

DATE 11/01/2010

Columbia County Building Permit
This Permit Must Be Prominently Posted on Premises During Construction**PERMIT**
000028972

APPLICANT JOSIE GASKINS PHONE 752-4089

ADDRESS 8912 S HWY 441 LAKE CITY FL 32025

OWNER RAMONA PARK CHURCH PHONE 752-7517

ADDRESS 8912 S HWY 441 LAKE CITY FL 32025

CONTRACTOR DWIGHT RHODES PHONE 752-4089

LOCATION OF PROPERTY 441 S, 1 MILE PAST ROSE CREEK ON RIGHT, 1/4 MILE BEFORE
ARCHY ROGERS HOME

TYPE DEVELOPMENT CHURCH ADDITION ESTIMATED COST OF CONSTRUCTION 100000.00

HEATED FLOOR AREA 1368.00 TOTAL AREA 1368.00 HEIGHT 12.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING AG-3 MAX. HEIGHT 35

Minimum Set Back Requirements: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO. _____

PARCEL ID 03-5S-17-09103-000 SUBDIVISION _____

LOT _____ BLOCK _____ PHASE _____ UNIT _____ TOTAL ACRES 1.54

CBC057549

Culvert Permit No. _____ Culvert Waiver _____ Contractor's License Number CBC057549 Applicant/Owner/Contractor Josie Gaskins

EXISTING 10-0442 BK _____ TC _____ N _____

Driveway Connection _____ Septic Tank Number _____ LU & Zoning checked by _____ Approved for Issuance _____ New Resident _____

COMMENTS: ADDITION TO CHURCH

DOT APPROVAL ON FILE

SRWMD ON FILE, FIRE DEPT. PLAN APPROVAL ON FILE

Check # or Cash 33330**FOR BUILDING & ZONING DEPARTMENT ONLY**

(footer/Slab)

Temporary Power _____ Foundation _____ Monolithic _____
date/app. by _____ date/app. by _____ date/app. by _____

Under slab rough-in plumbing _____ Slab _____ Sheathing/Nailing _____
date/app. by _____ date/app. by _____ date/app. by _____

Framing _____ Insulation _____
date/app. by _____ date/app. by _____

Rough-in plumbing above slab and below wood floor _____ Electrical rough-in _____
date/app. by _____ date/app. by _____

Heat & Air Duct _____ Peri. beam (Lintel) _____ Pool _____
date/app. by _____ date/app. by _____ date/app. by _____

Permanent power _____ C.O. Final _____ Culvert _____
date/app. by _____ date/app. by _____ date/app. by _____

Pump pole _____ Utility Pole _____ M/H tie downs, blocking, electricity and plumbing _____
date/app. by _____ date/app. by _____ date/app. by _____

Reconnection _____ RV _____ Re-roof _____
date/app. by _____ date/app. by _____ date/app. by _____

BUILDING PERMIT FEE \$ 500.00 CERTIFICATION FEE \$ 6.84 SURCHARGE FEE \$ 6.84

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$ _____

FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ _____ **TOTAL FEE** 588.68

INSPECTORS OFFICE [Signature] CLERKS OFFICE [Signature]

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

APP fee

33330

For Office Use Only Application # 1010-12 Date Received 10/7/10 By LH Permit # 28972
Zoning Official BLK Date 29.10.10 Flood Zone X Land Use A-3 Zoning A-3
FEMA Map # N/A Elevation N/A MFE N/A River N/A Plans Examiner J.C. Date 10-27-10

Comments

☒ NOC ☒ EH ☒ Deed or PA ☒ Site Plan ☐ State Road Info ☐ Parent Parcel #
☐ Dev Permit # ☐ In Floodway ☒ Letter of Auth. from Contractor ☐ F/W Comp. letter

IMPACT FEES: EMS _____ Fire _____ Corr _____ Road/Code _____
School _____ = TOTAL Suspended

