

DESIGN CODE: 2020 FLORIDA BUILDING CODE - RESIDENTIAL

DESIGN IS BASED ON THE DESIGN WIND SPEED AND THE DATE OF THE ORIGINAL PLANS, UNLESS PLANS HAVE BEEN REVIEWED FOR CODE COMPLIANCE.

DESIGN LOADS: ACTUAL AND UNIFORM

	ROOF (code=1.25)	FLOOR (code=1.00)
ROOF LOADING:		
TOP CHORD LIVE LOAD	20 psf	40 psf
TOP CHORD DEAD LOAD	7 psf (ARCH SHINGLES)	10 psf
TOP CHORD DEAD LOAD	20 psf (TILE SHINGLES)	10 psf
BOTTOM CHORD LIVE LOAD	10 psf	5 psf
BOTTOM CHORD DEAD LOAD	5 psf	

DEFLECTION CRITERIA:

ROOF FRAMING: LIVE L/240; TOTAL LOAD L/180

FLOOR FRAMING: LIVE LOAD L/360 & TOTAL LOAD L/240

0.75" MAX ANY CASE

COMPONENTS & CLADDING ALLOWABLE DESIGN PRESSURES			
TRIBUTARY AREA (sf)	INTERIOR		EDGE STRIP (PSF):
	ZONE (PSF)		'o' = 4'-6"
10	+24.61	-26.70	+24.61 -32.93
50	+23.42	-25.51	+23.42 -30.58
100	+22.01	-24.09	+20.91 -27.74

THE VALUES ABOVE ARE ALLOWABLE WIND PRESSURE VALUES (ASD). THE ABOVE PRESSURES HAVE BEEN REDUCED BY 0.50 AS PERMITTED BY THIS ALLOWABLE STRESS DESIGN METHODOLOGY. NO FURTHER REDUCTION SHALL BE PERMITTED.

COMPONENT & CLADDING WALL ELEMENTS SHALL BE DESIGNED FOR BOTH POSITIVE AND NEGATIVE PRESSURES SHOWN IN TABLE ABOVE.

LINEAR INTERPOLATION IS PERMISSIBLE.

PLUS = PRESSURE AND MINUS = SUCTION.

DESIGN OF WINDOWS/DOORS FASTENING TO THE WALL FRAMING IS THE RESPONSIBILITY OF THE WINDOW/DOOR MANUFACTURER/SUPPLIER & SHALL MEET THE ABOVE NOTED POSITIVE AND NEGATIVE PRESSURES.

GARAGE DOOR PRESSURES (PSF)	
1 CAR GARAGE DOOR (8'x7')	
2 CAR GARAGE DOOR (16'x7')	

CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
USP A35	450	450	(9)10d1 $\frac{1}{2}$ " EA.	
USP RT7	585	495	(5)8d EA. END	
USP RT8A	775	655	(5)10d1 $\frac{1}{2}$ " EA. END	
USP MTW12	1195	860	(7)10d1 $\frac{1}{2}$ " EA. END	
USP HTW20	1450	1245	(12)10d1 $\frac{1}{2}$ " EA. END	
USP MSTA24	1640	1455	(9)10d EA. END	
USP MSTA36	2065	2065	(13)10d EA. END	
USP LTS20B	1105	1105	$\frac{1}{2}$ " ϕ ROD TO FTG.	
USP JJS28	1305	1305	(6)10d TO HEADER	
USP HT116	4290	4290	$\frac{3}{4}$ " ϕ ROD TO FTG.	
USP HT122	5370	5370	$\frac{3}{4}$ " ϕ ROD TO FTG.	
USP PAU44	2535		$\frac{3}{4}$ " ϕ ROD w/ (12)16d	
USP PAU68	2535		$\frac{3}{4}$ " ϕ ROD w/ (12)16d	
USP MSTM24	1545	1455	(5) $\frac{1}{4}$ "x2- $\frac{1}{2}$ " TAPCONS	

CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
USP A35	450	450	(9)10d1 $\frac{1}{2}$ " EA.	
USP RT7	585	495	(5)8d EA. END	
USP RT8A	775	655	(5)10d1 $\frac{1}{2}$ " EA. END	
USP MTW12	1195	860	(7)10d1 $\frac{1}{2}$ " EA. END	
USP HTW20	1450	1245	(12)10d1 $\frac{1}{2}$ " EA. END	
USP MSTA24	1640	1455	(9)10d EA. END	
USP MSTA36	2065	2065	(13)10d EA. END	
USP LTS20B	1105	1105	$\frac{1}{2}$ " ϕ ROD TO FTG.	
USP JJS28	1305	1305	(6)10d TO HEADER	
USP HT116	4290	4290	$\frac{3}{4}$ " ϕ ROD TO FTG.	
USP HT122	5370	5370	$\frac{3}{4}$ " ϕ ROD TO FTG.	
USP PAU44	2535		$\frac{3}{4}$ " ϕ ROD w/ (12)16d	
USP PAU68	2535		$\frac{3}{4}$ " ϕ ROD w/ (12)16d	
USP MSTM24	1545	1455	(5) $\frac{1}{2}$ "x2- $\frac{1}{2}$ " TAPCONS	

SIMPSON CONNECTORS				
CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
A35	450	450	12-Bdx1½"	10446.4
DT2.5T	600	520	5-8d EA. END	11478.3
HTS18	1150	1085	16-10d EA. END	10456.6
MTS12	1000	850	7-10dx1½" EA. END	10456.3
HTS20	1450	1245	24-10dx1½" EA. END	13872.3
MSTA24	1785	1270	9-10d EA. END	13872.4
MSTA36	2050	1670	13-10d EA. END	13872.8
HTT4	3480	3080	18-16d TO TRUSS/BEAM 1-¾" Ø ROD TO FTG.	11496.2
HTT5	5250	4670	32-16d TO TRUSS/BEAM 1-¾" Ø ROD TO FTG.	11496.2
LUS28	930	780	6-10d TO HEADER 4-10d TO JOIST	10655.113
HU410	905	785	14-16d TO HEADER 6-16d TO JOIST	10531.36
ABU44	2200		¾" Ø ROD EPOXIED 6" MIN	10849.6
ABU66	2300		¾" Ø ROD EPOXIED 6" MIN	10849.6
SET	N/A	N/A	SIMPSON EPOXY-TIE	11506.4
LTT20B	1675	1675	10-16d TO STUD/BEAM/POST 1-½" Ø ROD TO FTG.	11496.3
LSTA12	805	895	10-10d	13872.5
CS16	1705	1705	13-8d	10852.1

PLANS AND METHODS.

THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES; FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE FOR ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

PLANS AND METHODS.

THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES; FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE FOR ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

THE FOLLOWING: CONTINUOUS LOAD PATH FOR WIND UPLIFT, WOOD PANEL SHEARWALLS, WALL FRAMING AND REQUIRED SHEATHING AND HEADERS DIRECTLY SUPPORTING ROOF FRAMING. ITEMS NOT DESIGNED PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSSES, FLOOR FRAMING NOT SPECIFICALLY ADDRESSED, BRUSS-TO-BRASS CONNECTION, AND ANY ARCHITECTURAL, MECHANICAL OR ELECTRICAL SYSTEM.

LOOR SHEATHING SPECIFICATIONS:
23/32" TAG OSB OR PLYWOOD SHEATHING, GLUE AND NAIL WITH 10d COMMON @ 6" O.C. EDGE & FIELD

ROOF SHEATHING SPECIFICATIONS:
SINGLE- MIN. 15/32", 32/16, APA RATED OSB OR PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).
TILE - MIN. 15/32" 32/16, APA RATED PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).
METAL - MIN. 1/2", 24/16, APA RATED PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

WALL SHEATHING SPECIFICATIONS:
BRICK - MIN. 1/2", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL EXTEND FULL HEIGHT FROM BOTTOM PLATE TO UPPER TOP PLATE. FLEXIBLE FINISH WALLS INCLUDE: WOOD, CEMENT, OR VINYL SPOING, HARD PLATE & BRICK. OTHER WALL SHALL BE CONSIDERED BRITTLE FINISH.

STUCCO FINISH- MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL ORIENTED WITH THE LONG DIMENSION PERPENDICULAR TO THE STUDS. CONTRACTOR MAY USE 3/8" STRUCTURAL I GRADE SHEATHING OR 5/8" OSB SHEATHING AND ORIENT THE PANELS VERTICALLY.

MASONRY SPECIFICATIONS:
MASONRY SHALL BE DESIGNED IN ACCORDANCE WITH ACI 530-05, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI530.1-05. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM OF 28 DAY COMPRESSIVE STRENGTH OF 2000 psi per ASTM C1019. GROUT SHALL HAVE A MAXIMUM COURSE AGGREGATE SIZE OF 3/4" PLACED AT AN 8" TO 11" SLUMP. MORTAR SHALL CONFORM TO ASTM C270 AND TYPE M OR S. TYPE N MORTAR MAY BE USED IN BRICK VENEER. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL FLASHING.

CONCRETE MASONRY UNITS (CMU):
CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 psi (f'm=1500 psi). GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

MASONRY STEWALLS: ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90, E GRADE NO.1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE "S" MORTAR. WALL COURSE SHALL BE RUNNING BOND, STACK BOND SHALL NOT BE USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI PEA ROCKET CONCRETE GROUT. SPICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIMENSIONS. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH #4 @ 4'-0" C. MAX. AND AT EACH CORNER, WALL END, AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS, 1/2" MINIMUM OF 40 BAR DIMENSIONS INTO EACH ELEMENT. AT STEWALL CONNECTIONS, CONSTRUCTED OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCEMENT AT 16" C. VERTICALLY, (EVERY OTHER COURSE), AND VERTICAL REINF. SHALL BE INCREASED AS NOTED ON S/D'S. UNLESS NOTED OTHERWISE, LAP JOINT REINFORCING SHALL BE A MINIMUM OF 6".

CONCRETE SPECIFICATIONS:
ALL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS
CONCRETE AT GARAGE AND PORCH SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

GENERAL NOTES:

FOOTING AND FOUNDATIONS: SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES. FOOTING HAVE BEEN DESIGNED WITH A SOIL BEARING (DESIGN MAXIMUM) OF 2000 PSF. A SOILS INVESTIGATION REPORT IS RECOMMENDED TO VERIFY SUITABLE SUBSURFACE CONDITIONS. IF THE FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED OR UNSTABLE SOIL, THE ENGINEER SHALL BE NOTIFIED. SOIL SHALL BE FREE OF ORGANIC MATERIAL AND COHESIVE (CLAY) SOILS. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.

FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION, FOR EXISTENCE, FEATURES, CONDUITS, ELECTRICAL, EMBEDED, STEP HEIGHTS, ETC., SEE ARCHITECTURAL PLANS. DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE FOUNDATION PLAN SHOWN ON S1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT DETERMINED ON PLAN THEN CONTACT ENGINEER OF RECORD (EOR).
UNLESS OTHERWISE NOTED ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 3" IN FOOTINGS AND MESH SHALL BE CENTERED IN SLAB ON GRADE. IN ALL CONTINUOUS FOOTINGS PROVIDE #3 @ 48" O.C. OR ROD CHAIRS PROVIDE CONTINUITY OF REINFORCING THROUGH JOINTS.

CONCRETE SLABS ON GRADE:
SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL SOLI TREATMENT FOR PREVENTION OF SUBTERRANEAN TERMITES. **SAWCUTS:** FOR

WOOD FRAMING SPECIFICATIONS:
ALL WOOD FRAMING HAS BEEN DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL SHALL BE PRESURE-TREATED, IF, ACQ OR NON-DOM BORATE PRESERVATIVE TREATMENT IS USED, ALL ATTACHED FASTENERS SHALL BE HOT DIPPED GALVANIZED. IF ACZA PRESERVATIVE IS USED, ALL ATTACHED FASTENERS SHALL BE STAINLESS STEEL.

PRE-ENGINEERED WOOD TRUSSES:
SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE WHERE PROJECT IS BEING BUILT AND SHALL COMPLY WITH NFPA, TPI, AND AITC 100. CONTRACTOR SHALL VERIFY THAT ADEQUATE TRUSS BEARING IS INSTALLED AT ALL TRUSSES AS INDICATED IN THE TRUSS SHOP

DRAWINGS, ALL TRUSS-TO-TRUSS CONNECTIONS AND TRUSS PROFILES ARE THE RESPONSIBILITY OF THE DELEGATED TRUSS ENGINEER. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER "COMMENTARY" AND RECOMMENDATION FOR HANDLING, INSTALLING & BRACING MEANT PLATE CONNECTED WOOD TRUSSES, HEB-91," AT MULTIPLE PLATE CONNECTIONS, SPREAD STRAPS TO AVOID NAILING CONFLICTS THROUGH TRUSSES. WHEN USING (2) STRAPS ON SINGLE PLY TRUSSES, PLACE STRAPS DIAGONALLY ACROSS DELT TOP PLATE FROM EA. OTHER.

ROOF COVERING SPECIFICATIONS:

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGS SHALL COMPLY WITH ASTM D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. CLAY AND TILE ROOFS SHALL BE

INSTALLER FOR THE CONCRETE AND GUT ROOF FLEE INSTALLATION MANUAL, AND THE MANUFACTURER'S REQUIREMENTS. STANDING SEAM METAL ROOF SHALL COMPLY WITH ASTM E574 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS.

WATERPROOFING:
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN/INSTALLATION OF ALL WATER PROOFING.

MEMBERS	CONNECTION TYPE	FASTENER
TOP PLATE TO TOP PLATE	FACE NAIL	2-GUN NAILS @ 12" ST
TOP PLATE, LAP/INTERSECTION	FACE NAIL	(2-16d) 3-GUN NAIL
DBL. TOP PLATE TO STUD	FACE NAIL	(2-16d) 3-GUN NAIL
RN JOIST TO TOP PLATE	TOE NAIL	(8d @ 6") GUN NAIL @
CEILING JOIST TO TOP PLATE	TOE NAIL	(3-8d) 5-GUN NAILS
CEILING JOIST, OVER PARTITIONS	FACE NAIL	(3-16d) 4-GUN NAILS
CEILING JOIST TO ROOF RAFTER	FACE NAIL	(6-16d) 8-GUN NAILS
JOIST/TRUSS TO PLATE	TOE NAIL	(2-16d) 3-GUN NAILS
RAFTER TO PLATE	TOE NAIL	(3-8d) 3-GUN NAILS
KICK RAFTER TO HIP	TOE NAIL	(3-10d) 4-GUN NAILS
ROOF RAFTER TO 2x ₄ RIDGE BM.	FACE NAIL	16d @ 18" O.C. EDGE
CONT. HEADER, TWO PIECES	FACE NAIL	(3-16d) 4-GUN NAILS
CONT. HEADER TO STUD	TOE NAIL	(3-16d) 4-GUN NAILS
STUD TO SOLE PLATE	TOE NAIL	(3-16d) 4-GUN NAILS
SOLE PLATE TO JOIST/BLOCKING	FACE NAIL	(16d @ 18") GUN NAIL

NAIL SPECIFICATIONS	
3"x0.131" = 6d	2"x0.113" = SHANK RANK
2"x0.113" = 4d	2 1/2"x0.131" = 8d
3"x0.148" = 10d	3 3/8"x0.162" = 16d
1 1/2"x0.148" = 10d(1x2)	1 1/2"x0.131" = 8d(1x1)

LINTEL DIMENSION	MIN. BRG.	MAX. SPAN
$1\frac{3}{4} \times 3\frac{3}{4} \times \frac{1}{4}$	4"	6'-0"
$1\frac{1}{2} \times 3 \times \frac{1}{4}$	6"	8'-0"
$1\frac{5}{8} \times 3\frac{1}{2} \times \frac{1}{4}$		10'-0"
$1\frac{1}{2} \times 3 \times \frac{1}{4}$	6"	12'-0"
$1\frac{7}{8} \times 3 \times \frac{1}{4}$	6"	16'-0"

