DATE <u>02/1</u> :	5/2008					networkien		
A BDI ICANT	CHEANE		e Frommently Fost					0026760
		- 2003 No.	r.			380.023.0012	-	32025
				_		386 961 1086		32023
				_		300.701.1000	-	32055
		12 miles (1991)		-		386.623.9141		
			FERY ROAD TO H				- O	
		-						
TYPE DEVELO	OPMENT	SFD/UTILITY	I	ESTIMATED CO	ST OF CO	NSTRUCTION	20935	0.00
HEATED FLO	OR AREA	2690.00	TOTAL A	REA 4187.00		HEIGHT 2	20.00 S	TORIES 2
FOUNDATION	CONC	WALI	S FRAMED	ROOF PITCH	7'12	F	LOOR CO	DNC
LAND USE &	ZONING	<u>A-3</u>			MAX	. HEIGHT	35	
Minimum Set E	Back Requir	ments: STREET-	FRONT 30.0	00	REAR	25.00	SIDE	25.00
NO. EX.D.U.	0	FLOOD ZONE	XPP	DEVELOPM	ENT PERM	MIT NO.		
PARCEL ID	08-3S-16-0	02032-108	SUBDIVIS	ION HILLS	OF HUNTS	SVILLE		
LOT 8	BLOCK	PHASE	UNIT		TOTA	AL ACRES5	.00	
000001557			CBC1253543		5119	SA. EX	rele.	
Culvert Permit	No.	Culvert Waiver C	ontractor's License N	umber		Applicant/Owne	/Contractor	
18"X 32'MITEF	RED	08-0145	BLK		J	TH		N
Driveway Conn	ection	Septic Tank Number	LU & Zo	ning checked by	App	proved for Issuan	ce Ne	w Resident
COMMENTS:	MFE @ 99	9.5'.ELEVATION CO	NFIRMATION LETT	ER REQUIRED				
						Check # or C	ash 287	r.
		FOR BU	ILDING & ZON	ING DEPAR	TMENT	ONLY		
Temporary Pow	ver						(1	ooter/Slab)
		date/app. by				_ Monontine _	date	e/app. by
Under slab roug	gh-in plumbi	ing	Slab	20000		Sheathing		
		date/ap				<del></del>		date/app. by
Framing	date/apr	a by	Rough-in plumbing	above slab and b	elow wood	d floor		
Electrical rough	- 11		Heat & Air Duct					e/app. by
	-	date/app. by	Treat & All Duct	date/app. 1	by	Peri. beam (Lint	el)	date/app. by
Permanent power			C.O. Final			Culvert		
M/II tio down				date/app. by			date/a	pp. by
M/H tie downs,	blocking, el	ectricity and plumbing	date/a	app. by		Pool _	data/ar	n by
Reconnection		· · · · · · · · · · · · · · · · · · ·	Pump pole		Utility Pol			р. бу
M/H Pole	PHONE   386.623.6612							
	e/app. by	-		date/app. by	<del>-</del> ,	Ke-1001	date/app	. by
BUILDING PER	This Permit Must Be Prominently Posted on Premise During Construction   000026760							
MISC. FEES \$	0.00	ZONING	CERT. FEE \$ 50.0	00 FIRE FEE	E \$ 0.00	WAST	E FEE \$	
FLOOD DEVEL	OPMENT I	FEE S FLO	D ZONE FEE \$ 25	5.00 CULVEF	RT FEE \$	25.00 <b>TO</b>	TAL FEE	1191.86
INSPECTORS (	OFFICE	,001	/				* *1	V.
n tor be rone t	\_							

PERMIT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

#### BOARD OF COUNTY COMMISSIONERS OFFICE OF

# **BUILDING & ZONING**COLUMBIA COUNTY, FLORIDA

#### **BUILDING PERMIT RECEIPT**

RECEIPT NUMBER / PERMIT NUMBER 00002	28492 DATE <u>04/16/2010</u>
APPLICANT JAMES M. LIPSCOMB	
OWNER COUNTRY CLUB PROPERTIES, LLC.	
CONTRACTOR JAMES M. LIPSCOMB	
PARCEL ID NUMBER 08-3S-16-02032-108	NUMBER OF EXISTING DWELLINGS 0
TYPE OF DEVELOPMENT RENEWAL 26760 SFD	
COMMENTS: RENEWAL OF PERMIT 26760 - 1/4 OF	F THE TOTAL INSPECTIONS REMAIN,
ELEVATION CONFIRMATIN RECIEVED, SEE 26760 F	FOR PLANS & DOCUMENTS
FEES:  BUILDING PERMIT 0.00  ZONING FEE  FLOOD ZONE FEE	CERTIFICATION FEE 0.00  SURCHARGE FEE 0.00  FLOOD DEVELOPMENT PERMIT
MOBILE HOME PERMIT	RELOCATION PERMIT
TRAVEL TRAILER PERMIT	RENEWAL PERMIT 262.50
UTILITY POLE PERMIT	WASTE ASSESSMENT FEE
FIRE FEE (5 ACRES OR LESS)	CULVERT PERMIT
FIRE FEE (MORE THAN 5 ACRES)	
CHECK NUMBER	TOTAL FEES CHARGES 262.50

MAKE CHECKS PAYABLE TO: BCC (Board of County Commissioners)

NOTE: A SEPARATE CHECK IS REQUIRED FOR THE CULVERT WAIVER PERMITS

135 NE HERNANDO AVE. SUITE B-21

LAKE CITY, FL 32055 Phone: 386-758-1008 Fax: 386-758-2160



Columbia County Building Permit Application For Office Use Only Application # **Date Received** Plans Examiner OKJIH Deed or PA \Site Plan - State Road Info - Parent Parcel # Dev Permit # □ In Floodway □ Letter of Authorization from Contractor □ Unincorporated area 🔞 Incorporated area 🔞 Town of Fort White 🗈 Town of Fort White Compilance letter Septic Permit No. Name Authorized Person Signing Permit Fee Simple Owner Name & Address Bonding Co. Name & Address Architect/Engineer Name & Address MARK DISOSWAY Mortgage Lenders Name & Address Circle the correct power company - FL Power & Light - Clay Elec. + Suwannee Valley Elec. - Progress Energy 02032-108 Estimated Cost of Construction 235,000 Number of Existing Dwellings on Property 0 Lot Size Do you need a <u>Culvert Permit</u> or <u>Culvert Walver</u> or <u>Have an Existing Drive</u> Side 199' 8" Actual Distance of Structure from Property Lines - Front  $200^{\prime}$ Number of Stories Heated Floor Area 2690 Total Floor Area 4870 Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards

Page 1 of 2 (Both Pages must be submitted together.)

of all laws regulating construction in this jurisdiction.

\_ 11., SPOKE W/ SUSAN 2.8.08

- Cett 287-

#### Columbia County Building Permit Application

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

<u>YOU ARE HEREBY NOTIFIED</u> as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

<u>OWNERS CERTIFICATION:</u> I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

<u>CONTRACTORS AFFIDAVIT:</u> By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's License Number <u>CBC 1253543</u>
Contractor's Signature (Permitee)

Competency Card Number

Affirmed under penalty of perjury to by the <u>Contractor</u> and subscribed before me this <u>3/</u> day of <u>Jan</u> 20<u>08</u>.

Personally known or Produced Identification

Subon J. Holdn SEAL:

State of Florida Notary Signature (For the Contractor)





## BRITT SURVEYING

830 West Duval Street • Lake City, FL 32055 Phone (386) 752-7163 • Fax (386) 752-5573



03/24/08

L-19200

Permit # 26760

To Whom It May Concern:

C/o: Lipscomb & Eagle

Re: Lot 8 of Hills of Huntsville

The elevation of the foundation is found to be 102.87 feet. The centerline of the adjacent road NW Levi Glen is 102.58 feet and the centerline of the adjacent road NW Milo Terrace is 100.14 feet. The highest adjacent natural grade is 102.30 feet. The lowest natural adjacent grade is 101.80 feet. There is an elevation benchmark set on the SE corner of lot 8 elevation = 102.60 feet. The elevations shown hereon are based on NGVD 29 Datum.

L. Scott Britt PLS #5757

;380;No. 5793 P. 1 # 2/ 3



#### STATE OF FLORIDA DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 08-0145

		PAR	TII - SITE PLAN		
Scale: Each block	represents 5 feet and	1 inch = 50 feet	. j		
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Site Plan submitt	ted by:	Sign	alure		Title
Plan Approved _	X		Approved		Date J. 13.1
- HILLYPPIOVEG _	<u>, , , , , , , , , , , , , , , , , , , </u>	Fr.			Date Or 1 Y (
By Sale	he tord.	1311	Columbia	CHN	County Health Depa
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ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 10/90 (Replaces HRS-H Form 4015 which may be used) into a River Rumber K744, 1525, 455 F. A.

Received 2-13-08

Para 7 (

Inst. Number: 200712027832 Book: 1138 Page: 2317 Date: 12/18/2007 Time: 11:32:00 AM Page 1 of 2

18.50

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID 07-445 POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328

RETURN TO:

TERRY MCDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328 Inst:200712027832 Date: 12/18/2007 Time:11:32 AM Doc Stamp-Deed:8645.00 DC,P.DeWitt Cason,Columbia County Page 1 of 2

Property Appraiser's Parcel Identification No. R02032-101, 102, 103, 104, 105, 106, 107, 108, 109, 114, 117, 118, 119, 120, 121, 122, 123, 124, 125

#### WARRANTY DEED

THIS INDENTURE, made this 17th day of December, 2007, between WESTRIDGE, INC., a corporation existing under the laws of the State of Florida, whose post office address is: Post Office Box 1733, Lake City, FL 32056 and having its principal place of business in the County of Columbia, State of Florida, party of the first part, and COUNTRY CLUB PROPERTIES, LLC, A Florida Limited Liability Company whose Document Number is L05000091407, FEI Number is and whose post office address is: Post Office Box 3659, Lake City, FL 32056, of the State of Florida, party of the second part,

WITNESSETH: that the said party of the first part, for and in consideration of the sum of Ten Dollars (\$10.00), to it in hand paid, the receipt whereof is hereby acknowledged, has granted, bargained, sold, aliened, remised, released, conveyed and confirmed, and by these presents doth grant, bargain, sell, alien, remise, release, convey and confirm unto the said party of the second part, their heirs and assigns forever, all that certain parcel of land lying and being in the County of Columbia and State of Florida, more particularly described as follows:

Lots 1, 2, 3, 4, 5, 6, 7, 8, 9, 14, 17, 18, 19, 20, 21, 22, 23, 24 and 25, HILLS OF HUNTSVILLE SUBDIVISION, a subdivision according to the plat thereof as recorded in Plat Book 8, Pages 126-129 of the public records of Columbia County, Florida.

SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year.

TOGETHER with all the tenements, hereditaments and appurtenances, with every privilege, right, title, interest and estate, reversion, remainder and easement thereto belong or in anywise appertaining:

TO HAVE AND TO HOLD the same in fee simple forever.

And the said party of the first part doth covenant with said party of the second part that it is lawfully seized of said premises; that they are free of all encumbrances, and that it has good right and lawful authority to sell the same; and the said party of the first part does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

IN WITNESS WHEREOF, the party of the first part has caused these presents to be signed in its name by its Vice President, the day and year above written.

Signed, sealed and delivered in our presence:

WESTRIDGE, INC.

Vice President

Witness Terry McDavid

Witness: DeFite F Brown

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 17th day of December, 2007, by CHRIS BULLARD, as Vice President of WESTRIDGE, INC., a State of Florida corporation, on behalf of the corporation. He is personally known to me and did not take an oath.

(Seal)

Notary Public
My Commission Expires:

TERRY MCDAVID
MY OCHANISMON & DD 500786
E4P/ACS Fortherly 16, 2010
Statist The Nation Public Underwriters

# FLORIDA DEPARTMENT OF STATE DIVISION OF CORPORATIONS Home Contact Us E-Filing Services Document Searches Forms Help Previous on List Next on List Return To List No Events No Name History Entity Name Search

#### **Detail by Entity Name**

#### Florida Limited Liability Company

COUNTRY CLUB PROPERTIES, LLC

#### Filing Information

Document Number L05000091407

**FEI Number** 

203608875

Date Filed

09/08/2005

State

FL

Status

**ACTIVE** 

#### **Principal Address**

164 NW MADISON ST

STE 102

LAKE CITY FL 32055

Changed 04/02/2007

#### Mailing Address

PO BOX 3659

LAKE CITY FL 32056

Changed 04/02/2007

#### Registered Agent Name & Address

CRAPPS, DANIEL 164 NW MADISON ST STE 102 LAKE CITY FL 32055

Address Changed: 04/02/2007

#### Manager/Member Detail

#### Name & Address

Title MGRM

CRAPPS, DANIEL PO BOX 3659 LAKE CITY FL 32056

Title MGRM

EAGLE, THOMAS H 116 NW EGRET LANE LAKE CITY FL 32055

#### Annual Reports

Report Year Filed Date

# COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 \* FAX: (386) 758-1365 \* Emuil: ron\_croft@columbiacountyfla.com

#### Addressing Maintenance

To maintain the Countywide Addressing Polloy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE	REQUESTED:
- LARES	AND WELDER FILE.

1/24/2008

DATE ISSUED:

2/1/2008

**ENHANCED 9-1-1 ADDRESS:** 

879

NW MILO

TER

LAKE CITY FL 32055 PROPERTY APPRAISER PARCEL NUMBER:

08-3\$-16-02032-108

Remarks:

LOT 8 HILLS OF HUNTSVILLE

Address Issued By: Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

Approved Ad	dress
-------------	-------

1127

FEB 0 1 2008

911Addressing/GIS Dept

Susan Engles

NW LEY! GLEN

# HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS



DONALD AND MARY HALL OWNERS

June 12, 2002

NOTICE TO ALL CONTRACTORS

2201221022

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank, you,

Donald D. Hall

DDH/ik

## **Columbia County Building Department Culvert Permit**

# Culvert Permit No. 000001557

DATE $02/1$	8/2008	PARCEL ID # _0	08-38-16-02032-108	
APPLICANT	SUSAN EAGLE		PHONE	386.623.6612
ADDRESS _	872 SW JAGUA	R DRIVE	LAKE CITY	FL 32025
OWNER JA	MES MACK LIPSC	OMB	PHONE 3	386.623.6612
ADDRESS 8	79 NW MILO TI	ERRACE	LAKE CITY	FL 32055
CONTRACTO	R COUNTRY CL	JB PROPERTIES,LLC	PHONE	386.623.9141
LOCATION O	F PROPERTY	LAKE JEFFERY TO HUNTS	VILLE CHURCH ROAD,TL	TO MILO TERRACE
& IT'S @ THE CO	RNER OF MILO &	LEVI GLN.		
SIGNATURE	Culvert size will driving surface thick reinforced INSTALLATIO a) a majority b) the drivew Turnouts shooncrete or current and Culvert installation		with a total lenght of 32 d 4 foot with a 4 : 1 slope of 4 foot with a 4 : 1 slope of 4 foot with a 5 follows: I get driveway turnouts are pived or formed with conca minimum of 12 feet wher is greater. The width the turnouts.  Approved site plan standard stallation approved standard stallation stallation approved standard stallation stallatio	e and poured with a 4 inch paved, or; rete. ide or the width of the shall conform to the dards.

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



#### NOTICE OF COMMENCEMENT

# STATE OF FLORIDA COUNTY OF Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in the Notice of Commencement.

1. Description of property:

Inst:200812003425 Date:2/20/2008 Time:1:18 PM
\_\_\_\_DC,P.DeWitt Cason,Columbia County Page 1 of 1

#### Lot 8 Hills of Huntsville

- 2. General description of improvement: Construction of Dwelling
- 3. Owner information:
  - a. Name and address: Country Club Properties, LLC
     872 SW Jaguar Drive
     Lake City, FL 32025
  - b. Interest in property: Fee Simple
  - c. Name and address of fee simple title holder (if other than Owner): None
- 4. Contractor: James Mack Lipscomb
- 5. Surety n/a
  - a. Name and address:
  - b. Amount of bond:
- 6. Lender: n/a
- 7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None
- 8. In addition to himself, Owner designates to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

Signature of Owner

My commission expires:

The foregoing instrument was acknowledged before me this 20 day of Feb. 2008 by Thomas Eagle who are personally known to me and who did not take an oath

Notary Public

Susan L. Holton
Commission #DD431203
Expires: MAY 19, 2009
www.AARONNOTARY.com

1,0

Project Name:

Address:

Permitting Office: Columbia

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

801261LipscombEagleDevelopment

Lot: 8, Sub: Hills of Huntsv, Plat:

City, State: , FL Owner: Devane Residence Climate Zone: North	Permit Number: 26766  Jurisdiction Number: 221002
1. New construction or existing 2. Single family or multi-family 3. Number of units, if multi-family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area (ft²) 7. Glass type 1 and area: (Label reqd. by 13-104.4.5 if not default) a. U-factor:	12. Cooling systems a. Central Unit  b. N/A  c. N/A  13. Heating systems a. Electric Heat Pump  b. N/A  c. N/A  14. Hot water systems a. Electric Resistance  b. N/A  c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)  15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan. PT-Programmable Thermostat, MZ-C-Multizone cooling. MZ-H-Multizone heating)
Glass/Floor Area: U U9	uilt points: 30766 se points: 35527  PASS
I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy	Review of the plans and specifications covered by this

this calculation are in compliance with the Florida Energy

Code.

PREPARED BY:

DATE:

# **SUMMER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 8, Sub: Hills of Huntsv, Plat: , , FL,

PERMIT #:

	BASE			AS-BUILT								
GLASS TYPES .18 X Condition Floor A	oned X B	SPM =	Points	Type/SC	Ove Ornt	erhang Len	Hgt	Area X	SPI	иχ	SOF	= Points
.18 2690	0.0	20.04	9703.4	Double, Clear	S	15.0	6.0	20.0	35.8	7	0.44	314.3
				Double, Clear	S	15.0	7.0	24.0	35.8		0.45	385.1
				Double, Clear	SE	18.0	6.0	11.7	42.7		0.38	189.8
				Double, Clear	S	12.0	6.0	15.0	35.8		0.45	242.7
				Double, Clear	SW	99.0	6.0	11.7	40.1		0.37	172.9
				Double, Clear	E	51.0	6.0	20.0	42.0		0.36	300.2
				Double, Clear	S	1.5	0.0	30.0	35.8		0.43	464.8
				Double, Clear	S	1.5	0.0	7.0	35.8		0.43	108.4
				Double, Clear	W	1.5	4.5	16.0	38.5		0.85	523.6
				Double, Clear	N	1.5	7.0	15.0	19.2		0.96	275.0
				Double, Clear	N	9.0	7.0	60.0	19.2		0.67	775.0
				Double, Clear	N	1.5	7.0	18.0	19.2		0.96	330.1
				As-Built Total:				248.4				4081.8
WALL TYPES	Area X	BSPM	= Points	Туре		R-V	/alue	Area	Х	SPN	/I =	Points
Adjacent	304.0	0.70	212.8	Frame, Wood, Exterior			13.0	2061.6		1.50		3092.4
Exterior	2061.6	1.70	3504.7	Frame, Wood, Adjacent			13.0	304.0		0,60		182.4
Base Total:	2365.6		3717.5	As-Built Total:				2365.6				3274.8
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	Χ	SPN	A =	Points
Adjacent	20.0	1.60	32.0	Exterior Insulated				40.0		4.10		164.0
Exterior	60.0	4.10	246.0	Exterior Insulated				20.0		4.10		82.0
				Adjacent Insulated				20.0		1.60		32.0
Base Total:	80.0		278.0	As-Built Total:				80.0				278.0
CEILING TYPE	S Area X	BSPM	= Points	Туре	- 1	R-Value	e A	Area X S	SPM	X SC	= MC	Points
Under Attic	2190.0	1.73	3788.7	Under Attic			30.0	3222.0	1.73 >	1.00		5574.1
Base Total:	2190.0		3788.7	As-Built Total:				3222.0				5574.1
FLOOR TYPES	Area X	BSPM	= Points	Туре		R-V	/alue	Area	X	SPN	1 =	Points
Slab	275.0(p)	-37.0	-10175.0	Slab-On-Grade Edge Insulation	n	and the same of th	0.0	275.0(p	-	41.20	-	-11330.0
Raised	300.0	-3.99	-1197.0	Raised Wood, Adjacent			19.0	300.0		0.40		120.0
Base Total:			-11372.0	As-Built Total:				575.0				-11210.0

# **SUMMER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 8, Sub: Hills of Huntsv, Plat: , , FL, PERMIT #:

BASE	AS-BUILT								
INFILTRATION Area X BSPM = Poi	ts Area X SPM = Points								
2690.0 10.21 2746	1.9 2690.0 10.21 27464.9								
Summer Base Points: 33580.5	Summer As-Built Points: 29463.6								
Total Summer X System = Cooling Points Multiplier Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)								
33580.5 0.4266 14325	(sys 1: Central Unit 51000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS) 29464								

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 8, Sub: Hills of Huntsv, Plat: , , FL,

PERMIT #:

	BASE			AS-BUILT								
GLASS TYPES .18 X Condition Floor A	oned X B	SWPM =	Points	Type/SC	Ove Ornt	erhang Len	Hgt	Area X	WF	PM X	Wol	= = Poin
.18 2690	0.0	12.74	6168.7	Double, Clear	S	15.0	6.0	20.0	13.	30	3.61	960.0
				Double, Clear	S	15.0	7.0	24.0	13.		3.54	1128.2
				Double, Clear	SE	18.0	6.0	11.7	14.		2.65	456.0
				Double, Clear	S	12.0	6.0	15.0	13.		3.51	699.2
				Double, Clear	SW	99.0	6.0	11.7	16.		2.03	397.7
				Double, Clear	E	51.0	6.0	20.0	18.		1.51	566.3
				Double, Clear	S	1.5	0.0	30.0	13.		3.66	1460.1
				Double, Clear	S	1.5	0.0	7.0	13.		3.66	340.7
				Double, Clear	W	1.5	4.5	16.0	20.		1.04	345.8
				Double, Clear	N	1.5	7.0	15.0	24.		1.00	369.2
				Double, Clear	N	9.0	7.0	60.0	24.		1.02	1506.0
				Double, Clear	N	1.5	7.0	18.0	24.		1.00	443.1
				As-Built Total:	eses.			248.4			,,,,,,,	8672.3
WALL TYPES	Area X	BWPM	= Points	Туре		R-V	/alue	Area	X	WPM	=	Points
Adjacent	304.0	3.60	1094.4	Frame, Wood, Exterior			13.0	2061.6		3.40		7009.4
Exterior	2061.6	3.70	7627.9	Frame, Wood, Adjacent			13.0	304.0		3.30		1003.2
Base Total:	2365.6		8722.3	As-Built Total:				2365.6				8012.6
DOOR TYPES	Area X	BWPM	= Points	Туре				Area	X	WPM	=	Points
Adjacent	20.0	8.00	160.0	Exterior Insulated				40.0		8.40		336.0
Exterior	60.0	8.40	504.0	Exterior Insulated				20.0		8.40		168.0
			0.000 00.00	Adjacent Insulated				20,0		8.00		160.0
Base Total:	80.08		664.0	As-Built Total:				80.08				664.0
CEILING TYPE	<b>S</b> Area X	BWPM	= Points	Туре	R-	Value	Ar	ea X W	PM.	X WC	M =	Points
Under Attic	2190.0	2.05	4489.5	Under Attic			30.0	3222.0	2.05	X 1.00		6605.1
Base Total:	2190.0		4489.5	As-Built Total:				3222.0				6605.1
FLOOR TYPES	Area X	BWPM	= Points	Туре		R-V	/alue	Area	X	WPM	=	Points
Slab	275.0(p)	8.9	2447.5	Slab-On-Grade Edge Insulation	n		0.0	275.0(p		18.80		5170.0
Raised	300.0	0.96	288.0	Raised Wood, Adjacent		je	19.0	300.0		2.20		660.0
Base Total:			2735.5	As-Built Total:				575.0				5830.0

# WINTER CALCULATIONS

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 8, Sub: Hills of Huntsv, Plat: , , FL, PERMIT #:

	BASE		AS-BUILT								
INFILTRATION	Area X BWF	PM = Points	Area X WPM = Poin								
	2690.0 -0	.59 -1587.1	2690.0 -0.59 -1587								
Winter Base	Points:	21192.9	Winter As-Built Points: 28197.								
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Heati Component Ratio Multiplier Multiplier Multiplier Poin (System - Points) (DM x DSM x AHU)								
21192.9	0.6274	13296.4	(sys 1: Electric Heat Pump 51000 btuh ,EFF(7.9) Ducts:Unc(S),Unc(R),Int(AH),R6.0         28197.0       1.000 (1.069 x 1.169 x 0.93) 0.432 1.000 14145.1         28197.0       1.00 1.162 0.432 1.000 14145.1								

PERMIT #:

# WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 8, Sub: Hills of Huntsv, Plat: , , FL,

BASE					AS-BUILT								
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X	Credit Multiplie		
3		2635.00		7905.0	40.0	0.93	3		1.00	2606.67	1.00	7820.0	
					As-Built To	tal:						7820.0	

	CODE COMPLIANCE STATUS												
BASE						,	AS-	BUILT	* independent				
Cooling Points	+	Heating Points	+	Hot Water Points	п	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
14325		13296		7905		35527	8801		14145		7820		30766

PASS



# **Code Compliance Checklist**

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 8, Sub: Hills of Huntsv, Plat: , , FL,

PERMIT #:

#### 6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.  EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas 612.1 Spas & heated pools must have covers (except solar heated). Non-commerce		Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems			
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation 604.1, 602.1 Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 b Common ceiling & floors R-11.		Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides.  Common ceiling & floors R-11.	

