

JAMB DEPTH FINISHED FLOOR ELEV. 1/2" 1-1/2" 1 1/2" 1 1/2" F.O. WIDTH PE 2

TYP. O.H. DOOR RECESS DETAIL

GENERAL FOUNDATION NOTES

- 1. ALL REINFORCING STEEL SHALL BE GRADE 60 DEFORMED BARS.
- ALL POURED IN PLACE SHALL BE F'C 3000 PSI MIN @ 28 DAYS.
- WELDED WIRE MESH SHALL MEET ASTM A-185.
- FOUNDATION AND FOOTING SIZING BASED ON
- ASSUMED SOIL BEARING CAPACITYOF 2500 PSF.
 THE UPPER 12" OF BEARING SOIL IN FOOTING SHALL
 BE COMPACTED TO 95% OF THE STANDARD PROCTOR.
 MIN. REINFORCING STEEL COVER AT EARTH: 3"
 SLUMP RANGE AT POINT OF DISCHARGE: 3"-6"

- OVERLAP ALL WWF A MINIMUM OF 8". LAP ALL REINFORCING STEEL A MINIMUM OF 48 DIAMETERS.
- 10. REMOVE TOPSOIL & ORGANIC MATERIAL FROM TOP 12" OF EXISTING GRADE.
- 11. FDN. IS DESIGNED TO MEET THE 2020 ED. OF THE FBC CODE. (IBC 2018) W.S. IS 119 MPH. EXP. B
- 12. BOTTOM OF FTG. SHOULD BE A MIN. OF 12" BELOW GRADE

- 13. PROOF ROLL 5' DUTSIDE OF BUILDING FOOTPRINT WITH VIBRATORY COMPACTOR.
- 14 FILL TO WITHIN 4' OF FINISHED FLOOR ELEVATION
- WITH CLEAN SAND FILL.
 COMPACT TOP 6' OF FILL MATERIAL TO 95% OF
 MODIFIED PROCTOR DENSITY. (MIN)
- SAW INDICATED CRACK CONTROL JOINTS WITHIN 8 HOURS OF PLACEMENT OF CONCRETE.
- 17. SOIL IN FOOTING TRENCHES SHALL BE FREE OF ORGANIC MATERIAL OR CLAY: IF EITHER IS ENCOUNTERED IN FOOTING TRENCHES, REMOVE IT &
- REPLACE WITH COMPACTED SAND.

 18. CONTRACTOR TO REVIEW FOUNDATION DRAWINGS AND CHECK FOR COMPLIANCE WITH ERECTION DRAWINGS BEFORE COMENCMENT OF CONSTRUCTION. ANY DISCREPANCY SHOULD BE BROUGHT TO ENGINEER'S ATTENTION.
- 19. TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMICIDES OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE AS A PREVENTITIVE TREATMENT TO NEW CONSTRUCTION.

REVISIONS					DRAWING STATUS [X] FOR CONSTRUCTION	BUILDI	NGS AND MORE	TOWN HOMES, LLC
REV. DESCRIPT	Ni DATI	E DTLR D	ATE CHKR	APPD	[] FOR PERMIT ONLY	PROJECT	100.0' X 150.0' X 28.0'	FOUNDATION DETAIL PAGE
					[] FOR APPROVAL	ID	8459	DESIGNI CE DRAFTI CE CHECK CEI
					[] OTHER, EXPLAIN	PROJECT	LAKE CITY, FL 32025	DATE: 2/7/24 SHEET FNDVG-2
		1 1		•		ADDRESS	LANE CITY, LE SEVES	

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	February 7, 2024

BUILDING PROFILE

Width (ft) = 100Eave Height (ft) = 28 Length (ft) = 150Roof Slope (Rise/12) = 2.0:12

BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 23 / 8TH EDITION

B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

OCCUPANCY/RISK_CATEGORY	<u>il — Normal</u> <u>le</u> 1.00
WIND LOAD ULTIMATE	119 MPH NOMINAL 92.18 MPH WIND EXPOSURE B
CLOSURE TYPE	Enclosed
INTERNAL WIND COEFFICIENT	_0.18 / 0.18
COLLATERAL DEAD LOAD	PSF
ROOF LIVE LOAD	20.00 PSF (REDUCIBLE Yes)
DEAD_LOAD	2.000 PSF (FOR ROOF PANELS AND PURLINS)
SEISMIC	
SPECTRAL RESPONSE Ss 0.085	53 S1 0.0502 Sds 0.0907 Sd1 0.0800
SITE CLASS <u>d</u>	DESIGN RISK CATEGORY B Cs 0.0302
RESPONSE MODIFICATION FACTOR, R	3.000* FRAMES 3.000* BRACING
	TEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES
	TEM (ENDWALLS) = ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYS	TEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES

= EQUIVALENT LATERAL FORCE PROCEDURE

STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR

	MINIM	NUM DESIG	N DEFLECTIONS		
Endwall Column	-	180	Roof Panel (Live)	=	60
Endwall Rafter (Live)	=	180	Roof Panel (Wind)	=	60
Endwall Rafter (Wind)	=	180	Rigid Frame (Horz)	=	60
Wall Girt	=	90	Rigid Frame (Vert)	=	180
Roof Purlin (Live)	=	180	Rigid Frame (Seismic)	=	50
Roof Purlin (Wind)	-	150			
Wall Panel	=	60			

GENERAL NOTES

SERVICEABILITY CRITERIA

ANALYSIS PROCEDURE

- A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.
- B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.
- 1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: " AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS—ALLOWABLE STRESS DESIGN" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
- 2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
- 3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1. AS ADOPTED BY THE BUIDLING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
- 4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
- C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD-FORMED SECTIONS, CONFORM TO ASTM-A529 OR A572. FLANGES AND WEB MATERIAL CONFORMS TO ASTM-A529 OR A572 GRADE 55 WITH A MINIMUM YIELD POINT OF 55,000 psi.
- 2) MATERIAL PROPERTIES OF HSS ROUND SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM YIELD POINT OF 42,000 psi. 3) MATERIAL PROPERTIES OF HSS RECT. OR SQUARE SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM
- YIELD POINT OF 46,000 psi. 4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992
- WITHMINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992WITH MINIMUM YIELD POINT OF 50,000 PSI. 5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR
- A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.

 6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY IN ACCORDANCE WITH A755 SPECIFICATIONS.
- 7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT
- 8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.

 9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY
- FRAMING MUST BE "SNUG-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A-325 BOLTS IF:
- BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC—BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F". b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
- c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS REVERSALS ON THE CONNECTIONS
- d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 SC".

- 10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UND.
- 11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36.
- ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307.

 D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.
- E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

APPROVAL NOTES

- THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:
- A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:
- 1) BE MADE IN CONTRASTING INK.
- 2) HAVE_ALL INSTANCES OF CHANGE_CLEARLY INDICATED.
- 3) BE LEGIBLE AND UNAMBIGUOUS.
- B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.
- C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.
- D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED
- E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION, MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

SAFETY COMMITMENT

- THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.
- IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.
- LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY.
- MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.
- E) DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED

ERECTOR / CONTRACTOR RESPONSIBILITIES

- A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREPMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.
- B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS
- C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, LATEST ED.)
- D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE LATEST ED.)
 E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING
- MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACT-URER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.
- F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.
- G) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR IMMEDIATELY UPON ARRIVAL CLAMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NA THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNIESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER, THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.
- IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.
- THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.
- H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

 1) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED.
- FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION. (SECT, 7,10,3 AISC CODE OF STANDARD PRACTICE, LATEST ED.)
- J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION. IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)
- K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

COMPONENTS & CLADDING (unfactored) Wall Field Values = 22,630 psf / -24.516 psf Wall Edge Values = 22.630 psf / -30.174 psf



BUILDINGS AND MORE

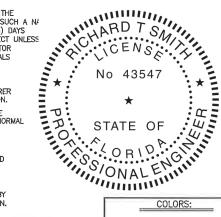
FLORIDA PRODUCT APPROVAL NUMBER PBR ROOF PANEL PBR WALL PANEL 36876.1

T IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A
DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA.
THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

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U SECTION:	RO	RO	RO	RO	RO	RO	RO	
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D SECTION:	RO	RO	RO	RO	RO	RO	RO	
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R SECTION:	RO	RO	RO	RO	RO	RO	RO	
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WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED.



