

ELECTRICAL LEGEND CEILING FAN (PRE-WIRE FOR LIGHT KIT) 2×4 FLUORESCENT BATH EXAUST FA BATH EXAUST FAN DUPLEX OUTLET 220V OUTLET GFI DUPLEX OUTLET SMOKE DETECTOR WAY WALL SWITCH WATER PROOF GFI OUTLET PHONE JACK TELEVISION JACK

GARAGE DOOR OPENER

WALL HEATER

E-1 Wire all appliances, HVAC units and other equipment per manufactors specifications.

E-2 Consult the owner for the number of seperate telephone lines to be installed.

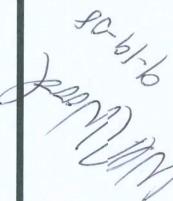
E-3 All installations shall be per national electric code.

E-4 All smoke detectors shall be 120V w/battery backup of the photoelectric type, and shall be interlocked together Install inside and near all bedrooms.

E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners direction and in accordance with applicable sections or national electrical code latest edition.

E-6 Electrical contractor shall be responsible for the design and sizing of electrical service and circuits.



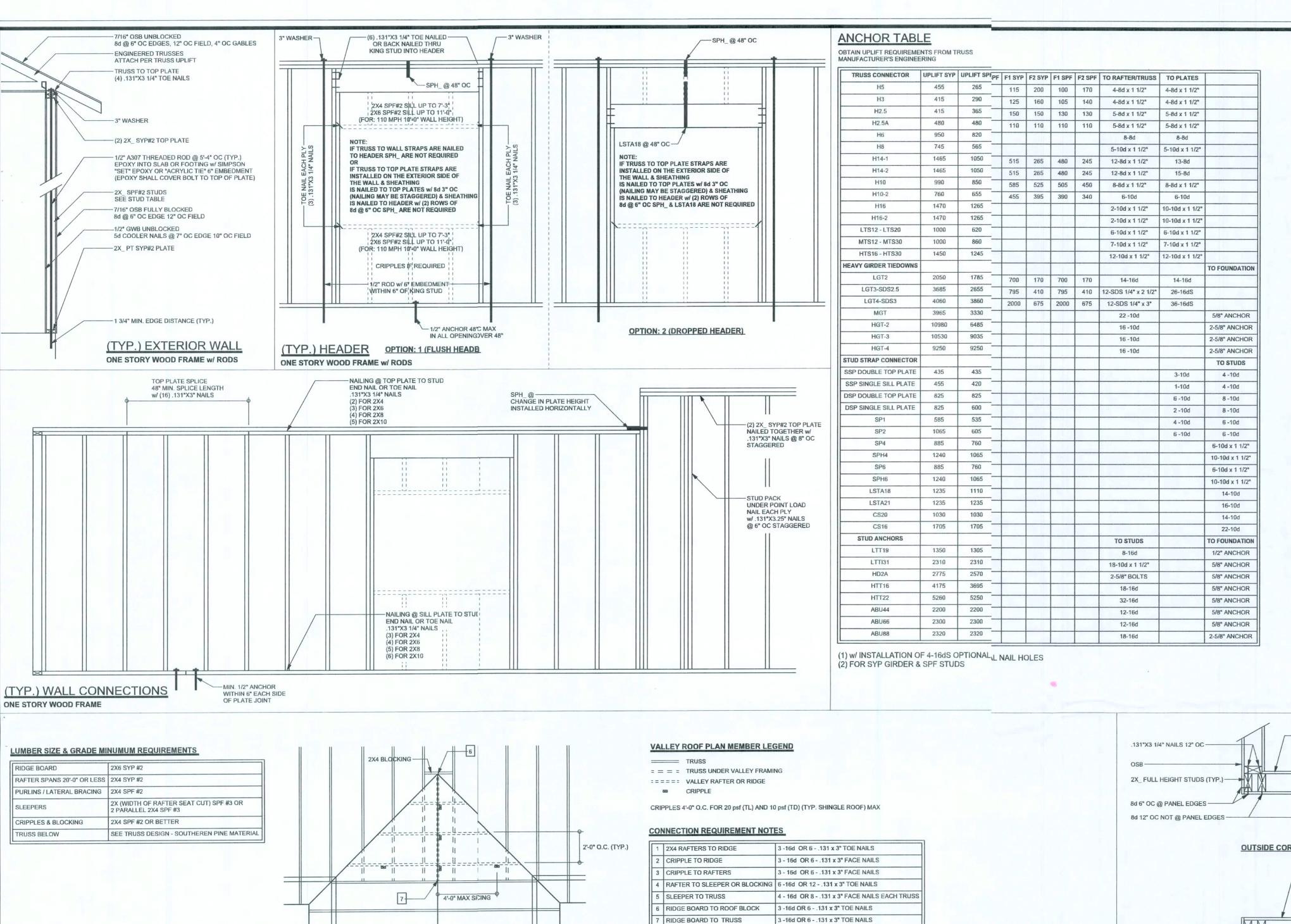


Drafted by: William G. Wood 386-365-1184

Corp

Addition Matthews / in Columbia C

SheetNumber



8 PURLIN TO TRUSS (TYP.)

TRUSS TO BLOCKING

CRIPPLE TO TRUSS

CRIPPLE TO PURLIN

GENERAL NOTES

MAXIMUM RAFTER SPANS

VALLEY ROOF PLAN

1/2" SHEATHING

RETROFIT ROOF OVER FRAMING & RACING DETAIL

\* ATTACMENT CAN BE MADE DIRECTLY

BY CUING A 2" x 4" NOTCH IN SHEATHING

BEVEL RAFTER CUT AS REQ'D FOR PITCH

OR THOUGH PLYWOOD SHEATHING

(NOT RE(IRED IF SLEEPERS ARE USED)

PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)

6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.

MINIMUM OF 8 - 8d COMMON WIRE NAILS.

