfer grilles
over Bed 2, 3 Doors.
14x14 RA in Master Bed
20x30 RA in Hall

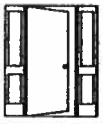
Job #: Performed by for: AMERICAS HOME PLACE OCALA, FL	SUPERIOR A/C & HTG INC. PO BOX 4491 OCALA, FL 34478 Phone: 352-237-5535 Fax: 352-237-5535	Scale: 1 : 127 Page 1 Right-Suite Residential (tm) 5.5.11 RSR26807 2005-Sep-25 10:17:45 uments\Wrightsoft HVAC\ahp-schue

Installation Instructions Pre-Hung Door Systems In High Wind Velocity Areas

These instructions apply to all Therma-Tru wood-framed door systems. Some apply specifically to:

Inswing Doors 

Outswing Doors 

Doors with sidelights 

Double Doors 

Patio Doors 

Read all instructions before starting.

THERMA-TRU® DOORS

The Most Preferred Brand in the Business™

P.O. Box 8780 Maumee, Ohio 43537

Printed 3/02 Questions? Comments? Call 1-800-THERMATRU (1-800-843-7628) Part No. MSDRISTH

KEY DIFFERENCES CHECKLIST

Rough openings are 1/4" smaller on each side and 1/4" smaller on the header than standard Therma-Tru rough opening dimensions.

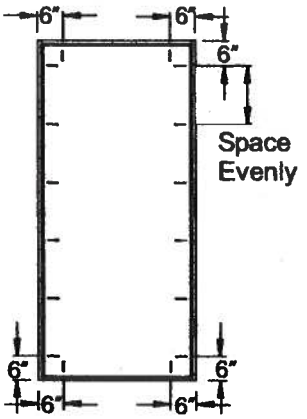
Use only a maximum of 1/4" thick shims.

Predrill the jambs and headers using 1/8" drill bit. Predrilling increases structural performance.

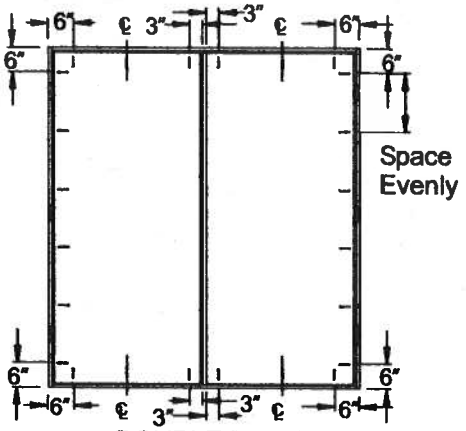
Use only 2-1/2" wood screws (#8 and #10) when securing the pre-hung frame to the rough opening buck. Drywall screws, deck screws, and nails are insufficient anchors, and should not be substituted for wood screws.

Be certain of the number and placement of all screws securing the frame to the rough opening buck. Consult the architect or engineer of record concerning fastening the buck to the rest of the building structure.

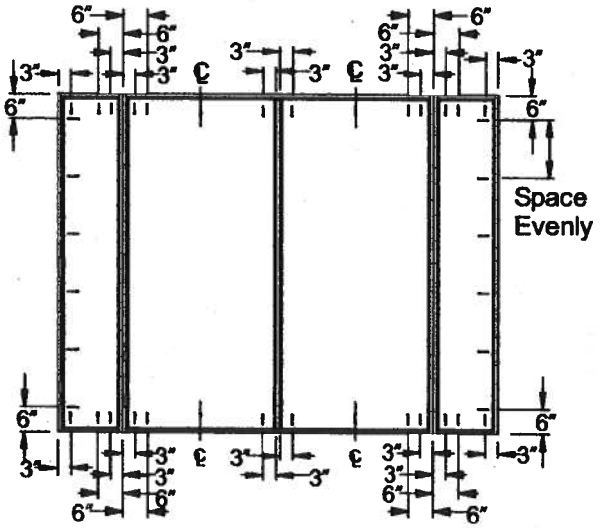
For structural strength, the rough opening buck must be either 2" x 4" or 2" x 6" construction, not "1 by" construction.



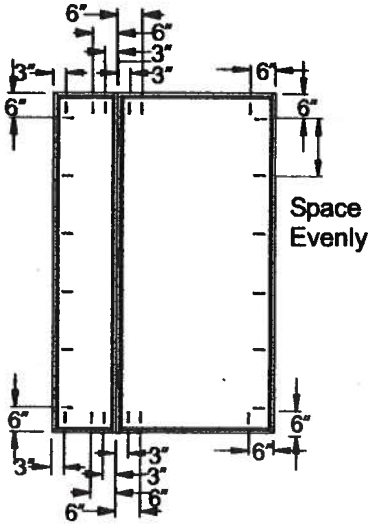
SINGLE DOOR



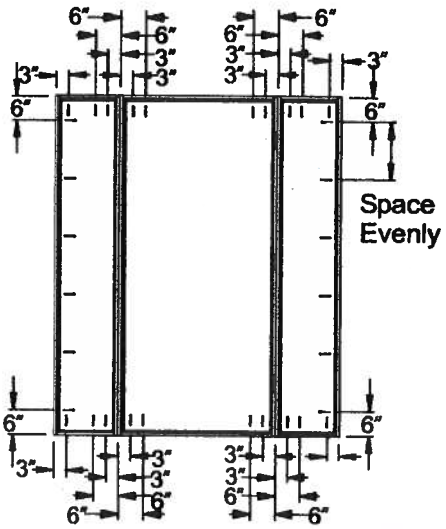
DOUBLE DOOR



DOUBLE DOOR W/SIDELITES



SINGLE DOOR W/SIDELITE



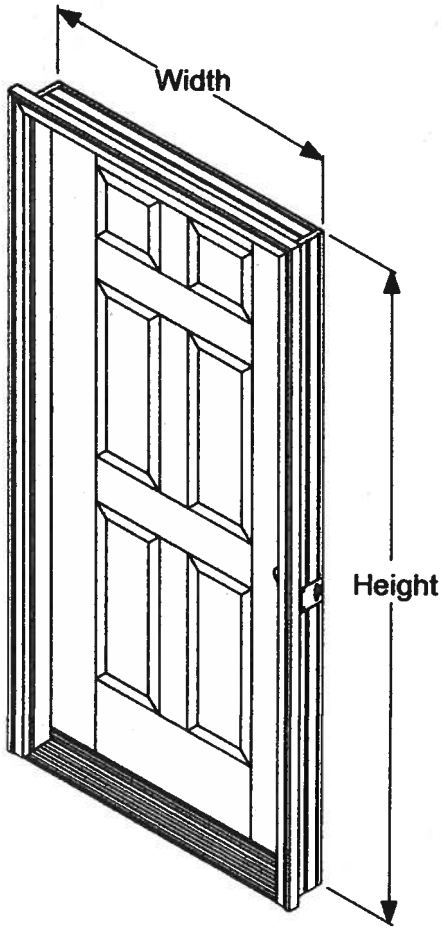
SINGLE DOOR W/SIDELITES

1 CHECK DOOR UNIT.

Check width and height.

Measure size of frame (width and height), not brickmold.

Remove cleats and packaging, but keep door fastened closed with transport clip. Do not remove clip or open door until instructed to do so.



2 CHECK AND PREPARE OPENING.

Is subfloor level and solid? *Provide a flat, level, clean bearing surface so the sill may be caulked and sealed to the opening. Scrape, sand, or fill as required.*

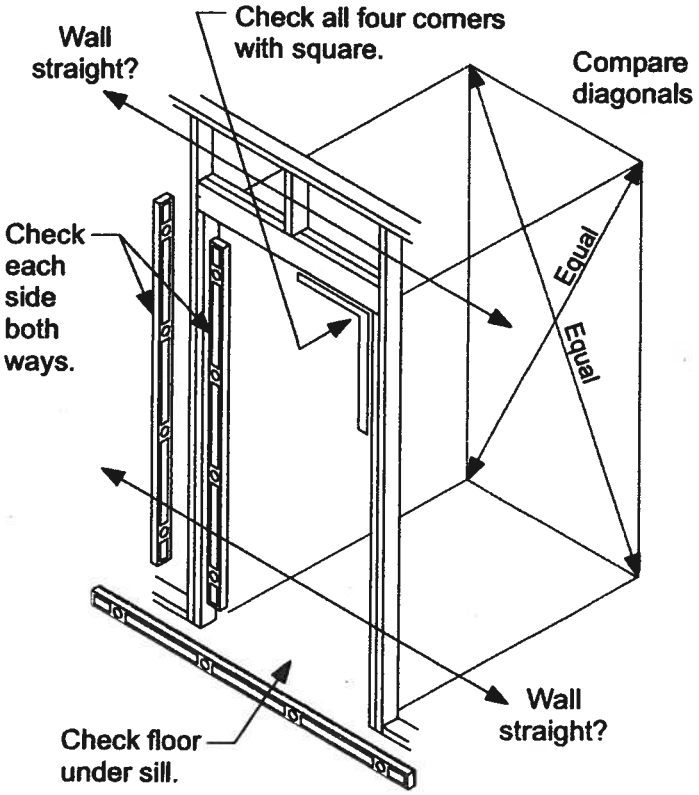
Shim subfloor for floor covering clearance, if required. *If shimming, caulk under shims.*

