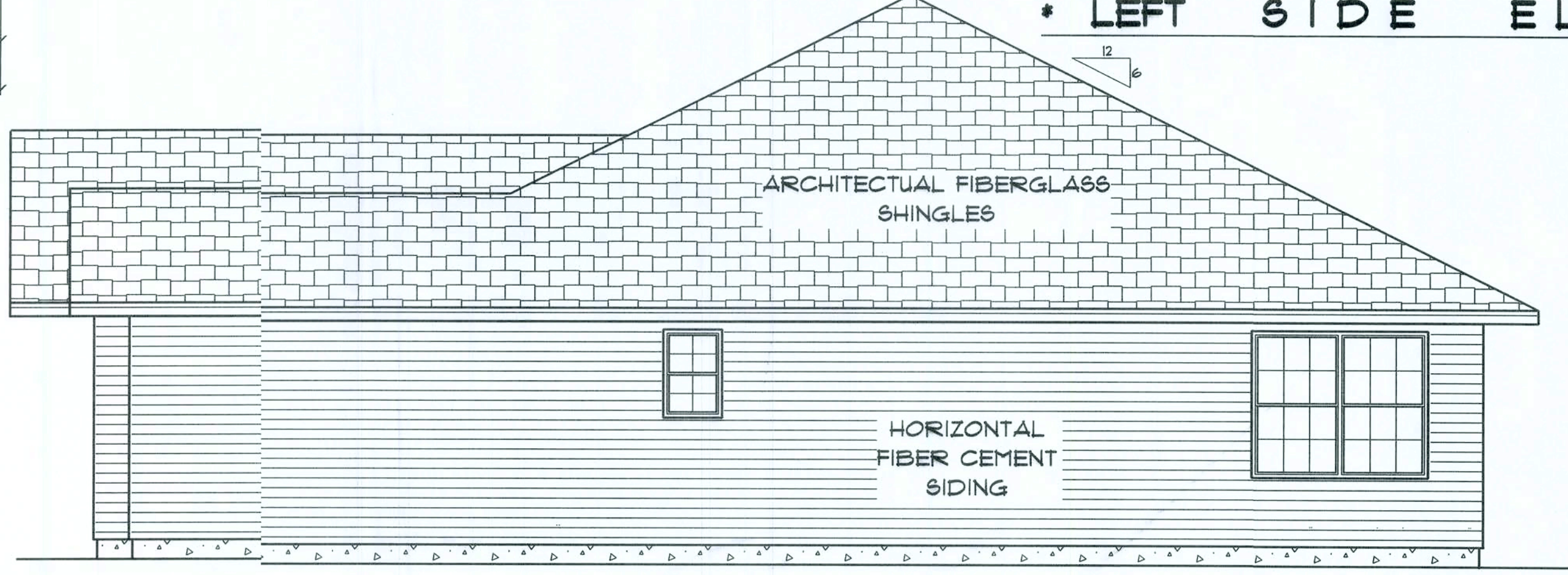
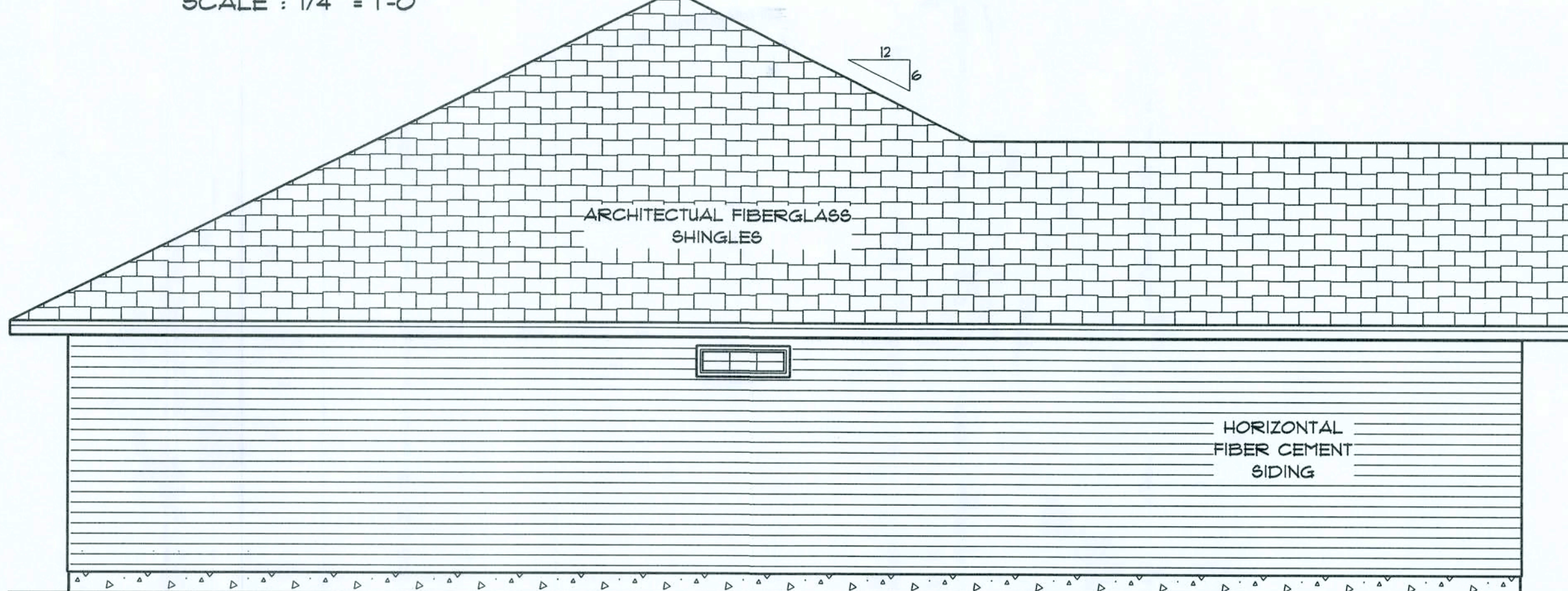
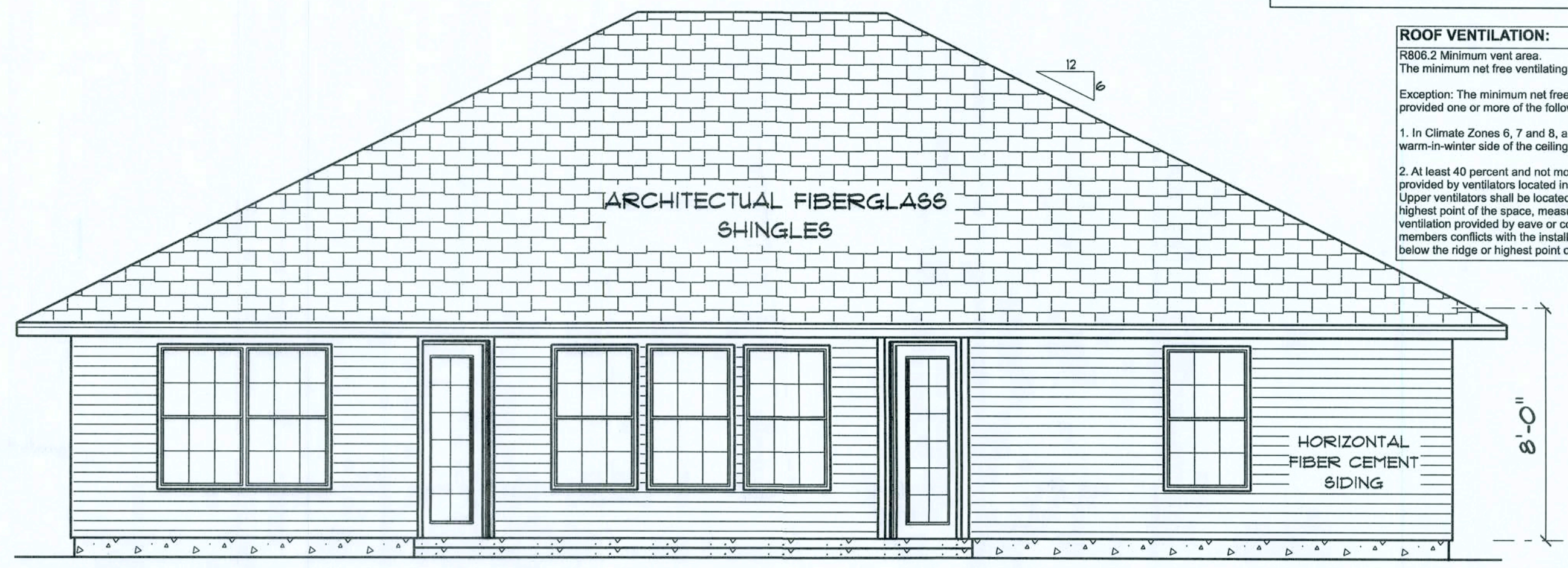
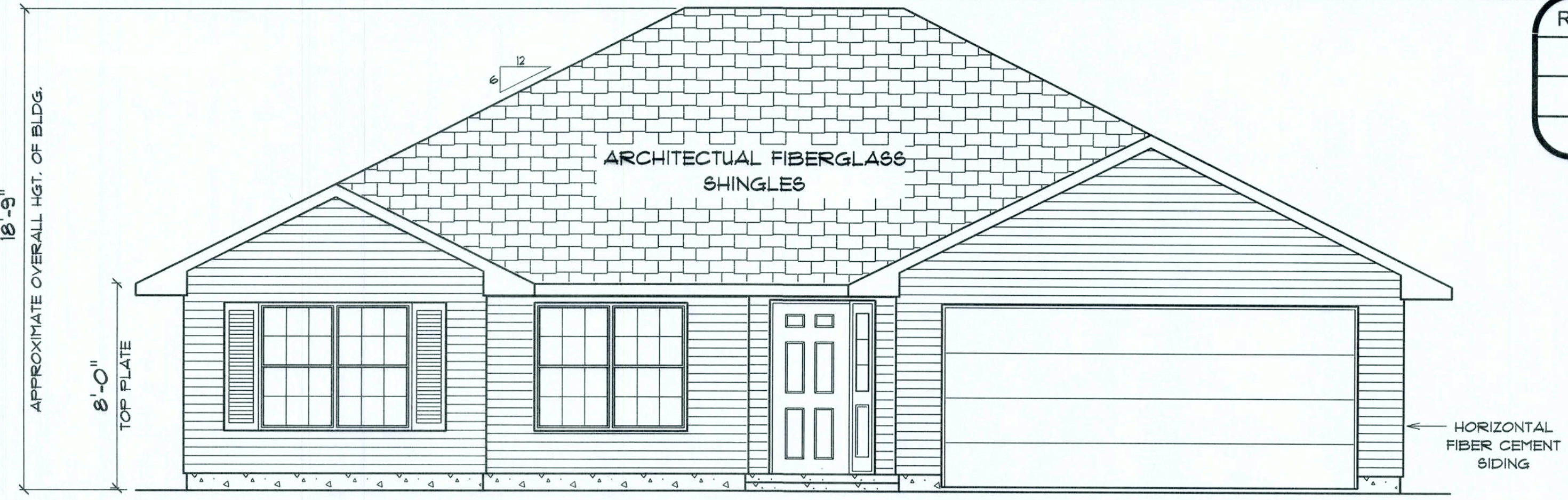