DRAWING INDEX PAGE REV. DESCRIPTION COVER PAGE ANCHOR BOLT LAYOUT 1.1 ANCHOR BOLT DETAILS 1.2 ANCHOR BOLT REACTIONS ROOF FRAMING LAYOUT RIGID FRAME CROSS SECTION 2.1-2.5 SIDEWALL FRAMING LAYOUT ENDWALL FRAMING LAYOUT 5-5.3 FRAMING DETAILS ROOF PANELS & TRIM 6.1 ROOF PANEL DETAILS SIDEWALL PANELS & TRIM SIDEWALL PANEL DETAILS ENDWALL PANELS & TRIM ENDWALL PANEL DETAILS SPECIAL DETAILS



This item has been digitally signed and sealed by Richard T Smith on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic

	COLORS:	copies.
ROOF:	GALVALUME	DRAWING STATUS
WALLS:	COLOR	FOR APPROVAL:
GABLE:	COLOR	THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL
EAVE:	COLOR	REPRESENTATION ONLY. THEIR PURPOSE IS TO
CORNER:	COLOR	CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCT CAN BE CONSIDERED AS COMPLETE.
FRAMED OPENINGS	S: COLOR	FOR PERMIT:
CUTTER	COLOR	THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE, MARKINGS AF NOT IDENTIFIED. ONLY DRAWINGS ISSUED FOR
DOWNSPOUTS:	COLOR	CONSTRUCTION " CAN BE CONSIDERED AS COMPLETE.
BASE:	COLOR	FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION

DR LC. NEWELL 닙 HOMES 능 SE TOWN LAKE 133

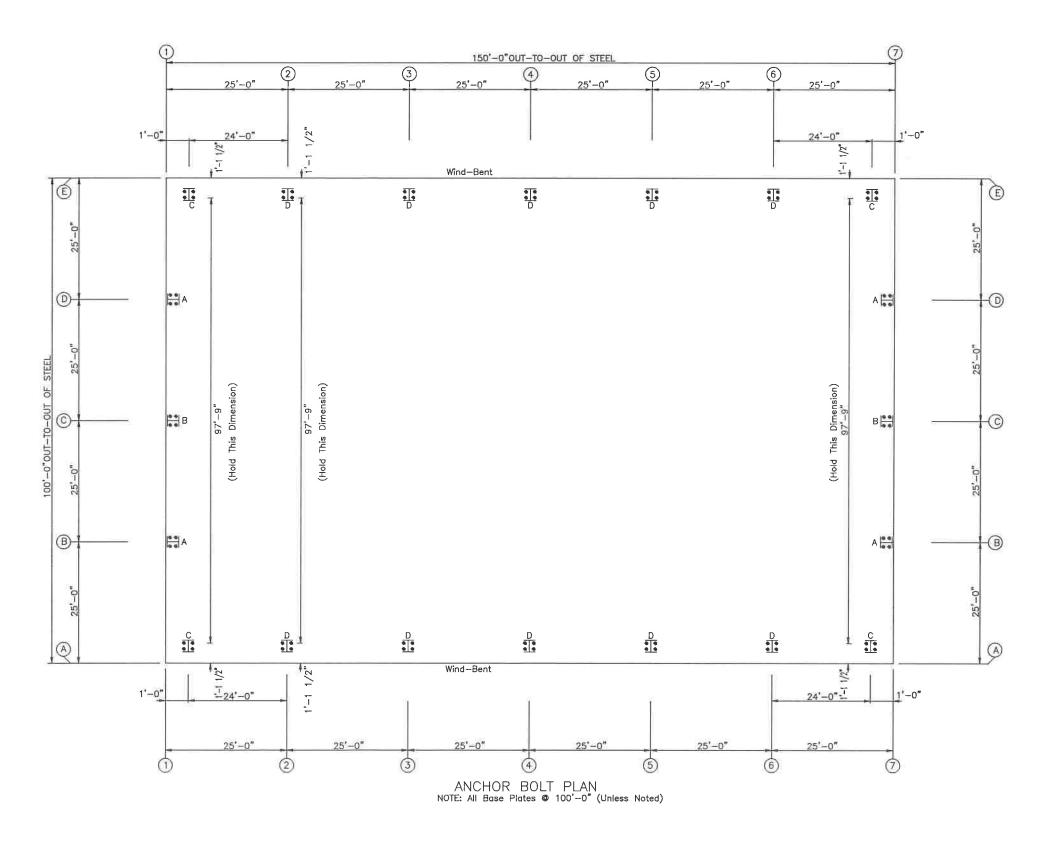
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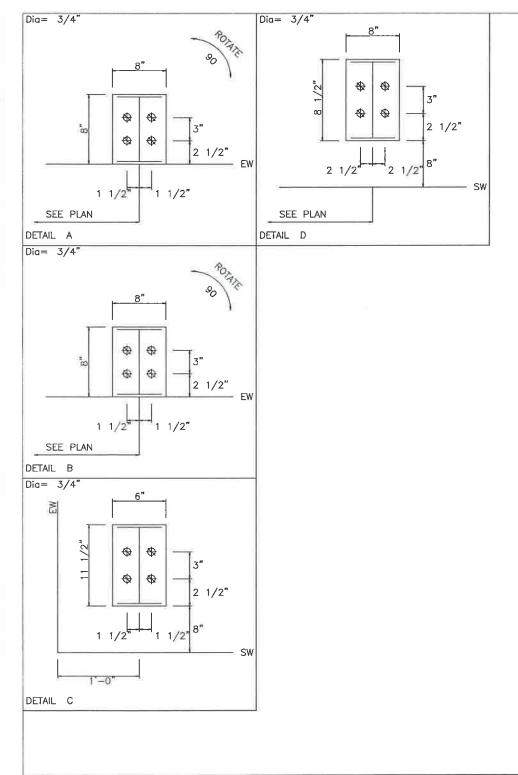
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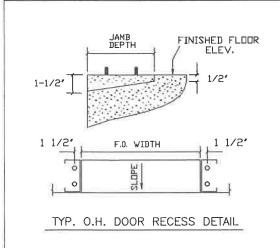
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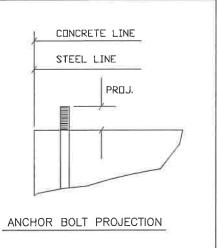
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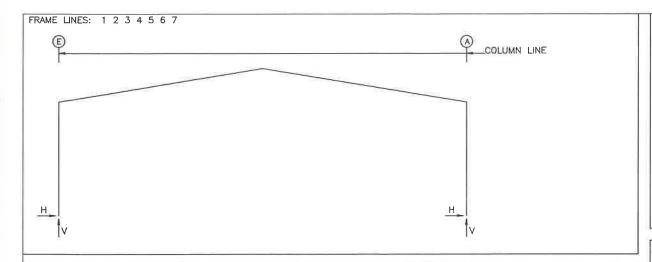
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ANCHOR BOLT	LAYOUT					
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RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Col Anc._Bolt Base_Plate (in) Grout Line Line Qty Dia Width Length Thick (in) 4 0.750 6.000 11.50 0.375 0.0 4 0.750 6.000 11.50 0.375 0.0

1* Frame lines: 1 7

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Col Anc._Bolt Base_Plate (in) Grout Width Length Thick (in) 4 0.750 8.000 8.500 0.500 0.0 4 0.750 8.000 8.500 0.500 0.0

2* Frame lines: 2 5 6

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Col Anc._Bolt Base_Plate (in) Grout Line Line Qty Dia Width Length Thick (in) 4 0.750 8.000 8.500 0.625 0.0 4 0.750 8.000 8.500 0.625 0.0

3* Frame lines: 3 4

	END	WALL	COI	_UMN:	ANCHOR BOLTS & BASE PLATES					
	Frm Line	Col Line	AncBolt Qty Dia		Base_ Width	_Plate (i Length	n) Thick	Grout (in)		
l	1	D	4	0.750	8.000	8.000	0.250	0.0		
I	1	С	4	0.750	8.000	8.000	0.250	0.0		
I	1	В	4	0.750	8.000	8.000	0.250	0.0		
I	7	В	4	0.750	8.000	8.000	0.250	0.0		
I	7	C	4	0.750	8.000	8.000	0.250	0.0		
	7	D	4	0.750	8.000	8.000	0.250	0.0		
İ										

ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Туре	Projection (in)
\$	24	Endwall	3/4"	GR36	1.50
	56	Frame	3/4"	GR36	2.50

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

T														
RIG	GID FRAI	ME: ı	BASIC COL	UMN REA	CTIONS (k)								
Frailine 1* 1* 2* 2* 3* 3*		Horiz 1.3 -1.3 2.2 -2.2 2.3 -2.3	-Dead Vert 3.0 3.0 4.5 4.5 4.6 4.6	Collo Horiz 0.4 0.4 0.8 0.8 0.8 0.8	teral- Vert 0.7 0.7 1.3 1.3 1.3	Horiz 4.5 -4.5 9.2 -9.2 9.4 -9.4	-Live Vert 7.8 7.8 15.0 15.0 15.0	Wind Horiz -11.3 3.5 -16.9 3.9 -17.0 4.1	Left1- Vert -16.4 -11.1 -23.0 -15.8 -23.0 -15.8	-Wind_Horiz -3.5 11.3 -3.9 16.9 -4.1 17.0	Right1- Vert -11.1 -16.4 -15.8 -23.0 -15.8 -23.0	Wind Horiz -10.0 2.2 -14.7 1.8 -14.8 2.0	_Left2- Vert -11.4 -6.1 -13.3 -6.1 -13.3 -6.1	
Frailine 1* 1* 2* 2* 3* 3*		-Wind_ Horiz -2.2 10.0 -1.8 14.7 -2.0 14.8	Right2- Vert -6.1 -11.4 -6.1 -13.3 -6.1 -13.3	Wind Horiz -2.6 3.3 -5.5 6.9 -5.7 7.1	_Long1 - Vert -10.8 -9.0 -20.7 -17.3 -20.7 -17.3	Wind Horiz -3.3 2.6 -6.9 5.5 -7.1 5.7	Long2- Vert -9.0 -10.8 -17.3 -20.7 -17.3 -20.7	-Seism Horiz -0.2 -0.2 -0.2 -0.2 -0.2	ic_Left Vert -0.1 0.1 -0.1 0.1 -0.1 0.1	Seismic. Horiz 0.2 0.2 0.2 0.2 0.2 0.2	_Right Vert 0.1 -0.1 0.1 -0.1 -0.1			
1* 2* 3*	Frame lii Frame lii Frame lii	nes:	1 7 2 5 6 3 4											