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

#### ESTIMATED ENERGY PERFORMANCE SCORE\* = 86.0

The higher the score, the more efficient the home.

Devane Residence, Lot: 8, Sub: Hills of Huntsv, Plat: , , FL,

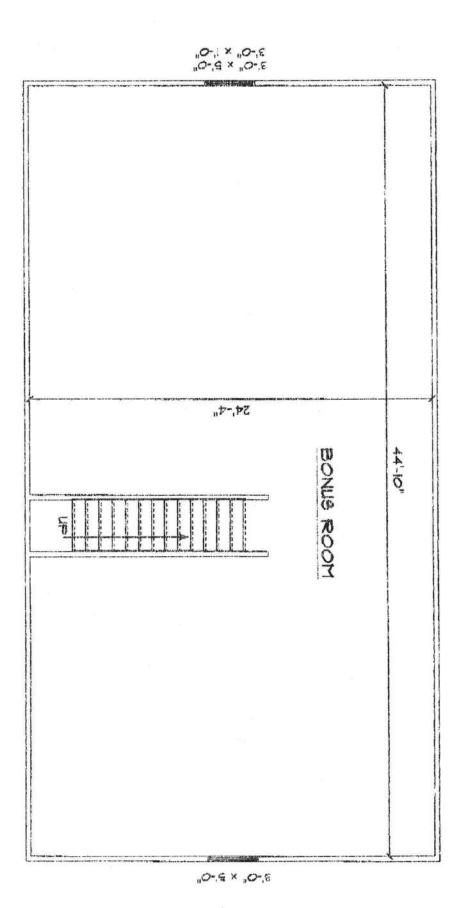
	w construction or existing	New	-		Cooling systems	
	gle family or multi-family	Single family		a.	Central Unit	Cap: 51.0 kBtu/hr
	mber of units, if multi-family	1				SEER: 13.00
4. Nui	mber of Bedrooms	3	_	b.	N/A	
<ol><li>Is the</li></ol>	his a worst case?	Yes				
6. Con	iditioned floor area (ft2)	2690 ft <sup>2</sup>		c.	N/A	
7. Gla	ss type 1 and area: (Label reqd. by 1	3-104.4.5 if not default)				
a. U-fa	actor:	Description Area		13.	Heating systems	
(or	Single or Double DEFAULT) 7a.				Electric Heat Pump	Cap: 51.0 kBtu/hr
b. SHO						HSPF: 7.90
(01	r Clear or Tint DEFAULT) 7b.	(Clear) 248.4 ft <sup>2</sup>		b.	N/A	
8. Floo		(5)(4) 2 (5) (1)				
	b-On-Grade Edge Insulation	R=0.0, 275.0(p) ft		C.	N/A	_
	sed Wood, Adjacent	R=19.0, 300.0ft <sup>2</sup>				
c. N/A	100 0			14	Hot water systems	
	ll types				Electric Resistance	Cap: 40.0 gallons
	me, Wood, Exterior	R=13.0, 2061.6 ft <sup>2</sup>			Electric Testimentes	EF: 0.93
	me, Wood, Adjacent	R=13.0, 304.0 ft <sup>2</sup>		h	N/A	La : 51,75
c. N/A	70 5 30%	10.00, 2007.00 11		0.	1772	
d. N/A				D.	Conservation credits	
e. N/A				0.	(HR-Heat recovery, Solar	
	ing types				DHP-Dedicated heat pump)	
	ler Attie	R=30.0, 3222.0 ft <sup>2</sup>		1.5	HVAC credits	
b. N/A		K 30.0. 3444.0 II		10,		
e. N/A					(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan,	
11. Due		0 0 00 010 0 0			PT-Programmable Thermostat,	
	: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 210.0 ft			MZ-C-Multizone cooling.	
b. N/A					MZ-H-Multizone heating)	
Legrtify	that this home has complied	with the Florida Energ	v Effi	rione	v Code For Building	
	ction through the above energ					OF THE STATE
						130
	nome before final inspection. (		Displa	y Ca	ra will be completed	15/2012
	n installed Code compliant fea	nures.				GING CONTROL OF THE PARTY OF TH
Builder	Signature:		Date:			18

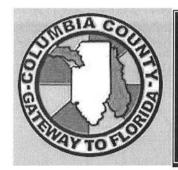
\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStdT designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

City/FL Zip:

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLR2PB v4.1)

Address of New Home:





From: The Columbia County Building & Zoning Department Plan Review 135 NE Hernando Av.

DO D 1520

P.O. Box 1529

Lake City Florida 32056-1529

Reference to a building permit application Number: 0801-166

Applicant: Susan Eagle

Owner: Country Club Properties LLC Contractor: James Mack Lipscomb

Property Identification # 08-3s-16-02032-108

On the date of February 6, 2008 building permit application number 0801-166 and the submitted plans for construction of a single family dwelling were reviewed. The following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0801-166 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Codes 2004 only and doesn't make any consideration toward the land use and zoning requirement

- 1. Please provide a copy of a signed released site plan from the Columbia County Environmenta Health Department, which confirms approval of the waste water disposal system.
- 2. Please submit a recorded (with the Columbia County Clerk Office) notice of commencement, prior any inspections can be performed by the Columbia County Building Department.
- 3. Please submit floor plan of the bonus room area. If a bedroom(s) is the intended use of this are please show an emergency escape and rescue opening (window(s)) for the bedroom in the bonus area. Emergency escape and rescue opening shall have a minimum net clear opening 5.7 square feet.

Thank You:

Joe Haltiwanger Plan Examiner County Building Department

# Residential System Sizing Calculation

Summary Project Title:

Devane Residence

Project Title: 801261LipscombEagleDevelopment

Class 3 Rating Registration No. 0 Climate: North

, FL

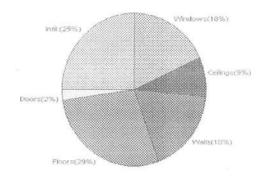
1/30/2008

Location for weather data: Gaine Humidity data: Interior RH (50%)			itude(29) Altitude(152 ft.) Temp Ran	ge(M)	
Winter design temperature	33		Summer design temperature	92	F
Winter setpoint	70		Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	43948	Btuh	Total cooling load calculation	32707	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	116.0	51000	Sensible (SHR = 0.75)	144.4	38250
Heat Pump + Auxiliary(0.0kW)	116.0	51000	Latent	205.2	12750
			Total (Electric Heat Pump)	155.9	51000

#### WINTER CALCULATIONS

Winter Heating Load (for 2690 sqft)

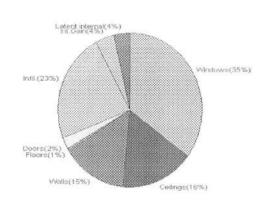
Load component			Load	
Window total	248	sqft	7996	Btuh
Wall total	2366	sqft	7769	Btuh
Door total	80	sqft	1036	Btuh
Ceiling total	3222	sqft	3797	Btuh
Floor total	See detail rep	ort	12563	Btuh
Infiltration	266	cfm	10787	Btuh
Duct loss			0	Btuh
Subtotal			43948	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOS	SS		43948	Btuh



#### SUMMER CALCULATIONS

Summer Cooling Load (for 2690 sqft)

Load component			Load	
Window total	248	sqft	11501	Btuh
Wall total	2366	sqft	4759	Btuh
Door total	80	sqft	784	Btuh
Ceiling total	3222	sqft	5336	Btuh
Floor total			181	Btuh
Infiltration	137	cfm	2553	Btuh
Internal gain			1380	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			26494	Btuh
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			5014	Btuh
Latent gain(ventilation)	0	Btuh		
Latent gain(internal/occu	1200	Btuh		
Total latent gain	6214	Btuh		
TOTAL HEAT GAIN	32707	Btuh		



For F

For Florida residences only

EnergyGauge® System Sizing

DATE:

DATE: 1-70-09

# **System Sizing Calculations - Winter**

# Residential Load - Whole House Component Details

Devane Residence

Project Title: 801261LipscombEagleDevelopment

Class 3 Rating Registration No. 0 Climate: North

, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F This calculation is for Worst Case. The house has been rotated 315 degrees.

1/30/2008

Component		

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btul
2	2, Clear, Metal, 0.87	NW	24.0	32.2	773 Btul
3	2, Clear, Metal, 0.87	W	11.7	32.2	377 Btul
4	2, Clear, Metal, 0.87	NW	15.0	32.2	483 Btul
5	2, Clear, Metal, 0.87	N	11.7	32.2	377 Btul
6	2, Clear, Metal, 0.87	SW	20.0	32.2	644 Btul
7	2, Clear, Metal, 0.87	NW	30.0	32.2	966 Btul
8	2, Clear, Metal, 0.87	NW	7.0	32.2	225 Btul
9	2, Clear, Metal, 0.87	NE	16.0	32.2	515 Btul
10	2, Clear, Metal, 0.87	SE	15.0	32.2	483 Btul
11	2, Clear, Metal, 0.87	SE	60.0	32.2	1931 Btul
12	2, Clear, Metal, 0.87	SE	18.0	32.2	579 Btul
	Window Total		248(sqft)		7996 Btul
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	2062	3.3	6770 Btul
2	Frame - Wood - Adj(0.09)	13.0	304	3.3	998 Btul
-	Wall Total	, 0.0	2366	0.0	7769 Btul
Doors	Туре		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btul
2	Insulated - Exterior		20	12.9	259 Btul
3	Insulated - Exterior		40	12.9	518 Btul
	Door Total		80		1036Btul
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	3222	1.2	3797 Btul
	Ceiling Total	00,0	3222	1.12	3797Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Raised Wood - Adj	19	300.0 sqft	1.9	557 Btuh
2	Slab On Grade	0	275.0 ft(p)	43.7	12007 Btul
<del>***</del>	Floor Total	9	575	10.1	12563 Btuh
	T loor Total		070		12000 Ditti
		Z	Zone Envelope S	subtotal:	33161 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.66	24210	266.3	10787 Btuh
	ivaturai	0.00	24210	200.5	10707 Blui
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic) (DLM of 0.00)			(DLM of 0.00)	0 Btuh
Zone #1	Sensible Zone Subtotal			total	43948 Btul

# **Manual J Winter Calculations**

Residential Load - Component Details (continued)
Project Title:

Devane Residence

801261LipscombEagleDevelopment

Class 3 Rating Registration No. 0

, FL

Climate: North

WHOLE HOUSE TO	TALS	1/30/2008
	Subtotal Sensible	43948 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	43948 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



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# **System Sizing Calculations - Winter**

## Residential Load - Room by Room Component Details

Devane Residence

Project Title:

Class 3 Rating Registration No. 0

, FL

801261LipscombEagleDevelopment

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F This calculation is for Worst Case. The house has been rotated 315 degrees.

1/30/2008

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btul
2	2, Clear, Metal, 0.87	NW	24.0	32.2	773 Btu
3	2, Clear, Metal, 0.87	VV	11.7	32.2	377 Btu
4	2, Clear, Metal, 0.87	NW	15.0	32.2	483 Btul
5	2, Clear, Metal, 0.87	N	11.7	32.2	377 Btul
6	2, Clear, Metal, 0.87	SW	20.0	32.2	644 Btul
7	2, Clear, Metal, 0.87	NW	30.0	32.2	966 Btu
8	2, Clear, Metal, 0.87	NW	7.0	32.2	225 Btul
9	2, Clear, Metal, 0.87	NE	16.0	32.2	515 Btul
10	2, Clear, Metal, 0.87	SE	15.0	32.2	483 Btul
11	2, Clear, Metal, 0.87	SE	60.0	32.2	1931 Btul
12	2, Clear, Metal, 0.87	SE	18.0	32.2	579 Btul
	Window Total		248(sqft)	1000000	7996 Btul
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	2062	3.3	6770 Btul
2	Frame - Wood - Adj(0.09)	13.0	304	3.3	998 Btul
	Wall Total		2366		7769 Btul
Doors	Туре		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btu
2	Insulated - Exterior		20	12.9	259 Btul
3	Insulated - Exterior		40	12.9	518 Btul
	Door Total		80		1036Btul
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	3222	1.2	3797 Btul
	Ceiling Total	2.7.5	3222	7.500	3797Btul
Floors	Туре	R-Value	Size X	HTM=	Load
1	Raised Wood - Adj	19	300.0 sqft	1.9	557 Btul
2	Slab On Grade	0	275.0 ft(p)	43.7	12007 Btul
	Floor Total		575	137.1	12563 Btul
	7,007,7000				12000 Etai
		2	Zone Envelope S	Subtotal:	33161 Btul
Infiltration	Туре	ACH X	Zone Volume	CFM=	
mmacon	Natural	0.66	24210	266.3	10787 Btul
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic) (DLM of 0.00)			0 Btul	
Zone #1	Sensible Zone Subtotal			ototal	43948 Btul

# **Manual J Winter Calculations**

Residential Load - Component Details (continued)

Devane Residence

, FL

Project Title:

801261LipscombEagleDevelopment

Class 3 Rating Registration No. 0

Climate: North

WHOLE HOUSE TOT	ALS	1/30/2008
	Subtotal Sensible	43948 Btuh
	Ventilation Sensible	0 Btuh
6	Total Btuh Loss	43948 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



For Florida residences only

# System Sizing Calculations - Summer

# Residential Load - Whole House Component Details

Devane Residence

Project Title:

801261LipscombEagleDevelopment

Class 3 Rating Registration No. 0

Climate: North

, FL

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/30/2008

This calculation is for Worst Case. The house has been rotated 315 degrees.

#### Component Loads for Whole House

	Type*		Overhang W			low Area	a(sqft)	HTM		Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None, N, N	NW	15ft.	6ft.	20.0	0.0	20.0	29	60	1201	Btuh
2	2, Clear, 0.87, None,N,N	NW	15ft.	7ft.	24.0	0.0	24.0	29	60	1441	Btuh
3	2, Clear, 0.87, None, N, N	W	18ft.	6ft.	11.7	11.7	0.0	29	80	339	
4	2, Clear, 0.87, None,N,N	NW	12ft.	6ft.	15.0	0.0	15.0	29	60	901	Btuh
5	2, Clear, 0.87, None,N,N	N	99ft.	6ft.	11.7	0.0	11.7	29	29	339	
6 7	2, Clear, 0.87, None, N, N	SW	51ft. 1.5ft.	6ft.	20.0	20.0	0.0	29	63	579	
8	2, Clear, 0.87, None, N, N 2, Clear, 0.87, None, N, N	NW	1.5ft.	Oft.	30.0 7.0	0.0	30.0 7.0	29 29	60 60	1801 420	Btuh
9	2, Clear, 0.87, None,N,N	NE	1.5ft.	4.5ft.	16.0	0.0	16.0	29	60	961	Btuh Btuh
10	2, Clear, 0.87, None,N,N	SE	1.5ft.	7ft.	15.0	3.8	11.2	29	63	810	
11	2, Clear, 0.87, None,N,N	SE	9ft.	7ft.	60.0	60.0	0.0	29	63	1738	
12	2, Clear, 0.87, None, N, N	SE	1.5ft.	7ft.	18.0	4.6	13.4	29	63		Btuh
	Window Total				248 (	sqft)				11501	
Walls	Туре		R-Va	alue/U	-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13,0/0	0.09	206	61.6		2.1	4300	Btuh
2	Frame - Wood - Adj		13.0/0.09 304.0 1.5					459	Btuh		
	Wall Total			2366 (sqft)					4759	Btuh	
Doors	Туре					Area	(sqft)		HTM	Load	F
1	Insulated - Adjacent					20	0.0		9.8	196	Btuh
2	Insulated - Exterior					20	0.0		9.8	196	Btuh
3	Insulated - Exterior					40	0.0		9.8	392	Btuh
	Door Total					8	0 (sqft)			784	Btuh
Ceilings	Type/Color/Surface		R-Value			Area(sqft)			HTM	Load	
1	Vented Attic/DarkShingle			30.0		322	22.0		1.7	5336	Btuh
	Ceiling Total				3222 (sqft)				5336	Btuh	
Floors	Туре		R-Value			Si	ze		HTM Lo	Load	
1	Raised Wood - Adj		19.0			00 (sqft)		0.6	181	Btuh	
2	Slab On Grade			0.0			75 (ft(p))		0.0	0	
	Floor Total					575.	.0 (sqft)			181	Btuh
	ı					Z	one Enve	elope Si	ubtotal:	22560	Btuh
nfiltration	Type SensibleNatural		А	CH 0.34		Volum			CFM= 137.2	Load 2553	Btuh
Internal	OCHOINICI VALUI di		Occup	A MARIA TO LIST.			cupant	Λ	Appliance		Diun
gain			occup	6		( 23		-	0	Load 1380	Btuh
Duct load	Average sealed, R6.0,	Supply	(Attic)	_				DGM		0.0	Btuh
							Sensib	le Zone	Load	26494	Btuh

# **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Devane Residence

Project Title: 801261LipscombEagleDevelopment

Class 3 Rating Registration No. 0 Climate: North

, FL

1/30/2008

#### WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	26494	Btu
	Sensible Duct Load	0	Btul
	Total Sensible Zone Loads	26494	Btu
	Sensible ventilation	0	Btu
	Blower	0	Btu
Whole House	Total sensible gain	26494	Βtι
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	5014	Btu
	Latent ventilation gain	0	Btu
	Latent duct gain	0	Βtι
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btu
	Latent other gain	0	Btu
	Latent total gain	6214	Btu
	TOTAL GAIN	32707	Btu

\*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



For Florida residences only

# System Sizing Calculations - Summer

# Residential Load - Room by Room Component Details

Devane Residence

801261LipscombEagleDevelopment

Class 3 Rating Registration No. 0

Climate: North

, FL

Reference City: Gainesville (Defaults) Summer Temperature Difference This calculation is for Worst Case. The house has been rotated 315 degrees. Summer Temperature Difference: 17.0 F

1/30/2008

#### Component Loads for Zone #1: Main

	Type*		Overhang Win			dow Area	a(sqft)	HTM		Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hat	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None, N, N	NW	15ft.	6ft.	20.0	0.0	20.0	29	60	1201	Btuh
2	2, Clear, 0.87, None, N, N	NW	15ft.	7ft.	24.0	0.0	24.0	29	60	1441	Btuh
3	2, Clear, 0.87, None, N, N	W	18ft.	6ft.	11.7	11.7	0.0	29	80	339	Btuh
4	2, Clear, 0.87, None, N, N	NW	12ft.	6ft.	15.0	0.0	15.0	29	60	901	Btuh
5	2, Clear, 0.87, None, N, N	- N	99ft.	6ft.	11.7	0.0	11.7	29	29	339	
6	2, Clear, 0.87, None, N, N	SW	51ft.	6ft.	20.0	20.0	0.0	29	63	579	
7	2, Clear, 0.87, None, N, N	NW	1.5ft.	Oft.	30.0	0.0	30.0	29	60	1801	
8	2, Clear, 0.87, None, N, N	NW	1.5ft.	Oft.	7.0	0.0	7.0	29	60	420	Btuh
9	2, Clear, 0.87, None,N,N	NE	1.5ft.	4.5ft.	16.0	0.0	16.0	29	60	961	Btuh
10	2, Clear, 0.87, None,N,N	SE	1.5ft.	7ft.	15.0	3.8	11.2	29	63	810	Btuh
11	2, Clear, 0.87, None, N, N	SE	9ft.	7ft.	60.0	60.0	0.0	29	63	1738	
12	2, Clear, 0.87, None,N,N	SE	1.5ft.	7ft.	18.0	4.6	13.4	29	63	972	
	Window Total				248 (	- Inches				11501	Btuh
Walls	Туре		R-V	alue/U	-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/0			61.6		2.1	4300	
2	Frame - Wood - Adj			13.0/0	0.09		4.0		1.5	459	
	Wall Total					236	66 (sqft)			4759	Btuh
Doors	Туре					Area	(sqft)		HTM	Load	
1	Insulated - Adjacent					20	0.0		9.8	196	Btuh
2	Insulated - Exterior					20	0.0		9.8	196	Btuh
3	Insulated - Exterior					40	0.0		9.8	392	Btuh
	Door Total					8	30 (sqft)		52	784	Btuh
Ceilings	Type/Color/Surface		R-Value		Area(sqft)			HTM Loa			
1	Vented Attic/DarkShingle			30.0		322	22.0		1.7	5336	Btuh
	Ceiling Total					3222 (sqft)				5336	Btuh
Floors	Туре		R-Va	R-Value Size			HTM	Load			
1	Raised Wood - Adj			19.0 30		00 (saft)	0,6		181	Btuh	
2	Slab On Grade			0.0 275 (ft(p))			0.0	0	Btuh		
	Floor Total		575.0 (sc							181	Btuh
						Z	one Enve	elope Si	ubtotal:	22560	Btuh
nfiltration	Control of the Contro		Α	CH			e(cuft)		CFM=	Load	
	SensibleNatural			0.34			210	.00	137.2	2553	Btuh
Internal		(	Occup	pants		Btuh/od	ccupant	F	Appliance	Load	
gain				6		X 23	0 +		0	1380	Btuh
Duct load	Average sealed, R6.0,	Supply	(Attic)	, Retu	ırn(Attio	C)		DGM	= 0.00	0.0	Btuh
							Sensib	le Zone	Load	26494	Btuh

# **Manual J Summer Calculations**

Residential Load - Component Details (continued)
Project Title: Class

Devane Residence

, FL

801261LipscombEagleDevelopment

Class 3 Rating Registration No. 0 Climate: North

1/30/2008

#### WHOLE HOUSE TOTALS

		1	
	Sensible Envelope Load All Zones Sensible Duct Load	<b>26494</b> 0	Btuh Btuh
	Total Sensible Zone Loads	26494	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	26494	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	5014	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	0	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	6214	Btuh
	TOTAL GAIN	32707	Btuh

\*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value) (BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



For Florida residences only

# **Residential Window Diversity**

#### MidSummer

Devane Residence

, FL

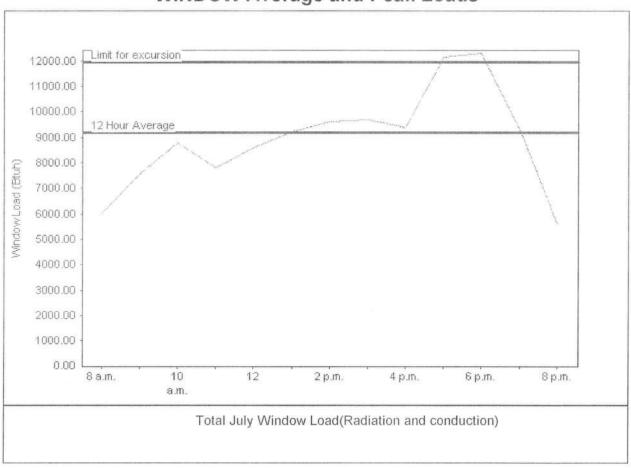
Project Title: 801261LipscombEagleDevelopment

Class 3 Rating Registration No. 0 Climate: North

1/30/2008

Weather data for: Gainesville - Def	aults			
Summer design temperature	92	F	Average window load for July	9204 Btuh
Summer setpoint	75	F	Peak window load for July	12306 Btu
Summer temperature difference	17	F	Excusion limit(130% of Ave.)	11965 Btu
Latitude	29	North	Window excursion (July)	341 Btuh

#### WINDOW Average and Peak Loads



Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residence

PREPARED BY

EnergyGauge® FLR2PB v4.1





Project Information for:

L266613

Builder:

Lipscomb Eagle

Lot:

Subdivision:

Hills of Huntsville

County:

Columbia

Truss Count:

Design Program: MiTek 20/20 6.3 **Building Code:** FBC2004/TPI2002

Truss Design Load Information: Gravity:

Wind:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): 55.0

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

James M. Lipscomb Florida License No. CBC1253543

Address: 255 Southeast Woods Terrace Lake City, Florida 32025 Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Drwg. #

J1931044

J1931045

J1931046

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Truss ID

T19

T20

T21

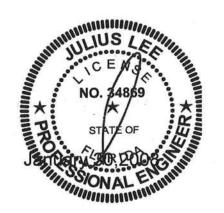
Date

1/30/08

1/30/08

1/30/08

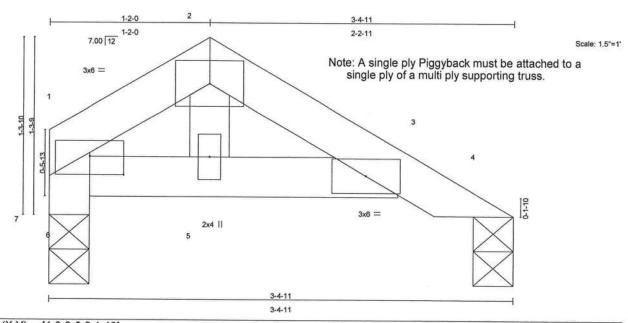
No.	Drwg. #	Truss ID	Date	No.
1	J1931016	PB05	1/30/08	29
2	J1931017	PB08	1/30/08	30
3	J1931018	PB17	1/30/08	31
4	J1931019	PB17G	1/30/08	
5	J1931020	PB20	1/30/08	
6	J1931021	PB20G	1/30/08	
7	J1931022	PB21	1/30/08	
8	J1931023	PB21G	1/30/08	
9	J1931024	T01G	1/30/08	
10	J1931025	T02	1/30/08	
11	J1931026	T03G	1/30/08	
12	J1931027	T04	1/30/08	
13	J1931028	T05G	1/30/08	
14	J1931029	T06	1/30/08	
15	J1931030	T07	1/30/08	
16	J1931031	T08	1/30/08	
17	J1931032	T08A	1/30/08	
18	J1931033	T08G	1/30/08	
19	J1931034	T09	1/30/08	
20	J1931035	T10G	1/30/08	
21	J1931036	T11	1/30/08	
22	J1931037	T12G	1/30/08	
23	J1931038	T13	1/30/08	
24	J1931039	T14	1/30/08	
25	J1931040	T15	1/30/08	
26	J1931041	T16G	1/30/08	
27	J1931042	T17	1/30/08	
28	J1931043	T18	1/30/08	



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	PB05	VALLEY	2	1	J1931016
		4x6 =	1880	0.0	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.10	Vert(LL)	-0.01	5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	3-5	>999	240	111120	211/100
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat						11/0	Weight: 11 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

TOP CHORD Structural wood sheathing directly applied or

4-5-7 oc purlins.