* FLOOR PLAN *

R302.5.1 Opening protection:
Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches in thickness, solid or honeycomb-core steel doors not less than 1 3/8 inches thick, or 20-minute fire-rated doors, equipped with a self-closing device.

SEPARATION	MATERIAL
From the residence and attic	Not less than 1/2-inch gypsum board or equivalent applied to the garage side
From all habitable rooms above the garage	Not less than 5/8-inch Type X gypsum board or equivalent
Structural(s) supporting floor/ceiling assemblies used for separation required by this section	Not less than 1/2-inch gypsum board or equivalent
Garages located less than 3 feet from a dwelling unit on the same lot	Not less than 1/2-inch gypsum board or equivalent applied to the interior side of exterior walls that are within this area

A/C LIVING AREA = 1500 S.F.
 FRONT ENTRY = 44 S.F.
 2-CAR GARAGE & MECH.CL. = 430 S.F.
 REAR PORCH (UNDER ROOF) = 100 S.F.
TOTAL AREA UNDER ROOF = 2074 S.F.



REVISIONS

SOFTPLAN
ARCHITECTURAL DRAIN SOFTWARE

ROOF VENTILATION:
 R806.2 Minimum vent area.
 The minimum net free ventilating area shall be 1/150 of the area of the vented space.
 Exception: The minimum net free ventilating area shall be 1/300 of the vented space provided one or more of the following conditions are met:
 1. In Climate Zones 6, 7 and 8, a Class 1 or II vapor retarder is installed on the warm-in-winter side of the ceiling.
 2. At least 40 percent and not more than 50 percent of the required ventilating area is provided by ventilators located in the upper portion of the attic or rafter space. Upper ventilators shall be located no more than 3 feet below the ridge or highest point of the space, measured vertically, with the balance of the required ventilation provided by soffit or cornice vents. Where the location of soffit or cornice vents conflicts with the installation of upper ventilators, installation shall be permitted.

1500 Model
- Right -

Lot 10 Haight Ashbury
Columbia County, FL

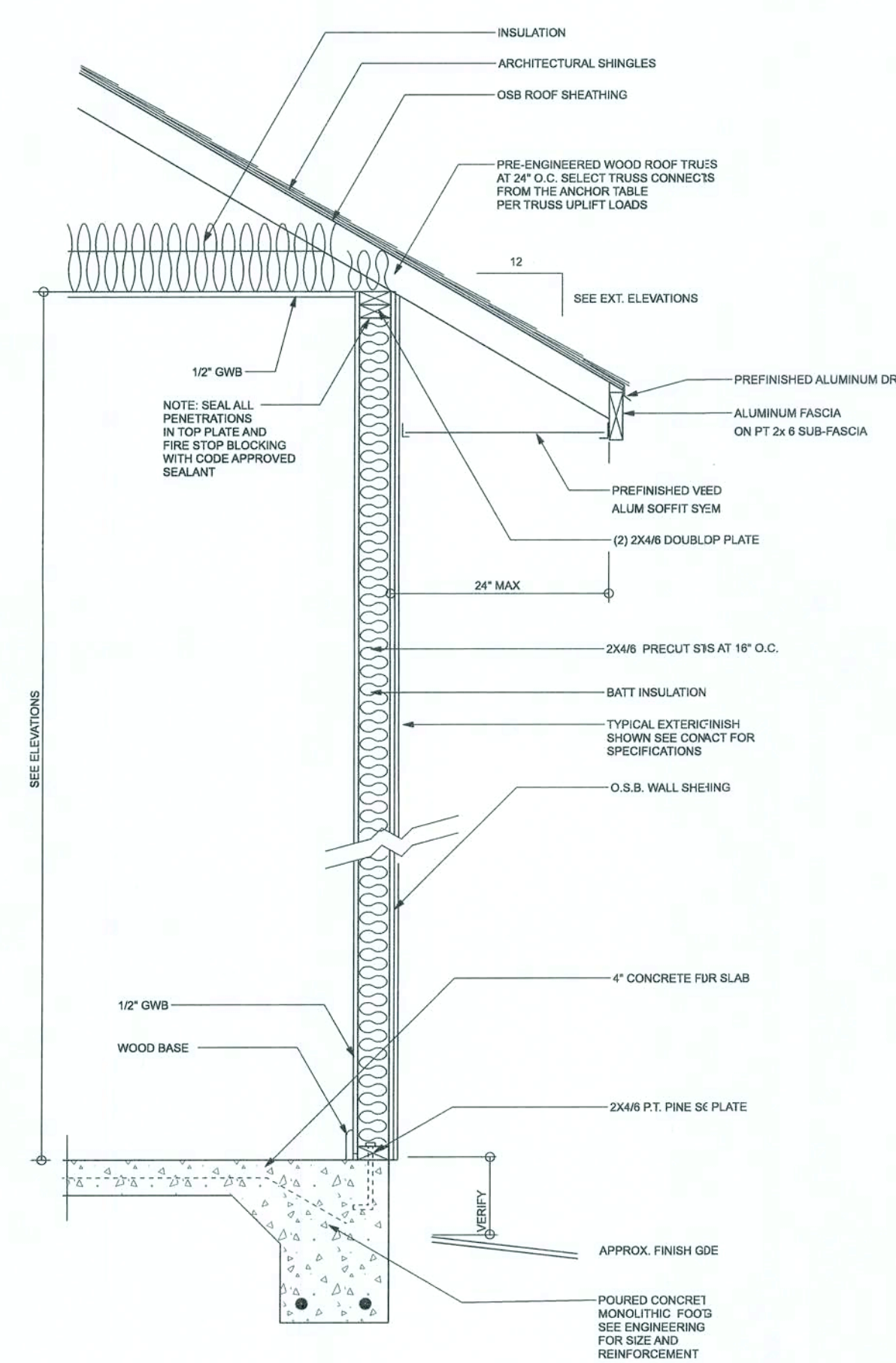
PRINTED DATE:
Friday, September 4, 2020

DRAWING NUMBER
A-1
OF 2 SHEETS



REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

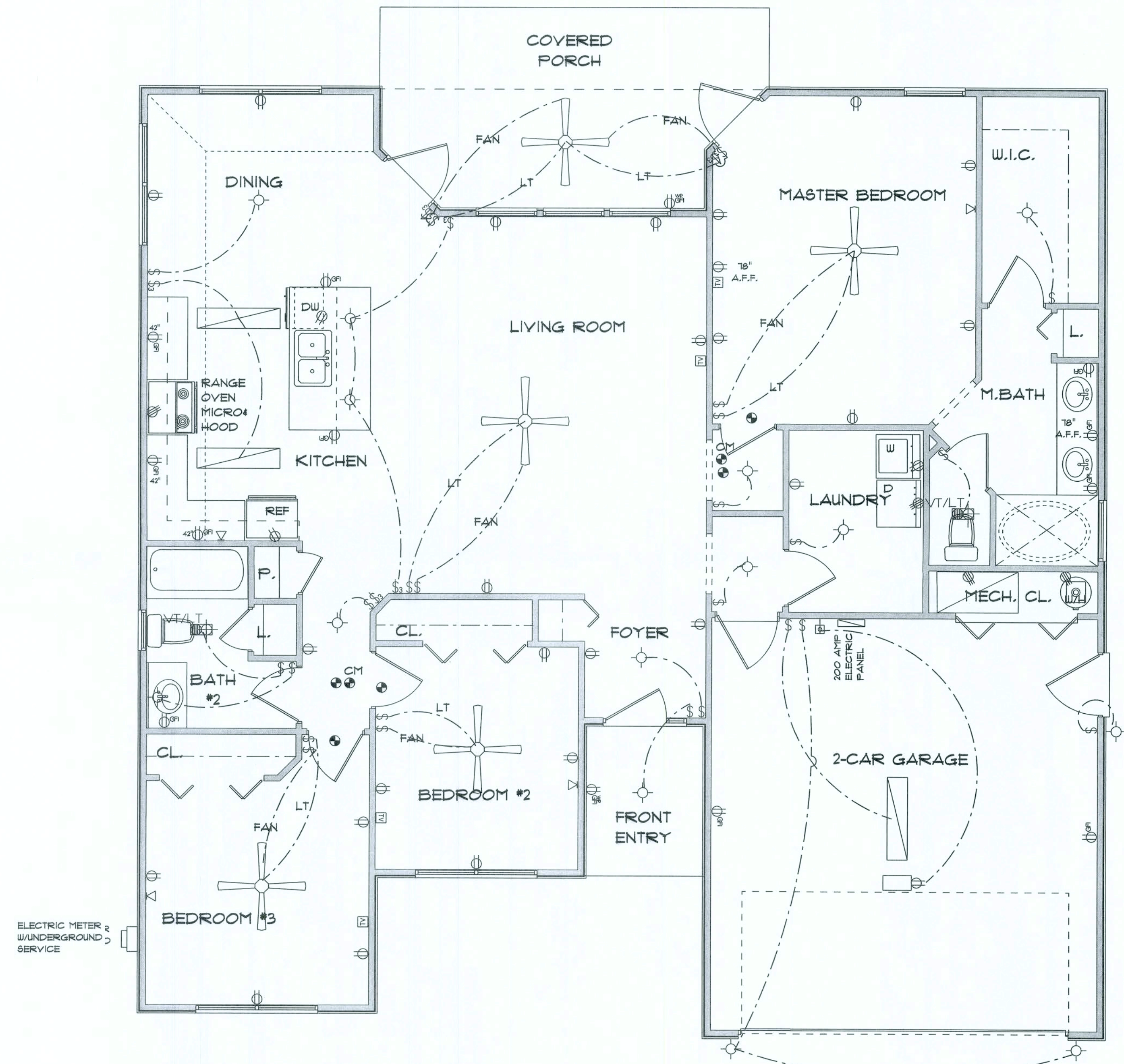


TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA

SCALE: 3/4\" = 1'-0"