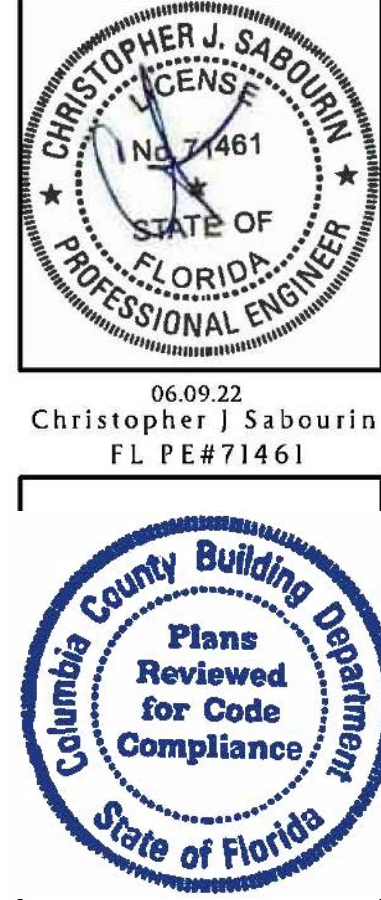
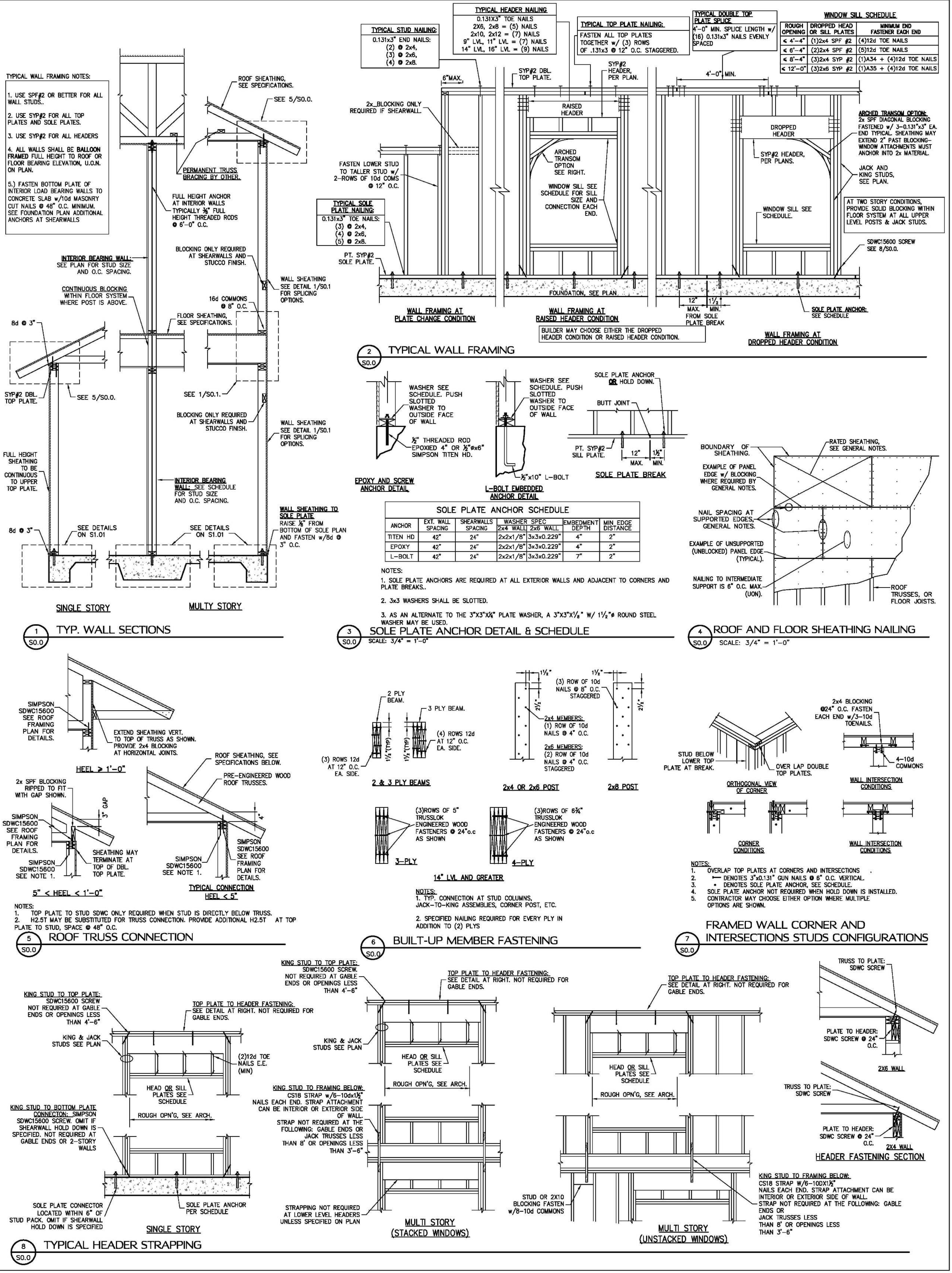
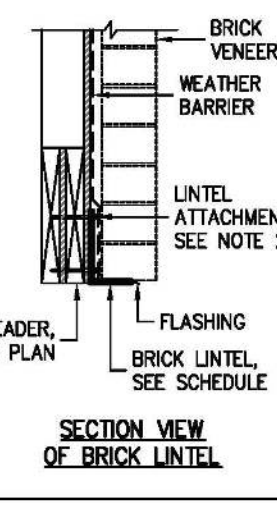
STEEL LINTEL MUST BE MINIMAL 36" LINTEL SIZE HAVE CORROSION RESISTANT COATING EPOXY BASED PAINT.

LINTEL MORE THAN 8'-0" SHOULD BE TERRAPLY SUPPORTED NOT TO EXCEED 6 C. $W \geq 2 \times 1/32"$ WD. SCREWS INTO ROOF DECKING AT $1/2"$ VERTICAL SLOTTED LIE FOR SCREW.

BRICK VENER ATTACHMENT: HORIZONTAL C. @ 24" C.C. VERT. TIES @ 12" C.C. OR 10" MPH WIND-ZONE VERT. TIES @ 16" C.C.). AT ALL OPENINGS SPACE TIES WITHIN 12" OF OPENING. PROVIDE $1/4"$ WELD LIES @ 33" C.C. IMMEDIATELY ABOVE FLASHING.

SECTION VIEW OF BRICK LINTEL

	INTERIOR LOAD BEARING WALL		BUILT-UP POST IN THE WALL
	GABLE X-BRACE, SEE DETAIL 10/501	(2)2x6-1/2	HEADER SIZE, JACK AND KING STUD QUANTITY.
DESIGNATES SHEARWALL, THE HIDDEN LINE DESIGNATES SIZE OF THE SHEARWALL SHEATHING TO BE APPLIED.			
8d @ 3/6 DESIGNATES 8d COMMONS @ 3' O.C. IN THE FIELD			
	SW 3' 6"		
<hr/>			
ADJ. = ADJACENT		LG = Long	
BM = BEAM		MANUF = Manufacture	
BOT = BOTTOM		MONO = Monolithic	
BRG = BEARING		OC = On Center	
CMU = CONCRETE MASONRY UNIT		OSB = Oriented Strand Board	
DBL = DOUBLE		PERP = Perpendicular	
DLA = DIAMETER		PRG ENG = Pre Engineered	
EA = EACH		PSF = Pounds per Square Foot	
EE = EACH END		PSF = Pounds per Square Inch	
ENR = ENGINEER OF RECORD		PT = PRESSURE TREATED	
EQ = EQUAL		QT = Quick Tie	
EXT = EXTERIOR		REINF = Reinforce	
FLD = FLORIDA BUILDING CODE		SF = Square Foot	
FTN = FOUNDATION		SPR = Spruce Pine Fir	
FT = FOOT		SYF = Southern Yellow Pine	
FTC = FOOTING		THRU = Through	
HDN = HEADER		TPY = Typical	
HZB = HORIZONTAL		UNF Unless Otherwise Noted	
JRS = JOISTS		VERT = Vertical	
		WWD = Welded Wire Fabric	



SABO
STRUCTURAL
ENGINEERING
CA#32529
235 9TH AVE N
JAX BEACH, FL 32250
904-712-5750
CHRIS@SABOENG.COM

PLAN NAME	
BZEC	
SSE No.	
22-0249	

ISSUE	DATE
PERMIT	06.09.22
REVISIONS	DATE

STRUCTURAL ENGINEERING FOR
THE PARNELL RESIDENCE

FIELD ALTERATION
 CONTRACTOR SHALL CONTACT SABO
 STRUCTURAL ENGINEERING PRIOR TO
 MAKING ANY STRUCTURAL FIELD
 MODIFICATIONS WHICH MAY VARY
 FROM THE INTENT OF THE ORIGINAL
 CONSTRUCTION DOCUMENTS. ANY
 FIELD ALTERATIONS MADE PRIOR TO
 BEING APPROVED BY CHRISTOPHER
 SABOURIN MAY RESULT IN ADDITIONAL
 ENGINEERING OR INSPECTION FEES.

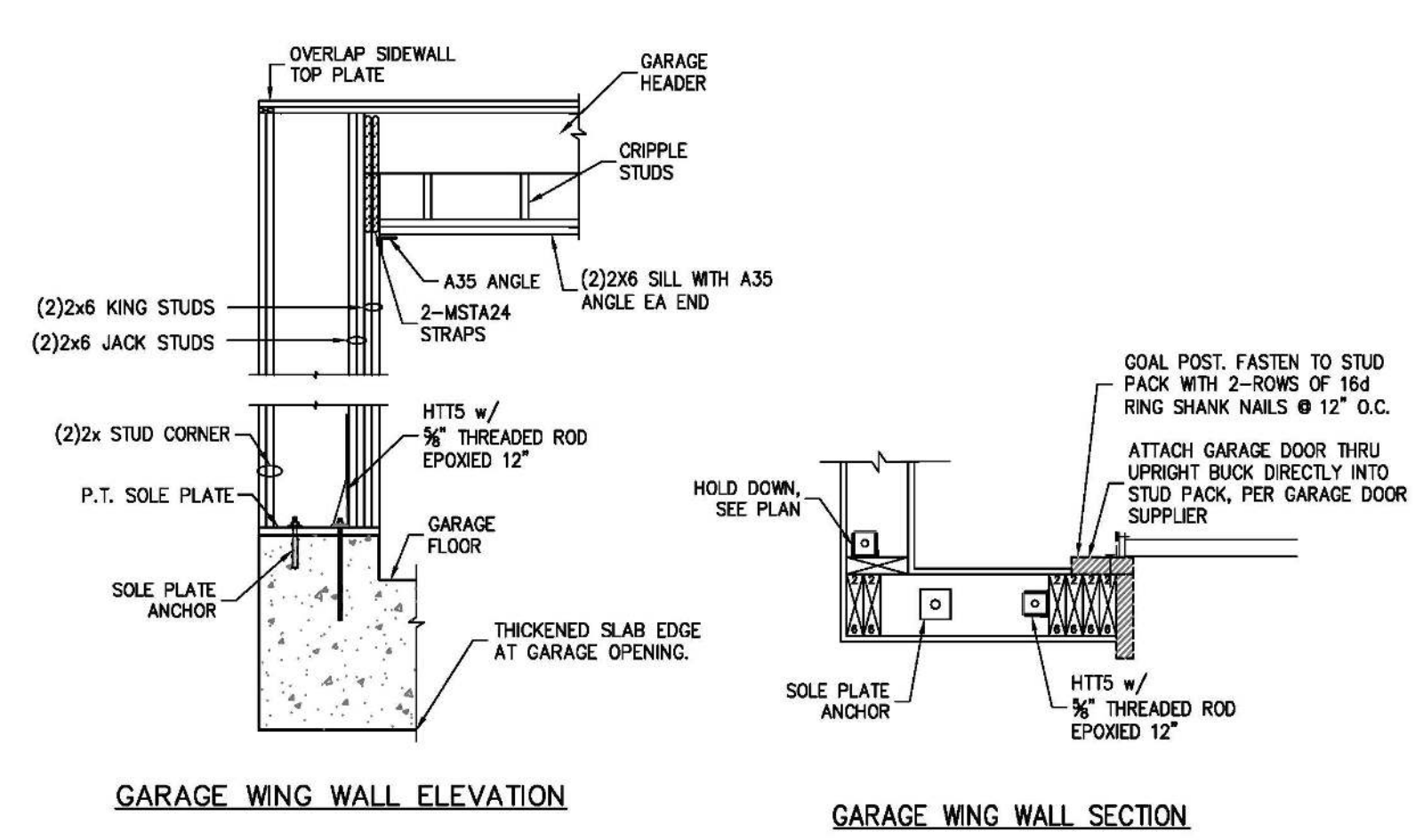
SCALING
 DO NOT SCALE DIMENSIONS FROM
 THESE DRAWINGS. IF A DIMENSION IS
 UNCLEAR REFER TO THE
 ARCHITECTURAL DRAWINGS OR
 CONTACT THE E.O.R.