-MAXIMUM WIND SPEED: 120 MPH

INSTALLED INTO RIDGE BOARD

**ENCLOSED BUILDING** 

MAXIMUM TOTAL LOADING: 40 psf

- MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS

MAXIMUM MEAN ROOF HEIGHT: 30 FEET

EXPOSURE CATEGORY "B", I = 1.0, Kzt = 1.0

CRIPPLE, BRACING, & BLOCKING NOTES

AS LONG AS THE PROPER NUMBER OF NAILS ARE

-SPANS (DISTANCS BETWEEN HEELS) 40'-0" OR LESS

MEETS FBC 2001/ASCE 7-98 WIND REQUIREMENTS

GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER,

- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED. - INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN

LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED,

16ft2 IN ZONES 2 & 3 , 24ft2 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN

OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A

THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:

-2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG

NAILED w/ 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE

OVER 10'-0" LONG REQURE TWO CLB's OR BOTH FACES w/ "T" OR SCAB. USE STRESS

- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE

WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES

PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED. PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM. IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM

MAXIMUM ROOF AREA PER SUPPORT

CS20 RIDGE TENSION STRAP w/ 8 - 8d

-2X4 VALLEY RAFTER

— SEE CRIPPLE, BRACING

& BLOCKING NOTES

4'-0" MAX SPACING

**EXISTING ROOF-**

FRAMING 2x SYP @ 24" O.C

2X6 -

RIDGE

BOARD

OR 2X4 COLLAR TIE 3 - 16d OR 4 - .131 x3"

4'-0" (TYP.)

2X4 PURLIN 24" O.C.