ELECTRICAL	COUNT	SYMBOL
ceiling fan	3	
200 amp service panel	1	
Light	2	
automatic garage dr opener	1	
cable tv outlet	4	
ceiling fan	2	
electric meter	1	
fluorescent lt fixture	3	
garage dr opener	1	
gfi waterproof outlet	2	
light	10	
outlet	29	
outlet 220v	4	
outlet gfi	10	
smoke detector	5	
switch	22	
switch 3 way	6	
telephone	4	
vent light combo	2	

ELECTRICAL PLAN NOTES:	
E - 1	WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
E - 2	CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
E - 3	ALL INSTALLATIONS SHALL BE PER NATL. ELECTRIC CODE.
E - 4	ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
E - 5	TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
E - 6	ELECTRICAL CONTRL SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
E - 7	ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
E - 8	ALL 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENs, BEDROOMS, SUN ROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, OR SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER, COMBINATION-TYPE INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT.
E - 9	ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION.
E - 10	A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING.
E - 11	SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.
E - 12	CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.
E - 13	ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER-RESISTANT PER NEC.



* ELECTRICAL PLAN *

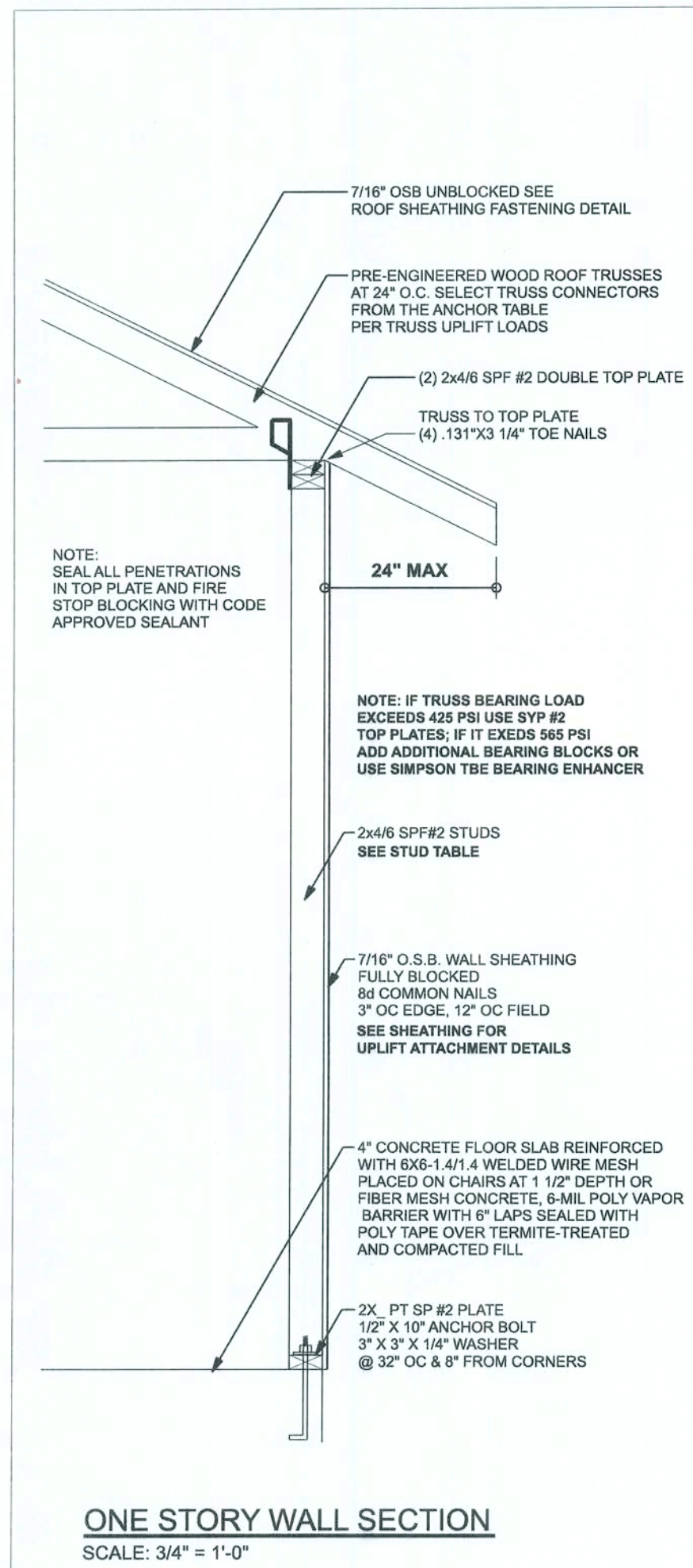
SCALE : 1/4" = 1'-0"

1500 Model
- Right -

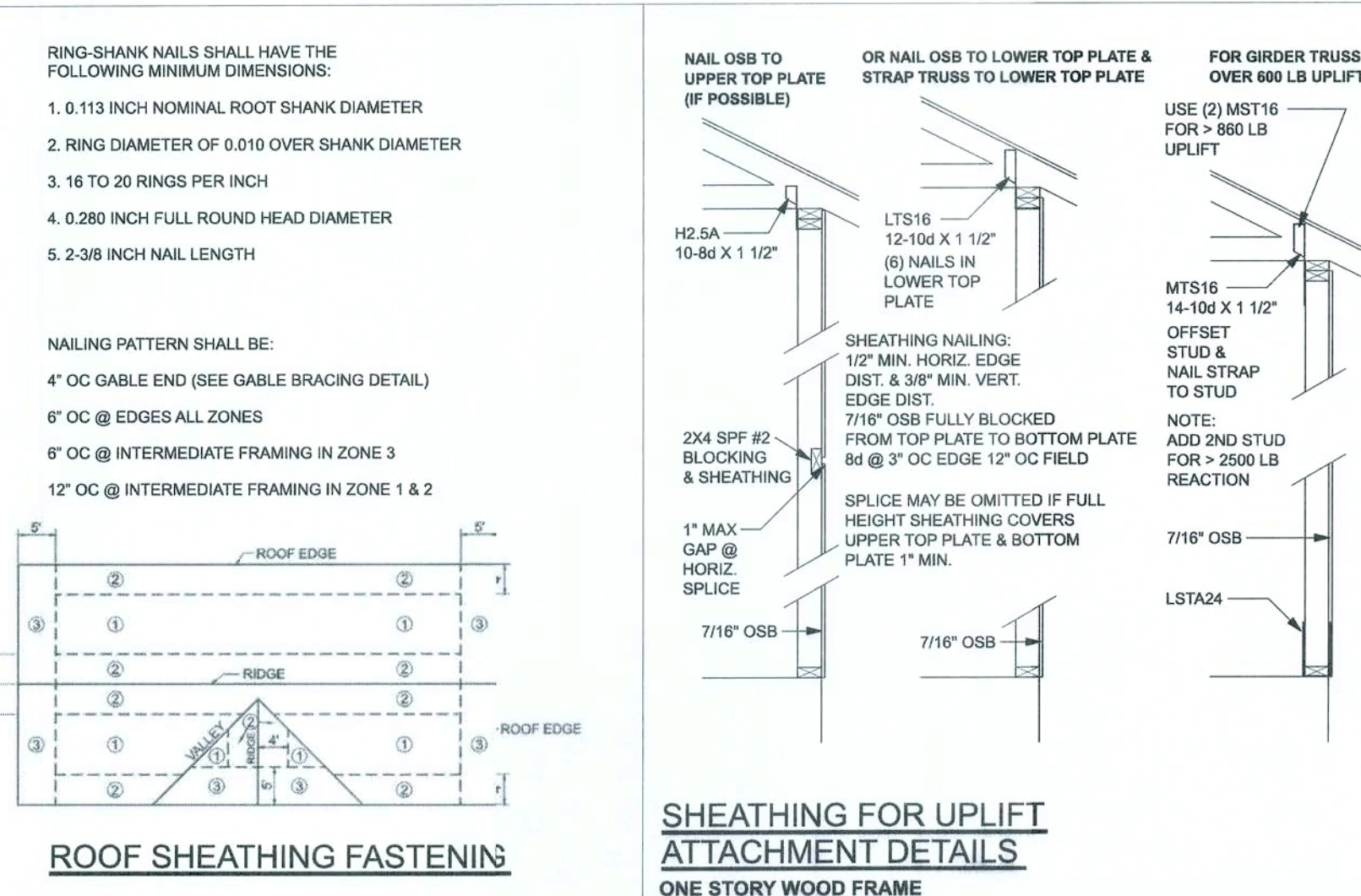
Lot 10 Haight Ashbury
Columbia County, FL