**BOT CHORD** 

BRACING

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=100/0-3-8, 7=99/0-3-8

Max Horz 7=-33(load case 4)

Max Uplift 4=-22(load case 7), 7=-21(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-38/26, 2-3=-53/40, 3-4=-44/30

BOT CHORD 5-6=-36/69, 3-5=-36/69

**WEBS** 2-5=-32/49, 6-7=-99/59, 1-6=-55/30

#### JOINT STRESS INDEX

1 = 0.07, 2 = 0.13, 3 = 0.22, 5 = 0.03 and 6 = 0.00

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 30,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building design and proper incorporation and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	PB05	VALLEY	2	1	J1931016
			7		Job Reference (optional)

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### NOTES

- 5) Bearing at joint(s) 4, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 4 and 21 lb uplift at joint 7.
- 7) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

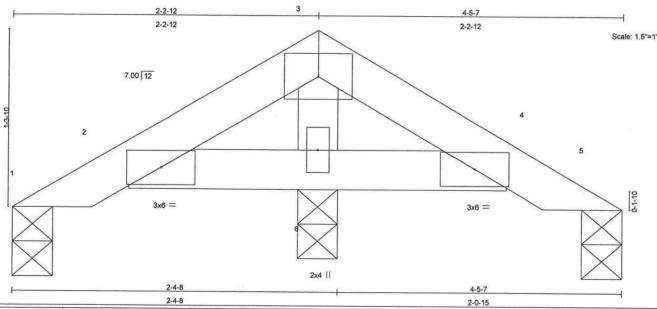
LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonda PE No. 24869 1109 Coestal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	PB08	VALLEY	1	1	J1931017
		4x6 =			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:45 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.03	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2	>999	240	20	2111100
BCLL	10.0	* Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mati	rix)			10.50			Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

**WEBS** 

2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

4-5-7 oc purlins.

**BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/size) 1=36/0-3-8, 5=36/0-3-8, 6=197/0-3-8

Max Horz 1=34(load case 5)

Max Uplift 1=-9(load case 6), 5=-16(load case 4), 6=-38(load case 6) Max Grav 1=45(load case 10), 5=45(load case 11), 6=197(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-28/34, 2-3=-23/79, 3-4=-23/79, 4-5=-19/15

**BOT CHORD** 

2-6=-46/55, 4-6=-46/55

**WEBS** 

3-6=-149/90

### JOINT STRESS INDEX

2 = 0.10, 3 = 0.07, 4 = 0.10 and 6 = 0.05

## **NOTES**

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 30,2008

Maming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component this installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	PB08	VALLEY	1	1	J1931017
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:45 2008 Page 2

### NOTES

- 5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 1, 16 lb uplift at joint 5 and 38 lb uplift at joint 6.
- 7) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job	Truss	Truss Ty	уре	Qty	Ply	LIP	SCOMB	& EAGLE	- HILL OF HUN	
L266613	PB17	VALLEY	′	2		1	_ 2			J1931018
Builders FirstS	ource, Lake City, FI 3205	5	6.300	) s Feb 15 200	6 MiTek	Job Industri	References, Inc. V	e (option Ved Jan	al) 30 11:44:46 2008	Page 1
-		7-5-12					14-11-7			ì
		7-5-12					7-5-12			Carlo - 4.07.0
				4x6 =						Scale = 1:27.9
Ī				4						
	7.00 12									
	7.50 12	2x4	//		/		2x4			
9		3				/	5			
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	2/							1. 108	6 7	
	3x6 =			ğ			×		3x6 =	를 다른
×		2x4		2x4			2x4		Σ	⊴
<b>—</b>	3-10-4	-	7-7-8		11-1-5				4-11-7	-1
	3-10-4		3-9-4		3-5-13			3	3-10-3	2.70
LOADING (ps		2-0-0 1.25	CSI TC 0.11	DEFL Vert(LL)	in -0.01	(loc) 6-8	l/defl >999	L/d	PLATES	GRIP
TCDL 7.	0 Lumber Increase		BC 0.07	Vert(TL)	-0.01	6-8	>999	360 240	MT20	244/190
BCLL 10. BCDL 5.		YES	WB 0.08 (Matrix)	Horz(TL)	0.00	7	n/a	n/a	\A/-:	
	0 00de 1 B02004/	1712002	(Watrix)						Weight: 56 II	0
LUMBER TOP CHORD	2 X 4 SYP No.2			BRACING TOP CHO		Structi	iral wood	shoothii	na diroctly appli	ad as
BOT CHORD	2 X 4 SYP No.2					6-0-0	oc purlins		ng directly appli	
OTHERS	2 X 4 SYP No.3			вот сно	RD	Rigid o	eiling dir	ectly app	lied or 6-0-0 oc	

bracing.

REACTIONS (lb/size) 1=61/0-3-8, 7=61/0-3-8, 9=296/0-3-8, 8=261/0-3-8, 10=261/0-3-8

Max Horz 1=119(load case 5)

Max Uplift 1=-24(load case 4), 7=-12(load case 7), 9=-17(load case 6), 8=-113(load

case 7), 10=-117(load case 6)

Max Grav 1=72(load case 10), 7=72(load case 11), 9=296(load case 1), 8=271(load

case 11), 10=271(load case 10)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-120/121, 2-3=-96/148, 3-4=-27/139, 4-5=-2/139, 5-6=-71/148, 6-7=-31/14

**BOT CHORD** 

2-10=-81/110, 9-10=-81/110, 8-9=-81/110, 6-8=-81/110

**WEBS** 

4-9=-274/83, 5-8=-213/185, 3-10=-213/185

### JOINT STRESS INDEX

2 = 0.24, 3 = 0.09, 4 = 0.22, 5 = 0.09, 6 = 0.24, 8 = 0.10, 9 = 0.10 and 10 = 0.10

## NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other

Continued on page 2

live loads.

January 30,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	PB17	VALLEY	2	1	J1931018
	A Section Address to	The state of the s			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:46 2008 Page 2

### NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 1, 12 lb uplift at joint 7, 17 lb uplift at joint 9, 113 lb uplift at joint 8 and 117 lb uplift at joint 10.
- 7) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EA	GLE - HILL OF HUNTS	
L266613	PB17G	VALLEY	1	1		J	11931019
Builders FirstSource,	Lake City, FI 32055	6.300	s Feb 15 2006 N	/liTek Ir	Job Reference (op	tional) an 30 11:44:47 2008 F	Dago 1
e film in it generale en her in in film little between the proposed film in it is an in the blooke of the			0 1 0 2 10 2 0 0 0 1		radotrico, mo. vved o	an 50 11.44.47 2000 P	age i
1	6-10-1: 6-10-1:		-1		13-9-9		
	0-10-1.	3	4x6 =		6-10-13	8	Scale = 1:24.
1 2 3x6 =		13	5 6		7	3x6 =	01-1-0
<b>—</b>	3-10-4	7-0-9		9-11-5		13-9-9	
	3-10-4	3-2-5		2-10-12		3-10-4	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 *	SPACING Plates Increase Lumber Increase Rep Stress Incr	2-0-0	Vert(TL)	0.01	(loc) I/defl L/ 2-14 >999 36 2-14 >999 24 9 n/a n/	0 MT20 0	<b>GRIP</b> 244/19
BCDL 5.0	Code FBC2004/TP		11012(11)	0.00	9 11/a 11/	Weight: 60 lb	
			BRACING TOP CHORI BOT CHORI	) ) F	Structural wood shea 5-0-0 oc purlins. Rigid ceiling directly bracing.	athing directly applied	or

REACTIONS (lb/size) 1=53/0-3-8, 9=53/0-3-8, 12=269/0-3-8, 14=246/0-3-8, 10=246/0-3-8

Max Horz 1=-137(load case 4)

Max Uplift 1=-25(load case 4), 9=-21(load case 7), 12=-51(load case 6), 14=-179(load

case 6), 10=-175(load case 7)

Max Grav 1=67(load case 10), 9=67(load case 11), 12=269(load case 1), 14=255(load

case 10), 10=255(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-136/138, 2-3=-109/170, 3-4=-39/129, 4-5=-16/133, 5-6=-3/133, 6-7=-8/129, TOP CHORD

7-8=-84/170, 8-9=-29/14

**BOT CHORD** 2-14=-103/124, 13-14=-103/124, 12-13=-103/124, 11-12=-103/124, 10-11=-103/124

, 8-10=-103/124

**WEBS** 5-12=-173/18, 3-14=-182/169, 4-13=-51/42, 6-11=-51/40, 7-10=-182/167

## **JOINT STRESS INDEX**

2 = 0.27, 3 = 0.09, 4 = 0.02, 5 = 0.05, 6 = 0.02, 7 = 0.09, 8 = 0.27, 10 = 0.09, 11 = 0.02, 12 = 0.06, 13 = 0.02 and 14 = 0.09

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	PB17G	VALLEY	1	1	J1931019
	1				Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:47 2008 Page 2

#### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1, 21 lb uplift at joint 9, 51 lb uplift at joint 12, 179 lb uplift at joint 14 and 175 lb uplift at joint 10.
- 8) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
- Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 24869 1109 Gestal Bay Blvd Bovnton Beach, Ft. 20406



Job '	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	PB20	VALLEY	2	1	J1931020
					Job Reference (optional)
Builders FirstSor	urce, Lake City, FI 3:	2055 6.3	00 s Apr 19 2006 N	ITek Ind	lustries, Inc. Wed Jan 30 12:37:07 2008 Page 1

16-8-1 8-4-0 8-4-0 Scale = 1:30.2 4x6 = 7.00 12 2x4 || 2x4 II 5  $\bigotimes_{10}$ 8  $\bigotimes$ 3x6 = 2x4 II 2x4 || 2x4 || 3-10-4 12-9-13 16-8-1 3-10-4 4-4-0 3-10-4 LOADING (psf) SPACING 2-0-0 CSI DEFL in **PLATES** GRIP (loc) I/defl L/d TCLL 20.0 Plates Increase 1.25 TC 0.14 Vert(LL) -0.016-8 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.09 Vert(TL) -0.016-8 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.11 Horz(TL) 0.01 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 62 lb LUMBER BRACING TOP CHORD

TOP CHORD 2 X 4 SYP No.2

**BOT CHORD** 2 X 4 SYP No.2

**WEBS** 2 X 4 SYP No.3

**OTHERS** 2 X 4 SYP No.3 Structural wood sheathing directly applied or 6-0-0

oc purlins.

**BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=65/0-3-8, 9=343/0-3-8, 10=289/0-3-8, 8=289/0-3-8, 7=65/0-3-8

Max Horz 1=133(load case 5)

Max Uplift 1=-28(load case 4), 9=-22(load case 6), 10=-130(load case 6), 8=-126(load case 7),

7=-11(load case 7)

Max Grav 1=76(load case 10), 9=343(load case 1), 10=301(load case 10), 8=301(load case

11), 7=76(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-135/136, 2-3=-109/158, 3-4=-32/153, 4-5=-2/153, 5-6=-77/158, 6-7=-33/13

**BOT CHORD** 

2-10=-86/116, 9-10=-86/116, 8-9=-86/116, 6-8=-86/116

WEBS

4-9=-313/97, 3-10=-238/204, 5-8=-238/204

## JOINT STRESS INDEX

2 = 0.27, 3 = 0.11, 4 = 0.33, 5 = 0.11, 6 = 0.27, 8 = 0.12, 9 = 0.11 and 10 = 0.12

### NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 30,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conner Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occi. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job '	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS, LOT 8
L266613	PB20	VALLEY	2	1	J1931020
			Ī		Job Reference (optional)

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### NOTES

- 5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1, 22 lb uplift at joint 9, 130 Ib uplift at joint 10, 126 lb uplift at joint 8 and 11 lb uplift at joint 7.
- 7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

em lesion Endineer Pis No. 34868 emaid Bay Blvd n Besch, FL 98496



Job Truss Truss Type Qty Ply LIPSCOMB & EAGLE - HILL OF HUNTS, LOT 8 J1931021 L266613 PB20G VALLEY 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:49 2008 Page 1 15-6-2 7-9-1 7-9-1 Scale = 1:27.0 4x6 = 7.00 12 X 3x6 = 13 11 3-10-4 7-7-5 11-7-14 15-6-2 3-10-4 3-9-1 4-0-9 3-10-4 LOADING (psf) **SPACING** 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.11 Vert(LL) 0.01 2-14 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.08 Vert(TL) -0.01 8-10 >999 240 BCLL 10.0 Rep Stress Incr WB YES 0.07 Horz(TL) 0.00 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 66 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins. **WEBS** 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc **OTHERS** 2 X 4 SYP No.3 bracing. (lb/size) 1=61/0-3-8, 9=61/0-3-8, 12=319/0-3-8, 14=267/0-3-8, 10=267/0-3-8 REACTIONS Max Horz 1=154(load case 5)

Max Uplift 1=-34(load case 4), 9=-20(load case 7), 12=-66(load case 6), 14=-197(load

case 6), 10=-192(load case 7)

Max Grav 1=71(load case 10), 9=71(load case 11), 12=319(load case 1), 14=279(load case 10), 10=279(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-156/157, 2-3=-124/144, 3-4=-52/98, 4-5=-24/128, 5-6=-0/128, 6-7=-4/98,

7-8=-72/144, 8-9=-31/14

**BOT CHORD** 2-14=-80/112, 13-14=-80/112, 12-13=-80/112, 11-12=-80/112, 10-11=-80/112,

8-10=-80/112

**WEBS** 5-12=-233/39, 3-14=-192/175, 4-13=-47/53, 6-11=-47/52, 7-10=-192/173

### JOINT STRESS INDEX

2 = 0.28, 3 = 0.09, 4 = 0.03, 5 = 0.15, 6 = 0.03, 7 = 0.09, 8 = 0.28, 10 = 0.10, 11 = 0.03, 12 = 0.08, 13 = 0.03 and 14 = 0.10

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	PB20G	VALLEY	1	1	J1931021
	D				Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:49 2008 Page 2

#### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 1, 20 lb uplift at joint 9, 66 lb uplift at joint 12, 197 lb uplift at joint 14 and 192 lb uplift at joint 10.
- 8) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
- Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Plonda PE No. 24869 1 109 Coastal Bay Blvd



Job Truss Truss Type LIPSCOMB & EAGLE - HILL OF HUNTS, LOT 8 Qty Ply J1931022 L266613 **PB21** VALLEY 18 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:50 2008 Page 1 9-10-11 19-9-5 9-10-11 9-10-11 Scale = 1:33.8 4x6 = 5 7.00 12 X 3 3x6 = 12 3-10-4 7-10-4 15-11-1 19-9-5 3-10-4 4-0-0 4-0-13 4-0-0 3-10-4 LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP in TCLL 20.0 Plates Increase 1.25 TC 0.15 Vert(LL) -0.01 8-10 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.08 Vert(TL) -0.01 8-10 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.07 Horz(TL) 0.01 9 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 85 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins. **OTHERS** 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. REACTIONS (lb/size) 1=90/0-3-8, 10=281/0-3-8, 11=253/0-3-8, 14=281/0-3-8, 13=253/0-3-8, 9=90/0-3-8 Max Horz 1=-158(load case 4) Max Uplift 1=-20(load case 4), 10=-118(load case 7), 11=-64(load case 7), 14=-123(load case 6), 13=-75(load case 6) Max Grav 1=91(load case 10), 10=285(load case 11), 11=253(load case 1), 14=285(load case 10), 13=253(load case 1), 9=91(load case 11) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-165/154, 2-3=-139/77, 3-4=-69/72, 4-5=-32/84, 5-6=-32/84, 6-7=-43/72, 7-8=-132/77, 8-9=-40/0 **BOT CHORD** 2-14=-17/160, 13-14=-17/160, 12-13=-17/160, 11-12=-17/160, 10-11=-17/160, 8-10=-17/160 **WEBS** 5-12=-31/1, 7-10=-225/198, 6-11=-201/153, 3-14=-225/198, 4-13=-201/153 2 = 0.27, 3 = 0.33, 4 = 0.33, 5 = 0.26, 6 = 0.33, 7 = 0.33, 8 = 0.27, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33 and 14 = 0.33

### NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	PB21	VALLEY	18	1	J1931022
		The second secon			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:50 2008 Page 2

#### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 1, 118 lb uplift at joint 10, 64 lb uplift at joint 11, 123 lb uplift at joint 14 and 75 lb uplift at joint 13.
- 8) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job Truss Truss Type Qty Ply LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8 J1931023 L266613 PB21G VALLEY 2 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:51 2008 Page 1 9-3-11 18-7-7 9-3-11 9-3-11 Scale: 3/8"=1 4x6 = 6 5 7.00 12 3x6 =  $\bigotimes$ 17 15 13 7-10-4 10-9-3 14-9-3 18-7-7 4-0-0 2-10-15 4-0-0 3-10-4 LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** in GRIP TCLL 20.0 Plates Increase 1.25 TC 0.11 Vert(LL) 0.01 13 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.08 Vert(TL) -0.01 10-12 >999 240 **BCLL** 10.0 Rep Stress Incr YES WB 0.05 Horz(TL) 0.00 11 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 89 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins. **WEBS** 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc **OTHERS** 2 X 4 SYP No.3 bracing. REACTIONS (lb/size) 1=66/0-3-8, 11=66/0-3-8, 12=279/0-3-8, 14=243/0-3-8, 18=279/0-3-8. 16=243/0-3-8 Max Horz 1=-186(load case 4) Max Uplift 1=-35(load case 4), 12=-201(load case 7), 14=-91(load case 7), 18=-205(load case 6), 16=-109(load case 6) Max Grav 1=75(load case 10), 11=75(load case 11), 12=283(load case 11), 14=243(load case 1), 18=283(load case 10), 16=243(load case 1) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-191/186, 2-3=-154/129, 3-4=-84/96, 4-5=-55/116, 5-6=-10/89, 6-7=-4/87. 7-8=0/116, 8-9=-15/80, 9-10=-116/129, 10-11=-33/4 2-18=-66/152, 17-18=-66/152, 16-17=-66/152, 15-16=-66/152, 14-15=-66/152, **BOT CHORD** 13-14=-66/152, 12-13=-66/152, 10-12=-66/152 s Less 2 Cesign Engineer 3a PE No. 34866 1 Cassal Rey Elvel Mon Beach, FL 20406 **WEBS** 6-15=-74/8, 9-12=-194/175, 7-14=-150/92, 3-18=-194/177, 5-16=-150/101, 4-17=-48/61, 8-13=-48/61 JOINT STRESS INDEX

2 = 0.29, 3 = 0.33, 4 = 0.33, 5 = 0.33, 6 = 0.26, 7 = 0.33, 8 = 0.33, 9 = 0.33, 10 = 0.29, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33 and 18 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	PB21G	VALLEY	2	1	J1931023
				1130	Job Reference (optional)

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#### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Bearing at joint(s) 1, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1, 201 lb uplift at joint 12, 91 lb uplift at joint 14, 205 lb uplift at joint 18 and 109 lb uplift at joint 16.
- 8) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
- Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonds PE No. 34889 1 109 Coestel Bey Blvd Boynon Besch



Job	Truss	Truss Type	Q	ty Ply	LIPSCOMB & EAGLE - HI	ILL OF HUNTS. LOT 8 J19310
.266613	T01G	GABLE	1	1	Inh Deference (anti-col)	010010
Builders FirstSou	rce, Lake City, FI 3	2055	6.300 s Feb 15 200	06 MiTek Ind	Job Reference (optional) dustries, Inc. Wed Jan 30 1	1:44:52 2008 Page 1
-1-6-0	-	5-7-8			11-3-0	12-9-0
1-6-0		5-7-8			5-7-8	1-6-0 Scale = 1:2
			4x6 = 5			
Ī			•			
3-3-10	3x6 = 3	7.00 12 2x4 II		2x4	  6   3x6 ≈  7 3x6 ≈	8
	3	x6 = 4	13 2x4	B	3x6 \Rightarrow 7 3x6 \Rightarrow 1	9

Plate Of	ffsets (X,)	(): [2:0-3-8,Edge], [2:	0-0-7,Edg	e], [8:0-	3-8,Edge	e], [8:0-0-7,Ed	ge]					
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.00	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	9	n/r	90		2111100
BCLL	10.0	* Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)		VEC11001001	360	1.80.30	3.00,560	Weight: 55 lb	

11-3-0

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No 2

**OTHERS** 

2 X 4 SYP No.2 2 X 4 SYP No.3

# BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 2=188/11-3-0, 8=188/11-3-0, 11=111/11-3-0, 12=198/11-3-0, 10=198/11-3-0

Max Horz 2=108(load case 5)

Max Uplift 2=-141(load case 6), 8=-154(load case 7), 12=-115(load case 6),

10=-118(load case 7)

Max Grav 2=191(load case 10), 8=191(load case 11), 11=111(load case 1), 12=200(load case 10), 10=200(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/39, 2-3=-65/66, 3-4=-58/78, 4-5=-17/86, 5-6=-17/86, 6-7=-16/73, 7-8=-23/26,

8-9=0/39

BOT CHORD 2-12=-24/106, 11-12=-24/106, 10-11=-24/106, 8-10=-24/106

WEBS 5-11=-104/6, 4-12=-163/135, 6-10=-163/137

Julius Lee Truss Cesign Engineer Florida PE No. 34869 1 100 Ceastal Ray Blyd Boynton Beach, FL 33436

## JOINT STRESS INDEX

2 = 0.21, 2 = 0.00, 3 = 0.00, 3 = 0.16, 3 = 0.16, 4 = 0.07, 5 = 0.06, 6 = 0.07, 7 = 0.00, 7 = 0.16, 7 = 0.16, 8 = 0.21, 8 = 0.00, 10 = 0.08, 11 = 0.04 and 12 = 0.08

### NOTES

Unbalanced roof live loads have been considered for this design.