DESIGN
CRITERIA AND
GENERAL
NOTES

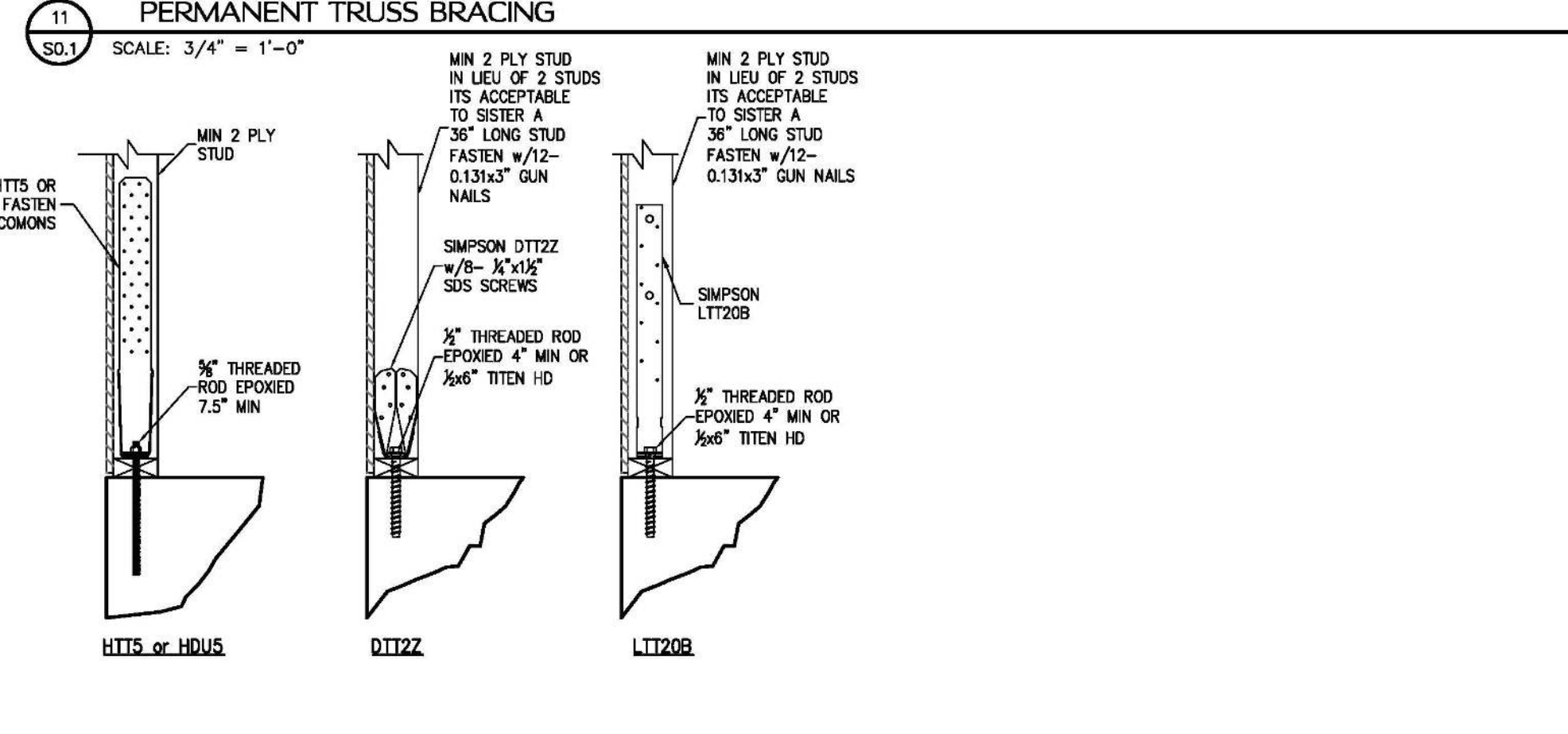
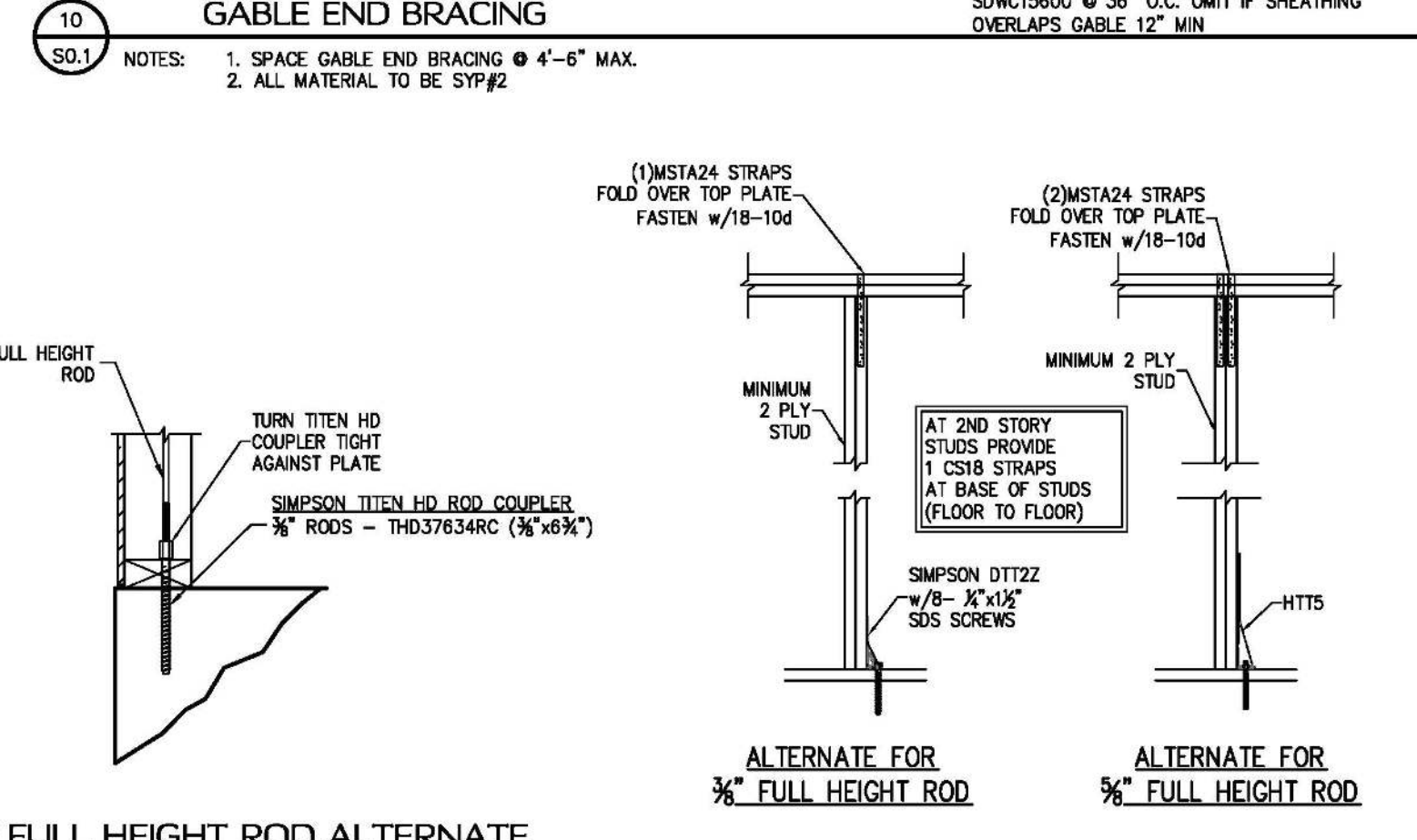
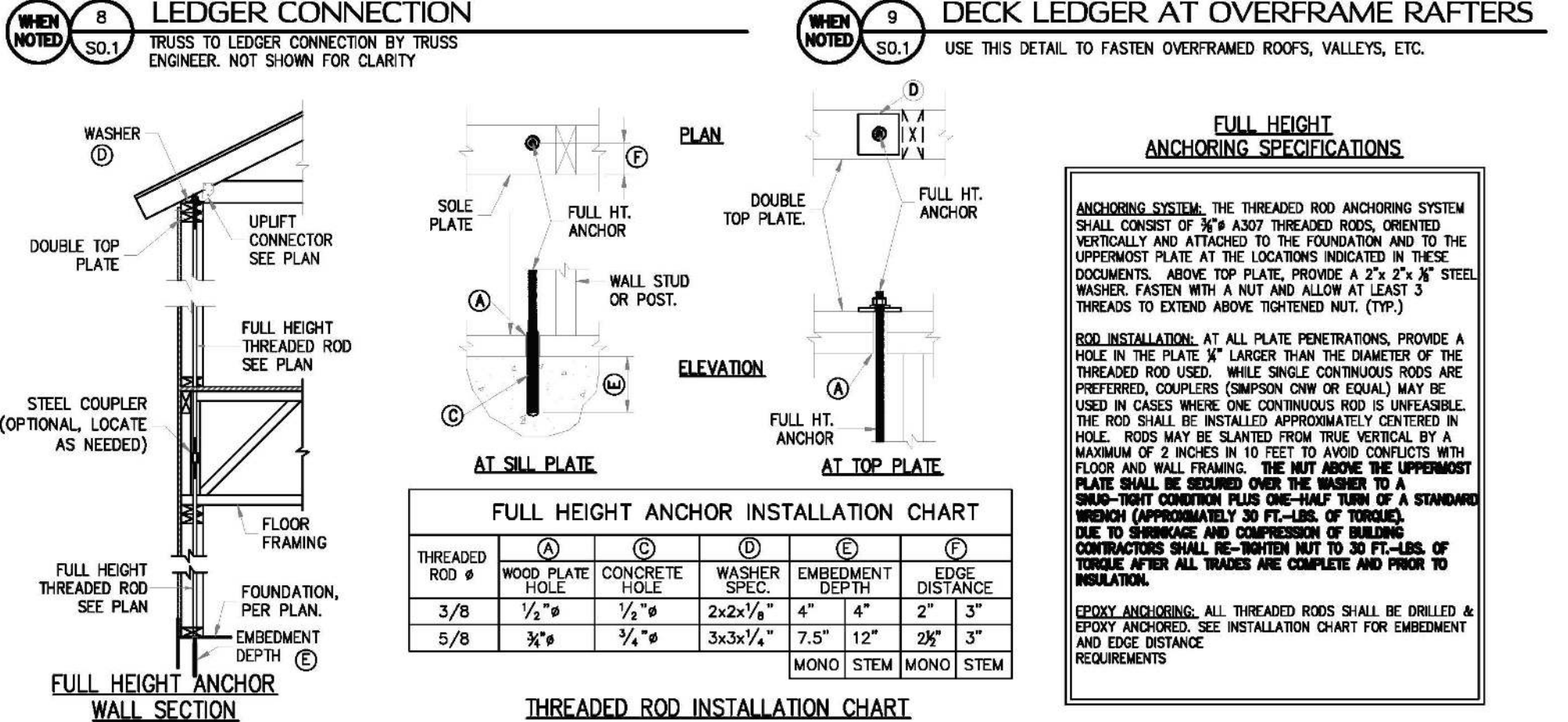
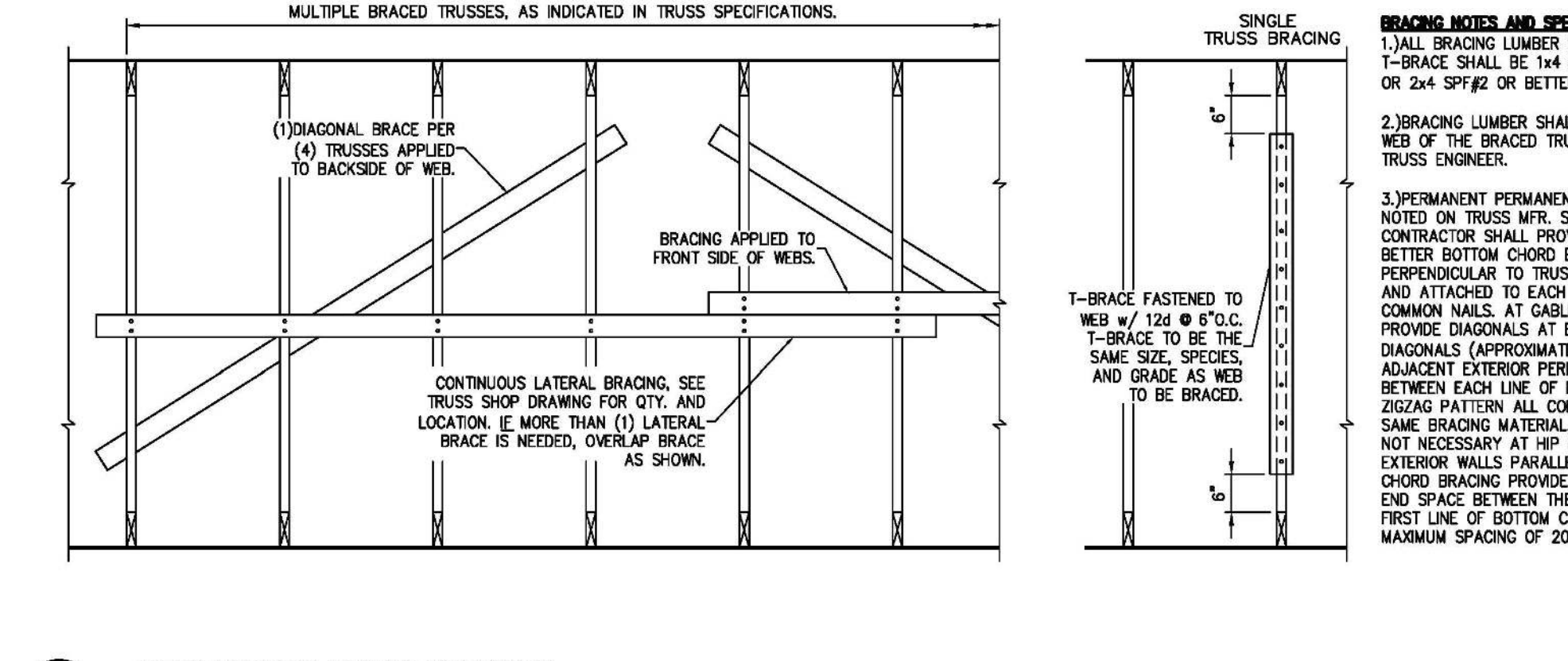
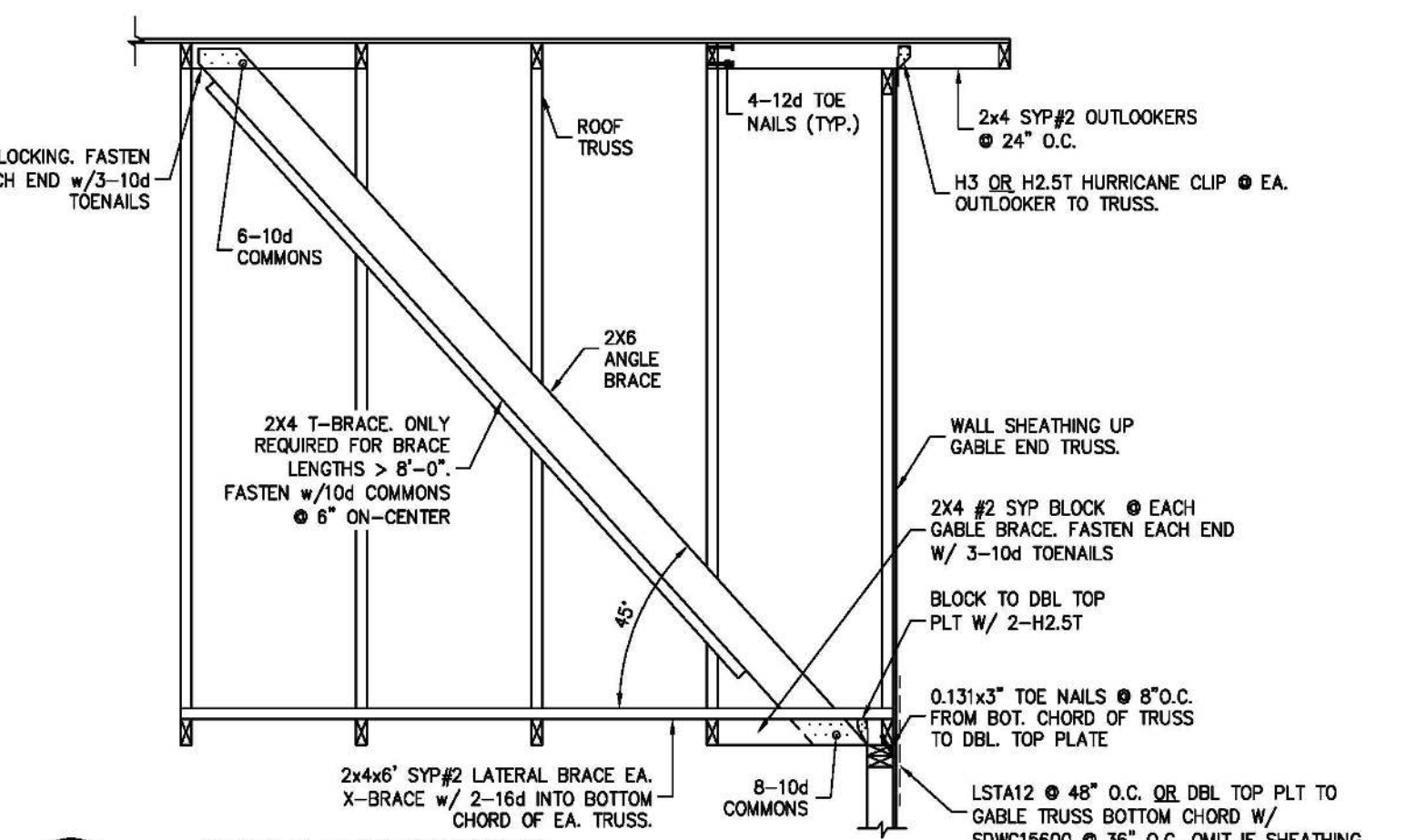
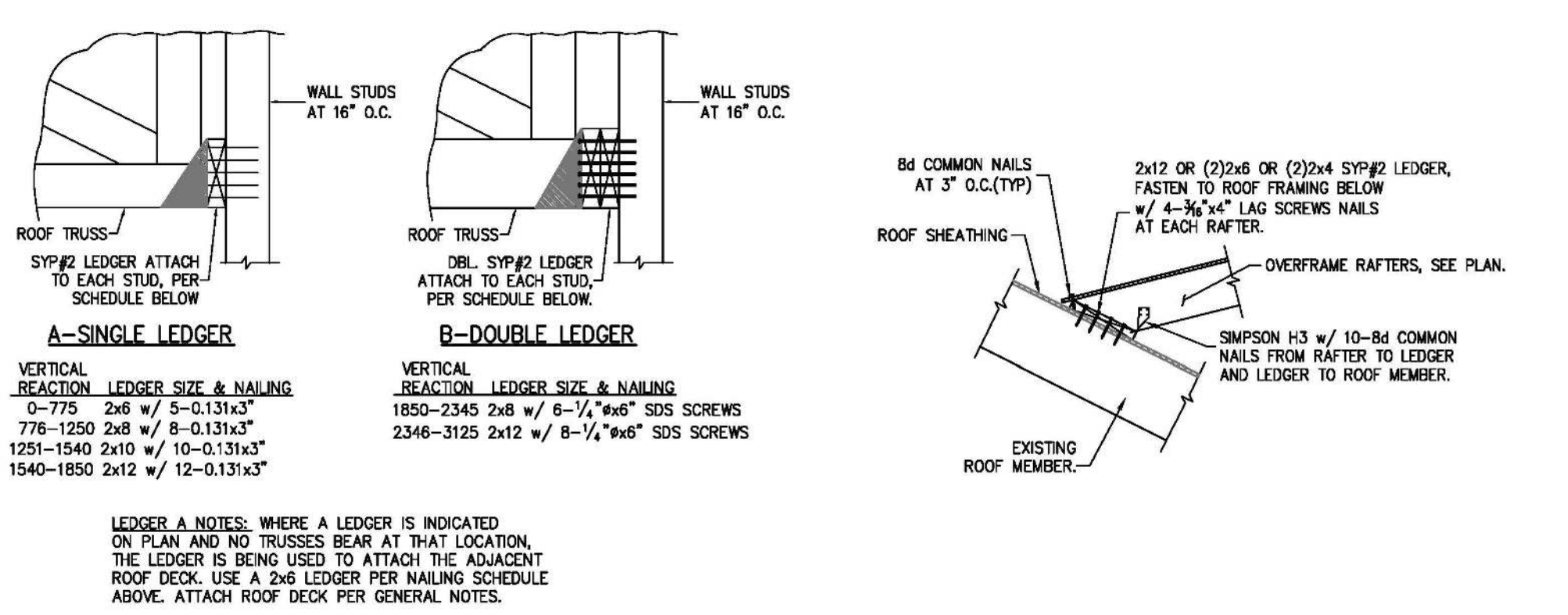