Is opening square? Check all corners with a framing square. Double-check by comparing diagonal measurements. *Fix any problems now.*

Are framing and walls plumb? Use a 6-foot level and check both sides of opening, both ways. *Fix any problems now.*

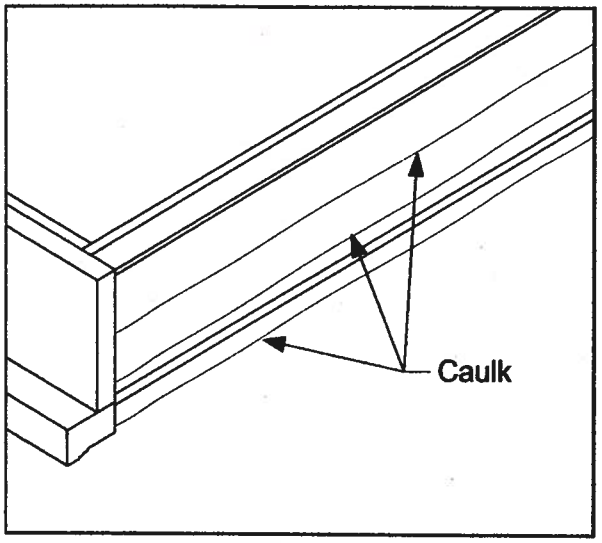
Are the wall surfaces around the opening in the same plane? There must be no "warps" or "jogs". *Fix any problems now.*

Is the opening the correct size? Check it against the door frame size now, before installation. Opening should be frame height plus **1/4"**, and frame width plus **1/2"**. Remember to use only 1/4" shims.



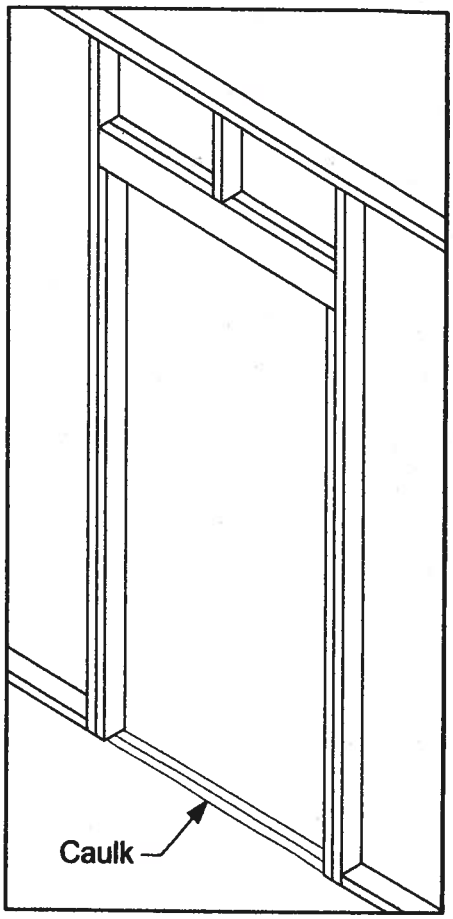
3 CAULK SUBFLOOR AND SILL BOTTOM.

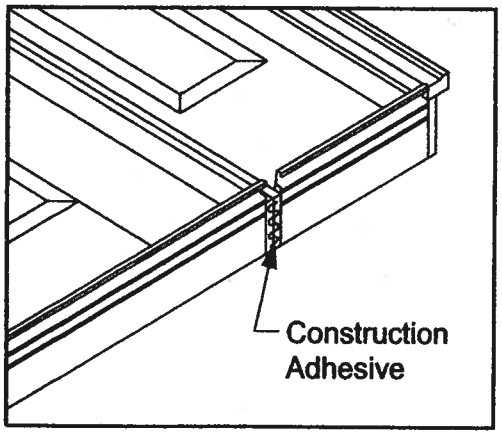
Lay door unit on edge or face so that bottom side of sill can be caulked. Place very large beads of caulk across full width at front edge of sill. Place one or more very large caulk beads at parallel lines across bottom surfaces which will bear on subfloor.



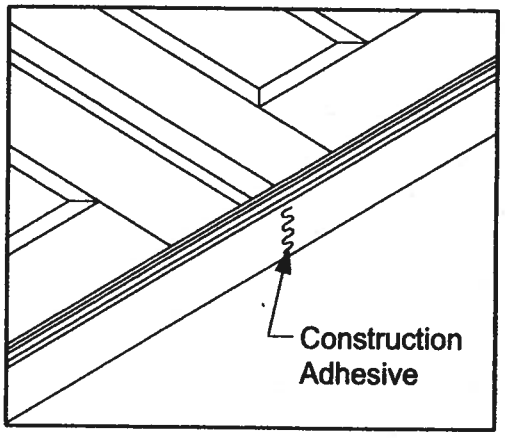
Avoid callbacks due to leaks! Use an entire tube of caulking to seal between sill and subfloor.

On subfloor at opening, place very large beads of caulk that will match with those placed on sill. Run beads full width of opening.





NOTE: If installing **units with sidelights,** place construction adhesive under points where door jambs will bear on floor. Complete installation before adhesive cures.



NOTE: If installing **double door** or **patio units,** place construction adhesive under mullion or point where doors meet at center. Complete installation before adhesive cures.

4 PLACE UNIT IN OPENING AND TEMPORARILY FASTEN HINGE JAMB. DO NOT FASTEN THROUGH BRICKMOULD.

If the jamb and head does not come with pilot holes, drill 1/8" pilot holes before using screws.

For single or double doors, note hinge locations and mark jamb faces near door surface, for fastener placement later.

Lift unit up. With top edge tilted away from opening, center unit and place sill down onto caulk beads. Tilt into place.

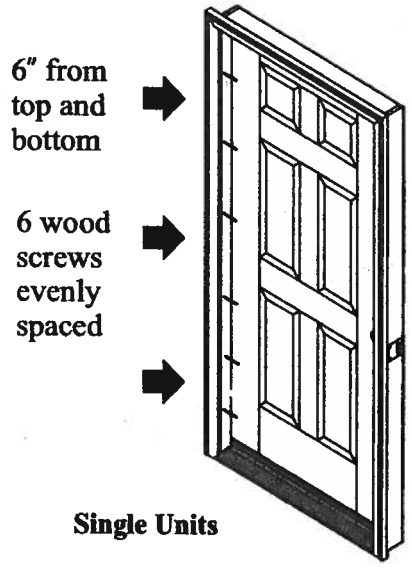
Work from the side of the door that is weather-stripped. (*outside* for **inswing** doors, *Inside* for **outswing** doors)

Plumb hinge side jamb both ways. Use a 6-foot level.

Use shims totaling a maximum of 1/4" thickness, not the usual 1/2".

Use 2-1/2" wood screws. Do NOT substitute nails, deck screws, or drywall screws . Place **six** #8 wood screws through jambs into "two-by" studs, at each location where shown in diagrams. **For single or double doors,** refer to marks on jambs and place fasteners *below* each hinge location, so that shims may be placed behind hinges. Fasteners will keep shims from falling down while adjustments are made.

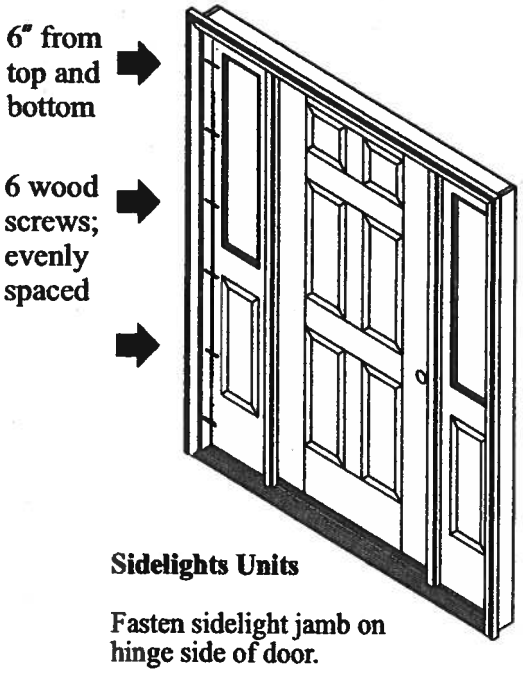
Do not drive screws completely in at this time.



Single Units

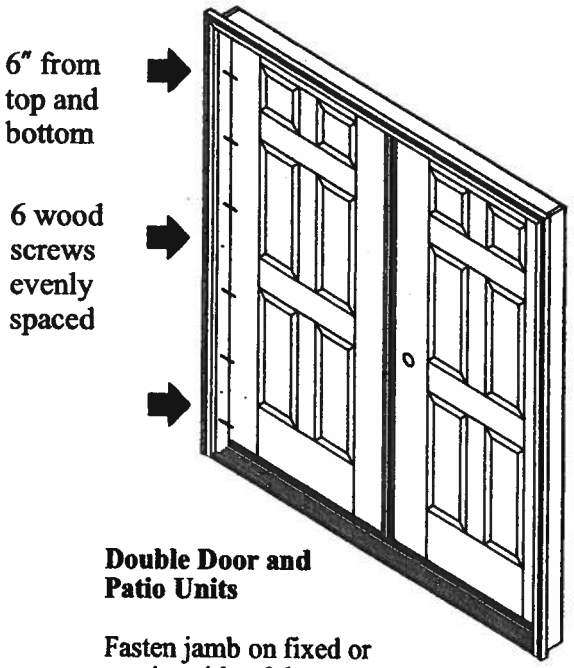
Fasten hinge jamb.