END	WALL	COL	UMN:	BASIC	COLUMN	REACTIONS	(k)
Frm Line 1 1 1 7 7	Col Line D C B B C	Dead Vert 0.6 1.1 0.6 0.6 1.1	Wind Press Horz -7.1 -8.1 -7.1 -8.1 -7.1	Wind Suct Horz 7.9 8.9 7.9 8.9 7.9	Seis Long Vert 0.0 0.0 0.0 0.0			

NOTES FOR REACTIONS	6
Building reactions are based the following building data:	
Width (ft) Length (ft) Eave Height (ft) Roof Slope (Rise/12)	= 100.0 = 150.0 = 28.0/ 28.0 = 20/ 20
Dead Load (psf)	= 2.0/2.0 = 2.0 = 1.0 = 20.0
Frame Live Load(psf) Wind Speed (mph) Wind Code	= 12.0 = 119.0 = FBC 23 (8TH EDITION)
Exposure Closure Importance Wind	= B = Enclosed = 1.00
Importance Seismic Seismic Zone Seismic Coeff (Fa*Ss)	= 1.00 = B = 0.14

BUILDING	BRACING	REACTIONS

	Col Line	——W		—Sei	smic –	Panel_S (lb/ft Wind S	.)
L_EW 1 F_SW A	3,4	6.5	13.5	0.7	1.4		(h) (b)
R_EW 7 B_SW E	3,4	6.5	13.5	0.7	1.4		{p}

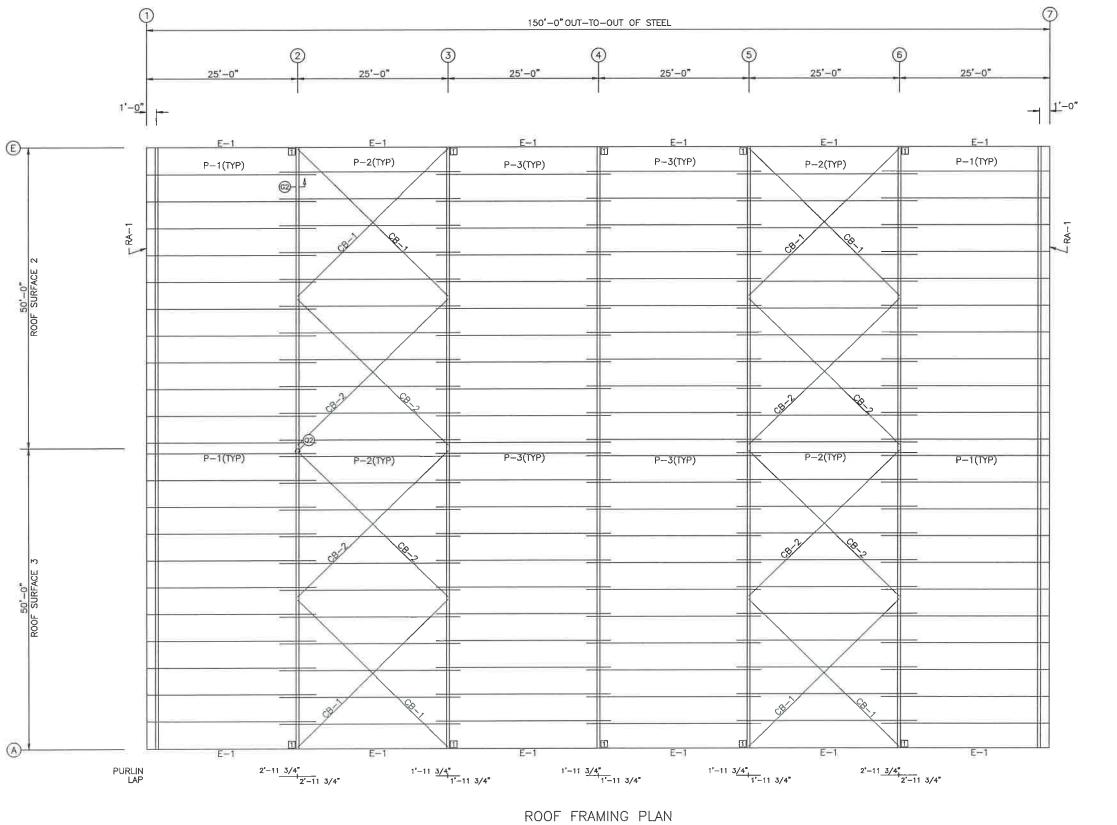
(b)Wind bent in bay, base above finish floor (h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh

DATE:	/ 6/:	24
	DATE:	DATE: 2/ 6/.

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

0,00		4.	0/201
LOCATION:			
LAKE CITY, FL	32025		
DRAWING NAME:			
ANCHOR BOLT	REACTIONS		
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 1.2	CTW	SPW	NONE



CONNECTION PLATES
ROOF PLAN
DID MARK/PART
1 ES-1

ISSUE		DET	CHK	DATE
UILDINGS AND M	ORE			
TOWN HOMES LLC	¥:			
5 NO:		DATE	17	0
8459		2	/ 6/	24
CATION:				
LAKE CITY, FL 320	025			
AWING NAME:				
ROOF FRAMING LA	YOUT			
AWING NO:	DRAWN BY:	CHECKED BY	; SC	ALE:
PAGE 2	CTW	SPW	_	NONE

MEMBER TABLE SPLICE BOLT TABLE Outside Flange W x Thk x Length 6 x 1/2" x 18"-5 3/4" 6 x 1/2" x 2"-0" 6 x 3/8" x 6"-8 13/16" 6 x 1/4" x 3'-0 15/16" 6 x 1/4" x 20'-0" 6 x 1/4" x 7"-4 13/16"
 Web_Depth
 Web_PLATE

 Start/End
 THICK
 Length

 10.0/17.2
 0.135
 9'-11 11/16"

 17.2/28.0
 0.135
 14'-11"

 28.0/28.0
 0.188
 2'-8 5/8"
 Inside Flange W x Thk x Length 6 x 5/8" x 18'-6 1/8" 6 x 5/8" x 2'-0" 6 x 1/2" x 4'-5 1/16" Qty Top Bot Int TYPE DIA Length Weight 967 MARK MARK RF1-1 4 4 2 A325 3/4" 2 1/2" 4 4 0 A325 5/8" 2" 28.0/12.1 12.1/ 9.5 9.5/ 9.5 9.5/14.4 14.4/16.0 0.135 14'-11" 0.135 2'-5 1/8" 0.135 10'-5 7/16" 0.135 14'-11" 0.135 5'-1" 6 x 5/16" x 17'-4 15/16" 6 x 1/4" x 10'-5 7/16" BASE PLATE TABLE RF1-2 COL PLATE SIZE
MARK Width THICK Length
BP-1 6" 3/8" 11 1/2" 6 x 1/4" x 20'-0" 6 x 1/4" x 19'-9 3/8" RF1-3 FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in)
A - L2x2x14 D/F CAP6 9 11/16" 9 11/16" 1'-0" 11'-0" 27'-7"
26 GA. PBR, GALVALUME 11 @ 4'-6 3/16" <u>______2"</u> FB230A(1) FB3B0A(1) FB350A(1) 25'-3 3/16" CLEARANCE 25'-3 3/16" CLEARANCE 93'-10" CLEARANCE E 100'-0" OUT-TO-OUT OF STEEL RIGID FRAME ELEVATION: FRAME LINE 1 7 DET CHK DATE ISSUE NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION. BUILDINGS AND MORE 6 TOWN HOMES LLC. DATE: 2/ 6/24 8459

LAKE CITY, FL 32025

PAGE 2.1

RIGID FRAME CROSS SECTION

DRAWN BY

CHECKED BY: SCALE: NONE

 SPLICE BOLTS

 Splice Mark
 Quan Top/ Bot Type
 Dia Length

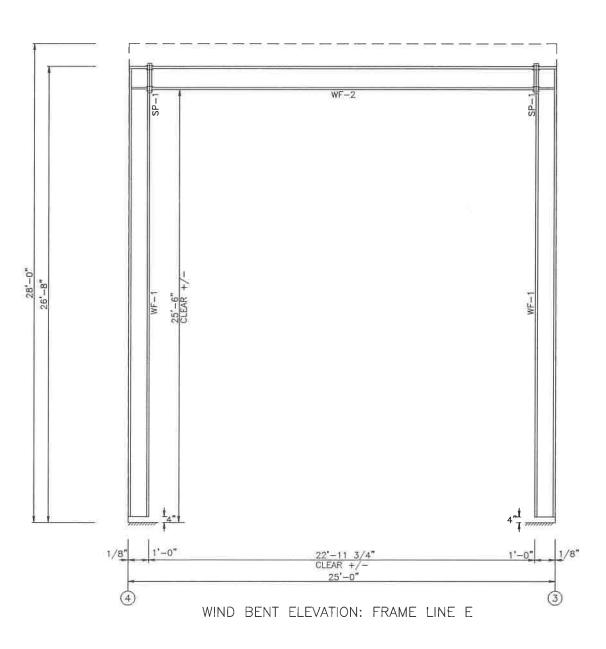
 SP- 1
 4
 4
 A325
 7/8" 3"

