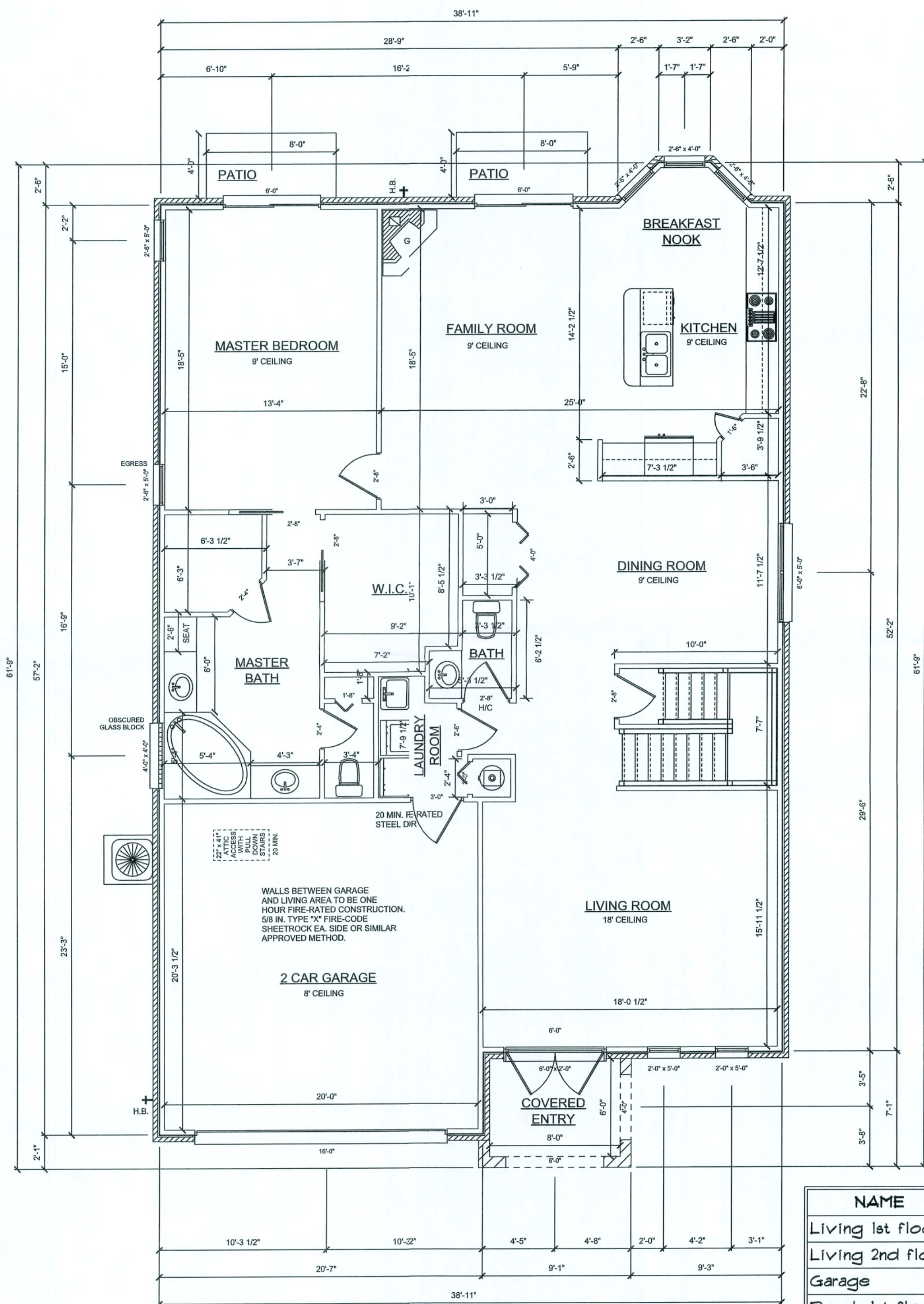
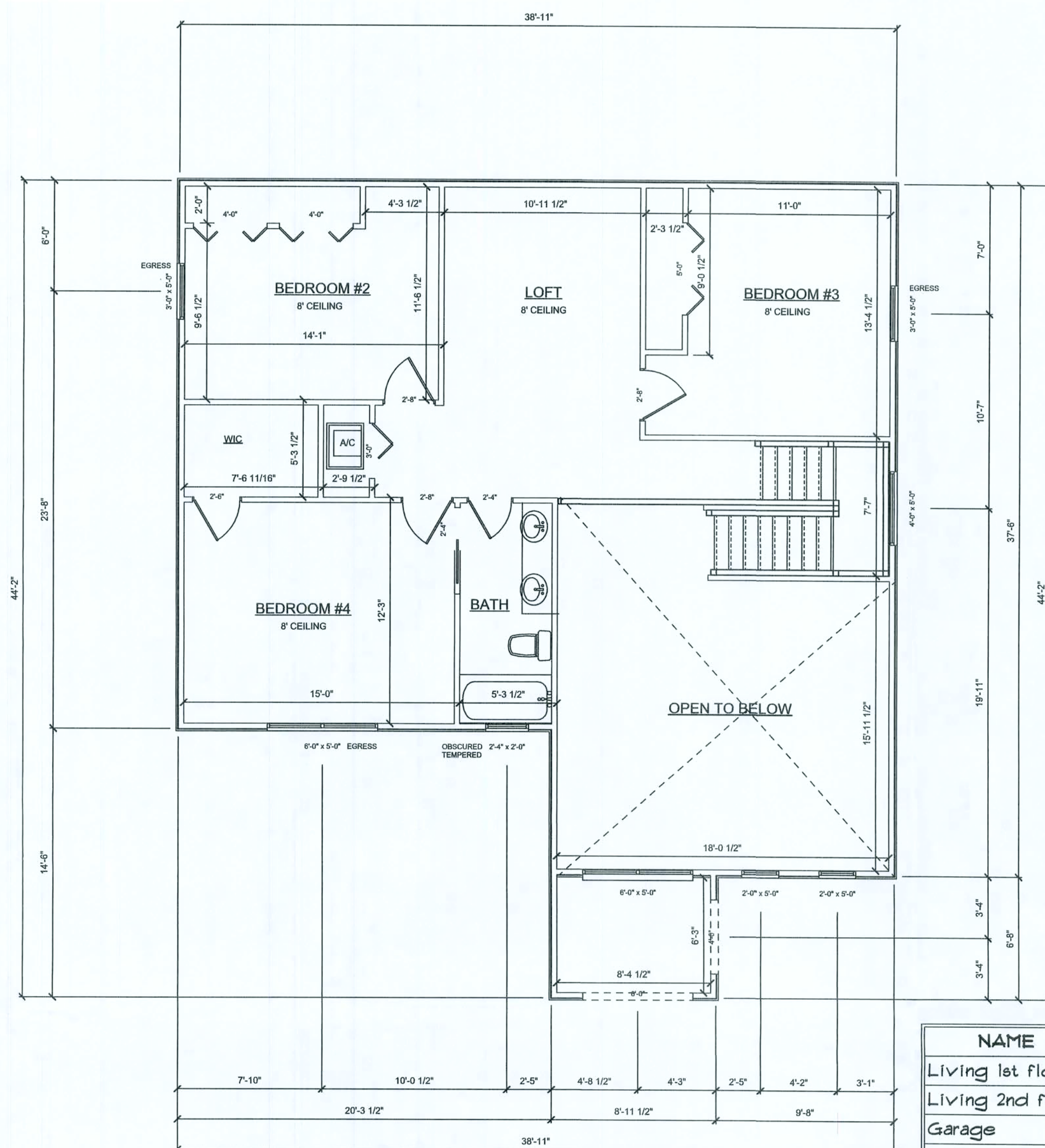