Continued on page 2

January 30,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors, Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult 8CSI-1 or HIB-91 Handling Installing and Bracing Recommendation availed from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T01G	GABLE	1	1	J1931024
					Job Reference (optional)

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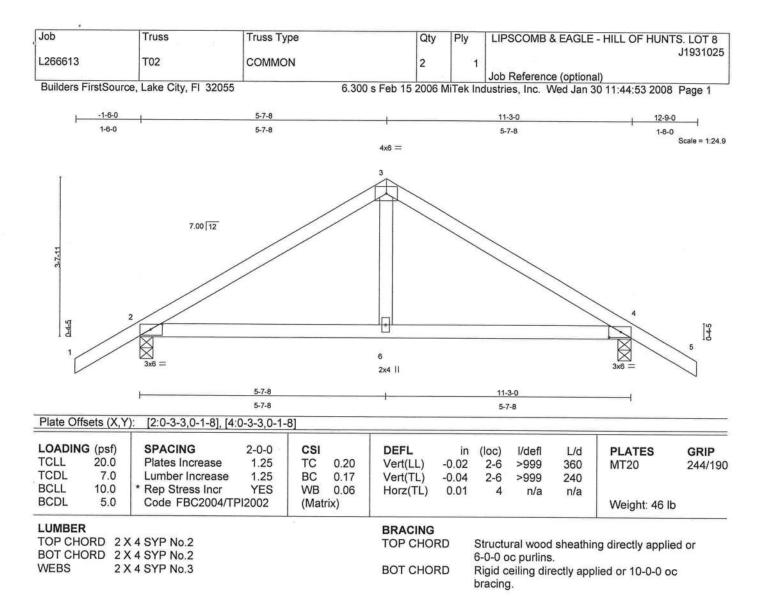
#### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 2, 154 lb uplift at joint 8, 115 lb uplift at joint 12 and 118 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda FE No. 34869 1 100 Ceestal Bay Blvd Boynton Beach, Ft. 33436





REACTIONS (lb/size) 2=440/0-3-8, 4=440/0-3-8

Max Horz 2=-91(load case 4)

Max Uplift 2=-150(load case 6), 4=-150(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-452/187, 3-4=-452/187, 4-5=0/40

**BOT CHORD** 2-6=-22/325, 4-6=-22/325

**WEBS** 3-6=0/189

### JOINT STRESS INDEX

2 = 0.31, 3 = 0.51, 4 = 0.31 and 6 = 0.13

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 30,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occ. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS, LOT 8
L266613	T02	COMMON	2	1	J1931025
					Job Reference (optional)

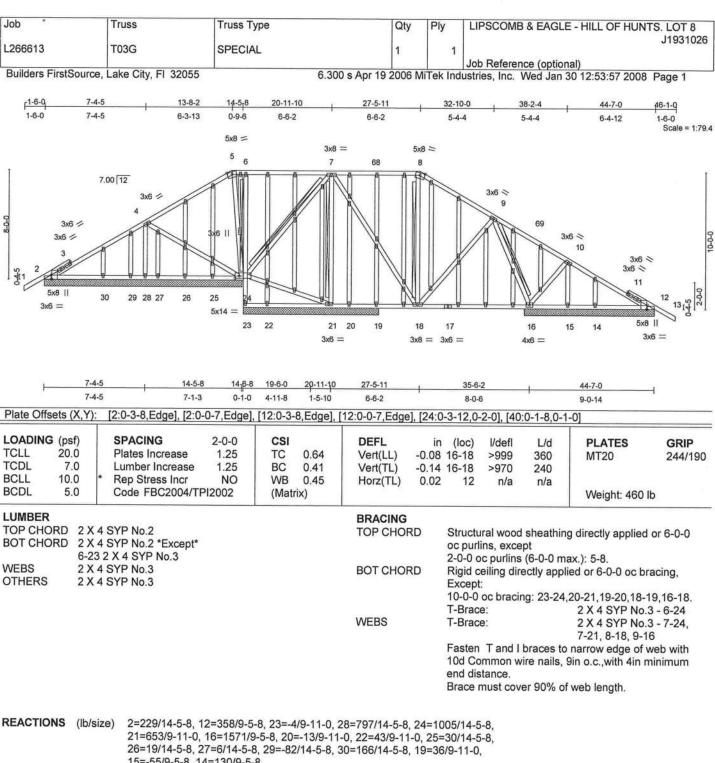
6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:53 2008 Page 2

### **NOTES**

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 2 and 150 lb uplift at joint 4.

LOAD CASE(S) Standard





15=-55/9-5-8, 14=130/9-5-8

Max Horz 2=-333(load case 4)

Max Uplift 2=-120(load case 6), 12=-205(load case 7), 23=-10(load case 2), 28=-484(load case 6), 24=-602(load case 5), 21=-350(load case 4), 16=-757(load case 7), 20=-14(load case 11), 29=-82(load case 10), 30=-59(load case 6), 19=-14(load

case 4), 15=-55(load case 11), 14=-26(load case 7)

Max Grav 2=234(load case 10), 12=361(load case 11), 23=4(load case 7), 28=813(load case 3) 10), 24=1005(load case 1), 21=658(load case 11), 16=1588(load case 11), 20=22(load case 2), 22=128(load case 2), 25=71(load case 2), 26=59(load case 2), 27=46(load case 2), 29=62(load case 6), 30=167(load case 10), 19=37(load case January 30,2008

11), 15=22(load case 7), 14=149(load case 2) Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conne Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building does. For general guidance regarding storage, delivery, erec and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T03G	SPECIAL	1	1	J1931026
5 111 51 10					Job Reference (optional)

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-5/63, 2-3=-306/242, 3-4=-322/507, 4-5=-133/342, 5-6=-79/242, 6-7=-74/235, 7-68=-305/356, 8-68=-305/356,

8-9=-508/358, 9-69=-127/514, 10-69=-96/264, 10-11=-47/166, 11-12=-88/48, 12-13=-5/63

2-30=-301/362, 29-30=-301/362, 28-29=-301/362, 27-28=-301/362, 26-27=-301/362, 25-26=-301/362, 24-25=-301/362, **BOT CHORD** 

23-24=0/0, 6-24=-365/318, 22-23=-8/9, 21-22=-8/9, 20-21=-104/250, 19-20=-104/250, 18-19=-104/250, 17-18=-105/156,

16-17=-105/156, 15-16=-24/158, 14-15=-24/158, 12-14=-24/158

**WEBS** 4-28=-760/462, 4-24=-42/147, 5-24=-427/259, 21-24=-108/253, 7-24=-444/290, 7-21=-638/345, 7-18=-164/437,

8-18=-498/238, 9-18=-98/256, 9-16=-1226/592, 10-16=-469/340

#### JOINT STRESS INDEX

2 = 0.52, 2 = 0.21, 3 = 0.00, 3 = 0.43, 3 = 0.43, 4 = 0.42, 5 = 0.99, 6 = 0.40, 7 = 0.59, 8 = 0.91, 9 = 0.43, 10 = 0.46, 11 = 0.00, 11 = 0.41. 11 = 0.41, 12 = 0.43, 12 = 0.21, 14 = 0.34, 15 = 0.34, 16 = 0.38, 17 = 0.22, 18 = 0.59, 19 = 0.34, 20 = 0.34, 21 = 0.35, 22 = 0.34, 23 = 0.34, 24 = 0.96, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 34 = 0.34, 34 = 0.34, 35 = 0.34, 3635 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.47, 40 = 0.34, 41 = 0.16, 42 = 0.34, 43 = 0.34, 40 = 0.34, 40 = 0.47, 40 = 0.34, 400.34, 44 = 0.34, 45 = 0.34, 45 = 0.34, 46 = 0.34, 47 = 0.34, 48 = 0.34, 49 = 0.34, 50 = 0.34, 51 = 0.34, 51 = 0.34, 52 = 0.34, 53 = 0.34, 53 = 0.34, 51 = 0.34, 51 = 0.34, 51 = 0.34, 52 = 0.34, 5354 = 0.34, 54 = 0.34, 55 = 0.34, 56 = 0.34, 57 = 0.34, 58 = 0.34, 59 = 0.34, 59 = 0.34, 60 = 0.34, 61 = 0.34, 62 = 0.34, 63 = 0.34, 640.34, 64 = 0.34, 65 = 0.34, 66 = 0.34 and 67 = 0.34

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 2, 205 lb uplift at joint 12, 10 lb uplift at joint 23, 484 lb uplift at joint 28, 602 lb uplift at joint 24, 350 lb uplift at joint 21, 757 lb uplift at joint 16, 14 lb uplift at joint 20, 82 lb uplift at joint 29, 59 lb uplift at joint 30, 14 lb uplift at joint 19, 55 lb uplift at joint 15 and 26 lb uplift at joint 14.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-87(F=-33), 5-68=-87(F=-33), 8-68=-114(F=-60), 8-69=-114(F=-60), 13-69=-87(F=-33), 2-24=-10, 12-23=-10

esian Engineer



Job Truss Truss Type Qty Ply LIPSCOMB & EAGLE - HILL OF HUNTS, LOT 8 J1931027 T04 L266613 **SPECIAL** 2 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:56 2008 Page 1 -1-6-0 14-6-8 21-3-1 28-0-11 38-9-3 44-7-0 46-1-0 1-6-0 6-9-6 6-3-13 1-5-5 6-8-9 6-9-9 5-4-4 5-9-13 1-6-0 Scale = 1:82.0 5x6 = 2x4 || 5x14 = 4 5 7 7.00 12 5x6 > 8x14 = 18 2x4 || 16 15 14 12 2x4 || 3x8 = 14-2-0 14-6-8 21-3-1 28-0-11 36-1-1 44-7-0 6-9-6 7-4-10 0-4-8 6-8-9 6-9-9 8-0-6 8-5-15 Plate Offsets (X,Y): [2:0-3-4,0-1-8], [9:0-3-0,0-3-0], [10:0-6-0,0-0-6], [17:0-4-12,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl **PLATES** GRIP in (loc) L/d Plates Increase TCLL 20.0 1.25 TC 0.38 Vert(LL) >999 360 0.11 17-18 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.39 >999 Vert(TL) -0.20 10-12 240 BCLL 10.0 \* Rep Stress Incr YES WB 0.61 Horz(TL) 0.02 10 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 286 lb LUMBER BRACING Structural wood sheathing directly applied or TOP CHORD 2 X 4 SYP No.2 TOP CHORD BOT CHORD 2 X 4 SYP No.2 \*Except\* 5-2-5 oc purlins, except 5-16 2 X 4 SYP No.3 2-0-0 oc purlins (6-0-0 max.): 4-7. **WEBS** 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. Except: T-Brace: 2 X 4 SYP No.3 -5-17 **WEBS** T-Brace: 2 X 4 SYP No.3 -3-17, 6-17, 7-15, 8-14 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length. REACTIONS (lb/size) 2=300/0-3-8, 17=1796/0-3-8, 10=916/0-3-8 Max Horz 2=-265(load case 4)

Max Uplift 2=-286(load case 6), 17=-632(load case 5), 10=-269(load case 7)
Max Grav 2=367(load case 10), 17=1796(load case 1), 10=942(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-275/309, 3-4=-187/522, 4-5=-123/469, 5-6=-123/464, 6-7=-284/286,

7-8=-720/431, 8-9=-1191/572, 9-10=-1384/582, 10-11=0/40

BOT CHORD 2-18=-288/193, 17-18=-288/193, 16-17=0/92, 5-17=-281/181, 15-16=-13/85,

14-15=0/569, 13-14=-159/843, 12-13=-159/843, 10-12=-357/1122

WEBS 3-18=-283/243, 3-17=-486/654, 4-17=-426/237, 15-17=-56/307, 6-17=-1061/631,

Continued on page 25=-210/456, 7-15=-528/260, 7-14=-199/430, 8-14=-436/318, 8-12=-139/356,

Julius Lee Truse Design Engineer Flonda PE No. 24869 1 109 Ceastel Rey Blvd Bovnton Besch, Ft. 33435





Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T04	SPECIAL	2	1	J1931027
	105 30		-		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:44:56 2008 Page 2

### JOINT STRESS INDEX

2 = 0.40, 3 = 0.40, 4 = 0.61, 5 = 0.33, 6 = 0.38, 7 = 1.00, 8 = 0.40, 9 = 0.40, 10 = 0.66, 12 = 0.47, 13 = 0.28, 14 = 0.38, 15 = 0.57, 16 = 0.83, 17 = 0.37 and 18 = 0.33

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 632 lb uplift at joint 17 and 269 lb uplift at joint 10.

LOAD CASE(S) Standard

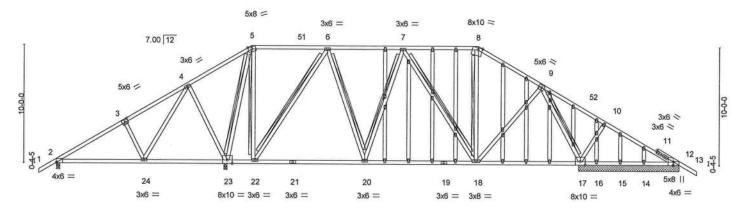
Julius Lee Truss Design Engineer Flonda PE No. 34869 1 100 Coesial Bay Slvd



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T05G	HIP	1	1	J1931028
					Job Reference (optional)

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7-4-11	14-3-12	16-6-5	26-1-8	35-8-11	44-5-4	52-10-0
7-4-11	6-11-1	2-2-9	9-7-3	9-7-3	8-8-9	8-4-12

Plate Of	fsets (X,Y):	[2:Edge,0-0-4], [3:0-	-3-0,0-3-0]	[8:0-4-1	,Edge], [1	[2:0-3-8,Edge]	, [12:0-0	)-7,Edg	e]			
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)		20-22	>983	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.29	20-22	>999	240	20	2111100
BCLL	10.0	Rep Stress Incr	NO	WB	0.98	Horz(TL)	-0.05	17	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	,					Weight: 434 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

OTHERS 2 X 4 SYP No.3

BRACING

WEBS

TOP CHORD

Structural wood sheathing directly applied or 5-1-8

oc purlins, except

2-0-0 oc purlins (5-5-3 max.): 5-8.

BOT CHORD Rigid ceiling directly applied or 5-5-1 oc bracing.

T-Brace:

2 X 4 SYP No.3 - 5-23, 5-22, 6-20, 7-20, 7-18,

8-18, 9-17

-18, 9-17

2 X 6 SYP No.1D - 6-22

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size)

(lb/size) 12=202/8-6-8, 2=406/0-3-8, 23=2115/0-3-8, 17=2515/8-6-8, 16=-44/8-6-8,

15=-8/8-6-8, 14=119/8-6-8

Max Horz 2=-342(load case 4)

Max Uplift 12=-153(load case 7), 2=-340(load case 6), 23=-2053(load case 5), 17=-2427(load

case 4), 16=-119(load case 2), 15=-18(load case 5), 14=-56(load case 7)

Max Grav 12=206(load case 11), 2=414(load case 10), 23=2115(load case 1), 17=2533(load case 11), 16=162(load case 5), 15=70(load case 2), 14=119(load case 11)

Julius Lee Truss Design Engineer Flonda PE No. 3-1869 1-109 Coestel Bay Blyd Boyston Basson Et 20405

January 30,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, Wi 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, Wi 53719.



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T05G	HIP	1	1	J1931028
Dullidere Fire 10					Job Reference (optional)

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FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/40, 2-3=-376/307, 3-4=-242/352, 4-5=-255/355, 5-51=-214/402, 6-51=-214/402, 6-7=-1113/1464, 7-8=-993/1338, TOP CHORD

8-9=-1297/1545, 9-52=-559/749, 10-52=-433/487, 10-11=-415/454, 11-12=-344/231, 12-13=-21/63

**BOT CHORD** 2-24=-291/259, 23-24=-77/286, 22-23=-232/196, 21-22=-1011/957, 20-21=-1011/957, 19-20=-1288/1235,

18-19=-1288/1235, 17-18=-508/524, 16-17=-275/447, 15-16=-275/447, 14-15=-275/447, 12-14=-275/447

3-24=-265/264, 4-24=-571/362, 4-23=-422/512, 5-23=-1713/1828, 5-22=-1507/1205, 6-22=-1389/1544, 6-20=-721/526, 7-20=-418/358, 7-18=-475/531, 8-18=-115/73, 9-18=-766/730, 9-17=-2272/2350, 10-17=-467/465

### JOINT STRESS INDEX

2 = 0.26, 3 = 0.45, 4 = 0.36, 5 = 0.60, 6 = 0.93, 7 = 0.50, 8 = 0.93, 9 = 0.75, 10 = 0.34, 11 = 0.00, 11 = 0.37, 11 = 0.37, 12 = 0.43, 12 = 00.12, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.44, 18 = 0.73, 19 = 0.46, 20 = 0.50, 21 = 0.47, 22 = 0.93, 23 = 0.34, 24 = 0.46, 25 = 0.46, 2526 = 0.34, 27 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 3 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.34, 41 = 0.34, 42 = 0.34, 43 = 0.34, 44 = 0.34, 44 = 0.34, 44 = 0.34, 44 = 0.34, 4545 = 0.34, 46 = 0.34, 47 = 0.34, 48 = 0.34, 48 = 0.34, 49 = 0.34 and 50 = 0.34

#### NOTES

WEBS

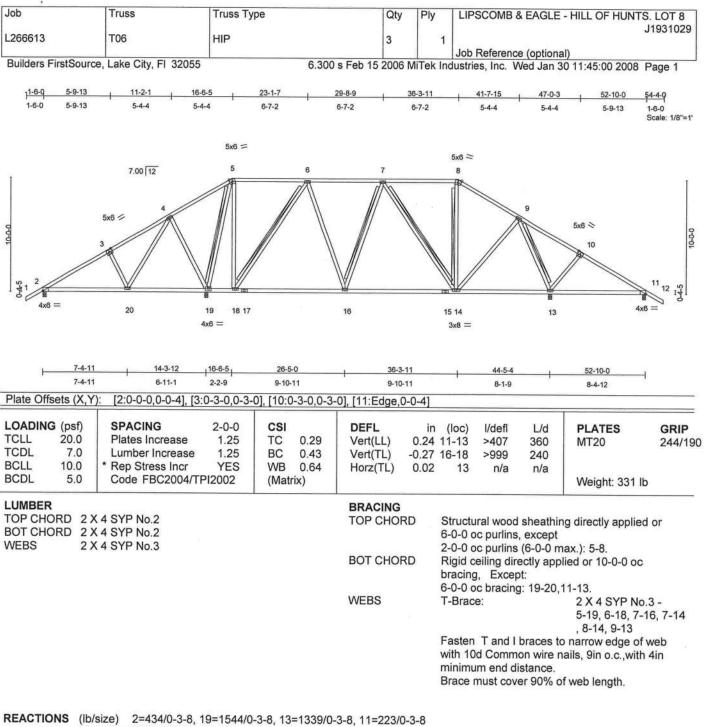
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 12, 340 lb uplift at joint 2, 2053 lb uplift at joint 23, 2427 lb uplift at joint 17, 119 lb uplift at joint 16, 18 lb uplift at joint 15 and 56 lb uplift at joint 14.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-54, 5-51=-54, 8-51=-114(F=-60), 8-52=-114(F=-60), 13-52=-87(F=-33), 2-12=-10





Max Horz 2=-268(load case 4)

Max Uplift 2=-271(load case 6), 19=-743(load case 5), 13=-482(load case 4),

11=-223(load case 7)

Max Grav 2=440(load case 10), 19=1544(load case 1), 13=1356(load case 11), 11=229(load case 11)

January 30,2008

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T06	HIP	3	1	J1931029
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:00 2008 Page 2

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-426/364, 3-4=-292/409, 4-5=-172/303, 5-6=-129/146, 6-7=-613/421, 7-8=-508/407, 8-9=-657/401,

9-10=-44/334, 10-11=-71/169, 11-12=0/40

BOT CHORD 2-20=-283/302, 19-20=-37/214, 18-19=0/335, 17-18=-123/532, 16-17=-123/532, 15-16=-155/658, 14-15=-155/658

, 13-14=0/221, 11-13=-122/118

3-20=-265/254, 4-20=-574/366, 4-19=-423/512, 5-19=-1144/506, 5-18=-246/675, 6-18=-747/432, 6-16=-110/311,

7-16=-166/165, 7-14=-302/158, 8-14=-45/107, 9-14=-122/447, 9-13=-1129/466, 10-13=-283/284

### JOINT STRESS INDEX

2 = 0.25, 3 = 0.45, 4 = 0.35, 5 = 0.57, 6 = 0.48, 7 = 0.48, 8 = 0.47, 9 = 0.40, 10 = 0.58, 11 = 0.42, 13 = 0.48, 14 = 0.57, 15 = 0.31, 16 = 0.48, 17 = 0.23, 18 = 0.43, 19 = 0.30 and 20 = 0.45

### NOTES

**WEBS** 

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 3x6 MT20 unless otherwise indicated.

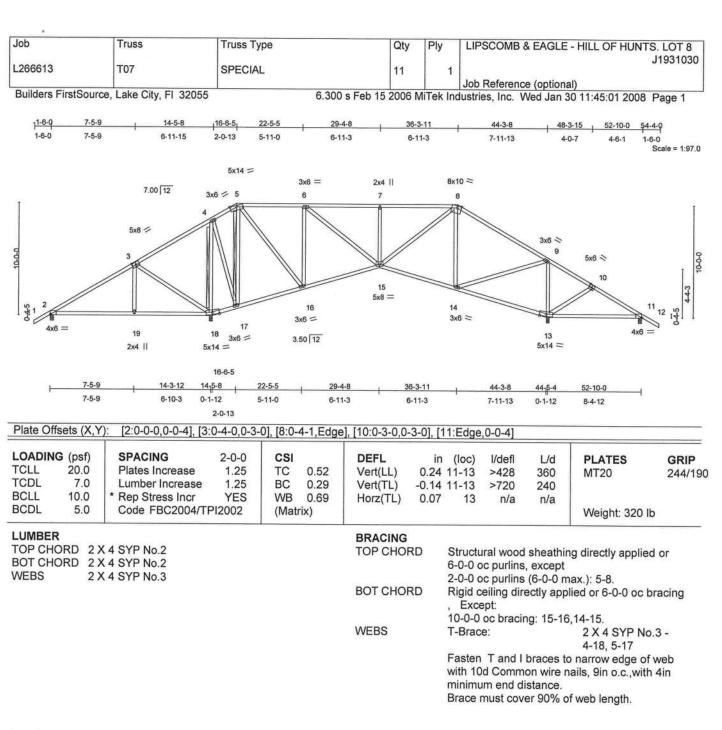
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 271 lb uplift at joint 2, 743 lb uplift at joint 19, 482 lb uplift at joint 13 and 223 lb uplift at joint 11.

LOAD CASE(S) Standard

dulius Les Truse Design Engineer Florida PE No. 248eb 1 100 Coastel Bay Blvd Boynton Beach, FL 30435





**REACTIONS** (lb/size) 2=295/0-3-8, 18=1705/0-3-8, 13=1467/0-3-8, 11=73/0-3-8

Max Horz 2=268(load case 5)

Max Uplift 2=-285(load case 6), 18=-773(load case 5), 13=-505(load case 4),

11=-245(load case 7)

Max Grav 2=314(load case 10), 18=1705(load case 1), 13=1488(load case 11), 11=73(load case 11)

Julius Lee Truss Cesian Engineer Plonda PE No. 34889 1 108 Cesstel Bay Blvd Boynton Beach, Ft. 22436

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T07	SPECIAL	11	1	J1931030
			1000		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:01 2008 Page 2

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-144/288, 3-4=-259/556, 4-5=-92/211, 5-6=-407/281, 6-7=-853/393, 7-8=-853/393, 8-9=-636/309,

9-10=-132/568, 10-11=-143/407, 11-12=0/40

BOT CHORD 2-19=-209/174, 18-19=-205/177, 17-18=-457/515, 16-17=-197/431, 15-16=-102/433, 14-15=-43/474,

13-14=-490/327, 11-13=-329/119

3-19=-288/240, 3-18=-495/657, 4-18=-1249/509, 4-17=-260/893, 5-17=-913/342, 5-16=-334/834, 6-16=-726/356, 6-15=-148/571, 7-15=-384/244, 8-15=-220/529, 8-14=-475/182, 9-14=-190/958, 9-13=-1164/487, 10-13=-181/199

# JOINT STRESS INDEX

2 = 0.34, 3 = 0.64, 4 = 0.71, 5 = 0.55, 6 = 0.34, 7 = 0.33, 8 = 0.81, 9 = 0.51, 10 = 0.23, 11 = 0.53, 13 = 0.60, 14 = 0.50, 15 = 0.56, 16 = 0.64, 17 = 0.81, 18 = 0.69 and 19 = 0.33

#### NOTES

**WEBS** 

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

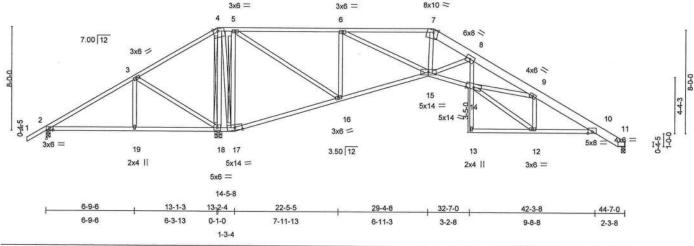
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 2, 773 lb uplift at joint 18, 505 lb uplift at joint 13 and 245 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Las Truss Cesign Engineer Florida PE No. 24868 1400 Coesial Bay Blvd 400nton Beach, FL 22425



Ply Job Truss Truss Type LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8 Qty J1931031 L266613 T08 SPECIAL 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:02 2008 Page 1 r1-6-0 6-9-6 13-1-3 14-5-8 22-5-5 44-7-0 46-1-9 29-9-4 32-7-0 37-5-4 42-3-8 1-6-0 6-9-6 6-3-13 1-4-5 7-11-13 7-3-15 2-9-12 4-10-4 4-10-4 2-3-8 1-6-0 Scale = 1:83.5 8x10 = 8x10 = 3x6 = 3x6 = 5 8 7



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.17	14	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.32	14-15	>999	240		211110
BCLL	10.0	* Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.19	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Mat	rix)						Weight: 286 lb	

COMPLIX	
TOP CHORD	2 X 4 SYP No.2 *Except*

LIMBED

7-11 2 X 8 SYP 2400F 2.0E

**BOT CHORD** 2 X 4 SYP No.2 \*Except\*

8-13 2 X 4 SYP No.3 **WEBS** 2 X 4 SYP No.3

BRACING

**BOT CHORD** 

Structural wood sheathing directly applied or TOP CHORD

6-0-0 oc purlins, except

2-0-0 oc purlins (5-10-0 max.): 4-7.