MEMBER SIZE TABLE

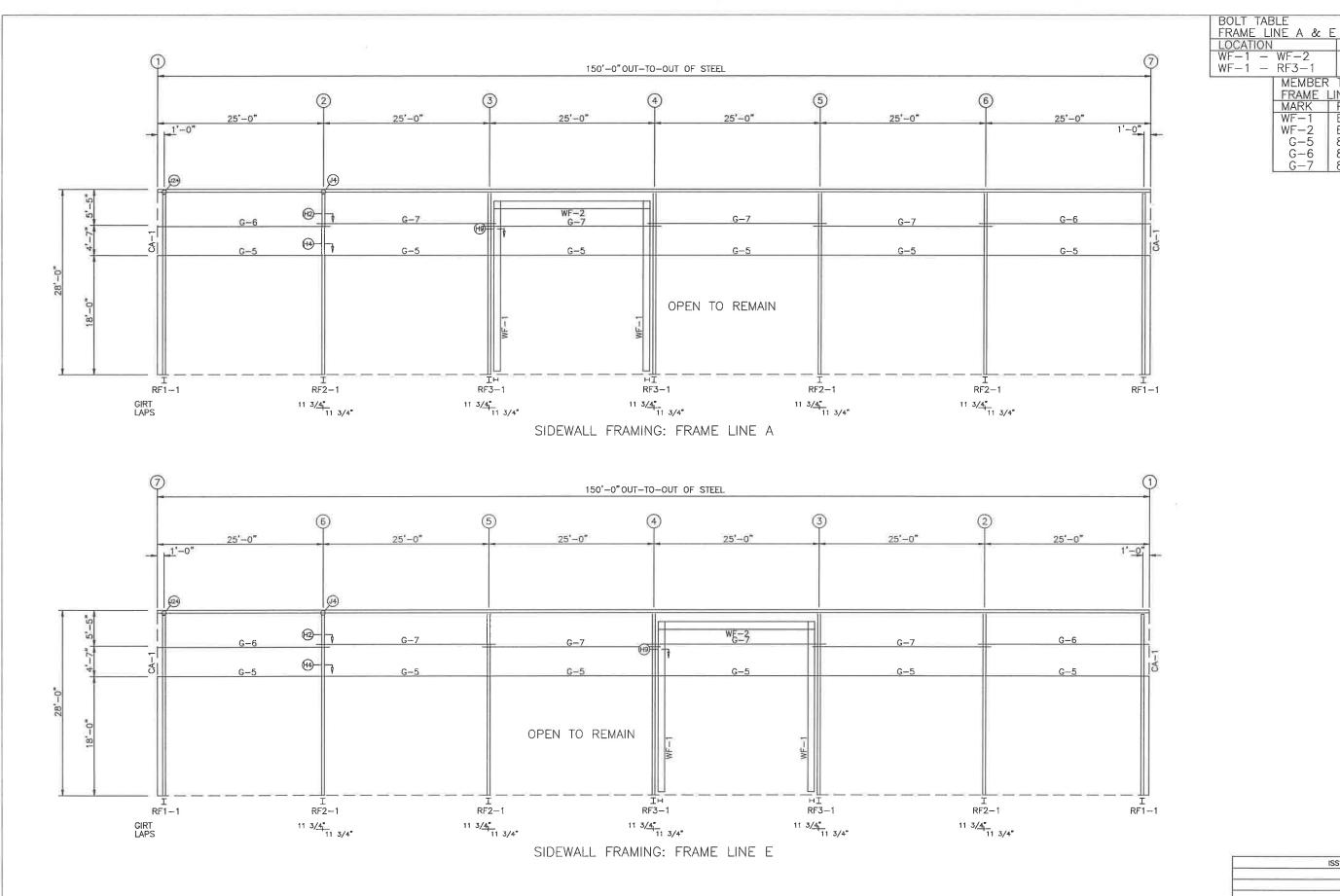
MARK MEMBER LENGTH

WF-2 B14563 22'-11"

WF-1 B12063 26'-4"



	ISSUE	DET	CHK	DATE
BUILDINGS AND	MORE			
CUSTOMER: TOWN HOMES	LLC.			
лов мо: 8459		DATE:	/ 6/	′24
LAKE CITY, FL	32025			
RIGID FRAME	BOVERN.	N		
DRAWING NO: PAGE 2.5	DRAWN BY: CTW	SPW	r: SC	NONE



ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING

REINFORCEMENT.

ISSL	JE	DET	CHK	DATE
BUILDINGS AND N	MORE			
USTOMER: TOWN HOMES LL	C.			
8459		DATE:	/ 6/	24
LAKE CITY, FL 3.	2025			
SIDEWALL FRAMIN				
RAWING NO: PAGE 3	CTW	SPW SPW	se se	NONE

 QUAN
 TYPE
 DIA
 LENGTH

 8
 A325
 7/8"
 3"

 4
 A325
 5/8"
 2"

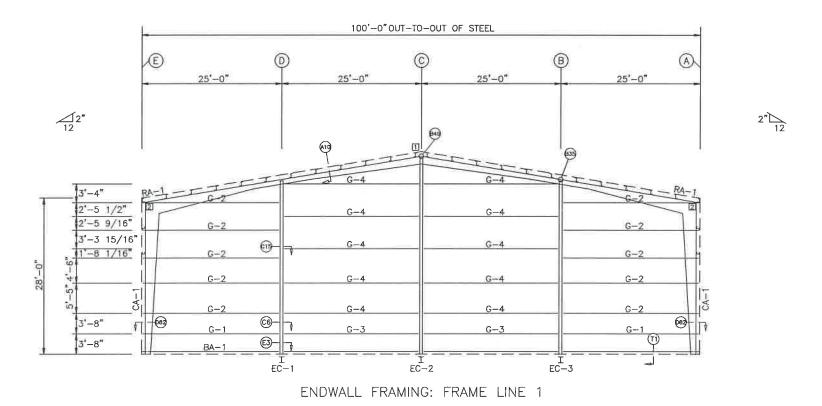
LENGTH
26'-4"
22'-11"
24'-11 1/2"
25'-11 1/2"
26'-11 1/2"

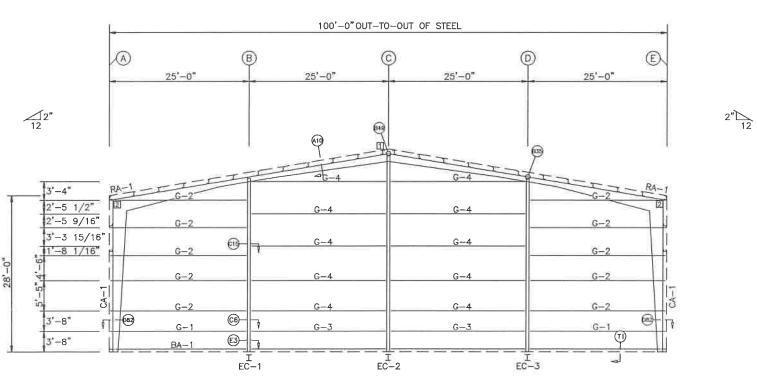
MEMBER TABLE FRAME LINE A & E MARK | PART

WF-1

WF-2 G-5 G-6 G-7

B12063 B14663 8x25C16 8x25Z16 8x25Z16





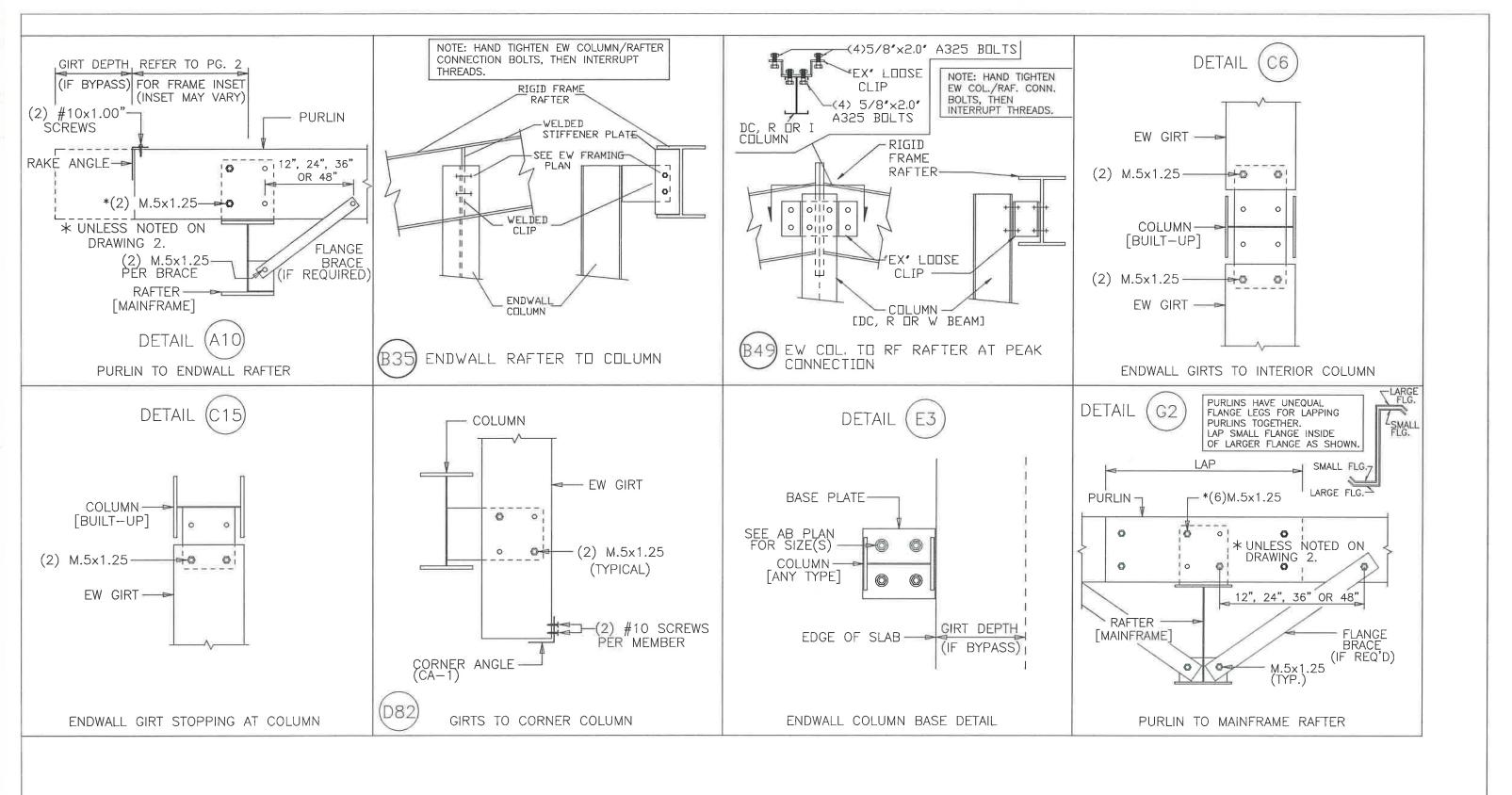
ENDWALL FRAMING: FRAME LINE	_INE 7	/
-----------------------------	--------	---

