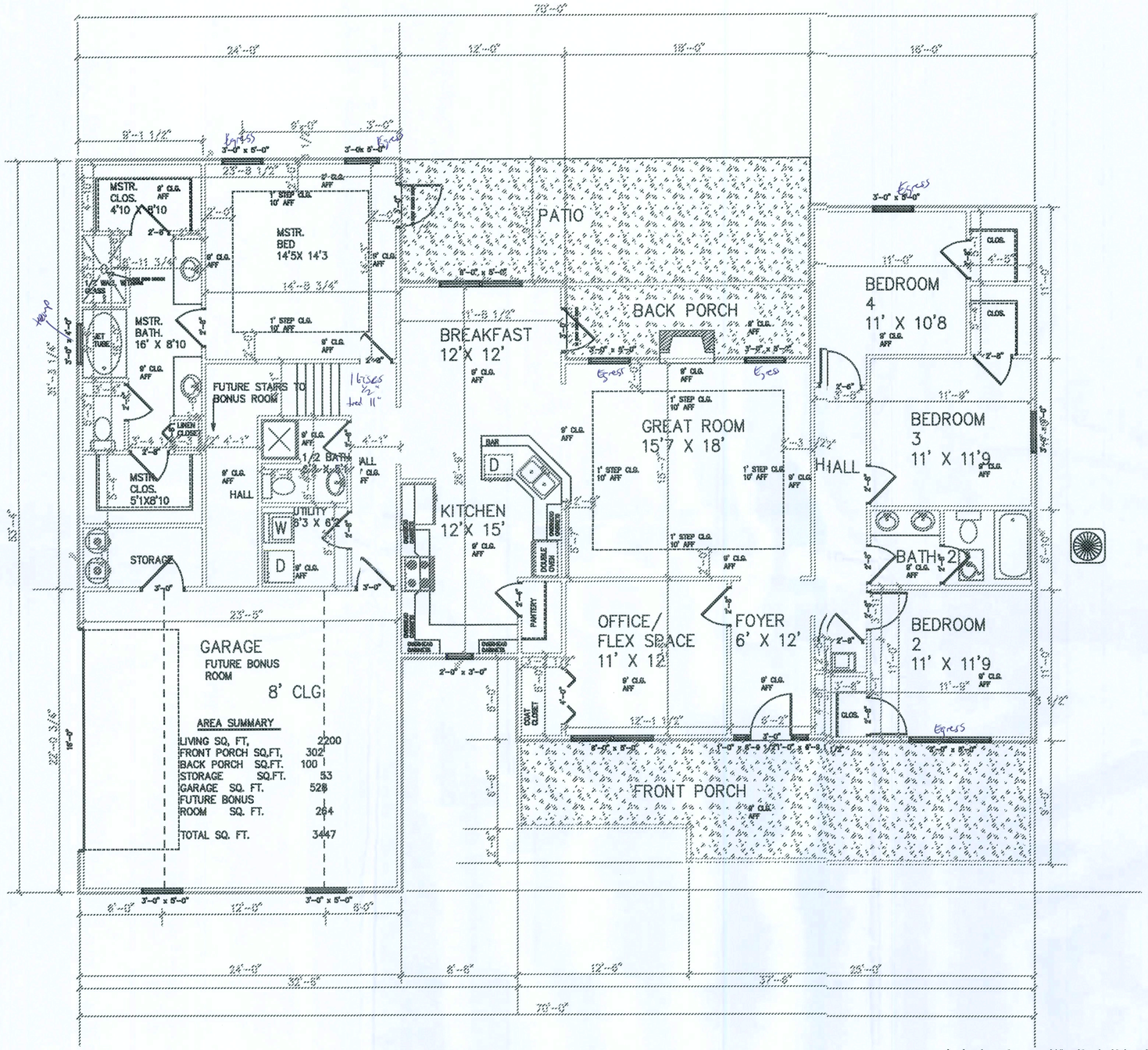


I do hereby certify that this drawing or plan and related specifications meet all local code requirements and are in substantial conformity with the Department of Veterans Affairs Minimum Property Requirements, (including the energy conservation standards of the 1992 Council of American Building Officials (CABO) Model Energy Code(MECA).