Rigid ceiling directly applied or 5-4-1 oc

bracing. Except:

1 Row at midpt 10-13

**WEBS** T-Brace:

2 X 4 SYP No.3 -4-18, 5-17

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 11=682/0-3-8, 2=-244/0-3-8, 18=2488/0-6-7

Max Horz 2=227(load case 5)

Max Uplift 11=-170(load case 7), 2=-547(load case 11), 18=-633(load case 5) Max Grav 11=694(load case 11), 2=7(load case 10), 18=2488(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/40, 2-3=-161/1262, 3-4=-535/1515, 4-5=-301/1056, 5-6=-179/152, TOP CHORD

6-7=-1015/397, 7-8=-1135/397, 8-9=-1905/710, 9-10=-1342/622, 10-11=-299/157

**BOT CHORD** 2-19=-1057/339, 18-19=-1057/339, 17-18=-1276/772, 16-17=-1126/726,

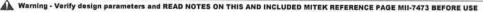
15-16=-173/275, 14-15=-359/1600, 13-14=0/66, 8-14=-222/550, 12-13=-1/22,

10-12=-479/1229

**WEBS** 3-19=-250/210, 3-18=-448/595, 4-18=-2110/862, 4-17=-518/1347, 5-17=-1084/496,

Continued on page 26=-515/1268, 6-16=-906/446, 6-15=-308/1085, 7-15=0/252, 8-15=-605/354.

January 30,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	Т08	SPECIAL	1	1	J1931031
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:02 2008 Page 2

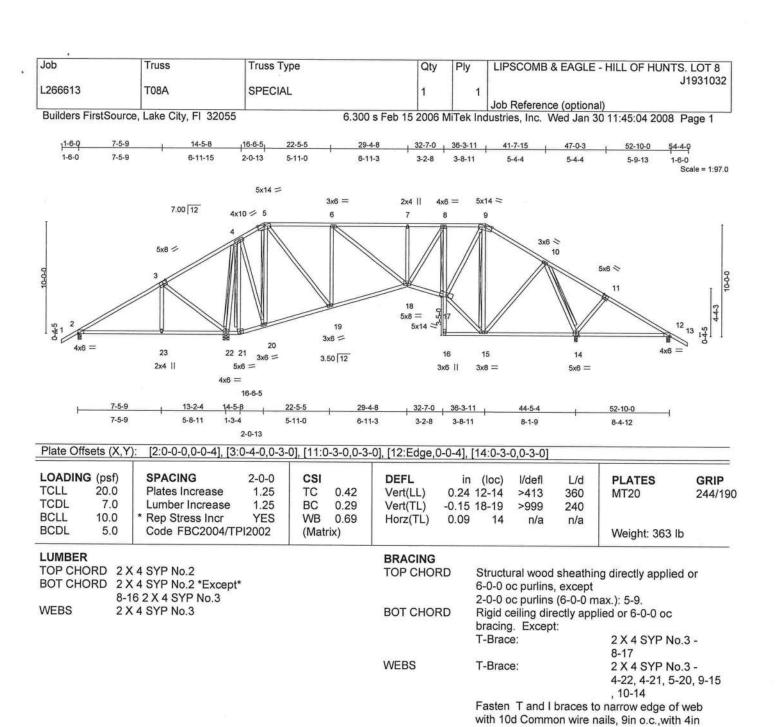
### JOINT STRESS INDEX

2 = 0.48, 3 = 0.40, 4 = 0.76, 4 = 0.00, 5 = 0.72, 6 = 0.60, 7 = 0.81, 8 = 0.15, 9 = 0.30, 10 = 0.41, 12 = 0.86, 13 = 0.50, 14 = 0.86, 13 = 0.50, 14 = 0.86, 13 = 0.0.44, 15 = 0.50, 16 = 0.76, 17 = 0.56, 18 = 0.36 and 19 = 0.33

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 11, 547 lb uplift at joint 2 and 633 lb uplift at joint 18.

LOAD CASE(S) Standard





minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=244/0-3-8, 14=1530/0-3-8, 22=1716/0-6-7, 12=50/0-3-8

Max Horz 2=268(load case 5)

Max Uplift 2=-273(load case 6), 14=-520(load case 4), 22=-749(load case 5),

12=-243(load case 7)

Max Grav 2=265(load case 10), 14=1538(load case 11), 22=1716(load case 1), 12=52(load case 11)

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T08A	SPECIAL	1	1	J1931032
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:04 2008 Page 2

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-61/314, 3-4=-269/604, 4-5=-35/187, 5-6=-503/376, 6-7=-937/479, 7-8=-937/479, 8-9=-708/438,

9-10=-487/392, 10-11=-146/665, 11-12=-174/482, 12-13=0/40

BOT CHORD 2-23=-242/184, 22-23=-244/187, 21-22=-297/381, 20-21=-302/394, 19-20=-65/328, 18-19=-150/533,

17-18=-157/743, 16-17=-18/15, 8-17=-503/223, 15-16=-6/7, 14-15=-66/143, 12-14=-385/178
WEBS 3-23=-268/221, 3-22=-470/616, 4-22=-1305/560, 4-21=-43/46, 4-20=-271/875, 5-20=-830/299

3-23=-268/221, 3-22=-470/616, 4-22=-1305/560, 4-21=-43/46, 4-20=-271/875, 5-20=-830/299, 5-19=-325/826, 6-19=-723/353, 6-18=-131/553, 7-18=-304/221, 8-18=-157/427, 15-17=-35/478, 9-17=-244/733, 9-15=-696/216,

10-15=-157/548, 10-14=-1319/536, 11-14=-285/283

### JOINT STRESS INDEX

2 = 0.33, 3 = 0.64, 4 = 0.61, 5 = 0.50, 6 = 0.34, 7 = 0.33, 8 = 0.29, 9 = 0.65, 10 = 0.45, 11 = 0.60, 12 = 0.42, 14 = 0.59, 15 = 0.56, 16 = 0.15, 17 = 0.36, 18 = 0.45, 19 = 0.64, 20 = 0.79, 21 = 0.20, 22 = 0.30 and 23 = 0.33

#### **NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 273 lb uplift at joint 2, 520 lb uplift at joint 14, 749 lb uplift at joint 22 and 243 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 24866 1 100 Cessial Bay Blvd Boynton Beach, FL 33435



Job Truss Truss Type Qty Ply LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8 L266613 T08G **SPECIAL** 1 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 30 15:47:46 2008 Page 1 14-6-8 21-9-14 29-2-5 36-4-6 44-7-0 46-1-9 1-6-0 6-9-6 6-3-13 1-5-5 7-4-6 7-2-2 1-6-0 8-2-10 + Member must be laterally braced for horizontal wind loads. Scale = 1:79.4 Bracing and connections to be specified by the building designer. 5x12 MT20H≈ 5x14 = 4x6 = 4 5 45 7.00 12 3x6 / 46 3x6 < 3x6 > 3x6 > 3x6 = 22 21 5x8 II 5x14 = 1918 15 14 13 12 3x6 = 3x6 II 5x8 = 3x6 = 3x6 =3x6 II 3x6 = 14-6-8 6-9-6 13-1-3 21-9-14 1,4-2-0 29-2-5 36-4-6 44-7-0 6-9-6 6-3-13 1-0-13 7-3-6 7-4-6 7-2-2 8-2-10 0-4-8 Plate Offsets (X,Y): [2:0-3-4,0-1-8], [7:0-5-0,Edge], [10:0-0-7,Edge], [10:0-3-8,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES GRIP** 20.0 TCLL Plates Increase 1.25 TC 0.88 Vert(LL) 244/190 0.11 2-22 >999 360 MT20 TCDL 7.0 Lumber Increase 1.25 BC 0.56 Vert(TL) -0.12 17-18 >999 240 MT20H 187/143 BCLL 10.0 Rep Stress Incr NO WB 0.87 Horz(TL) 0.01 10 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 369 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 **BOT CHORD** 2 X 4 SYP No.2 \*Except\* oc purlins, except 5-19 2 X 4 SYP No.3 2-0-0 oc purlins (6-0-0 max.): 4-7. **WEBS** 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing, **OTHERS** 2 X 4 SYP No.3 Except: 10-0-0 oc bracing: 19-20,18-19 9-6-13 oc bracing: 17-18. T-Brace: 2 X 4 SYP No.3 - 5-20 **WEBS** T-Brace: 2 X 4 SYP No.3 - 6-20, 7-18, 7-17 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 10=211/10-6-0, 2=347/0-3-8, 20=2018/0-3-8, 14=1782/10-6-0, 15=45/10-6-0, 13=-73/10-6-0, 12=205/10-6-0

Max Horz 2=305(load case 5)

Max Uplift 10=-159(load case 7), 2=-329(load case 6), 20=-1699(load case 5), 14=-1300(load

case 4), 13=-74(load case 11), 12=-120(load case 7)

Max Grav 10=217(load case 11), 2=364(load case 10), 20=2018(load case 1), 14=1808(load case 11), 15=109(load case 2), 13=82(load case 7), 12=206(load case 11)

Julius Lee Trusa Design Engineer Florida PE No. 34888 1 109 Ceastal Bay Blvd Boynton Beach, Ft. 33435

January 30,2008

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T08G	SPECIAL	1	1	J1931033
77.75.75.75.75		0. 202			Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 30 15:47:46 2008 Page 2

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-268/2

1-2=0/40, 2-3=-268/246, 3-4=-284/381, 4-5=-262/358, 5-45=-250/345, 6-45=-250/345, 6-7=-543/588, 7-46=-462/580,

8-46=-801/656, 8-9=-387/583, 9-10=-299/287, 10-11=-21/63

**BOT CHORD** 

2-22=-222/291, 21-22=-222/291, 20-21=-257/448, 19-20=0/102, 5-20=-582/596, 18-19=0/95, 17-18=-436/545,

16-17=-350/432, 15-16=-350/432, 14-15=-350/432, 13-14=-350/432, 12-13=-350/432, 10-12=-350/432

**WEBS** 

3-22=-264/222, 3-21=-460/610, 4-21=-469/323, 4-20=-503/563, 18-20=-498/513, 6-20=-1218/1046, 6-18=0/207,

7-18=-46/63, 7-17=-499/430, 8-17=-717/1039, 8-14=-1781/1439

### JOINT STRESS INDEX

2 = 0.44, 3 = 0.42, 4 = 0.72, 5 = 0.85, 6 = 0.37, 7 = 0.94, 8 = 0.85, 9 = 0.00, 9 = 0.47, 9 = 0.47, 10 = 0.68, 10 = 0.21, 12 = 0.34, 13 = 0.34, 14 = 0.47, 15 = 0.34, 16 = 0.15, 17 = 0.61, 18 = 0.29, 19 = 0.38, 20 = 0.51, 21 = 0.35, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34, 40 = 0.34, 41 = 0.34, 42 = 0.34, 42 = 0.34, 43 = 0.34 and 44 = 0.34

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 7.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 10, 329 lb uplift at joint 2, 1699 lb uplift at joint 20, 1300 lb uplift at joint 14, 74 lb uplift at joint 13 and 120 lb uplift at joint 12.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-45=-54, 7-45=-114(F=-60), 7-46=-114(F=-60), 11-46=-87(F=-33), 2-20=-10, 10-19=-10

dullius Lee Tuse Design Engineer Florida FE No. 34868 1109 Caestal Bay Blvd Boynton Beach, FL 03436



Job Truss Truss Type Qty Ply LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8 J1931034 L266613 T09 SPECIAL 1 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:07 2008 Page 1 r1-6-Q 6-9-6 13-1-3 14-5-8 22-5-5 29-9-4 32-7-0 37-5-4 44-7-0 46-1-0 1-6-0 6-9-6 6-3-13 1-4-5 7-11-13 7-3-15 2-9-12 4-10-4 4-10-4 2-3-8 1-6-0 Scale = 1:83.5 5x6 = 5x8 = 8x10 < 3x6 = 4 5 6 6x8 > 7.00 12 3x6 / 4x6 > 15 5x14 = 16 3x6 = 19 18 17 3.50 12 13 12 2x4 || 5x8 = 2x4 II 3x6 = 3x8 = 6-9-6 13-1-3 14-3-13 22-5-5 32-7-0 42-3-8 44-7-0 6-9-6 6-3-13 1-2-10 7-11-13 2-3-8 6-11-3 3-2-8 9-8-8 0-1-11 [2:0-3-4,0-1-8], [4:13-1-3,8-4-5], [7:0-4-1,Edge] Plate Offsets (X,Y): LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.77 Vert(LL) -0.1414 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.43 Vert(TL) -0.27>999 14 240 BCLL 10.0 \* Rep Stress Incr YES WB 0.82 Horz(TL) 0.16 11 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 286 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 \*Except\* Structural wood sheathing directly applied or TOP CHORD 7-11 2 X 8 SYP 2400F 2.0E 6-0-0 oc purlins, except **BOT CHORD** 2 X 4 SYP No.2 \*Except\* 2-0-0 oc purlins (6-0-0 max.): 4-7. 8-13 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 5-3-6 oc **WEBS** 2 X 4 SYP No.3 bracing. Except: 10-13

**WEBS** 

1 Row at midpt

T-Brace:

2 X 4 SYP No.3 -

5-17

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 11=634/0-3-8, 2=-135/0-3-8, 17=2428/0-4-0

Max Horz 2=227(load case 5)

Max Uplift 11=-168(load case 7), 2=-471(load case 11), 17=-624(load case 4) Max Grav 11=642(load case 11), 2=98(load case 10), 17=2428(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-76/1103, 3-4=-368/1331, 4-5=-239/1115, 5-6=-19/220, 6-7=-778/333

, 7-8=-879/328, 8-9=-1605/626, 9-10=-1202/583, 10-11=-276/151

**BOT CHORD** 2-19=-920/291, 18-19=-920/291, 17-18=-1202/712, 16-17=-1304/774,

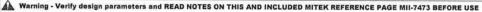
15-16=-239/343, 14-15=-285/1336, 13-14=0/66, 8-14=-207/503, 12-13=-1/21,

10-12=-443/1099

**WEBS** 3-19=-269/233, 3-18=-496/619, 4-18=-620/196, 5-18=-675/778, 5-17=-1880/1221,

Continued on page 26=-487/1216, 6-16=-890/440, 6-15=-311/1072, 7-15=0/168, 8-15=-567/340.

January 30,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occ. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T09	SPECIAL	1	1	J1931034
			i i		Job Reference (optional)

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### JOINT STRESS INDEX

2 = 0.42, 3 = 0.40, 4 = 0.56, 4 = 0.00, 5 = 0.55, 6 = 0.60, 7 = 0.85, 8 = 0.15, 9 = 0.30, 10 = 0.37, 12 = 0.77, 13 = 0.49, 14 = 0.38, 15 = 0.46, 16 = 0.73, 17 = 0.97, 18 = 0.83 and 19 = 0.33

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 11, 471 lb uplift at joint 2 and 624 lb uplift at joint 17.

LOAD CASE(S) Standard

Julius Les Truss Desian Engineer Flonda FE No. 34888 I 100 Ceastal Ray Blvd Boynton Beach, FL 22425



Job *	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS, LOT 8 J1931035
L266613	T10G	SPECIAL	1	1	Job Reference (optional)
Builders FirstSource,	Lake City, FI 32055	6.300 s Ap	r 19 2006 N	liTek Ind	ustries, Inc. Wed Jan 30 15:51:14 2008 Page 1
11-6-0 7-5-9	14-5-8	16-6-5, 22-5-5 , 29-4-8	32-7-0	35-8-11 ,	41-1-0 , 46-5-4 , 52-10-0 54-4-0
1-6-0 7-5-9	6-11-15	2-0-13 5-11-0 6-11-3	3-2-8	3-1-11	5-4-4 5-4-4 6-4-12 1-6-0 Scale = 1:93.7
		5x14 =			
	7.00 12 8x	3x8 =		5 = 8x10	
1	1.55   12 8x	10 = 5 52 6	7 8	9	
00-00	5x8 = 4		23 5x8 = G2		6x8 = 10 53 11 3x6 = 3x6 = 12
91 2 1 1 2		24 4x6 =	8x10 ≥		1314 14
4x6 =	28 27	26 4.6	100	1 20	19 18 1716 15 5x8 II

-	7-5-9	11-9-12	14-5-8 16-6	-5, 22-5-5	29-4-8	32-7-0	35-8-11	44-6-5	52-10-0
25	7-5-9	4-4-3	2-7-12 2-0-	13 5-11-0	6-11-3	3-2-8	3-1-11	8-9-9	8-3-11

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)		23-24	>718	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	7775 3 3 3 3 3	23-24	>999	240	111120	277/100
BCLL	10.0	* Rep Stress Incr	NO	WB	0.93	Horz(TL)	-0.29	19	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat					1110	11100	Weight: 434 lb	

LUMBER		BRACING
TOP CHORD	2 X 4 SYP No.2 *Except*	TOP CHORD

5-9 2 X 4 SYP No.1D

BOT CHORD 2 X 4 SYP No.2 \*Except\*

8-21 2 X 4 SYP No.3 **WEBS** 2 X 4 SYP No.3 \*Except\*

10-19 2 X 4 SYP No.2

**OTHERS** 2 X 4 SYP No.3 **BOT CHORD** 

WEBS

TOP CHORD

Structural wood sheathing directly applied or 4-3-0

oc purlins, except

2-0-0 oc purlins (4-3-2 max.): 5-9.

Rigid ceiling directly applied or 3-8-15 oc bracing.

Except:

T-Brace: T-Brace:

2 X 4 SYP No.3 - 8-22

2 X 4 SYP No.3 - 4-27, 4-25, 5-25, 6-24, 6-23,

8-23, 9-22, 9-20, 10-20,

10-19

2 X 6 SYP No.1D - 5-24

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 13=-94/8-6-8, 2=-8/0-3-8, 27=2405/0-4-0, 19=2937/8-6-8, 18=-74/8-6-8, 17=13/8-6-8, 15=73/8-6-8

Max Horz 2=645(load case 5)

Max Uplift 13=-214(load case 10), 2=-276(load case 11), 27=-2458(load case 5),

19=-2830(load case 4), 18=-172(load case 2), 17=-42(load case 5), 15=-25(load

case 7)
Max Grav 13=315(load case 5), 2=362(load case 5), 27=2405(load case 1), 19=2937(load rough 5) 17=87(load case 2), 15=81(load case 5)

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Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T10G	SPECIAL	1	1	J1931035
	1 1 0 5 5 5				Job Reference (optional)

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/40, 2-3=-1392/755, 3-4=-1630/1026, 4-5=-205/388, 5-52=-1047/1955, 6-52=-1046/1955, 6-7=-1909/3405, 7-8=-1909/3405, 8-9=-1430/2517, 9-10=-1055/1760, 10-53=-1942/1420, 11-53=-1734/1158, 11-12=-1719/1134,

12-13=-1589/904, 13-14=-37/63

2-28=-620/684, 27-28=-621/687, 26-27=-238/240, 25-26=-243/251, 24-25=-362/120, 23-24=-1930/1097, 22-23=-2502/1496 **BOT CHORD** 

, 21-22=-15/37, 8-22=-961/1575, 20-21=0/159, 19-20=-116/180, 18-19=-871/1559, 17-18=-871/1559, 16-17=-871/1559

15-16=-871/1559, 13-15=-871/1559

3-28=-243/209, 3-27=-455/587, 4-27=-2107/3455, 4-26=-65/61, 4-25=-2172/1217, 5-25=-1202/1846, 5-24=-2610/1484,

6-24=-1350/1938, 6-23=-1847/1099, 7-23=-681/881, 8-23=-1637/900, 20-22=-1744/1146, 9-22=-2510/1573.

9-20=-1523/2164, 10-20=-1440/924, 10-19=-2732/4081, 11-19=-441/544

### JOINT STRESS INDEX

2 = 0.38, 3 = 0.70, 4 = 0.85, 5 = 0.70, 6 = 0.98, 7 = 0.50, 8 = 0.59, 9 = 0.80, 10 = 0.95, 11 = 0.34, 12 = 0.00, 12 = 0.30, 12 = 0.27, 13 = 0.53, 13 = 0.12, 15 = 0.34, 16 = 0.53, 17 = 0.34, 18 = 0.34, 19 = 0.73, 20 = 0.56, 21 = 0.18, 22 = 0.51, 23 = 0.54, 24 = 0.80, 25 = 0.826 = 0.20, 27 = 0.51, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.34, 42 = 0.34, 42 = 0.34, 43 = 0.34, 44 = 0.34, 45 = 0.34, 46 = 0.34, 47 = 0.34, 47 = 0.34, 47 = 0.34, 47 = 0.34, 4848 = 0.34, 49 = 0.34, 49 = 0.34, 50 = 0.34 and 51 = 0.34

### NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

4) Provide adequate drainage to prevent water ponding.

5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) All plates are 2x4 MT20 unless otherwise indicated.

7) Gable studs spaced at 2-0-0 oc.