BOLT TABLE FRAME LINE 1 & 7				
LOCATION	QUAN	TYPE	DIA	LENGTH
EC-1/FRAME	2	A325	5/8"	2"
EC-2/FRAME	10	A325	5/8"	2"
EC-3/FRAME	2	A325	5/8"	2"

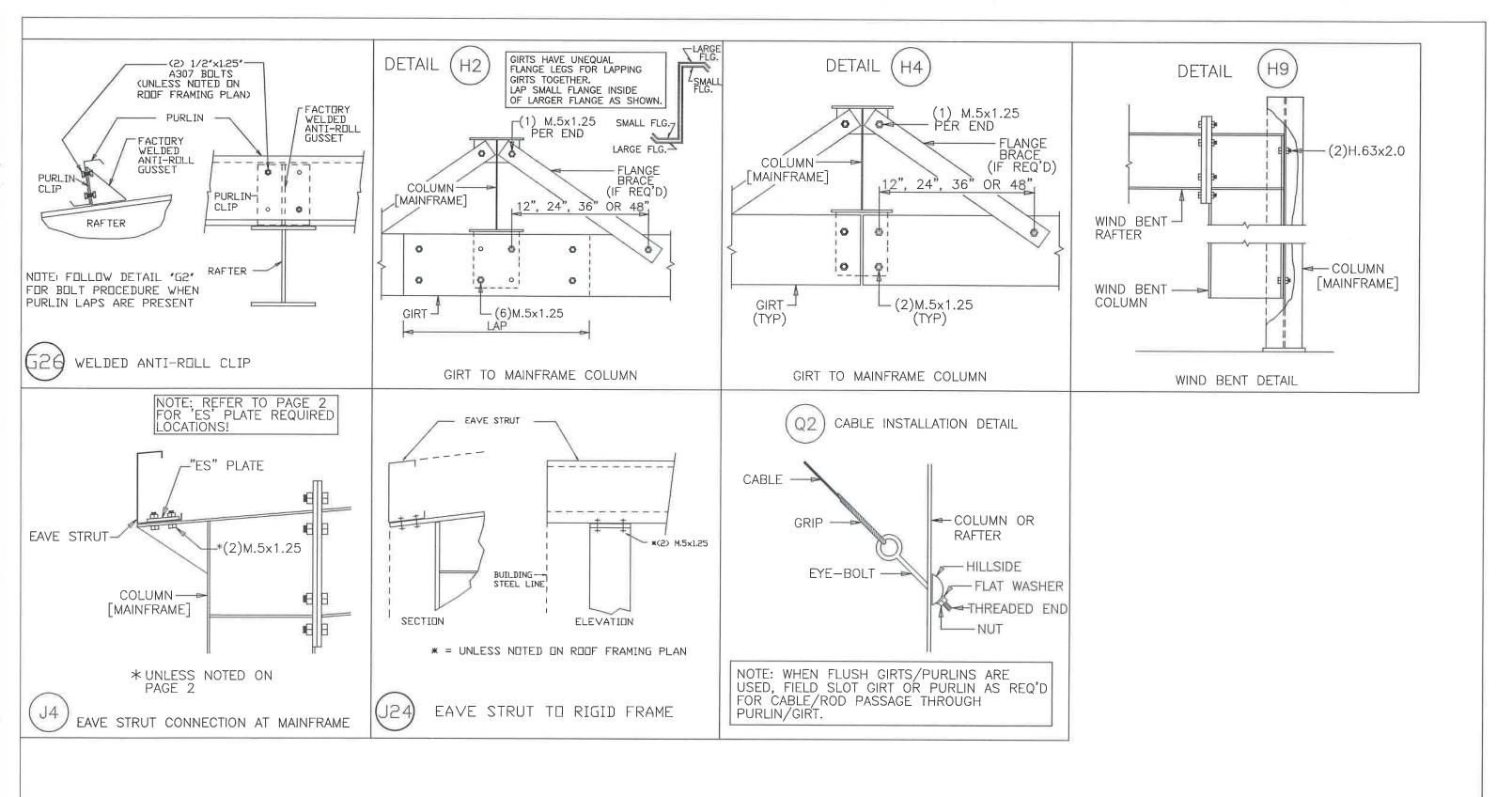
Ta:		
MEMBER	TABLE	
FRAME	LINE 1 & 7	
MARK	PART	LENGTH
EC-1	B08663	31'-2 5/16"
EC-2	W8X31	35'-4 5/16"
EC-3	B08663	31'-2 5/16"
G-1	8x25Z14	24'-7 1'/2"
G-2	8x25Z12	24'-7 1/2"
G-3	8x25Z14	24'-3 1/2"
G-4	8x25Z12	24'-3 1/2"

CONNECTION PLATES
FRAME LINE 1 & 7
DID MARK/PART
1 EX-1
2 SGC-1

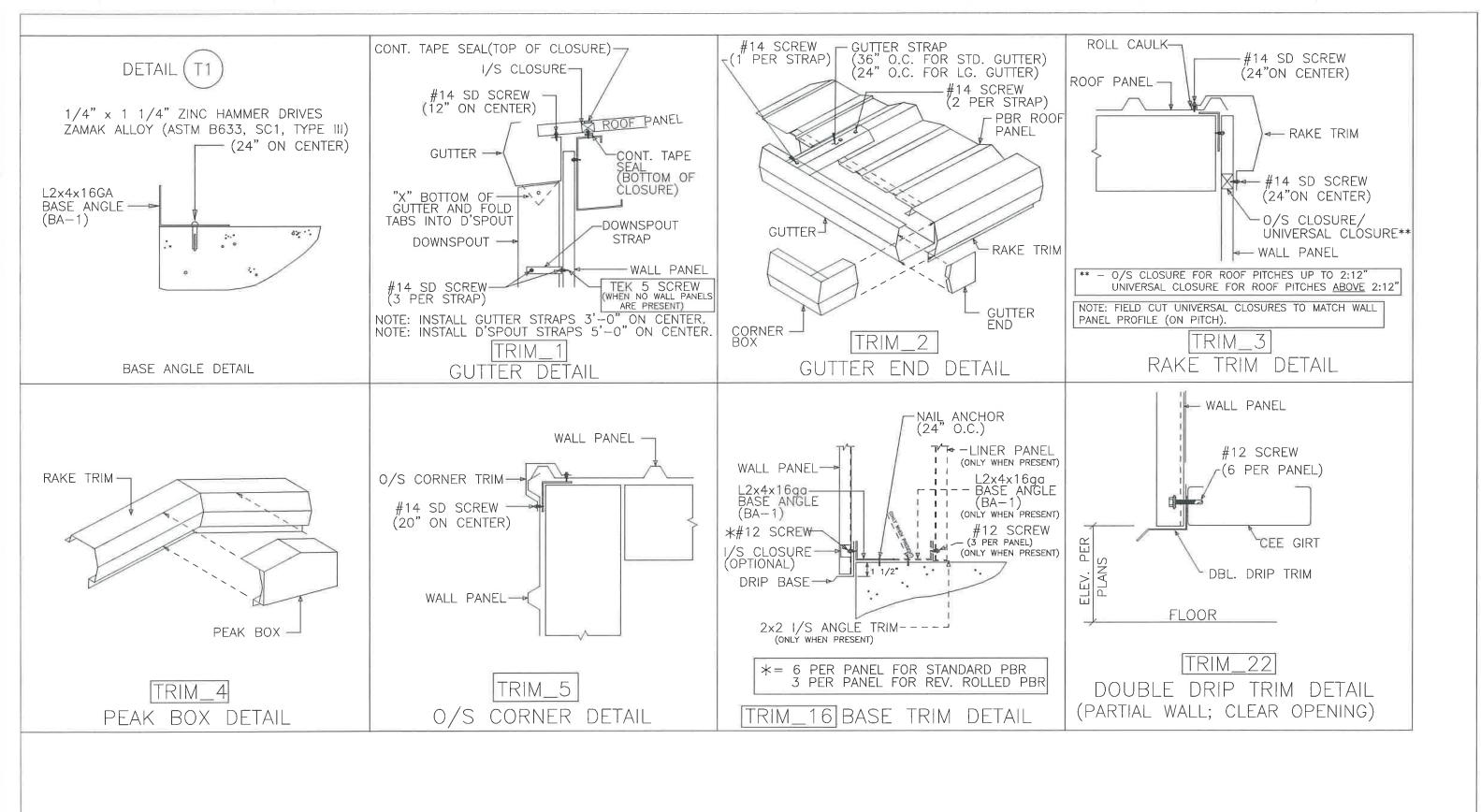
l:	SSUE	DET	СНК	DATE
BUILDINGS AND	MORE			
TOWN HOMES	LLC.			
JOB NO:		DATE:		
8459		2	/ 6/	24
LAKE CITY, FL	32025		111	
ENDWALL FRAM	IING LAYOUT			
PRAWING NO:	DRAWN BY:	CHECKED B	r: SC/	_
PAGE 4	I CTW	ISPW		NONE