Signature_____ Code Number A-660

BRYAN ZECHER CONSTRUCTION

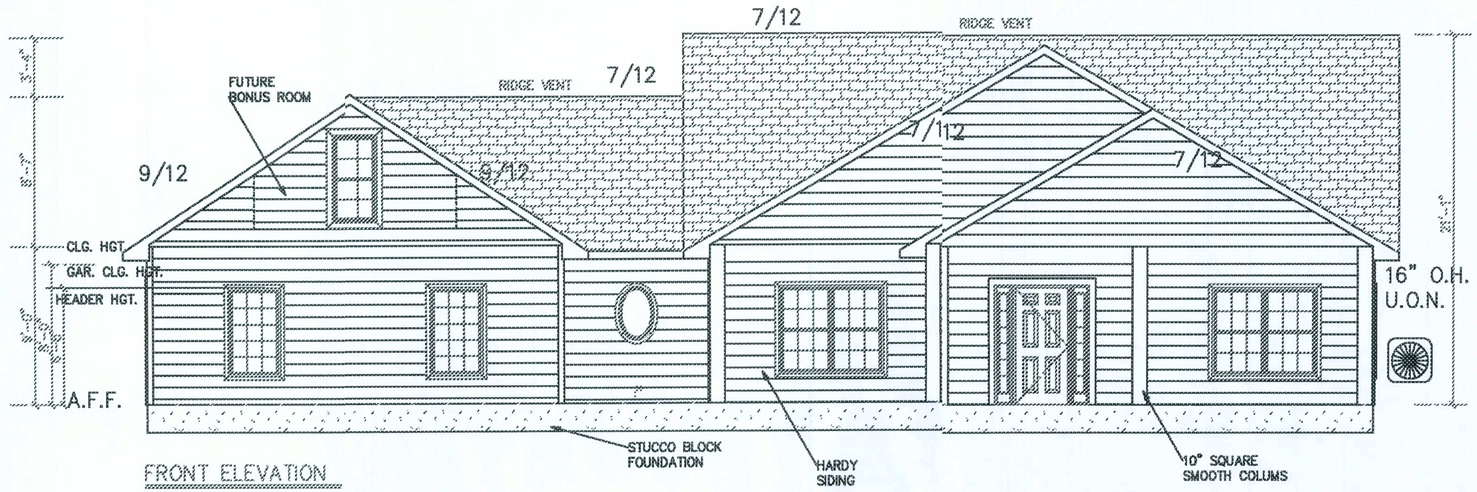
SCALE 1/4"=1'	APPROVED	DRAWN BY TB
DATE June 18, 2008		REVISED 8-18-08

JOB NAME: SHALLAR RES.

LARRY AND CHRISTINA SHALLAR
RESIDENT

ADDRESS
1048 SE WEEKS LN.
LAKE CITY FL 32025
COLUMBIA COUNTY

A1 OF 4 SHEETS | FLOOR PLAN



FRONT ELEVATION

REAR ELEVATIOI

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Signature_____ Code Number A-660

BRYAN ZECHER CONSTRUCTION

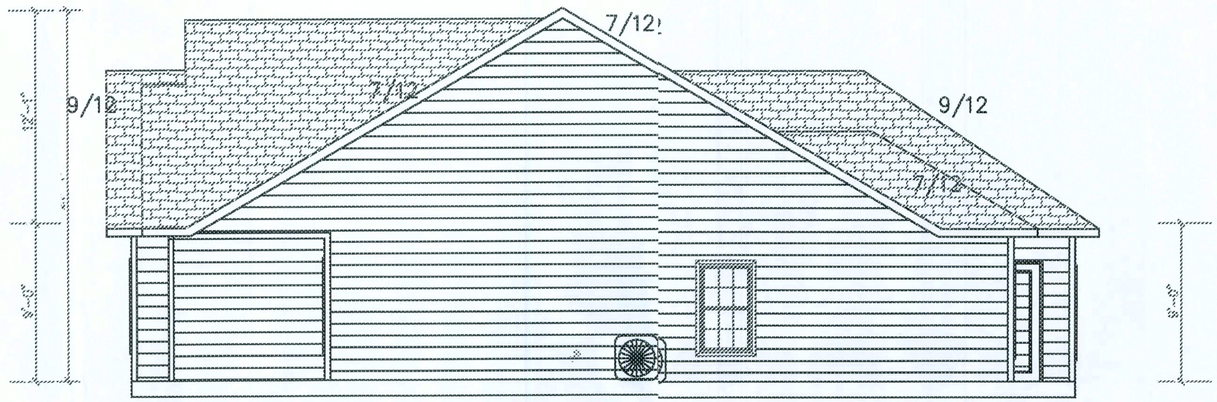
SCALE 1/4=1' | APPROVED | DRAWN BY JZB
DATE June 18, 2008 | REVISED 8-6-08

JOB NAME: SHALLAR RES.

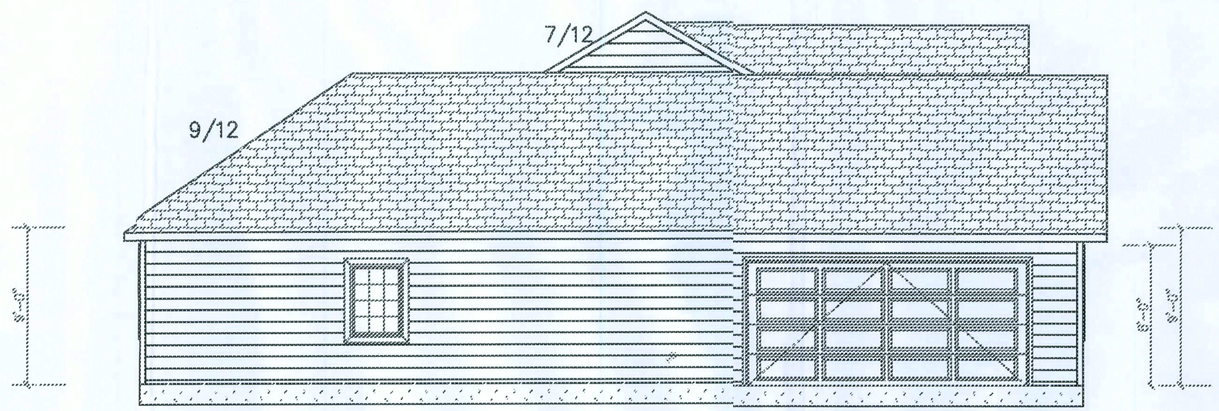
LARRY AND CHRISTINA SHALLAR
RESIDENT

ADDRESS
1048 SE WEEKS LN
LAKE CITY FL 32025
COLUMBIA COUNTY

A2 OF 4 SHEETS | ELEVATIONS



RIGHT ELEVATION



LEFT ELEVATION

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Signature_____ Code Number A-660