☒ VF formSeptic Permit No. 10-0442Fax 386-752-4848 288-8888 SwainName Authorized Person Signing Permit Josie Gaslins
Dwight RhodesPhone 386-752-4089Address 8912 S Hwy 441 Lake City FL 32025Owners Name Ramona Park Church Phone 386-752-7517911 Address 8912 S Hwy 441 Lake City FL 32025Contractors Name Dwight Rhodes Phone 386-752-4089Address 1262 SE Baya Dr. Lake City, FL 32025Fee Simple Owner Name & Address N/ABonding Co. Name & Address N/AArchitect/Engineer Name & Address Nicholas Paul Geisler 1758 NW Brown Rd.
Lake City FL 32055Mortgage Lenders Name & Address N/ACircle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress EnergyProperty ID Number 03-SS-17-09103-000 Estimated Cost of Construction 100,000.00Subdivision Name N/A Lot — Block — Unit — Phase —Driving Directions 41-441 S. towards Ellisville About 1 mile past
rose creek on (R) hand side. 1/4 mile before Aray RogersNumber of Existing Dwellings on Property 3 homeConstruction of Ramona Park Church AdditionTotal Acreage 1.540 Lot Size —Do you need a - Culvert Permit or Culvert Waiver or Have an Existing DriveTotal Building Height 12'Actual Distance of Structure from Property Lines - Front 51'-35" Side 107'-20" Side 282' Rear 50'Number of Stories 1 Heated Floor Area 1368 Total Floor Area 1368 Roof Pitch 6:12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

STW signed by Dwight Rhodes 10.29.10

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. It may be to your advantage to check and see if your property is encumbered by any restrictions.

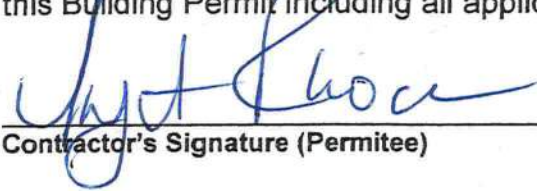
(Owners Must Sign All Applications Before Permit Issuance.)



Owners Signature

****OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

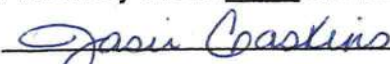


Contractor's Signature (Permittee)

Contractor's License Number CBC057549
Columbia County
Competency Card Number N/A

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 10th day of Sept 2010.

Personally known ☒ or Produced Identification _____



State of Florida Notary Signature (For the Contractor)

SEAL:



NOTICE OF COMMENCEMENT

Permit No. _____
Tax Folio No. _____

Inst:201012016823 Date:10/19/2010 Time:10:59 AM
DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1203 P:634

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): COMM NE COR OF NE1/4 OF SW1/4, RUN W 102 FT, S 472.42 FT, E 145.41 FT TO W R/W US-441, N ALONG R/W 468 FT, W 35.83 FT 8912 S Hwy 441 Lake City, FL 32025
2. General description of improvements: Building Addition
3. Owner Information
- a) Name and address: Ramona Park Church 8912 S Hwy 441 Lake City FL 32025
- b) Name and address of fee simple titleholder (if other than owner) _____
- c) Interest in property Building Addition
4. Contractor Information
- a) Name and address: Rhodes Brothers Inc. 1262 S2 Baya Dr. Lake City FL 32025
- b) Telephone No.: 386-752-4089 Fax No. (Opt.) 386-752-4848
5. Surety Information
- a) Name and address: _____
- b) Amount of Bond: _____
- c) Telephone No.: _____ Fax No. (Opt.) _____
6. Lender
- a) Name and address: _____ Phone No. _____
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
- a) Name and address: _____
- b) Telephone No.: _____ Fax No. (Opt.) _____
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
- a) Name and address: _____
- b) Telephone No.: _____ Fax No. (Opt.) _____
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF PINELLAS

10. Will Beardsley
Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager
Print Name

The foregoing instrument was acknowledged before me this 7th day of October, 2010, by

Will Beardsley as _____ (type of authority, e.g. officer, trustee,
attorney in fact) for Ramona Park Church (name of party on behalf of whom instrument was executed).