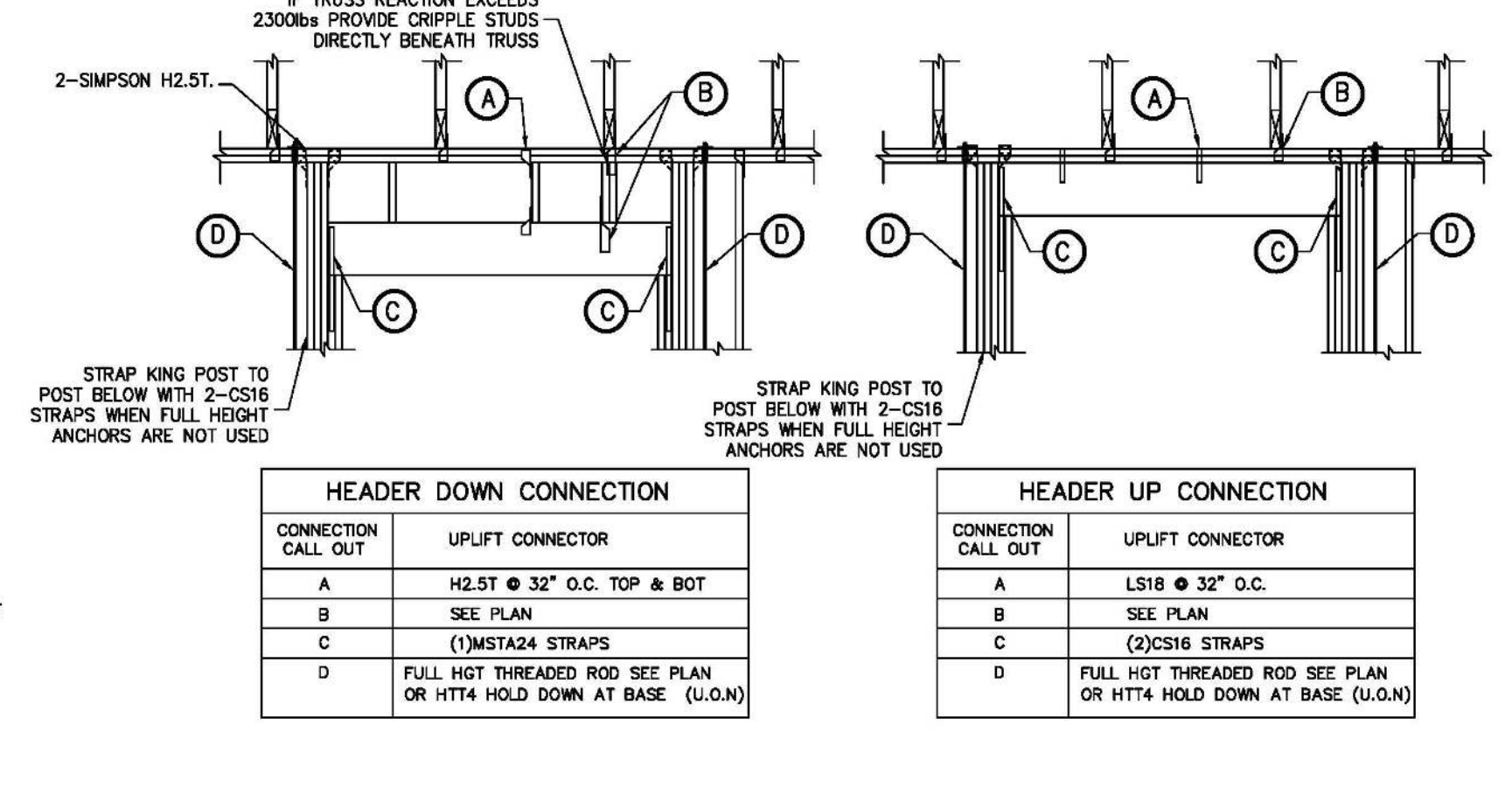
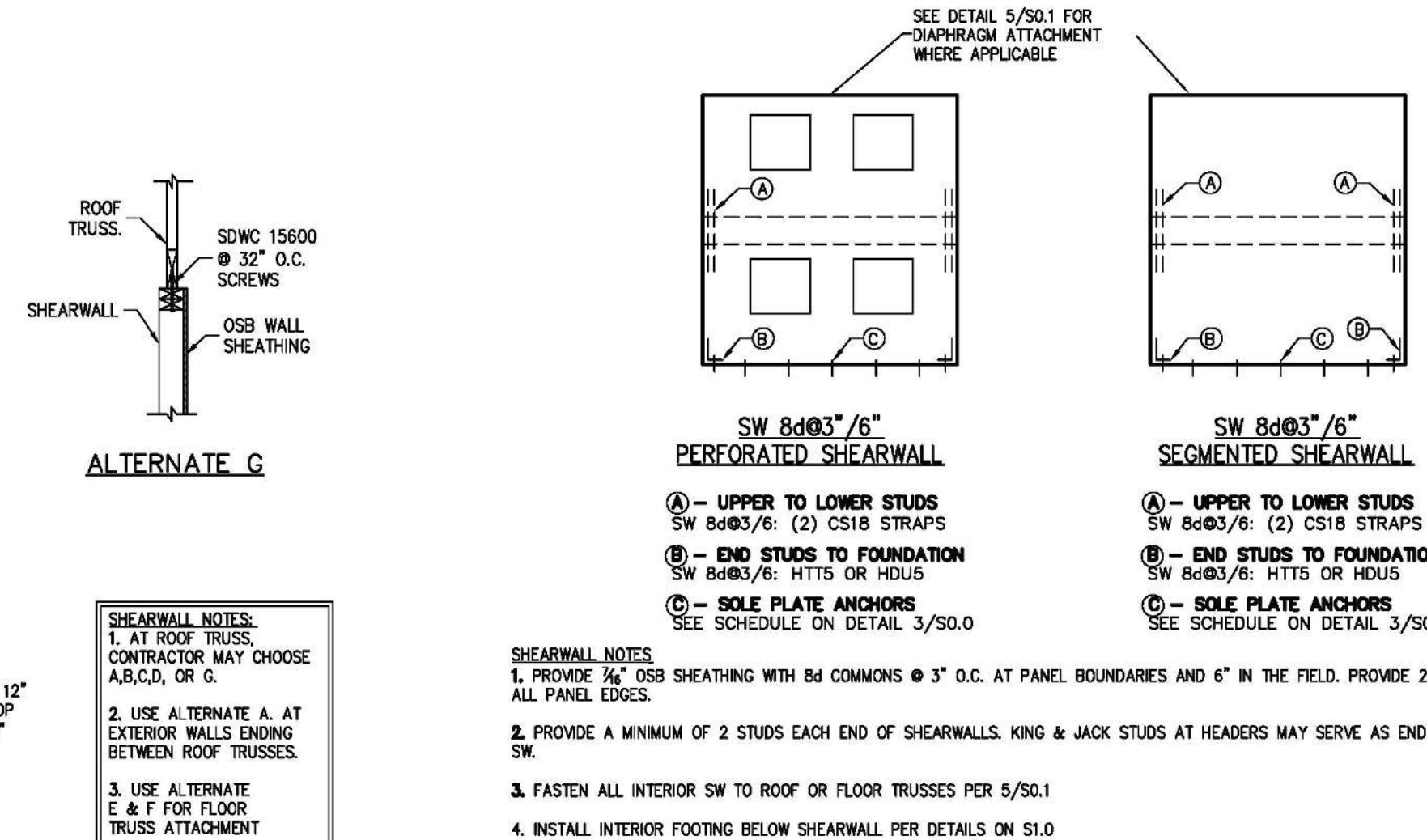
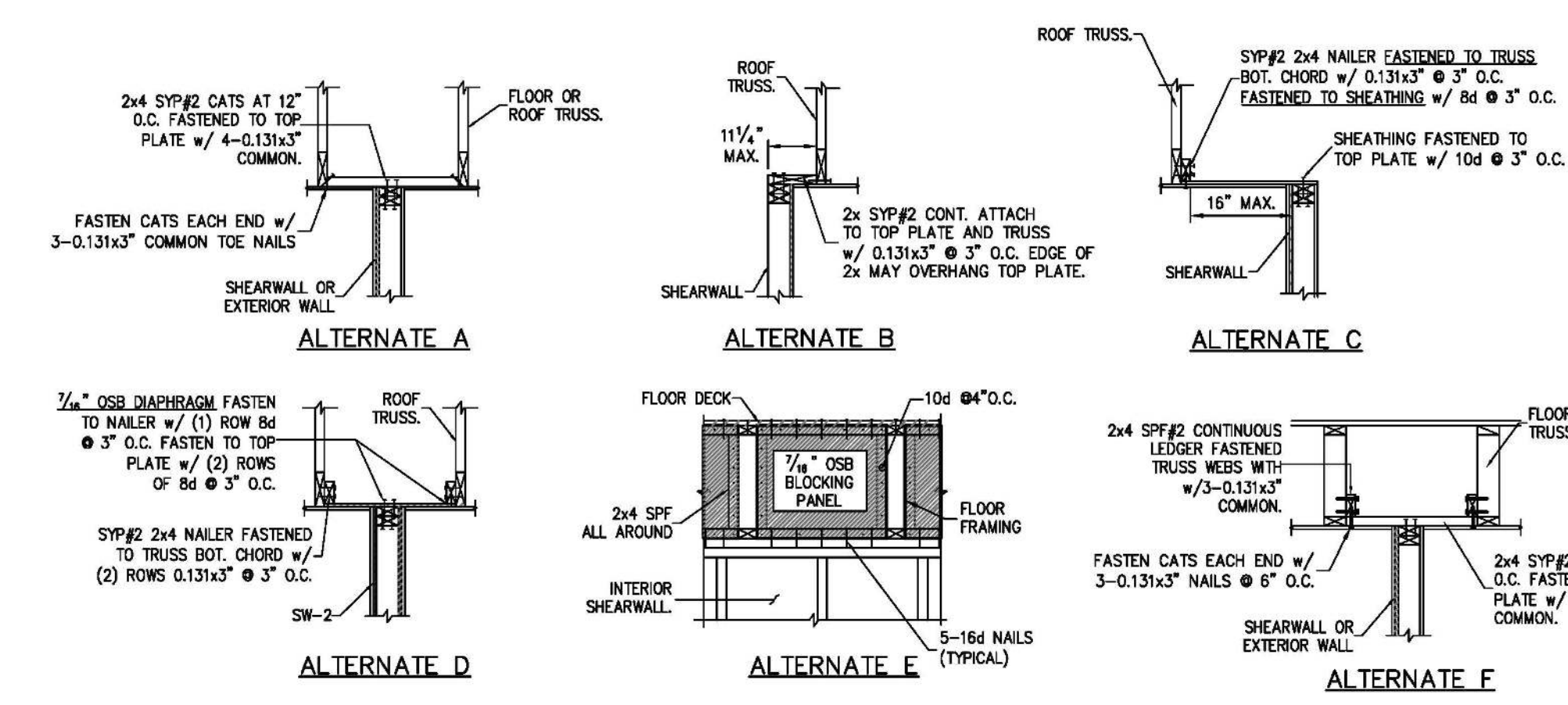
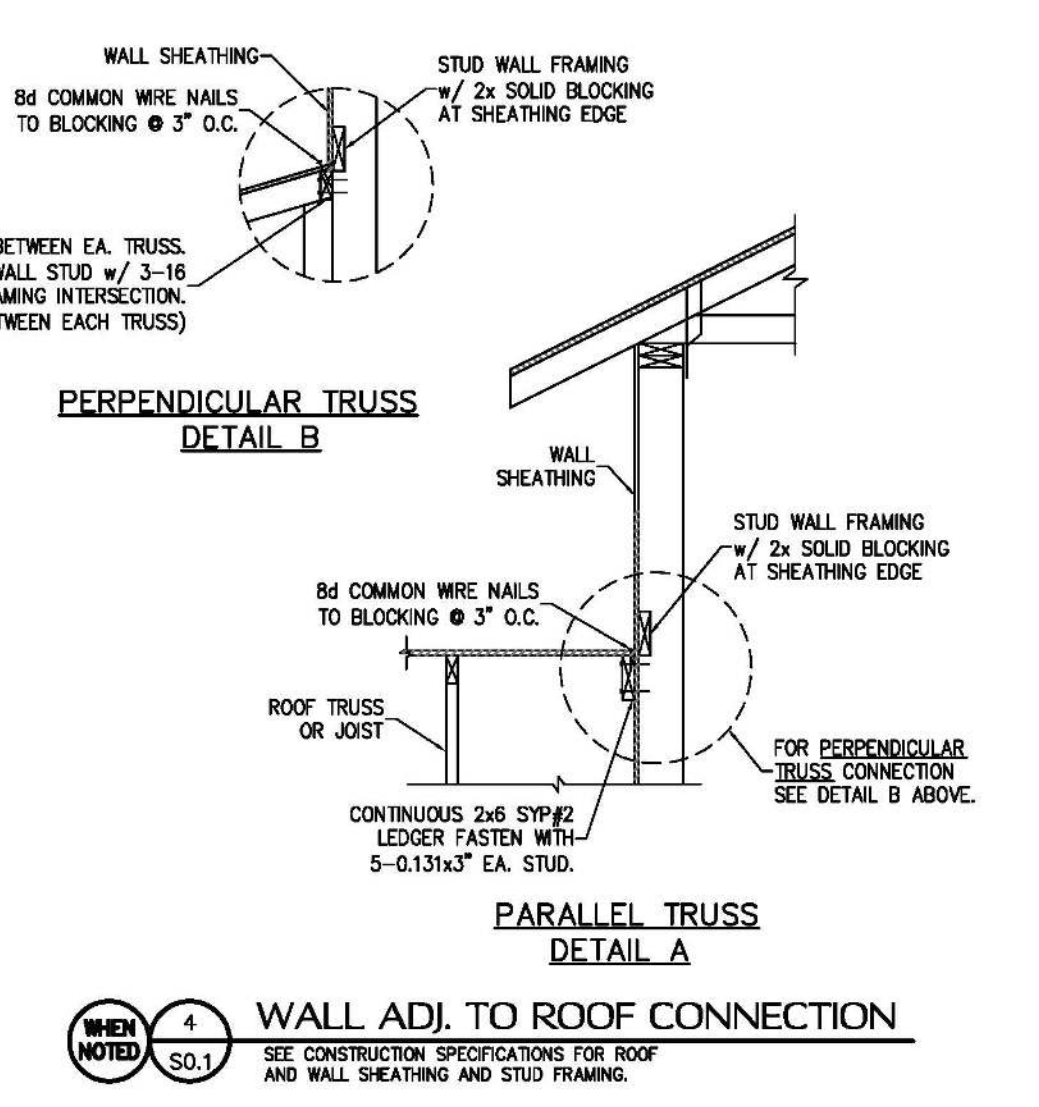
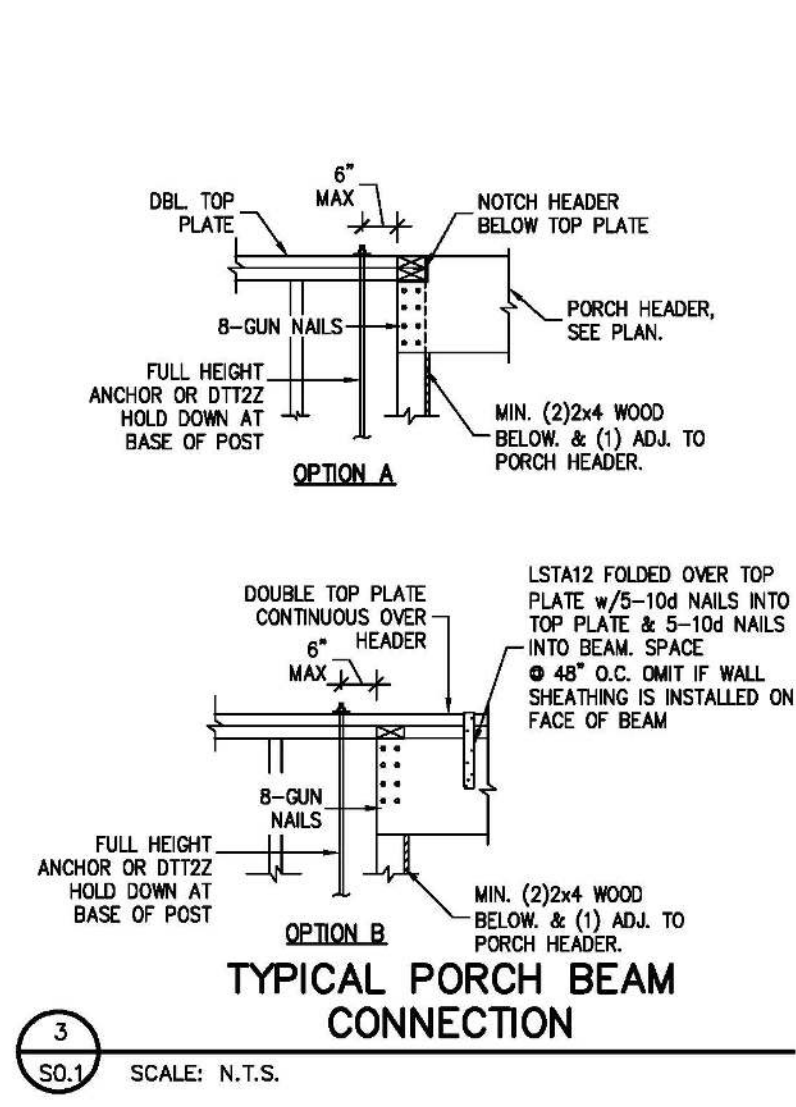
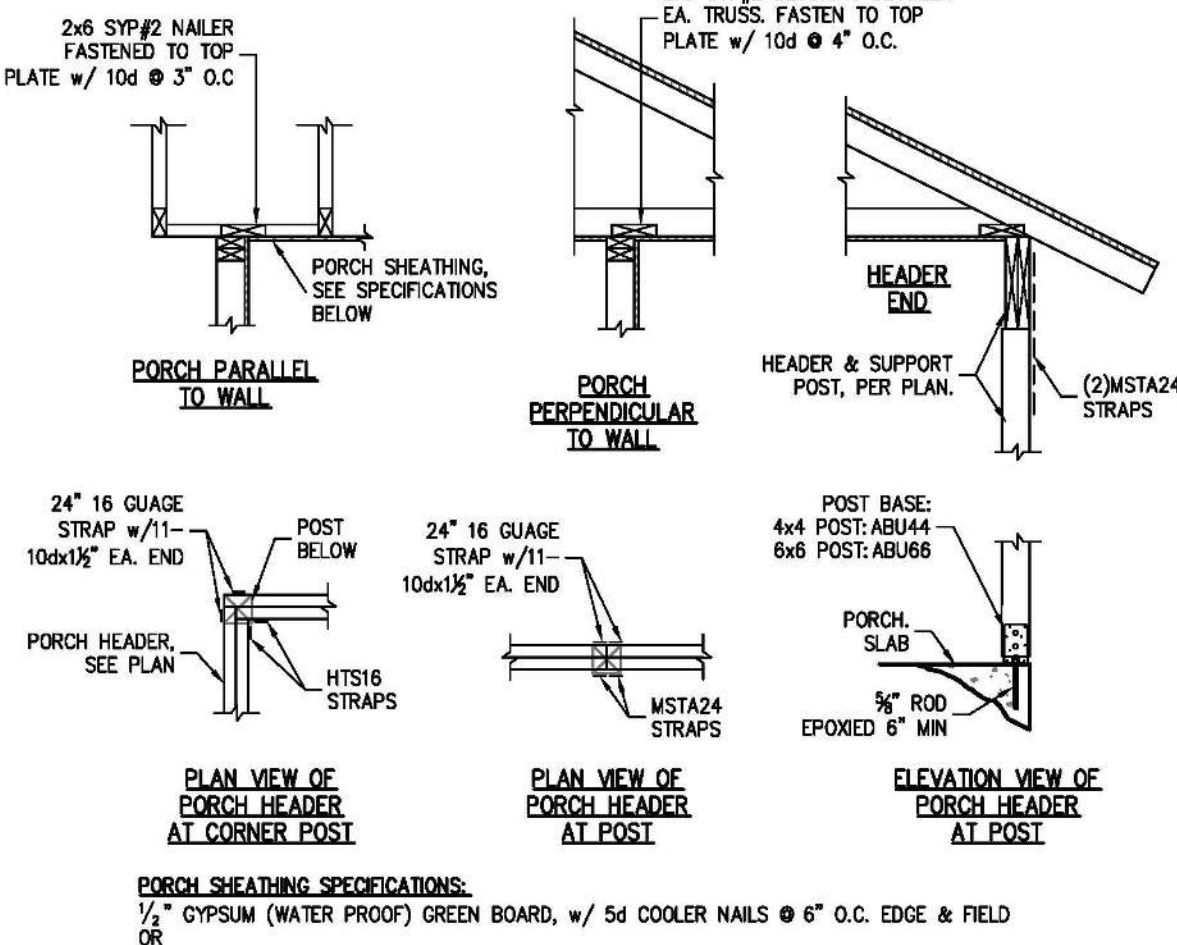
SHEET

