

UNION
L A S T E E L

METAL BUILDINGS

PO BOX 537 Lake Bulter, FL 32054
(386) 496-0681 (386) 496-4705 (fax)

BUILDING LOADS / DESCRIPTION:

WIDTH: 30 LENGTH: 40 HEIGHT: 10 /10
(BUILDING DIMENSIONS ARE NOMINAL. REFER TO PLANS).

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED
AND APPLIED AS REQUIRED BY : 2020 FBC

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY
WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD:	2.000	PSF (ROOF PANELS & PURLINS)		
COLLATERAL LOAD:	1	PSF	SNOW EXPOSURE:	1.0000
ROOF LIVE LOAD:	20.00	PSF	WIND EXPOSURE:	B
ROOF SNOW LOAD:	0	PSF	INTERNAL PRESSURE COEFF.:	
GROUND SNOW LOAD:	0	PSF	0.18 / -0.18	
BASIC WIND SPEED:	120	MPH	SPECTRAL RESPONSE COEFF.	MAPPED SPECTRAL RESPONSE ACC.
SEISMIC ZONE:	B	Sds	0.10	Ss 0.10
THERMAL FACTOR:	1.00	Sd1	0.09	St 0.05
IMPORTANCE FACTORS:		DESIGN BASE SHEAR, V:		

WIND LOAD	1.00	EXPANDED FORMULA	0.667*Ie*Fo*Ss*W/R
SNOW LOAD	1.0000	LONGITUDINAL	0.30
SEISMIC LOAD	1.00	TRANSVERSE	0.34

GENERAL NOTES:

- 1) MATERIALS : MINIMUM YIELD:
- | | |
|------------------------|-----------------------|
| HOT ROLLED BAR | Fy = 50.0000 ksi MIN. |
| STRUCTURAL STEEL SHEET | Fy = 55.0000 ksi MIN. |
| STRUCTURAL STEEL PLATE | Fy = 50.0000 ksi MIN. |
| COLD FORMED SHAPES | Fy = 57.0000 ksi MIN. |
| WALL SHEETING | Fy = 60.0000 ksi MIN. |
| ROOF SHEETING | Fy = 60.0000 ksi MIN. |
| BOLTS | A307 & A325 |
- THE METAL BUILDING MANUFACTURER RESERVES THE RIGHT TO
SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.

- 2) BOLT TIGHTENING REQUIREMENTS:
- ALL HIGH STRENGTH BOLTS ARE A325 UNLESS NOTED OTHERWISE.
HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE TURN OF THE NUT METHOD
IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR
STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". A325 BOLTS SHALL BE
INSTALLED WITH OUT WASHERS WHEN TIGHTENED BY THE "TURN OF THE NUT"
METHOD. ALL BOLTED CONNECTIONS, FOR SHEAR/BEARING CONNECTION TYPE
WITH BOLT THREADS EXCLUDED FROM THE SHEAR PLANE SHALL BE SNUG TIGHT
ONLY.

- 3) ALL STRUCTUAL STEEL TO RECEIVE A RUST INHIBITIVE PRIMER. THIS PAINT
IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.



ROOF PANELS:

COLOR: Galvalume Plus

WALL PANELS:

COLOR: Need Sill Poly Color

TRIM COLORS:

CABLE: Need Sill Poly Color

CORNER: Need Sill Poly Color

EAVE: Need Sill Poly Color

FRAMED OPENINGS: Need Sill Poly Color

LINER PANELS:

COLOR: N/A

LINER TRIM:

COLOR: N/A



DEFLECTION LIMTS:

EW COL:	180
EW RAF LIVE:	180
EW RAF WIND:	180
WALL GIRT:	90
PURL LIVE:	180
PURL WIND:	150
WALL PANEL:	60
ROOF PANEL LIVE:	60
ROOF PANEL WIND:	60
RF HORIZONTAL:	60
RF VERTICAL:	180
WIND BENT:	60
RF CRANE:	60
RF SEIS:	50
WIND BENT SEIS:	50

BUILDER / CONTRACTOR RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUILDER/CONTRACTOR TO INSURE THAT ALL PROJECT
PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY
GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND
DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN
AGREEMENT THAT THE METAL BUILDING SYSTEM MANUFACTURER OR ITS DESIGN ENGINEER IS
ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION
PROJECT.

THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE
APPROPRIATE AGENCY AS REQUIRED. APPROVAL OF THE METAL BUILDING SYSTEM
MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE METAL BUILDING
SYSTEM MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE
CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.2.1 AISC CODE OF STANDARD
PRACTICES, 9TH ED.) WHERE DISCREPANCIES EXIST BETWEEN THE METAL BUILDING SYSTEM
MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE
STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE
9TH ED.)

DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT
FURNISHED BY THE METAL BUILDING SYSTEM MANUFACTURER ARE THE RESPONSIBILITY OF
THE CONTRACTORS AND ENGINEERS OTHER THAN THE METAL BUILDING SYSTEM
MANUFACTURER'S ENGINEER UNLESS SPECIFICALLY INDICATED.

THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK
IN COMPLIANCE WITH THE METAL BUILDING SYSTEM MANUFACTURER "FOR CONSTRUCTION"
DRAWINGS.

ALL BRACING AS SHOWN AND PROVIDED BY THE METAL BUILDING SYSTEM MANUFACTURER
FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A
PERMANENT PART OF THE STRUCTURE.

TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR
OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND
FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE
THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS
COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING
FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING
FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH
UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION, OR COLLISION. (SECT. 7.9.1
AISC CODE OF STANDARD PRACTICE, 9TH ED.)

WARNING: IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION
WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSION EFFECTS ON
THE ALUMINUM ZINC ALLOY COATING WHEN THEY ARE USED IN CONTACT WITH GALVALUME
STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO
GALVALUME SHOULD BE AVOIDED.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS
APPROVAL DRAWINGS: IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS BE MADE
IN CONTRASTING INK (PREFERABLY RED INK), HAVE ALL INSTANCES OF CHANGE CLEARLY
INDICATED, AND BE LEGIBLE AND UNAMBIGUOUS. A SIGNATURE AND DATE IS REQUIRED ON
ALL PAGES. MANUFACTURER RESERVES THE RIGHT TO RE-SUBMIT DRAWINGS WITH EXTENSIVE
OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE
DELIVERY SCHEDULE. APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE
METAL BUILDING SYSTEM MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT
REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN WITH
INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY
MANUFACTURER. ANY CHANGES NOTED ON THHE DRAWINGS NOT IN COMFORMANCE WITH THE
TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER
ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED
AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION.
MANUFACTURER RECONGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING
APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED.
HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL
TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILIAR INDICATION
OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS
BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS
UNACCEPTABLE ALTERNATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE
CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS
CUSTOMER.

IMPORTANT NOTE: FINAL DETAILING, FABRICATION, AND DELIVERY DATE OF THIS PROJECT
CANNOT BE COMPLETED UNTIL THE SIGNED APPROVALS ARE RETURNED TO THE METAL
BUILDING MANUFACTURER.

SHEET 1 OF 13

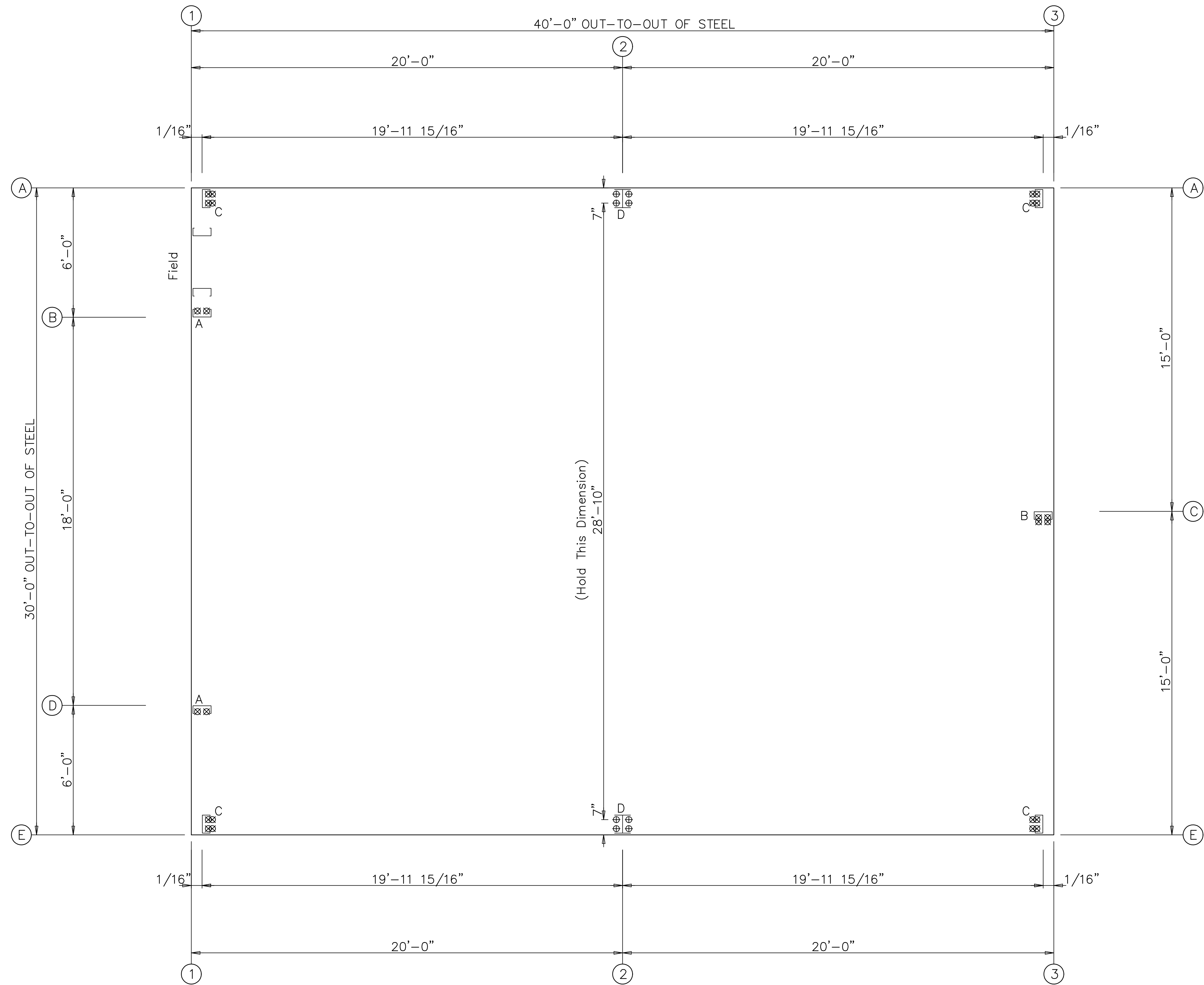
281 SW Stewart Loop
Lake City, Fl 32024

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△	08.20.21	FOR PERMIT
REV.	DATE	REVISION

PURCHASER: Deryl Williams

PROJECT: Shop

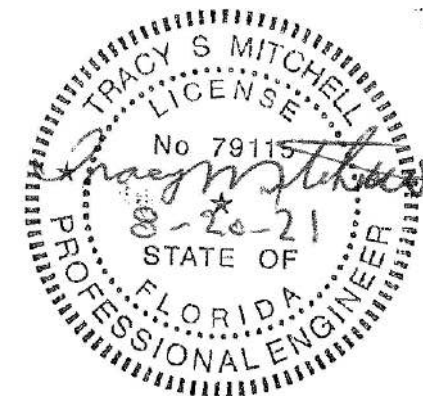
JOB NUMBER: DerylWilliams30x40



⊗ Dia= 5/8"

⊕ Dia= 3/4"

ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)



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AMERICAN STRUCTURAL GROUP
CONSULTING STRUCTURAL ENGINEERS
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PH 513-402-7600
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Union LaSteel				
PROJECT	Shop		ANCHOR BOLT PLAN	
ID	DerylWilliams30x40		DESIGN:	DRAFT: CHECK:
PROJECT ADDRESS	281 SW Stewart Loop Lake City, FL 32024		DATE: 7/28/21	SHEET 2 OF 13