Personally Known ☒ OR Produced Identification _____

Notary Signature Josie Gaskins

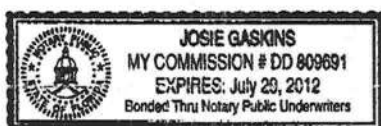
Type of Identification Produced _____

Name (print) Josie Gaskins

OR

Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

FORMS/NOC.rvsd2010



Will Beardsley
Signature of Natural Person Signing (in line # 10.) Above

Ramona Park Church

SUBCONTRACTOR VERIFICATION FORM

Jorie

APPLICATION NUMBER _____

CONTRACTOR _____

PHONE 752-4089

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

fx 752-4848

In Columbia County one permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

Lib 210	ELECTRICAL	Print Name: <u>John A. Coleman</u>	Signature: <u>[Signature]</u>	Phone #: _____
		License #: <u>ER0802038</u>		
1038	MECHANICAL	Print Name: <u>Tim Taylor</u>	Signature: <u>On other sheet</u>	Phone #: <u>386-364-8525</u>
	A/C <u>B</u>	License #: <u>CAC 057170</u>		
Liv	PLUMBING/	Print Name: <u>Buck Boyette</u>	Signature: <u>C. L. Boyette</u>	Phone #: <u>386-752-0776</u>
	GAS <u>730</u>	License #: <u>CFC021540</u>		
Liv	ROOFING	Print Name: <u>Dineal Roofing</u>	Signature: <u>[Signature]</u>	Phone #: <u>386-752-7578</u>
	<u>512</u>	License #: <u>CCC016346</u>		
	SHEET METAL	Print Name: _____	Signature: _____	Phone #: _____
		License #: _____		
	FIRE SYSTEM/ SPRINKLER	Print Name: _____	Signature: _____	Phone #: _____
		License #: _____		
	SOLAR	Print Name: _____	Signature: _____	Phone #: _____
		License #: _____		

Specialty License	License Number	Sub-Contractors Printed Name	Sub Contractors Signature
MASON - sub	000246	Ed Dennard	[Signature]
CONCRETE FINISHER - s	000218	TONY E. JORDAN SR	[Signature]
FRAMING ✓	CBC057549	Dwight Rhodes	[Signature]
INSULATION ✓		↓	
STUCCO	N/A	↓	
DRYWALL ✓		↓	
PLASTER	N/A	↓	
CABINET INSTALLER		↓	
PAINTING <u>1003</u>	CBC057549	Dwight Rhodes	[Signature]
ACOUSTICAL CEILING ✓		↓	
GLASS - s <u>611</u>	CBC057050	John A. Neal	[Signature]
CERAMIC TILE - s	651	Roland Wayne Wallace	[Signature]
FLOOR COVERING - s	710	Marc Vann	[Signature]
ALUM/VINYL SIDING - s	481	Todd L. Hunt	[Signature]
GARAGE DOOR	N/A		
METAL BLDG ERECTOR	N/A		

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each

Ramona Park Church

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 1010-12 CONTRACTOR Dwight Rhodes PHONE 752-4089
THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT 752-4848

In Columbia County one permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL 210	Print Name: <u>John M. Coleman</u> License #: <u>ER0002038</u>	Signature: <u>[Signature]</u> Phone #: <u></u>
MECHANICAL/A/C B	Print Name: <u>Tim Taylor</u> License #: <u>CAC057170</u>	Signature: <u>On other sheet</u> Phone #: <u>386-364-8525</u>
PLUMBING/GAS 730	Print Name: <u>Buck Boyette</u> License #: <u>CFC021540</u>	Signature: <u>C. L. Boyette</u> Phone #: <u>386-752-0774</u>
ROOFING 512	Print Name: <u>Dineal Roofing</u> License #: <u>CCC016346</u>	Signature: <u>[Signature]</u> Phone #: <u>386-752-7578</u>
SHEET METAL	Print Name: <u></u> License #: <u></u>	Signature: <u></u> Phone #: <u></u>
FIRE SYSTEM/SPRINKLER	Print Name: <u></u> License #: <u></u>	Signature: <u></u> Phone #: <u></u>
SOLAR	Print Name: <u></u> License #: <u></u>	Signature: <u></u> Phone #: <u></u>

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON - sub	000246	Ed Dennard	<u>[Signature]</u>
CONCRETE FINISHER - s	000218	TONY E. JORDAN SR	<u>[Signature]</u>
FRAMING ✓	CBC057549	Dwight Rhodes	<u>[Signature]</u>
INSULATION ✓		↓	
STUCCO	N/A	↓	
DRYWALL ✓		↓	
PLASTER	N/A	↓	
CABINET INSTALLER		↓	
PAINTING 1003	CBC057549	Dwight Rhodes	
ACOUSTICAL CEILING ✓		↓	
GLASS - s 511	CCC057550	John O'Neal	<u>[Signature]</u>
CERAMIC TILE - s	651	Roland Wayne Wallace	
FLOOR COVERING - s	710	MARC VANN	<u>[Signature]</u>
ALUM/VINYL SIDING - s	481	TODD L HUNT	<u>[Signature]</u>
GARAGE DOOR	N/A		
METAL BLDG ERECTOR	N/A		

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

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No Events **No Name History**

Detail by Entity Name

Florida Non Profit Corporation

RAMONA PARK CHURCH, INC.