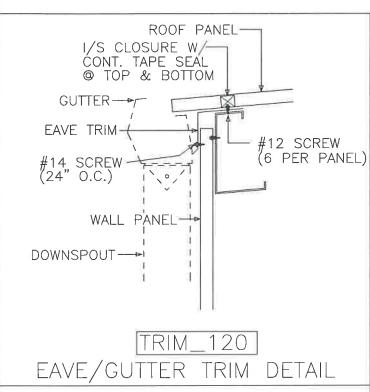
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MORE			
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32025			
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		MORE LC. DATE: 2 32025 S DRAWN BY: CHECKED BY	MORE LC. DATE: 2 / 6 / 32025

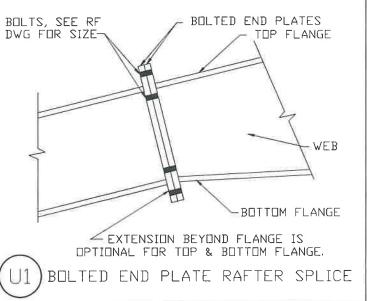


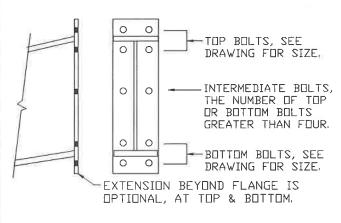
15	SSUE	DET	СНК	DATE
				_
BUILDINGS AND	MORE			
CUSTOMER: TOWN HOMES I	LLC.			
JOB NO: 8459		DATE:	/ 6/	24
LAKE CITY, FL	32025			
DRAWING NAME: FRAMING DETAIL				
PAGE 5.1	DRAWN BY: CTW	SPW	r: SCA	NONE



ISS	UE	DET	CHK	DATE
BUILDINGS AND	MORE			
CUSTOMERS TOWN HOMES LL	.C.			
NOB NO: 8459		DATE:	/ 6/	24
LAKE CITY, FL 3	2025			
FRAMING DETAILS	S			
PAGE 5.2	DRAWN BY: CTW	SPW	Y: SC	NONE







BOLTED END PLATE CONNECTION

STRUCTURAL BOLTED CONNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK.
- [2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- [3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- [4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
- 5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

MORTISE PREPPED PERSONNEL DOORS

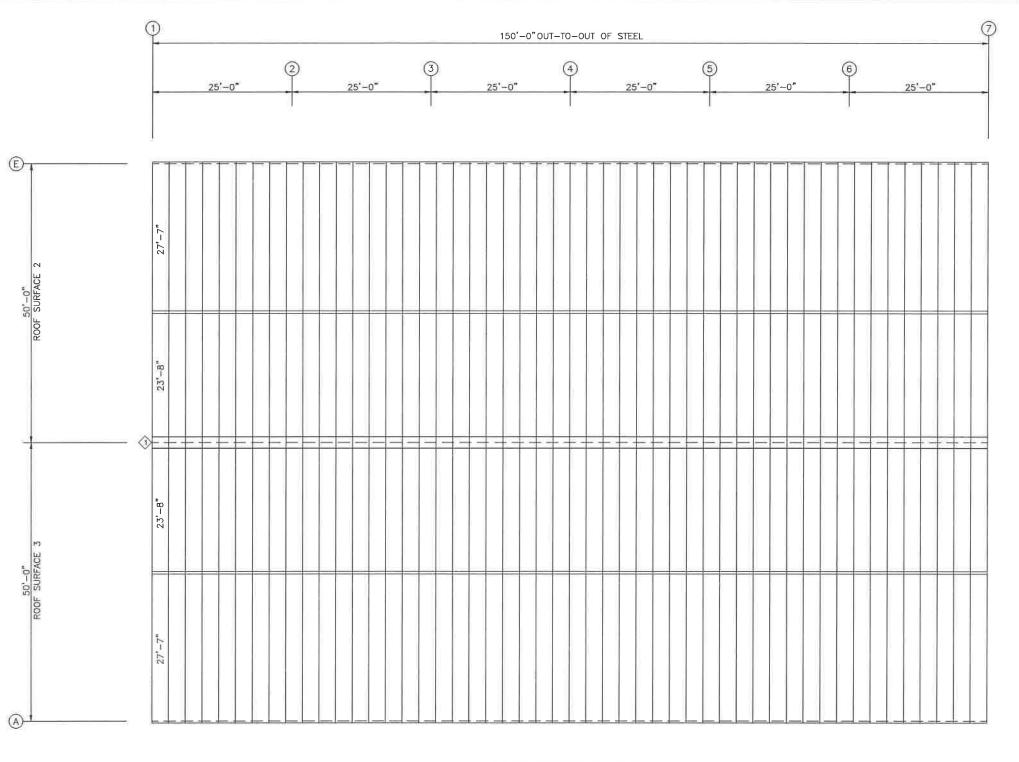
ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RE-SPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.

BUILT-UP MEMBER LEGEND				
BEAM TYPE	BEAM DEPTH	FLANGE		WEB THK.
B	08	5	4	
BUILT-UP	08= 8" 10= 10" 12= 12" 14= 14" ETC.	5,6,8,10 OR 12 (INCHES)	MEASURED IN 16ths. (4= 1/4", 5= 5/16" ETC.)	1= 10ga 3= 3/16" ETC.

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BUILDINGS AND	MORE			
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IOWIN HOWES I	LLC.	DATE:	_	
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LAKE CITY, FL	32025			
FRAWING NAME:				
FRAMING DETAIL	LS			
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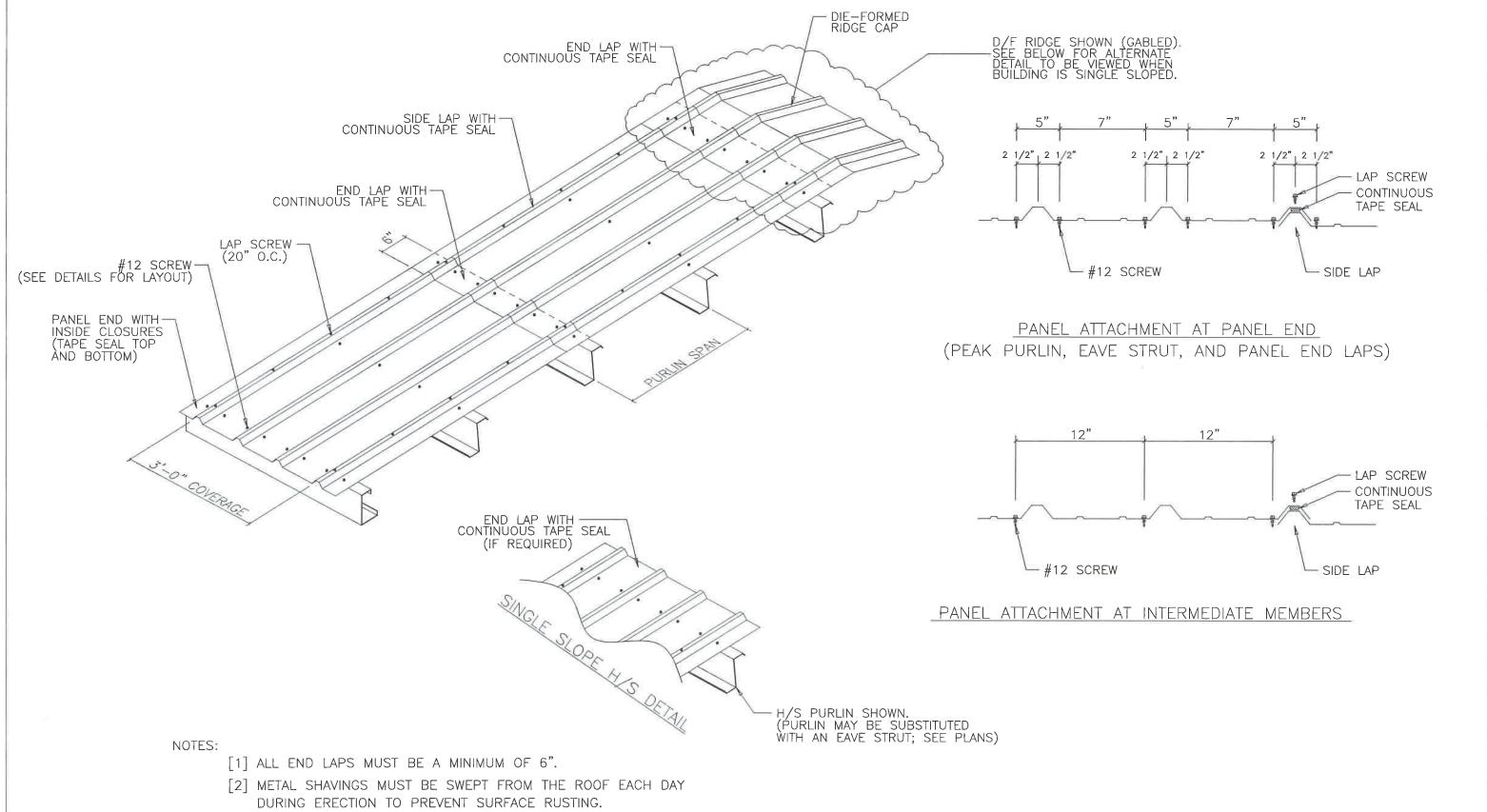
ROOF SHEETING PLAN PANELS: 26 GA. PBR – GALVALUME