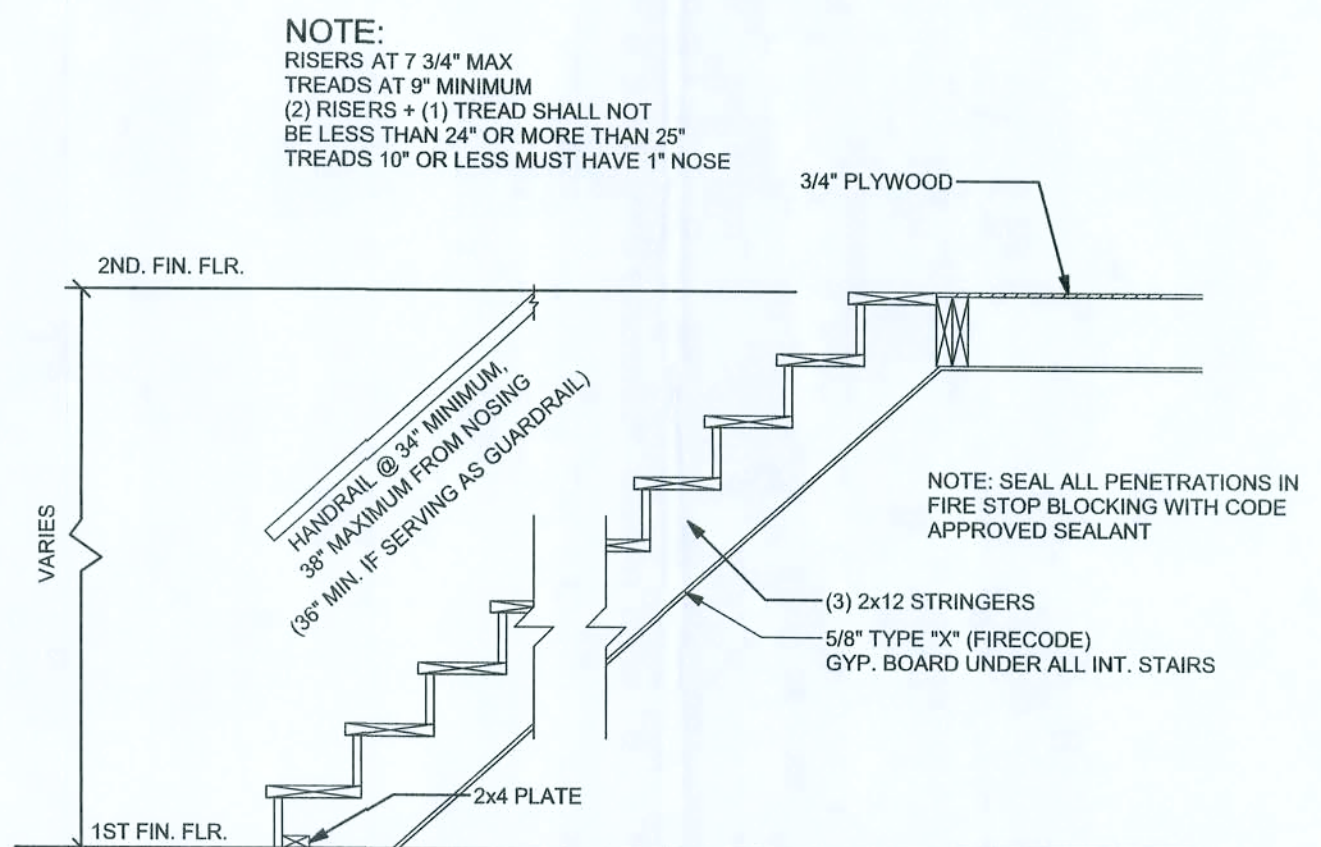
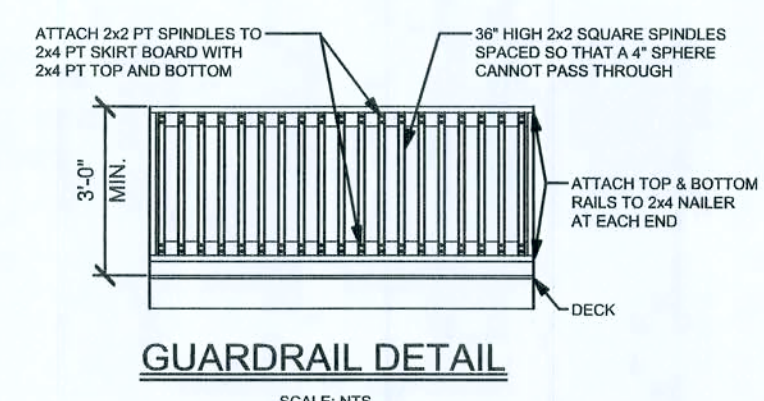
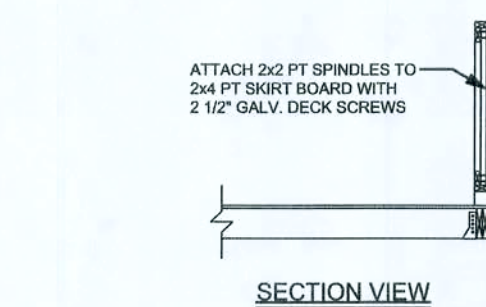
1ST FLOOR PLAN

NAME	AREA
Living 1st floor	1791 S.F.
Living 2nd floor	950 S.F.
Garage	435 S.F.
Porch 1st floor	64 S.F.
Porch 2nd floor	64 S.F.
Total	3310 S.F.



2ND FLOOR PLAN

NAME	AREA
Living 1st floor	1791 S.F.
Living 2nd floor	950 S.F.
Garage	435 S.F.
Porch 1st floor	64 S.F.
Porch 2nd floor	64 S.F.
Total	3310 S.F.



1403 STAIR DETAIL
SCALE: NTS

Permit#
0812-33

Lipscomb Eagle
Development, Inc.

Mediterranean Model

ADDRESS:
Lot 43 Preserve S/D
Columbia County, Florida
Lipscomb Eagle
Development, Inc.
872 SW Jaguar Drive
Lake City, Florida 32025
Phone: (386) 719 - 6960
Fax: (386) 719 - 9586

PRINTED DATE:
January 07 2008
DRAWN BY:
TJ Clanton
CHECKED BY:
JUSAN HOLTON
DESIGNED BY:

FINALES DATE:
14 / Dec / 07

JOB NUMBER:

DRAWING NUMBER

A-2

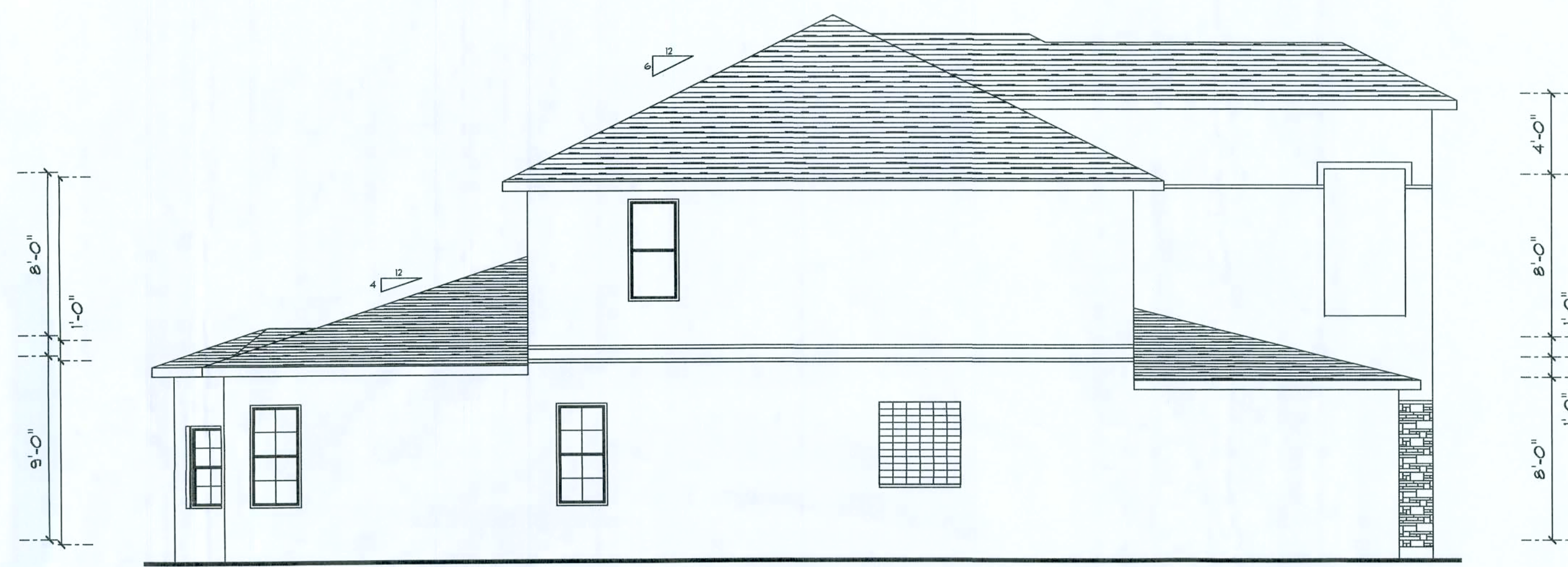
2 OF 3 SHEETS

REVISIONS	

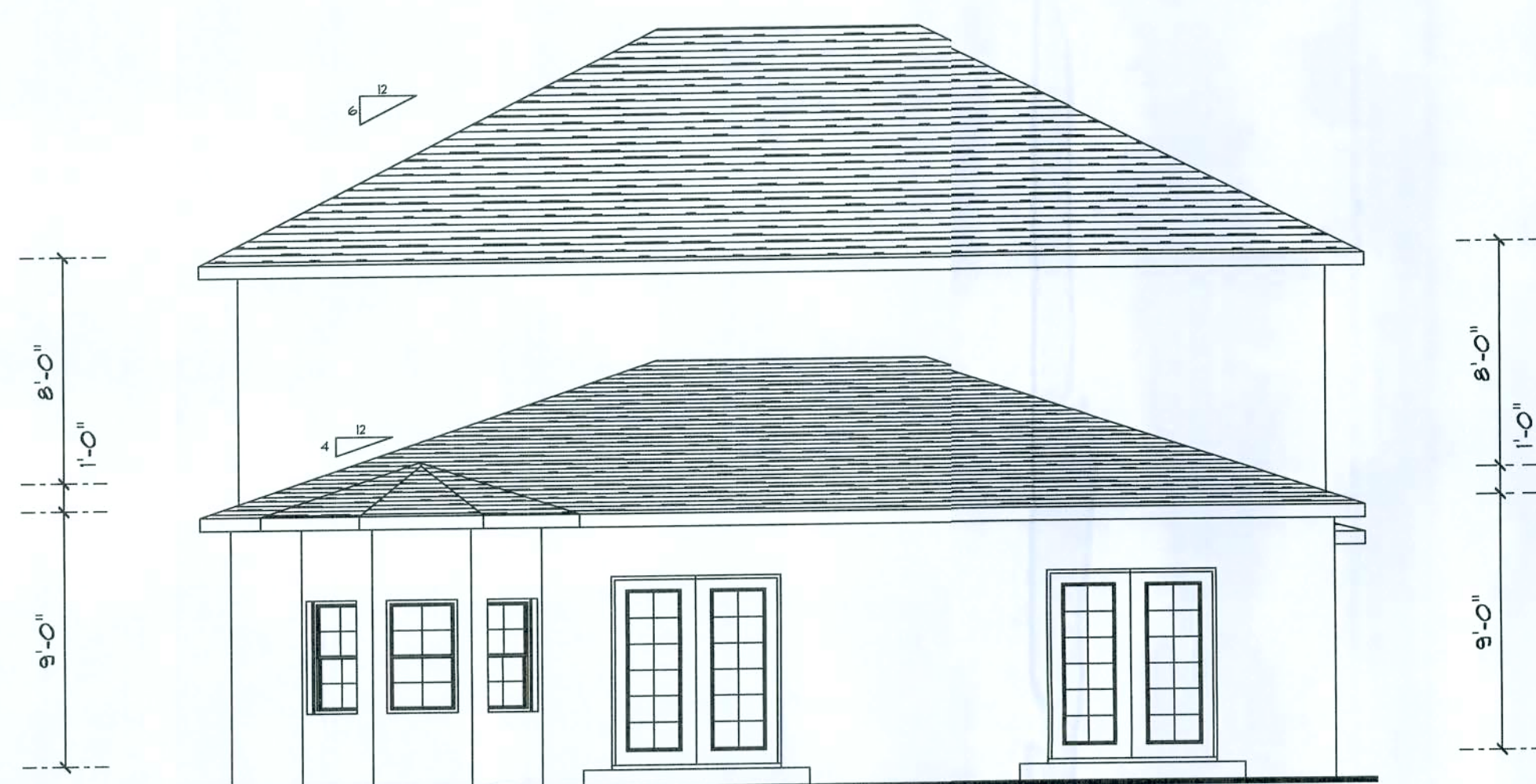
SOTPLAN
ARCHITECTURAL DESIGN SOFTWARE



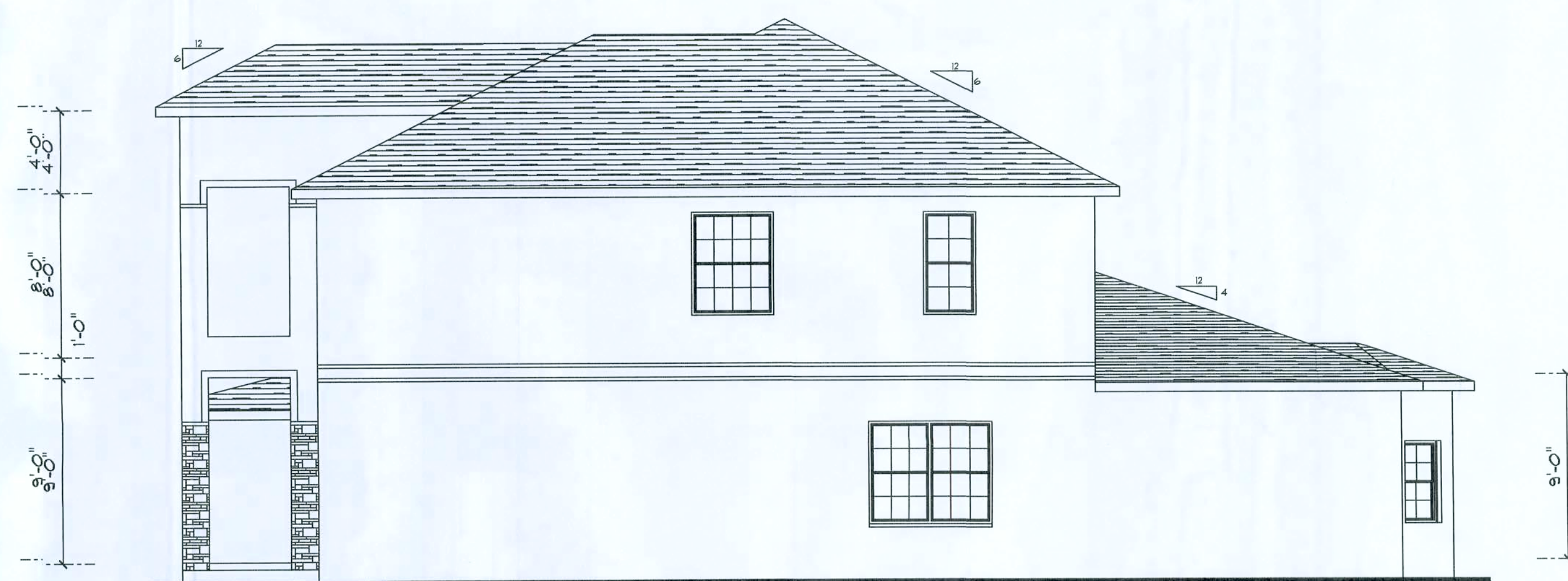
FRONT ELEVATION



LEFT ELEVATION



REAR ELEVATION



RIGHT ELEVATION

Lipscomb Eagle
Development, Inc.

Mediterranean Model

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Lot43 Preserve S/D
Columbia County, Florida

Lipscomb Eagle
Development, Inc.
872 SW Jaguar Drive
Lake City, Florida 32025
Phone (386) 719 - 6960
Fax: (386) 719 - 9586

*PRINTED DATE:
December 14, 2007

DRAWN BY: TJ Clinton
CHECKED BY: SUSAN HOLTON
DESIGNED BY:

FINALES DATE:
14 / Dec 07

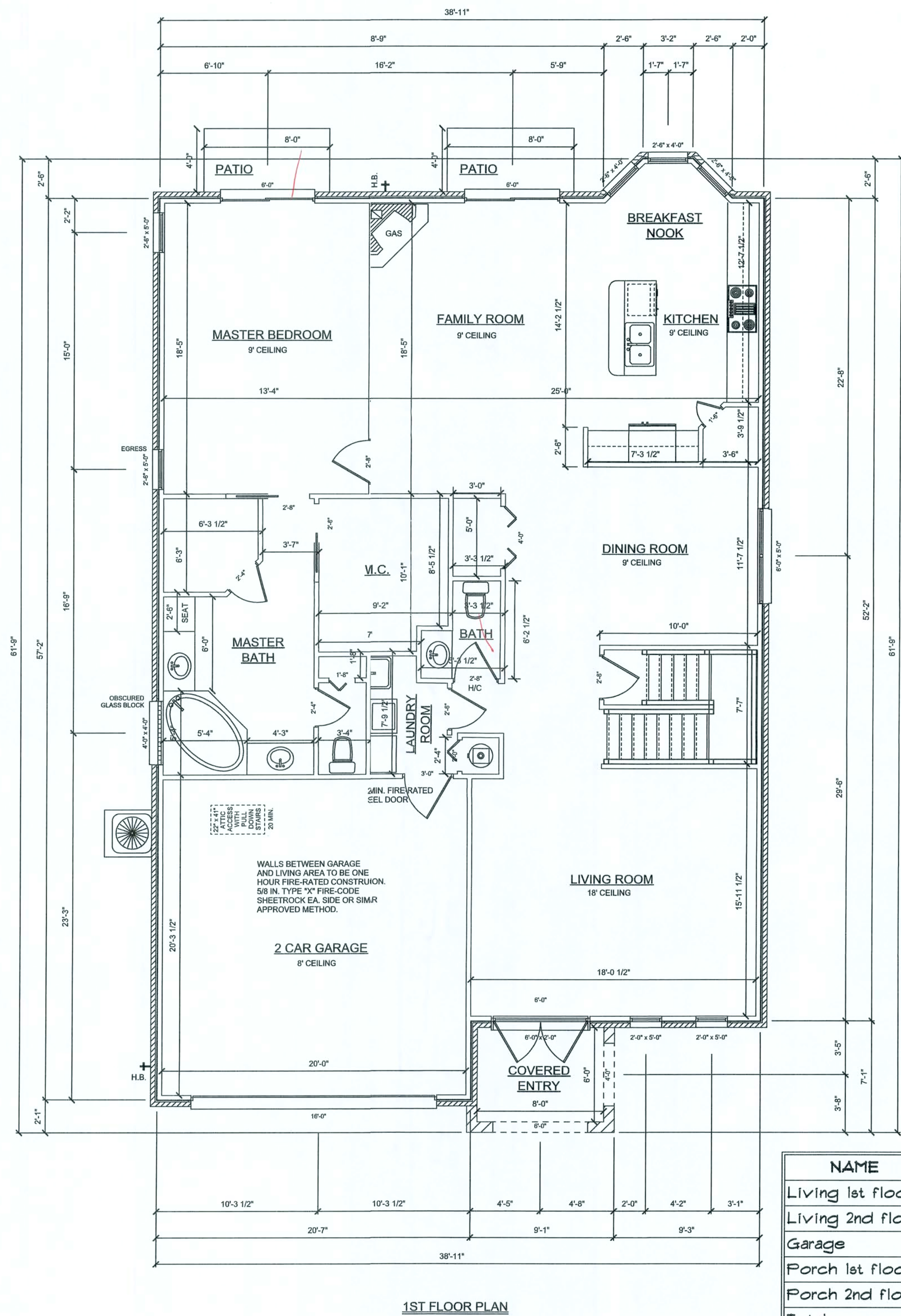
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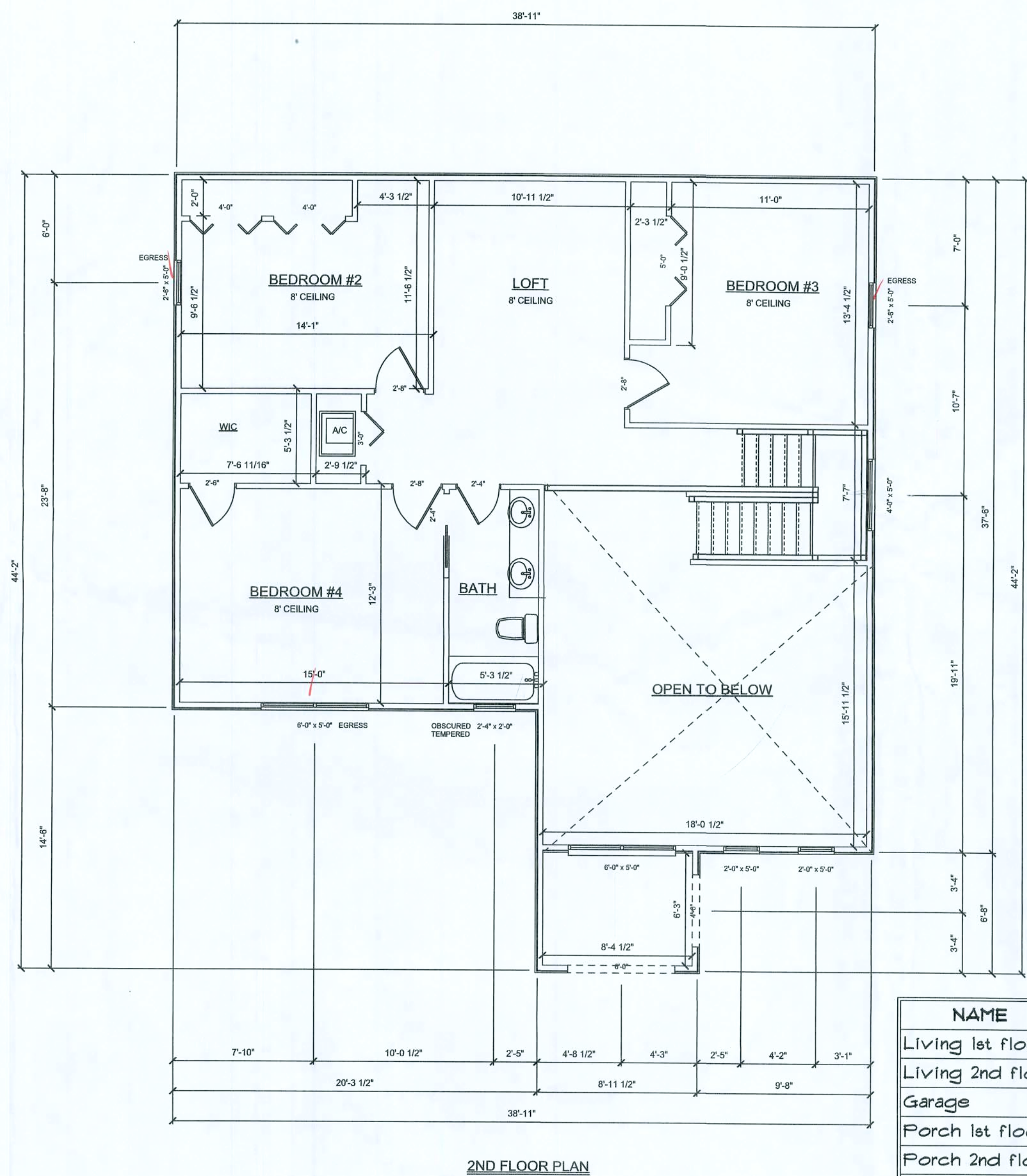
A-1

10F 3 SHEETS

REVISIONS	



NAME	AREA
Living 1st floor	1797 S.F.
Living 2nd floor	950 S.F.
Garage	435 S.F.
Porch 1st floor	64 S.F.
Porch 2nd floor	64 S.F.
Total	3310 S.F.



NAME	AREA
Living 1st floor	1797 S.F.
Living 2nd floor	950 S.F.
Garage	435 S.F.
Porch 1st floor	64 S.F.
Porch 2nd floor	64 S.F.
Total	3310 S.F.

Lipscomb Eagle Development, Inc.

Mediterranean Model

ADDRESS:
Lot 4: Preserve S/D
Columbia County, Florida
Lipscomb Eagle Development, Inc.
872 SW Jaguar Drive
Lake City, Florida 32025
Phone: (866) 719 - 6960
Fax: (316) 719 - 9586

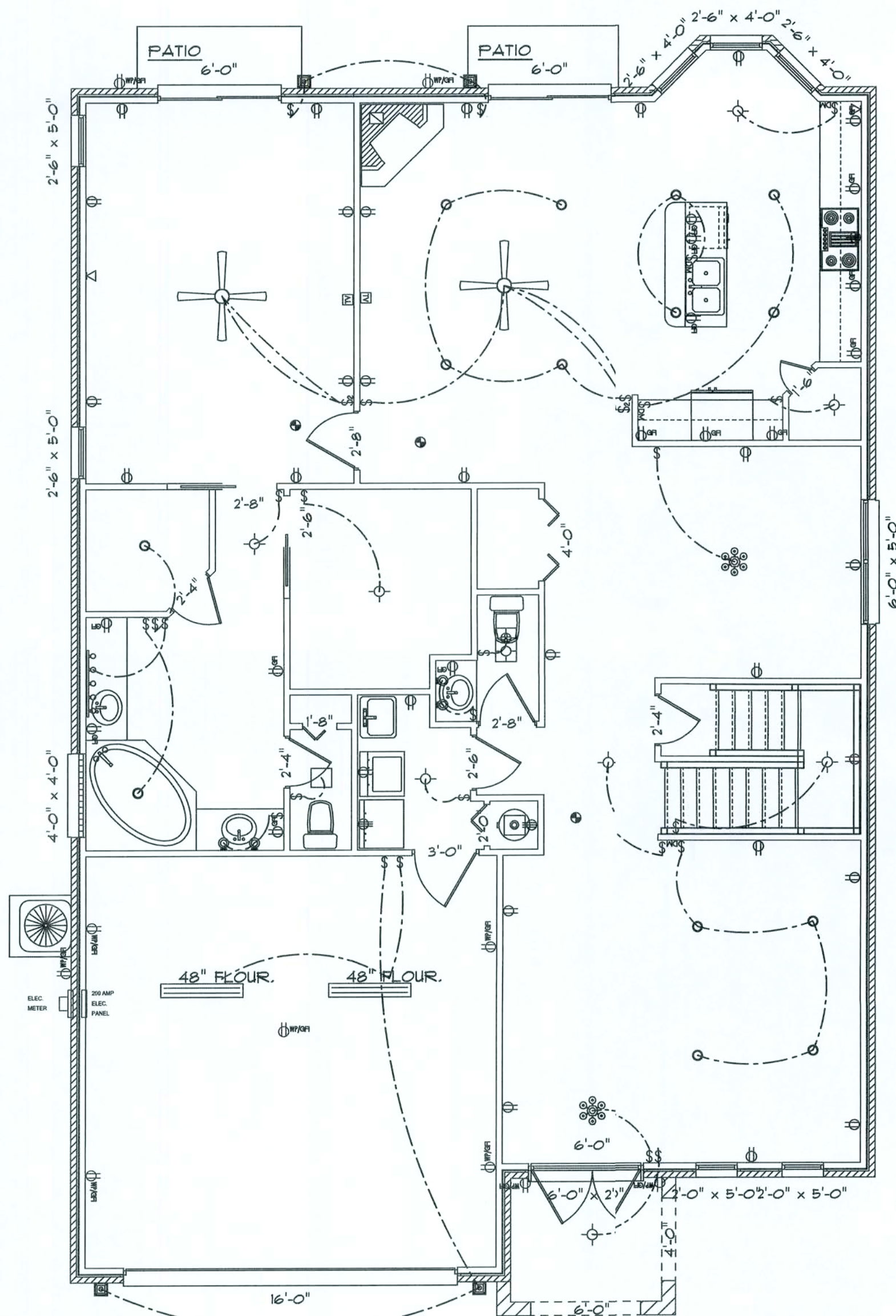
PRINTED DATE:
December 14, 2007
DRAWN BY: TJ Clanton
CHECKED BY: SUSAN HOLTON
DESIGNED BY:

FINALES DATE:
14 / Dec / 07

JOBNUMBER:

DRAWING NUMBER
A-2
2 OF 3 SHEETS

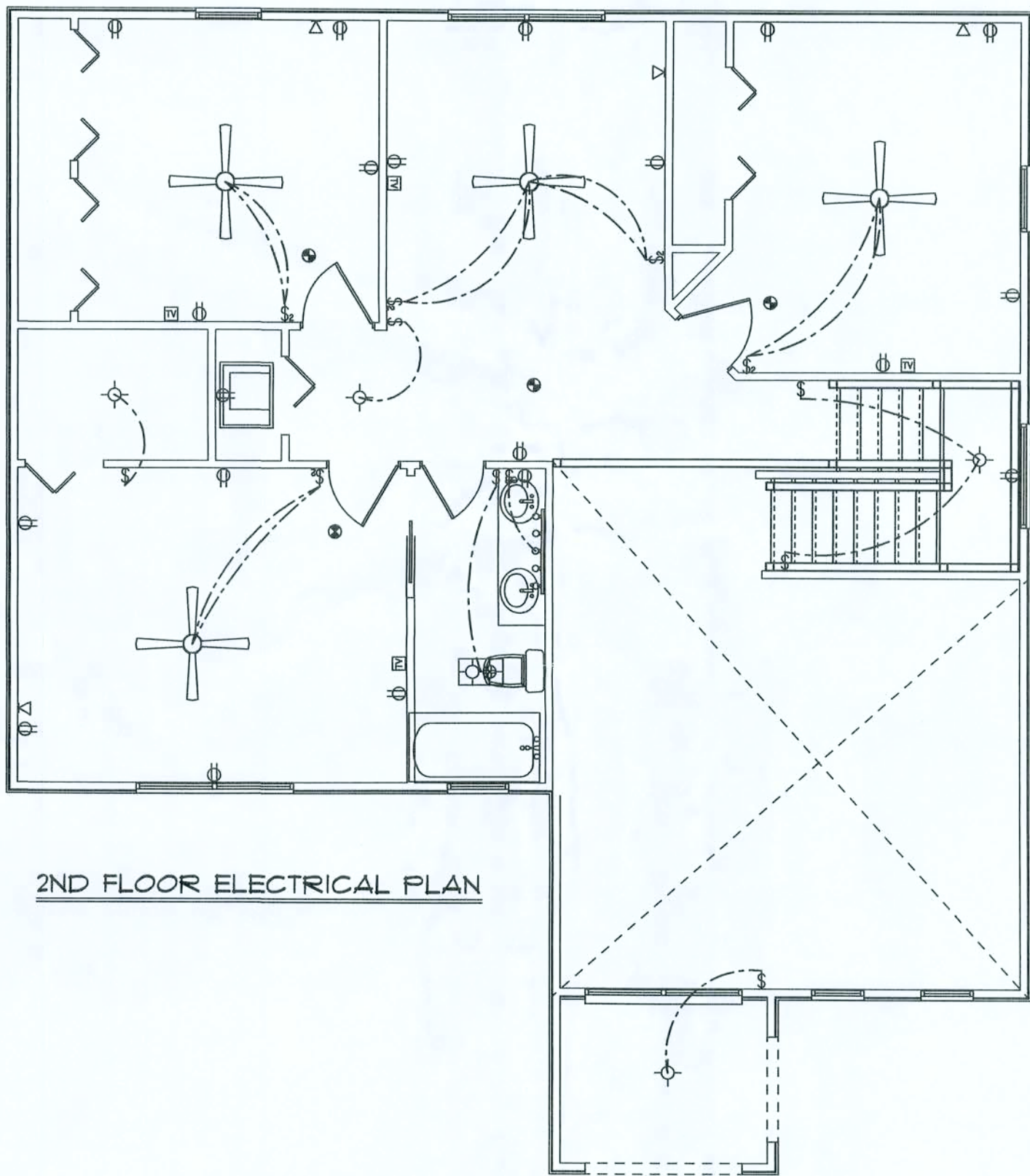
REVISIONS	



1ST FLOOR ELECTRICAL PLAN

ELECTRICAL	COUNT	SYMBOL
ceiling fan globe 1	2	
ceiling light vent square	2	
chandelier	2	
fluorescent fixture	1	
pot light	14	
vanity bar light	1	
wall mount 1	4	
wall mount 2	4	
electrical panel	2	
cable tv outlet	2	
dimmer switch	4	
light	8	
outlet	27	
outlet gfi	23	
smoke detector	2	
switch	22	
switch double	2	
telephone	2	

ELECTRICAL	COUNT	SYMBOL
ceiling fan globe 1	4	
vanity bar light	1	
Fan Light - bathroom	1	
cable tv outlet	4	
light	4	
outlet	19	
outlet gfi	1	
smoke detector	4	
switch	7	
switch double	5	
telephone	4	



2ND FLOOR ELECTRICAL PLAN

Lipscomb Eagle
Development, Inc.

Mediterranean Model

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Lot 4 Preserve S/D
Columbia County, Florida

Lipscomb Eagle
Development, Inc.
872 SW Jaguar Drive
Lake City, Florida 32025
Phone: (386) 719 - 6960
Fax: (386) 719 - 9586

PRINTED DATE:
December 14, 2007

DRAWN BY: T.J. Clanton
CHECKED BY: SUSAN HOLTON

DESIGNED BY:

FINALS DATE:
14 / Dec / 07

JOE NUMBER:

DRAWING NUMBER

A-3
3 OF 3 SHEETS

REVISIONS

SOFTPLAN
 ARCHITECTURAL DESIGN SOFTWARE

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 335	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6 -10d, 1 1/2"
< 1240	< 1065	SPH4			10 -10d, 1 1/2"
< 885	< 760	SP6			6 -10d, 1 1/2"
< 1240	< 1065	SPH6			10 -10d, 1 1/2"
< 1235	< 1165	LSTA18	14 -10d		
< 1235	< 1235	LSTA21	16 -10d		
< 1030	< 1030	CS20	18 -8d		
< 1705	< 1705	CS16	28 -8d		
STUD ANCHORS*					
< 1350	< 1305	LTT19	5 -16d		12" AB
< 2310	< 2310	LTT31	18 -10d, 1 1/2"		12" AB
< 2775	< 2570	H20A	2 -5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 -16d		5/8" AB
< 1400	< 1400	PAH242	16 -16d		
< 3335	< 3335	HPAH222	16 -16d		
< 2200	< 2200	ABU44	12 -16d		12" AB
< 2300	< 2300	ABU66	12 -16d		12" AB
< 2320	< 2320	ABU88	18 -16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS 700 LB EACH END. CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 8" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A188, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTOFFS TO BE 12FT. DO NOT CUT W.W.R. OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI. ALL LAP SPICES 4" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, $F_b = 2400$ PSI, $E = 1800000$ UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (13d), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE OR 12" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 9/64"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY SPECIAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
2.1A	Compressive strength	8" block bearing walls $F_m = 1500$ psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, $F_y = 60$ ksi, Lap splices min 48 bar dia, (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class GR60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

INTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"

2 STORY INTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"

ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

TWO STORY WALL SECTION

SCALE: 3/4" = 1'-0"

INTERIOR SHEAR WALL DETAIL

SCALE: 1/2" = 1'-0"

GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.

GRADE & SPECIES TABLE

	Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200
2x10	SYP #2	1050
2x12	SYP #2	975
GLB	24F-V3 SP	2400
LSL	TIMBERSTRAND	1700
LVL	MICROLAM	1600
PSL	PARALAM	2900

CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.

SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.

BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.

BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

SCALE: N.T.S.