Filing Information

Document Number 723237
FEI/EIN Number 237209051
Date Filed 04/21/1972
State FL
Status ACTIVE

Principal Address

8170 S US HWY 441
LAKE CITY FL 32025 US
Changed 04/24/2009

Mailing Address

U S 41 SOUTH
PO BOX 1575
LAKE CITY FL 32056 US
Changed 04/12/2004

Registered Agent Name & Address

PETRY, JAMES
6301 CR252
WELLBORN FL 32094 US
Name Changed: 04/30/2003
Address Changed: 04/30/2003

Officer/Director Detail

Name & Address

Title PD
WELSH, RANDY
248 NW FOREST MEADOWS AVE
LAKE CITY FL 32055

Title VP
LEE, DAVID
277 NW JESSUP CT
LAKE CITY FL 32055

Title SD
DINGES, JON M
247 NW DOGWOOD TERRACE
LAKE CITY FL 32055

Title T
BEARDSLEY, WILLIAM S
533 SW COZY GLN
LAKE CITY FL 32024

Title ATD
KRUMMRICH, JERRY T

260 SW SUMMERHILL GLN
LAKE CITY FL 32024

Annual Reports

Report Year Filed Date

2008	04/15/2008
2009	04/24/2009
2010	04/20/2010

Document Images

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State of Florida, Department of State

Columbia County Property Appraiser

DB Last Updated: 8/5/2010

2009 Tax Roll Year

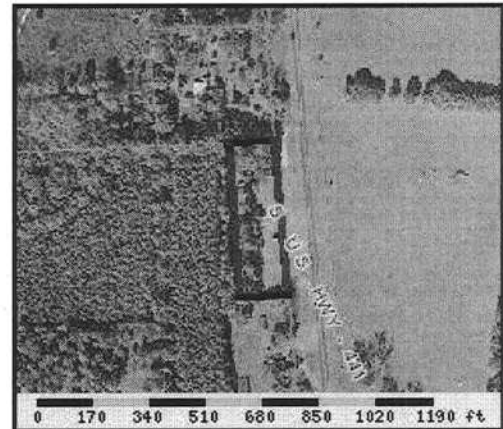
Parcel: 03-5S-17-09103-000

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[Next Higher Parcel >>](#)
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[Tax Estimator](#)
[Property Card](#)
[Parcel List Generator](#)
[Interactive GIS Map](#)
[Print](#)

Search Result: 1 of 1

Owner & Property Info

Owner's Name	RAMONA PARK CHURCH INC		
Mailing Address	P O BOX 1575 LAKE CITY, FL 32056		
Site Address	8192 S HIGHWAY 441		
Use Desc. (code)	CHURCHES (007100)		
Tax District	3 (County)	Neighborhood	3517
Land Area	1.540 ACRES	Market Area	02
Description	NOTE: This description is not to be used as the Legal Description for this parcel in any legal transaction. COMM NE COR OF NE1/4 OF SW1/4, RUN W 102 FT, S 472.42 FT, E 145.41 FT TO W R/W US-441, N ALONG R/W 468 FT, W 35.83 FT TO POB. ORB 287-79		



Property & Assessment Values

2009 Certified Values		
Mkt Land Value	cnt: (0)	\$17,380.00
Ag Land Value	cnt: (1)	\$0.00
Building Value	cnt: (3)	\$126,607.00
XFOB Value	cnt: (2)	\$1,300.00
Total Appraised Value		\$145,287.00
Just Value		\$145,287.00
Class Value		\$0.00
Assessed Value		\$145,287.00
Exempt Value	(code: 02)	\$145,287.00
Total Taxable Value	Cnty: \$0 Other: \$0 Schl: \$0	

2010 Working Values

NOTE:
2010 Working Values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

[Show Working Values](#)

Sales History

[Show Similar Sales within 1/2 mile](#)

Sale Date	OR Book/Page	OR Code	Vacant / Improved	Qualified Sale	Sale RCode	Sale Price
NONE						

Building Characteristics

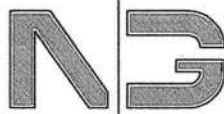
Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	CHURCH (009100)	1972	CONC BLOCK (15)	3360	3580	\$67,922.00
2	CHURCH (009100)	1990	CONC BLOCK (15)	1920	2058	\$50,796.00
3	MOBILE HME (000800)	1973	AVERAGE (05)	720	880	\$3,933.00
Note: All S.F. calculations are based on exterior building dimensions.						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0294	SHED WOOD/	0	\$1,000.00	0000001.000	0 x 0 x 0	(000.00)
0296	SHED METAL	2002	\$300.00	0000120.000	10 x 12 x 0	AP (050.00)