All screws used outside should be coated or galvanized to prevent rusting and staining.



Sidelights Units

Fasten sidelight jamb on hinge side of door.



Double Door and Patio Units

Fasten jamb on fixed or passive side of door.

5 SHIM BEHIND HINGES AND SECURE HINGE JAMB.

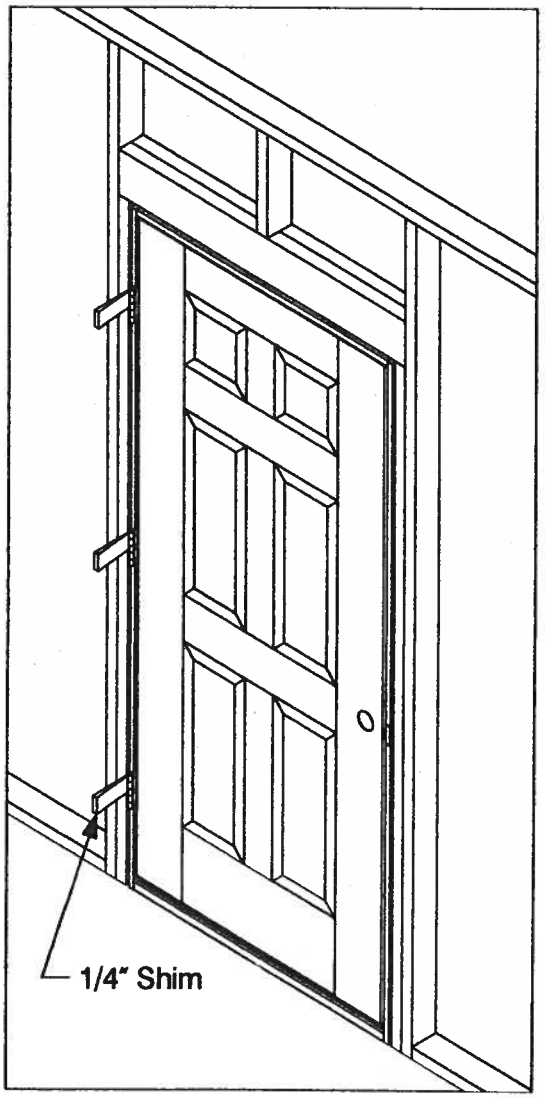
Go through another opening to work on swing side of door.

Leave door fastened closed with transport clip.

Shim above fasteners, behind each hinge, between jamb and opening. Shim a total of 1/4" maximum per side.

Recheck hinge jamb to ensure it is plumb and straight. Use a 6-foot level.

Go back through another opening to other side of door to secure hinge jamb. Finish driving the wood screws.



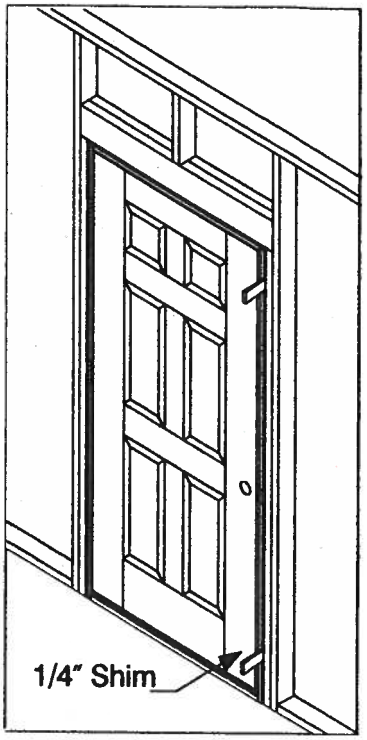
6 ADJUST REST OF FRAME AND FASTEN.

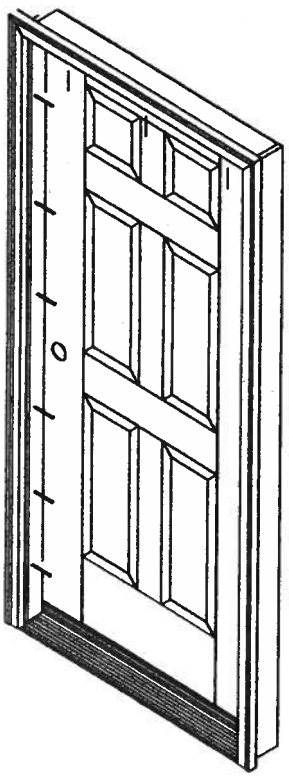
From swing side of door, shim near top and bottom of unfastened jamb.

Make frame adjustments so margins between door and frame are all even.

Adjust frame so frame face is flush with door face all around.

NOTE: For **double door units,** make adjustments that affect alignment and margins and weatherstrip contact *between doors.*





From weatherstrip side of door, check weatherstrip margins and contact.

Adjust frame as required so contact and margins are equal all around door.

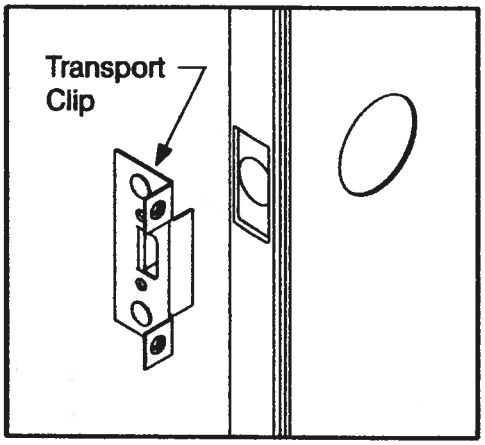
Evenly space six screws per side. Review the figures on page 3.

For single doors, on the head, use **three screws** for each door: 6" from each end and at the center of the door.

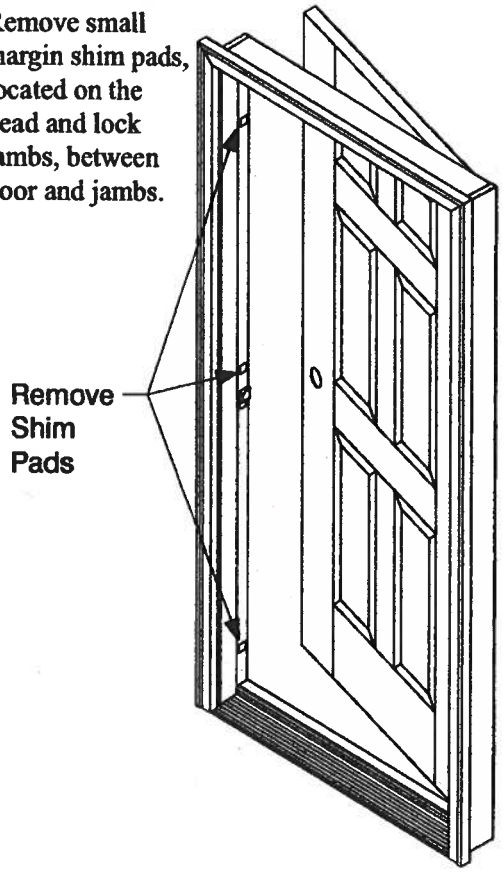
When a sidelite is present, add an additional screw 3" from the end of each door and use three screws for each sidelite head: 3" from each end and 6" from door-side end. Finish driving the

7 UNCLIP AND OPEN DOOR. INSTALL HARDWARE. COMPLETE JAMB FASTENING.

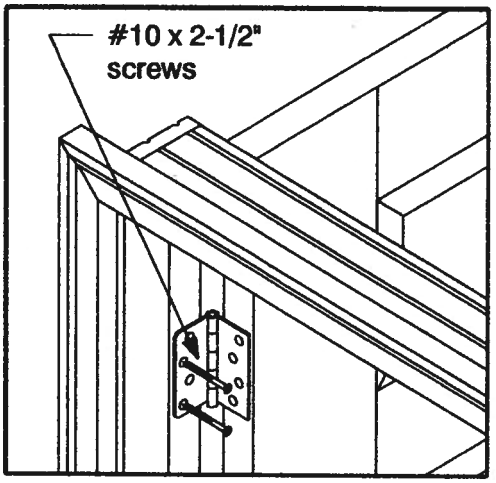
Remove transport clip. Open and close door to ensure smooth operation.



Remove small margin shim pads, located on the head and lock jambs, between door and jambs.



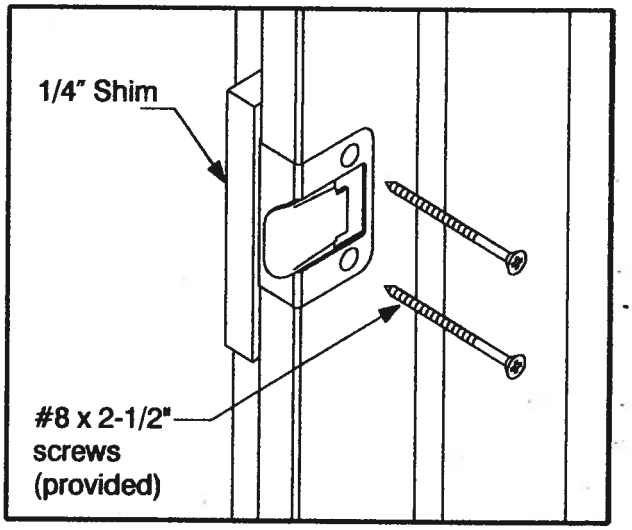
With door open, at hinges where labels indicate, drill 1/8" dia. pilot holes and fasten #10 x 2-1/2" screws (provided) through hinges to anchor door frame and prevent sagging.