BRYAN ZECHER
CONSTRUCTION

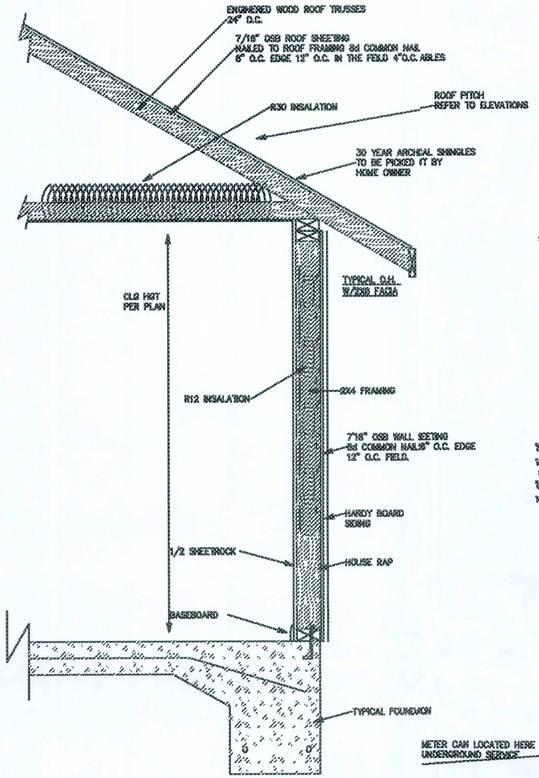
SCALE 1/4"=1'	APPROVED	DRAWN BY JB
DATE June 19, 2009		REVISED 6-19-09

JOB NAME: SHALLAR RES.