Land Breakdown

28972



**NICHOLAS
PAUL
GEISLER**
ARCHITECT
N.C.A.R.B. Certified

1758 NW Brown Rd.
Lake City, FL 32055
386/755-6608



22 NOVEMBER 2010

BUILDING OFFICIAL
COLUMBIA COUNTY BUILDING DEPARTMENT
COLUMBIA COUNTY COURTHOUSE ANNEX
LAKE CITY, FLORIDA

RE: RAMONA PARK CHURCH ADDITION
PERMIT Nr.: 1010-12

DEAR SIR:

PLEASE BE ADVISED THAT THE FLOOR PLAN FOR THE ABOVE
REFERENCED PROJECT HAS BEEN MODIFIED TO INCLUDE A
1/2 BATH IN THE SUNDAY SCHOOL ROOM AS SHOWN ON
ATTACHMENT 'A'.

SHOULD YOU HAVE ANY QUESTIONS, PLEASE DO NOT HESITATE TO
CALL FOR ANY ASSISTANCE.

YOURS TRULY,
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005



NICHOLAS
PAUL
GEISLER
ARCHITECT
N.C.A.R.B. Certified

1758 NW Brown Rd.
Lake City, FL 32055
386/755-6608



14 DECEMBER 2010



BUILDING OFFICIAL
COLUMBIA COUNTY BUILDING DEPARTMENT
COLUMBIA COUNTY COURTHOUSE ANNEX
LAKE CITY, FLORIDA

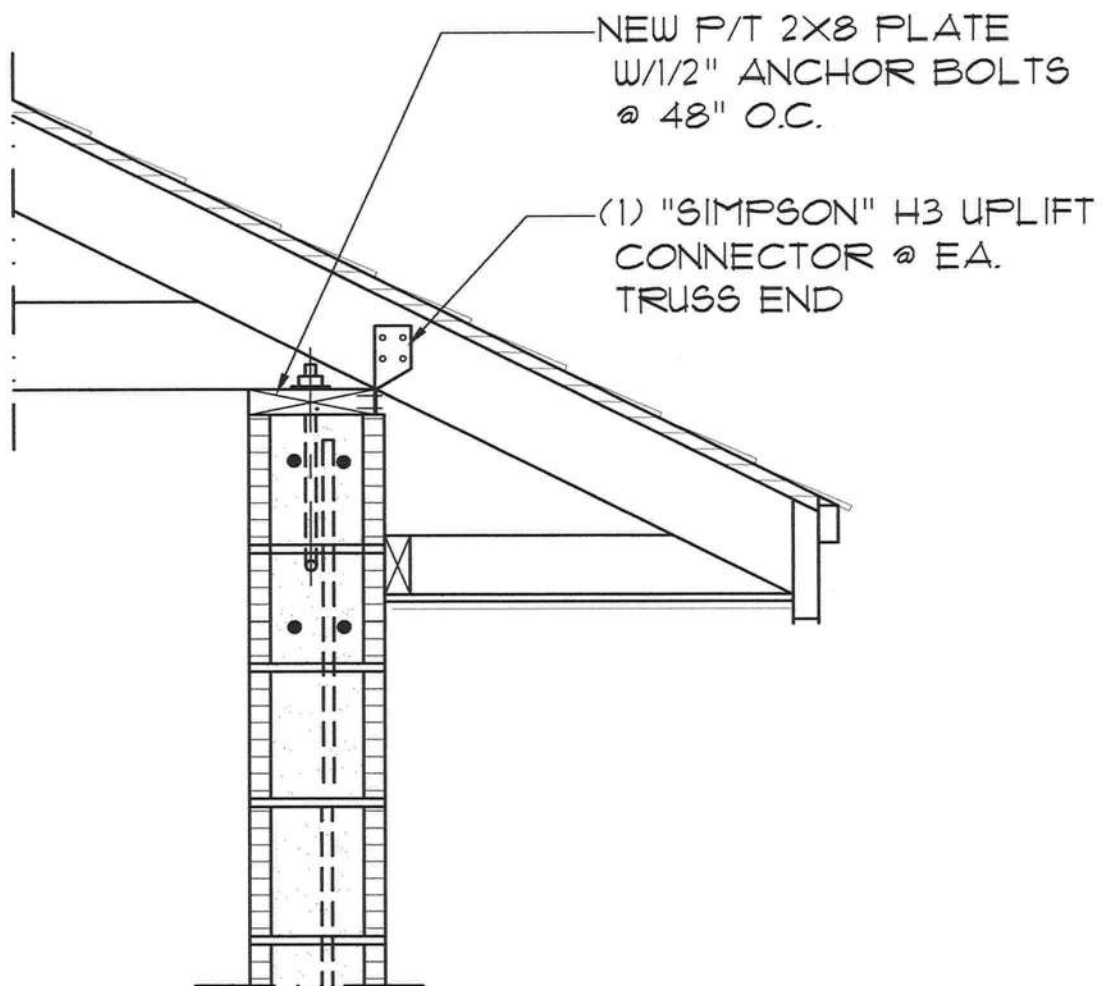
RE: RAMONA PARK CHURCH ADDITION
PERMIT Nr.: 28972