PRINTED DATE:
Friday, September 4, 2020

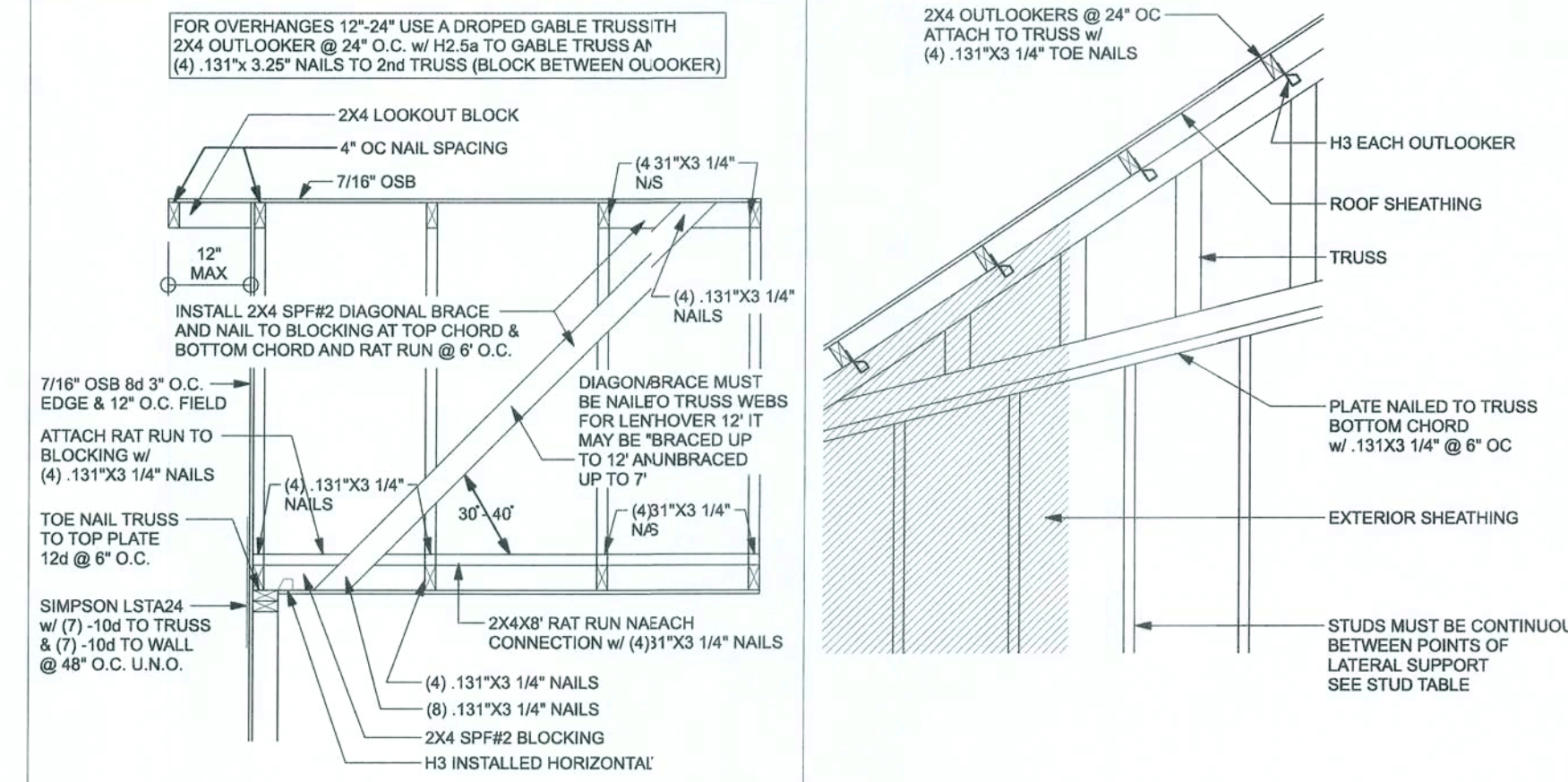
DRAWING NUMBER
A-2
OF 2 SHEETS



ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

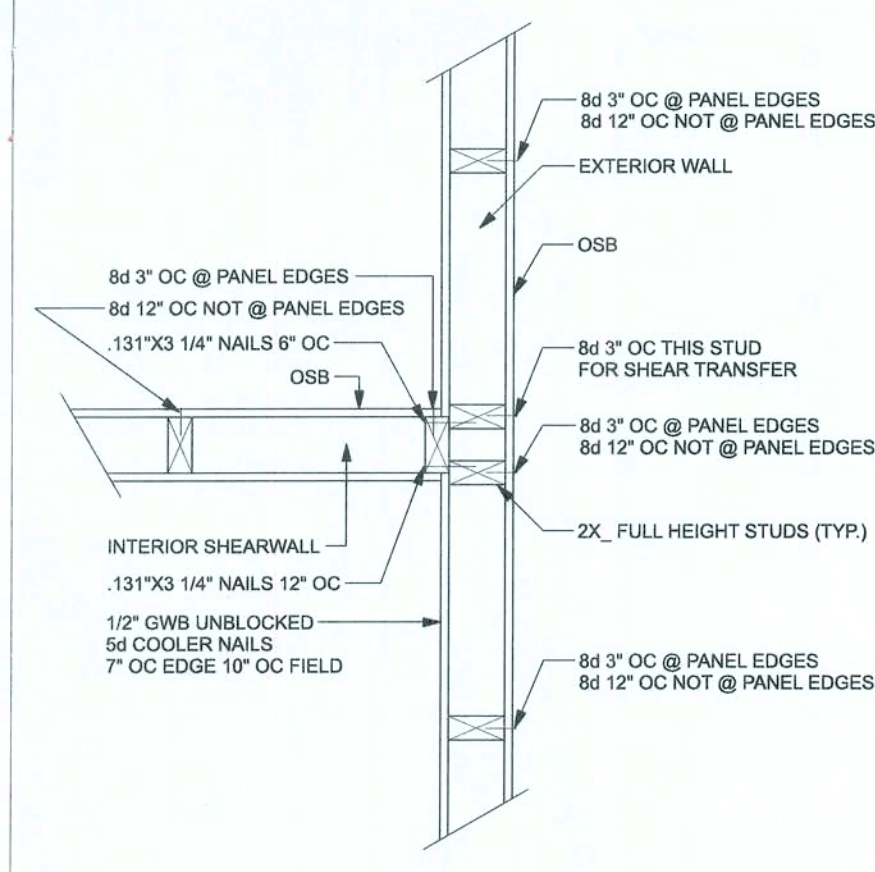


ROOF SHEATHING FASTENING



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL
WOOD FRAME

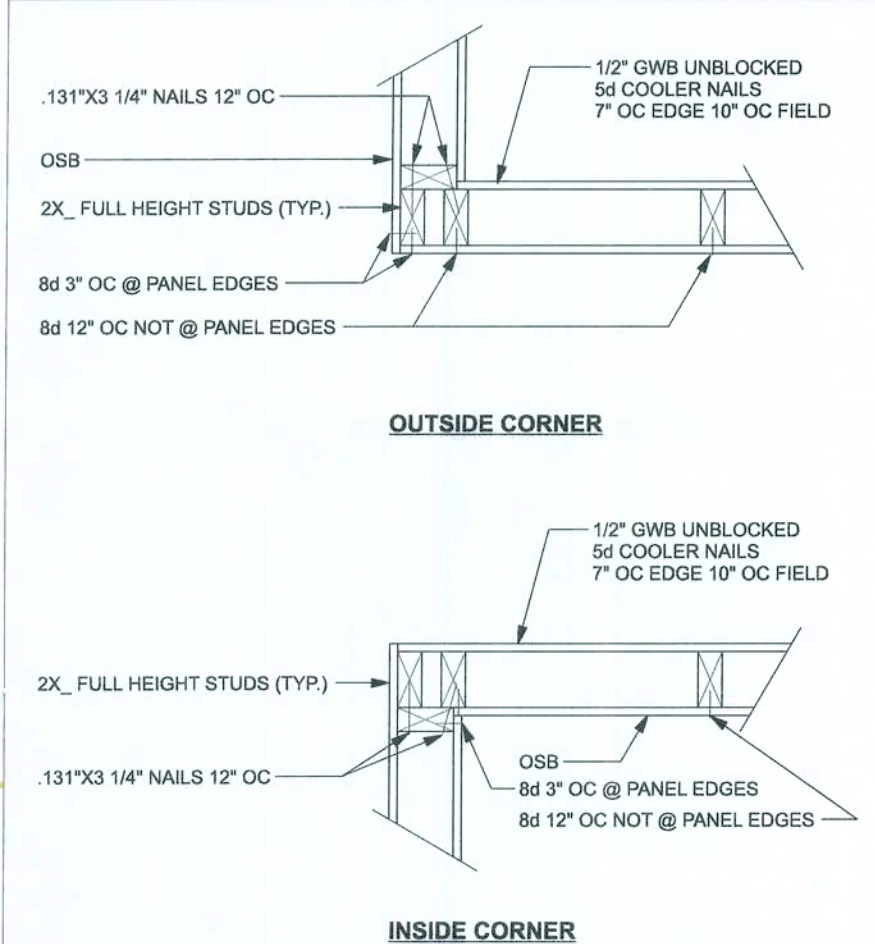


(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

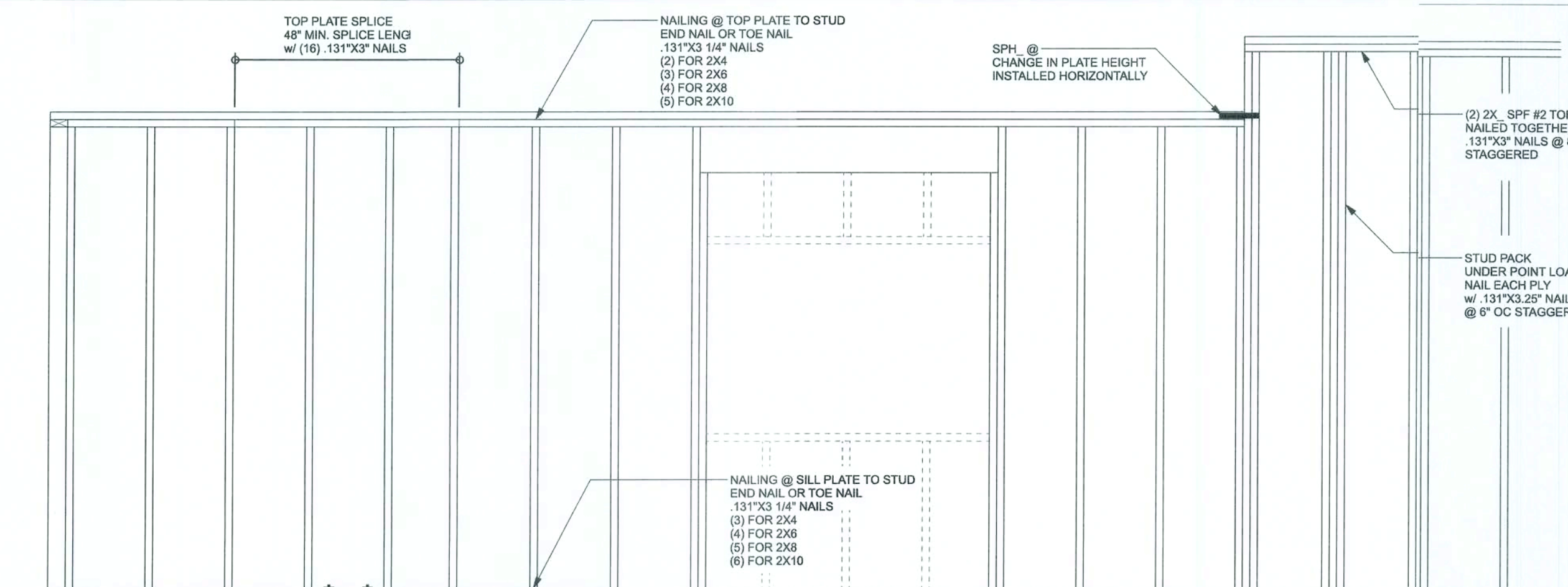
TYPICAL HEADER STRAPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

HEADER STRAP TABLE		
Uplift	Top Connection	Bottom Connection