S0.0

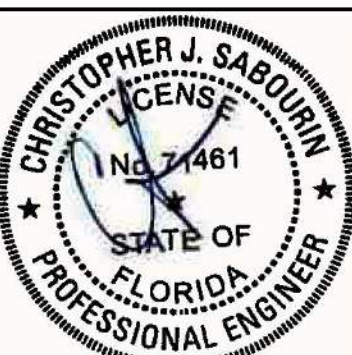
SHEET 1 OF 7



1 WHEN NOTED S0.1 GARAGE HEADER FRAMING SCALE: N.T.S.



SYMBOLS LEGEND	
	DESIGNATES FOOTING LINE
	DESIGNATES SAWCUT LINE
	STEM WALL FOOTING
	DESIGNATES SLAB RECESS



06.09.22
Christopher J. Sabourin
FL PE #71461

SABO
STRUCTURAL
ENGINEERING
CA#32529
235 9TH AVE N
JAX BEACH, FL 32250
904-712-5750
CHRIS@SABOENG.COM

PLAN NAME	BZEC
SSE No.	22-0249

ISSUE	DATE
PERMIT	06.09.22
REVISIONS	DATE

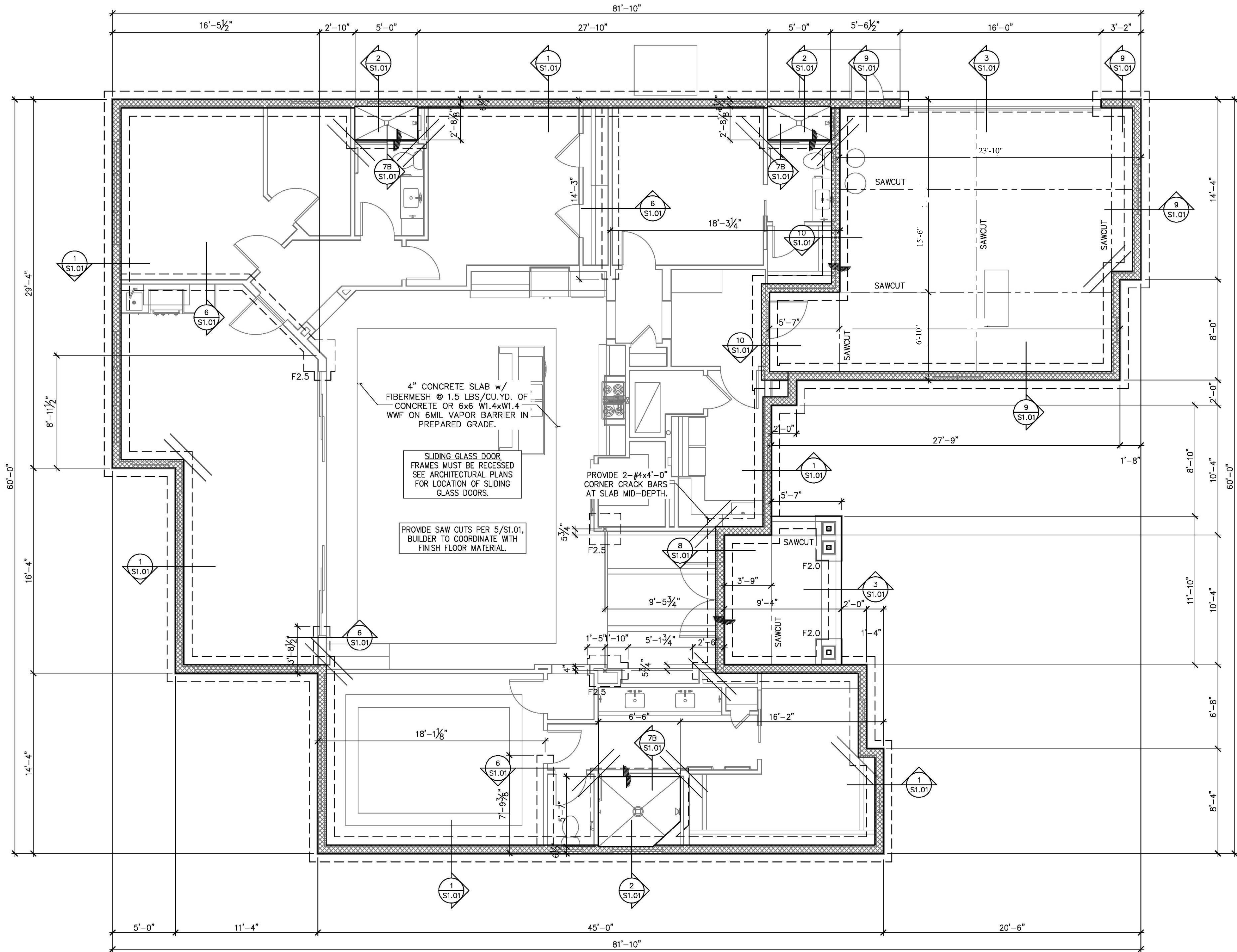
STRUCTURAL ENGINEERING FOR
THE PARNELL RESIDENCE

FIELD ALTERATION
CONTRACTOR SHALL CONTACT SABO
STRUCTURAL ENGINEERING PRIOR TO
MAKING ANY STRUCTURAL FIELD
MODIFICATIONS WHICH MAY VARY
FROM THE INTENT OF THE ORIGINAL
CONSTRUCTION DOCUMENTS. ANY
FIELD ALTERATIONS MADE PRIOR TO
BEING APPROVED BY CHRISTOPHER
SABOURIN MAY RESULT IN ADDITIONAL
ENGINEERING OR INSPECTION FEES.

SCALING
DO NOT SCALE DIMENSIONS FROM
THESE DRAWINGS. IF A DIMENSION IS
UNCLEAR REFER TO THE
ARCHITECTURAL DRAWINGS OR
CONTACT THE E.O.R.

**FOUNDATION
PLAN**

SHEET
S1.0
SHEET 3 OF 7

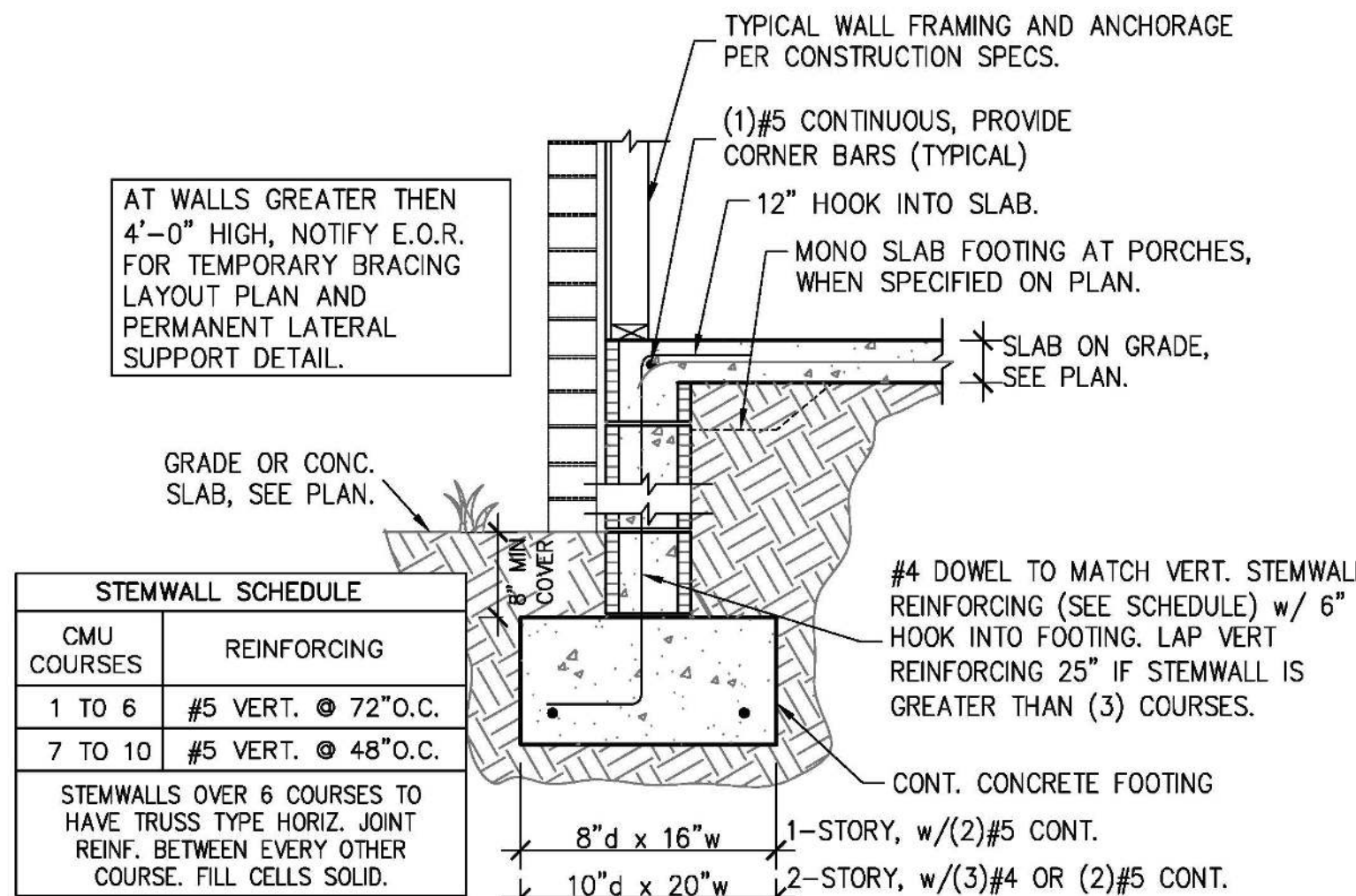


FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

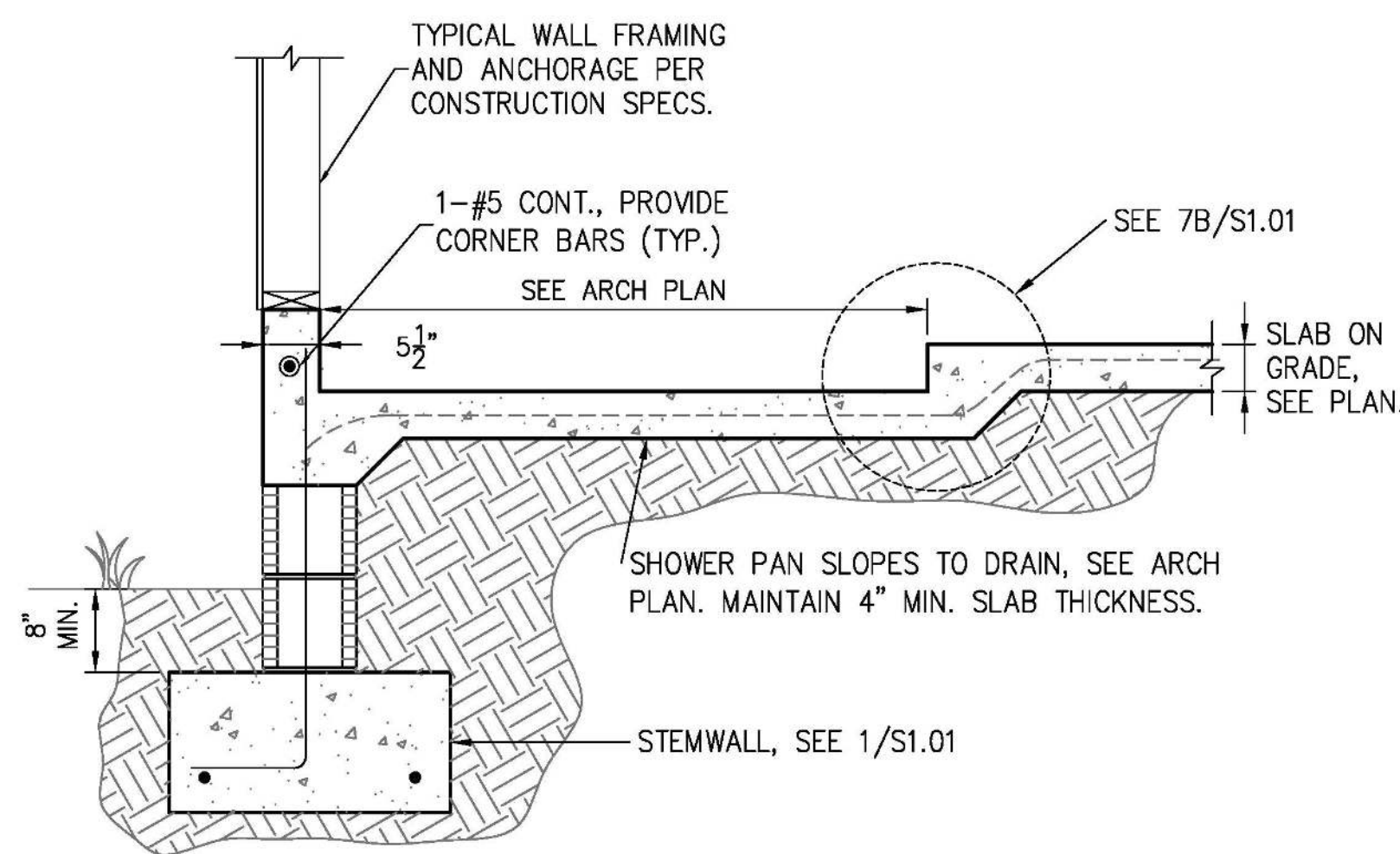
FOOTING SCHEDULE AND NOTES

TYPE	LENGTH	WIDTH	DEPTH	BOTTOM BARS
F2.0	2'-0"	2'-0"	1'-0"	3-#5 EA. WAY BOT.
F2.5	2'-6"	2'-6"	1'-0"	3-#5 EA. WAY BOT.
F3.0	3'-0"	3'-0"	1'-0"	3-#5 EA. WAY BOT.
F3.5	3'-6"	3'-6"	1'-0"	4-#5 EA. WAY BOT.
F4.0	4'-0"	4'-0"	1'-0"	4-#5 EA. WAY BOT.
F4.5	4'-6"	4'-6"	1'-0"	4-#5 EA. WAY BOT.