DEAR SIR:

PLEASE BE ADVISED THAT THE "SIMPSON" METAL W/SS GALVANIZED
SADDLES @ EA. TRUSS TO WALL CONNECTION HAS BEEN REVISED
TO INCLUDE A NEW P/T 2X8 TOP PLATE W/1/2" ANCHOR BOLTS AT
48" O.C. AND (1) "SIMPSON" H3 UPLIFT CONNECTOR AT EA. TRUSS END.
SEE ATTACHMENT "A" FOR DETAILS.

SHOULD YOU HAVE ANY QUESTIONS, PLEASE DO NOT HESITATE TO
CALL FOR ANY ASSISTANCE.

YOURS TRULY,
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005



New C.M.U. Wall Top Plate and Truss Attachment Detail

SCALE : NONE

[Signature]
AR 7005 15 Dec 2010

ATTACHMENT 'A'



COLUMBIA COUNTY FIRE RESCUE

P.O. BOX 1529 Lake City, Florida 32056
Office (386) 754-7071 Fax (386) 754-7064

Division Chief
David L. Boozer

08 April 2011

TO: Troy Crews
Columbia County Building and Zoning

FROM: David L. Boozer
Division Chief / Fire Marshal

RE: Permit # 28972
Ramona Park Church

A Fire Safety Inspection was performed at the above listed facility on April 7, 2011. At the time of my inspection this building met the requirements as set forth in Chapter 12, of the Florida Fire Prevention Code, 2004 edition. I recommend approval.

Should you require any additional information, please feel free to contact my office.

Sincerely,

David L. Boozer



STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL
SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

CR # 10-5038

PERMIT NO. 9791648
DATE PAID: 9/23/10
FEE PAID: \$90.00
RECEIPT #: 1497339

APPLICATION FOR:

[X] New System [] Existing System [] Holding Tank [] Innovative
[] Repair [] Abandonment [] Temporary []

APPLICANT: ROMANA PARK CHURCH INC.

AGENT: RHODES BROTHERS INC.

TELEPHONE: (386) 752-4089

MAILING ADDRESS: 1262 SE BAYA DRIVE

LAKE CITY

FL 32025

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3) (m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: N/A BLOCK: N/A SUBDIVISION: METES AND BOUNDS PLATTED:

PROPERTY ID #: 03-05S-17-09103-000 ZONING: COM I/M OR EQUIVALENT: [NO]

PROPERTY SIZE: 1.540 ACRES WATER SUPPLY: [] PRIVATE PUBLIC [X] <=2000GPD [] >2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [NO] DISTANCE TO SEWER: N/A FT

PROPERTY ADDRESS: 8192 SOUTH HWY. 441

DIRECTIONS TO PROPERTY: 441 SOUTH SITE ON RIGHT AFTER JIM WITT RD.

BUILDING INFORMATION [] RESIDENTIAL [X] COMMERCIAL

Unit No.	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
----------	-----------------------	-----------------	--------------------	--

1	CHURCH	0		220 SEATS
---	--------	---	--	-----------

2	existing sanctuary	3300 SQ FT.		Sunday School notured at
---	--------------------	-------------	--	--------------------------

3	existing fellowship hall	1920 SQ FT.		same time as church.
---	--------------------------	-------------	--	----------------------

4	new addition	1368 SQ FT.		Nursery added to seats above.
---	--------------	-------------	--	-------------------------------

		total	6648 SQ FT.	children
--	--	-------	-------------	----------

[] Floor/Equipment Drains [] Other (Specify) No regular meals.