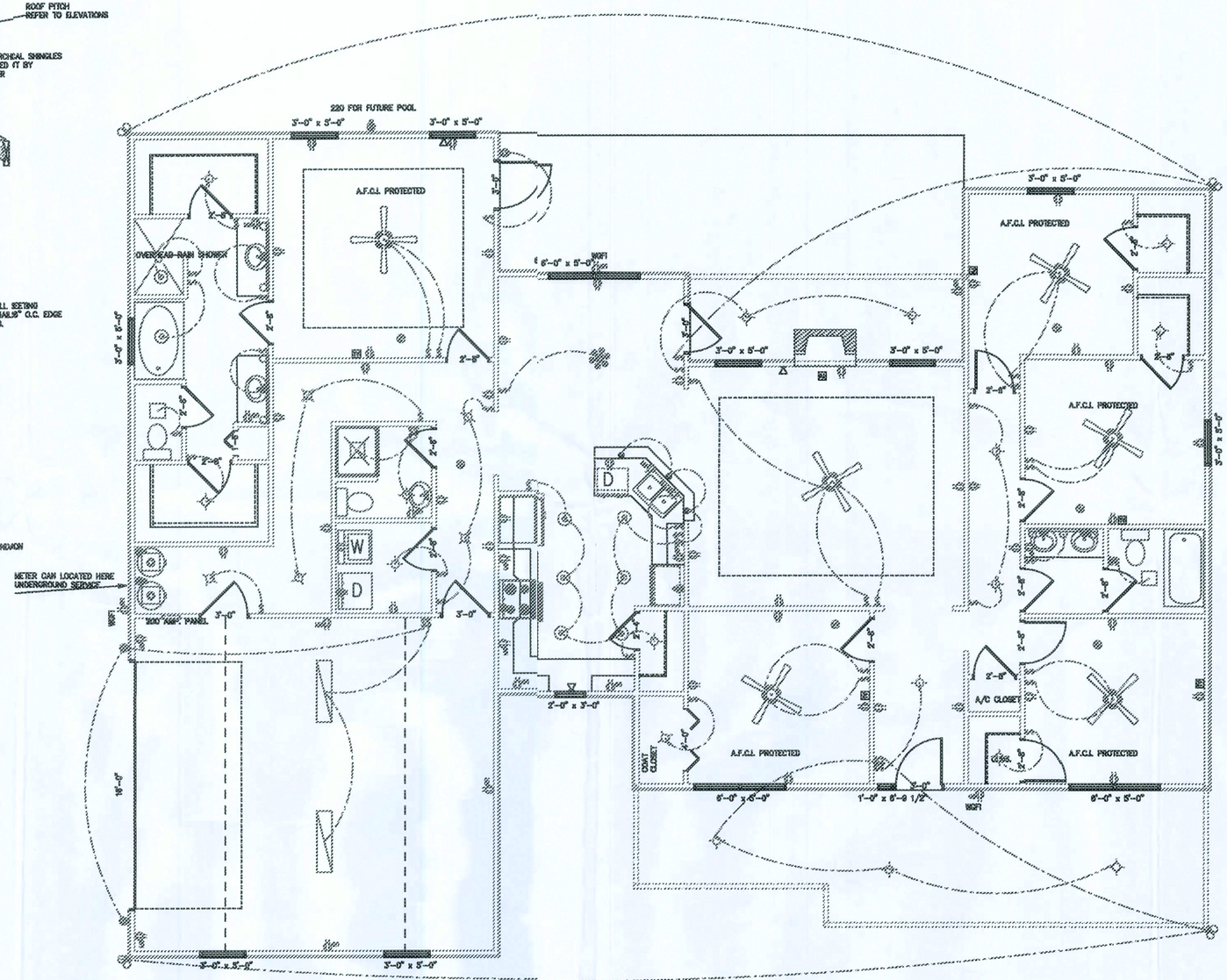
LARRY AND CHRISTINA SHALLAR
RESIDENT

ADDRESS
1048 SE WEEKS LN
LAKE CITY FL 32025
COLUMBIA COUNTY

A3 OF 4 SHEETS | ELEVATIONS



TYPICAL WALL SECTION



ELECTRIC PLAN

ELECTRICAL NOTES

E-1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUFACTER SPECS

E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED

E-3 ALL INSTALATIONS SHALL BE PER NATIONAL ELECTRIC CODE

E-4 ALL SMOKE DETECTORS SHALL BE 120V W/BATTERY BACKUP OF THE P-O ELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER INSTALL INSIDE AND NEAR BEDROOMS

E-5 TELEPHONE TEVELMISION AND OTHER LOW DEVICES OR OUTLETS SHALL BE AS PER THE OWNERS DIRECTION AND IN ACCORDANCE WITH APPLICABLE SECTIONS ON NATIONAL ELECTRIC CODE LATEST ADDITION

E-6 ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ELECTRICAL SERVICE AND CIRCUITS

E-7 ENTRY OF SERVICE UNDERGROUND OR OVERHEAD TO BE DETERMINED BY THE POWER COMPANY

E-8 ALL BEDROOMS RECEPTICALS ARE TO BE A.F.C.I.

ELECTRICAL	SYMBOL
ceiling fan spotlights 1	
recessed can lights	
ceiling light vent square	
chandelier	
double spotlight	
pendant globe	
track light	
wall mount 1	
electrical panel	
cable tv outlet	
light	
outlet	
outlet 220v	
outlet gfi	
smoke detector	
switch	S
telephone	T
fluorescent light 2 x 4	

I do hereby certify that this drawing or plan and related specifications meet all local code requirements and are in substantial conformity with the Department of Veterans Affairs Minimum Property Requirements, (including the energy conservation standards of the 1992 Council of American Building Officials (CABO) Model Energy Code(MECA).
Signature_____ Code Number A-660

BRYAN ZECHER CONSTRUCTION

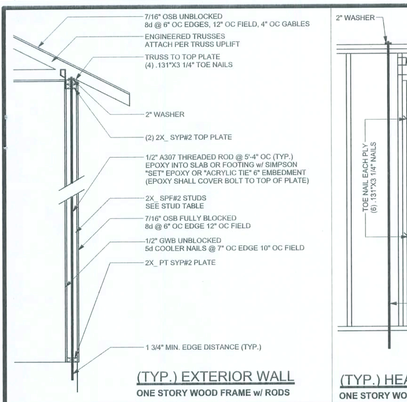
SCALE 1/4=1'	APPROVED	DRAWN BY JB
DATE June 18, 2008		REVISED 6-11-08

JOB NAME: SHALLAR RES.

LARRY AND CHRISTINA SHALLAR
RESIDENT

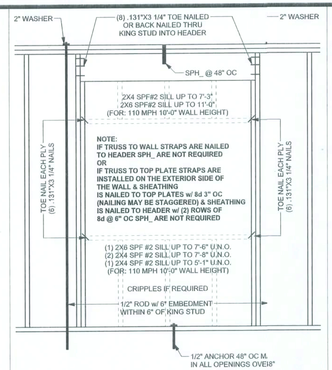
ADDRESS
1048 SE WEEKS LN
LAKE CITY FL 32025
COLUMBIA COUNTY

A4 OF 4 SHEETS	ELECTRIC PLAN



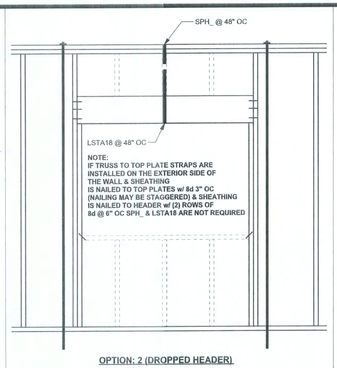
(TYP.) EXTERIOR WALL

ONE STORY WOOD FRAME w/ RODS



(TYP.) HEADER OPTION: 1 (FLUSH HEADER)

ONE STORY WOOD FRAME w/ RODS



OPTION: 2 (DROPPED HEADER)