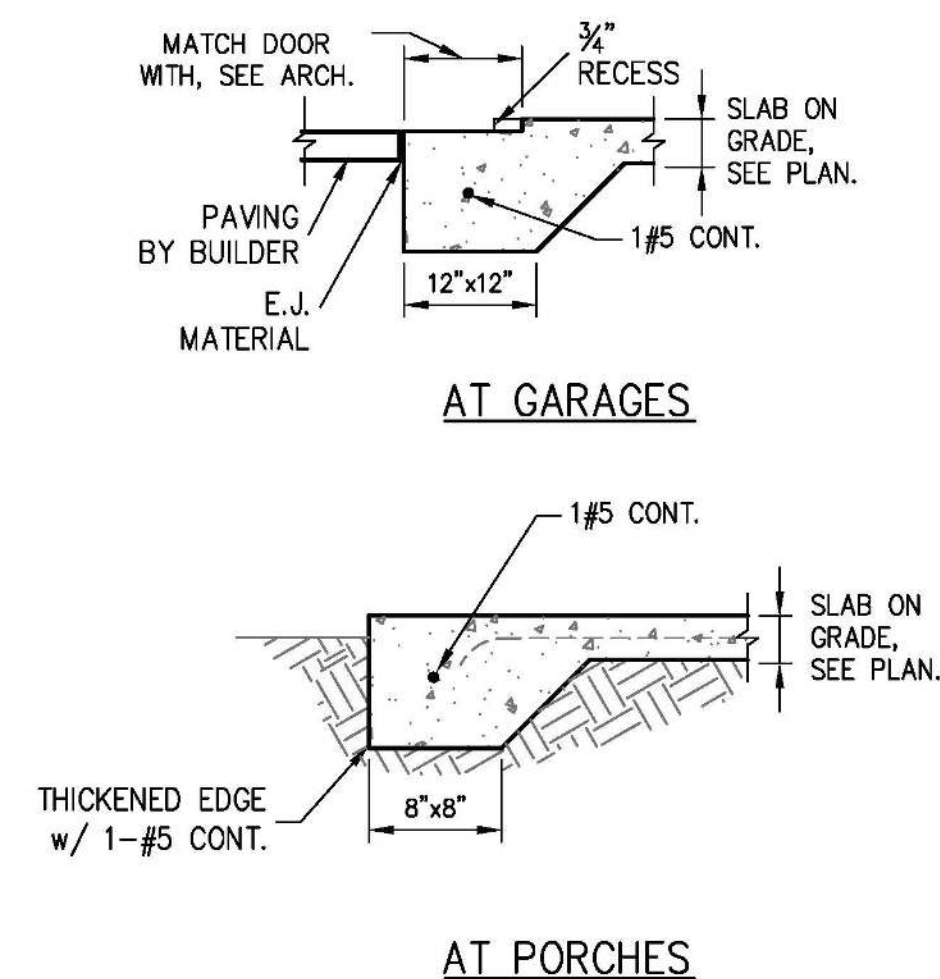
- THIS FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFO. RELATED TO THE FOUNDATION. FOR GENERAL FEATURES, DIMENSIONS, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ECT., SEE ARCH. PLAN. ARCHITECTURAL PLAN SHOWN HERE IN FOR REFERENCE ONLY.
- FTGS. & FND. SHALL BE IN ACCORDANCE w/ LOCAL BUILDING CODES.
- SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.



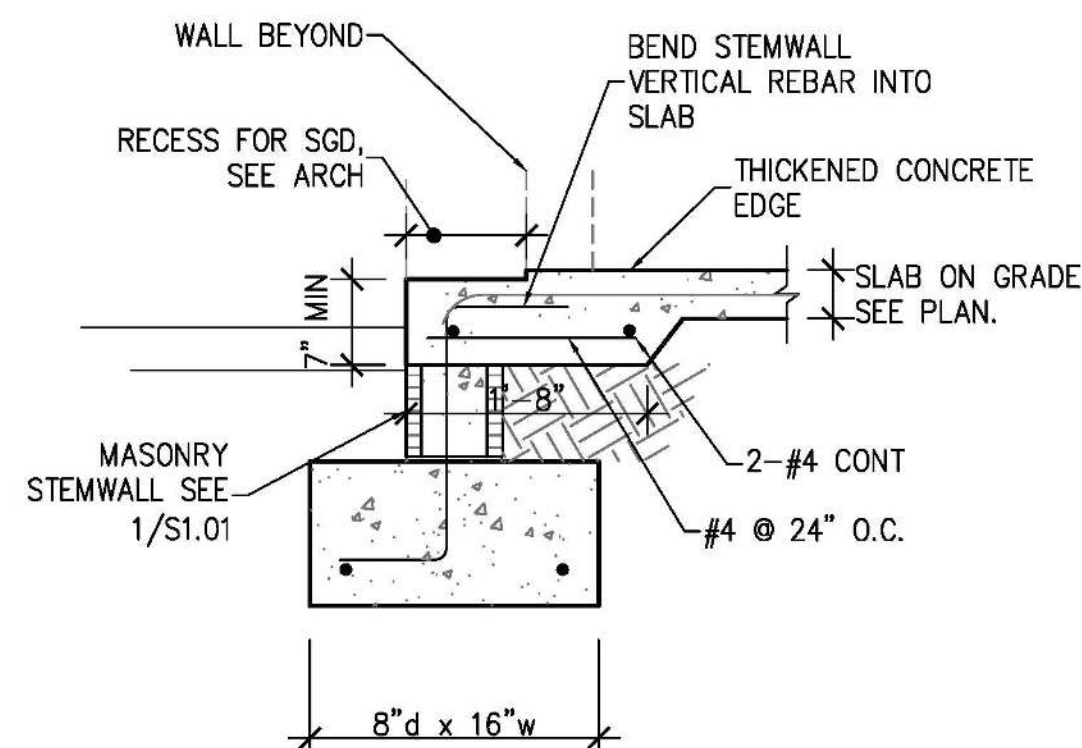
1 STEMWALL FOOTING
S1.01 SCALE: 3/4" = 1'-0"



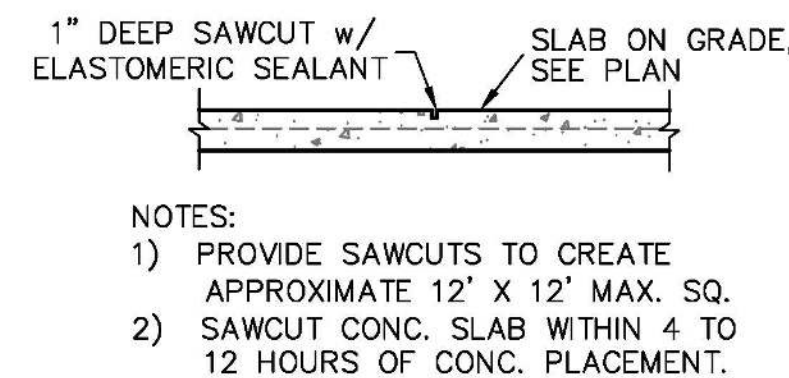
2 FOOTING W/ SHOWER RECESS
S1.01 SCALE: 3/4" = 1'-0"



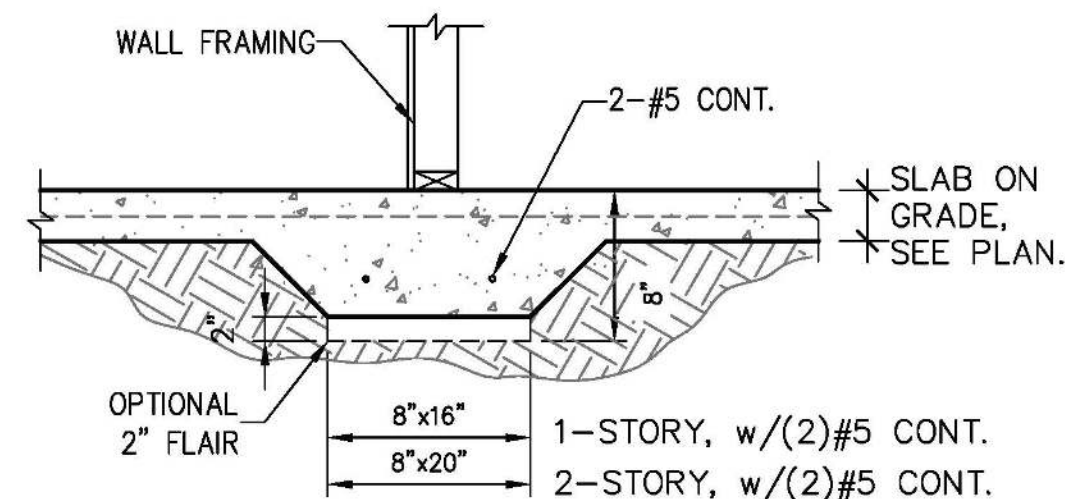
3 THICKENED SLAB
S1.01 SCALE: 3/4" = 1'-0"



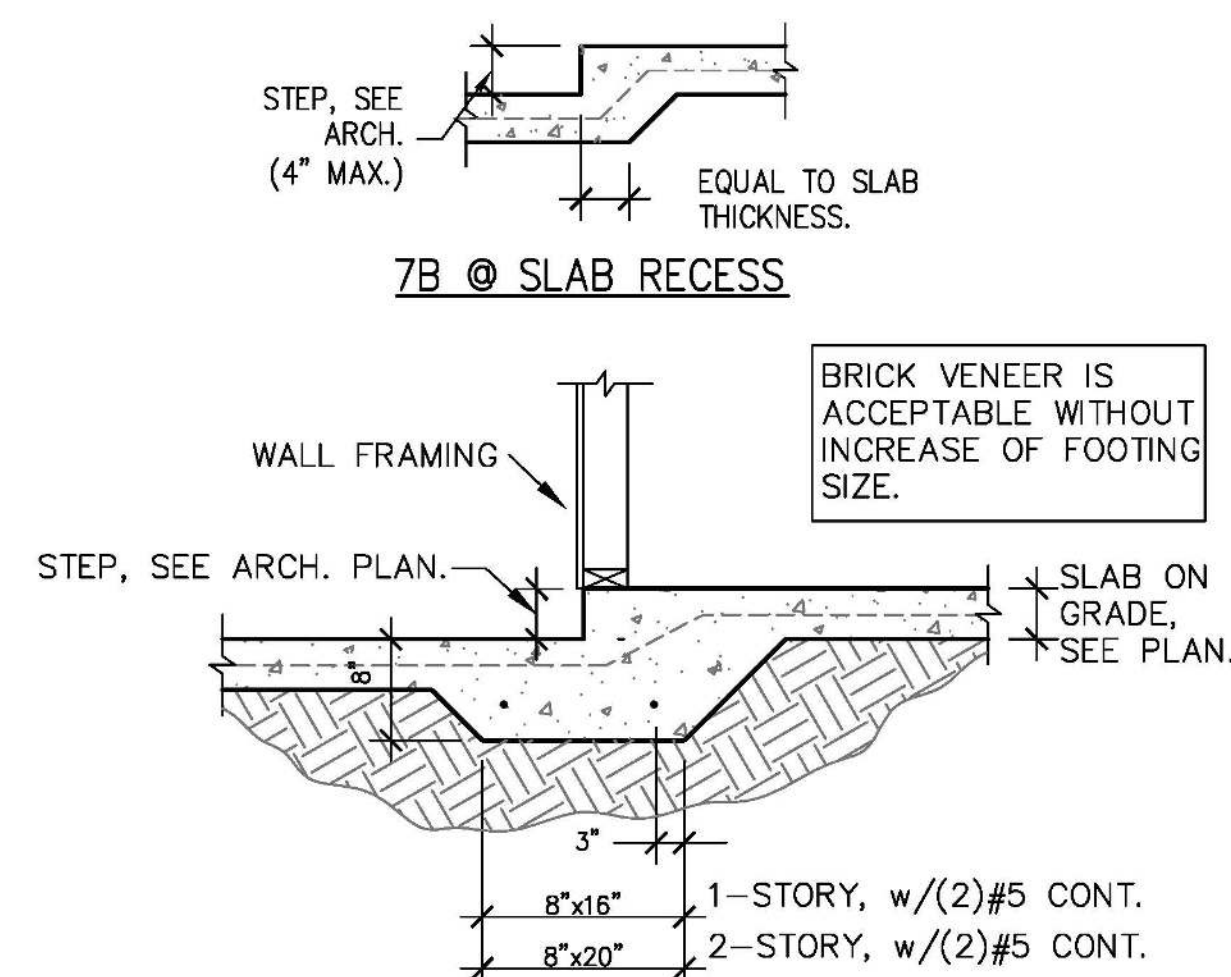
4 STEMWALL FOOTING AT SLIDER
S1.01 SCALE: 3/4" = 1'-0"



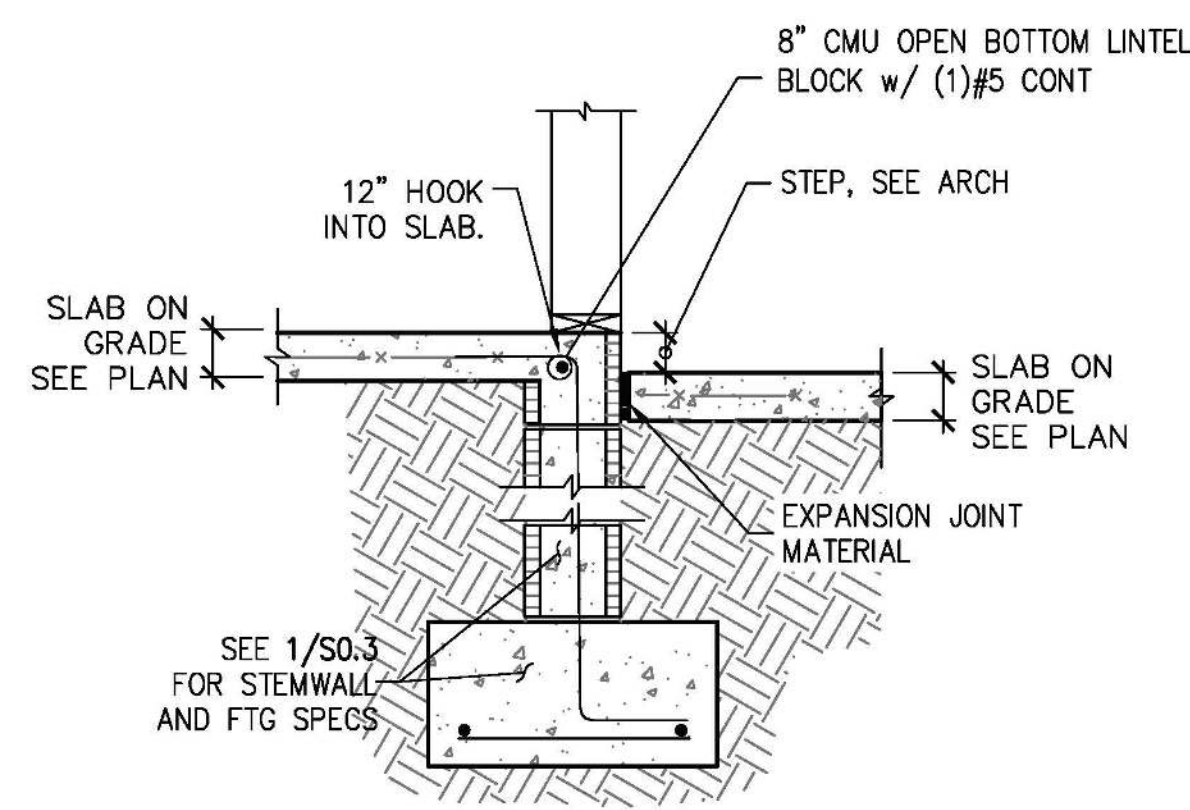
5 SAW CUT DETAIL
S1.01 SCALE: 3/4" = 1'-0"