Close door. Carefully shim between jamb and opening behind latch area.

Open door and install lockset hardware.

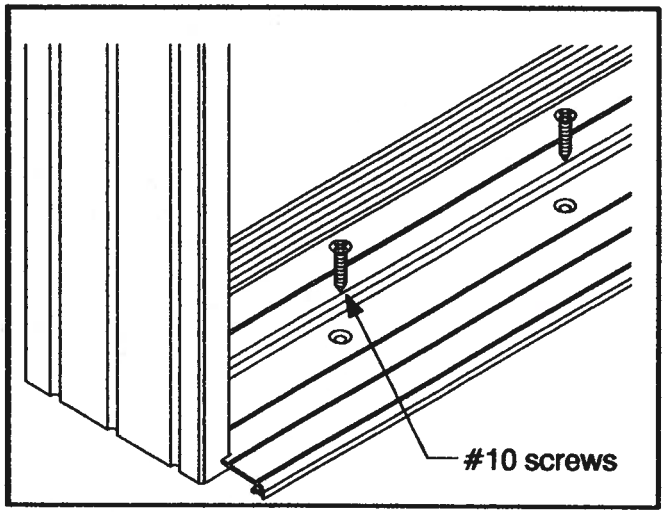
Drill 1/8" dia. pilot holes and place (2) #8 x 2-1/2" screws (provided) through strike mounting holes to secure lock jamb center and provide security. Adjust strike in or out for proper door operation and tighten screws.



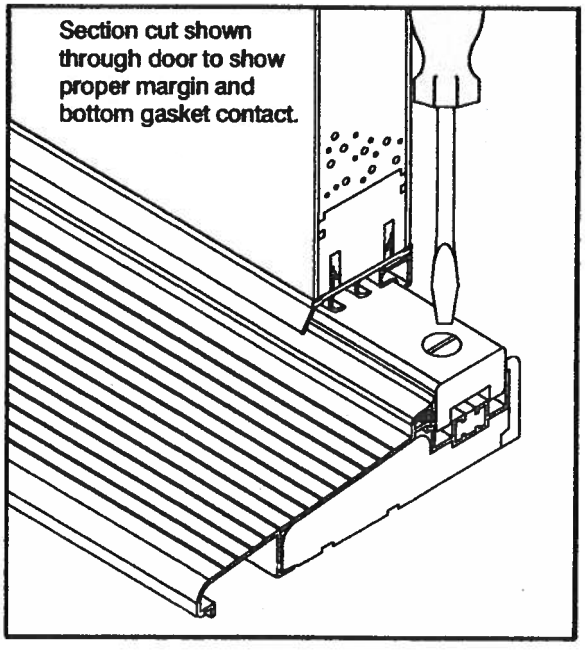
8 ANCHOR SILL, ADJUST SILL.

For sills prepared for anchor screws, place #10 screws through sill into subfloor. If not prepared, pre-drill 1/8" holes 6" from each end and at the center of each door. Countersink as needed.

If a sidelite is used, drill an additional pilot hole 3" from end where the door and sidelight meet.



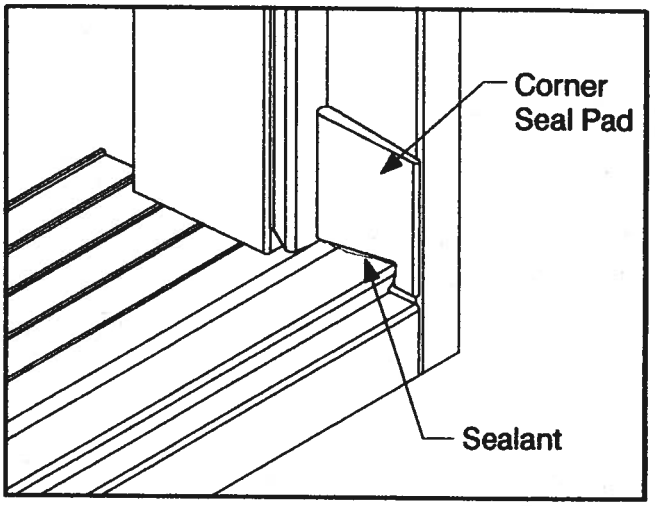
For sill with screw-adjustable thresholds, follow directions on sill to adjust threshold to meet door bottom gasket.



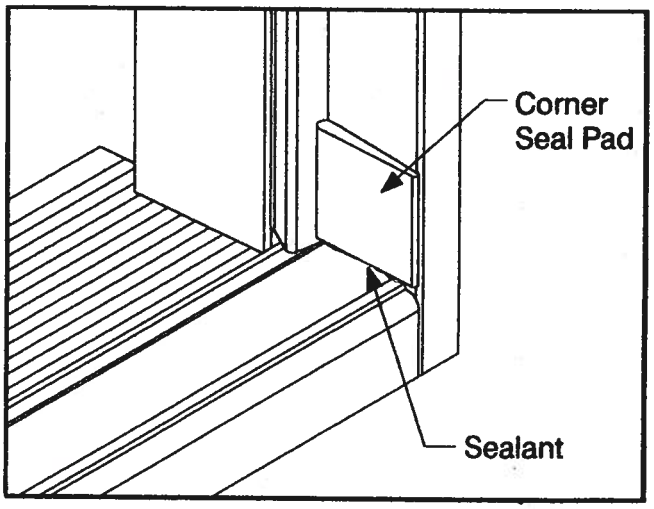
9 CHECK OR INSTALL CORNER SEAL PADS. (SWING-IN DOORS ONLY)

Corner seal pads are an important part of a swing-in door's weatherproofing integrity. Check to see that they are installed properly, in place and with sealant under the pad at the jamb and sill joint.

Self-Adjusting Sills (shop applied)



Adjustable Sills



For sills with adjustable thresholds, pads are sent in a poly bag with unit, not installed. They are installed after final threshold adjustment. Follow directions with pads to install.

PROCEED TO STEP 10.

10 WEATHERPROOF, FINISH, AND MAINTAIN ENTRY DOOR SYSTEM.

Place and set galvanized finish nails through brickmould around perimeter. Cover all countersunk fasteners with an exterior-grade putty.

Caulk around entire unit on "weather" side: seal brickmold to siding or facing, seal front bottom edge of sill, seal all joints between jambs and moldings.

Seal joints between exterior hardware trim and door face to prevent air and water infiltration.

Provide and maintain a properly-installed cap flashing to protect top surfaces from water damage.

Paint or stain according to Therma-Tru instructions. Do not paint gasketing or weatherstripping.

Bare unprotected wood will weather and degrade and change color. All bare wood surfaces exposed to weather must be primed or stained and painted or finished within two weeks of exposure.

Maintain or replace sealants and finishes as soon as any deterioration is evident. For semi-gloss and glossy paints or clearcoats, do this when surface becomes dull or rough. More severe exposures require more frequent maintenance.

Swing-out doors must have all edges - sides, top, and bottom - finished. Inspect and maintain these edges as regularly as all other surfaces.

FINISHING INSTRUCTIONS

TO PAINT A STEEL OR SMOOTH-STAR DOOR:
Clean first with detergent and water. Do not wash doorlite frames and moldings (see below). Rinse and let dry completely. Use only an exterior high-quality 100% acrylic latex paint following manufacturer's directions for application. Paint edges and exposed ends of door.

TO PAINT STEEL DOORLITE FRAMES AND MOLDINGS:
Clean first by lightly wiping with a clean cloth. The material has a factory-applied surface preparation to improve finish adhesion. (Do not use stripping solvents on doorlite frames. This will damage or remove the surface preparation.) Mask off glass, prime first with an alkyd-based primer, then finish with the same paint used for the door.

TO FINISH CLASSIC-CRAFT OR FIBER-CLASSIC DOORLITE FRAMES AND PANEL INSERTS:
Clean first by lightly wiping with a clean cloth. The material has a factory-applied surface preparation to improve finish adhesion. (Do not use stripping solvents on frames. This will damage or remove the surface preparation.) Mask off glass. Stain or paint using the same materials as for the door. (See below. If painting, prime first with an alkyd-based primer.) To balance color when staining, apply stain more lightly on doorlite frames and panels than on door. Topcoat when stain is completely dry.

TO PAINT OR STAIN ALL CLASSIC-CRAFT OR FIBER-CLASSIC DOORS:
Clean first. Use a dry rag or a clean cloth and mineral spirits or detergent and water. Allow to dry before finishing. Edges and exposed ends should also be finished. Only apply finish when the temperature is between 50 and 90 F. with humidity less than 85%. Do not work with the door in direct sunlight. Follow directions above for doorlite frames and panel inserts.