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES	
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"	
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"	
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"	
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"	
H6	950	820					8-8d	8-8d	
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"	
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d	
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d	
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"	
H10-2	760	655	455	305	390	340	6-10d	6-10d	
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"	
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"	
LTS12 - LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"	
MTS12 - MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"	
HTS16 - HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"	
HEAVY GIRDER TIEDOWNS									**TO FOUNDATION**
LGT2	2050	1785	700	170	700	170	14-16d	14-16d	
LGT3-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS	
LGT4-SDS3	4060	3860	2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS	
MGT	3965	3330					22 -10d		5/8" ANCHOR
HGT-2	10980	6485					16 -10d		2-5/8" ANCHOR
HGT-3	10530	9035					16 -10d		2-5/8" ANCHOR
HGT-4	9250	9250					16 -10d		2-5/8" ANCHOR
STUD STRAP CONNECTOR									**TO STUDS**
SSP DOUBLE TOP PLATE	435	435						3-10d	4 -10d
SSP SINGLE SILL PLATE	455	420						1-10d	4 -10d
DSP DOUBLE TOP PLATE	825	825						6 -10d	8 -10d
DSP SINGLE SILL PLATE	825	600						2 -10d	8 -10d
SP1	585	535						4 -10d	6 -10d
SP2	1065	605						6 -10d	6 -10d
SP4	885	760							6-10d x 1 1/2"
SPH4	1240	1065							10-10d x 1 1/2"
SP6	885	760							6-10d x 1 1/2"
SPH6	1240	1065							10-10d x 1 1/2"
LSTA18	1235	1110							14-10d
LSTA21	1235	1235							16-10d
CS20	1030	1030							14-10d
CS16	1705	1705							22-10d
STUD ANCHORS							**TO STUDS**		**TO FOUNDATION**
LTT19	1350	1305					8-16d		1/2" ANCHOR
LTTI31	2310	2310					18-10d x 1 1/2"		5/8" ANCHOR
HD2A	2775	2570					2-5/8" BOLTS		5/8" ANCHOR
HTT16	4175	3695					18-16d		5/8" ANCHOR
HTT22	5260	5250					32-16d		5/8" ANCHOR
ABU44	2200	2200					12-16d		5/8" ANCHOR
ABU66	2300	2300					12-16d		5/8" ANCHOR
ABU88	2320	2320					18-16d		2-5/8" ANCHOR

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL
(2) FOR SYP GIRDER & SPF STUDS

IL HOLES

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR ENO ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GRADE & SPECIES TABLE

		Fb (psi)	E (10^6 psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 65KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH BI COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED 8DND BEAM OR 10" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

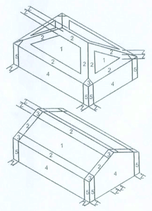
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.) BASIC WIND SPEED = 110 MPH
2.) WIND EXPOSURE = C
3.) WIND IMPORTANCE FACTOR = 1.0
4.) BUILDING CATEGORY = II
5.) ROOF ANGLE = 10-45 DEGREES
6.) MEAN ROOF HEIGHT = <30 FT
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft2)			
	10		100	
1	27.8	-30.5	25.3	-25.3
2	27.8	-35.7	25.3	-30.5
2 OTig		-56.8		-56.8
3	27.8	-35.7	25.3	-30.5
3 OTig		-95.6		-58.3
4	30.5	-33.0	25.9	-28.5
5	30.5	-40.7	25.9	-31.6

Doors & Windows Worst Case (Zone 5, 10 ft2)	30.5	-40.7
8x7 Garage Door	27.3	-32.0
16x7 Garage Door	25.9	-29.4

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS		

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER:
Mark Disosway, PE
No.53915, POB 868, Lak City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all quations to Mark Disosway, P.E. for isolution. Do not proceed without cerification.

CERTIFICATION: I hereby certify that I have examined this plan, and let the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida buildingcode residential 2007, to the best of my knowleqe.

LIMITATION: This desiqn is valid for one building, at specified locaion.

MARK DISOSWAY
P.E. 53915

SEAL

Bryan Zecher
Constrction

Shallar Rsidence

ADDRESS:
1048 SE Weks Lane
Lake City, Florida

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386)754 - 5419
Fax: (386) 259 - 4871