SIGNATURE: [Signature] DATE: 9/20/2010

DH 4015, 08/09 (Obsoletes previous editions which may not be used)
Incorporated 64E-6.001, FAC

Held for engineer flow letter RC'd 10-4-10

Page 1 of 4
RECEIVED 9/23/10

10-0442

Notes:

**FAX
MEMORANDUM****MEMORANDUM****FLORIDA DEPARTMENT OF TRANSPORTATION**

To: Mr. John Kerce, Dept. Director
Columbia Co. Building & Zoning Dept.
Fax No: 386-758-2160

From: Dale L. Cray, FDOT Permits Insp.
Date: 9-9-2010 **Fax No.** 386-961-7183
Attention: Col Co. Building Zoning Dept.

☐ Sign and return. ☐ For your files. ☐ Please call me. ☒ FYI ☐ For Review

REF: Existing Driveway

PROJECT: Ramona Park Church

PARCEL ID No: 03-5s-17-09103-000 **Permit No :** N/A **Sec No :**

MILE POST: N/A

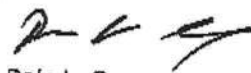
Mr. Kerce

Please accept this as our legal notice of final passing inspection for (Ramona Park Church) for an existing residential driveway. The project address is, 8192 NW HWY 41 Lake City, FL 32055.

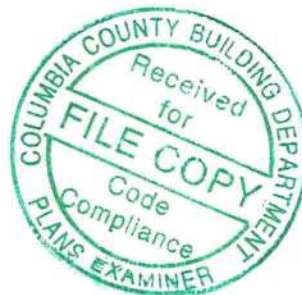
The existing residential Access has been Inspected and (Approved) and, meets FDOT Standard Requirements for a commercial driveway.

If further information is required on this project please do not hesitate to contact this office for additional access permitting information details. My office number is 961-7193 or 961-7146.

Sincerely,



Dale L. Cray
Access Permits Inspector





COLUMBIA COUNTY FIRE RESCUE

P.O. BOX 1529 Lake City, Florida 32056
Office (386) 754-7071 Fax (386) 754-7064

Division Chief
David L. Boozer

22 October 2010

TO: Troy Crews
Columbia County Building and Zoning

FROM: David L. Boozer
Division Chief / Fire Marshal

RE: Application # 1010-12
Ramona Park Church

A plan review was performed of the submitted plans for an addition to the Ramona Park Church located on US 41/441 South. This building falls under Chapter 38 of the Florida Fire Prevention Code, 2007 Edition. I recommend approval.

Should you require any additional information, please feel free to contact my office.

Sincerely,

David L. Boozer



**SUWANNEE
RIVER
WATER
MANAGEMENT
DISTRICT**

9225 CR 49
LIVE OAK, FLORIDA 32060
TELEPHONE: (386) 362-1001
TELEPHONE: 800-226-1066
FAX (386) 362-1056

NOTICED GENERAL PERMIT

PERMITTEE:
RAMONA PARK CHURCH
POST OFFICE BOX 1575
LAKE CITY, FL 32056

PERMIT NUMBER: ERP88-0078M
DATE ISSUED: 09/20/2010
DATE EXPIRES: 09/20/2013
COUNTY: COLUMBIA
TRS: S3/T5S/R17E

PROJECT: RAMONA PARK CHURCH SECOND ADDITION

Approved entity to whom operation and maintenance may be transferred pursuant to rule 40B-4.1130, Florida Administrative Code (F.A.C.):

RAMONA PARK CHURCH
POST OFFICE BOX 1575
LAKE CITY, FL 32056

Based on information provided, the Suwannee River Water Management District's (District) rules have been adhered to and an environmental resource noticed general permit is in effect for the permitted activity description below:

Previous permit issued for 0.04 acres of impervious surface on 1.50 acres. Modification consists of construction and operation of a surfacewater management system serving 0.09 acres of impervious surface on a total project area of 1.50 acres in a manner consistent with the application package submitted by Romona Park Church on September 13, 2010.

It is your responsibility to ensure that adverse off-site impacts do not occur either during or after construction. Any additional construction or alterations not authorized by this permit may result in flood control or water quality problems both on and off site and will be a violation of District rule.

You or any other substantially affected persons are entitled to request an administrative hearing or mediation. Please refer to enclosed notice of rights.