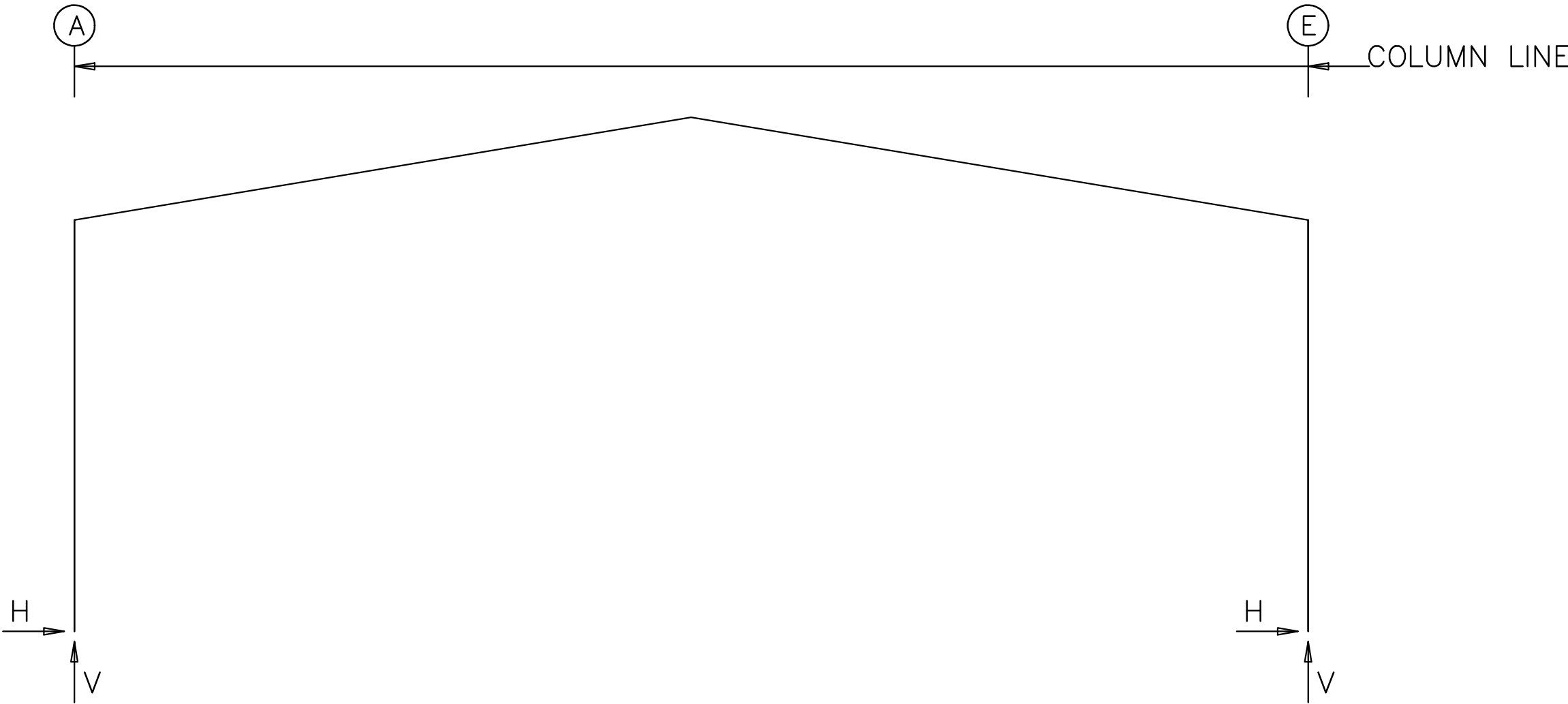
NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	30.0
Length (ft)	=	40.0
Eave Height (ft)	=	10.0/ 10.0
Roof Slope (rise/12)	=	2.0/ 2.0
Dead Load (psf)	=	2.0
Collateral Load (psf)	=	1.0
Roof Live Load (psf)	=	20.0
Frame Live Load (psf)	=	12.0
Wind Speed (mph)	=	120.0
Wind Code	=	FBC 17 (IBC 15)
Exposure	=	B
Closed/Open	=	C
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Zone	=	B
Seismic Coeff (Fa*Ss)	=	0.16

ID	Description
1	Dead+Collateral+Live
2	0.6Dead+0.6Wind_Left1
3	0.6Dead+0.6Wind_Right1
4	0.6Dead+0.6Wind_Left2
5	0.6Dead+0.6Wind_Right2
6	0.6Dead+0.6Wind_Long1L
7	0.6Dead+0.6Wind_Long2L
8	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
9	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
10	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
11	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L

FRAME LINES: 2



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty Dia		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
2	A	1	2.3	5.9	2	-2.3	-3.7	4	0.750	8.000	9.94	0.375	0.0
2	E	3 1	2.3 -2.3	-3.7 5.9	1 3	-2.3 2.3	5.9 -3.7	4	0.750	8.000	9.94	0.375	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	-----Dead-----		-----Collateral-----		-----Live-----		-----Wind_Left1-----		-Wind_Right1-		--Wind_Left2--	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2	A	0.4	1.1	0.2	0.4	1.8	4.5	-4.2	-7.2	0.3	-4.6	-4.1	-4.3
2	E	-0.4	1.1	-0.2	0.4	-1.8	4.5	-0.3	-4.6	4.2	-7.2	-0.4	-1.7

Frame Line	Column Line	-Wind_Right2-		--Wind_Long1-		--Wind_Long2-		-Seismic_Left		Seismic_Right	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2	A	0.4	-1.7	-0.4	-6.3	-0.9	-5.3	-0.1	-0.1	0.1	0.1
2	E	4.1	-4.3	0.9	-5.3	0.4	-6.3	-0.1	0.1	0.1	-0.1

ENDWALL COLUMN REACTIONS(k)

MAXIMUM VERTICAL Dead+Collateral+Live = 4.4
MAXIMUM VERTICAL Dead+Wind = -3.2
MAXIMUM HORIZONTAL Dead+Wind = 1.7

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
⊗ 24	Endwall	5/8"	A307	3.00
⊕ 8	Frame	3/4"	A307	3.00

BUILDING BRACING REACTIONS

---Wall Loc	---Col Line	± Reactions(k)				Panel_Shear (lb/ft)	
		---Wind---		---Seismic---		Wind	Seis
L_EW	1					114	27
F_SW	E					35	4
R_EW	3					34	8
B_SW	A					35	4



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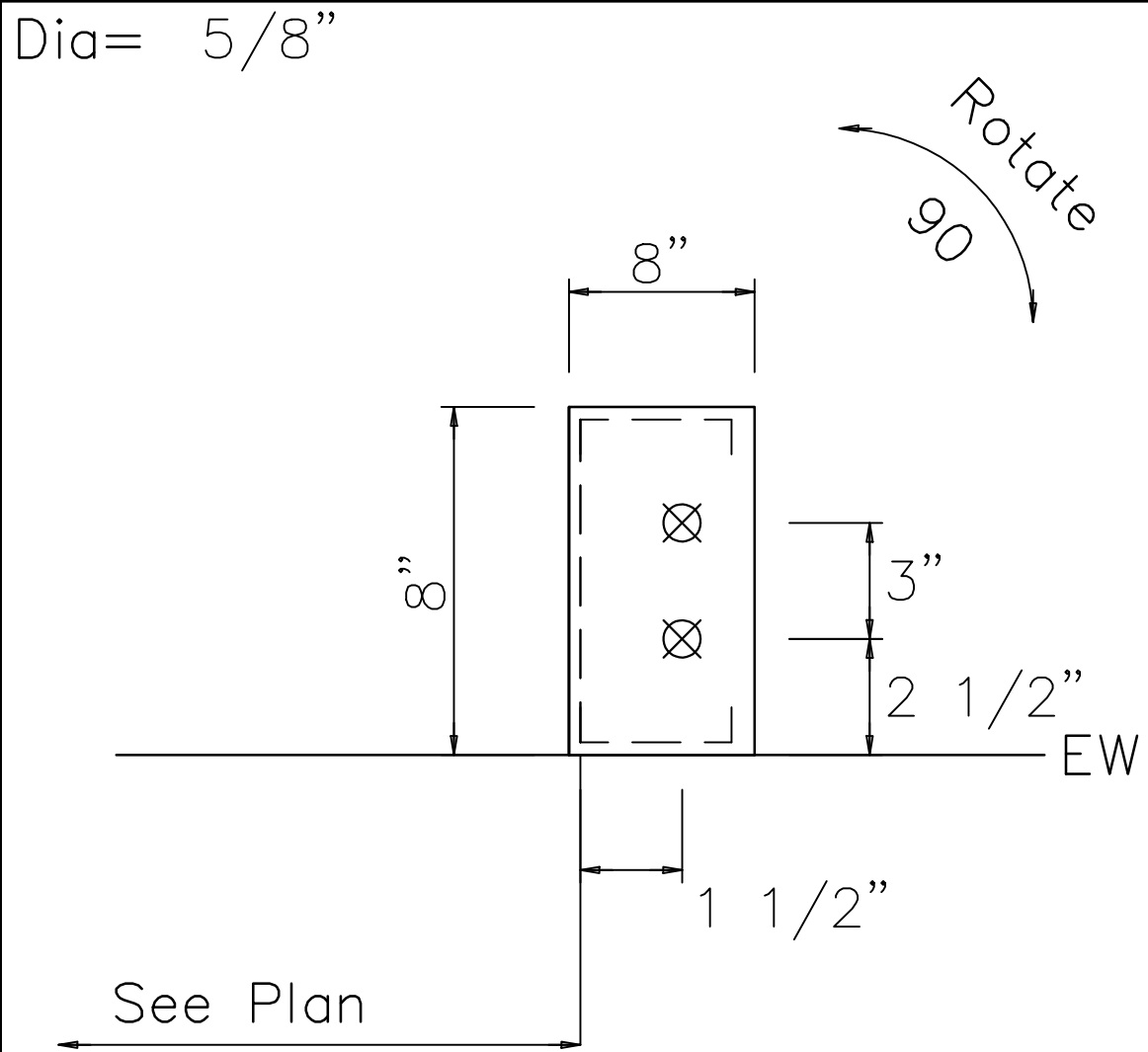
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CONSULTING STRUCTURAL ENGINEERS