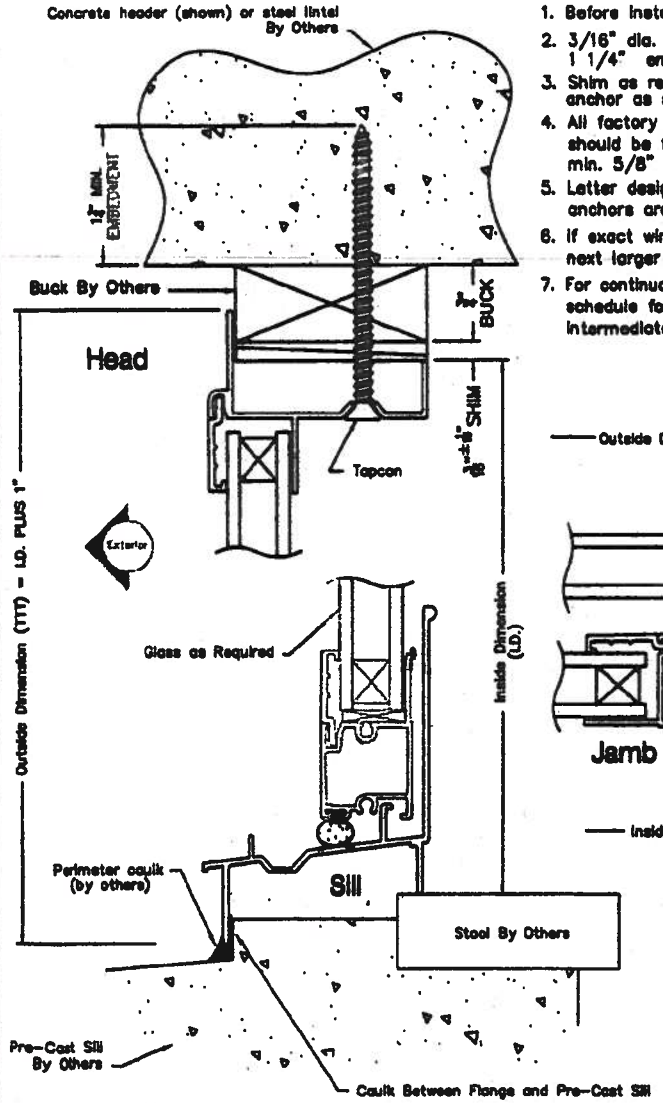
TO PAINT:
Prime first with an alkyd-based primer. Allow the primer to cure completely. Finish with an exterior-grade oil-based or alkyd or 100% acrylic latex paint.

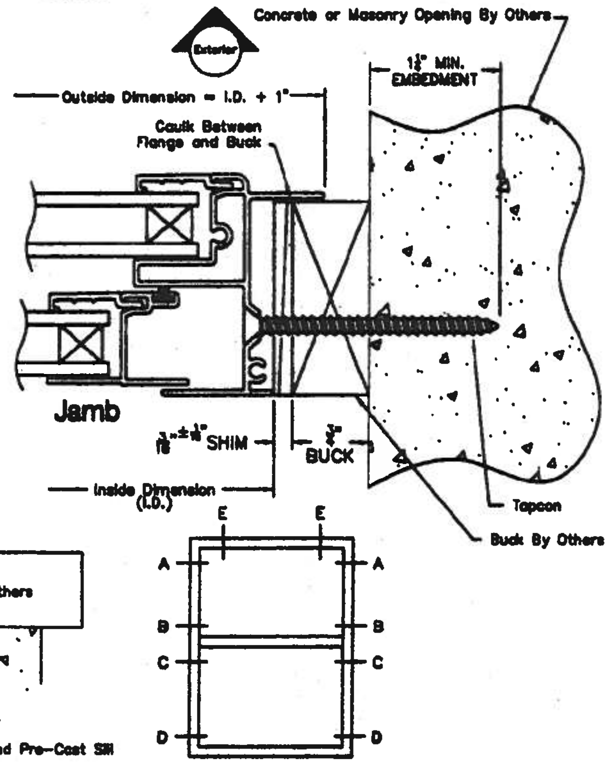
TO STAIN:
For best results, use only the complete Therma-Tru Finishing System. Follow directions enclosed with Finishing System.

Issued: 3/02 Printed in U.S.A

"ONE BY" (3/4") BUCKS (SHOWN)

1. Before installation, caulk back of flange, or face of buck.
2. 3/16" dia. masonry Tapcon must be of a length to have 1 1/4" embedment into masonry or concrete.
3. Shim as required with load bearing shims at each installation anchor as shown.
4. All factory applied holes not designated for Tapcon anchor should be filled with #10 screws of sufficient length to provide min. 5/8" embedment into wood buck.
5. Letter designations on the Tapcon location chart indicate where anchors are to be installed using the elevation as a key.
6. If exact window size is not given, use anchor quantity for next larger window in chart.
7. For continuous head and sill twins & triples, use the same fastener schedule for each unit in the main frame except ignore the intermediate jamb.





"TWO BY" (1 1/2") BUCKS

"TWO BY" bucks are engineered and fastened to the masonry opening BY OTHERS.

Follow the same instructions and fastener requirements for "one by" bucks except use #10 screws of sufficient length for 1 1/4" minimum embedment into buck.

MI HOME PRODUCTS BetterBilt / Capitol

* TAPCON LOCATION CHART

CODE SIZE	WINDOW ID SIZE	FASTENER LOCATIONS UP TO DP35	DP35.1 TO DP55	DP55.1 TO DP69.3
12	18 1/8 x 25	A, D & E	A, D & E	A, D & E
13	18 1/8 x 37 3/8	A, D & E	A, D & E	A, D & E
14	18 1/8 x 49 5/8	A, D & E	A, D & E	A, D & E
15	18 1/8 x 62	A, D & E	A, D & E	A, D & E
16	18 1/8 x 71	A, D & E	A, D & E	A, D & E
17	18 1/8 x 83	A, D & E	A, D & E	A, D & E
1/2 32	25 1/2 x 25	A, D & E	A, D & E	A, D & E
1/2 33	25 1/2 x 37 3/8	A, D & E	A, D & E	A, D & E
1/2 34	25 1/2 x 49 5/8	A, D & E	A, D & E	A, D & E
1/2 35	25 1/2 x 62	A, D & E	A, D & E	A, D & E
1/2 36	25 1/2 x 71	A, D & E	A, D & E	A, D & E
1/2 37	25 1/2 x 83	A, D & E	A, D & E	A, C, D & E
22	36 x 25	A, D & E	A, D & E	A, D & E
23	36 x 37 3/8	A, D & E	A, D & E	A, D & E
24	36 x 49 5/8	A, D & E	A, D & E	A, D & E
25	36 x 62	A, D & E	A, D & E	A, D & E
26	36 x 71	A, D & E	A, D & E	A, C, D & E
27	36 x 83	A, D & E	A, C, D & E	A, C, D & E
32	52 1/8 x 25	A, D & E	A, D & E	A, D & E
33	52 1/8 x 37 3/8	A, D & E	A, D & E	A, D & E
34	52 1/8 x 49 5/8	A, D & E	A, D & E	A, D & E
35	52 1/8 x 62	A, D & E	A, C, D & E	A, C, D & E
36	52 1/8 x 71	A, D & E	A, C, D & E	A, B, C, D & E
37	52 1/8 x 83	A, D & E	A, C, D & E	A, B, C, D & E
2040	23 3/8 x 47 5/8	A, D & E	A, D & E	A, D & E
2050	23 3/8 x 59 5/8	A, D & E	A, D & E	A, D & E
2060	23 3/8 x 71 5/8	A, D & E	A, D & E	A, D & E
2070	23 3/8 x 83 5/8	A, D & E	A, D & E	A, D & E
3040	35 3/8 x 47 5/8	A, D & E	A, D & E	A, D & E
3050	35 3/8 x 59 5/8	A, D & E	A, D & E	A, D & E
3060	35 3/8 x 71 5/8	A, D & E	A, D & E	A, C, D & E
3070	35 3/8 x 83 5/8	A, D & E	A, C, D & E	A, C, D & E
4040	47 3/8 x 47 5/8	A, D & E	A, D & E	A, D & E
4050	47 3/8 x 59 5/8	A, D & E	A, D & E	A, C, D & E
4060	47 3/8 x 71 5/8	A, D & E	A, C, D & E	A, C, D & E
4070	47 3/8 x 83 5/8	A, D & E	A, C, D & E	A, B, C, D & E
4450	51 3/8 x 59 5/8	A, D & E	A, D & E	A, C, D & E
4460	51 3/8 x 71 5/8	A, D & E	A, C, D & E	A, B, C, D & E
4470	51 3/8 x 83 5/8	A, D & E	A, C, D & E	A, B, C, D & E

*"TAPCON" TYPE HARDENED MASONRY SCREWS INCLUDE TAPCON, RAWL, & SIMPSON

SYM	REVISION	DATE	BY
A	REMOVED SILL INSTALLATION ANCHOR SCREWS	7/19/04	SB

STATE OF FLORIDA — Product Technology Corporation, Winter Park, Florida 32789

MI HOME PRODUCTS
GRATZ, PA

TITLE: 185/3185 SINGLE HUNG FLANGE FRAME INSTALLATION DETAILS & FASTENER SCHEDULE

PREPARED BY: PTC Product Technology Corporation — Phone 407.622.6334 Fax 407.622.6335

DRAWN: DRL	DATE: 06/15/04	SCALE: N.T.S.	DWG. NO.: MIHP0059
REV. LETTER: A	SHEET: 1 OF 1		

Universal Forest Products

Re: SO# 55023251
America's Home Place - Schueneman Roof, Oxford S/D

The truss drawing(s) referenced below have been prepared by Universal Forest Products, Inc. under my direct supervision. Pages or sheets covered by this seal include the following:

55023251 A01	55023251 V01
55023251 A02	55023251 V02
55023251 A03	55023251 V03
55023251 A04	55023251 V04
55023251 B01	55023251 V05
55023251 B01G	55023251 V06
55023251 B02	55023251 V07
55023251 B03	55023251 V08
55023251 B04	55023251 V09
55023251 C01	55023251 V10
55023251 C01G	55023251 V11
55023251 C02	
55023251 D01	
55023251 D01G	
55023251 E01	
55023251 E01G	
55023251 E02	
55023251 F01	
55023251 F01G	
55023251 M01	

Loading (psf) TCLL 20 TCDL 10 BCLL 0 BCDL 10
Wind: ASCE 7-02 per FBC2004; 120 mph; h = 15 ft; TCDL = 6.0 psf; BCDL = 6.0 psf; occupancy category II; exposure B; enclosed

My license renewal date for the state of Florida is February 28, 2007

9-28-05

Joseph W. Chandler License # 60527 date

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Sec. 2.