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
< 1455	MSTA24, 18-10d header to jacks	DTT22
< 1800	MSTA24, 18-10d header to jacks	DTT22
< 2910	MSTA24, 18-10d header to jacks	HTT4

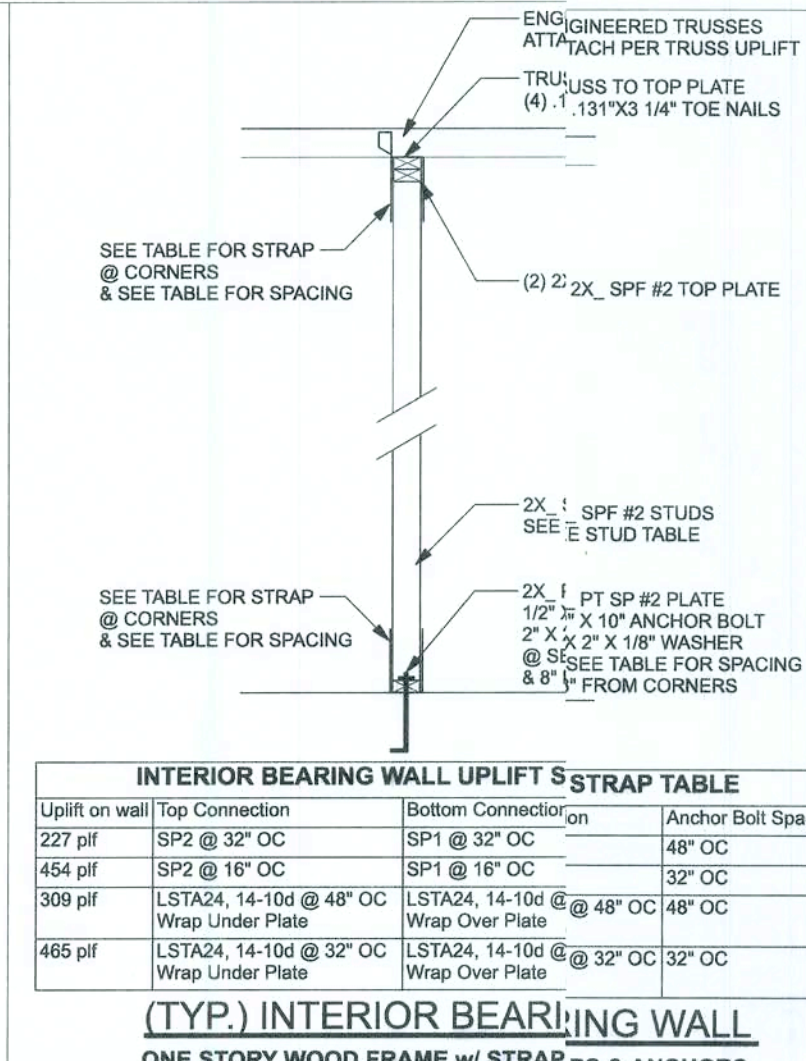
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT			
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2.9	
	(1) 2x4	(2) 2x4	(2) 2x6
130 MPH EXP. C	5'-3"	7'-6"	11'-3"



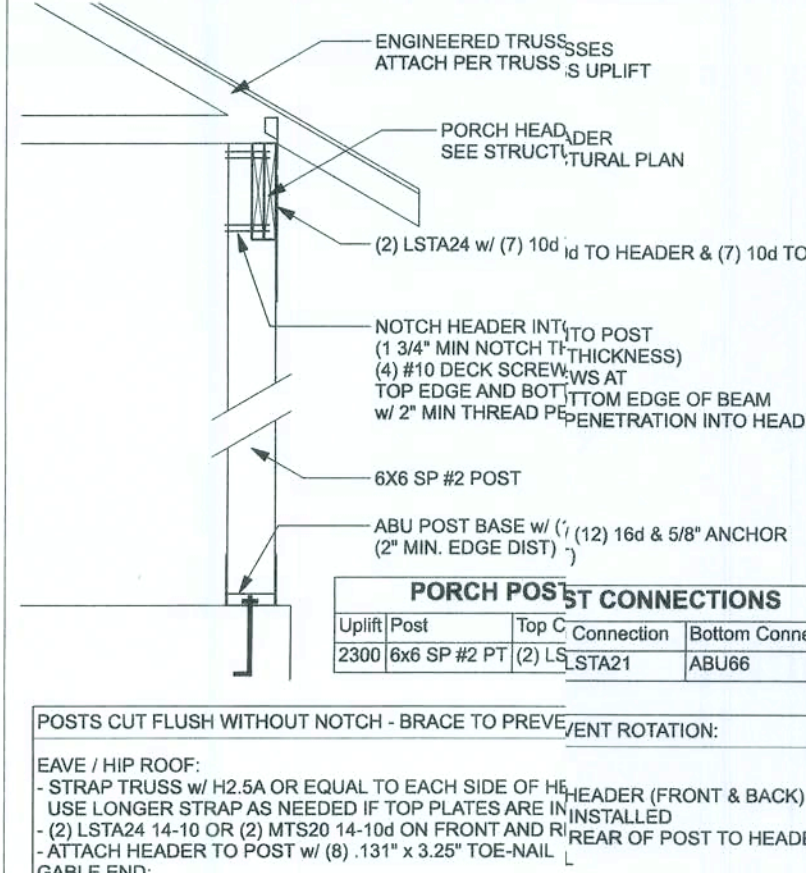
(TYP.) CORNER FRAMING
WOOD FRAME



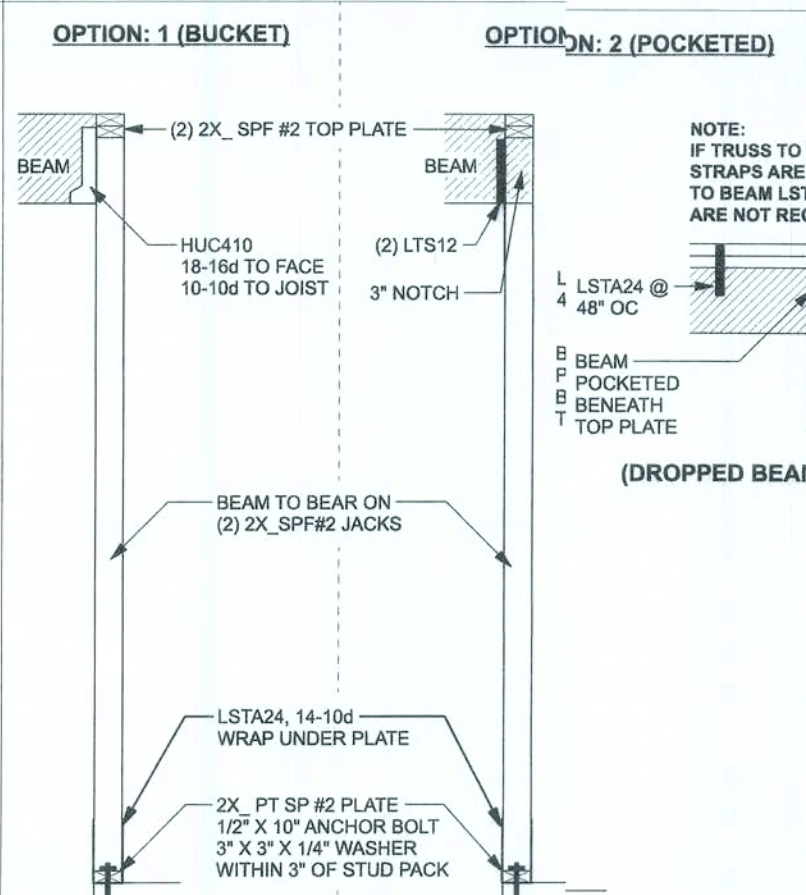
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