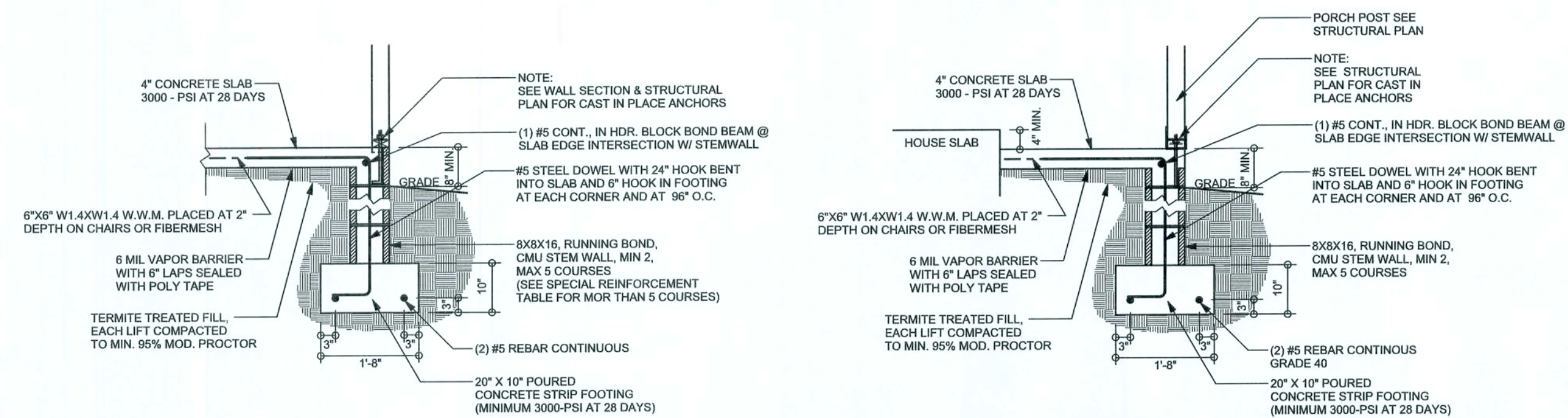
TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

SUPPORTIVE POST TO BEAM DETAIL FOR DOUBLE BEAM

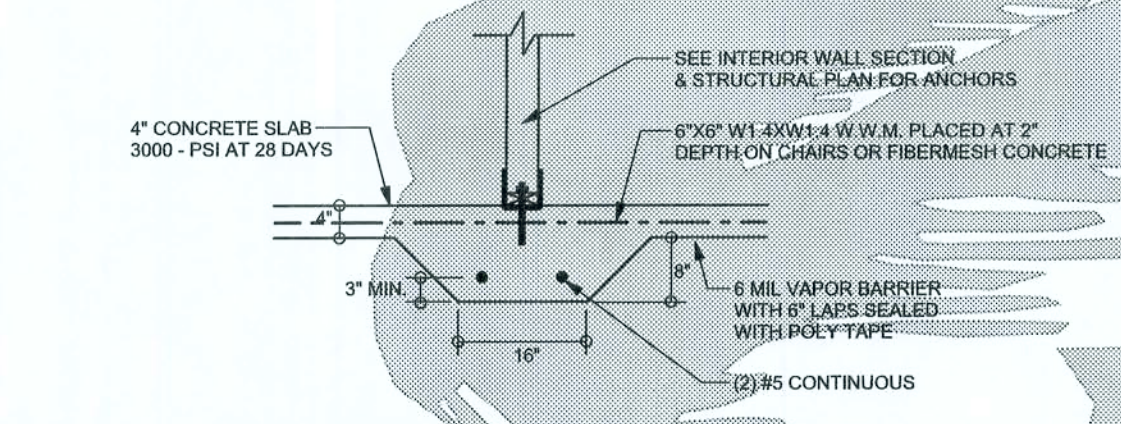
REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

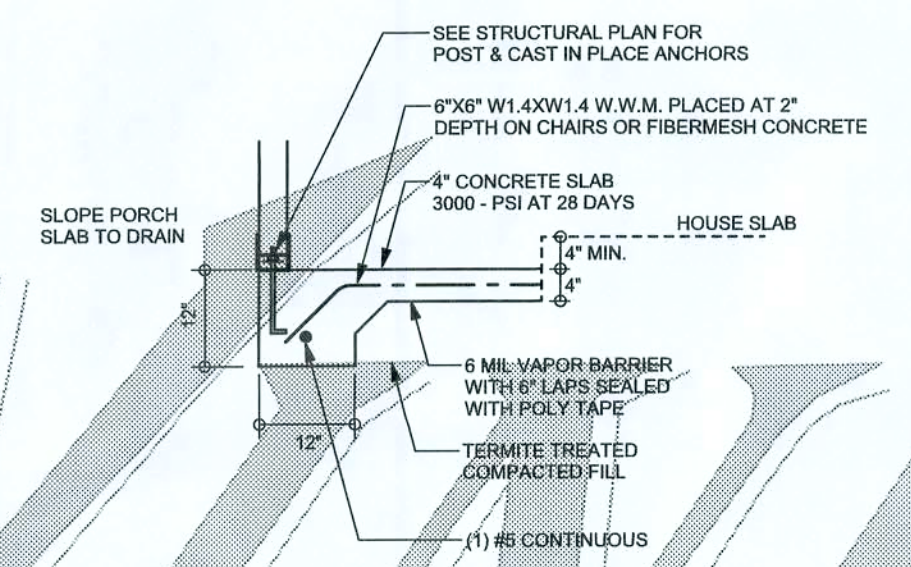


F9 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

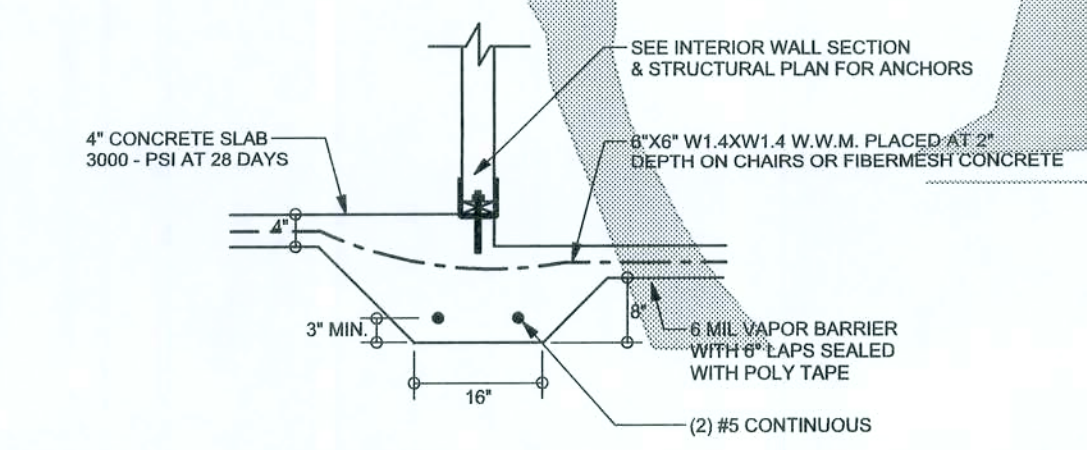
F12 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



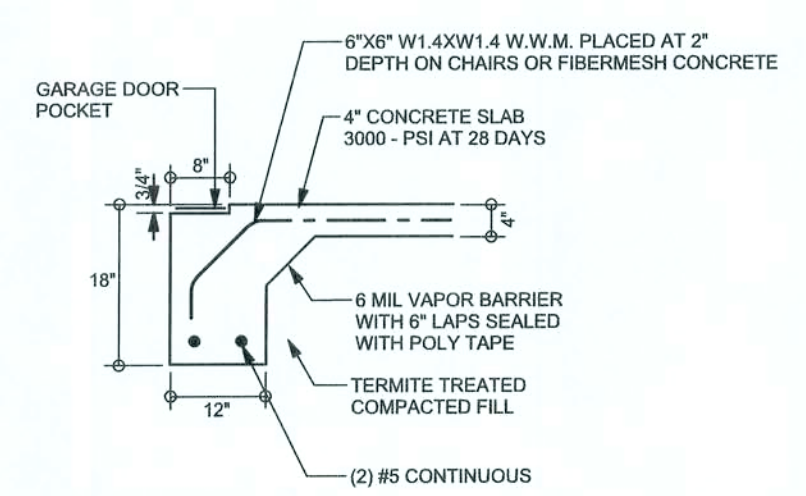
F2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