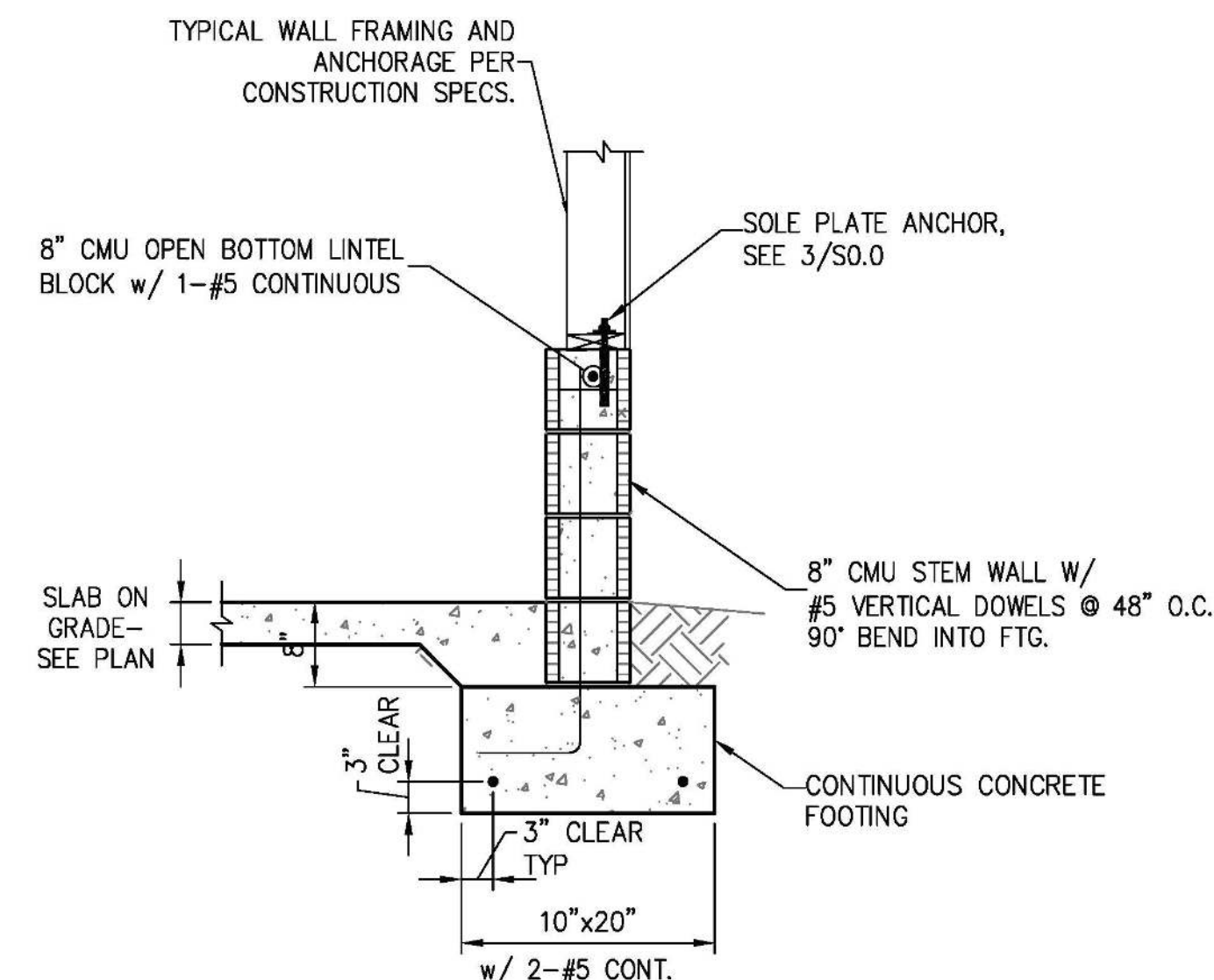
6 BEARING AT INTERIOR
S1.01 SCALE: 3/4" = 1'-0"



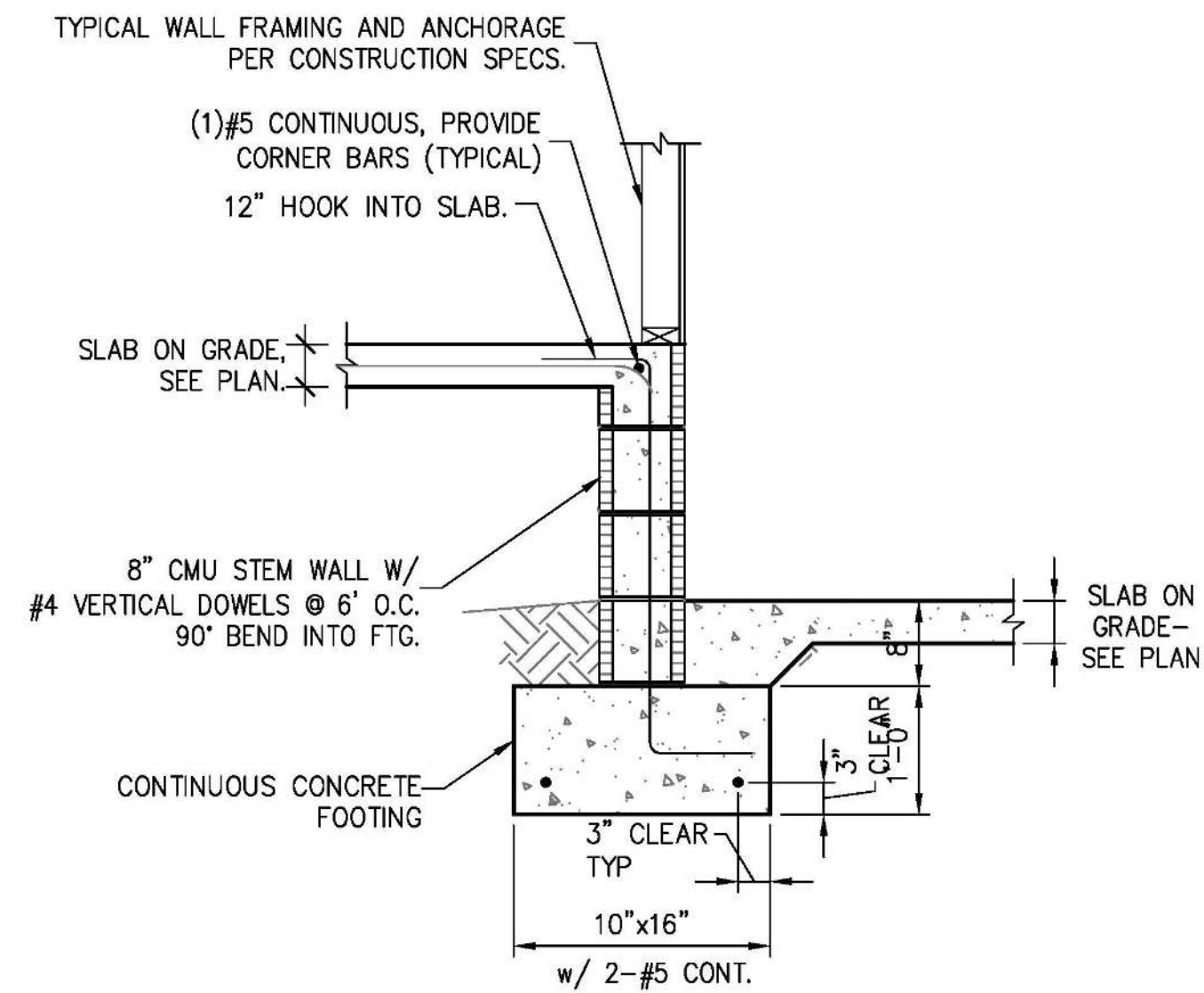
7 MONO. FOOTING AT STEP-DOWN
S1.01 SCALE: 3/4" = 1'-0"



8 STEP AT STEMWALL
S1.01 SCALE: 3/4" = 1'-0"



9 GARAGE STEM WALL
S1.01 SCALE: 3/4" = 1'-0"



10 STEMWALL AT GARAGE
S1.01 SCALE: 3/4" = 1'-0"



06.09.22
Christopher J. Sabourin
FL P.E.#71461

SABO
STRUCTURAL
ENGINEERING
CA#32529
235 9TH AVE N
JAX BEACH, FL 32250
904-712-5750
CHRIS@SABOENG.COM

PLAN NAME
B2EC
SSE No.
22-0249

ISSUE	DATE
PERMIT	06.09.22
REVISIONS	DATE

STRUCTURAL ENGINEERING FOR
THE PARNELL RESIDENCE

FIELD ALTERATION
CONTRACTOR SHALL CONTACT SABO STRUCTURAL ENGINEERING PRIOR TO MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY FROM THE INTENT OF THE ORIGINAL CONSTRUCTION DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR TO BEING APPROVED BY CHRISTOPHER SABOURIN MAY RESULT IN ADDITIONAL ENGINEERING OR INSPECTION FEES.

SCALING
DO NOT SCALE DIMENSIONS FROM THESE DRAWINGS. IF A DIMENSION IS UNCLEAR REFER TO THE ARCHITECTURAL DRAWINGS OR CONTACT THE E.O.R.

FOUNDATION
DETAILS

SHEET
S1.01
SHEET 4 OF 7



06.09.22
Christopher J. Sabourin
FL PE #71461

SABO
STRUCTURAL
ENGINEERING
CA#32529
235 9TH AVE N
JAX BEACH, FL 32250
904-712-5750
CHRIS@SABOENG.COM

PLAN NAME
B2EC
SSE No.
22-0249

ISSUE	DATE
PERMIT	06.09.22
REVISIONS	DATE

STRUCTURAL ENGINEERING FOR
THE PARNELL RESIDENCE

FIELD ALTERATION
CONTRACTOR SHALL CONTACT SABO
STRUCTURAL ENGINEERING PRIOR TO
MAKING ANY STRUCTURAL FIELD
MODIFICATIONS WHICH MAY VARY
FROM THE INTENT OF THE ORIGINAL
CONSTRUCTION DOCUMENTS. ANY
FIELD ALTERATIONS MADE PRIOR TO
BEING APPROVED BY CHRISTOPHER
SABOURIN MAY RESULT IN ADDITIONAL
ENGINEERING OR INSPECTION FEES.

SCALING
DO NOT SCALE DIMENSIONS FROM
THESE DRAWINGS. IF A DIMENSION IS
UNCLEAR REFER TO THE
ARCHITECTURAL DRAWINGS OR
CONTACT THE E.O.R.

ROOF TRUSS
PLACEMENT
PLAN

SHEET
S1.2
SHEET 6 OF 7

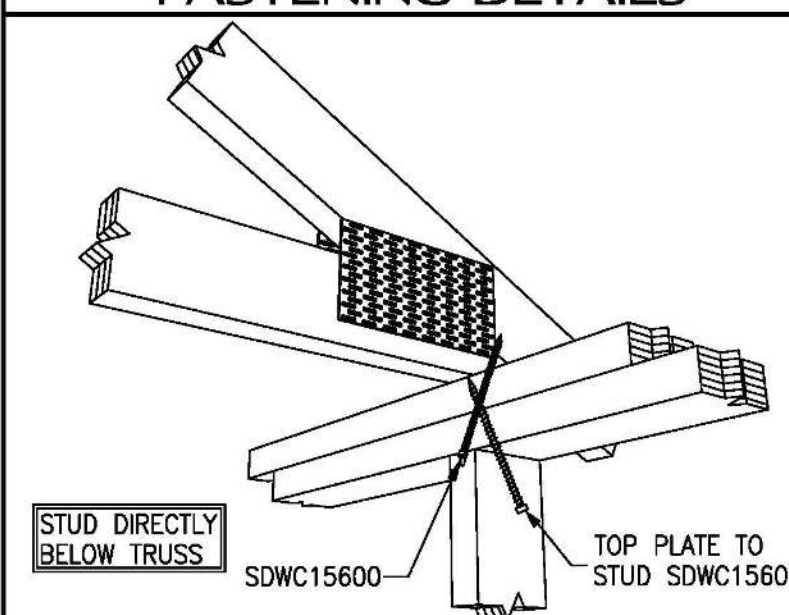
SYMBOLS LEGEND

HTS16 DESIGNATES UPLIFT CONNECTION.

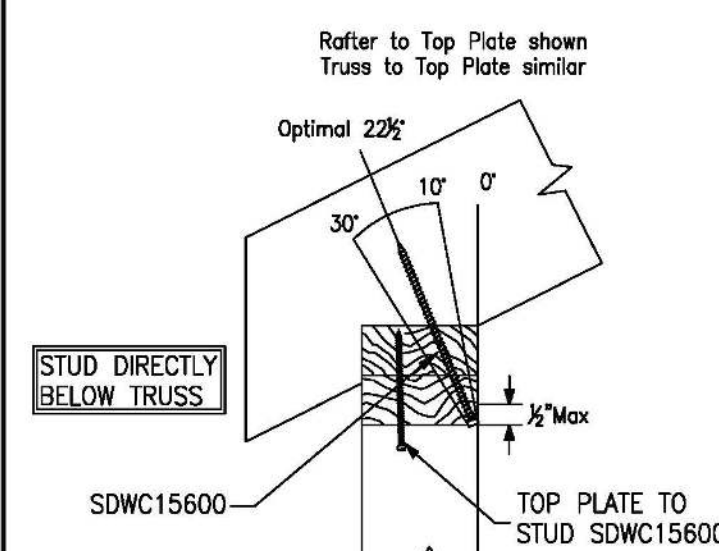
FRAMING PLAN NOTES

- FOR TYPICAL ROOF SHEATHING AND FRAMING, SEE SHEET S0.0.
- FOR SPECIFIC UPLIFT CONNECTORS, SEE PLAN, MIN. (1)SDWC CONNECTOR.
- FOR GENERAL DESIGN SPECIFICATIONS SEE SHEET S0.0.
- WHEN USING (2)H2.5T CLIPS ON 1 1/2" WIDE LUMBER, PLACE CLIPS DIAGONALLY ACROSS DOUBLE TOP PLATE FROM EACH OTHER.

TRUSS FASTENING DETAILS

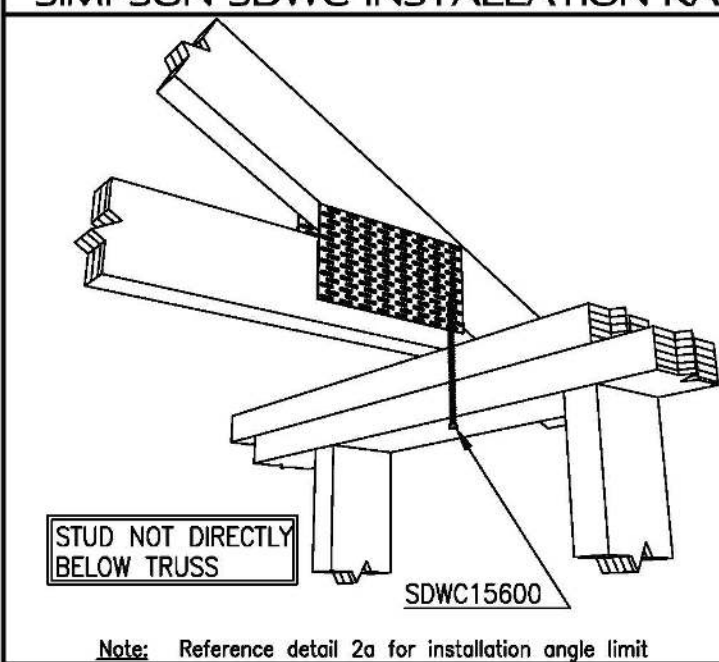


TRUSS TIE DOWN WITH SIMPSON SDWC

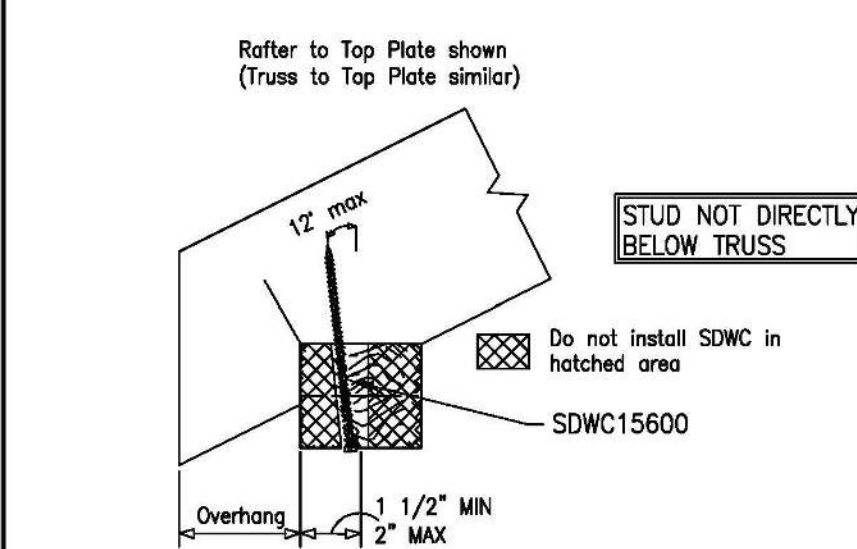


Notes: 1. Sloped-roof rafters may be sloped up to and including a 12:12 pitch and must be "birdsmouth" cut.
2. Reference detail 4 for installation instructions.