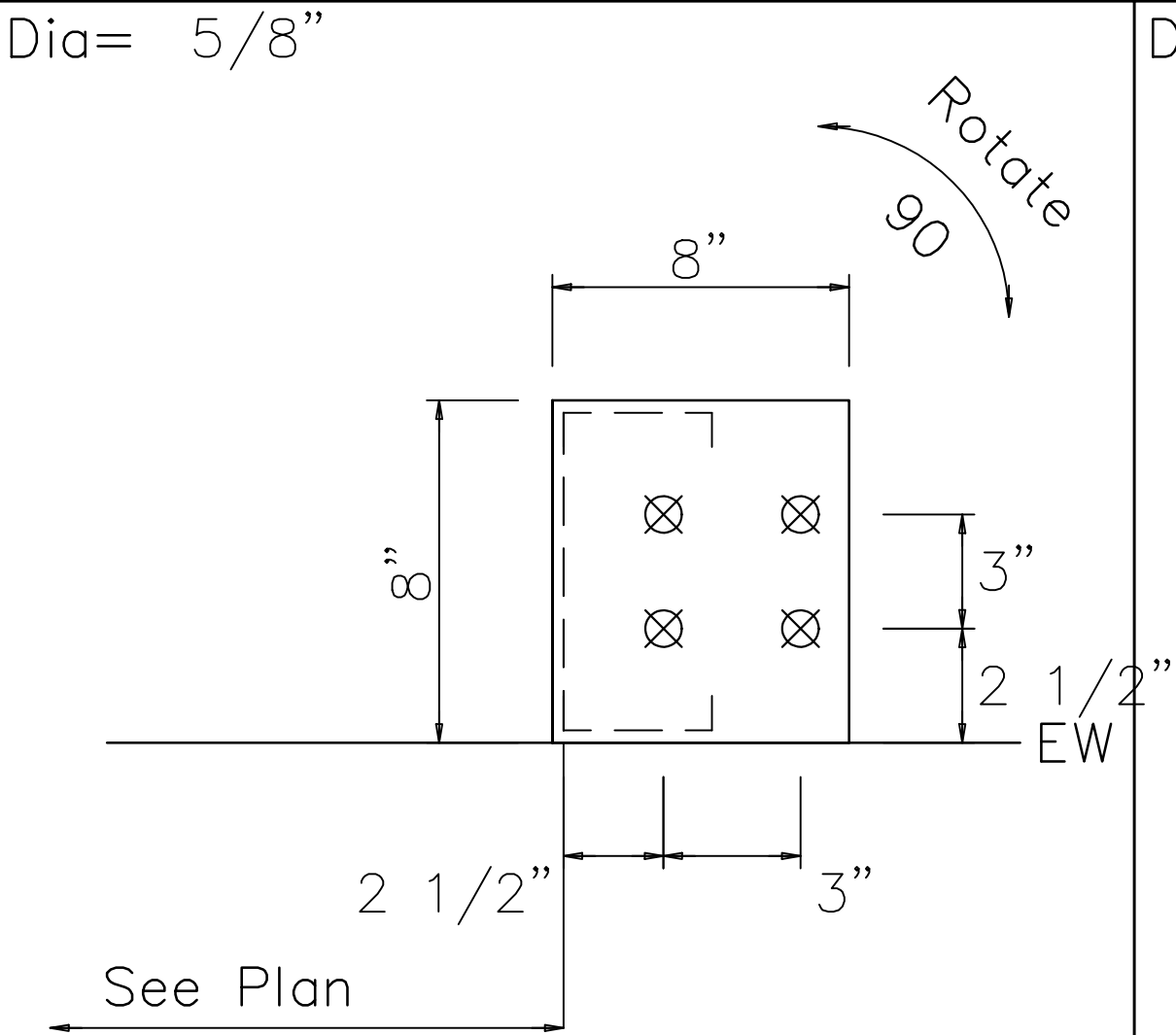
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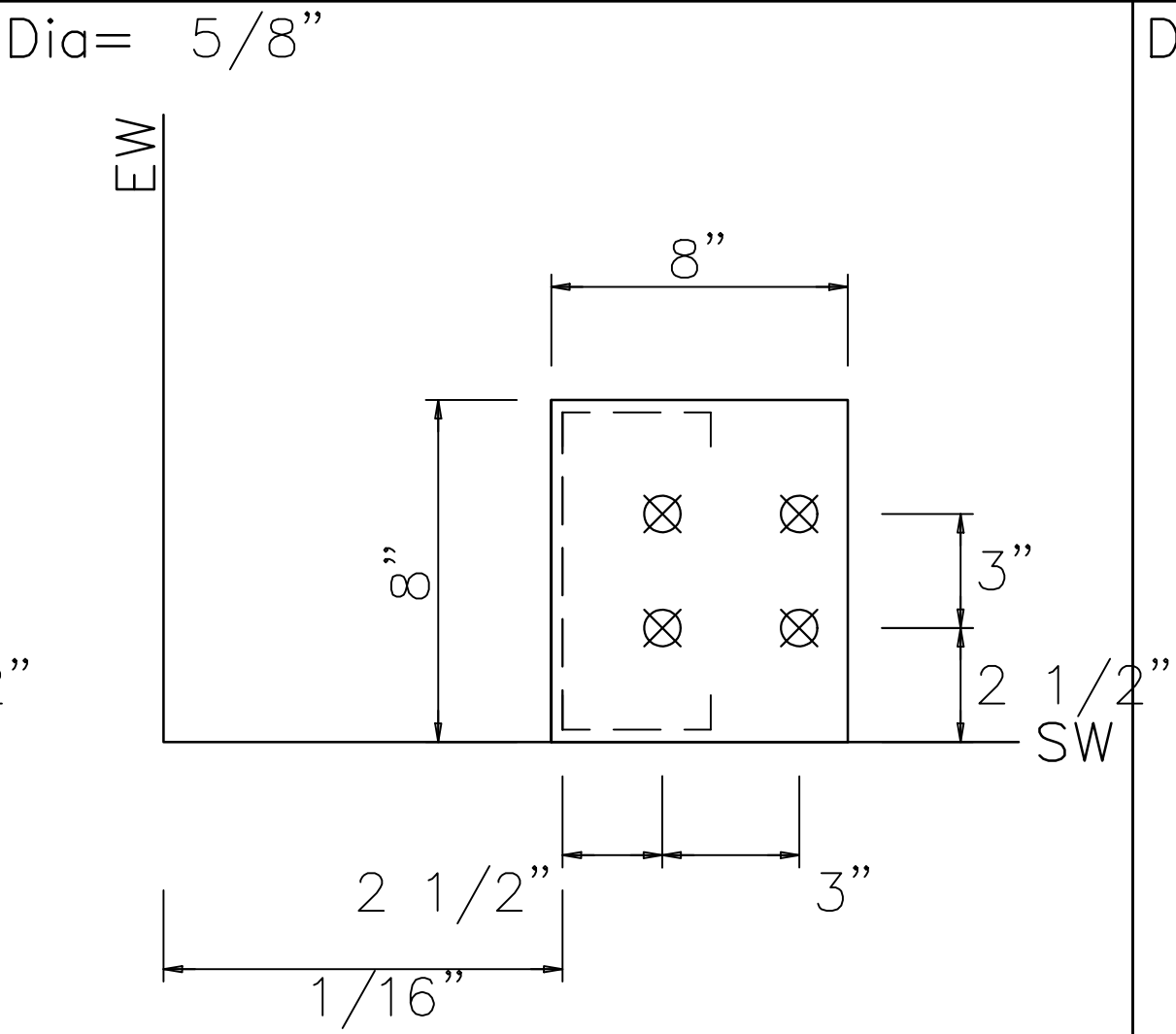
Union LaSteel					
PROJECT	Shop		ANCHOR BOLT REACTIONS		
ID	DerylWilliams30x40		DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	281 SW Stewart Loop Lake City, FL 32024		DATE: 7/28/21	SHEET 3 OF 13	



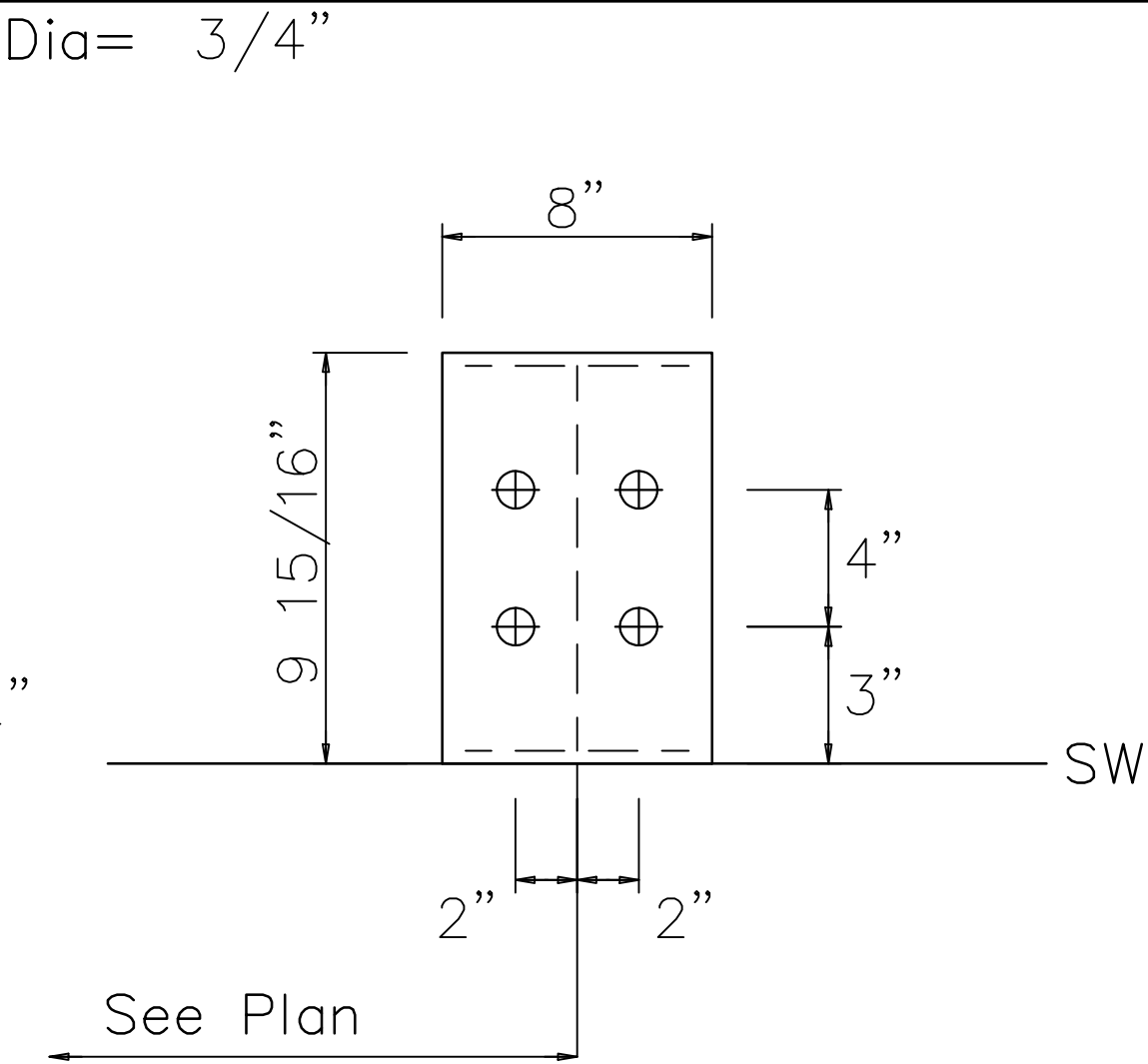
DETAIL A



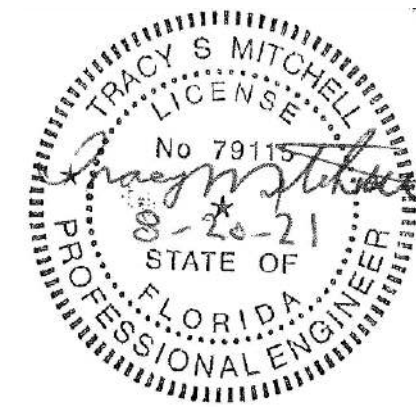
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DETAIL C