SECTION CUT PARALLEL TO VALLEY RAFTER

(WHERE NO SHEATHING IS APPLIED)

-16d OR 6 - .131 x 3" NAILS

-16d OR 8 - .131 x 3" NAILS

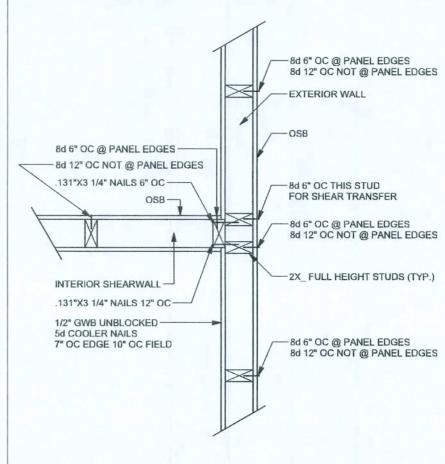
-16d OR 6 - .131 x 3" END NAILS

-16d OR 6 - .131 x 3" FACE NAILS

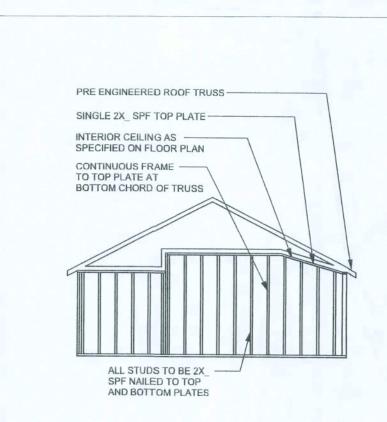
3 -16d OR 6 - .131 x 3" FACE NAILS

# ------1/2" GWB LINBLOCKED 5d COOLER NAILS 7" OC EDGE 10" OC FIELD **OUTSIDE CORNER** - 1/2" GWB UNBLOCKED 7" OC EDGE 10" OC FIELD 2X\_FULL HEIGHT STUDS (TYP.) -.131"X3 1/4" NAILS 12" OC --8d 6" OC @ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES -

#### INSIDE CORNER (TYP.) CORNER FRAMING WOOD FRAME



(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



**EXTERIOR WALL STUD TABLE** 

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B,

EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B.

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

**GRADE & SPECIES TABLE** 

SYP #2

SYP #2

SYP #2

24F-V3 SP

MICROLAM

PARALAM

LSL TIMBERSTRAND

2x10

2x12

GLB

Fb (psi) | E (10<sup>6</sup> psi)

1.6

1.6

1.6

1.8

1.7

2.0

2.0

1200

1050

975

2400

2900

2900

TO 13'-0" STUD HEIGHT

TO 18'-10' STUD HEIGHT

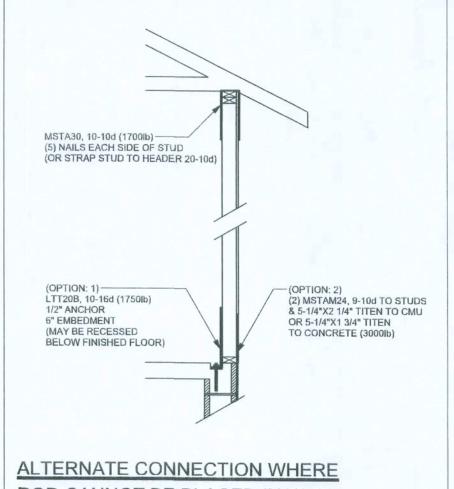
FOR SPF #2 STUDS

(1) 2x4 @ 12" OC

(1) 2x6 @ 16" OC

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL WOOD FRAME



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTION BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS

WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END. SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

(25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O. GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING. UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS. ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT
AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNG

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES

#### **BUILDER'S RESPONSIBILITY**

DIAPHRAGM BOUNDARY: 4"OC. UNO

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND **DESIGN PRESSURES** PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRE LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.