Eastern Division
5631 South NC 62 Burlington, NC 27215 Tel: (336) 226-9356 Fax: (336) 476-9146

From:	The Columbia County Building Department Plans Review 135 NE Hernando Av. P. O Box 1529 Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0512-22

America's Home Place Inc. / Owner Fred Schueneman 182 SW Honeybee Court

On the date of December 12, 2005 application 0512-22 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0512-22 when making reference to this application.

1. On the elevation drawing of the structure please show the total height of the structure from the established grade to the highest roof ridge.
2. Please submit product approval specification and product approval number(s) as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 for all material which will be on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products, **EXTERIOR DOORS, (include the overhead garage door) WINDOWS, ROOFING**, SKYLIGHTS and

GLASS BLOCKS: More information about statewide product approval can be obtained at www.floridabuilding.org

3. The soils in the area in which this building permit application make reference to are considered questionable soil by Engineering Service Group Inc. as described in section 1802.2.1 of the FBC-2004 therefore please follow the prescribed testing methods of chapter 18 to reveal the soil load bearing capacities. Please have a registered professional conduct subsurface explorations at the project site upon which foundations are to be constructed, a sufficient number (not less than four, one boring on each corner of the building foundation) borings shall be made to a depth of not less than 10 feet (3048 mm) below the level of the foundations to provide assurance of the soundness of the foundation bed and its load-bearing capacity.
4. Please correct the discrepancy between the Florida Energy Code for Building Construction (Form 600A-2001) Line 6 condition floor area (ft2) = 2,215. The structural plans show 2,203 condition floor areas (ft2) along with the building permit application 0512-22. These numbers must correspond with each other.
5. In the garage area show the method of protecting the appliances as required by the FMC-2004 sections 303.4 Protection from damage. Appliances shall not be installed in a location where subject to mechanical damage unless protected by approved barriers.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

E|S|G Engineering Services Group, Inc. CA# 8886
1299 W. Fairbanks Ave. Suite B. Winter Park, FL 32789 (407) 740-7111 / fax (407) 740-7656
229 S. Osprey Ave. Apt. 102 Sarasota, FL 34236 (941) 953-9711 / fax (941) 953-4711

To: Building Department
Subject: Plan Review comments
Project: Schueneman Residence
Permit #

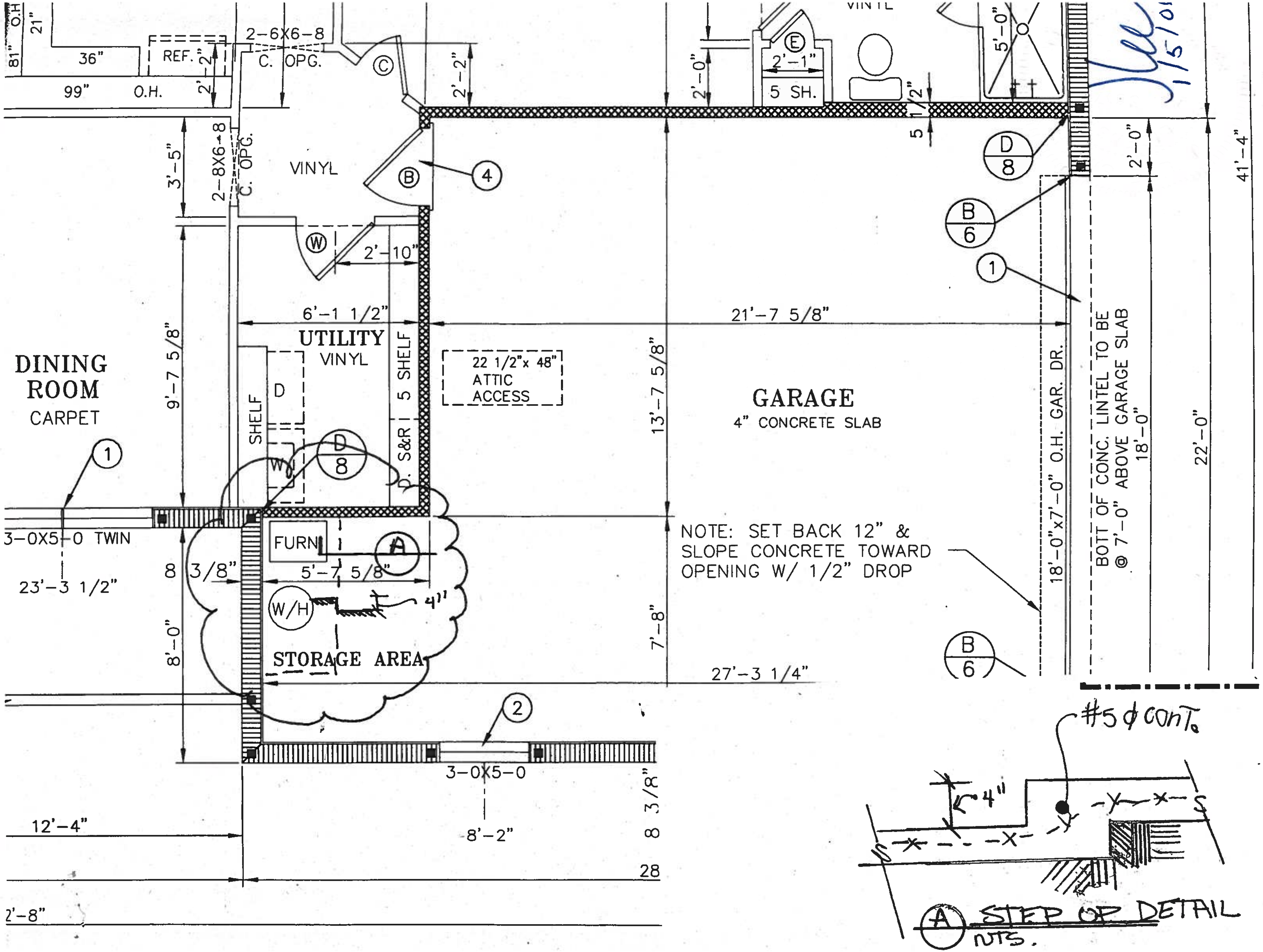
Dear Building Official:

In lieu of the original plan specifications, we have reviewed the subject / project and approve of the following:
1.) A Geotechnical Engineer is not required. The foundation has been designed for 2000 psf soil bearing pressure.
2.) At garage provide 4" step up per detail A.

If you have any questions please call me at the Winter Park office.

Thank you,

J. Lee Smith
FL PE# 36177
1/5/06



DINING ROOM
CARPET
3-0X5-0 TWIN
23'-3 1/2"
UTILITY
VINYL
SHELF
D
W
S&R
5 SHELF
6'-1 1/2"
2'-10"
2-8X6-8
C. OPG.
2-6X6-8
C. OPG.
VINYL
REF.
36"
99"
O.H.
81"
21"
2'-2"
3'-5"
9'-7 5/8"
8'-0"
8 3/8"
FURN
5'-7 5/8"
W/H
4"
STORAGE AREA
22 1/2"x 48"
ATTIC
ACCESS
GARAGE
4" CONCRETE SLAB
21'-7 5/8"
13'-7 5/8"
7'-8"
27'-3 1/4"
NOTE: SET BACK 12" & SLOPE CONCRETE TOWARD OPENING W/ 1/2" DROP
18'-0"x7'-0" O.H. GAR. DR.
BOTT OF CONC. LINTEL TO BE @ 7'-0" ABOVE GARAGE SLAB
18'-0"
22'-0"
41'-4"
2'-0"
5 1/2"
5 SH.
2'-1"
5'-0"
3-0X5-0
8'-2"
12'-4"
28
2'-8"
#5 ϕ CONT.
A STEP OF DETAIL
NTS.
1/5/01

GEO-TECH, INC.
ENGINEERING CONSULTANTS IN GEOTECHNICAL • ENVIRONMENTAL • CONSTRUCTION MATERIALS TESTING

January 4, 2006
Project No. 06139.50G

America's Home Place
3101 S.W. 34th Avenue, Suite 902
Ocala, FL 34474

Attention: Mr. Lee Meador

Project: Proposed Schueneman Residence
Columbia County, FL
Preliminary Site Exploration

Dear Mr. Meador:

As requested, Geo-Technologies, Inc. (Geo-Tech) has completed a preliminary geotechnical engineering evaluation at the project site. The purpose of the borings was to identify the shallow subsurface soils within the proposed building site and briefly comment on the suitability of the shallow soils for a conventional foundation system.

