#### **STRUCTURAL NOTES: CAST IN PLACE CONCRETE** WOOD CONSTRUCTION **UPLIFT CONNECTORS** CODES: 1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 1. WOOD CONSTRUCTION SHALL CONFORM TO THE NDS "NATIONAL DESIGN 1. UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS 28 DAYS OF 2500 PSI, A SLUMP OF 3" FOR FOOTINGS/FOUNDATIONS SPECIFICATION FOR WOOD CONSTRUCTION", LATEST EDITION. AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS AND 4" FOR SLABS 2. ALL EXTERIOR WOOD STUD WALLS, BEARING WALLS, SHEAR WALLS AND THAT ARE EXPOSED TO UPLIFT FORCES. INTERIOR LOAD BEARING MISC. STRUCTURAL WOOD FRAMING MEMBERS, (I.E. BLOCKING OR GABLE 2. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS END BRACING ) SHALL BE EITHER SOUTHERN PINE, OR S.P.F. NUMBER STEEL CONFORMING TO ASTM A-615 GRADE 40. OF THESE WALLS WOULD NOT NEED TO HAVE CONNECTORS APPLIED. 3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. WWF SHALL 2 GRADE OR BETTER SHALL BE USED REGARDLESS OF SPECIES. PLEASE CONSULT THE TRUSS ENGINEERING FOR THE LOCATION OF BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE 3. ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN THESE WALLS. THE CENTER OF THE STUD UP TO 1" DIA. SHALL HAVE STUD PROTECTION WITHIN THE 6". **FIELD REPAIR NOTES** 4. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS SHIELDS FOR ALL HOLES OVER 1" IN DIA. FOR PLUMBING LINES, ETC. SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES, TYP., U.N.O. OF BEAMS. 4. FASTENERS FOR PRESSURE PRESERVATIVE AND FIRE-RETARDANT-TREATED 5. HORIZONTAL FOOTING BARS SHALL HAVE 1'-0" HOOK LENGTH OR 1. MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED WOOD SHALL BE OF HOT-DIPPED GALVANIZED STEEL, STAINLESS STEEL, CORNER BARS WITH A 2'-1" LAP PROVIDED W/ 1/2" DIA. EPOXY ANCHORS WITH 6" EMBEDMENT. SIMPSON **LIVE LOADS:** SILICON BRONZE OR COPPER. 6. MINIMUM LAP SPLICES ON ALL REINFORCING BAR SPLICES SHALL BE 40 "SET" EPOXY ADHESIVE BINDER FOLLOWING ALL MANUFACTURERS BAR DIAMETERS TYP. RECOMMENDATIONS. SEE PLAN FOR EMBEDMENT DEPTH AT FLOOR 7. CONCRETE COVER MIN. 3" WHEN EXPOSED TO EARTH OR 1 1/2" TO PREFABRICATED WOOD TRUSSES 2. FOR MISSED VERT. DOWELS DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REBAR, AND INSTALL A 32" LONG #5 1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EMBEDMENT EPOXY TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR CONCRETE REINFORCING STEEL ( SIMPSON "SET", EPOXY ), MIXED PER MANUFACTURER'S INSTRUCTIONS. STRENGTH ASSURE THAT ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED 2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE ALL REINFORCING STEEL SHALL BE NEW DEFORMED BARS FREE FROM RUST, @ 28 DAYS FROM THE HOLE BY BRUSHING AND AND USING COMPRESSED AIR PRIOR WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION SCALE & OIL & SHALL MEET ASTM A-615 REINFORCING FOR FOOTING SHALL TO APPLYING THE EPOXY. ALLOW THE EPOXY TO CURE TO FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED **REINFORCING:** BE SUPPORTED ON PRE-CAST CONCRETE PADS, TOP REINFORCING SHALL BE MANUFACTURER'S SPECIFICATIONS, THEN FILL THE CELL IN THE NORMAL BY THE NATIONAL FOREST PRODUCTS ASSOCIATION. POSITIVELY SUPPORTED BY TEMPORARY STRINGERS. DOWELS FOR COLUMNS WAY DURING BOND BEAM POUR. 3. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH & FILLED CELLS SHALL BE SECURED IN PLACE BY USING ADDITIONAL CROSS-3. FOR MORTER JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT. IN CONC. A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF REINFORCING TIED TO FOOTING REINFORCING. SPLICES IN REINFORCING FILLED CELL EACH SIDE OF THE JOINT ( BAR DOES NOT HAVE TO BE 25%) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL WHERE PERMITTED SHALL BE THE FOLLOWING MINIMUM, UNLESS OTHERWISE CONT. TO FOOTING) DEAD LOAD. INDICATED ON THE DRAWINGS: CONCRETE 4. MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE 4. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY FTGS, WALLS, COLUMNS, BEAMS, SLABS: 36 DIA. OR 2'-0" MIN. **MASONRY** SUBSTITUTED WITH (1) SIMPSON MTSM16 TWIST STRAP W/ (4) 1/4" X 21/4" THE TRUSS MANUFACTURER UNLESS NOTED ON THE PLANS. FILLED CELL REINFORCING: 40 DIA. OR 2'-1" MIN. UNITS: TITENS TO MASONRY AND (7)-10d NAILS TO TRUSS FOR UPLIFTS LESS 5. TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION TEMPERATURE REINFORCING: 20 DIA. OR 1'-0" MIN. THAN 860 LBS (USE (2) MTSM16 FOR UPLIFTS LESS THAN 1720#). OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE WELDED WIRE MESH: 8" LAP NO MORE THAN 10 STRAPS MAY BE SUBSTITUTED OR NO MORE THAN DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE **STRUCTURAL MASONRY WALL CONST.**