ISSUE		DET	CHK	DATE
BUILDINGS AND M	IORE			
CUSTOMER: TOWN HOMES LLC	· ·			
JOB NO: 8459		DATE:	/ 6/	24
LAKE CITY, FL 32	2025			
ROOF PANELS &	TRIM			
PAGE 6	CTW	SPW SPW	SC	NONE

TRIM TABLE
ROOF PLAN

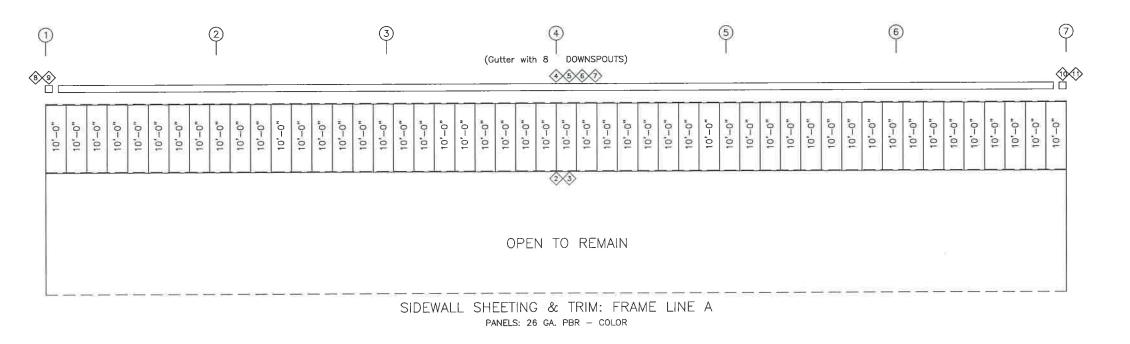
◇ID PART LENGTH

1 D/F CAP6 3'-0"



- [3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.
- [4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL—TO—PANEL ATTACHMENTS. ALL FASTENERS ARE SELF—DRILLING.

ISS	UE	DET	CHK	DATE
BUILDINGS AND	MORE			
CUSTOMER				
TOWN HOMES LI	_C.			
JOB NO:		DATES	75 30	92.10
8459 2/6/24				24
LOCATION:	11.0000048127.2			
LAKE CITY, FL 3	2025			
DRAWING NAME:				
ROOF PANEL DE	TAILS			
DRAWING NO:	DRAWN BY:	CHECKED BY	r: 50	ÀLE:
PAGE 6.1	CTW	SPW		NONE

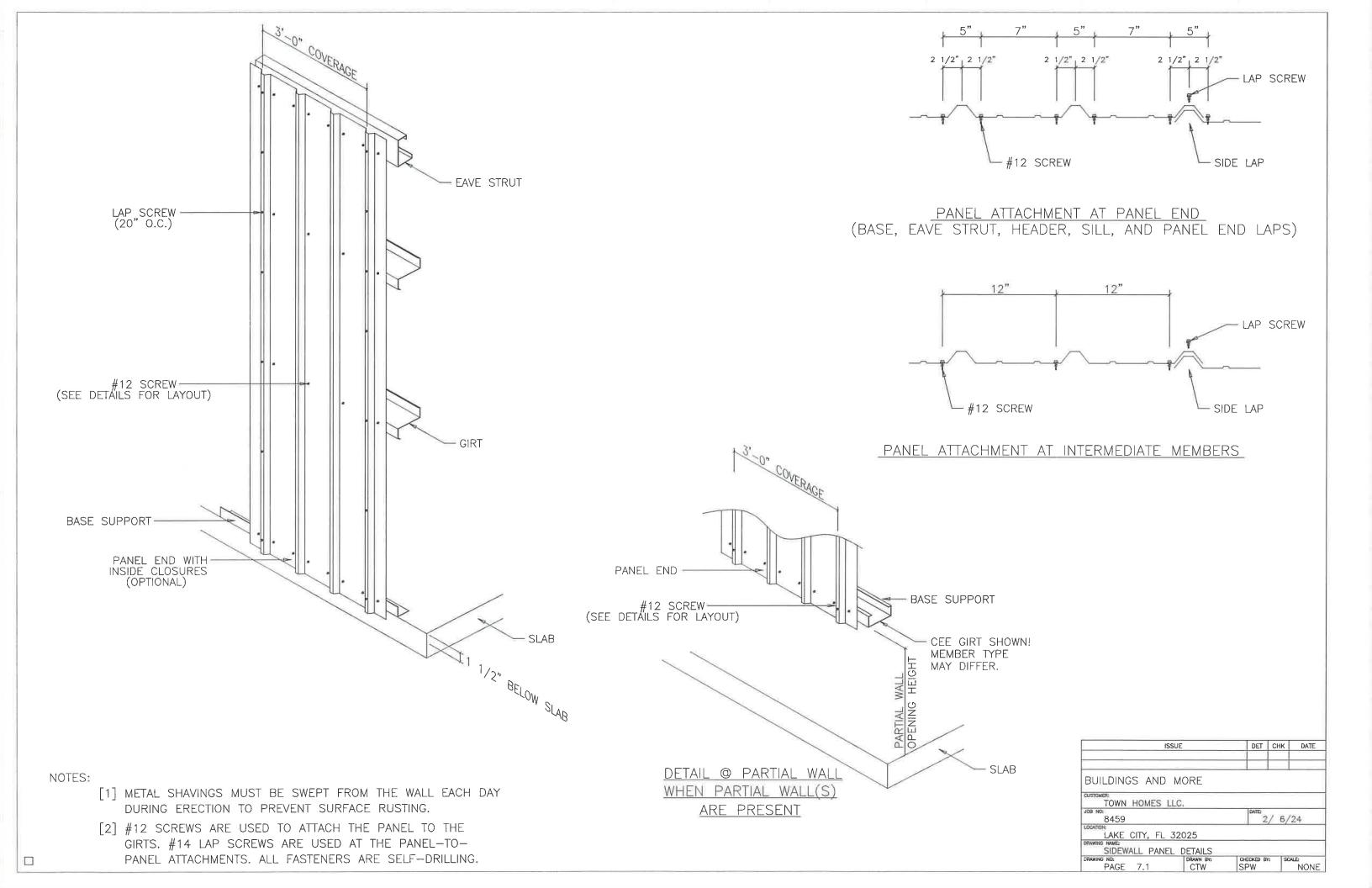


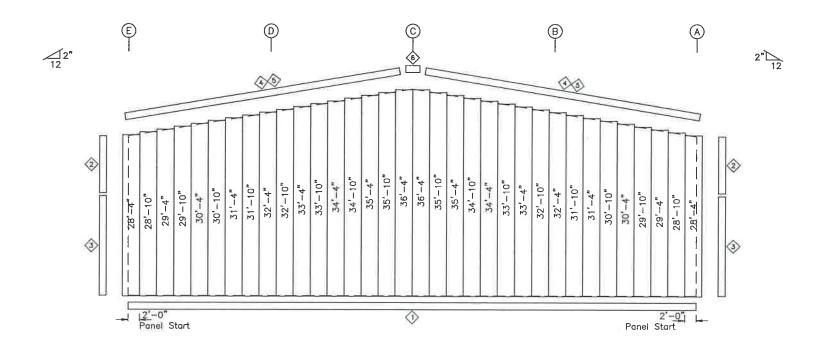
∅∅∅	6	(5)	(Gutter with 8 DOWNSPOUTS)	(3)	(2)	
10'-0" 10'-0" 10'-0"		10,-0, 10,				10'-0"
			OPEN TO REMAIN			
L		SIDEWALL	SHEETING & TRIM: FRAME	 E LINE E		

PANELS: 26 GA. PBR - COLOR

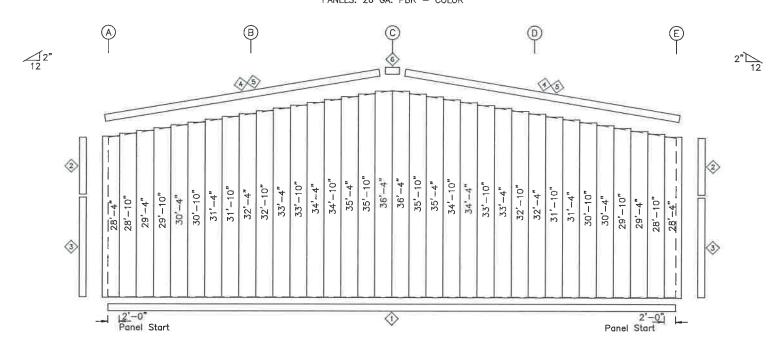
	M TABLE	^ =	
FRA	ME LINE A		I committee
♦ID	PARI	LENGTH	DETAIL
2	DBLBASTR	20'-3"	TRIM_22
3	DBLBASTR	10'-3"	TRIM_22
4	GUTTER	20'-3"	TRIM_1
5	GUTTER	10'-3"	TRIM_1
6	EAVE TRM	20'-3"	TRIM_120
1 7	EAVE TRM	10'-3"	TRIM_120
8	GUTEND L	1"	TRIM_2
9	CORBOX L	1'-0"	TRIM 2
10	GUTEND R	1"	TRIM 2
11	CORBOX R	1'-0"	TRIM_2
			- WING-WOSES-D