### DESIGN DATA



12 PSF (12:12 AND GREATER)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY

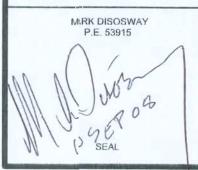
ALLOWABLE UPLIFT:

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

REVISIONS

NDI OAD ENGINEER: Mark Disosway No.53915, FDB 868, Lake City, FL MENSIONS: ated dimensions supercede scaled nensions. Retr all questions to Mark Disosway P. E. for resolution not proceed vithout clarification. PYRIGHTS IND PROPERTY RIGHTS: se instrumens of service. This document is t to be reprodiced, altered or copied in any rm or manner vithout first the express written rmission and onsent of Mark Disosway. CERTIFICATIOI: I hereby certify that I have amined this pan, and that the applicable rtions of the pan, relating to wind engineer comply with secon R301.2.1, florida building code residential2004, to the best of my owledge. IMITATION: The design is valid for one

building, at spedied location.



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Matthews Addition

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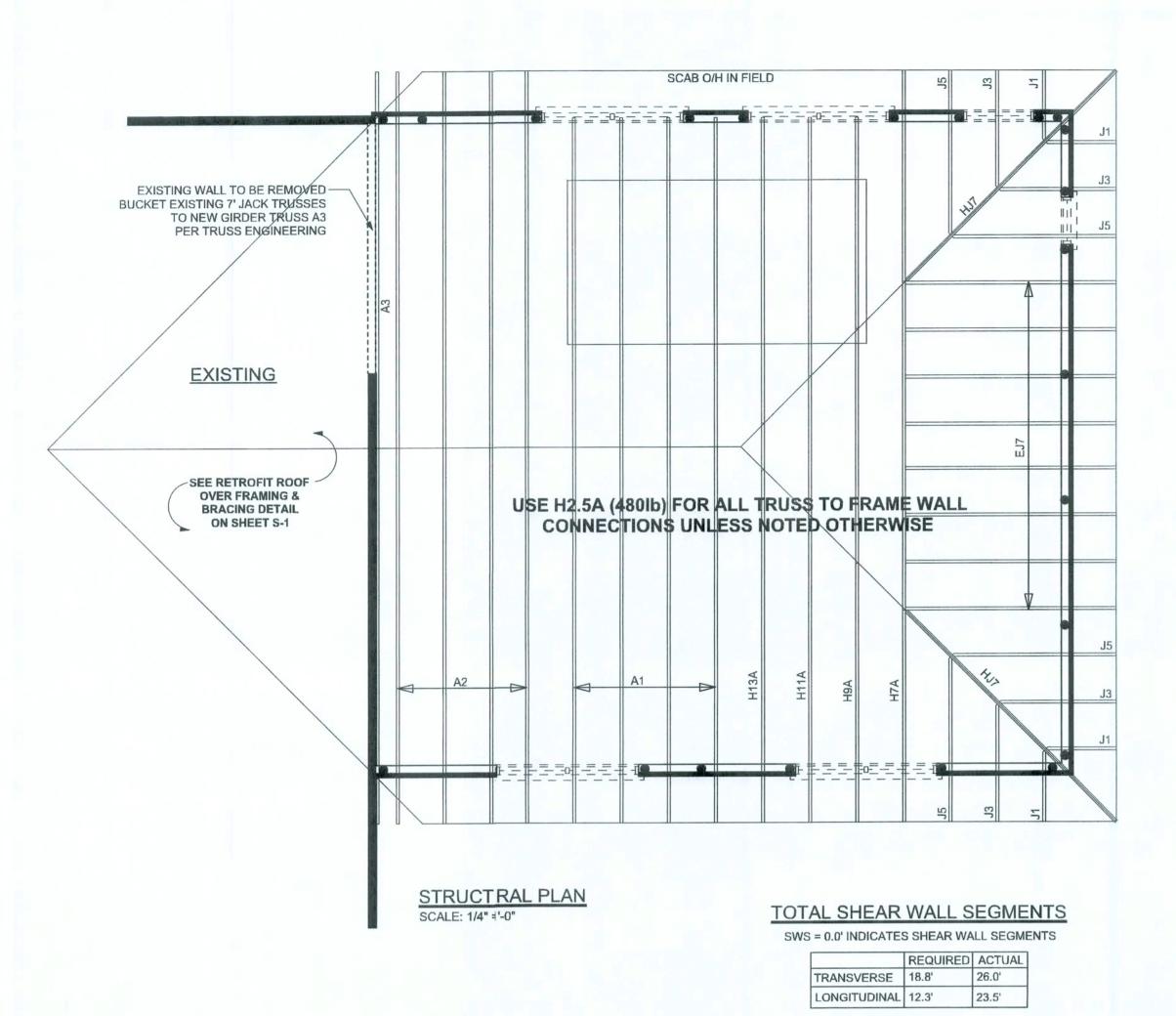
September 10, 2008 STRUCTURAL BY DRAWN BY David Disosway