8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 13, 276 lb uplift at joint 2, 2458 lb uplift at joint 27, 2830 lb uplift at joint 19, 172 lb uplift at joint 18, 42 lb uplift at joint 17 and 25 lb uplift at joint 15.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

> Vert: 1-5=-54, 5-52=-54, 9-52=-114(F=-60), 9-53=-114(F=-60), 14-53=-87(F=-33), 2-26=-10, 23-26=-10, 22-23=-10, 13-21=-10

> > re PENO 34866 restel Bay Elve n Desch, FL 00436



Job Truss Truss Type Qty Ply LIPSCOMB & EAGLE - HILL OF HUNTS, LOT 8 J1931036 L266613 T11 SPECIAL 3 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:10 2008 Page 1 11-6-0 32-7-0 16-6-5 22-5-5 29-4-8 36-3-11 41-7-15 47-0-3 52-10-0 54-4-0 1-6-0 7-5-9 6-11-15 2-0-13 5-11-0 6-11-3 3-2-8 3-8-11 5-9-13 1-6-0 Scale = 1:97.0 5x14 = 3x6 = 2x4 || 4x6 = 5x14 = 7.00 12 3x8 / 5 6 3x6 > 5x8 / 10 5x6 > 5x8 = 19 5x14 3x6 = 4x6 = 4x6 = 23 22 21 16 15 14 3.50 12 3x6 = 2x4 || 5x6 = 3x6 II 3x8 = 5x6 = 7-5-9 11-9-12 14-5-8 16-6-5 22-5-5 29-4-8 52-10-0 7-5-9 4-4-3 2-7-12 2-0-13 5-11-0 6-11-3 3-2-8 3-8-11 8-1-9 8-4-12 Plate Offsets (X,Y): [2:0-0-0,0-0-4], [3:0-4-0,0-3-0], [11:0-3-0,0-3-0], [12:Edge,0-0-4], [14:0-3-0,0-3-0] LOADING (psf) SPACING 2-0-0 CSI DEFL L/d in (loc) I/defl **PLATES** GRIP TCLL 20.0 Plates Increase TC 1.25 0.43 Vert(LL) 0.24 12-14 >414 360 MT20 244/190 TCDL Lumber Increase 7.0 BC 1.25 0.29 Vert(TL) -0.17 18-19 >999 240 **BCLL** 10.0 Rep Stress Incr YES WB 0.67 Horz(TL) 0.09 14 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 362 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 \*Except\* 5-10-4 oc purlins. 8-16 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc **WEBS** 2 X 4 SYP No.3 bracing. Except: T-Brace: 2 X 4 SYP No.3 -8-17 **WEBS** T-Brace: 2 X 4 SYP No.3 -4-22, 5-20, 9-15, 10-14 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=188/0-3-8, 14=1618/0-3-8, 22=1713/0-3-8, 12=23/0-3-8

Max Horz 2=268(load case 5)

Max Uplift 2=-247(load case 6), 14=-536(load case 4), 22=-713(load case 5),

12=-241(load case 7)

Max Grav 2=208(load case 10), 14=1618(load case 1), 22=1713(load case 1),

12=28(load case 11)

January 30,2008

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T11	SPECIAL	3	1	J1931036
					Job Reference (optional)

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-117/376, 3-4=-270/648, 4-5=-187/287, 5-6=-623/458, 6-7=-1038/550, 7-8=-1038/550,

8-9=-768/482, 9-10=-511/411, 10-11=-165/718, 11-12=-193/535, 12-13=0/40

BOT CHORD 2-23=-295/201, 22-23=-297/203, 21-22=-105/276, 20-21=-107/287, 19-20=-47/236, 18-19=-211/657,

17-18=-188/805, 16-17=-18/15, 8-17=-579/254, 15-16=-6/7, 14-15=-74/146, 12-14=-428/199

3-23=-260/211, 3-22=-454/584, 4-22=-1400/627, 4-21=-49/45, 4-20=-228/762, 5-20=-736/259, 5-19=-316/809, 6-19=-705/344, 6-18=-117/528, 7-18=-304/221, 8-18=-194/514, 15-17=-50/506, 9-17=-283/812, 9-15=-753/244,

10-15=-179/599, 10-14=-1404/585, 11-14=-286/284

### JOINT STRESS INDEX

2 = 0.32, 3 = 0.65, 4 = 0.94, 5 = 0.51, 6 = 0.34, 7 = 0.33, 8 = 0.29, 9 = 0.65, 10 = 0.48, 11 = 0.61, 12 = 0.42, 14 = 0.57, 15 = 0.58, 16 = 0.15, 17 = 0.36, 18 = 0.45, 19 = 0.62, 20 = 0.69, 21 = 0.19, 22 = 0.27 and 23 = 0.33

### **NOTES**

WEBS

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 2, 536 lb uplift at joint 14, 713 lb uplift at joint 22 and 241 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee Florida FE No. 24869 1 100 Cassial Rey Blvd Doyalton Beach, FL 22425



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T12G	GABLE	1	1	J1931037
		Contract Con			Job Reference (optional)

6

Builders FirstSource, Lake City, FI 32055

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+ Member must be laterally braced for horizontal wind loads. Bracing and connections to be specified by the building designer.

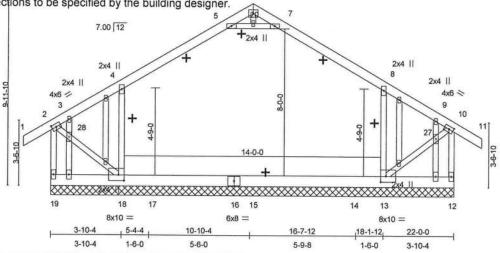


Plate Offsets (X,)	(): [2:0-2-14,0-2-0], [1	0:0-2-14,0-2	2-0], [13:	0-3-7,0-3	-12], [18:0-3-7	,0-3-12]					
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	2-0-0 1.25 1.25 YES	CSI TC BC WB (Mate	0.13 0.10 0.21	DEFL Vert(LL) Vert(TL) Horz(TL)		(loc) 15-17 14-15 12	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 196 lb	<b>GRIP</b> 244/190

LUMBER

TOP CHORD 2 X 6 SYP No.1D

BOT CHORD 2 X 8 SYP 2400F 2.0E WFBS

2 X 4 SYP No.3

**OTHERS** 2 X 4 SYP No.3 BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

(lb/size) 19=802/0-3-8, 12=802/0-3-8, 15=646/0-3-8, 17=497/0-3-8, 14=497/0-3-8 REACTIONS

Max Horz 19=-381(load case 4)

Max Uplift 19=-169(load case 7), 12=-168(load case 6), 17=-192(load case 5), 14=-188(load

Max Grav 19=802(load case 1), 12=802(load case 1), 15=646(load case 1), 17=619(load case

11), 14=619(load case 12)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/45, 2-3=-550/144, 3-4=-524/157, 4-5=-613/285, 5-6=-97/20, 6-7=-97/25, TOP CHORD

7-8=-613/285, 8-9=-524/156, 9-10=-550/142, 10-11=0/45, 2-19=-780/202, 10-12=-780/202

**BOT CHORD** 18-19=-334/349, 17-18=-107/443, 16-17=-107/443, 15-16=-107/443, 14-15=-107/443,

13-14=-107/443, 12-13=-30/66

5-20=-396/389, 7-20=-396/389, 4-18=-412/184, 8-13=-412/183, 6-20=-26/55,

2-28=-103/568, 18-28=-100/572, 13-27=-99/572, 10-27=-102/568, 3-28=-10/10,

9-27=-10/10

em lesion Engineer PE No. 34866 Pestal May Alva n Desch, FL 33406

### JOINT STRESS INDEX

WEBS

2 = 0.32, 3 = 0.34, 4 = 0.16, 5 = 0.15, 6 = 0.20, 7 = 0.15, 8 = 0.16, 9 = 0.34, 10 = 0.32, 12 = 0.15, 13 = 0.10, 16 = 0.09, 18 = 0.10, 19 = 0.100.15, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.16, 24 = 0.34, 25 = 0.34, 26 = 0.16, 27 = 0.71 and 28 = 0.71

January 30,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connet Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building doe. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T12G	GABLE	1	1	J1931037
	1 1 0" 51 0				Job Reference (optional)

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### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- 7) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-20, 7-20; Wall dead load (5.0 psf) on member(s).4-18, 8-13
- 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 17-18, 15-17, 14-15, 13-14
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 169 lb uplift at joint 19, 168 lb uplift at joint 12, 192 lb uplift at joint 17 and 188 lb uplift at joint 14.

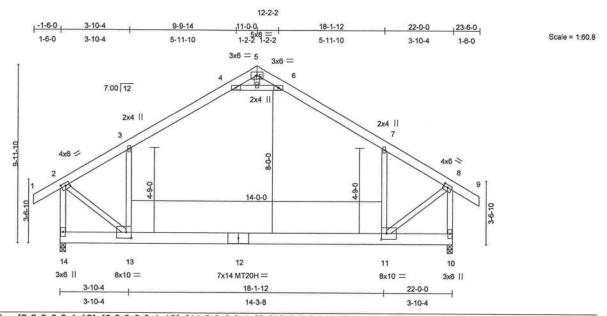
LOAD CASE(S) Standard

Julius Les Trues Design Engineer Flonda PE No. 24868 I 109 Coestal Bay Blvd Boynton Besch, FL 33436



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T13	ATTIC	2	1	J1931038
			_		Job Reference (optional)

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)		11-13	>520	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.80	11-13	>326	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.01	10	n/a	n/a		1017110
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)					,,,,	Weight: 176 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D BOT CHORD 2 X 8 SYP 2400F 2.0E

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or

2-2-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 14=1622/0-3-8, 10=1622/0-3-8

Max Horz 14=-298(load case 4)

Max Uplift 14=-25(load case 6), 10=-25(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-1425/132, 3-4=-1148/275, 4-5=-19/994, 5-6=-19/994, 6-7=-1148/275

, 7-8=-1425/132, 8-9=0/45, 2-14=-1934/178, 8-10=-1934/178

BOT CHORD 13-14=-266/296, 12-13=0/1063, 11-12=0/1063, 10-11=-1/65

4-15=-2223/353, 6-15=-2223/353, 3-13=-6/520, 7-11=-6/520, 5-15=-22/247,

2-13=0/1316, 8-11=0/1316

### JOINT STRESS INDEX

2 = 0.80, 3 = 0.47, 4 = 0.68, 5 = 0.81, 6 = 0.68, 7 = 0.47, 8 = 0.80, 10 = 0.31, 11 = 0.21, 12 = 0.65, 13 = 0.21, 14 = 0.31 and 15 = 0.33

### **NOTES**

**WEBS** 

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for CoMMCES for mactions specified.

dulhis Les Truss Design Engineer Florida PE No. 24869 1 100 Coastal Bay Blvd. Boynton Beach, FL 22425

January 30,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T13	ATTIC	2	1	J1931038
					Job Reference (optional)

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### **NOTES**

- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) Ceiling dead load (5.0 psf) on member(s). 3-4, 6-7, 4-15, 6-15; Wall dead load (5.0psf) on member(s).3-13, 7-11
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 14 and 25 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Plonda PE No. 24869 1 109 Geestal Bey Blvd Bovnton Beach Et 23425



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T14	ATTIC	Q	4	J1931039
		. ATTIO	0		Job Reference (optional)

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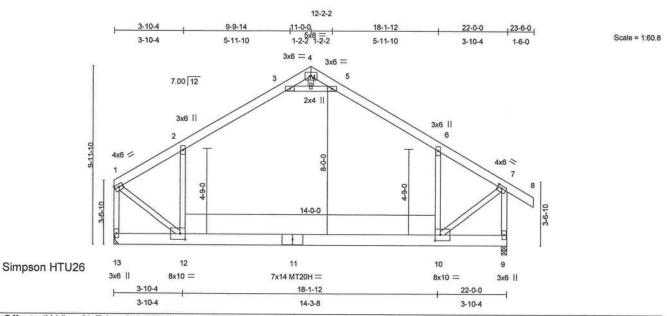


Plate Of	ffsets (X, Y	'): [1:Edge,0-1-12], [	7:0-3-0,0-	1-12], [1	0:0-3-8,0	)-4-0], [12:0-3	-8,0-4-0	)]				
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.50	10-12	>517	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.80	10-12	>325	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.01	9	n/a	n/a		1017110
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)			55.0	7,000,000	0.500	Weight: 172 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D

BOT CHORD 2 X 8 SYP 2400F 2.0E

WEBS.

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

2-2-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 13=1530/Mechanical, 9=1625/0-3-8

Max Horz 13=-267(load case 4) Max Uplift 9=-19(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1425/87, 2-3=-1152/261, 3-4=0/1004, 4-5=0/996, 5-6=-1153/257,

6-7=-1431/110, 7-8=0/45, 1-13=-1832/102, 7-9=-1942/151

BOT CHORD 12-13=-214/288, 11-12=0/1068, 10-11=0/1068, 9-10=-1/64

WEBS 3-14=-2240/303, 5-14=-2240/303, 2-12=-9/517, 6-10=-2/521, 4-14=-17/249,

1-12=-38/1310, 7-10=0/1323

### JOINT STRESS INDEX

1 = 0.82, 2 = 0.21, 3 = 0.68, 4 = 0.81, 5 = 0.68, 6 = 0.21, 7 = 0.82, 9 = 0.31, 10 = 0.21, 11 = 0.65, 12 = 0.21, 13 = 0.31 and 14 = 0.33

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for Correction is pagified.

Julius Les Truss Design Engineer Florida PE No. 24868 1100 Cossial Bay Blvd Boynton Besch, FL 23436

January 30,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T14	ATTIC	8	1	J1931039
	1				Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:14 2008 Page 2

### NOTES

- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) Ceiling dead load (5.0 psf) on member(s). 2-3, 5-6, 3-14, 5-14; Wall dead load (5.0 psf) on member(s).2-12, 6-10
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 10-12
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34869 1109 Coastel Bay Blvd Boynton Beach, Ft. 33436



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T15	ATTIC	1		J1931040
				2	Job Reference (optional)

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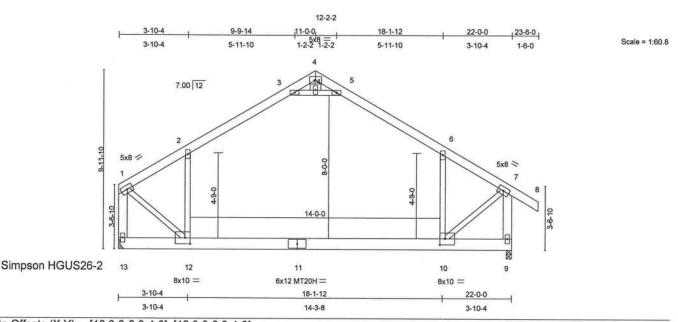


Plate Of	fisets (X, Y	<u>'): [10:0-3-8,0-4-0], [</u>	12:0-3-8,0	-4-0]								
LOADIN	IG (psf)	SPACING	2-8-8	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.33	10-12	>794	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.52	10-12	>500	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.28	Horz(TL)	0.01	9	n/a	n/a		1017110
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	Comment in the	100 de 100 d	1.76			Weight: 355 lb	)

LUMBER

TOP CHORD 2 X 6 SYP No.1D

BOT CHORD 2 X 8 SYP 2400F 2.0E

**WEBS** 

2 X 4 SYP No.3 \*Except\*

2-12 2 X 4 SYP No.1D, 6-10 2 X 4 SYP No.1D

4-14 2 X 8 SYP 2400F 2.0E

1-13 2 X 6 SYP No.1D, 7-9 2 X 6 SYP No.1D

BRACING TOP CHORD

**BOT CHORD** 

JOINTS

P CHORD 2-0-0 oc purlins (6-0-0 max.), except end

erticals

(Switched from sheeted: Spacing > 2-0-0).

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 1, 4, 7

REACTIONS (lb/size) 13=2064/Mechanical, 9=2200/0-3-8

Max Horz 13=-363(load case 3) Max Uplift 9=-28(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-1939/0, 2-3=-1542/125, 3-4=0/1308, 4-5=0/1298, 5-6=-1543/113, 6-7=-1942/0,

7-8=0/64, 1-13=-2514/0, 7-9=-2675/0

**BOT CHORD** 

12-13=-307/423, 11-12=0/1424, 10-11=0/1424, 9-10=0/129

WEBS

3-14=-2943/8, 5-14=-2943/8, 2-12=0/766, 6-10=0/767, 4-14=0/329, 1-12=0/1697,

7-10=0/1740

Julius Lee Truss Cesign Engineer Floride FE No. 34889 1400 Cessel Rey Blord Boynton Beach, FL 33436

### JOINT STRESS INDEX

1 = 0.61, 2 = 0.15, 3 = 0.45, 4 = 0.53, 5 = 0.45, 6 = 0.15, 7 = 0.61, 9 = 0.22, 10 = 0.14, 11 = 0.69, 12 = 0.14, 13 = 0.22 and 14 = 0.15

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T15	ATTIC	1		J1931040
				2	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:15 2008 Page 2

### NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) All plates are MT20 plates unless otherwise indicated.

7) All plates are 3x6 MT20 unless otherwise indicated.

- 8) Ceiling dead load (5.0 psf) on member(s). 2-3, 5-6, 3-14, 5-14; Wall dead load (5.0psf) on member(s).2-12, 6-10
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 10-12

10) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 9.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

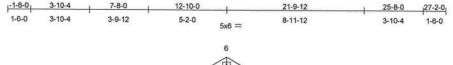
Julius Les Truss Design Engineer Florida PE No. 24868 1109 Crestel Rey Blvd Boynton Besch, Ft. 22425



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T16G	GABLE	1	1	J1931041
	100000000		,		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:16 2008 Page 1

Scale = 1:70.9



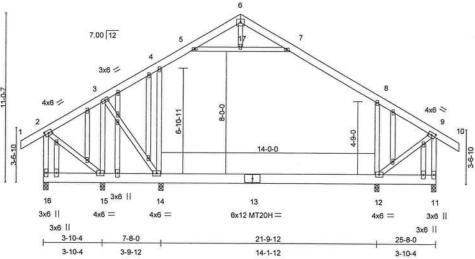


Plate Offsets (X,Y): [2:0-2-14,0-2-0], [9:0-2-14,0-2-0]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)		12-14	>702	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.38	12-14	>451	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.37	Horz(TL)	0.01	12	n/a	n/a		1017111
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)		212.1				Weight: 256 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D

BOT CHORD 2 X 8 SYP 2400F 2.0E

**WEBS** 

2 X 4 SYP No.3

**OTHERS** 

2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing, Except: 6-0-0 oc bracing: 11-12.

REACTIONS (lb/size) 16=1291/0-3-8, 15=-379/0-3-8, 12=1195/0-3-8, 11=980/0-3-8,

14=1359/0-3-8

Max Horz 16=-418(load case 4)

Max Uplift 16=-430(load case 7), 15=-554(load case 10), 12=-20(load case 4),

11=-494(load case 7), 14=-40(load case 5)

Max Grav 16=1291(load case 1), 15=31(load case 2), 12=1264(load case 12),

11=980(load case 1), 14=1579(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-7/72, 2-3=-787/345, 3-4=-1031/497, 4-5=-988/497, 5-6=-474/168,

6-7=-426/167, 7-8=-1134/470, 8-9=-989/317, 9-10=-7/72, 2-16=-1151/461,

9-11=-1407/451

15-16=-367/384, 14-15=-194/601, 13-14=-123/824, 12-13=-123/824, 11-12=-40/65

5-17=-495/394, 7-17=-495/394, 3-15=-742/316, 8-12=-666/318, 2-15=-254/747,

9-12=-201/1118, 4-14=-418/158, 3-14=-133/388, 6-17=0/59

### JOINT STRESS INDEX

**BOT CHORD** 

**WEBS** 

2 = 0.34, 3 = 0.41, 4 = 0.33, 5 = 0.33, 6 = 0.23, 7 = 0.33, 8 = 0.33, 9 = 0.50, 11 = 0.29, 12 = 0.45, 13 = 0.78, 14 = 0.28, 15 = 2 = 0.34, 3 = 0.41, 4 = 0.33, 5 = 0.33, 0 = 0.23, 7 = 0.33, 0 = 0.33, 0 = 0.33, 21 = 0.33, 22 = 0.15, 23 = 0.33, 24 = 0.33, 25 = 0.15, January 30,2008 Contiffue -0.33 page -20.33, 28 = 0.33, 29 = 0.15 and 30 = 0.33

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conners Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erec and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



dius Les nuss Design Engineer londe PE No., 24869 180 Caestel Rey Blvd oynton Besch. Ft. 33435

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T16G	GABLE	1	1	J1931041
				4	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:16 2008 Page 2

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-17, 7-17; Wall dead load (5.0 psf) on member(s).8-12, 4-14
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
- 10) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint 16, 554 lb uplift at joint 15, 20 lb uplift at joint 12, 494 lb uplift at joint 11 and 40 lb uplift at joint 14.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 14-16=-10, 12-14=-110, 11-12=-10, 1-2=-87(F=-33), 2-4=-87(F=-33), 4-5=-97(F=-33), 5-6=-87(F=-33), 6-7=-87(F=-33), 7-8=-97(F=-33), 8-9=-87(F=-33), 9-10=-87(F=-33), 5-7=-10

Drag: 8-12=-10, 4-14=-10

Julius Lee Truse Clesion Engineer Flonda PE No. 24889 1 100 Caastal Ray Blvd Boynton Besch, Ft. 20436



Job Truss Truss Type Qty Ply LIPSCOMB & EAGLE - HILL OF HUNTS, LOT 8 J1931042 L266613 T17 ATTIC 8 1 Job Reference (optional) Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:17 2008 Page 1

5x8 =

-1-6-0 3-10-4 7-6-4 12-10-0 21-9-12 25-8-0 27-2-0 1-6-0 3-10-4 3-8-0 5-3-12 8-11-12 3-10-4

Scale = 1:67.2

1-6-0

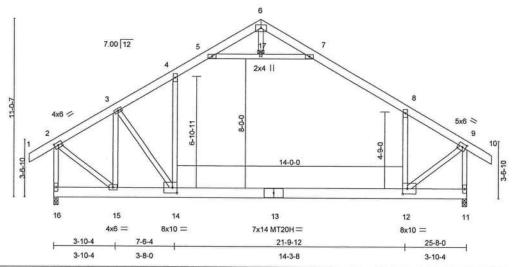


Plate Of	fsets (X,Y	(): [2:0-2-14,0-2-0], [9	9:0-2-9,0-2	2-8], [12	:0-3-8,0-	4-0], [14:0-3-8	3,0-4-0]					
LOADIN TCLL TCDL BCLL BCDL	G (psf) 20.0 7.0 10.0 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/TF	2-0-0 1.25 1.25 YES	CSI TC BC WB (Mat	0.85 0.53 0.57	DEFL Vert(LL) Vert(TL) Horz(TL)	- 22000	(loc) 12-14 12-14 11	I/defl >674 >431 n/a	L/d 360 240 n/a	PLATES MT20 MT20H Weight: 223 lb	<b>GRIP</b> 244/190 187/143

LUMBER

TOP CHORD 2 X 6 SYP No.1D

BOT CHORD 2 X 8 SYP 2400F 2.0E

**WEBS** 

2 X 4 SYP No.3

### BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-10-1 oc purlins, except end verticals.

**BOT CHORD** 

Rigid ceiling directly applied or 9-6-15 oc

bracing.

**WEBS** 1 Row at midpt

REACTIONS (lb/size) 16=1633/0-3-8, 11=1867/0-3-8

Max Horz 16=-328(load case 4)

Max Uplift 16=-62(load case 6), 11=-18(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/45, 2-3=-1172/241, 3-4=-1767/236, 4-5=-1286/318, 5-6=0/319, 6-7=-35/205.

7-8=-1509/294, 8-9=-1718/133, 9-10=0/45, 2-16=-1523/319, 9-11=-2276/175

**BOT CHORD** 

15-16=-282/303, 14-15=-148/987, 13-14=0/1351, 12-13=0/1351, 11-12=-5/64

**WEBS** 

5-17=-1541/249, 7-17=-1541/249, 3-15=-1277/3, 8-12=-44/434, 2-15=-35/1254,

9-12=0/1715, 4-14=0/769, 3-14=-166/757, 6-17=0/99

### JOINT STRESS INDEX

2 = 0.57, 3 = 0.59, 4 = 0.31, 5 = 0.47, 6 = 0.65, 7 = 0.47, 8 = 0.17, 9 = 0.76, 11 = 0.36, 12 = 0.28, 13 = 0.59, 14 = 0.22, 15 = 0.59, 14 = 0.21, 15 = 0.59, 0.51, 16 = 0.24 and 17 = 0.33

1) Unbalanced roof live loads have been considered for this design.

aes Jesign Endineer PE No. 24ees reastal Bay Blyd n Descn. FL 99495

Continued on page 2

January 30,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, Wi 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T17	ATTIC	8	1	J1931042
	80.00		^		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:17 2008 Page 2

### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-17, 7-17; Wall dead load (5.0 psf) on member(s).8-12, 4-14
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 16 and 18 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34868 1 100 Ceastal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS, LOT 8
L266613	T18	ATTIC	1		J1931043
				2	Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 30 13:04:31 2008 Page 1

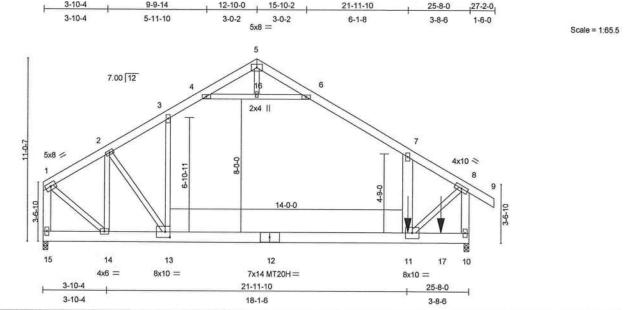


Plate Of	tsets (X,Y	):	[13:0-3-8,0-4-0]										
LOADIN	IG (psf)		SPACING	2-8-8	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.81	Vert(LL)	52503357777	11-13	>877	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.56	11-13	>536	240	MT20H	187/143
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.41	Horz(TL)	0.01	10	n/a	n/a	11112011	1077140
BCDL	5.0		Code FBC2004/TF	212002	(Mat	- 1000 d 110 - (4)		3.01				Weight: 462 II	0

LUMBER

TOP CHORD 2 X 6 SYP No.1D BOT CHORD 2 X 8 SYP 2400F 2.0E **WEBS** 2 X 4 SYP No.3 \*Except\*

7-11 2 X 8 SYP 2400F 2.0E, 1-15 2 X 6 SYP No.1D

8-10 2 X 6 SYP No.1D

BRACING

**JOINTS** 

TOP CHORD

2-0-0 oc purlins (6-0-0 max.), except end verticals (Switched from sheeted: Spacing > 2-0-0).