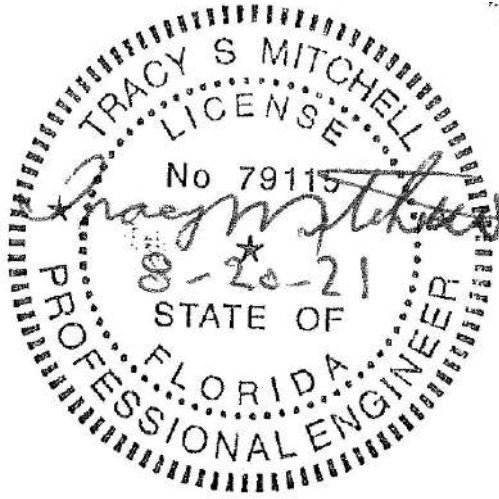
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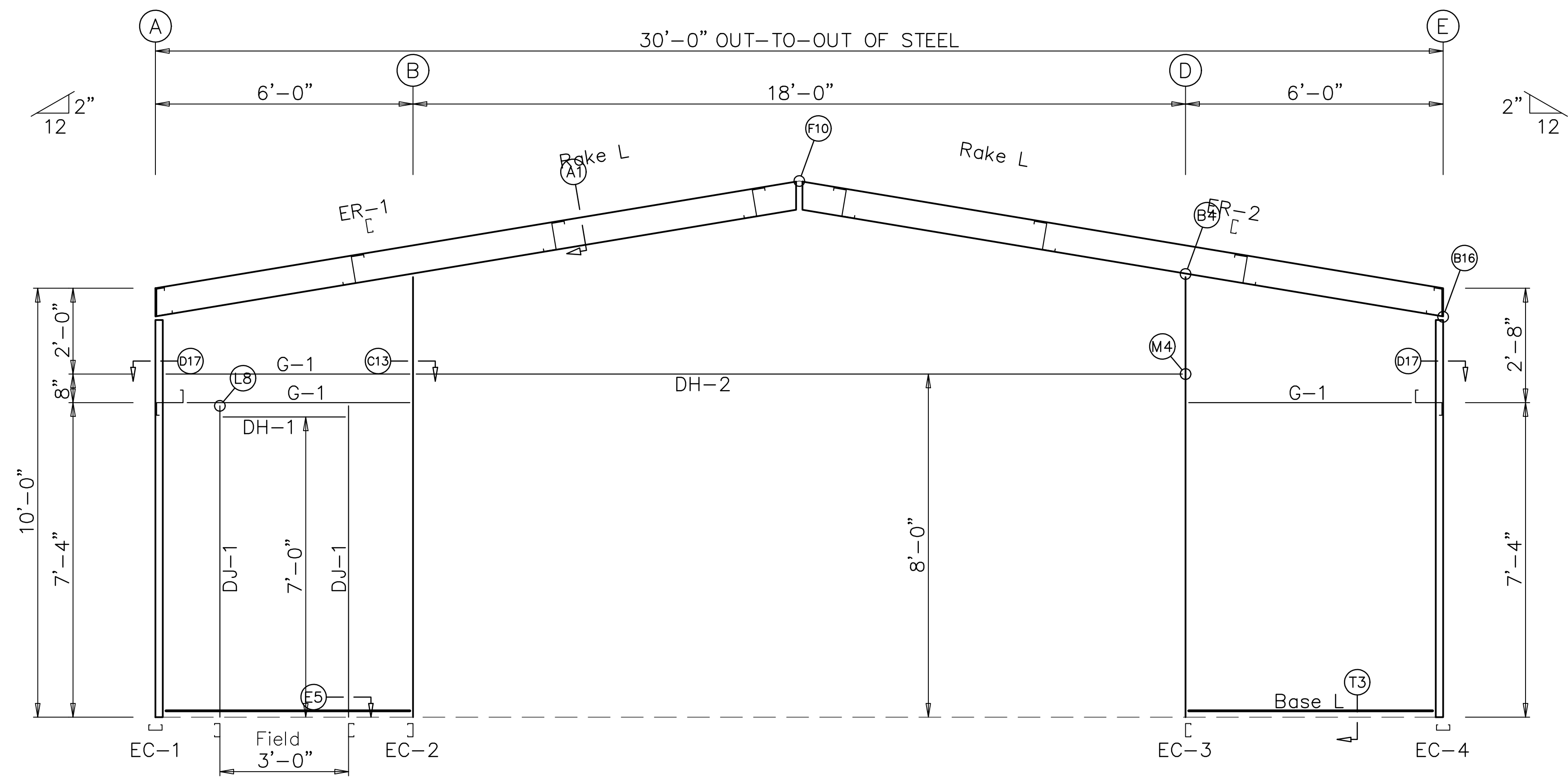
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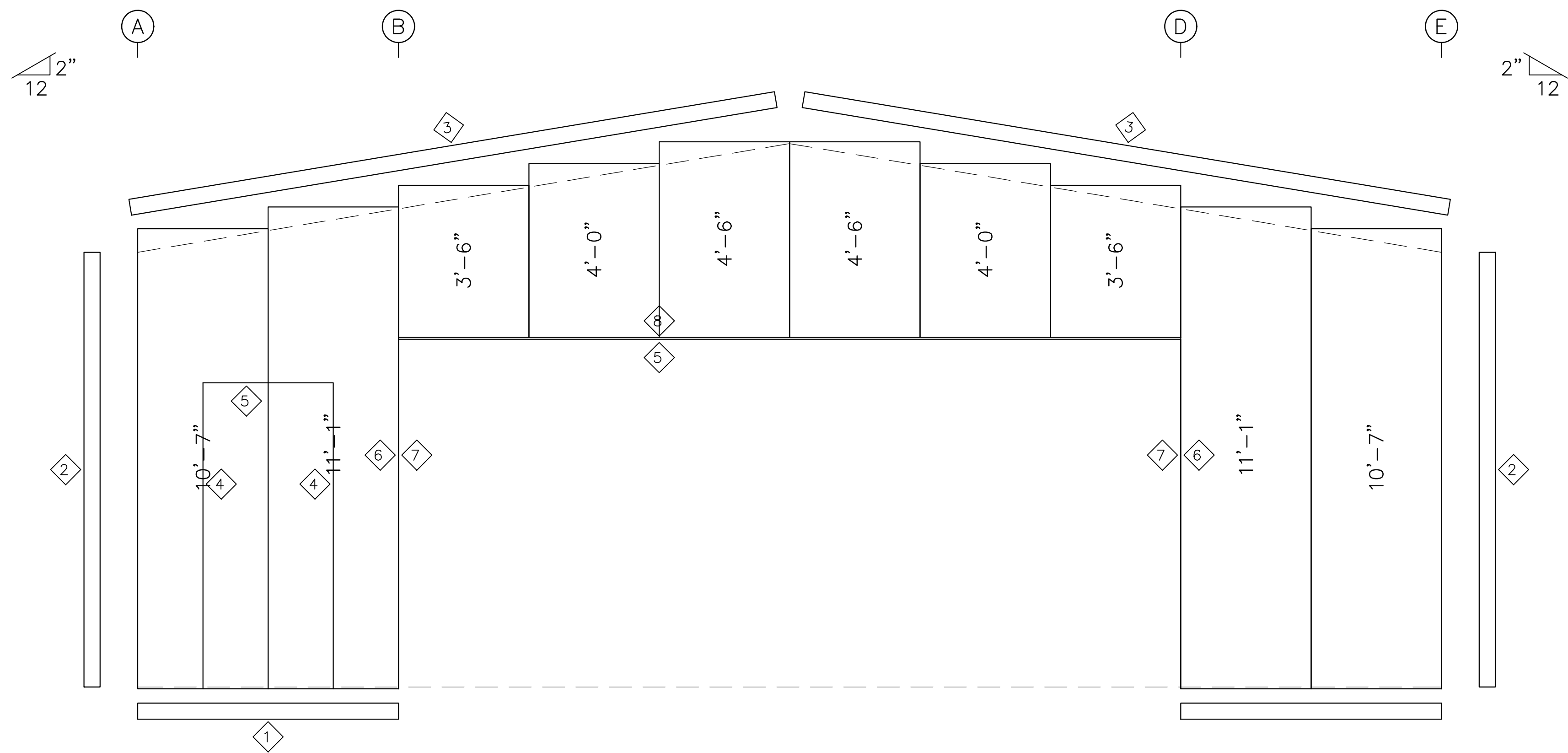
Door jamb 0.5" anchor bolts Sheeting notch Base angle Wall panel Member screw @ 12" o.c. JT-101 Field cut girt as required 0.5" 0.25" DOOR JAMB TRIM	END OF BUILDING GABLE RAKE ANGLE ROOF PURLIN #12 S. D. SCREW ENDWALL RAFTER BOLT, 0.5" BY 1" (TYP) 0.25" A1 SECTION THRU ENDWALL RAFTER	ENDWALL RAFTER BOLTS, 0.50" BY 1.50" ENDWALL COLUMN (ONE SHOWN, TWO MAY BE USED) B4 ENDWALL RAFTER TO COLUMN	ENDWALL RAFTER ENDWALL COLUMN CONNECTION PLATE SEE ENDWALL DRAWING FOR BOLT DIA AND TYPE. B16 CORNER COLUMN TO ENDWALL RAFTER	ENDWALL COLUMN BOLT, 0.5" BY 1.0" (TYP) WELDED PLATE ENDWALL GIRTS C4 ENDWALL COLUMN TO WALL GIRT	
WELDED PLATE BOLT, 0.5" BY 1.0" (TYP) ENDWALL COLUMN ENDWALL GIRT C13 ENDWALL COLUMN TO WALL GIRT	SIDEWALL GIRT W R COLUMN WELDED PLATE WELDED ANGLE ENDWALL GIRT D17 CORNER COLUMN TO WALL GIRT	ENDWALL COLUMN BASE PLATE PLAN ELEVATION E5 BASE PLATE TO ENDWALL COLUMN OR DOOR JAMB	ENDWALL RAFTER SEE ENDWALL DRAWING FOR BOLT DIA AND TYPE. F10 RAFTER SPLICE AT SURFACE CHANGE	ENDWALL RAFTER CONNECTION PLATE SEE ENDWALL DRAWING FOR BOLT DIA AND TYPE. F23 RAFTER SPLICE AT SURFACE CHANGE	(SEE ROOF PLAN) LAP 1/2" x 1 1/4" A307 HEX HD. (TYP.) 2 bolts std. ROOF PURLIN varies RIGID FRAME RAFTER RAFTER FLANGE BRACE 1 or 2 req., see RF Drawing G2 TYPICAL SECTION THROUGH PURLIN LAP
FRAME COLUMN COLUMN FLANGE BRACE WALL GIRT BOLTS, 0.5" x 1.0" (TYP) WELDED PLATE (FLANGE BRACES ONLY NEEDED WHEN SHOWN ON ELEV DWG) H6 WALL GIRT TO FRAME COLUMN	BOLT, TYP. 0.5" x 1.0" EAVE STRUT #12 S. D. SCREW GABLE RAKE ANGLE ENDWALL RAFTER I6 EAVE STRUT TO ENDWALL RAFTER	EAVE STRUT FRAME COLUMN BOLTS, 0.5" x 1.5" SECTION ELEVATION J1 EAVE STRUT TO RIGID FRAME	WALL GIRT DOOR JAMB BOLT, 0.5" x 1.5" SECTION ELEVATION L8 FRAMED OPENING TO WALL GIRT	WELDED PLATE HEADER BOLTS, 0.50" BY 1.0" (TYP.) SECTION ELEVATION M4 FRAMED OPENING HEADER TO JAMB	SIDEWALL COLUMN ANCHOR BOLTS BASE PLATE PLAN ELEVATION R2 ANCHOR BOLTS AT SIDEWALL COLUMN



Union LaSteel				
PROJECT	Shop	STRUCTURAL DETAILS		
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ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. SX – Need Sill Poly Color