INTERIOR BEARING WALL UPLIFT STRAP TABLE
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

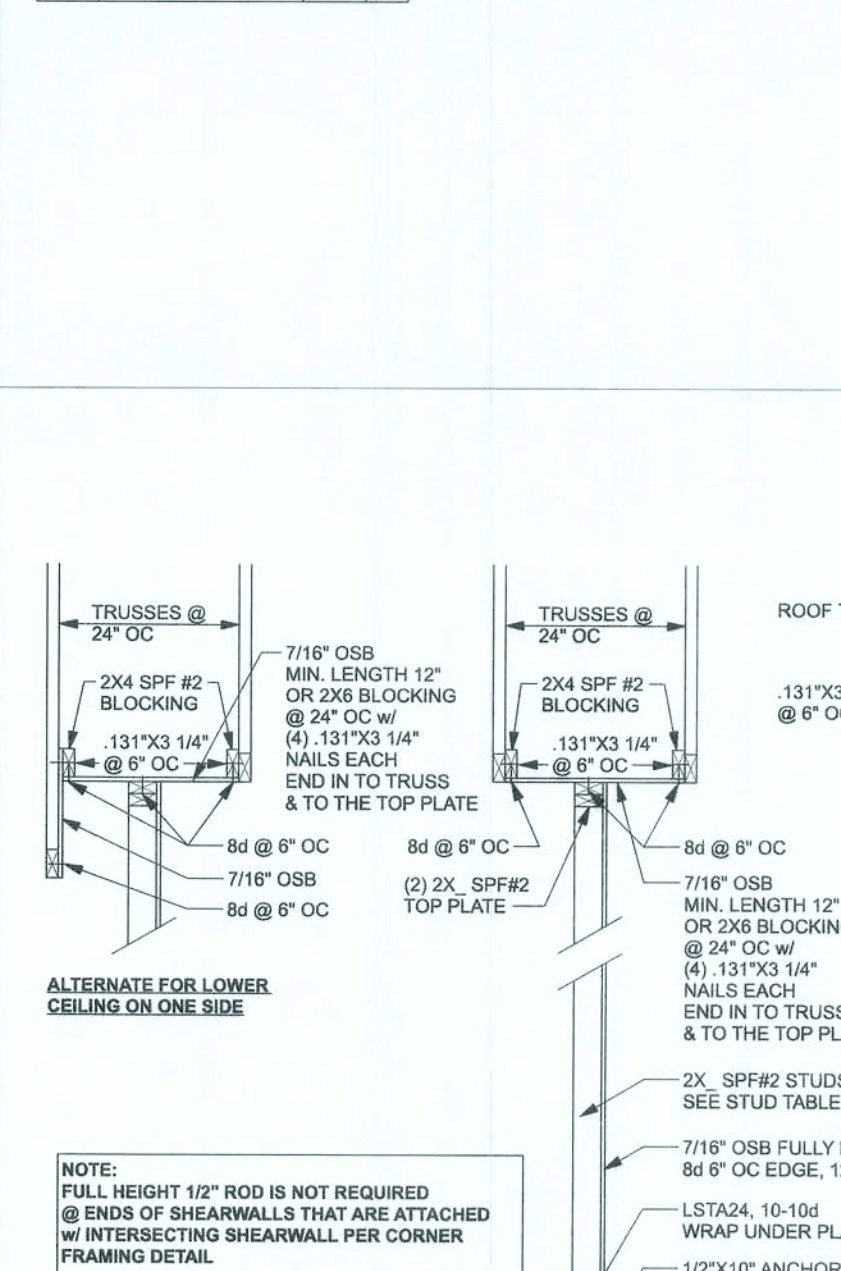
CONNECTOR TABLE				
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600		
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"
575	485	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"
1340	1015	H10A	9-10d x 1 1/2"	9-10d x 1 1/2"
720	620	LTS12-20	6-10d x 1 1/2"	6-10d x 1 1/2"
1000	860	MTS12-30	7-10d x 1 1/2"	7-10d x 1 1/2"
1450	1245	HTS20-30	12-10d x 1 1/2"	12-10d x 1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA31	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stemmwall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	12"x12" Titan HD
4235	3640	HTT4	18-16d x 1/2"	12"x12" Titan HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	12"x6" Titan HD
4235	3640	HTT4	18-16d x 1/2"	12"x12" Titan HD
Uplift SP	Uplift SPF	Post Bases @ Stemmwall	To Post	Anchor
2200	ABU44		5/8"x12" Drill & Epoxy	
2300	ABU66		5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44		5/8"x7" Drill & Epoxy	
2300	ABU66		5/8"x7" Drill & Epoxy	

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE		
2x8	SP #2	F _b E
2x10	SP #2	800 1.4
2x12	SP #2	750 1.4
GLB	24F-V3 SP	2600 1.9
LVL	TIMBERSTRAND	1700 1.7
PSL	PARALAM	2900 2.0



INTERIOR SHEAR WALL
ONE STORY WOOD FRAME w/ STRAPS & AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, F_s = 85ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A186 LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C-1116. SUPPLIER TO PROVIDE ASTM C-1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI, ALL LAP SPLICES 40" DB (25" FOR BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DIRECTION, AND FLOOD ZONE.

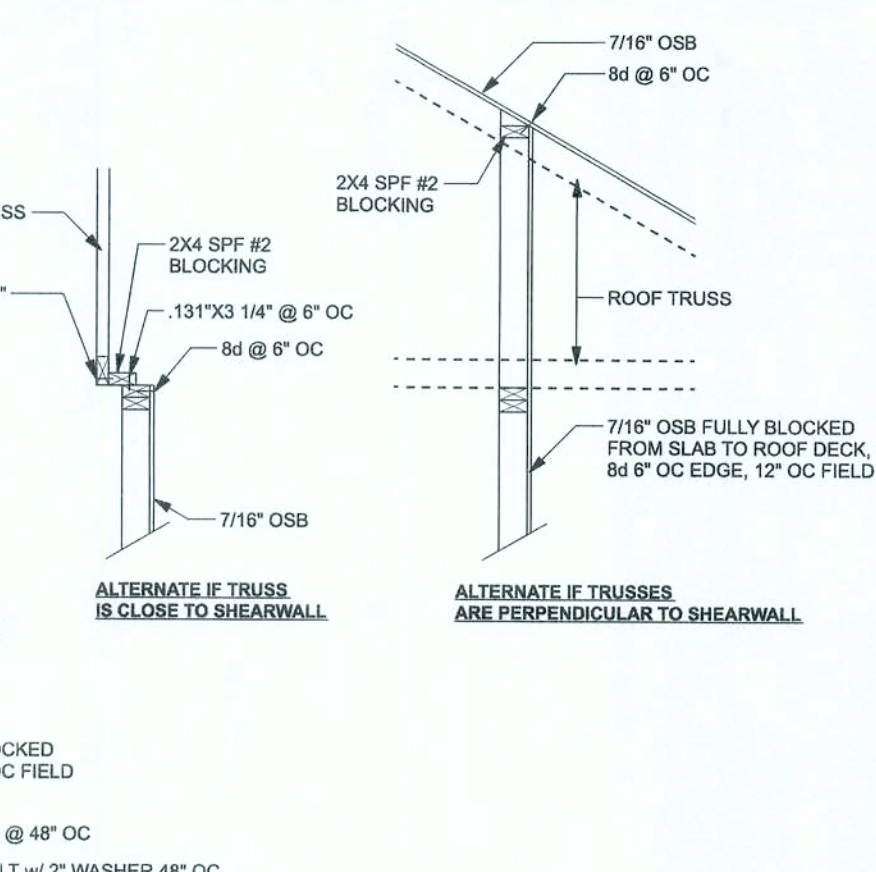
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

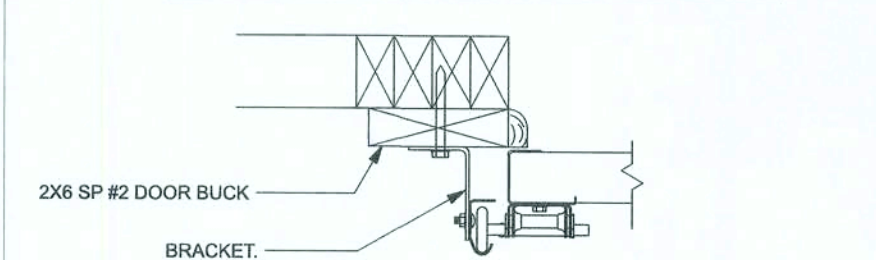


INTERIOR SHEAR WALL
ONE STORY WOOD FRAME w/ STRAPS & AB

2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d 16d OR (2) ROWS OF .131"x3 1/4" ON PER TABLE BELOW:

DOOR WIDTH	3/8"x4" LAG	16d STAGGER	(2) ROWS OF .131"x3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	16" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC



(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

DESIGN CRITERIA & LOADS:	
BUILDING CODE	16TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
WINDLOADS	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C) (V=10)		
EFFECTIVE WIND AREA (F _{T2})	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	-42.6	-46.2
21 - 40	-42.6	-46.2
41 - 60	-42.6	-46.2
61 - 80	-42.6	-46.2
81 - 100	-42.6	-46.2
101 - 120	-42.6	-46.2
121 - 140	-42.6	-46.2
141 - 160	-42.6	-46.2
161 - 180	-42.6	-46.2
181 - 200	-42.6	-46.2

Comerstone Development

1000 Model - right - Lot 10 Height Ashbury

PROJECT ADDRESS:
Lot 10 Height Ashbury
Columbia County, FL

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for clarification. Do not proceed without clarification.