On December 19, 2005, Geo-Tech performed four (4) auger borings at the site to a depth of approximately ten (10) feet below the existing site grade in the proposed building area. The building area was staked by you. Representative samples were obtained from the borings and returned to our laboratory for visual classification. General soil stratification is based on a visual review of recovered soil samples and interpretation of field boring logs by a geotechnical engineer.

Subsurface Conditions

As interpreted from the auger borings the soils at the site generally consisted of a surface layer of fine sand approximately eight (8) to nine (9) feet thick underlain by slightly clayey sand to the depth augered.

The free groundwater level was not found at our boring location to the depth augered. In Geo-Tech's opinion, groundwater levels are not expected to influence near surface construction at this site. However, after periods of prolonged rainfall water may become perched above the slightly clayey sand soils and deeper foundations systems may encounter a perched water condition. A graphic presentation of the borings is presented on the attached sheets. The interface between soil layers is typically more gradual than suggested by the horizontal lines on the soil profiles.

Preliminary Evaluation and Recommendations

Based on the information found at our boring locations, it is Geo-Tech's opinion that the slightly clayey sand soils are at a depth that should not affect the foundation system for this project.

Should you encounter these clay soils at shallower depths during the earthwork phase of construction, a minimum separation of two (2) feet from the bottom of the slab and foundation should be maintained (see Figure 1). If undercutting, the depth of the undercut excavation should be controlled so that a "bathtub effect" that will trap water is not created. The bottom of the undercut should be graded to drain to a positive gravity outfall. If it is not feasible to have a positive gravity outfall, an underdrain should be placed in the bottom of the excavation to drain stormwater that may accumulate in the excavation. Backfill soils should consist of clean sand soils compacted in twelve (12) inch loose lifts. Generally sand with between three (3) and twelve (12) percent fines passing a No. 200 sieve is used for this purpose. The upper sand soils found on site should meet these requirements and can be used if kept separate from the clay soils during the earthwork phase of construction. The structural fill soils should be compacted to a minimum of 95 percent of the maximum dry density per the modified proctor determination. We wish to emphasize that the excavation and replacement of the underlying clay soils from beneath the building is not a guarantee that the deeper clays will not cause foundation movements. However the risk is reduced significantly.

Proof-Rolling

Proof-rolling of the cleared surface is recommended to: (1) locate any soft areas or unsuitable surface or near surface soils, (2) increase the density of the near surface soils, and (3) prepare the existing surface for the addition of fill soils (if required). Proof-rolling of the building areas should consist of a least ten (10) passes of a self-propelled static compactor. Each pass of the compactor should overlap the preceding pass by thirty (30) percent to insure complete coverage. If deemed necessary, in areas that continues to "yield", remove all deleterious material and replace with a clean, compacted sand backfill. **The proof-rolling should occur after cutting and before filling. Vibratory compaction equipment should not be used within one-hundred (100) feet of neighboring structures.**

Closure

The preliminary recommendations and conclusions presented in this report are based on the limited data obtained from the shallow soil borings. Deeper soil profiles were not requested for this project. Variations in soils may be present adjacent to or between the borings which were not apparent in the boring logs presented. If variations are found during construction of the project, it will be necessary to review the preliminary recommendations found in this report.

Geo-Tech trusts this report is sufficient to meet your immediate needs. Should you have any questions concerning this report or if we may be of further assistance, please do not hesitate to contact the undersigned.

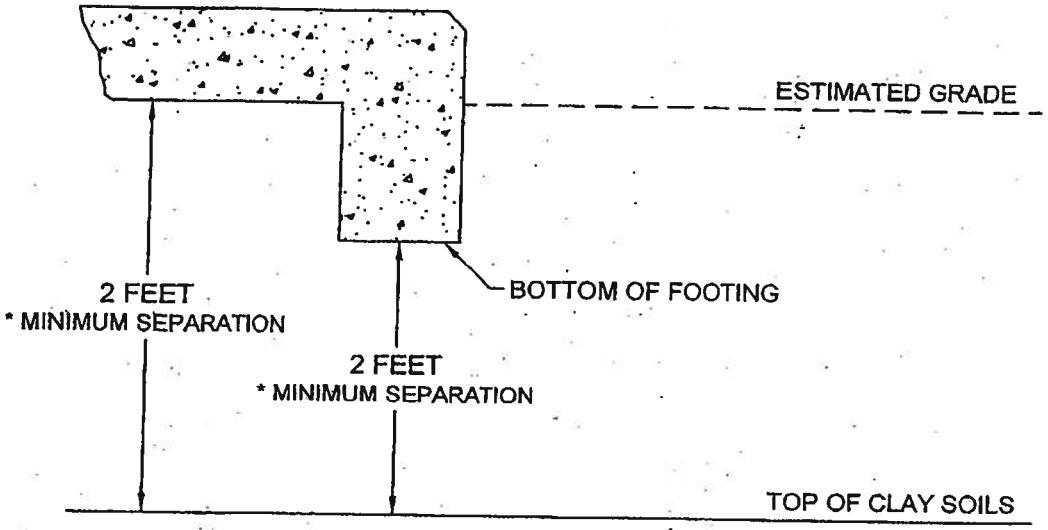
Sincerely,

Donald "Bubba" Youngblood
Branch Manager

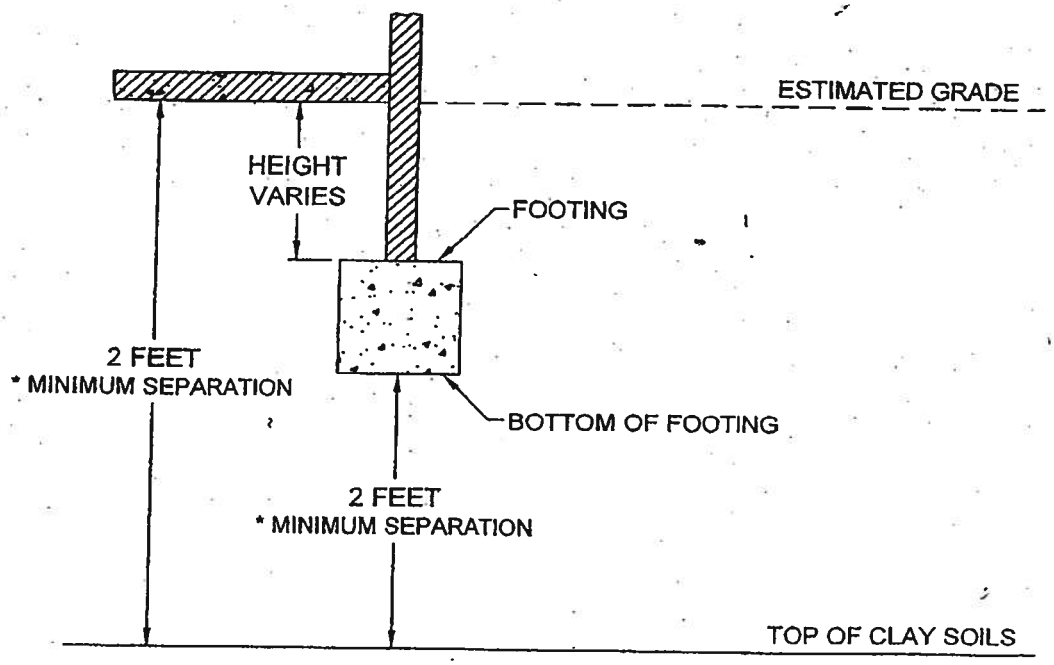
David A. Cappa, P.E.
Florida Registration No. 58334

DY/DAC:dy

FIGURE 1



MONOLITHIC FOOTING DETAIL
NOT TO SCALE



STEM WALL FOOTING DETAIL
NOT TO SCALE

*MAY VARY DEPENDING UPON CONSTRUCTION TECHNIQUE

GEO TECH INC.
ENGINEERING CONSULTANTS

Log of Borehole: A-1

Project: PROPOSED SCHUENEMAN RESIDENCE, COLUMBIA CO., FL
Project No: 06139.50G
Boring Location: N.W. CORNER AREA OF PROPOSED RESIDENCE
Engineer: NJH/DAC
Client: AMERICA'S HOME PLACE
Enclosure: NONE

GEO-TECH, Inc.
Engineering Consultants
4000 SW 35th Terr., Suite C
Gainesville, Florida 32608

SUBSURFACE PROFILE				SAMPLE	
Depth	Symbol	Description	Depth/Elev.	Number	Remarks
0		Ground Surface	0.0		
		FINE SAND TAN FINE SAND (SP)		1	
7			7.0		
		SLIGHTLY CLAYEY SAND ORANGE AND TAN SLIGHTLY CLAYEY SAND (SP-SC)		2	
10			10.0		
		End of Borehole			
11					