- 1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 1900 PSI (f'm = 1500 PSI)
- 2. MORTAR SHALL BE TYPE "S", CONFORMING TO ASTM C270 3. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH
- AT 28 DAYS OF 3000 PSI SLUMP 8" TO 11". 4. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS
- WITH THE CELLS FILLED WITH COARSE GROUT. 5. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 192 BAR DIAMETERS. REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY
- CELL TYPICAL UNLESS OTHERWISE NOTED. 6. REINFORCING STEEL SHALL BE LAPPED A MINIMUM OF 40 BAR
- DIAMETERS, UNLESS OTHERWISE NOTED ON THE DRAWINGS. 7. GROUT STOPS SHALL BE PROVIDED BELOW BOND BEAM. PLASTIC SCREEN, METAL LATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT THE FLOW GROUT INTO CELLS BELOW. THE USE OF FELT PAPER AS A STOP IS PROHIBITED.
- FOLLOWING DESIGN LOADS:
- 6. DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE TPI LATEST EDITION. 7. PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE
- MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES . SUBMITTALS SHALL INCLUDE TRUSS FRAMING PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, ANCHORAGE, CONNECTIONS, TRUSS LOCATIONS, AND PERMANENT BRACING AND/OR BRIDGING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED STRUCTURAL ENGINEER. SUBMIT 2 COPIES FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
- 8. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS HANGERS.

#### SOIL BEARING VALUE:

ASSUMED ALLOWABLE SOIL BEARING PRESSURE AFTER COMPACTION: 2000 PSF SEE SOILS REPORT AND SPECIFICATIONS FOR COMPACTION REQUIREMENTS IF SOIL CONDITIONS IN THE PROJECT DO NOT MEET OR EXCEED THE CAPACITY THE GENERAL CONTRACTOR SHALL CONTACT THE ENGINEER PRIOR TO FOUNDATION POUR FOR VERIFICATION OF FOUNDATION DESIGN. SOIL TO BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY AS DETERMINED BY ASTM - D1557

3 IN A ROW. IF GIRDER TRUSS CONNECTIONS ARE MISSED CONTACT ENGINEER OF RECORD FOR SUBSTITUTION.

#### **TERMITE SPECIFICATIONS:**

**SECTION R318 PROTECTION AGAINST TERMITES** 

TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE A PREVENTIVE TREATMENT TO NEW CONSTRUCTION (SEE SECTION 202, REGISTERED TERMITICIDE). UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE

#### RADON:

WHERE PROJECT IS TO BE LOCATED IN KNOWN RADON GAS PREVALENT AREAS, APPENDIX "F" OF THE 2023 FLORIDA RESIDENTIAL BUILDING CODE IS TO BE IMPLEMENTED. CONCRETE STRENGTH IN THESE AREAS ARE TO BE A MINIMUM OF 3000 P.S.I.. THEREFORE, ANY AND ALL NOTES ON THESE PLANS THAT INDICATE 2500 PSI SHALL BE REPLACED WITH 3000 P.S.I. FOR THE CONCRETE STRENGTH.

FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

#### 2023 FLORIDA BUILDING CODE RESIDENTIAL 2023 FLORIDA FIRE PREVENTION CODE 2023 FLORIDA ACCESSIBILITY CODE NEC NFPA 70 & FBCEB

ACI 318-19 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 301-19 SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS ACI 530-19 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES 2020 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION 2020 WOOD FRAMED CONSTRUCTION MANUAL APA PLYWOOD DESIGN SPECIFICATION

STRUCTURAL DESIGN CRITERIA

ASCE/SEI 7-16 AMERICAN SOCIETY OF CIVIL ENGINEERS

20 PSF (REDUCIBLE) RESIDENTIAL FLOOR, UNLESS OTHERWISE INDICATED 40 PSF 60 PSF BALCONIES 40 PSF STAIRS 20 PSF LIGHT PARTITIONS (DEAD LOAD), U.N.O. 10 PSF ATTIC L.L

2500 PSI

3000 PSI

ASTM A1064/A1064M

MINIMUM 1.5 LBS. OF

FIBERS PER CUBIC YARD

ASTM A615-40 40,000 PS

ASTM A615-40 40,000 PSI

ALL CONCRETE UNLESS OTHERWISE INDICATED PEA GRAVEL CONCRETE FOR MASONRY CELLS ONLY (DO NOT USE FOR CONCRETE COLUMNS OR TIE BEAMS)

ALL REINFORCING BARS ALL STIRRUPS AND TIES POLYPROPYLENE FIBERS FOR SLABS ON GRADE

ASTM C90-01, STANDARD WEIGHT UNITS, fm=1500 PSI MORTAR TYPE "S" 1800 PSI

CONCRETE GROUT 3000 PSI

WOOD FRAMING:

TRUSSES:

CONTINUOUS MASONRY INSPECTION IS REQUIRED DURING CONSTRUCTION ALL STRUCTURAL AND MISCELLANEOUS STEEL A36 36,000 PSI, U.N.O

WELDED WIRE FABRIC SHALL CONFORM TO

SHOP AND FIELD WELDS: E70XX ELECTRODES STEEL:

ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307

BEAMS, RAFTERS, JOIST, PLATES, ETC. U.N.O. NO. 2 SOUTHERN YELLOW PINE (19% M.C.) ROOF DECK: PLYWOOD C-C/C-D, EXTERIOR, or OSB FLOOR SHEATHING: T&G A-C GROUP 1 APA RATED (48/24) WALL SHEATHING: PLYWOOD C-C/C-D, EXTERIOR OR OSB VERSA LAM BEAM Fb = 2900 PSI (2.0E)

WOOD COLS. PARALLAM 2.0E U.N.O. **DESIGN LOADS:** SHINGLE ROOF: **WOOD ROOF** TOP CHORD LIVE LOAD: 20 PSF 10 PSF TOP CHORD DEAD LOAD:

> BOTTOM CHORD DEAD LOAD: BOTTOM CHORD ATTIC LIVE LOAD: 10 PSF

SEE DRAWINGS FOR SPECIAL CONCENTRATED LOADS. DESIGN FOR NEW WIND UPLIFT AS PER SPECIFIED CODES, DEDUCTING A MAXIMUM OF 5 P.S.F. DEAD LOAD, BUT NOT EXCEEDING ACTUAL DEAD LOAD.

### **INDEX OF DRAWINGS**

L	COVER SHEET
2	FLOOR PLAN
	ELEVATIONS
3	FOUNDATION PLAN
	ELECTRICAL PLAN
	TRUSS LAYOUT

TITLE

SHT NO:

# **DETAILS**



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Digitally

signed by Randolph Wiggins Date: 2024.07.09 11:35:49

-04'00'

## WIND LOADING CRITERIA

WIND SPEED (ULTIMATE) WIND SPEED (ALLOWABLE) 101 MPH EXPOSURE CATEGORY BUILDING CATEGORY BUILDING TYPE ENCLOSURE CLASSIFICATION ENCLOSED INTERNAL PRESSURE COEFFICIENT +/- 0.18

#### **NOTICE TO BUILDER**

THIS DRAWING AND DESIGN IS VALID FOR 12 MONTHS AFTER THE DATE IT IS SIGNED AND SEALED OR WHILE **CURRENT CODE IS VALID** 

IT IS THE INTENT OF THIS DESIGNER THAT THESE PLANS ARE ACCURATE AND ARE CLEAR ENOUGH FOR THE LICENSED PROFESS-ONAL TO CONSTRUCT THIS PROJECT. IN THE EVENT THAT SOMETHING IS UNCLEAR OR NEEDS CLARIFICATION..STOP..AND CALL THE DESIGNER LISTED IN THIS TITLE PAGE. IT IS THE RESPONSIBILITY OF THE LICENSED PROFESSIONAL THAT IS CONSTRUCTING THIS PROJECT TO FULLY REVIEW THESE DOCUMENTS BEFORE CONSTRUCTION BEGINS AND ANY AND ALL CORRECTIONS, IF NEEDED, TO BE MADE BEFORE ANY WORK IS DONE. \*DO NOT SCALE DRAWINGS FOR CRITICAL DIMENSIONS

INSTEAD CALL THE DESIGNER LISTED IN TITLE PAGE\*

SHEET NO.

Residence SW Herlong

Miller Re 3642 SV Ft. White Columbi