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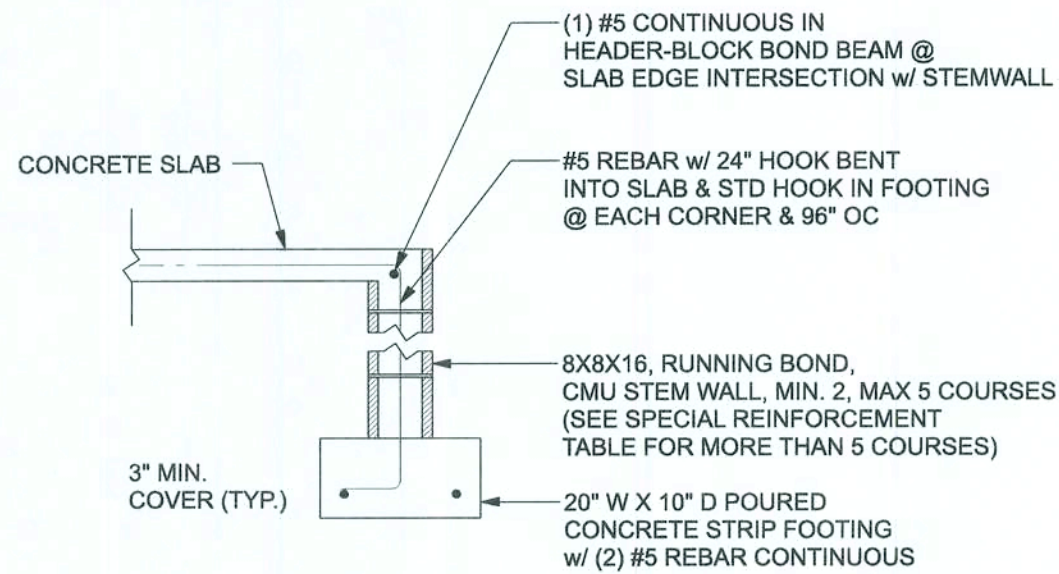
CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2017) to the best of my knowledge.

MARK DISOSWAY P.E. 53915

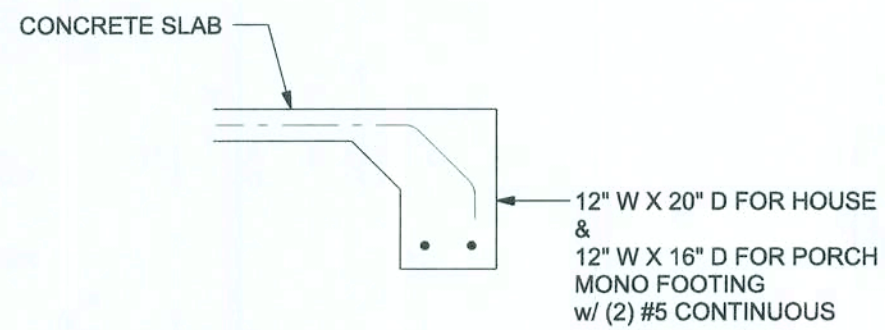
Mark Disosway P.E.
163 SW Miltown Place
Suite 103
Lake City, Florida 32025
386.74.5419
disoswaydesign@gmail.com

JOB NUMBER:
2008B5

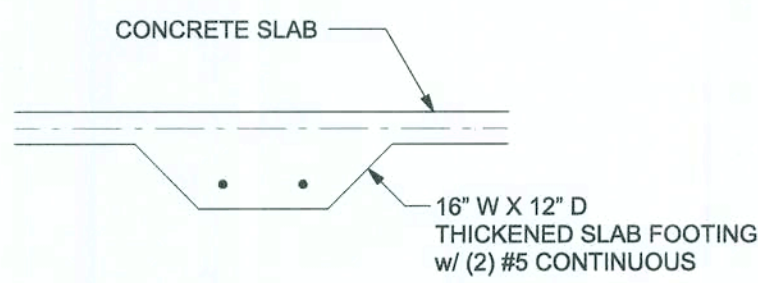
S-1
OF 3 SHEETS



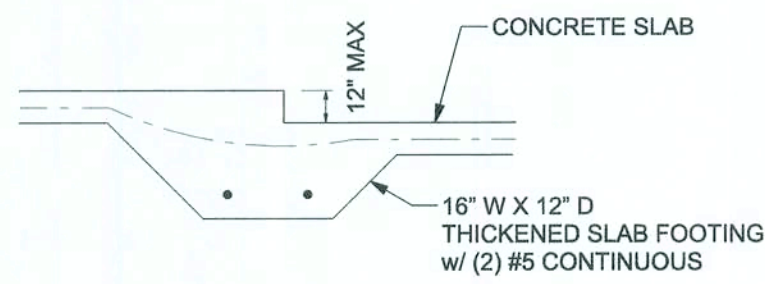
F1 S-2 OPTIONAL STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



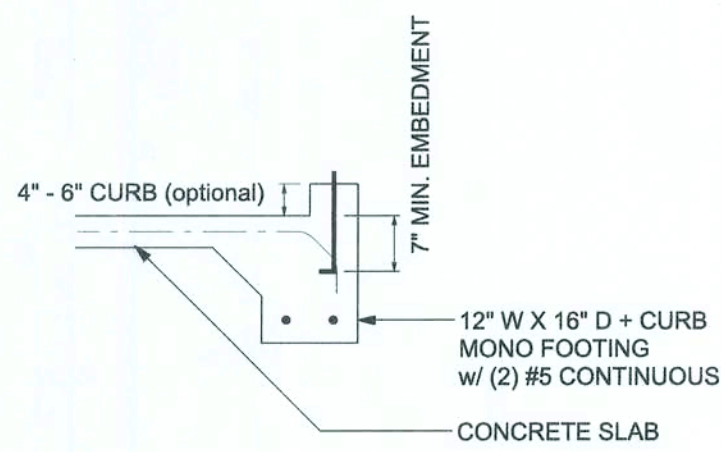
F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



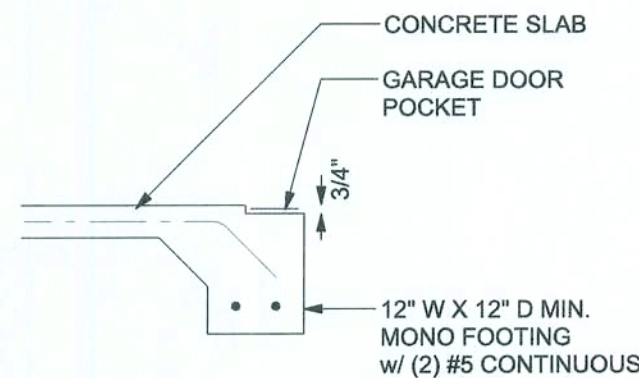
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 MONOLITHIC CURB FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING
SCALE: 1/2" = 1'-0"

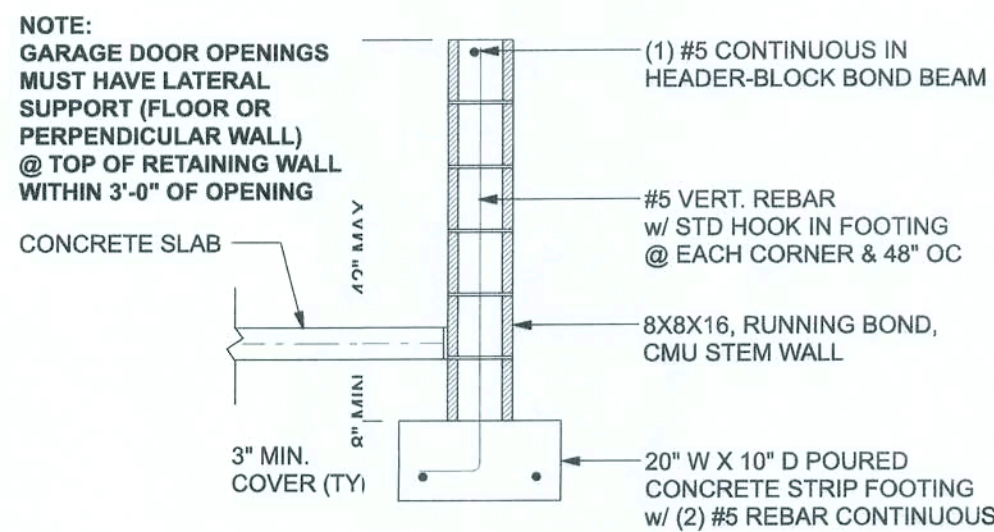
TALL STEM WALL GLE:
The table assumes 1/2" diameter reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at 1/2" top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add rowwall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1/2" continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKL HEIT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	0	96	96	96	96	96	96
4.0	3.	96	96	96	96	96	96
4.7	4.	88	96	96	96	96	96
5.3	5.	56	96	96	96	96	96
6.0	5.	40	80	96	80	96	96
6.7	6.	32	56	80	56	96	96
7.3	7.	24	40	56	40	80	96
8.0	7.	16	32	48	32	64	80
8.7	8.	8	24	32	24	48	64
9.3	9.	8	16	24	16	40	48