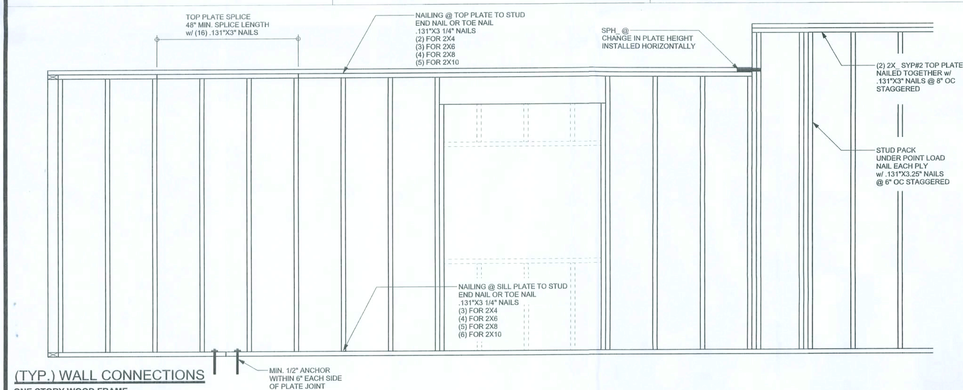
PRINTED DATE:
June 30, 2009

DRAWN BY: David Disosway
STRUCTURAL BY: David Disosway

FINALS DATE:
30Jun09

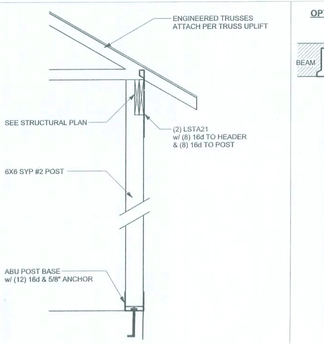
JOB NUMBER:
906251

DRAWING NUMBER
S-I
OF 3 SHEETS



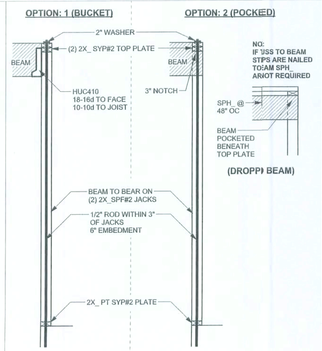
(TYP.) WALL CONNECTIONS

ONE STORY WOOD FRAME



(TYP.) INTERIOR BEARING WALL

ONE STORY WOOD FRAME w/ RODS



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, EN..OSED

(TYP.) GABLE BRACING DETA

WOOD FRAME

(TYP.) GABLE WALL w/ VAULTED CEILING

WOOD FRAME

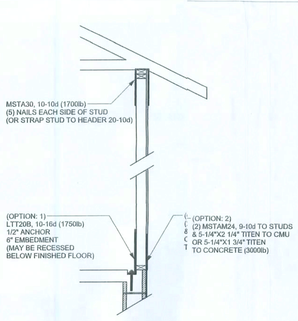
(TYP.) PORCH POST

ONE STORY WOOD

(TYP.) BEAM TO WALL

WOOD FRAME w/ RODS

ADWABLE UPLIFT:
17LB



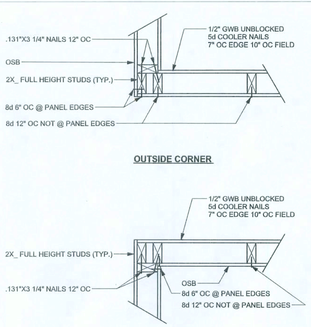
(TYP.) WALL CONNECTIONS

ONE STORY WOOD FRAME

ALTERNATE CONNECTION WHERE ROD CANNOT BE PLACED IN WALL

ONE STORY WOOD FRAME w/ RODS

ALLOWABLE UPLIFT:
1900 LB

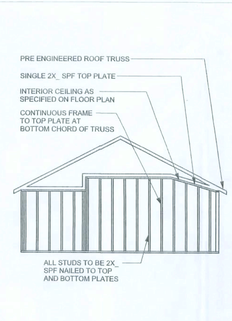


OUTSIDE CORNER

INSIDE CORNER

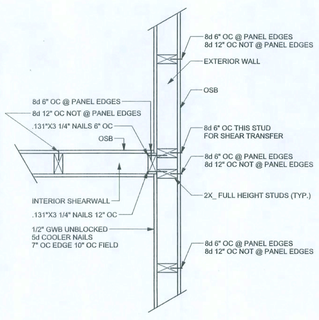
(TYP.) CORNER FRAMING

WOOD FRAME



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

WOOD FRAME



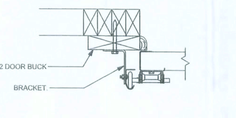
(TYP.) INTERSECTING WALL FRAMING

WOOD FRAME

2X6 SYP#2 GARAGE DOOR BUCK ATTACHMENT

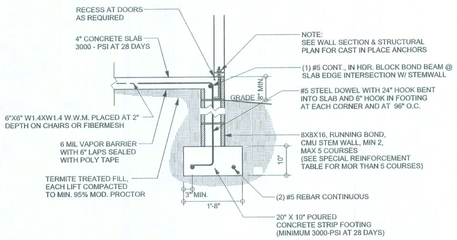
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"X4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131X3 1/4" GN PER TABLE BELOW:

DOOR WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC
20'	12" OC	2.5" OC	2.5" OC

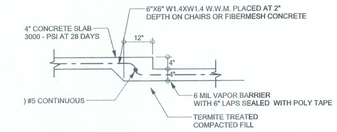


(TYP.) GARAGE DOOR BUCK INSTALLATION

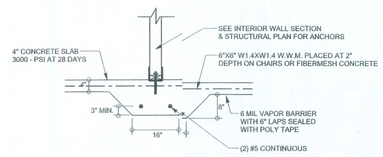
WOOD FRAME



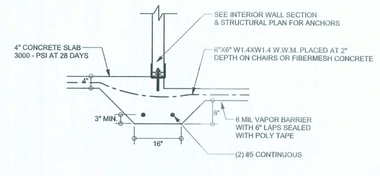
F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



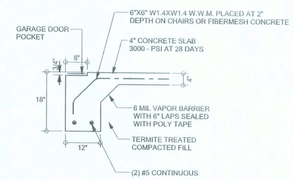
F S2 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



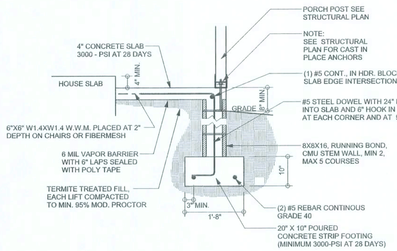
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



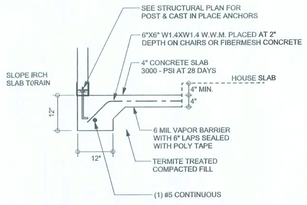
F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