Rigid ceiling directly applied or 10-0-0 oc bracing.

**BOT CHORD** 

1 Brace at Jt(s): 1, 5, 8

REACTIONS (lb/size) 15=2332/0-3-8, 10=4294/0-3-8

Max Horz 15=-403(load case 3)

Max Uplift 15=-67(load case 5), 10=-499(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1701/65, 2-3=-2888/151, 3-4=-2136/253, 4-5=-49/690, 5-6=-110/562, 6-7=-2426/196,

7-8=-3102/144, 8-9=0/64, 1-15=-2045/75, 8-10=-4110/89

**BOT CHORD** 14-15=-312/390, 13-14=-201/1431, 12-13=-14/2276, 11-12=-14/2276, 11-17=-63/378, 10-17=-63/378

4-16=-2887/261, 6-16=-2887/261, 2-14=-2378/180, 7-11=-251/1411, 1-14=0/1721, **WEBS** 

8-11=0/2585, 3-13=0/1204, 2-13=-329/1649, 5-16=0/165

### JOINT STRESS INDEX

1 = 0.31, 2 = 0.66, 3 = 0.23, 4 = 0.46, 5 = 0.57, 6 = 0.46, 7 = 0.22, 8 = 0.91, 10 = 0.77, 11 = 0.21, 12 = 0.41, 13 = 0.22, 14 = 0.36, 15 = 0.41, 12 = 0.41, 13 = 00.42 and 16 = 0.34

lbis Las 199 Design Engineer onda PE No. 24899 09 Coestal Bay Blyd cynton Besch, FL 33435

January 30,2008

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building dece. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, Wi 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T18	ATTIC	1		J1931043
				2	Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 30 13:04:31 2008 Page 2

### NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.

All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply
connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) All plates are MT20 plates unless otherwise indicated.

7) All plates are 3x6 MT20 unless otherwise indicated.

8) Ceiling dead load (5.0 psf) on member(s). 3-4, 6-7, 4-16, 6-16; Wall dead load (5.0 psf) on member(s).7-11, 3-13

9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

10) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 15 and 499 lb uplift at joint 10.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 13-15=-14, 11-13=-149, 10-11=-14, 1-3=-73, 3-4=-87, 4-5=-73, 5-6=-73, 6-7=-87, 7-8=-73, 8-9=-73, 4-6=-14

Drag: 7-11=-14, 3-13=-14

Concentrated Loads (lb)

Vert: 11=-1693(F) 17=-300(F)

Julius Lee Truss Design Engineer Flonda PE No. 24869 1109 Ceastal Bay Blvd Bovnton Beach, Fl. 22425



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T19	MONO HIP	1	_	J1931044
	1		-2-0	2	Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 30 12:46:37 2008 Page 1

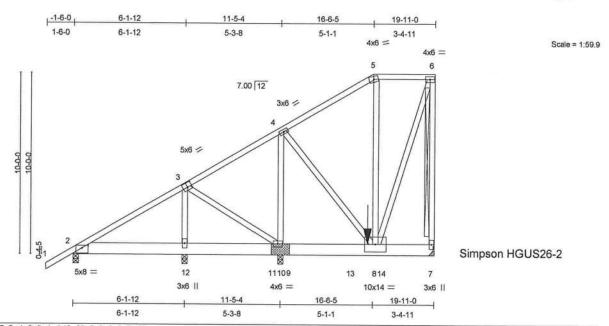


Plate Of	ffsets (X,Y)	[2:0-4-0,0-1-11], [3:	0-3-0,0-3-0	)]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.00	TC	0.67	Vert(LL)	-0.04	2-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.00	BC	0.50	Vert(TL)	-0.08	2-12	>941	240	20	2111100
BCLL	10.0	* Rep Stress Incr	NO	WB	0.38	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat		(/	0.00		100	11104	Weight: 359 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 8 SYP 2400F 2.0E

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing,

Except: 10-0-0 oc bracing: 7-8.

**WEBS** T-Brace: 2 X 4 SYP No.3 - 6-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum

end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=1693/Mechanical, 2=2088/0-3-8, 12=5054/0-3-8, 10=6128/0-3-10 (0-3-8 +

bearing block)

Max Horz 2=356(load case 5)

Max Uplift 7=-553(load case 3), 2=-637(load case 3), 12=-1618(load case 3), 10=-2001(load

case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD **BOT CHORD** 

1-2=0/46, 2-3=-177/165, 3-4=-178/313, 4-5=-750/228, 5-6=-587/189, 6-7=-1859/629

2-12=-111/61, 11-12=-113/62, 10-11=-113/62, 9-10=-217/99, 9-13=-217/99, 13-14=-217/99, 8-14=-217/99, 7-8=-3/7

**WEBS** 

3-12=-153/64, 3-10=-197/138, 4-10=-1741/509, 4-8=-460/1284, 5-8=-91/178,

6-8=-603/1879

### JOINT STRESS INDEX

2 = 0.20, 3 = 0.46, 4 = 0.64, 5 = 0.28, 6 = 0.79, 7 = 0.19, 8 = 0.27, 9 = 0.00, 9 = 0.00, 9 = 0.00, 10 = 0.25, 10 = 0.00, 10 = 0.00, 11 = 0.00, 11 = 0.00, 10 = 0.00.00, 11 = 0.00, 11 = 0.00 and 12 = 0.16 January 30,2008

Continued on page 2

### Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and/ or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T19	MONO HIP	1		J1931044
5 111 51 15		1021000000		2	Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 30 12:46:37 2008 Page 2

### NOTES

 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) 2 X 8 SYP 2400F 2.0E bearing block 12" long at jt. 10 attached to each face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SYP.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

5) Provide adequate drainage to prevent water ponding.

6) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 553 lb uplift at joint 7, 637 lb uplift at joint 2, 1618 lb uplift at joint 12 and 2001 lb uplift at joint 10.

### LOAD CASE(S) Standard

Regular: Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 1-5=-54, 5-6=-54, 2-13=-775(F=-765), 7-13=-10
 Concentrated Loads (lb)
 Vert: 14=-2064(F)

Julius Law Truss Design Engineer Florida PE No. 24869 1 109 Coastel Bay Blvd



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T20	MONO HIP	1	1	J1931045
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:20 2008 Page 1

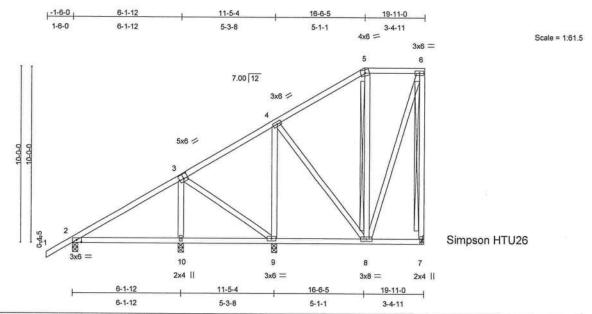


Plate Offsets (X,Y): [2:0-3-4,0-1-8], [3:0-3-0,0-3-0]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.03		>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)		2-10	>999	240	20	211110
BCLL	10.0	* Rep Stress Incr	YES	WB	0.30	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)			5.		111.64	Weight: 144 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
WERS	2 X 4 SYP No 3

BRACING TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc

bracing, Except: 6-0-0 oc bracing: 8-9.

**WEBS** 

T-Brace:

2 X 4 SYP No.3 - 6-7, 5-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=258/Mechanical, 2=289/0-3-8, 10=343/0-3-8, 9=455/0-3-8

Max Horz 2=353(load case 6)

Max Uplift 7=-94(load case 6), 2=-38(load case 6), 10=-88(load case 6), 9=-159(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/40, 2-3=-232/33, 3-4=-161/56, 4-5=-145/4, 5-6=-60/51, 6-7=-244/197

**BOT CHORD** 

2-10=-155/48, 9-10=-159/53, 8-9=-64/11, 7-8=-2/2

**WEBS** 

3-10=-267/173, 3-9=-53/115, 4-9=-374/239, 4-8=-13/87, 5-8=-210/177, 6-8=-166/191

### JOINT STRESS INDEX

2 = 0.33, 3 = 0.59, 4 = 0.41, 5 = 0.38, 6 = 0.49, 7 = 0.33, 8 = 0.65, 9 = 0.34 and 10 = 0.33

Continued on page 2

January 30,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T20	MONO HIP	1	1	J1931045
					Job Reference (optional)

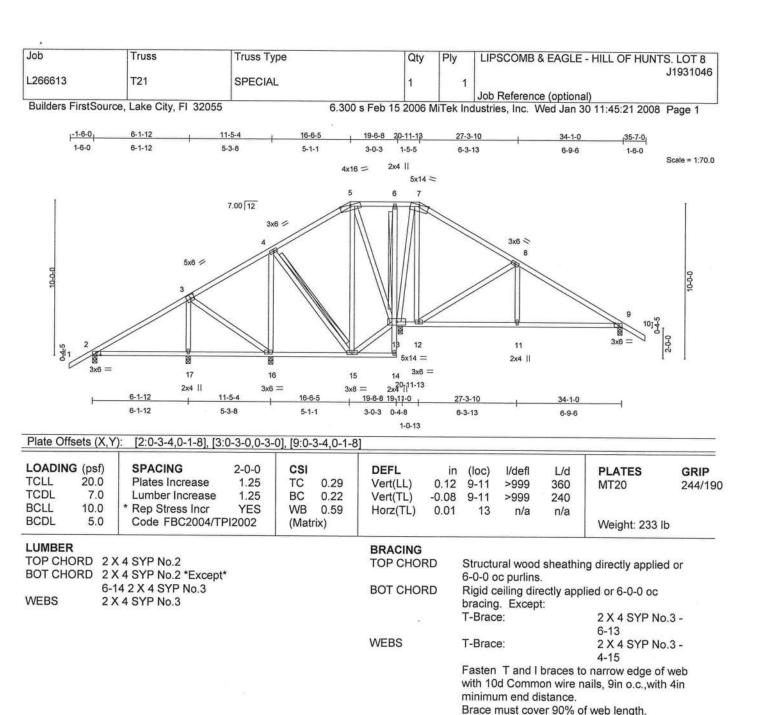
6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:20 2008 Page 2

### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 7, 38 lb uplift at joint 2, 88 lb uplift at joint 10 and 159 lb uplift at joint 9.

LOAD CASE(S) Standard





REACTIONS (lb/size) 2=279/0-3-8, 13=904/0-3-8, 9=477/0-3-8, 17=317/0-3-8, 16=363/0-3-8

Max Horz 2=266(load case 5)

Max Uplift 2=-84(load case 6), 13=-382(load case 6), 9=-347(load case 7),

17=-71(load case 5), 16=-147(load case 6)

Max Grav 2=292(load case 10), 13=904(load case 1), 9=480(load case 11),

17=351(load case 10), 16=363(load case 1)

January 30,2008

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB & EAGLE - HILL OF HUNTS. LOT 8
L266613	T21	SPECIAL	1	1	J1931046
11 majorista (100 maj	1 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1				Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 30 11:45:21 2008 Page 2

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-142/108, 3-4=-98/101, 4-5=-102/117, 5-6=-16/148, 6-7=-15/148, 7-8=-51/127, 8-9=-491/531,

9-10=0/40

BOT CHORD 2-17=-92/151, 16-17=-94/153, 15-16=-66/201, 14-15=-23/28, 13-14=0/20, 6-13=-80/56, 12-13=-36/212,

11-12=-309/348, 9-11=-309/348

3-17=-275/112, 3-16=-79/170, 4-16=-285/193, 4-15=-6/56, 5-15=-170/113, 13-15=-57/254, 5-13=-336/305,

7-13=-551/629, 7-12=-474/302, 8-12=-452/623, 8-11=-286/222

### JOINT STRESS INDEX

2 = 0.33, 3 = 0.59, 4 = 0.41, 5 = 0.78, 6 = 0.33, 7 = 0.78, 8 = 0.40, 9 = 0.43, 11 = 0.33, 12 = 0.34, 13 = 0.61, 14 = 0.33, 15 = 0.56, 16 = 0.34 and 17 = 0.33

### NOTES

**WEBS** 

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 2, 382 lb uplift at joint 13, 347 lb uplift at joint 9, 71 lb uplift at joint 17 and 147 lb uplift at joint 16.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonda PE No. 24868 1109 Coastal Bay Blvd Boynton Besch, FL 22436

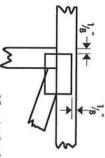


### Symbols

# PLATE LOCATION AND ORIENTATION



\*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



\*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



This symbol indicates the required direction of slots in connector plates.

### PLATE SIZE

4 × 4

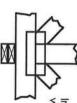
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING



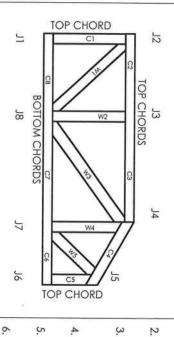
Indicates location of required continuous lateral bracing.

### BEARING



Indicates location of joints at which bearings (supports) occur.

# **Numbering System**



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

## CONNECTOR PLATE CODE APPROVALS

ВОСА

96-31, 96-67

ICBO

3907, 4922

9667, 9432A

WISC/DILHR 960022-W, 970036-N

SBCCI

NER

561



MiTek Engineering Reference Sheet: MII-7473

# n 🛕 General Safety Notes

### Failure to Follow Could Cause Property Damage or Personal Injury

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size and location dimensions shown indicate minimum plating requirements.
- Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- Do not overload roof or floor trusses with stacks of construction materials.
- 14. Do not cut or after truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling, erection and installation of trusses.
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### DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WIRN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 8402 AT REACH YED. MAY WEB TOTAL LENGTH IS 14°. MAX **GABLE** VERTICAL LENGTH VERTICAL LENGTH IN TABLE ABOVE. SPACING | SPECIES 24" O.C. 16 O.C. O.C. CONNECT DIAGONAL AT GABLE VERTICAL SPF SPF SPF DFL DFL DFLSP SP H SP H H ASCE NAOHS STANDARD #1 #2 #3 STUD STANDARD STANDARD STANDARD STANDARD GRADE STANDARD STUD STUD STUD WEB. おまた #3 古書 BRACE 7-02: \*\*\*WARDICK\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, MANDLING, SHIPPING, INSTALLING AND BRADING, RETER TO BES! 1-60 (BUILDING COPPIDENT SEFE IT INFORMATION, PUBLISED BY TPJ (TRUSS PLATE LISTIFILE, 583 DINDERTO BE, SUITE 200, MADISON, VI. 23759 AND VITA (VOOD TRUSS CIDACII) OF MATERIA, 6300 ENTERPRISE UN, MADISON, VI. 33719) FOR SAFETY PACTIFICS PRIDE TO PERFORMING THESE TAKTIONS, UNICES OF INFERVISE WIGIDATED, 109 CARDS SHALL HAVE PROPORT ATTACHED STRUCTURAL PARELS AND DOTTON CHORG SMALL HAVE A PROPERT ATTACHED RIGID CEILING. GABLE TRUSS 3, 10. BRACES 130 2X4 SP #2N, DF-L #2, SPF #1/#2. OR BETTER DIAGONAL BRACE; SINGLE OR DOUBLE CUT (AS SEDWN) AT GROUP A Ξ 5 0 MPH 1X4 "L" BRACE . 10 0, UPPER END. GROUP B 6, 10 7, 5, 6.0 WIND (1) 2X4 "L" CROUP A 7' 11" 8' 8" 8' 9" 11" SPEED GROUP B BRACE . REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH 15 SI MORY 18 (2) GROUP A 10 5 5 5 10' 5" MEAN 2X4 "L" EX4 MEN OR BETTER CONTINUOUS BEARING GROUP B BRACE .. 9'11' 9'11' 9'11' 11 10 5 5 8 9' 4" 9. 2. HEIGHT, 0 CONS. ENGINEERS P.A. GROUP A (1) 2Xe DELRAY BEACH, FL 33444-2161 12' 4" 10' 3" 10. No: 34869 STATE OF FLORIDA **(** ENCLOSED, GROUP B 10' 4, BRACE . 12' 9" GROUP A 12' 11' 12' 11' 12' 11' (2) ZXB 14' 0" 12' 0" Н r; WAX. MAX. BRACE CROUP B 11 13' 3" TOT. SPACING 1.00, LD ATTACH EACH 'L' BRACE WITH 104 MAILS. 4 FOR (1) 'L' BRACE: SPACE NAILS AT 2" O.C. 10 18" END ZONES AND 4" O.C. BETWEEN ZONES. 44 FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. 10 18" END ZONES AND 6" O.C. BETWEEN ZONES. CABLE END SUFFORTS LOAD FROM 4. 0" DUTLIDAKERS WITH 2" O" DVERHANG, DR 12" PLYWOOD OVERHANG. PROVIDE UPLAT CONNECTIONS FOR 136 FLF OVER CONTINUOUS BEARING (6 PSP TC DEAD LOAD). LIVE LOAD DEPLECTION CRITERIA IS L/240. MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB DOUGLAS FIR-LARCH SPRUCE-PINE-FIR #1 / #2 STANDARD #3 STUD BRACING GROUP SPECIES VERTICAL LENGTH LESS THAN 4.0° GREATER THAN 4.0°, BUT LESS THAN 11' 6° GREATER THAN 11' 6° EXPOSURE CABLE TRUSS 60 SOUTHERN 24.0 REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES. CABLE VERTICAL PLATE SIZES STANDARD PSF PIN REF DRWG MITEX STD GABLE 15 E HT DATE HEM-PIR GROUP GROUP DETAIL NOTES: 0 DOUGLAS FIR-LARCH 11/26/03 ASCE7-02-CAB13015 Ä STANDARD A: 13 NO SPLICE AND GRADES: 2.504 200 STANDARD GUIS

### ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE a

		M 2		W.S.	0	167		AI			6				.(	eco in		Ī	A 2	4		L	0-13-7	) . (			H	
W. C.	レモ	4	<i>U</i> .	)	111	I,	CIT	200		LH	j 1	ט. ר	)	111	I I I	OFF	CDF	200	L'F.L	1	<i>ن</i> .	)	TTT.	I,	OFF	CDF	NG SPECIES	GABLE VERTICAL
STANDARD	STUD	*3	#2	<i>‡</i> ]	STANDARD	STUD	*53	\$1 / #2	STANDARD	STUD	<b>†</b> 3	#22	<i>‡</i> 3	STANDARD	STUD	#3	£1 / #2	STANDARD	STUD	<b>‡</b> 3	#2	<i>‡</i> 1	STANDARD	STUD	₽\$	£1 / #2	CRADE	AL BRACE
4. 0.	4.	4. 2.	4. 4.	4. 5.	3' 11'		3 11"	4.	3. B.	3. 8.	3. 8.	3' 11'	4.	3. 7.	3' 7"	3. 7.	3. 8,	3.0	3' 3"	3. 3,		3' 8"	2. 11.	3' 1"	3' 1"	3.	BRACES	NO NO
5' 6"	6 4	6' 6"	6' 11"	6 11	55, 44,	6. 3	в. Ц	6. 11.	4. 9.	5. 6.	5. 3.	8' 4"	D. 4.		5' 6"	5. 5.	6. 4.				5' 6"		3' 9"		4. 5	5' 6'	GROUP A	(I) 1X4 T
5' 6"	6' 4"	6' 5"	7' 6"	7' 6"	5' 4'	6.3.	6. 3.	7' 2"	4' 9"	5, 8,	5' 7"		B' 10"		6, 2,	5, 5,	6' 6"	3' 10"		4. 6.	5' 11.	5' 11"	3' 9"	4, 5,	4. 5.	5° 8"	GROUP H	BRACE .
7' 3"	8' 3"	8' 3"	8 3	8 3	7' 1"		a' 3'	8' 3"	6' 3"	7' 3"	7' 4"	7' 8"	7' 6"		7' 2"	7, 5,	7' 6'	5, 1.	5' 11"	6, 0,	6' 6"	8' 8"	6' 0"	5' 10"	e, To.	6, 6,	GROUP A	(I) 2X4 T
7' 3"	8 5"	8, 8,	8' 11"	B' 11.	7' 1"	8° 3°	8' 3"	8' 6'	6' 3"	7' 3"	7 4"		8' 1"	6. 2.	7' 2"	7' 2"	7' 8"	5. 1.		6. 0.	7' 0"	7' 0"	5. 0.	5' 10"	5' 10"	6. 9.	GROUP B	L' BRACE .
8.8	9. 10.	9′ 10″	9, 10,	9° 10°	9' 6"		9, 10,	9. 10.	a' 5"	8' 11"	8. 11	8' 11"	8' 11"	8. 3.	8' 11"	8, 11,	8. 11	8' 11"	7' 10"	7, 10,,		7' 10"	6. 9.	7' 10'	7' 10"	7' 10'	GROUP A	(2) 2X4 T
9, 9,	10' 4"	10′ 4″	10' 7"	10, 5,	9, 6,	9' 10"	9' 10"	10. 1.	8, 2,	9' 5"	8. 9.	9, 7,	8, 2,	8.3.	8' 11.	8' 11"	9. 2.	6, 11,	8' 0"	8, 1,	8, 2,	8' 5"	6. 9.	7' 10"	7' 10"	8. 0.	GROUP B	BRACE **
11' 4"		12, 11,	.11 .21	12, 11,	11' 1"	12, 10,	12' 11"	12, 11,	8, 8,	11' 4"	11. 2.	11, 9,	11, 8,	9. 7.	11. 1	11' 2"	11' 9'	8' 0"	9' 3"	9. 4.	10′ 3″	10' 3"	7' 10"	9' 1"	9' 1"	10' 3"	GROUP A	(1) 2X6 "L"
11' 4"	13. 1.	13′ 3′	13, 11,	13, 11,	11' 1"	12' 10"	100	13' 4"	9' 9"		11' 6'	12' 8"	12' 8"	8. 7.	•	11, 5,	12' 1"	8. 0.	9' 3"	9.	11, 1.	11. 1	7. 10.	9′ 1″	9' 1"	10' 7"	GROUP	BRACE .
14' 0"	14. O.	14. 0,	14. 0"	14. 0"	14. 0.	14. 0"	14' 0"	14. 0.	13' 3"	14. 0.	14. 0.	14. 0	14. 0.	12, 11,	14. 0	14' 0"		10, 10,	12' 3"	12. 3.	12' 3"	12' 3"	10. 2.	12' 3"	12' 3"	12. 3.	B GROUP A GROUP	.T. 8X2 (2)
14' 0"	14. 0.	14. 0,	14. 0	14. 0.	14' 0"	14. 0*	14. 0,		13' 3"	14. 0.	14. 0.	14' D"	14' 0"	12. 11.	14' 0"	14' 0"	14. 0.	10. 10.	12' 6"	12, 8,	13′ 2″	13' 2"			12' 3"	12' 7'	CROUP B	BRACE .

DOUGLAS FIR-LARCH

STANDARD

STANDARD

GROUP B:

SPRUCE-PINE-FIR

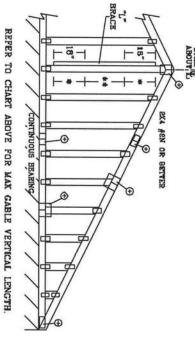
#1 / #2 STANDARD

#3 STUD

STANDARD

BRACING GROUP SPECIES AND GRADES:

GROUP A:



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WIRN DIAGONAL
BRACE IS USED. CONNECT
BRACE IS USED. MAY WEB
AT EACH END. MAY WEB
TOTAL LENGTH IS 14\*.

GABLE TRUSS

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZX1 SP OR
DF-L #2 OR
BETTSR DIAGONAL
BRACE SINGLE
OR DOUBLE
CUT (AS SHOWN)
AT UPPER END

CONNECT DIAGONAL AT AMBIENT OF VERTICAL WEB.

CABLE TRUSS DETAIL NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

LIVE LOAD DEPLECTION CRITERIA IS L/24d.

PROVIDE UPLIT CONNECTIONS FUR 180 PLF OVER CONTINUOUS BEARING (& ESP YC DEAD LOAD).

CABLE END SUPPORTS LOAD FROM 4: 0"

OUTLIDONGES WITH 2" 0" OVERHANG, OR 12"

PLYNOOD OVERHANG.

ATTACH EACH 'L' BRACE WITH 104 NAILS.