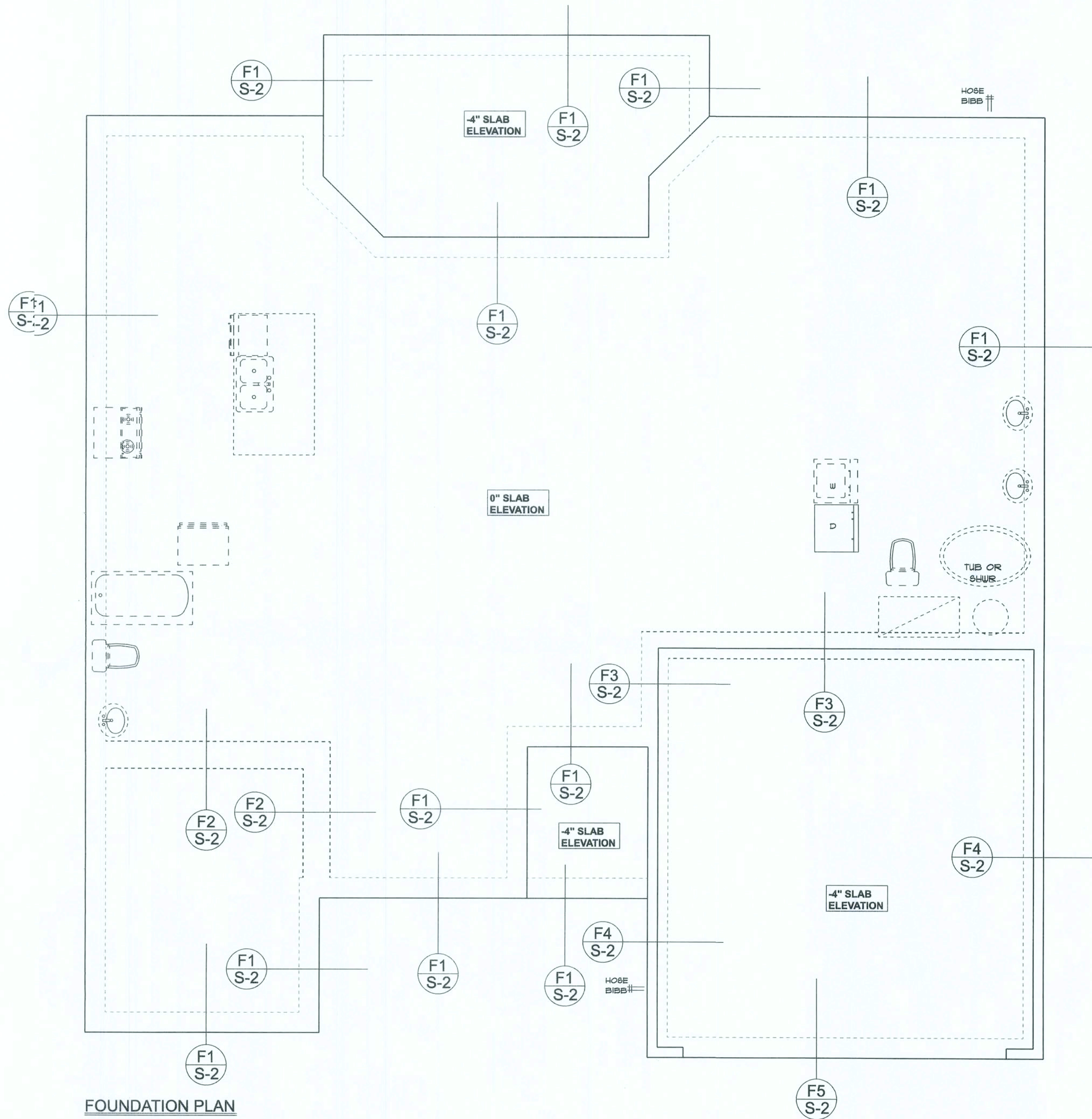
MASONRY NOTE
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 8/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 531-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTION TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU stander	ASTM C 90-02, Normal weight, hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick stander	ASTM C 216-02, Grade SW, Type FBS, 8"x2 7/8"x11 5/8"
2.4 Reinforcing ls, #3 - #11	ASTM A 615, Grade 60, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A 525, Class 660, 0.60 oz/ft ² or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A 153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2 Pipes, condu and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL PER FBC 2017 ES, SECTION R403.1.4



F4 S-2 OPTIOAL STEM WALL CURB FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED W/ (0.05-1.41) A WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER W/ 8" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL

Cornerstone Development

1500 Model - Right - Lot 10 Haight Ashbury

PROJECT ADDRESS:
1500 Model - Right - Lot 10 Haight Ashbury
Columbia County, FL

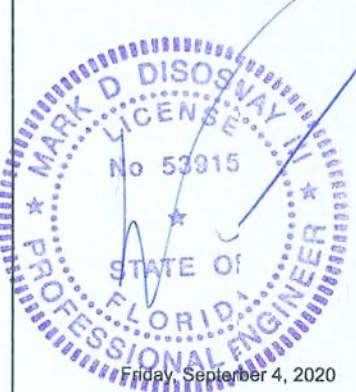
DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

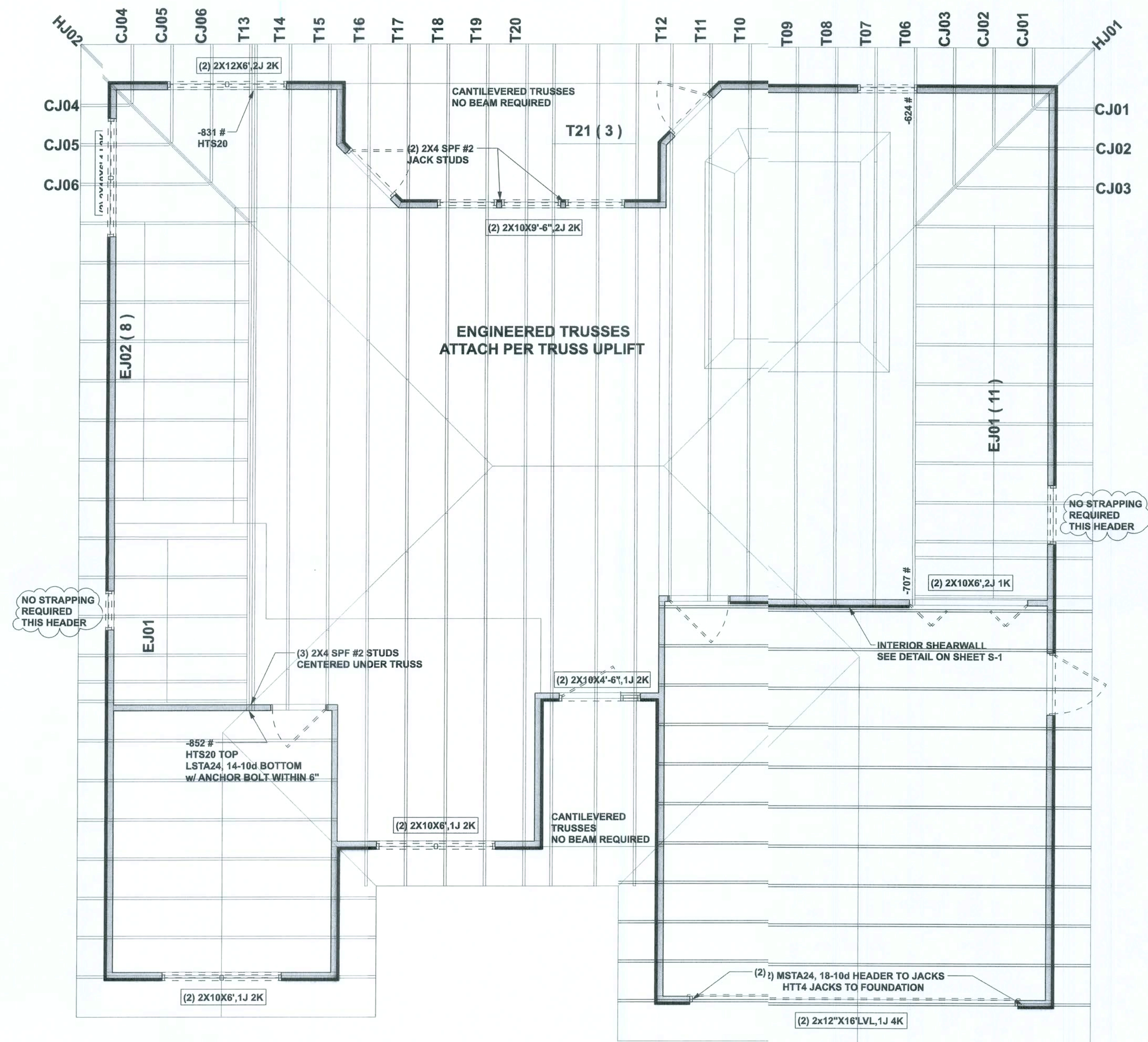
MARK DISOSWAY P.E. 53915



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Lake City, Florida 32025
386.751.5419
disoswaydesign@gmail.com

JOB NUMBER:
200885

S-2
OF 3 SHEETS

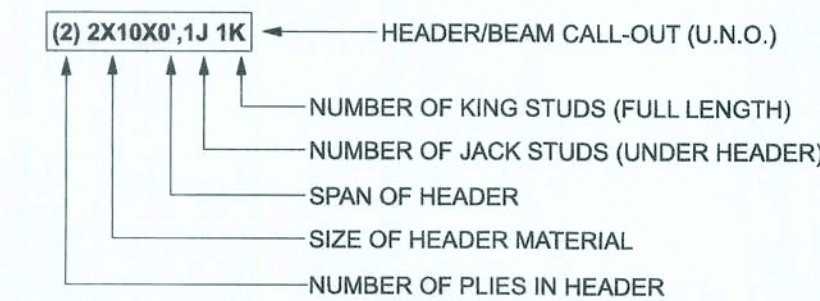


STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT, REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	25536 LBF	10728 LBF
REQUIRED	9102 LBF	8439 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #1752715

Cornerstone Development

1500 Model - right - Lot 10 Haight Ashbury

PROJECT ADDRESS:
Lot 10 Haight Ashbury
Columbia County, FL

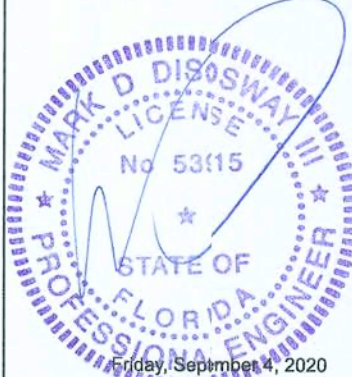
DIMENSIONS:
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LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915



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discoswaydesgn@gmail.com

JOB NUMBER:
201985

S-3
OF 3 SHEETS