FINALS DATE: Sept. 10, 2008

> JOE NUMBER 808151 DRAVING NUMBER

> > S-1 CF 2 SHEETS

ROD CANNOT BE PLACED IN WAL ONE STORY WOOD FRAME w/ RODS



#### STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL &ORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 S<sup>2</sup>#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL FADERS SHALL HAVE (1) JACK STUD & (1) KG STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHETS
  ARE NOT EXACT. REFER TO ARCHI:CTURAL
  FLOOR PLAN FOR ACTUAL DIMENSNS

SN-4

PERMANENT TRUSS BRACING IS TGE INSTALLED AT LOCATIONS AS SHOWN ON THE SE.ED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTFINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B4, BCSI-B5, BCSI-B

CONNECTIONS, WALL, & HEADER DESIN IS BASED ON REACTIONS & UPLIFTS FROM TRUSENGINEERING FURNISHED BY BUILDER. ANDERSON JUSS JOB # 8-221

#### THREADED ROD LEGEND

INDICATES LOCATION OF:
1ST FLOOR 1/2" A307 ALL THREADED ROD

INDICATES LOCATION OF:
2ND FLOOR 1/2" A307 ALL THREADED ROD

#### HEADER LEGEND

(2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.)

NUMBER OF KING STUDS (FULL LENGTH)

NUMBER OF JACK STUDS (UNDER HEADER)