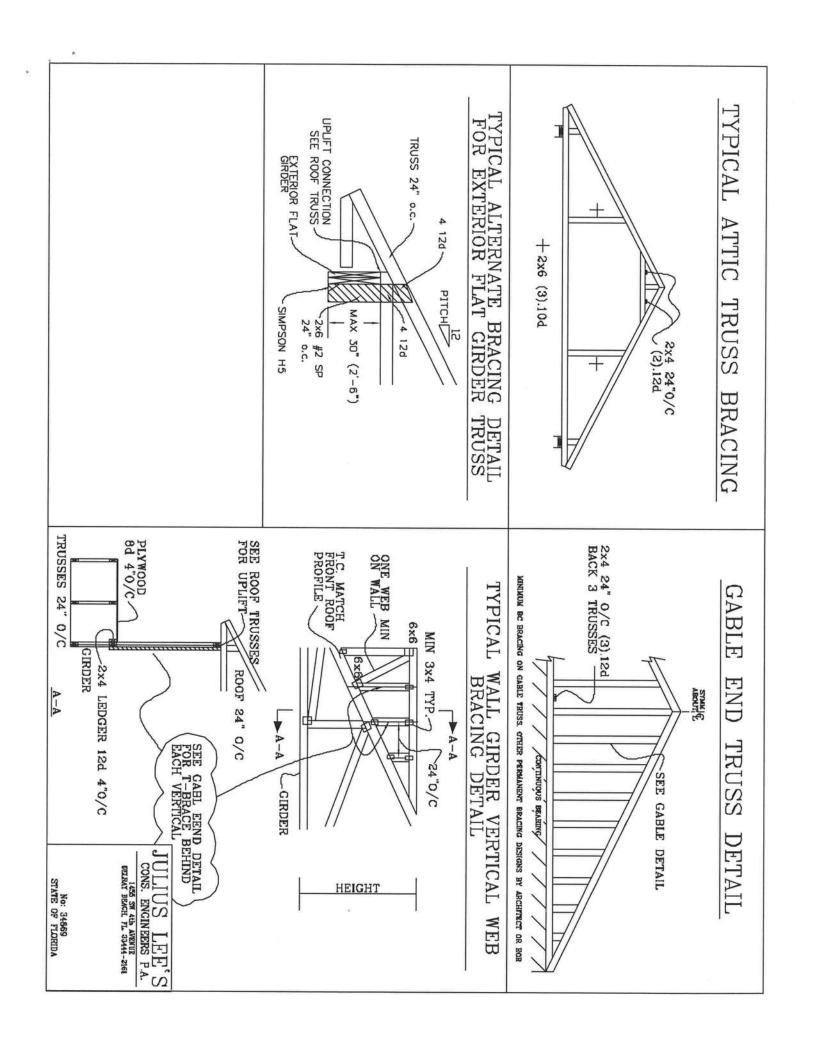
# FOR [1] 'L' BRACE: SPACE NAILS AT 2" O.C.
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

## FOR [2] 'L' BRACES: SPACE NAILS AT 3" O.C.
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L' BRACING MUST BE A MINIMUM OP 80% OF WEB
MEMBER LENGTH.

NO.	DESIGN	REFER TO COMMON TRUSS
	2.5X4	GREATER THAN 11' 6"
	200	CREATER THAN 4 D', BUT
223	1X4 DR	PSS THAN 4: 0"
Q	NO SPL	VEHINCAL LENGTH

	BACONG. RETER TO BEST 1-03 GRALING COMPENT SAFETY (REDWATEDAG, SUPPING, DISTALING AND BACONG. RETER TO BEST 1-03 GRALING COMPENT SAFETY (REDWATEDAG, PURLISHED BY FTY CRASS PARTY IN PROSECUTION, SUPPING COLORG. OF ARTERIOR OF AREAS				
No: 34869 STATE OF FLORIDA			DELRAY BEACH FL 33444-2161	CONS. ENGINEERS P.A.	S, HHI SHLIME
MAX. SPACING 24.0"	MAX. TOT. LD. 60 PSF		3		
	,	-ENG	DWG MITEK STD GABLE 30' E HT	DATE 11/26/03	REF ASCE7-02-GAB13030



TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

# PIGGYBACK DETAIL

TYPE

SPANS 34'

UP

5

WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENCINEER'S SEALED DESIGN FOR REQUIRED FURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE POLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDC, LCCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF 110 MPH WIND, 30' MEAN HCT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C. WIND TC DL=5 PSF, WIND BC DL=5 PSF

Ħ

5X4

OR 3X6 TRULOX AT 4'

DC,

D D B >

5X6

.5X3

1.5X4 5X5

1.5X4 5X6 30°

**5X8** 

3X5

2.5X4

2.5X4

88

52,

ATTACH TRULOX PLATES WITH (8) 0.120 X 1.375 EQUAL PER FACE PER PLY. (4) NAILS IN EACH BE CONNECTED. REFER TO DRAWING 160 TL FOR INFORMATION.

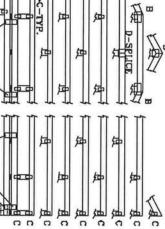
MEMBER TRULOX

70R

FRONT FACE (E,\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. MAX SIZE OF ZXIZ



EITHER PLATE LOCATION IS ACCEPTABLE



WEB LENGTH	REQUIRED BRACING
o' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 164 NAILS AT 4" OC.

\* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF PABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120° X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

NX T

\*ATTACH

PIGGYBACK WITH 3X6 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE

12

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B J.¥si

8

**A** =

			5.1
No: 34889	\$ }	DELRAY BEACH, FL. 3344-2161	JULIUS LEE'S
SPACING	47 PSF 1.15 DUR	50 PSF AT 1.25 DUR. FAC	MAX 55
73	PSF DUR.	PSF DUR.	MAX LOADING
24.0"	AT FAC.	AT FAC.	MAX LOADING

DRWG MITEK STD

PIGGY

DATE

THIS DRAWING REPLACES

8 1/4" DRAWINGS 634,016

634,017 & 847,045

PIGGYBACK

### VALLEYTRUSS DETAIL

TOP CHORD CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. 2X4 SP #3 OR BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- \* ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d ASCE 7-02 130 MPH WIND. 15' MEAN HEICHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN. APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING. EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

ENGINEERS' SEALED DESIGN. PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON OR

\* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS HENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

12 NAX.

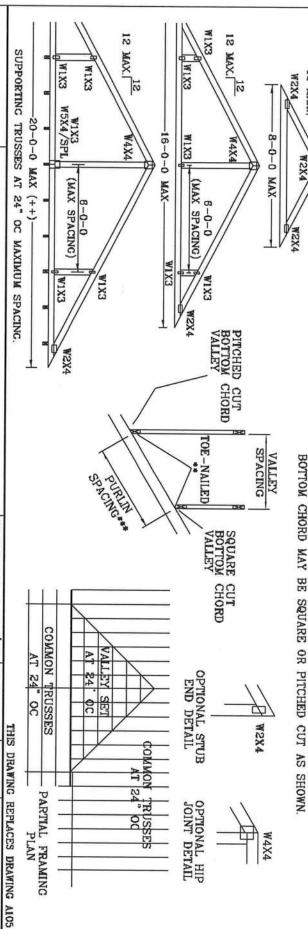
12

4-0-0

MAX

++ LARGER SPANS NAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



MAYARIMGAM TRUSSES REQUIRE EXTREME CAME (N FABRICATME, HANDLING, SHIPPING, ONSTALLIN BACING REFER TO BEST (1-50 GIULIDIG COPPUIDIT SAFETY INFORMATION), PIBLICISED BY TPI PLATE DASTITUE, 300 CONTROL DE, SUITE 201, MAISCIN, V., 53799 AND VICEA CHOOL TRUSE OF AMERICA, GAID ONTERPRISE UN NAISCON, VICESTYPS FOR SAFETY FRACTICES PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIDIO CELONG.

			ь	COUNCIL	ING AND	
STATE OF FLORIDA	No: 34869			DRIBAY BEACH, FL 35444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
SPACING	DUR.FAC. 1.25	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	25	32	0	ຜ	~2	20
24"	1.25	40	0	Ç	15	20
		PSF	PSF	PSF D	PSF	PSF
			-ENG JL	DRWG	PSF DATE	REF
			JL	VALTRUSS1103	11/26/03	VALLEY DETAIL

# TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE. END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

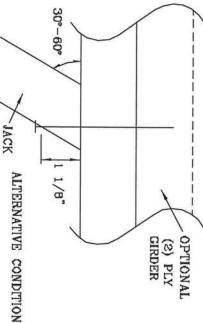
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

ALL VALUE	5	4,	အ	N	TOE-NAILS	NUMBER OF
ES MAY BE	493#	394#	296#	187#	1 PLY	SOUTHE
ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.	639#	511#	383#	256#	2 PLIES	SOUTHERN PINE
D BY APP	452#	361#	271#	181#	1 PLY	DOUGLAS
ROPRIATE	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
DURATION	390#	312#	234#	156#	1 PLY	
OF LOAD F	507#	406#	304#	203#	2 PLIES	HEM-FIR
ACTOR.	384#	307#	230#	154#	1 PLY	SPRUCE
	496#	397#	298#	189#	2 PLIES	SPRUCE PINE FIR

JACK 30°



THIS DRAWING REPLACES DRAWING 784040

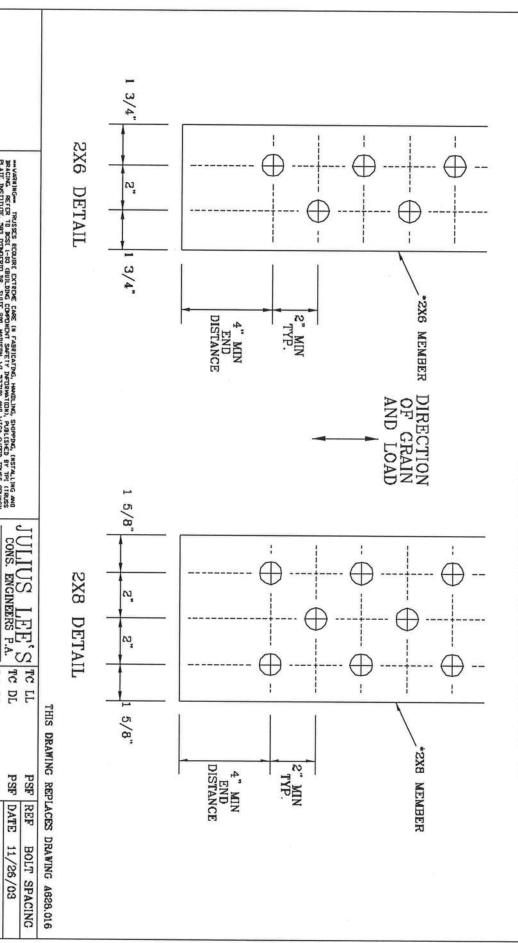
_		_				
			TS AND BOTTOM CHO	PRIDE TO	***VARNING*** TRUSSES REDURE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BEST 1-03 CHUILIDNG COMPONENT SAFETY (MFORWATION), PUBLISHED BY TRY CIRCLES	
STATE OF FLORIDA	No: 34869			DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	1.00	PSF	PSF	PSF	PSF	PSF
			-ENG JL	DRWG	DATE	REF
			IL	CNTONAIL1103	09/12/07	TOE-NAIL

## DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" OF 1/16" LARGER THAN BOLT DIAMETER. \* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN TO A MAXIMUM

> TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PATTERNS SHOWN BELOW. APPLIED

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



DELRAY BEACH, FL 33444-2161

BC DL

DRWG DATE

CNB0LTSP1103 11/26/03

-ENG

BC LL

TOT.

EĐ.

PSF PSF PSF PSF

No: 34869 STATE OF FLORIDA

SPACING

DUR. FAC

# TRULOX CONNECTION DETAIL

II GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (+).

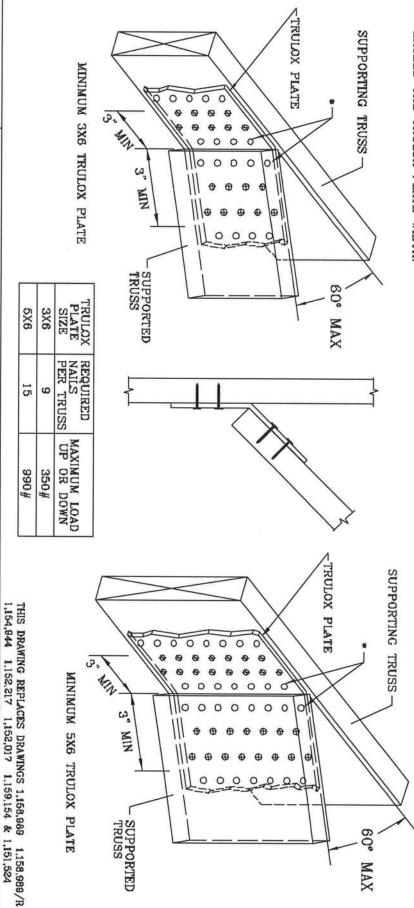
\* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FI

THIS DETAIL MAY BE USED WITH SO. PINE. DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



Gem TRUSSES REQUIRE EXTROME CARE IN FABRICATING, HANDLING, SHIPPING, REFER TO BOST 1-03 (BUILDING DOMENICH SAFETY DAFDRANTON, AUBLICH CONTROL OR, SUITE 20, MARISON, VI. 337199 AND VICA AVOITA, SAG DYNORRID OR, SUITE 20, MARISON, VI. 337199 FOR SAFETY PRACTICES PRIDRATING IN TO STATE TO MARISON AND SAFETY PRACTICES PRIDRATING IN TO STATE TO MARISON AND SAFETY PRACTICES PRIDRATING IN TO STATE TO SAFETY PRACTICES PRIDRATING IN TO STATE TO SAFETY PRACTICES PRIDRATING IN TO SAFETY PRACTICES PRIDRATING IN TO STATE TO SAFETY PRACTICES PRIDRATING IN TO

ULIUS LEE'S

REF

DATE

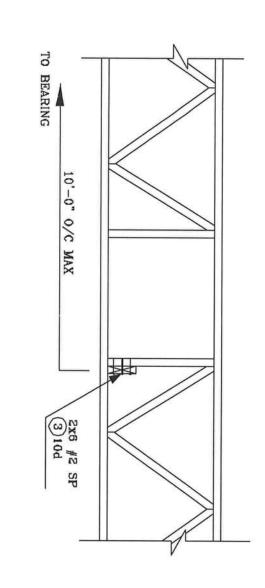
TRULOX 11/26/03

DRWG CNTRULOX1103

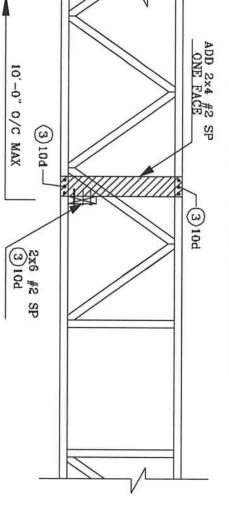
DELRAY BEACH, FL 33444-2181

No: 34869 STATE OF FLORIDA

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



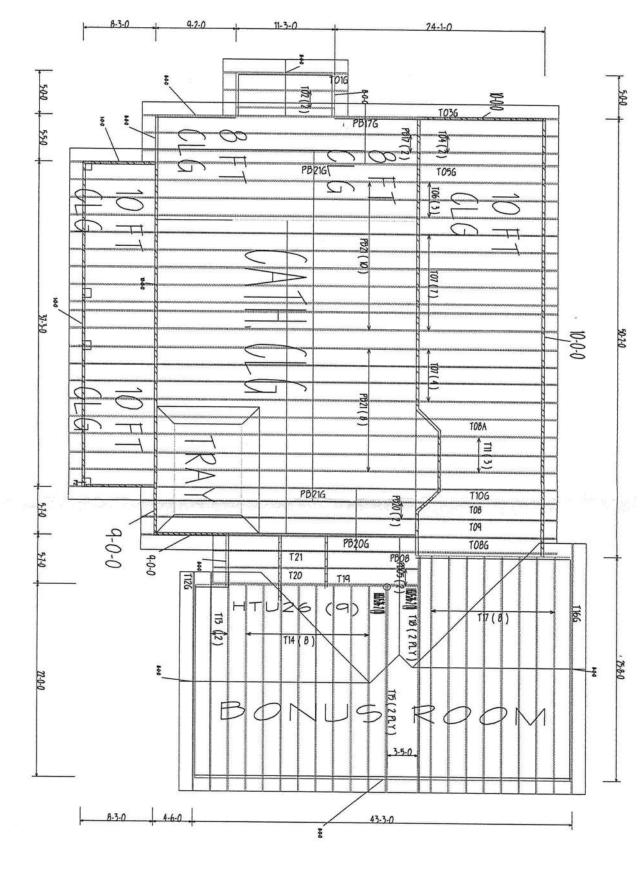
### ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S cons. ENGINEERS P.A. DELRAY BEACH, FL 33444-2161

No: 34869 STATE OF FLORIDA

TO BEARING



LOAD BEARING	O FI INITKNAL

F		2	n	
laposal Dévery lus :	N EXTEX CHARGES FOR TOLL	E RECEIVED DEFORE ANY TRUSSES WILL DE DUIL!	MUSS LAYOUTS, RETIEW AND APPROVAL OF THIS	TRUSSES AND YORS ALL PREYIOUS ARCHITECTUR

FirstSource
Bunnell
PHONE: 901-197-3944 FAX: 901-197-3944

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 306-755-6844 FAX: 386-753-7973

Sanford Hane: 407-322-0059 FAX: 407-322-5553

LIPSCOMB EAGLE
LIGA ARREYS. HILLS OF HUNTSVILLE
LOT 8
LOT 11/30/108 F.G. L266613

	45 TO 1	D DEFO	SIND	NO YO	NTOUT
NEXTEN CHARGES TO YOU	45 TO MEURE AGAINST CHANGES THAT WILL RESILT	D DEFORE ANY TRUSSES WILL BE DULT. YESTEY A	BUTS. BETTER AND APPROVAL OF THIS LAYOUT MAY	AND YOR'S ALL PREYYOUS ARCHITECTURAL OR OTHER	ATOUT IS THE SOLE SCIENCE FOR FADRICATION OF
CHARS	19AIVS	34400	NO APP	PEEYA	25.37
ES 10 Y	SHAME	P TIE	SOUTH (	5 AZCH	ECE FOR
2	1MIL 9	E DUL!	光光	LECTUR.	ENDE
	ALL R	TO NE	LAYAU	AL OK	CATION
	Ĕ	MY	Ē	景	8

THIS LAYOUT IS THE SALE SOURCE FOR FARKLATION OF	SHOP DRAWING APPROVAL	B.) DEAMMEADERAINTEL (HDR) TO DE FLORNESHED BY DUILDER	2.) ALL ROOF TRUSS HANGERS TO BE SHIPSON HUSEO THE RIMSE NOTED. ALL FLOOR TRUSS HANGERS TO BE SHIPSON THAN 2Z LINLESS OTHERWISE NOTED.	6) SY42 TRUSSES MUST DE INSTALLED WITH THE TOP DEING UP.	5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARTHS, UNLESS OTHERWISK NOTED.	
--	-----------------------	--	--	---	---	--

TRUSS HANGERS TO BE SIMPSON LESS OTHERWISE NOTED. ALL DASS HANGERS TO BE SIMPSON	5525 MUST DE INSTALLED TOP DEING UP:	CONSIDERED TO BE LOAD

3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRANCO BY BUILDER. Z) ALI TRUSSES (INCLUDING TRUSSES INDER VALLEY FRANKS) MUST DE COMPLETELY DECKED OR REFER TO DETAL VIOS FOR ALTERNATE DEXAMS REQUIREMENTS. H.) ALL TRUSSES ARE DESIGNED FOR 7 o.g. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

NOTES:
1) REFER TO HIS SI (RECOMMEDIATIONS FOR HAND, WE SHIFLLATION HIS TEMPORARY BULGHES) DEVANDES FOR PERMANENT BULCHNS RECORDED.

9'0" 10'0"

BEARING HEIGHT SCHEDULE

0.0

### New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

	nformation (Trea	ing Company Inforr	nation)	10 10 10 10 10 10 10 10 10 10 10 10 10 1				7.7
0	Annan Bant	Control, Inc.						
Company Name: _ Company Address				City	Lates City	State	Zip	20000
				Oity	Company Phor			E2-688-8781
Section 2: Builder In	formation							
Company Name:_	4.1980	mb + Fe	27/2		Company Phor	ne No		
Section 3: Property I	nformation				- 75			
Location of Structu	ure(s) Treated (St	reet Address or Leg	al Description, (	City, State and Zip	879 II	1 w m.	10 7	in
		e box may be chec tside			ent Crawl	Other	IQ,	. h
Section 4: Treatment	Information						30.5	
	7	7 11 - 111						2 "
Date(s) of Treatme	ent(s)	10.00						
Brand Name of Pr	oduct(s) Used	163-18						
Approximate Final								
Approximate Size	of Treatment Area	: Sq. ft32		inear ft. 3	56 Linea	r ft. of Masonry \	/oids 3	56
Approximate Total	Gallons of Solutio	Applied5	83					
Was treatment cor	npleted on exterio	or? 🗌 Yes 🗵	No					
Service Agreemen			No					
Note: Some stat	e laws require se	rvice agreements to	be issued. This	form does not pr	reempt state law.			
						11 15757	5 8	
Attachments (List)							1 3	
x								
Attachments (List) Comments								
x								2 11
x	5/200	Branna	n	Certification I	No. (if required by St	ate law)	04576	

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Authorized Signature

### PRODUCT APPROVAL SPECIFICATION SHEET

### Location:

### Project Name:

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at <a href="https://www.floridabuilding.org">www.floridabuilding.org</a>

Category/Subcategory	Manufacturer	Product Description	Approval Number(s
A. EXTERIOR DOORS			7 1 1 1 1 1 EC
1. Swinging	MASONITE	FIBERGLASS SIDE-HINGED WOR	5507
2. Sliding		, , ,	0.5
3. Sectional	RYCRAFT GARA	LEDOORS 18/x7 GATHLE DOOR	2792
4. Roll up			
5. Automatic			
6. Other		and the second s	
B. WINDOWS			4 7 3 3 3 3
Single hung	CAPITAL	SIXAB HUNG WINDOWS	675)
Horizontal Slider	97.111		1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1
3. Casement			1 11 11 11 11 11 11 11 11 11 11 11 11 1
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass -through	100		1.00
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action	+		
12. Other			1 2 2 1 4 2 7 6 6 kg
C. PANEL WALL	3		
1. Siding			1 1 1 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
2. Soffits		1	F- 7 - 1 - 2-, 195 m
3. EIFS			
4. Storefronts			
5. Curtain walls			7.5 T. T. T. W. T.
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			1951
1. Asphalt Shingles	TAMKO	31AB ASPHALT SHINGLE	1956
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			1 2 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
5. Built-Up Roofing	No. 1		
6. Modified Bitumen			
7. Single Ply Roofing Sys	1 1 11		
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

13. Liquid Applied Roof Sys  14. Cements-Adhesives – Coatings  15. Roof Tile Adhesive  16. Spray Applied Polyurethane Roof  17. Other  SHUTTERS  1. Accordion  2. Bahama  3. Storm Panels  4. Colonial  5. Roll-up  6. Equipment  7. Others  SKYLIGHTS  1. Skylight  2. Other	y/Subcategory (cont.) Manu	Approval Number
14. Cements-Adhesives — Coatings 15. Roof Tile Adhesive 16. Spray Applied Polyurethane Roof 17. Other 19. SHUTTERS 1. Accordion 2. Bahama 3. Storm Panels 4. Colonial 5. Roll-up 6. Equipment 7. Others 8. SKYLIGHTS 1. Skylight 2. Other 9. STRUCTURAL COMPONENTS 1. Wood connector/anchof purson 2. Truss plates 3. Engineered lumber 4. Railing 5. Coolers-freezers 6. Concrete Admixtures 7. Material 8. Insulation Forms 9. Plastics 10. Deck-Roof 11. Wall 12. Sheds 13. Other NEW EXTERIOR ENVELOPE PRODUCTS 1. e products listed below did not demonstrate product approval at plan review. I understand that at the e of inspection of these products, the following information must be available to the inspector on the		
15. Roof Tile Adhesive 16. Spray Applied Polyvertane Roof 17. Other 18. SHUTTERS 19. Accordion 2. Bahama 3. Storm Panels 4. Colonial 5. Roll-up 6. Equipment 7. Others 19. SKYLIGHTS 11. Skylight 2. Other 19. STRUCTURAL 2. Other 19. STRUCTURAL 3. Engineered lumber 4. Railing 5. Coolers-freezers 6. Concrete Admixtures 7. Material 8. Insulation Forms 9. Plastics 10. Deck-Roof 11. Wall 12. Sheds 13. Other NEW EXTERIOR ENVELOPE PRODUCTS 1. e products listed below did not demonstrate product approval at plan review. I understand that at the of inspection of these products, the following information must be available to the inspector on the	ements-Adhesives –	
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