ISSUE		DET	CHK	DATE
BUILDINGS AND	MORE			
CUSTOMER: TOWN HOMES LI	_C.			
8459		DATE:	/ 6/	24
LAKE CITY, FL 3	32025			
DRAWING NAME: SIDEWALL PANEL	S & TRIM			
DRAWING NO: PAGE 7	DRAWN BY: CTW	SPW	: SC	NONE



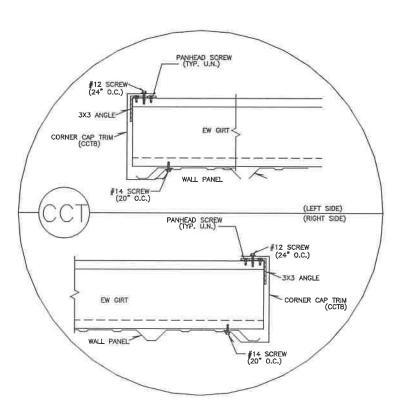




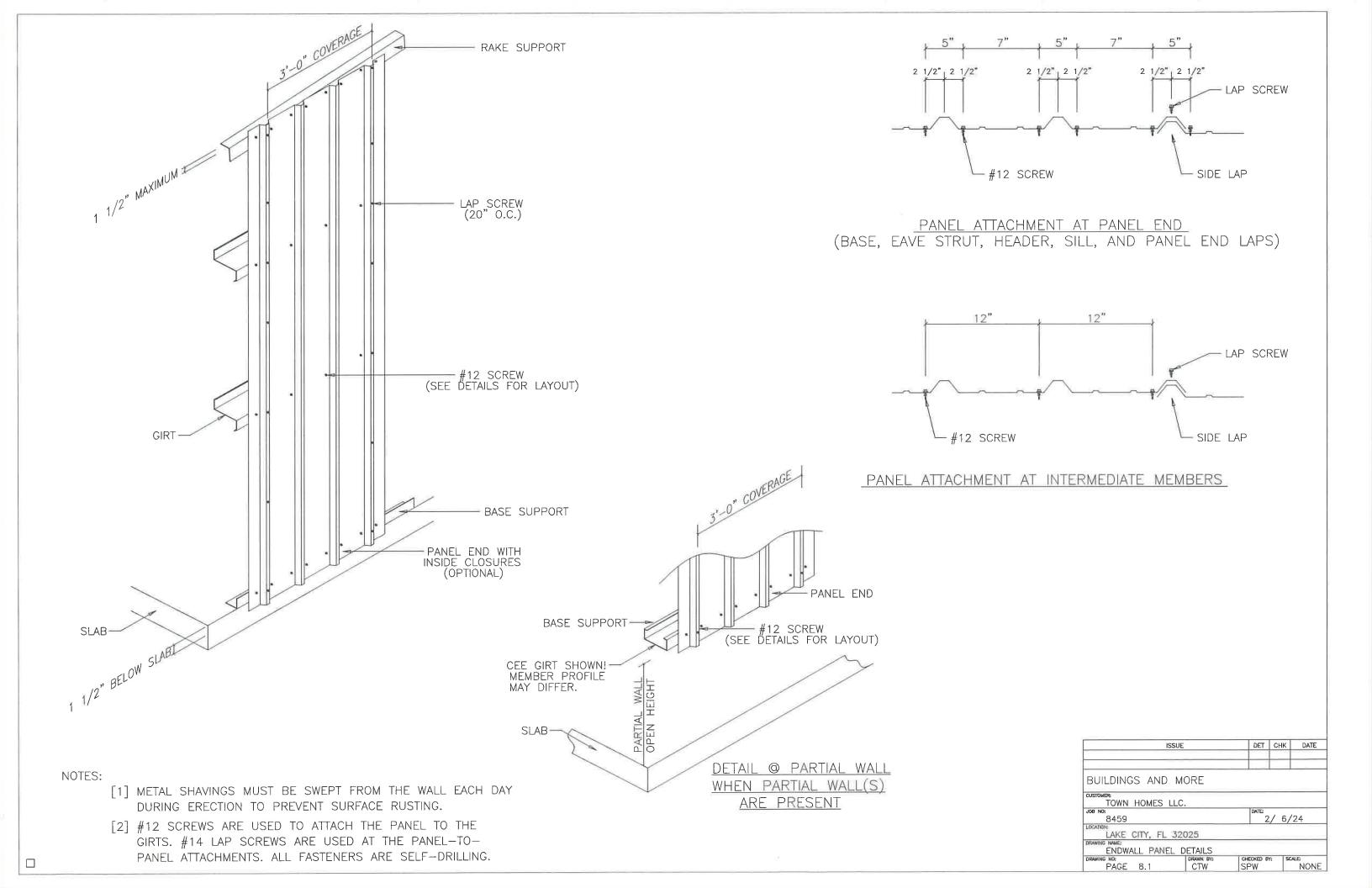


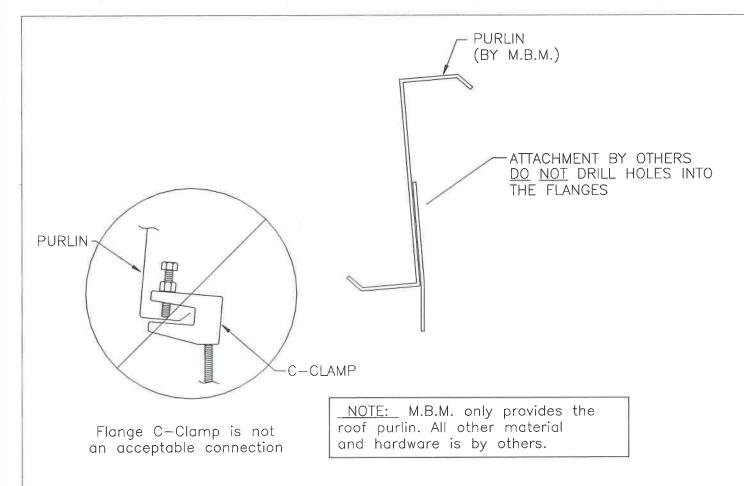
ENDWALL SHEETING & TRIM: FRAME LINE 7
PANELS: 26 GA. PBR - COLOR

TRIN	/ TABLE		
FRA	ME LINE 1	& 7	
♦ID	PART	LENGTH	DETAIL
1	DRIP BASE	20'-3"	TRIM_16
2	O/S CORN	10'-2"	TRIM_5
3	O/S CORN CCT8	18'-2"	CCT
	RAKE TRM	20'-3"	TRIM_3
5	RAKE TRM	10'-9"	TRIM_3
6	PEAK BOX	1'-4"	TRIM_4



IS	SUE	DET	CHK	DATE	
BUILDINGS AND	MORE				
CUSTOMER: TOWN HOMES I	LC.				
JOB NO: 8459	NO: DATE:		/ 6/	5/24	
LAKE CITY, FL	32025				
DRAWING NAME: ENDWALL PANE					
PAGE 8	DRAWN BY: CTW	SPW	r: SX	NONE	





Recommended Connection Detail

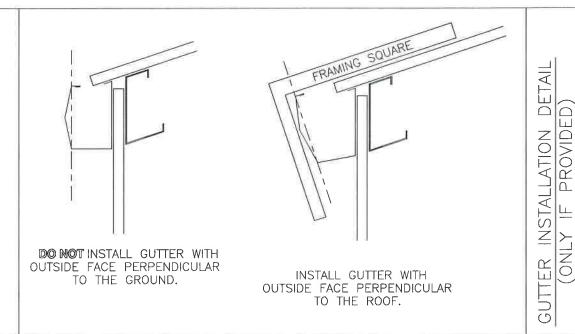
NOTE

MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT. OR SELF-DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS



GUTTER STRAP-GUTTER STRAP = #14 SCREWS 2 PER STRAP) @ 3'-0" CENTERS @ 3'-0" CENTERS FOR PBR ROOF. SEE SSR DETAILS FOR FOR PBR ROOF, SEE #14 SCREWS SSR DETAILS FOR 2 PER STRAP STANDING SEAM ROOF STANDING SEAM ROOF (WHEN APPLICABLE) (WHEN APPLICABLE) EAVE COVER TRIM **GUTTER-GUTTER** (IF PRESENT) "X" BOTTOM OF GUTTER AND -WALL X" BOTTOM OF GUTTER AND FOLD TABS INTO-PANEL #12 SCREW 24" O.C. D'SPOUT FOLD TABS INTO— D'SPOUT 700 #14 SCREW D'SPOUT STRAP STRAIGHT D'SPOUT-DOWNSPOUT OFFSET-DOWNSPOUT [TYPE 'A' OFFSET [TYPE 'A' SHOWN] SHOWN D'SPOUT D'SPOUT D'SPOUT STRAP @ D'SPOUT 5'-0" CENTERS STRAP @ 5'-0" CENTERS TEK5 SCREW-TEK5 SCREW @ OPEN WALLS W/O WALL PANELS @ PARTIAL WALLS W/ WALL PANELS

NOTE: REGARDLESS OF DOWNSPOUT OFFSET SCENARIO, TEK5 SCREWS MUST BE USED TO ATTACH DOWNSPOUT STRAPS TO PEMB FRAMING. WHEN WALL PANELS SPAN FROM GROUND TO EAVE (FULL SPAN), #14 SCREWS WILL BE USED TO ATTACH DOWNSPOUT STRAPS TO WALL PANELS.

ISSUE		CHK	DATE		
MORE		-10			
LC.					
	DATE				
9			2/ 6/24		
32025					
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CTW	SPW		NONE		
	MORE LC. 32025	MORE LC. DATE: 2 32025	MORE LC. DATE 2/6/ 32025 S DRAWN BY: CHECKED BY: SQL		

(ONL)