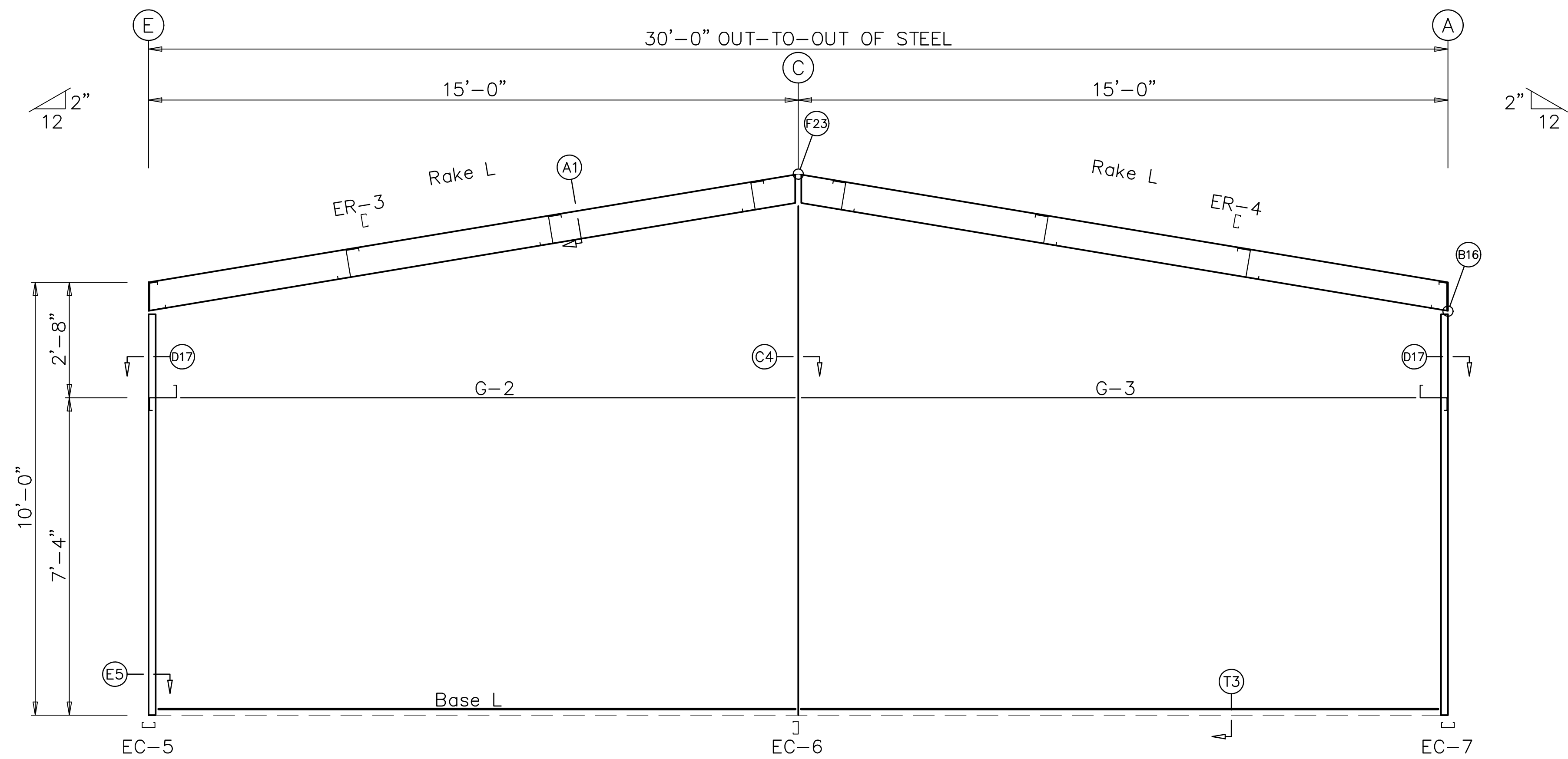
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BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	3	A325	1/2"	1 1/2"
Cor_Column/Raf	2	A325	5/8"	2"
Int_Column/Raf	2	A325	5/8"	1 1/2"

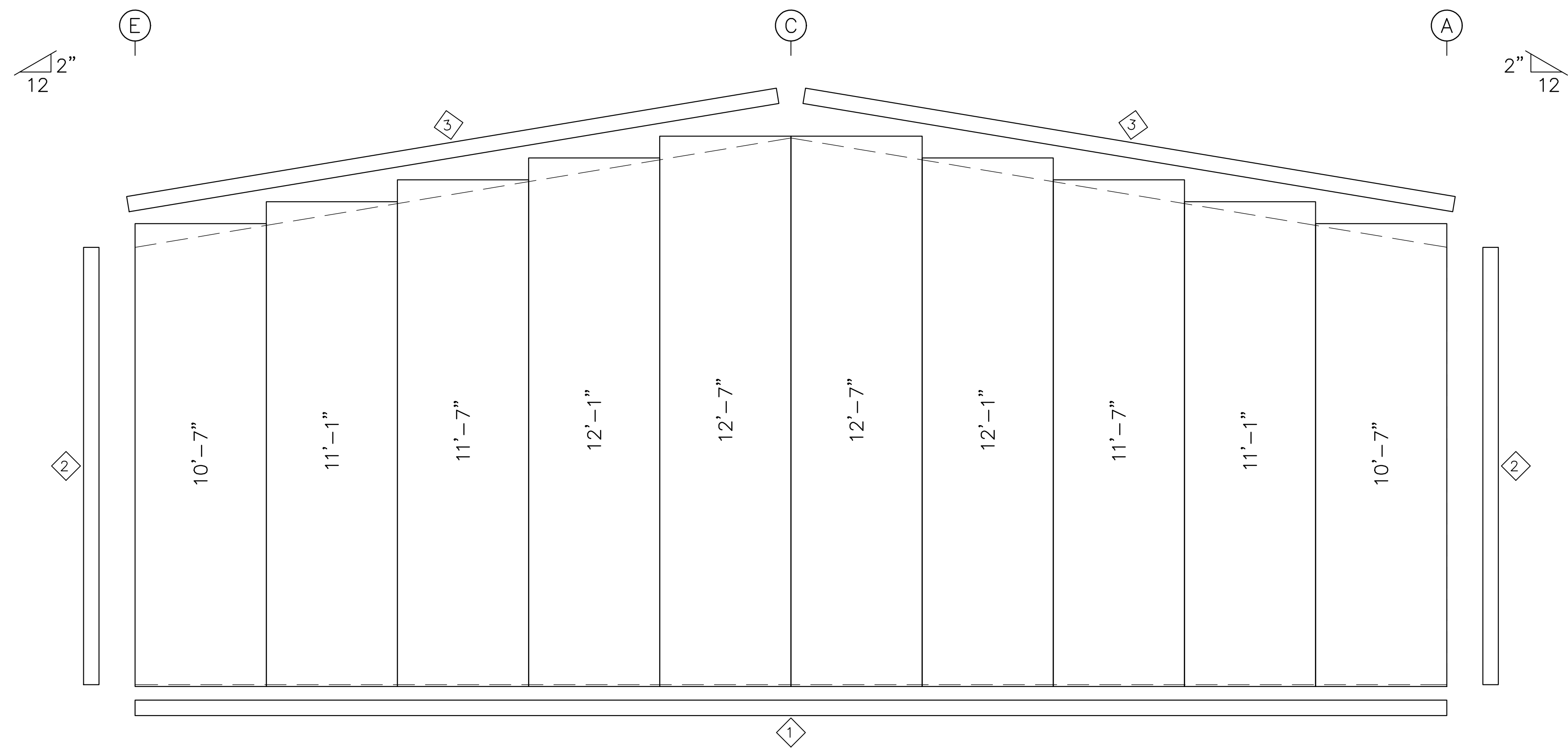
TRIM TABLE			
FRAME LINE 1			
ID	PART	LENGTH	DETAIL
1	BT-101	10'-3"	TRIM_68
2	CT-102	10'-3"	TRIM_323
3	RT-101	10'-3"	TRIM_229
4	JT-101	10'-3"	TRIM_401
5	HT-101	10'-3"	TRIM_232
6	MT-116B	10'-3"	TRIM_242
7	JT-101	10'-3"	TRIM_239
8	MT-116B	10'-3"	TRIM_235

MEMBER TABLE		
FRAME LINE 1		
MARK	PART	LENGTH
EC-1	8X35C16	9'-5 1/4"
EC-2	8X35C16	10'-3 5/8"
EC-3	8X35C16	10'-3 5/8"
EC-4	8X35C16	9'-5 1/4"
ER-1	8X35C14	15'-3 13/16"
ER-2	8X35C14	15'-3 13/16"
DJ-1	8X35C16	7'-4"
DH-1	8X35C16	3'-0"
DH-2	8X35C16	18'-0"
G-1	8X25Z16	4'-11 3/4"





ENDWALL FRAMING: FRAME LINE 3



ENDWALL SHEETING & TRIM: FRAME LINE 3

PANELS: 26 Ga. SX – Need Sill Poly Color

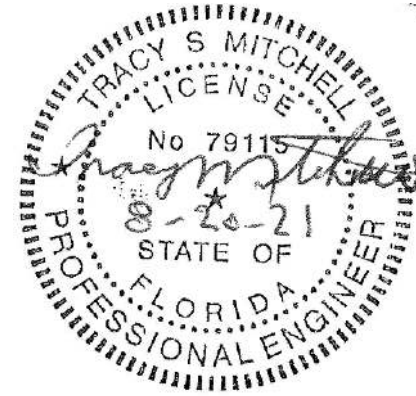


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BOLT TABLE FRAME LINE 3				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-3/ER-4	3	A325	1/2"	1 1/2"
Cor_Column/Raf	2	A325	5/8"	2"
EC-6/ER-4	2	A325	1/2"	1 1/2"

TRIM TABLE FRAME LINE 3			
◇ID	PART	LENGTH	DETAIL
1	BT-101	10'-3"	TRIM_68
2	CT-102	10'-3"	TRIM_323
3	RT-101	10'-3"	TRIM_229

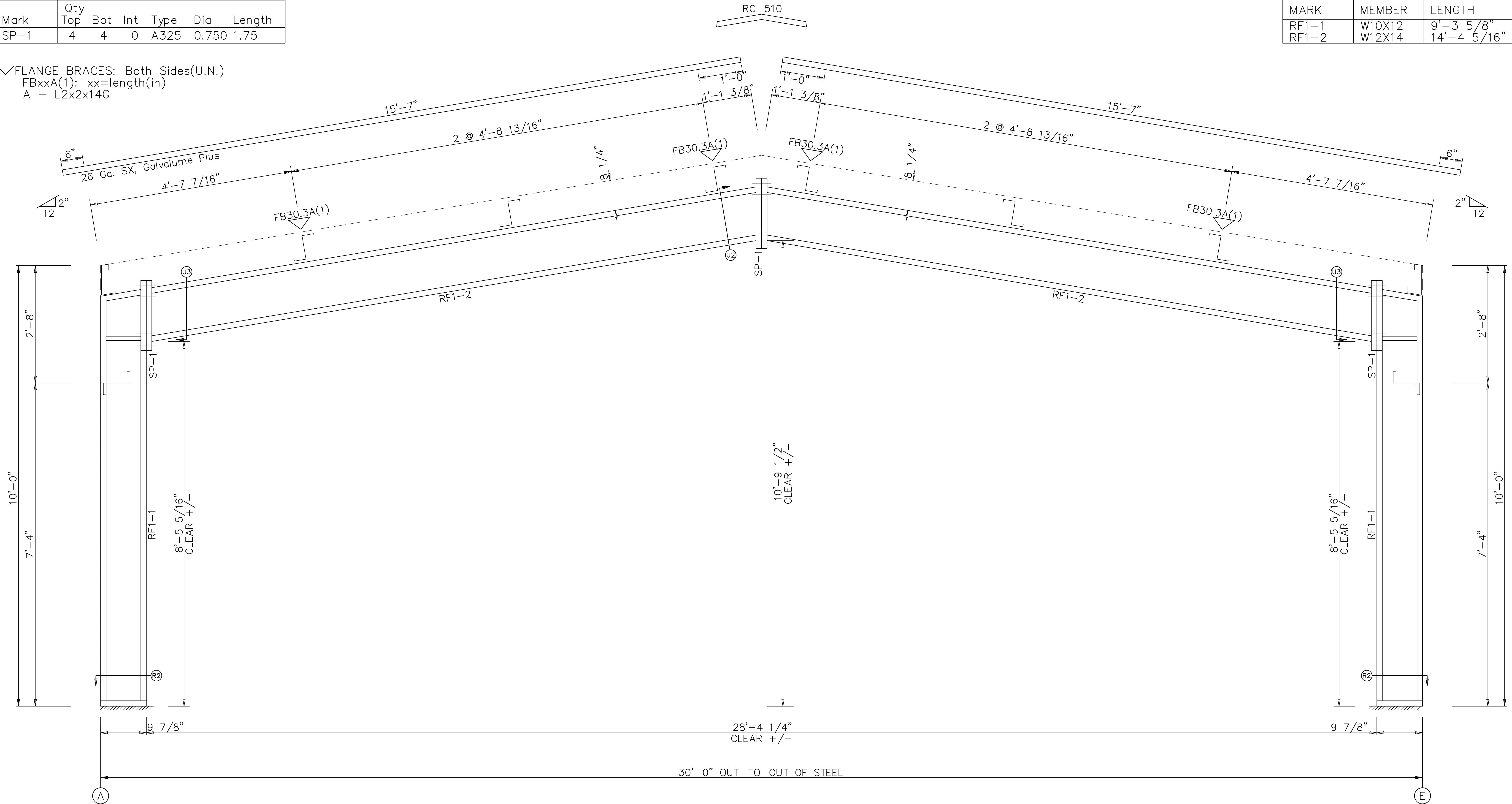
MEMBER TABLE FRAME LINE 3		
MARK	PART	LENGTH
EC-5	8X35C16	9'-3 3/16"
EC-6	8X35C16	11'-4 1/8"
EC-7	8X35C16	9'-3 3/16"
ER-3	10X35C14	15'-4 1/8"
ER-4	10X35C14	15'-4 1/8"
G-2	8X25Z16	13'-11 3/4"
G-3	8X25Z16	14'-3 1/2"