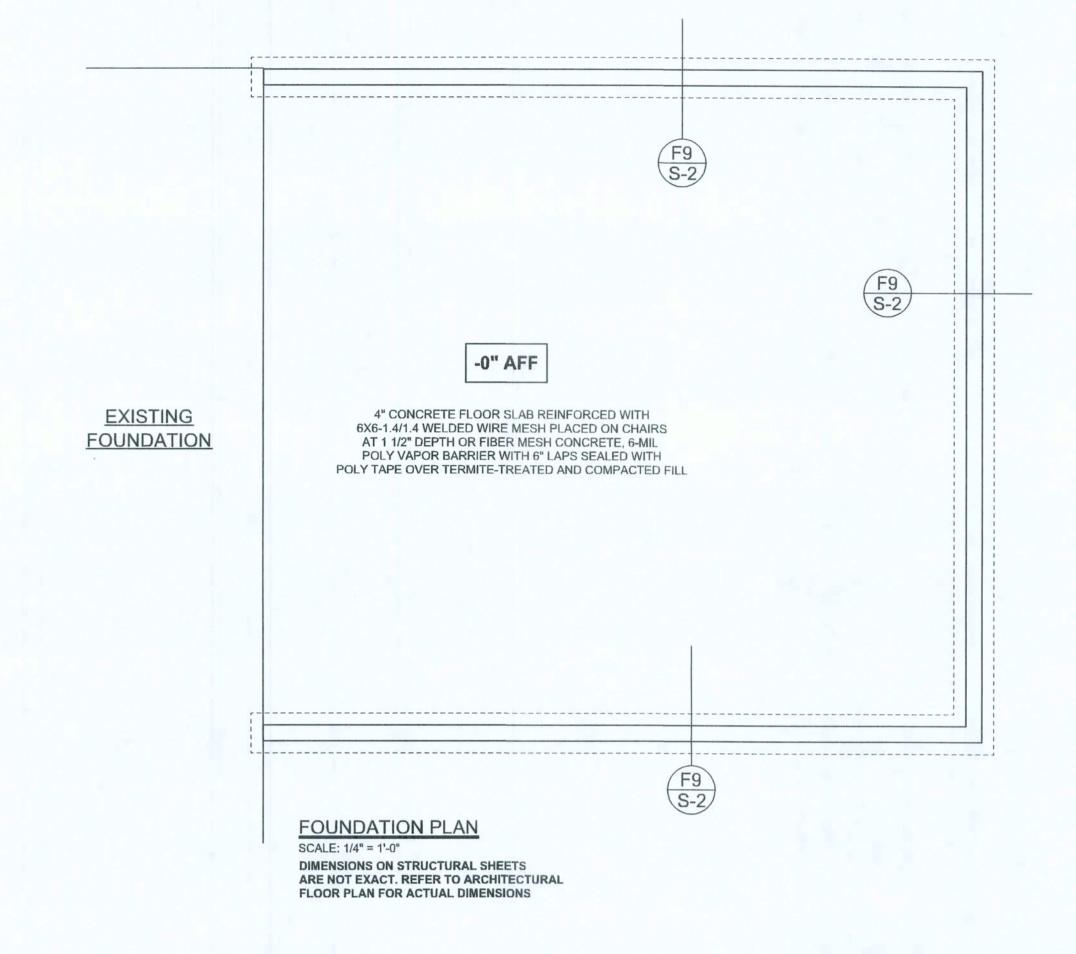
SPAN OF HEADER

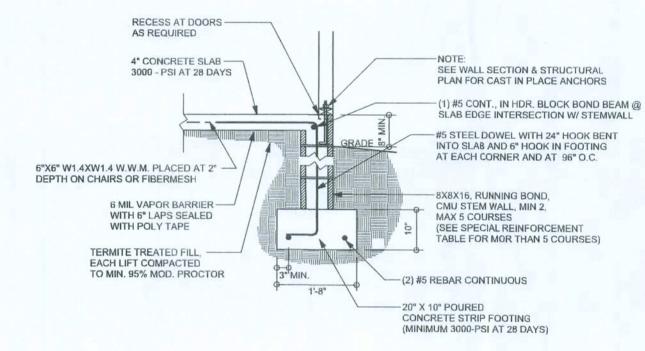
SIZE OF HEADER MATERIAL

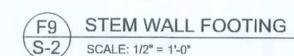
NUMBER OF PLIES IN HEADER

#### WALL LEGEND

(TERIOR WALL
TERIOR NON-LOAD BEARING WALL
TERIOR LOAD BEARING WALL w/ NO UPLIFT
TERIOR LOAD BEARING WALL w/ UPLIFT







#### TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL UNBALANCED HEIGHT BACKFILL (FEET) HEIGHT	BACKFILL	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
	#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48
			1				



REVISONS

SOTTPIAN

WINDLOAD ENGNEER: Mark Disosway, PE No.53915, PCB 868, Lake City, FL 32056, 386-754-4419

DIMENSIONS: Stated dimension supercede scaled dimensions. Refe all questions to Mark Disosway, I.E. for resolution. Do not proceed wthout clarification.

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CERTIFICATIONI hereby certify that I have examined this pla, and that the applicable portions of the pla, relating to wind engineering comply with sectin R301.2.1, florida building code residential 204, to the best of my knowledge.

LIMITATION: Thi design is valid for one building, at specied location.

MARK DISOSWAY

P.E. 53915

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PIINTED DATE:
Sepember 10, 2008

DRAWN BY: STRUCTURAL BY
David Disosway

FINALS DAT:: Sept. 10, 20)8

> JOBNUMBER: 808151 DRAVING NUMBER

> > **5-2** OF 2 SHEETS