F12 S-2 STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

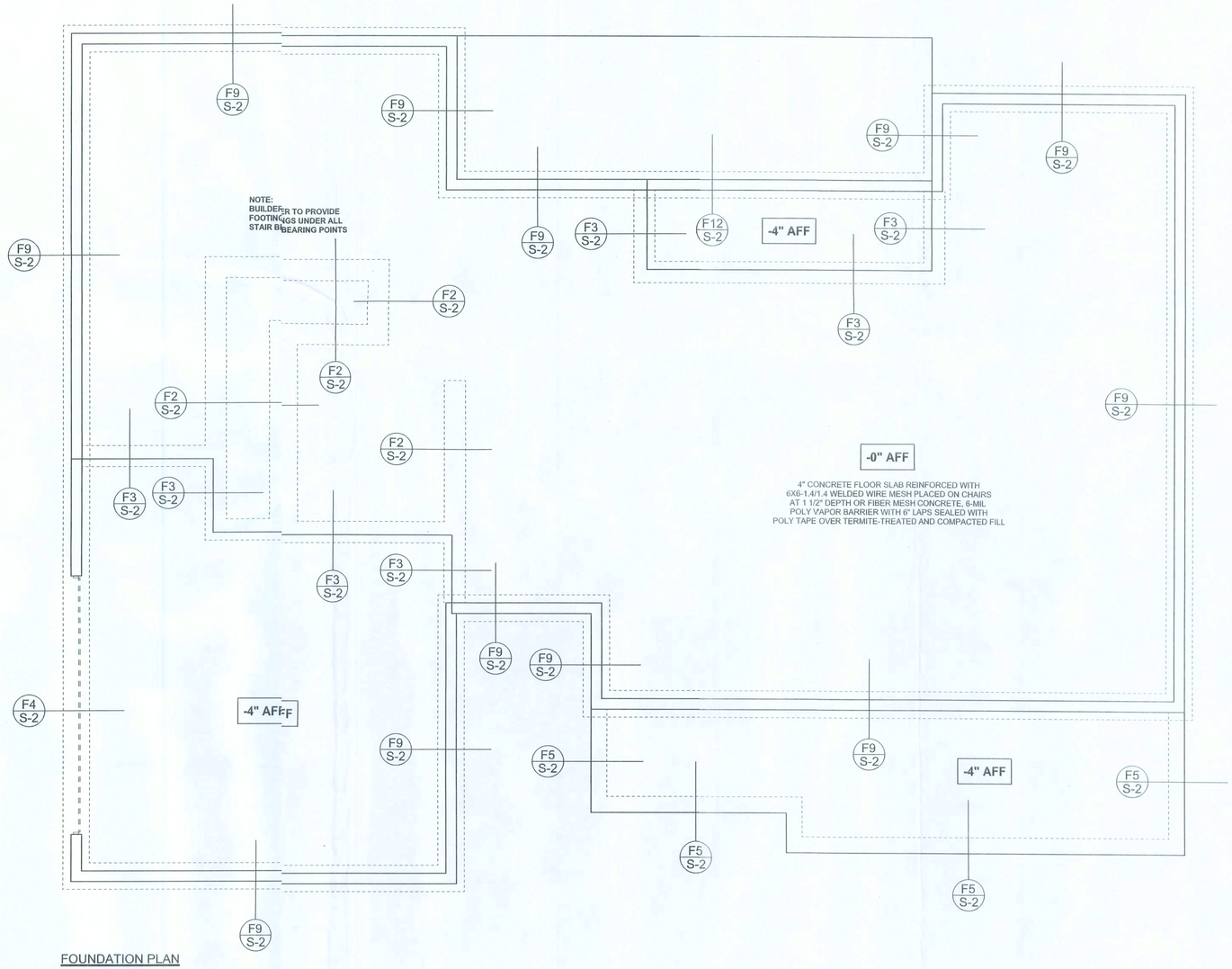


F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8"x CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER:
Mark Disosway, PE
No.53915, POB 868, Lee City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all qestions to Mark Disosway, P.E. fo resolution. Do not proceed without clarification.

CERTIFICATION: I herby certify that I have examined this plan, andhat the applicable portions of the plan, relating to wind engineering comp with section R301.2.1, florida buildrs code residential 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified loatio.

MARK DISOSWAY P.E. 5915

30Jun09

SEAL

Bryan Zecher Constraction

Shallar Residence

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Lake City Florida

Mark Disosway P.E.
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Lake City, Fbrida 32056
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Fax: (386) 69 - 4871

PRINTEI DATE: June 30, 2009

DRAWN BY: David Disosway

STRUCTURAL BY: David Disosway

FINALS DATE: 30Jun09

JOB NUMBER: 906251

DRAWINGNUMBER **S-2**

OF 3 SHEETS

STRUCTURAL PLAN NOES

SN-1 ALL LOAD BEARING FRAMWALL & PORCH HEADERS SHALL BE A MINIMUM OF (2)X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAMWALL HEADERS SHALL HAVE (1) JACK STUB (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTUAL SHEETS ARE NOT EXACT. REFER TARCHITECTURAL FLOOR PLAN FOR ACTUALIMENSIONS

SN-4 PERMANENT TRUSS BRACG IS TO BE INSTALLED AT LOCATIONS AS SHOWN OFHE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BRESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-I, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TJSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

THREADED ROD LEGEN

- INDICATES LOCAON OF: 1ST FLOOR 1/2" ,07 ALL THREADED ROD
- INDICATES LOCAON OF: 2ND FLOOR 1/2" ,07 ALL THREADED ROD

HEADER LEGEND

(2) 2X12X0',1J 1K ← HEADEREAM CALL-OUT (U.N.O.)