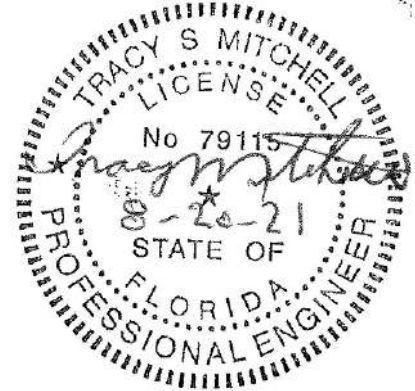
SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	0	A325	0.750	1.75

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1-1	W10X12	9'-3 5/8"
RF1-2	W12X14	14'-4 5/16"

▽ FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A - L2x2x14G



RIGID FRAME ELEVATION: FRAME LINE 2

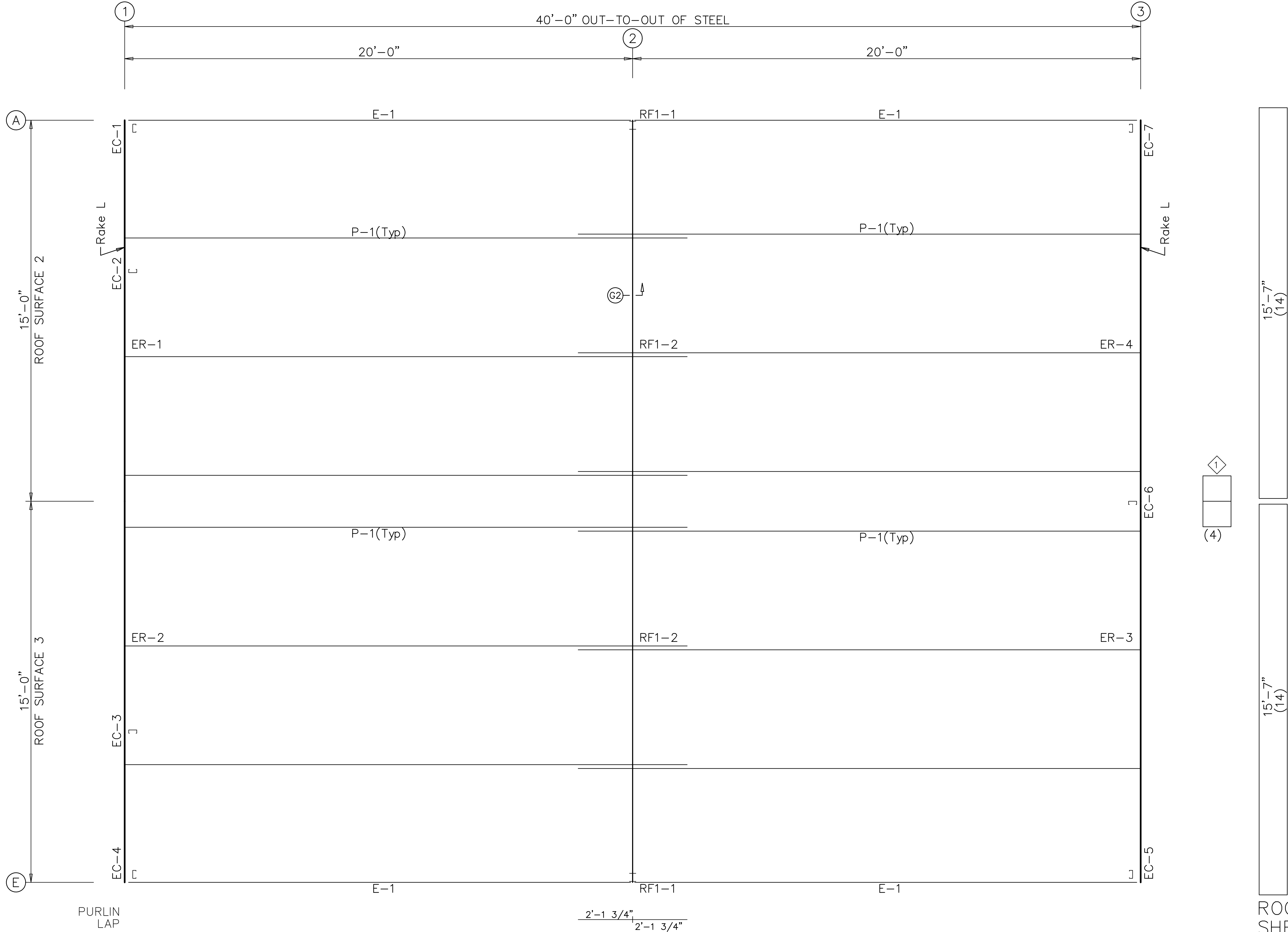


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PH 513-402-7600
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Union LaSteel				
PROJECT	Shop	RIGID FRAME ELEVATION		
ID	DerylWilliams30x40	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	281 SW Stewart Loop Lake City, FL 32024	DATE: 7/28/21	SHEET 8 OF 13	

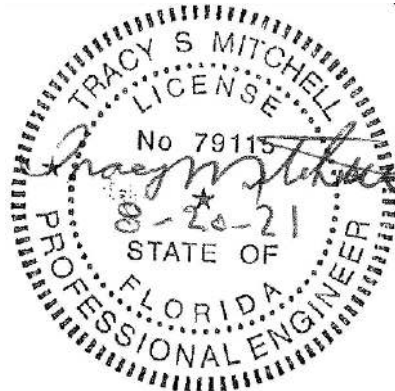
TRIM TABLE			
ROOF PLAN			
◇ID	PART	LENGTH	DETAIL
1	RC-510	10'-3"	TRIM_320

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8X25Z16	21'-9 7/16"
E-1	E085342L	19'-7 7/16"



ROOF FRAMING PLAN

ROOF SHEETING
PANELS: 26 Ga. SX
Galvalume Plus



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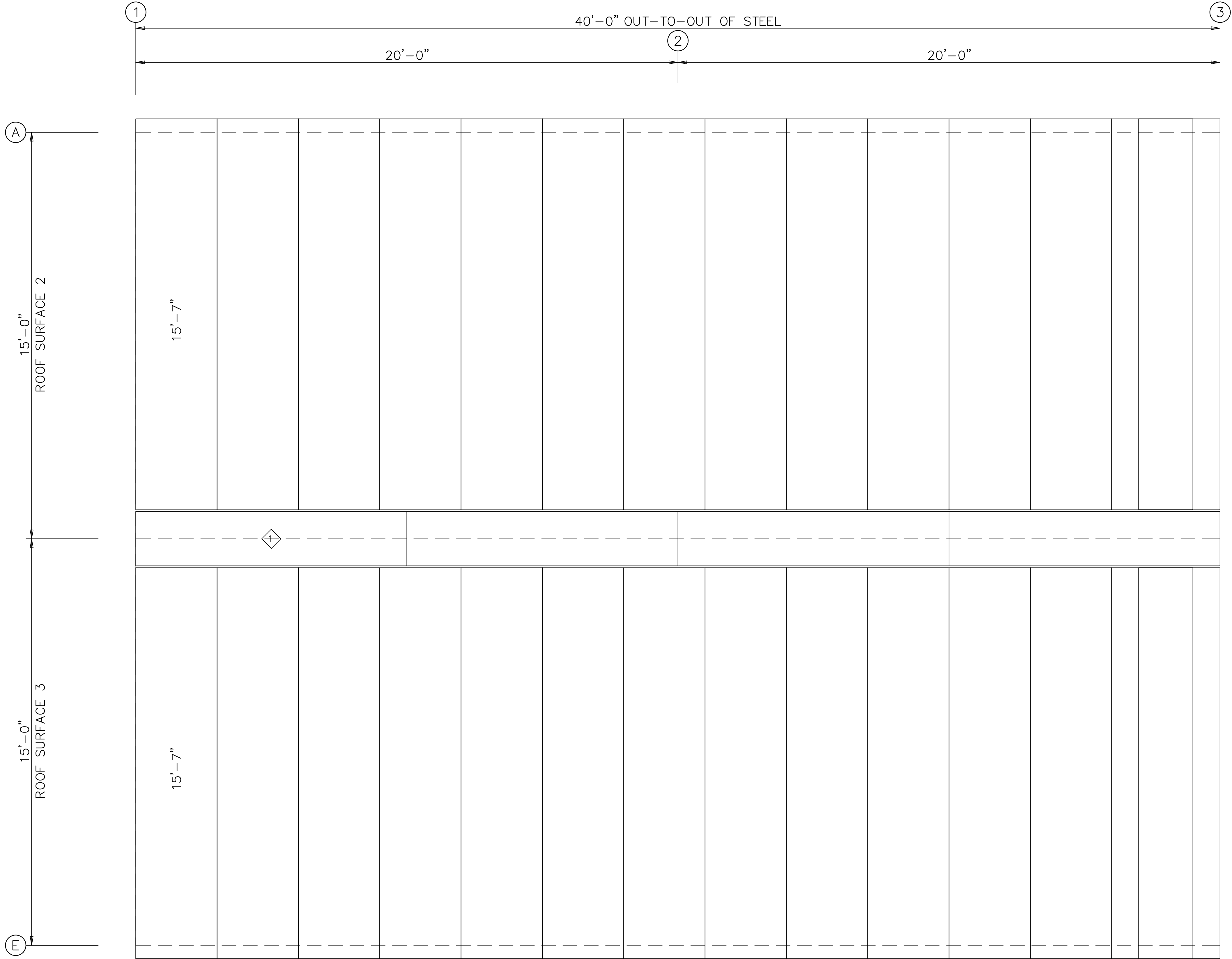
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PROJECT	Shop		ROOF FRAMING		
ID	DerylWilliams30x40		DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	281 SW Stewart Loop Lake City, FL 32024		DATE: 7/28/21	SHEET 9 OF 13	