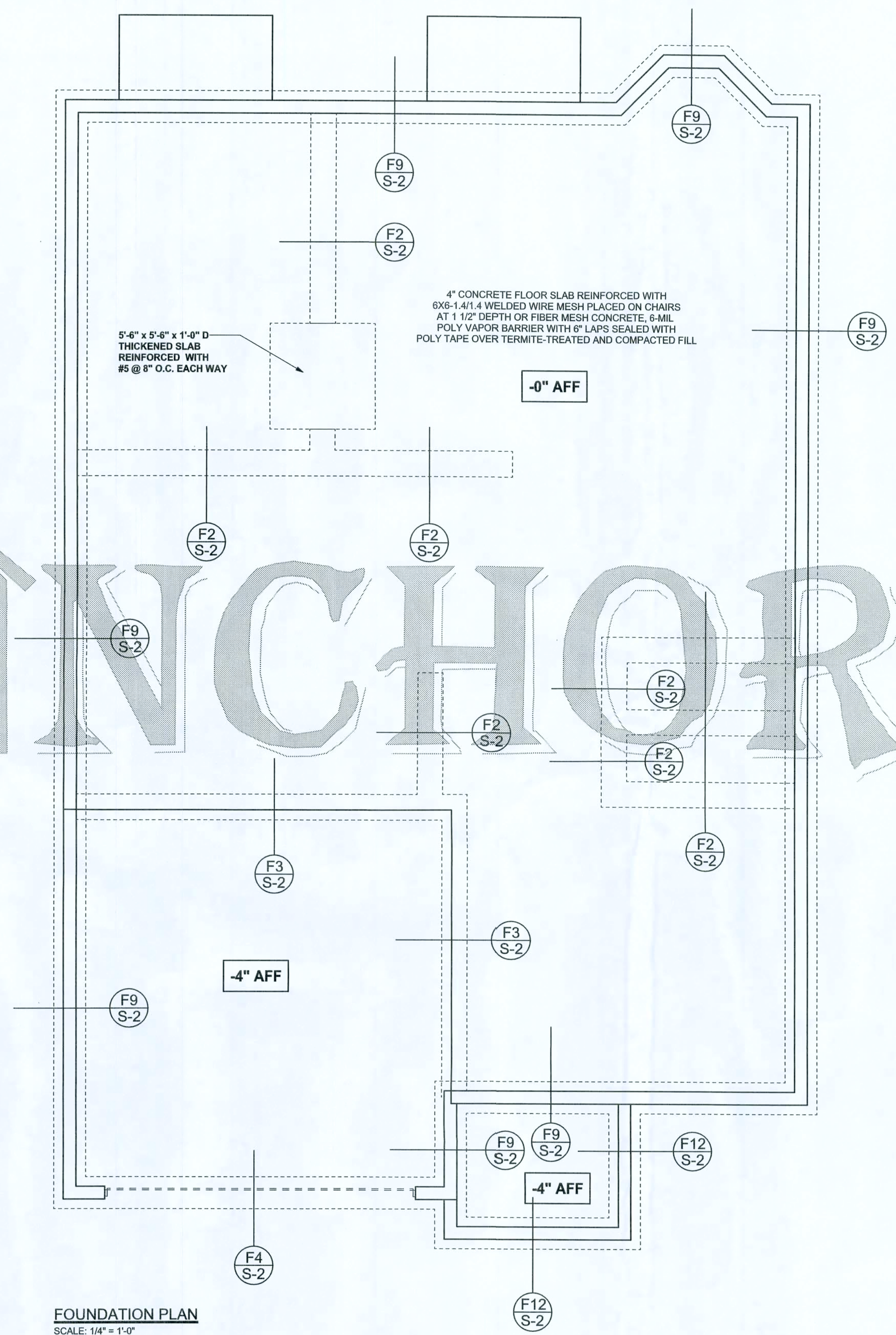


F4 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 185 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway,
PE No. 53915, PG 988, Lake City, FL
32056, 386-754-4119

DIMENSIONS:
Sited dimension supercede scaled
dimensions. Rule all questions to
Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section FDOT 2.1, Florida building
code residential 204, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOSWAY
P.E. 53915
Mark Disosway
14 DEC 07
SEAL

Lipscomb Eagle
Development

Mediterranean Model

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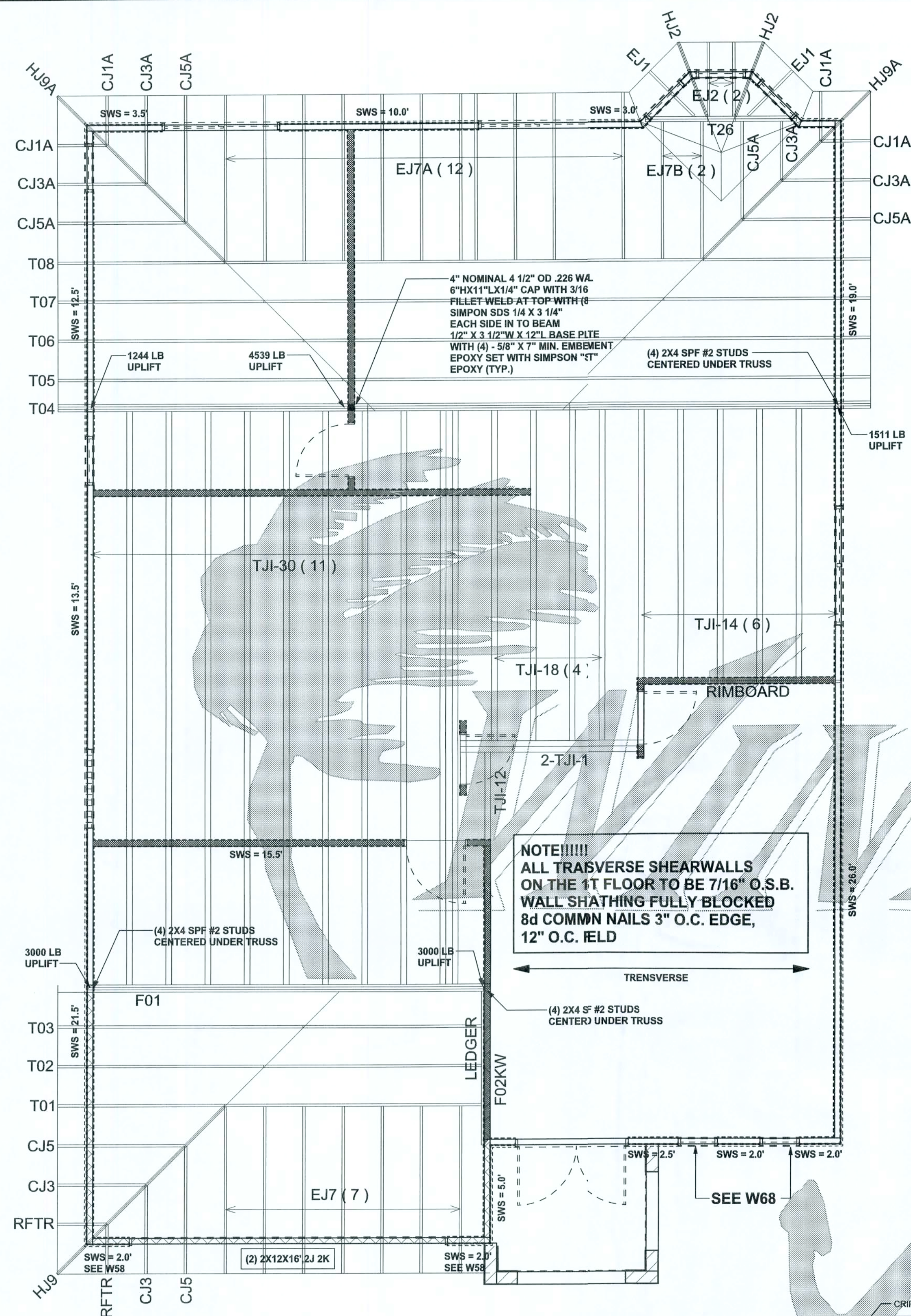
PRINTED DATE:
December 14, 2007
DRAWN BY: David Disosway
CHECKED BY:

FINALS DATE:
14 / Dec / 07

JOB NUMBER:
711294

DRAWING NUMBER
S-2
OF 3 SHEETS

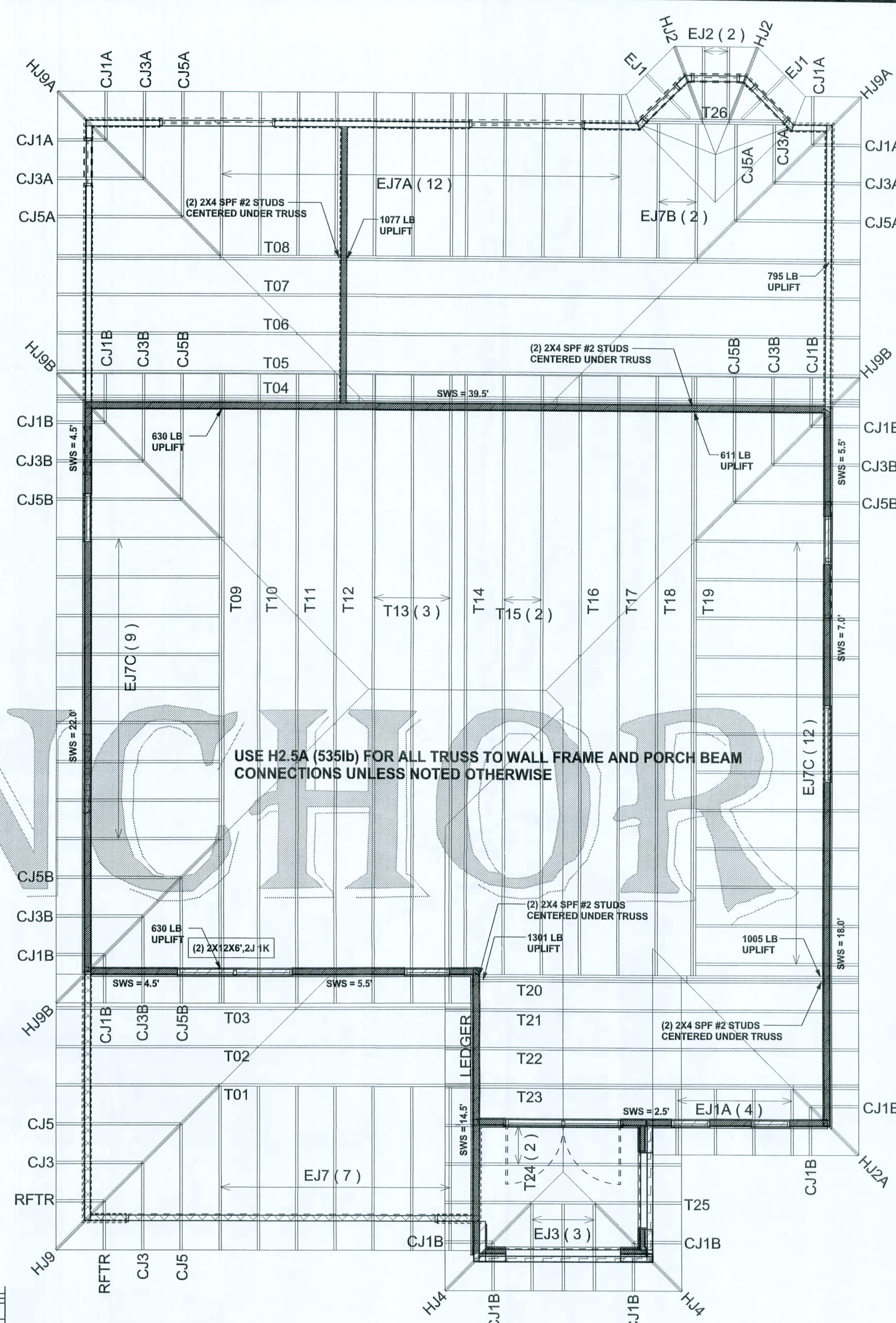
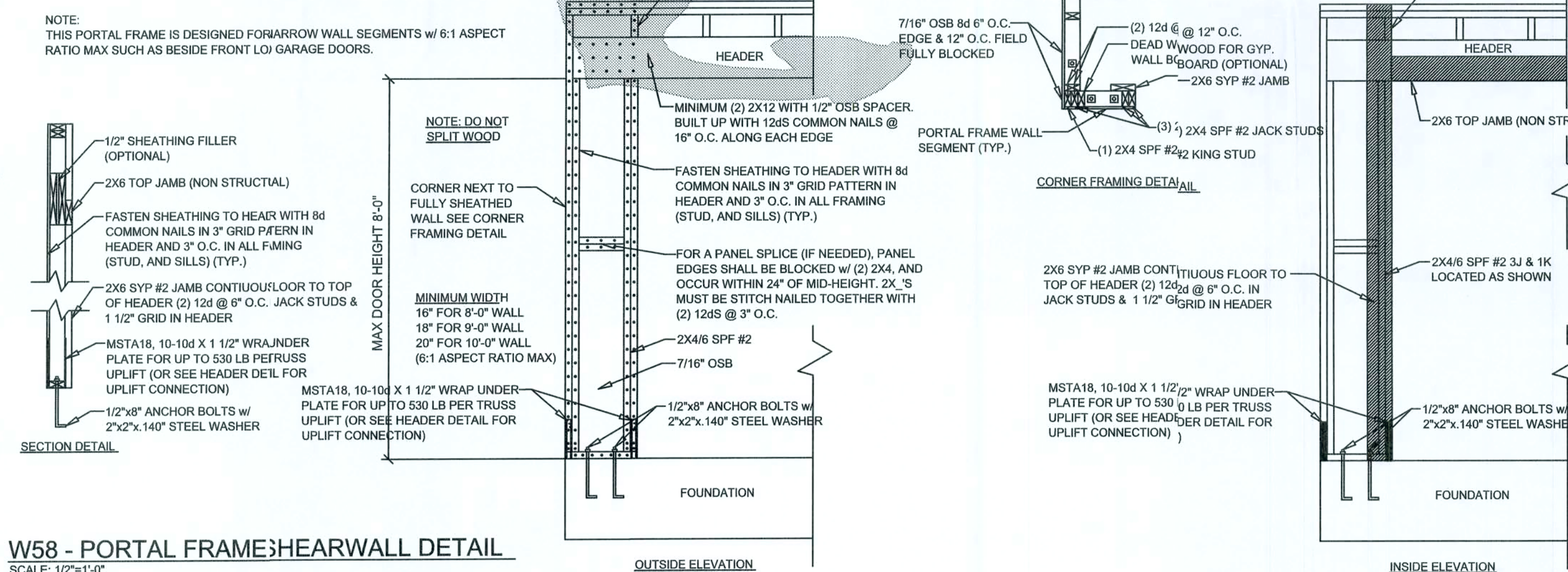
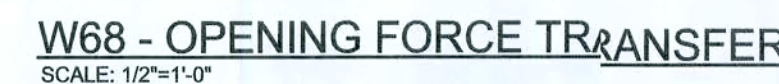
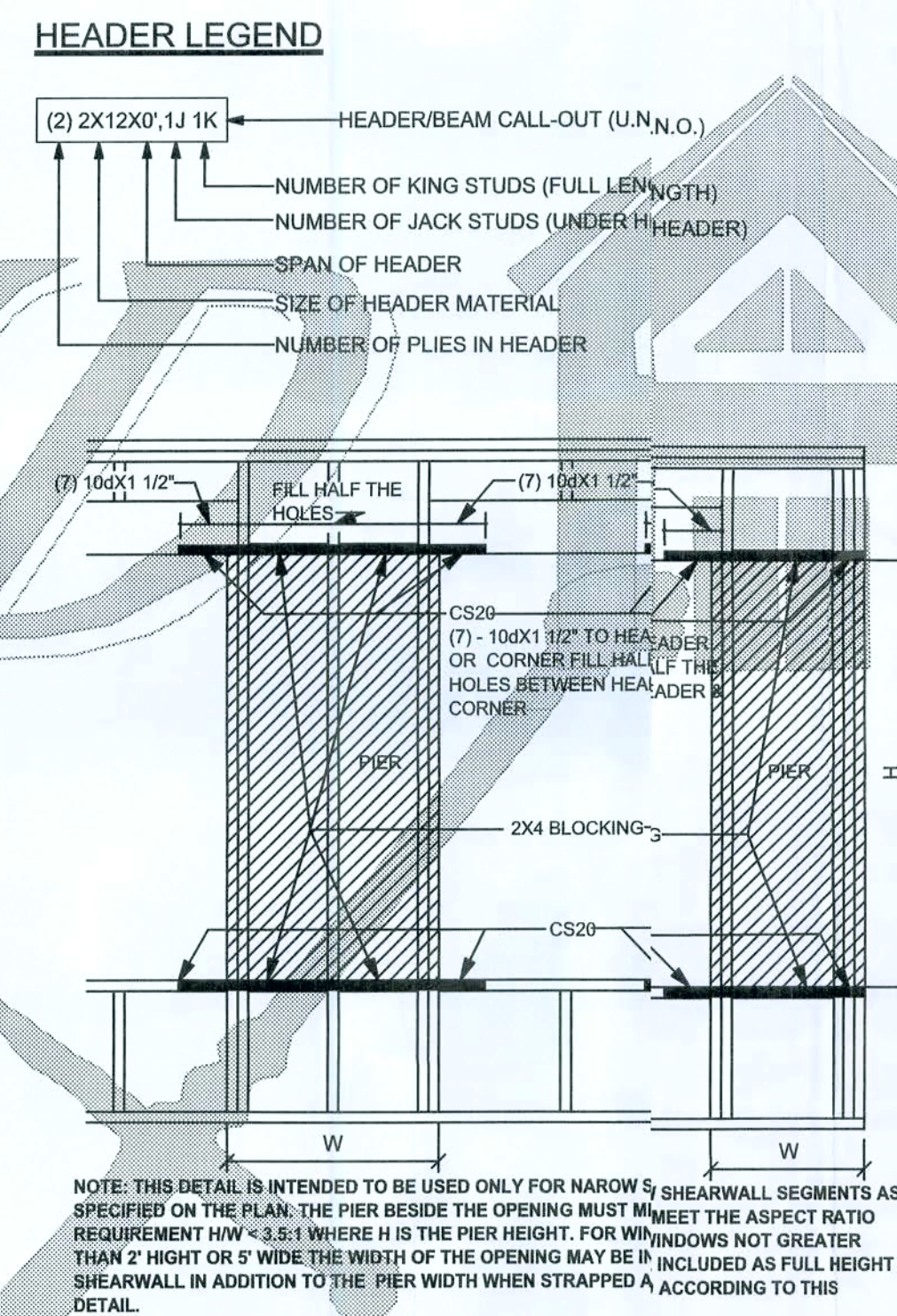
REVISIONS	



STRUCTURAL PLAN NOTES

- | | |
|------|--|
| SN-1 | ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.) |
| SN-2 | ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.) |
| SN-3 | DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS |
| SN-4 | PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS1H-3, BCS1H-8, BCS1H-8, BCS1H-8, BCS1H-8, & BCS1H-8 FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE |

WALL LEGEND	
SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL



2ND FLOOR TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	26.8'	52.0'
LONGITUDINAL	15.4'	71.5'

WINDLOAD ENGINEER: Mark Disoway
PE No. 53915, POE868, Lake City, FL
32056, 386-754-549

DIMENSIONS:
Stated dimensions upercede scaled
dimensions. Refer ll questions to
Mark Disoway, P.E. for resolution.
Do not proceed w/out clarification.

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CERTIFICATION: hereby certify that I have examined this plan and that the applicable portions of the plan relating to wind engineering comply with section R301.2.1, Florida building code residential 204, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY

MARK DISOSW

Lipscomb Eagle
Development

Mediterranean Model

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PRINTED DATE:
December 14, 2007

DRAWN BY: David Disosway	CHECKED BY:
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FINALS DATE	14 / Dec / 07
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JOB NUMBER
711294

DRAWING NUMBER

S-3

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. BUILDERS FIRST SOURCE
JOB #L235554