Depth to Ground Water: NOT FOUND
Drill Date: DECEMBER 19, 2005
Drilled By: MC/BB
Drill Method: ASTM D-4700

Remarks: (SP) Unified Soil Group Classification Symbol as Determined by Visual Review

Log of Borehole: A-2

Project: PROPOSED SCHUENEMAN RESIDENCE, COLUMBIA CO., FL ***Project No:*** 06139.50G

Boring Location: S.W. CORNER AREA OF PROPOSED RESIDENCE ***Engineer:*** NJH/DAC

Client: AMERICA'S HOME PLACE ***Enclosure:*** NONE

GEO-TECH, Inc.
Engineering Consultants
4000 SW 35th Terr., Suite C
Gainesville, Florida 32608

SUBSURFACE PROFILE				SAMPLE	
Depth	Symbol	Description	Depth/Elev.	Number	Remarks
0		Ground Surface	0.0		
		FINE SAND TAN FINE SAND (SP)		1	
8		***SLIGHTLY CLAYEY SAND*** ORANGE AND TAN SLIGHTLY CLAYEY SAND (SP-SC)	8.0	2	
10		End of Borehole	10.0		
11					

Depth to Ground Water: NOT FOUND
Drill Date: DECEMBER 19, 2005

Drilled By: MC/BB
Drill Method: ASTM D-4700

Remarks: (SP) Unified Soil Group Classification Symbol as Determined by Visual Review

Log of Borehole: A-3

Project: PROPOSED SCHUENEMAN RESIDENCE, COLUMBIA CO., FL
Project No: 06139.50G
Boring Location: S.E. CORNER AREA OF PROPOSED RESIDENCE
Engineer: NJH/DAC
Client: AMERICA'S HOME PLACE
Enclosure: NONE

GEO-TECH, Inc.
Engineering Consultants
4000 SW 35th Terr., Suite C
Gainesville, Florida 32608

SUBSURFACE PROFILE				SAMPLE	
Depth	Symbol	Description	Depth/Elev.	Number	Remarks
0		Ground Surface	0.0		
		FINE SAND TAN FINE SAND (SP)		1	
9		***SLIGHTLY CLAYEY SAND*** ORANGE AND TAN SLIGHTLY CLAYEY SAND (SP-SC)	9.0	2	
10		End of Borehole	10.0		
11					

Depth to Ground Water: NOT FOUND
Drill Date: DECEMBER 19, 2005
Drilled By: MC/BB
Drill Method: ASTM D-4700

Remarks: (SP) Unified Soil Group Classification Symbol as Determined by Visual Review

Log of Borehole: A-4

Project: PROPOSED SCHUENEMAN RESIDENCE, COLUMBIA CO., FL ***Project No:*** 06139.50G

Boring Location: N.E. CORNER AREA OF PROPOSED RESIDENCE ***Engineer:*** NJH/DAC

Client: AMERICA'S HOME PLACE ***Enclosure:*** NONE

GEO-TECH, Inc.
Engineering Consultants
4000 SW 35th Terr., Suite C
Gainesville, Florida 32608

SUBSURFACE PROFILE				SAMPLE	
Depth	Symbol	Description	Depth/Elev.	Number	Remarks
0		Ground Surface	0.0		
0–8		***FINE SAND*** TAN FINE SAND (SP)		1	
8			8.0		
8–10		***SLIGHTLY CLAYEY SAND*** ORANGE AND TAN SLIGHTLY CLAYEY SAND (SP-SC)		2	
10			10.0		
10		End of Borehole			

Depth to Ground Water: NOT FOUND
Drill Date: DECEMBER 19, 2005

Drilled By: MC/BB
Drill Method: ASTM D-4700

Remarks: (SP) Unified Soil Group Classification Symbol as Determined by Visual Review

GEO-TECH

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☑	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☑ See Boundary survey	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☑	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used fo the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☑	**Elevations including:**
☐	☑	a) All sides
☐	☑	b) Roof pitch
☐	☑	c) Overhang dimensions and detail with attic ventilation
☐	☑	d) Location, size and height above roof of chimneys
☐	☑	e) Location and size of skylights
☐	☐ See NOTE 1	f) Building height
☐	☑	e) Number of stories 1

Floor Plan including:

a) Rooms labeled and dimensioned

b) Shear walls all exterior walls or shearwalls

c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown) see NOTE 2

d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth N/A

e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails N/A

f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing

b) All posts and/or column footing including size and reinforcing

c) Any special support required by soil analysis such as piling see NOTE 3

d) Location of any vertical steel

Roof System:

a) Truss package including:

1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including:

1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall

1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, material manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termicide or alternative method)
10. Slab on grade
    a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ **b) Wood frame wall**

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anc bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, material manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☐ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

☑ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☐ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

☐ ☐ **Disclosure Statement for Owner Builders**

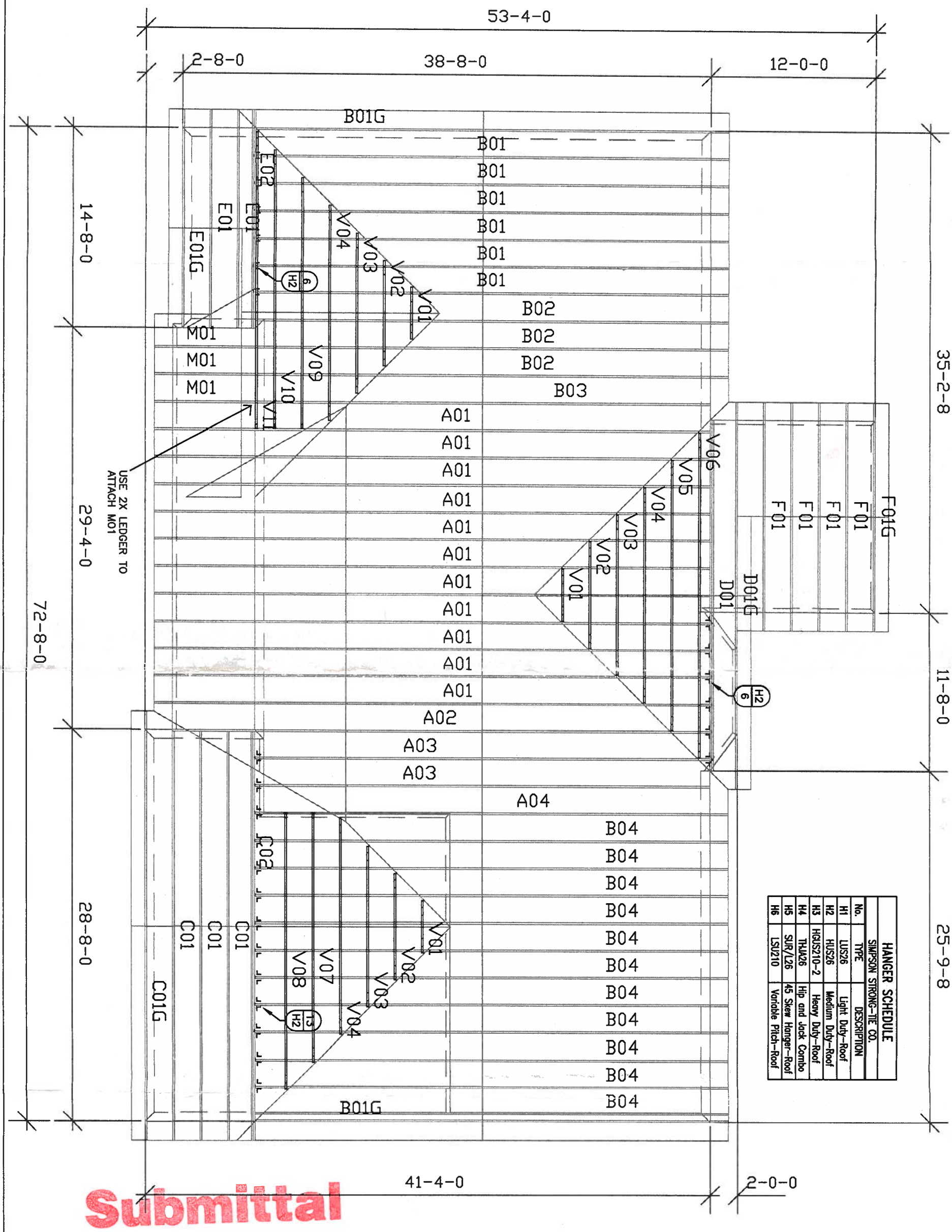
☑ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☐ ☐ **Private Potable Water**

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used



HANGER SCHEDULE		
	SIMPSON STRONG-TIE CO.	
No.	TYPE	DESCRIPTION
H1	LUS26	Light Duty-Roof
H2	HUS26	Medium Duty-Roof
H3	HGUS210-2	Heavy Duty-Roof
H4	THJA26	Hip and Jack Combo
H5	SUR/L26	45 Skew Hanger-Roof
H6	LSU210	Variable Pitch-Roof

Submittal Copy

DRAWING NUMBER 5090601
CHECKED BY:
DRAWN BY: DAVID BOGUE

CUSTOMER NAME: AMERICA'S HOME PLACE
PROJECT: SCHUENEMAN ROOF OXFORD
DATE 9-7-5
SCALE: NTS
QUALITY PRODUCTS FOR QUALITY BUILDERS
REVISION: DATE: BY:
REVISION: DATE: BY:

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