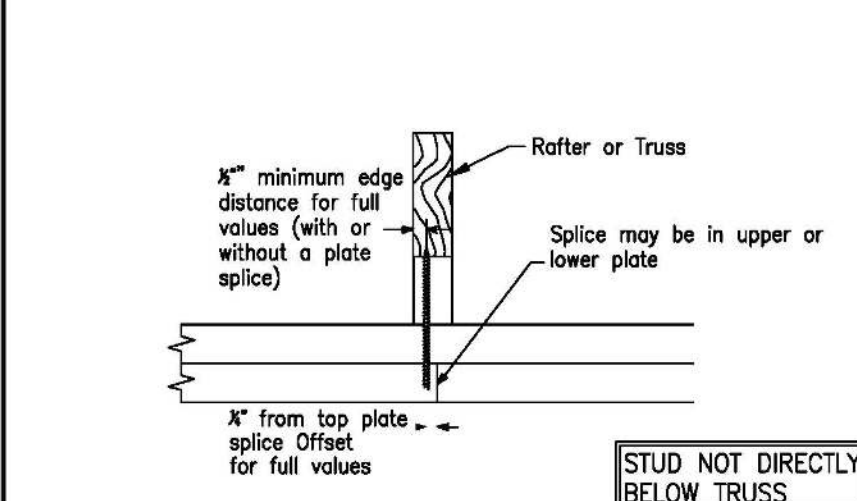
SIMPSON SDWC INSTALLATION RANGE



SDWC INSTALLATION



SDWC INSTALLATION RANGE

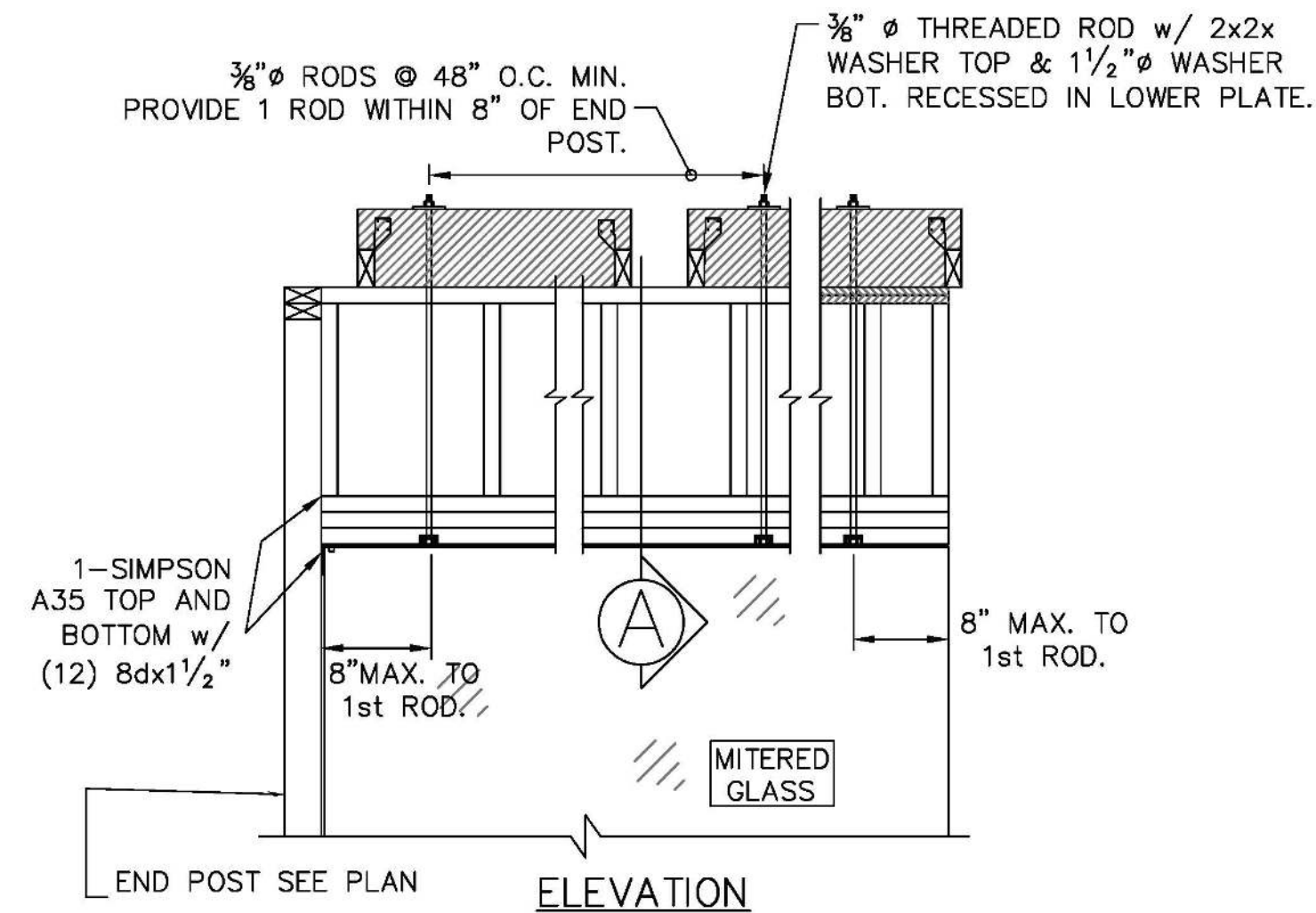
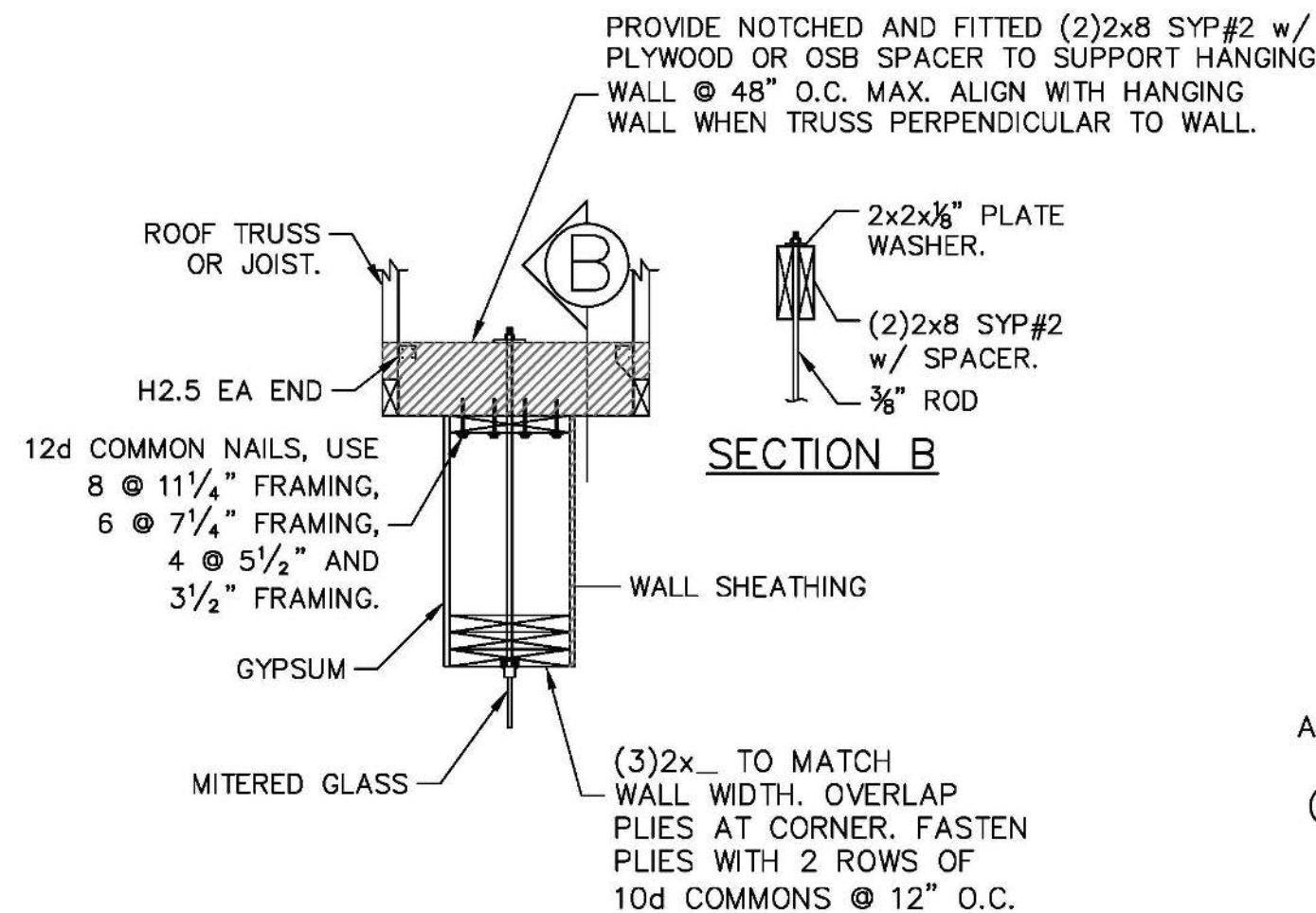
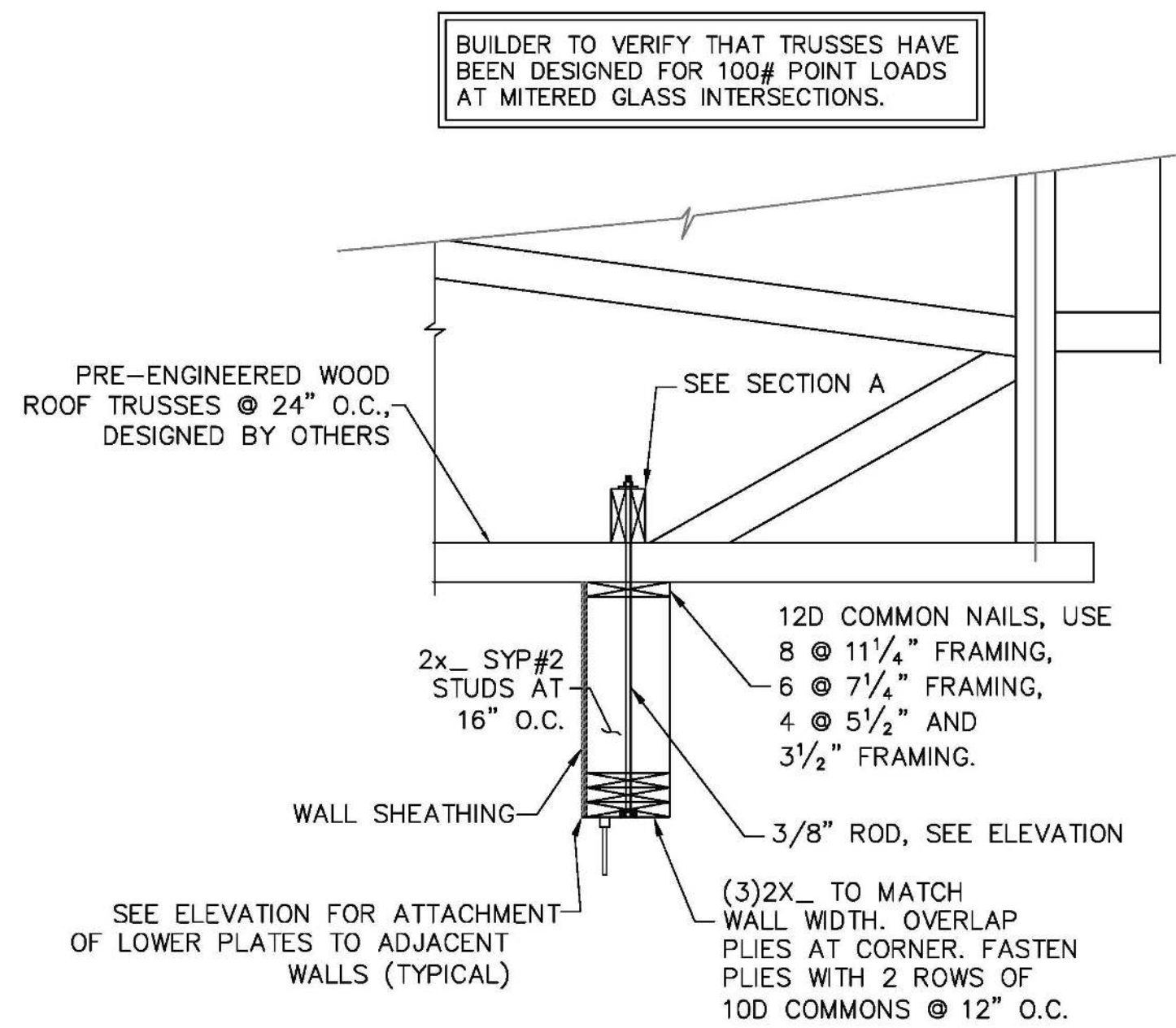


SDWC AT TOP PLATE SPLICE

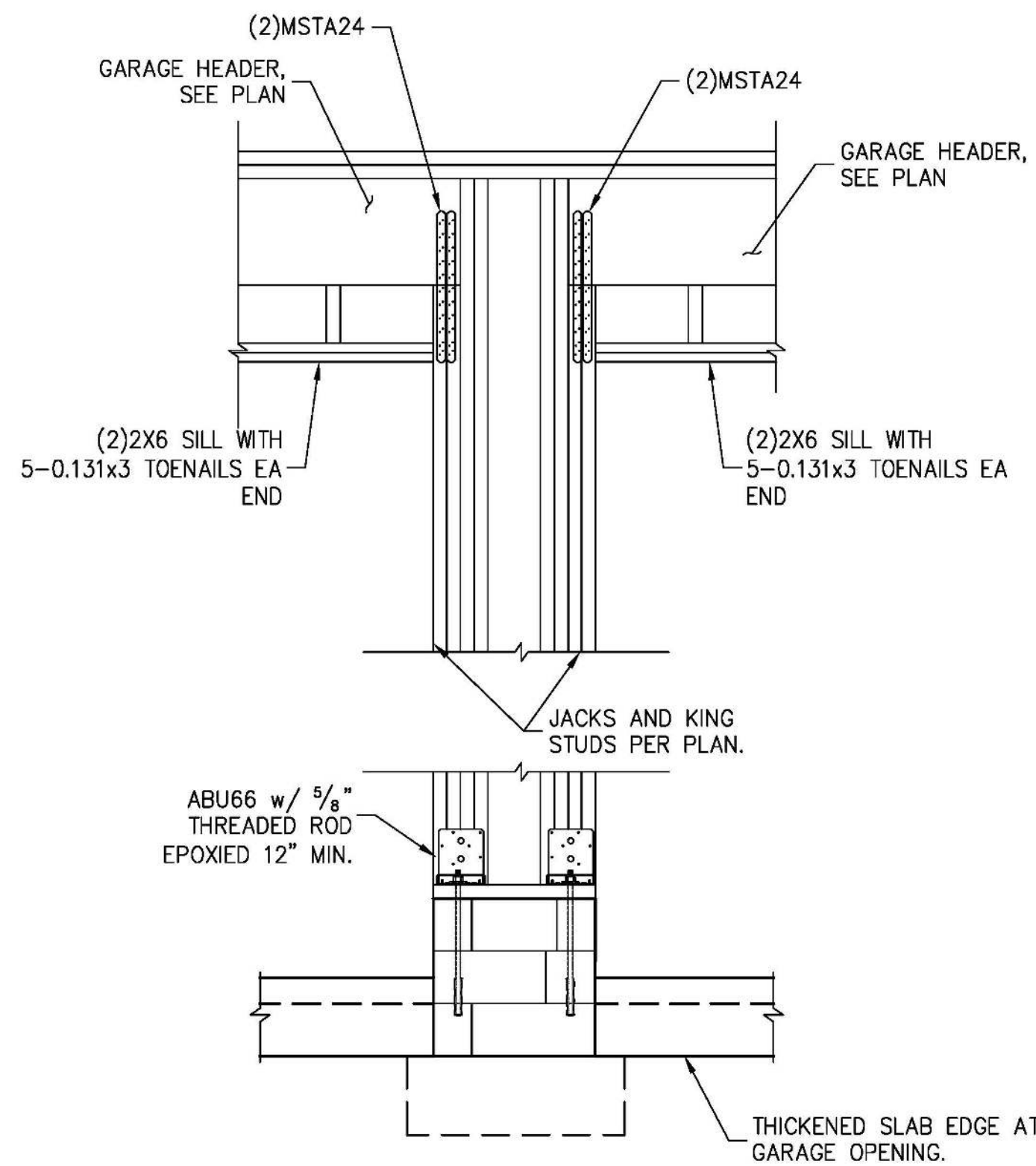
TRUSS / ROOF RAFTER NOTES: STRAPPING NOTES

- STRAP ROOF TRUSSES AND RAFTERS TO BEARING WITH 2-12D TOENAILS & 1-SIMPSON SDWC15600 SCREW UNLESS OTHERWISE NOTED

ROOF TRUSS PLACEMENT PLAN
SCALE: 1/4" = 1'-0"



1 MITERED WINDOW HEAD FRAMING
SCALE: N.T.S.



WHEN NOTED 2 GARAGE CENTER WALL FRAMING
SCALE: 3/4" = 1'-0"



06.09.22
Christopher J. Sabourin
FL PE #71461

SABO
STRUCTURAL
ENGINEERING
CA#32529
235 9TH AVE N
JAX BEACH, FL 32250
904-712-5750
CHRIS@SABOENG.COM

PLAN NAME
B2EC
SSE No.
22-0249

ISSUE	DATE
PERMIT	06.09.22
REVISIONS	DATE

STRUCTURAL ENGINEERING FOR
THE PARNELL RESIDENCE

FIELD ALTERATION
CONTRACTOR SHALL CONTACT SABO STRUCTURAL ENGINEERING PRIOR TO MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY FROM THE INTENT OF THE ORIGINAL CONSTRUCTION DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR TO BEING APPROVED BY CHRISTOPHER SABOURIN MAY RESULT IN ADDITIONAL ENGINEERING OR INSPECTION FEES.

SCALING
DO NOT SCALE DIMENSIONS FROM THESE DRAWINGS. IF A DIMENSION IS UNCLEAR REFER TO THE ARCHITECTURAL DRAWINGS OR CONTACT THE E.O.R.

MISC DETAILS

SHEET
S2.0
SHEET 7 OF 7