- NUMBER OF KG STUDS (FULL LENGTH)
- NUMBER OF JCK STUDS (UNDER HEADER)
- SPAN OF HEAER
- SIZE OF HEADR MATERIAL
- NUMBER OF PES IN HEADER

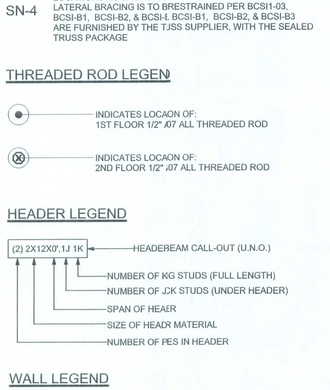
WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

TOTAL SHEAR WALL SEMENTS

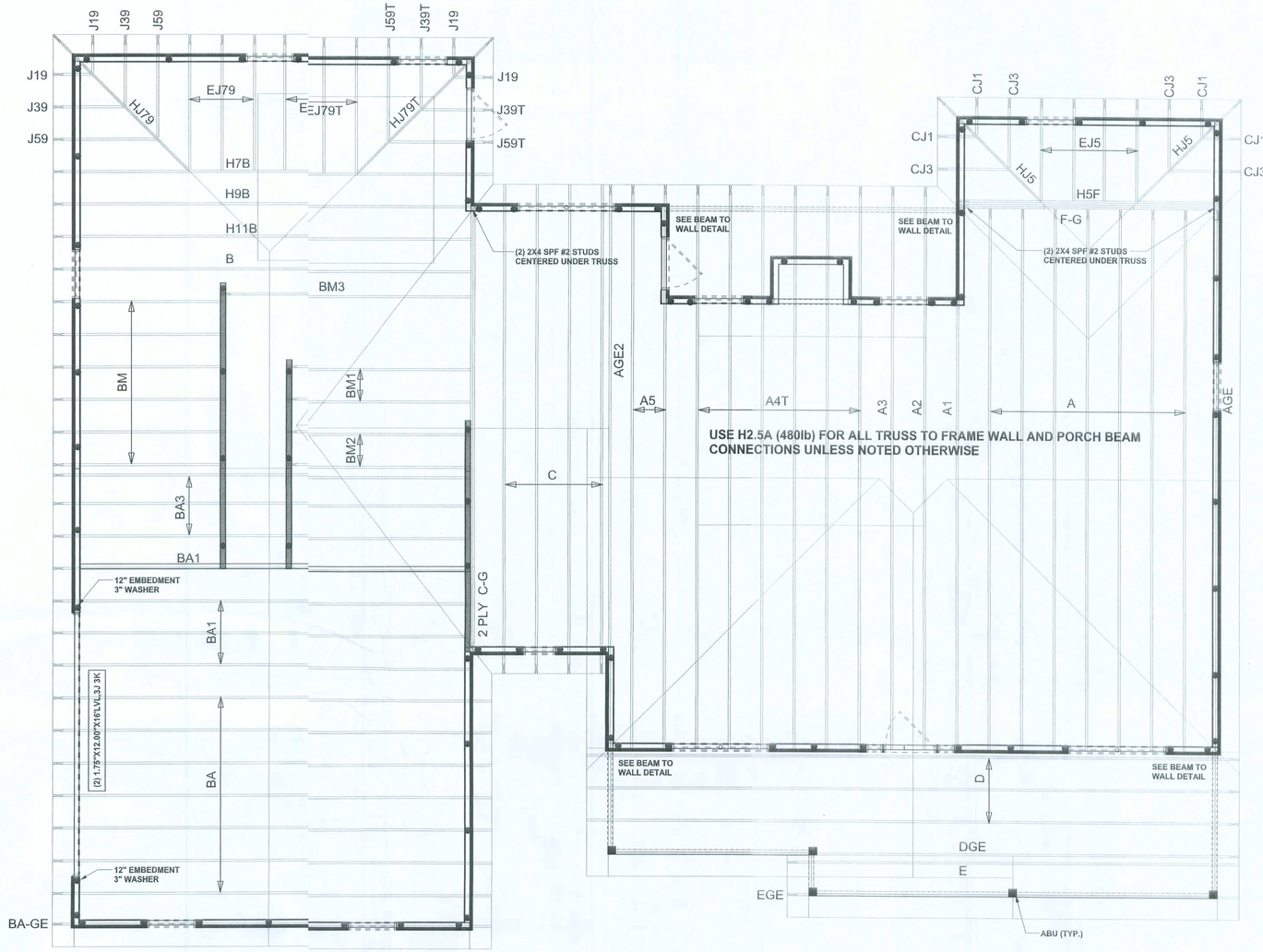
INDICATES SHEAR WALSEGMENTS

	REQUIRED	ATUAL
TRANSVERSE	42.0'	11.0'
LONGITUDINAL	38.4'	85'



W67 - BONUS ROOM GABLE END BRACING

SCALE: 1/2"=1'-0"



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO. JOB # 9-131

REVISIONS

SOFPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINER:
Mark Disoway, PE
No.53915, POB 868Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions apercede scaled dimensions. Refer a questions to Mark Disoway, P.E for resolution. Do not proceed without clarification.

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CERTIFICATION: I ereby certify that I have examined this plan, ind that the applicable portions of the plan, elating to wind engineering comply with section R301.2.1, florida building code residential 2007, to the best of my knwledge.

LIMITATION: This dsign is valid for one building, at specifiedlocation.

MARKDISOSWAY
P.I. 53915

EAL

Bryan Zecher Construction

ShallarResidence

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PRINED DATE: June 3t, 2009

DRAWN BY: David Disoway
STRUCTURAL BY: David Disoway

FINALS DATE: 30Jun09

JOB NUMBER: 906251

DRAWING NUMBER
S-3
OF 3SHEETS