TRIM TABLE			
ROOF PLAN			
◇ID	PART	LENGTH	DETAIL
1	RC-510	10'-3"	TRIM_320

ROOF SHEETING PLAN
PANELS: 26 Ga. SX – Galvalume Plus



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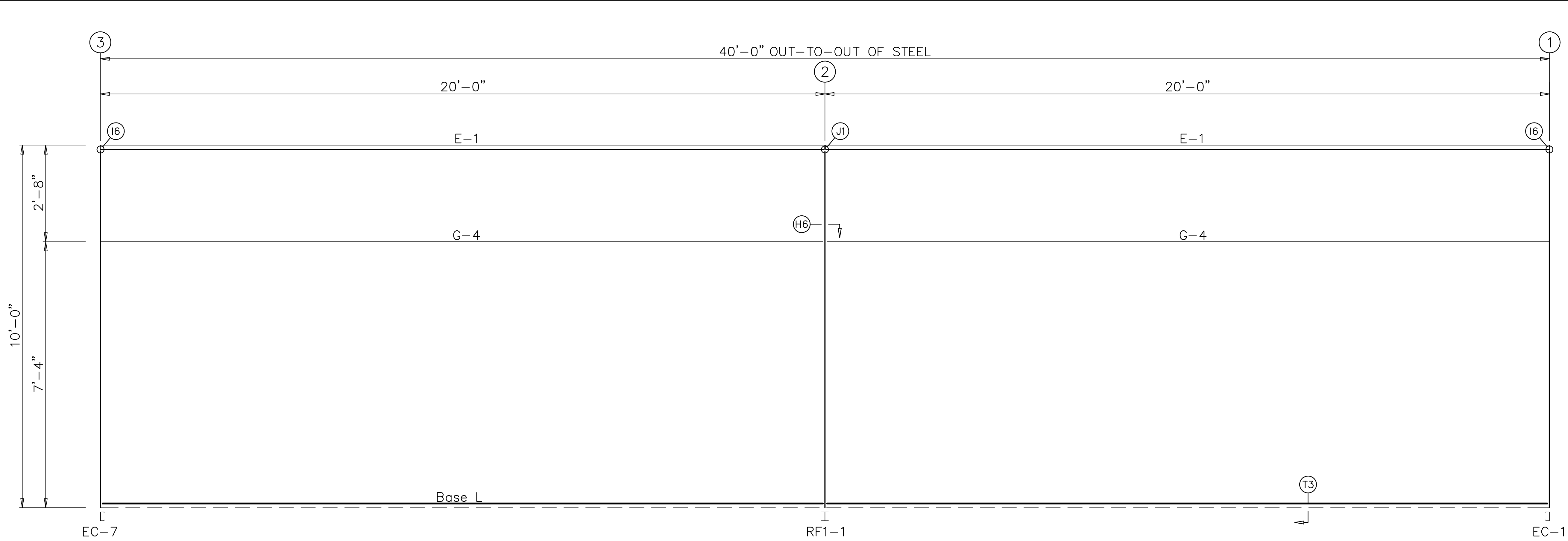
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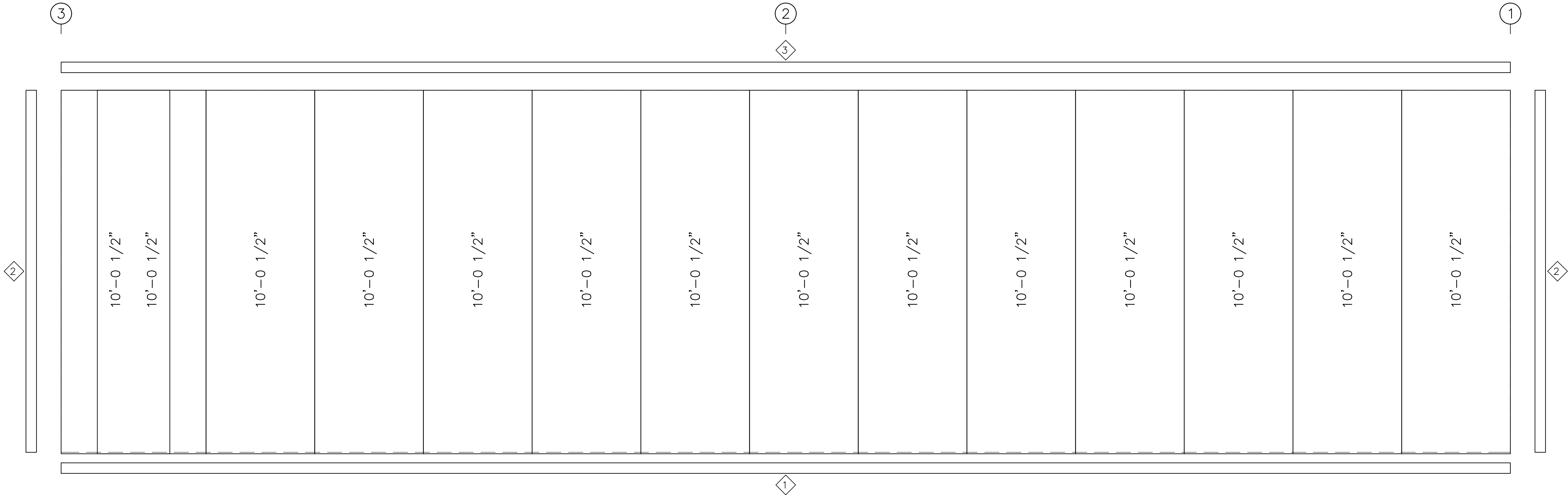
Union LaSteel					
PROJECT	Shop		ROOF SHEETING		
ID	DerylWilliams30x40		DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	281 SW Stewart Loop Lake City, FL 32024		DATE: 7/28/21		SHEET 10 OF 13



SIDEWALL FRAMING: FRAME LINE A

TRIM TABLE			
FRAME LINE A			
ID	PART	LENGTH	DETAIL
1	BT-101	10'-3"	TRIM_68
2	CT-102	10'-3"	TRIM_323
3	ET-702	10'-3"	TRIM_316

MEMBER TABLE		
FRAME LINE A		
MARK	PART	LENGTH
E-1	E085342L	19'-7 7/16"
G-4	8X25Z16	19'-4 5/16"

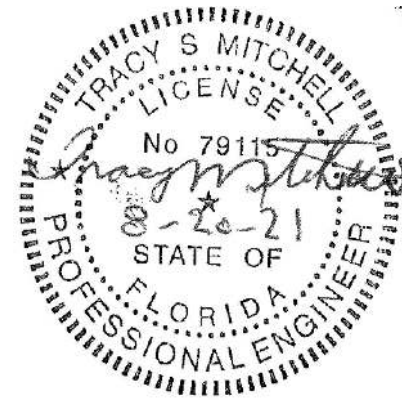


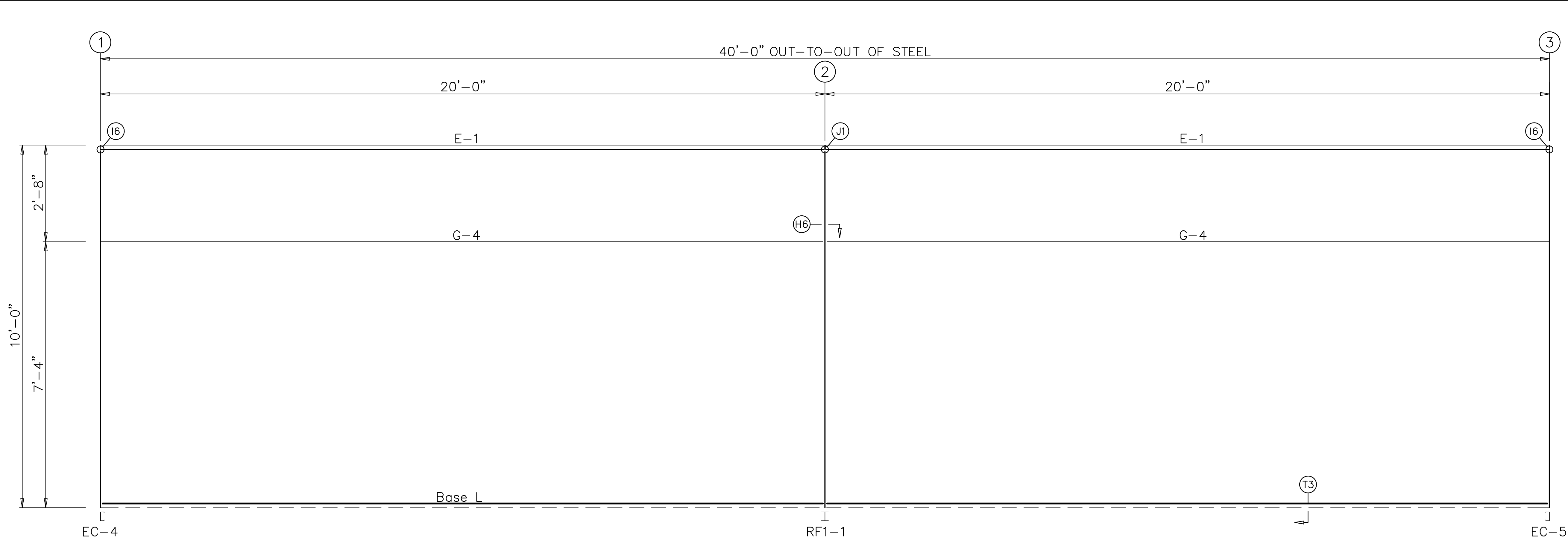
SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. SX – Need Sill Poly Color

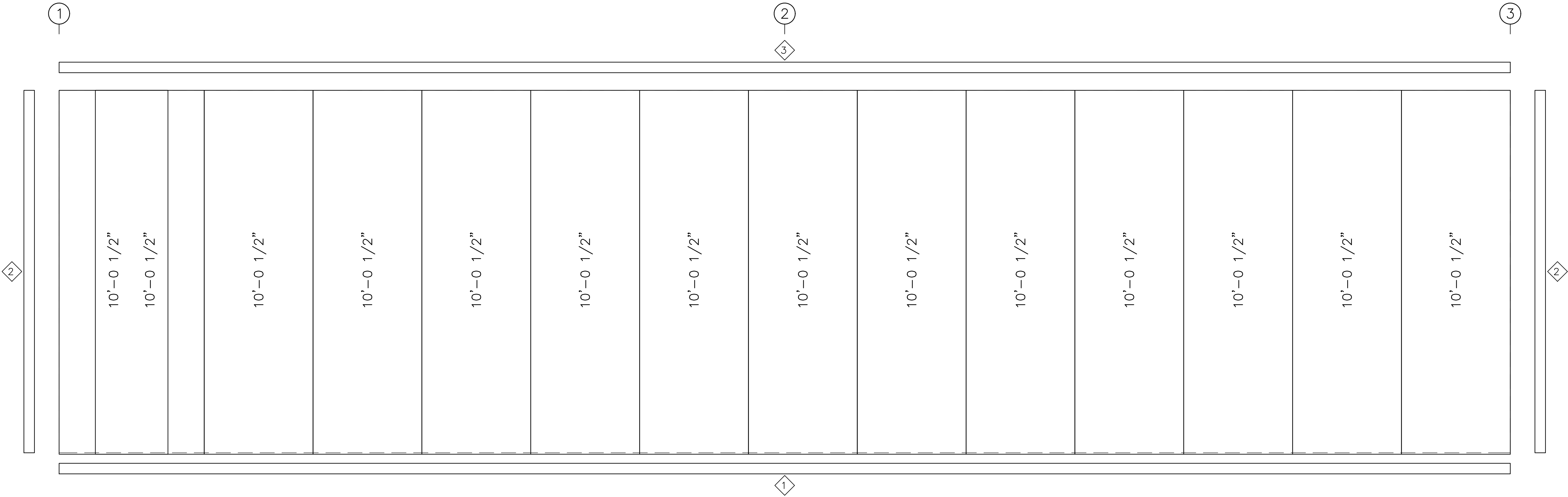


Union LaSteel				
PROJECT	Shop	SIDEWALL FRAMING		
ID	DerylWilliams30x40	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	281 SW Stewart Loop Lake City, FL 32024	DATE: 7/28/21	SHEET 11 OF 13	





SIDEWALL FRAMING: FRAME LINE E



SIDEWALL SHEETING & TRIM: FRAME LINE E

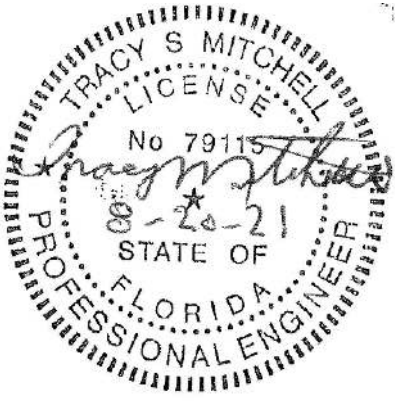
PANELS: 26 Ga. SX – Need Sill Poly Color

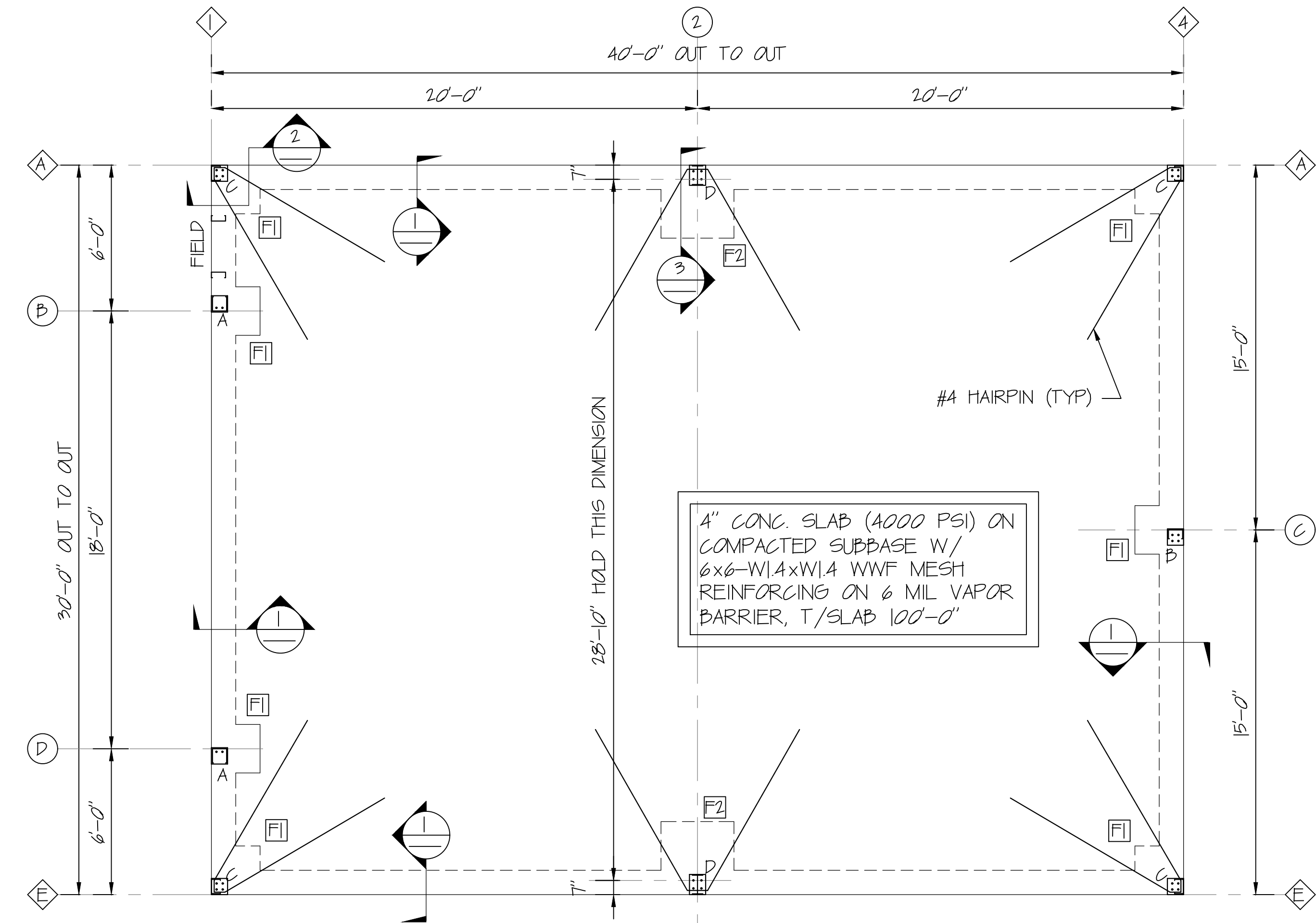


Union LaSteel				
PROJECT	Shop		SIDEWALL FRAMING	
ID	DerylWilliams30x40		DESIGN:	DRAFT: CHECK:
PROJECT ADDRESS	281 SW Stewart Loop Lake City, FL 32024		DATE: 7/28/21	SHEET 12 OF 13

TRIM TABLE FRAME LINE E			
ID	PART	LENGTH	DETAIL
1	BT-101	10'-3"	TRIM_68
2	CT-102	10'-3"	TRIM_323
3	ET-702	10'-3"	TRIM_316

MEMBER TABLE FRAME LINE E		
MARK	PART	LENGTH
E-1	E085342L	19'-7 7/16"
G-4	8X25Z16	19'-4 5/16"





FOUNDATION PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

FOOTING SCHEDULE			
FND MARK	FND SIZE (L x B x D)	REINFORCING	
		TOP, E.W.	BOTT, E.W.
F1	2'-0" x 2'-0" x 2'-0"	NA	#5's @ 11"
F2	3'-0" x 3'-0" x 2'-0"	NA	#5's @ 11"

THE SOIL BEARING PRESSURE IS ASSUMED TO BE 1500 PSF MAXIMUM. UPON ENCOUNTERING SOILS LESS THAN THIS CONTACT THE ENGINEER.

CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO ACI 301-18, "SPECIFICATIONS FOR BUILDINGS", EXCEPT AS NOTED BELOW. REFER TO CHAPTER 16 OF ACI 301 FOR REPORTS FROM TESTS THAT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER, ARCHITECT, OWNER, ANY APPLICABLE CONTRACTORS, CONCRETE SUPPLIER, AND BUILDING OFFICIAL.
- CONCRETE WORK IN COLD WEATHER SHALL CONFORM TO ALL REQUIREMENTS OF ACI 308R-16 "STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING" AND ACI 308R-16 "COLD WEATHER CONCRETING".
- CONCRETE WORK IN HOT WEATHER SHALL CONFORM TO ALL REQUIREMENTS OF ACI 308R-10 "HOT WEATHER CONCRETING". THE AIR TEMPERATURE, RELATIVE HUMIDITY, CONCRETE TEMPERATURE, AND WIND VELOCITY SHALL BE ENTERED INTO NOMOGRAPH FIG. 2.15 TO DETERMINE IF PRECAUTIONS AGAINST PLASTIC SHRINKAGE ARE REQUIRED.
- REBAR FABRICATION, PLACEMENT, SPLICING, ETC. SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF ACI 318, THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI) HANDBOOK AND ANY OTHER APPLICABLE PUBLICATIONS.
- A SLUMP TEST SHALL BE PERFORMED BEFORE ANY HRWR IS ADDED.
- CONCRETE MATERIAL SPECIFICATION: 3000 PSF f'c BASED ON 28 DAYS STRENGTH.
- UNLESS OTHERWISE NOTED, REINFORCEMENT BARS TO HAVE MINIMUM CONCRETE COVER AS SHOWN BELOW:

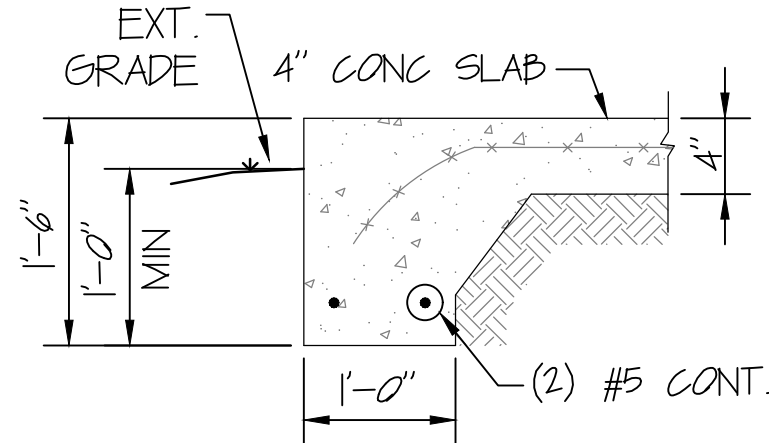
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 INCHES

CONCRETE EXPOSED TO EARTH OR WEATHER:
#5 BARS AND SMALLER 1 1/2 INCHES
OTHER 2 INCHES

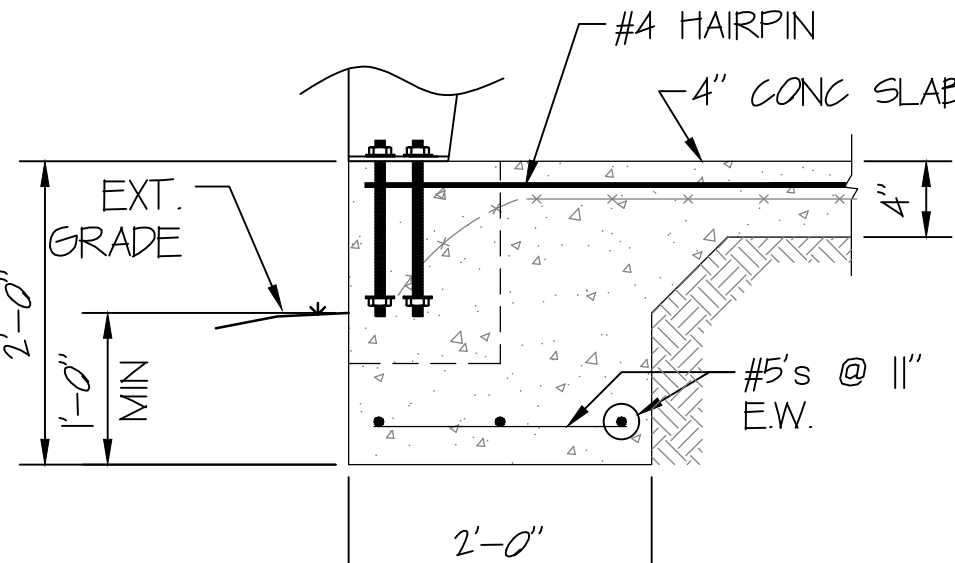
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
BEAM AND COLUMN BARS INCLUDING TIES, STIRRUPS AND SPIRALS 1 1/2 INCHES

SLABS, WALLS, AND JOISTS:
#11 BARS AND SMALLERS 3/4 INCHES
OTHERS 1 1/2 INCHES

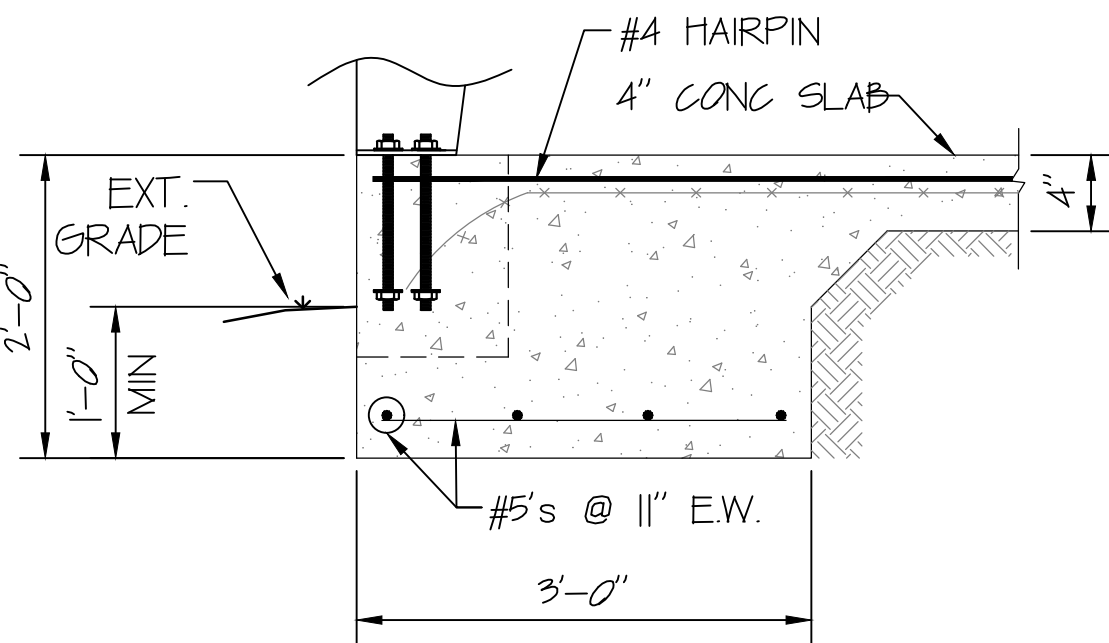
- ALL EXPOSED EDGES ARE TO BE CHAMFERED 3/4 INCH, UNO.
- LAP SPICE LENGTHS TO BE OF CLASS "B" PER ACI.
- CONCRETE REINFORCEMENT SHALL CONFORM TO ASTM A615 OR A616, GRADE 60.
- ISOLATED COLUMN FOOTINGS HAVE BEEN DESIGNED WITH AN ASSUMED ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF, UPON ENCOUNTERING ANY ADVERSE SOIL CONDITIONS CONTACT THE STRUCTURAL ENGINEER.
- DO NOT CAST CONCRETE AGAINST WATER OR FROZEN WATER.



SECTION 1
NT.S.



SECTION 2
NT.S.



SECTION 3
NT.S.



Union LaSteel				
PROJECT	Garage	FOUNDATION PLAN		
ID	DerylWilliams40x30	DESIGN: TSM	DRAFT: GSM	CHECK: TSM
PROJECT ADDRESS	281 SW Stewart Loop Lake City, FL 32024	DATE: 08/20/21	SHEET 13 OF 13	