This permit is issued under the provisions of chapter 373, F.S., chapter 40B-4, and chapter 40B-400, F.A.C. A noticed general permit authorizes the construction, operation, maintenance, alteration,

Permit No.: ERP88-0078M

Project: RAMONA PARK CHURCH SECOND ADDITION

Page 2 of 7

abandonment, or removal of certain minor surface water management systems. This permit authorizes the permittee to perform the work necessary to construct, operate, and maintain the surface water management system shown on the application and other documents included in the application. This is to notify you of District's agency action concerning Notice Of Intent. This action is taken pursuant to rule 40B-4 and 40B-400, F.A.C.

General Conditions for All Noticed General Permits:

1. The terms, conditions, requirements, limitations, and restrictions set forth in this section are general permit conditions and are binding upon the permittee for all noticed general permits in Part II of this chapter. These conditions are enforceable under Part IV of chapter 373, F.S.
2. The general permit is valid only for the specific activity indicated. Any deviation from the specified activity and the conditions for undertaking that activity shall constitute a violation of the permit. A violation of the permit is a violation of Part IV of chapter 373, F.S., and may result in suspension or revocation of the permittee's right to conduct such activity under the general permit. The District may also begin legal proceedings seeking penalties or other remedies as provided by law for any violation of these conditions.
3. This general permit does not eliminate the necessity to obtain any required federal, state, local and special District authorizations prior to the start of any construction, alteration, operation, maintenance, removal or abandonment authorized by this permit.
4. This general permit does not convey to the permittee or create in the permittee any property right, or any interest in real property, nor does it authorize any entrance upon or activities on property which is not owned or controlled by the permittee, or convey any rights or privileges other than those specified in the general permit and Part II of this chapter.
5. This general permit does not relieve the permittee from liability and penalties when the permitted activity causes harm or injury to human health or welfare, animal, plant or aquatic life, or property. It does not allow the permittee to cause pollution in contravention of Florida Statutes and District rules.
6. The permittee is hereby advised that s.253.77, F.S., states that a person may not commence any excavation, construction or other activity involving the use of sovereign or other lands of the state, the title to which is vested in the Board of Trustees of the Internal Improvement Trust Fund without obtaining the required lease, license, easement, or other form of consent authorizing the proposed use. Therefore, the permittee is responsible for obtaining any necessary authorizations from the Board of Trustees prior to commencing activity on sovereignty lands or other state-owned lands.

7. The authorization to conduct activities pursuant to general permit may be modified, suspended or revoked in accordance with chapter 120, and s.373.429, F.S.
8. This permit shall not be transferred to a third party except pursuant to s.40B-4.1130, F.A.C. The permittee transferring the general permit shall remain liable for any corrective actions that may be required as a result of any permit violations prior to sale, conveyance, or other transfer of ownership or control of the permitted system or the real property at which the permitted system is located.
9. Upon reasonable notice to the permittee, District staff with proper identification shall have permission to enter, inspect, sample and test the permitted system to insure conformity with the plans and specifications approved by the permit.
10. The permittee shall maintain any permitted system in accordance with the plans submitted to the District and authorized by this general permit.
11. A permittee's right to conduct a specific noticed activity under this noticed general permit is authorized for the duration on the front of this permit.
12. Construction, alteration, operation, maintenance, removal and abandonment approved by this general permit shall be conducted in a manner which does not cause violations of state water quality standards, including any antidegradation provisions of s.62-4.242(1)(a) and (b), 62-4.242(2) and (3), and 62-302.300, F.A.C., and any special standards for Outstanding Florida Waters and Outstanding National Resource Waters. The permittee shall implement best management practices for erosion, turbidity and other pollution control to prevent violation of state water quality standards. Temporary erosion control measures such as sodding, mulching, and seeding shall be implemented and shall be maintained on all erodible ground areas prior to and during construction. Permanent erosion control measures such as sodding and planting of wetland species shall be completed within seven days of any construction activity. Turbidity barriers shall be installed and maintained at all locations where the possibility of transferring suspended solids into wetlands or other surface waters exists due to the permitted activity. Turbidity barriers shall remain in place and shall be maintained in a functional condition at all locations until construction is completed and soils are stabilized and vegetation has been established. Thereafter the permittee shall be responsible for the removal of the barriers. The permittee shall correct any erosion or shoaling that causes adverse impacts to the water resources.
13. The permittee shall hold and save the District harmless from any and all damages, claims or liabilities which may arise by reason of the construction, alteration, operation, maintenance, removal, abandonment or use of any system authorized by the general permit.
14. The permittee shall immediately notify the District in writing of any previously submitted

Permit No.: ERP88-0078M

Project: RAMONA PARK CHURCH SECOND ADDITION

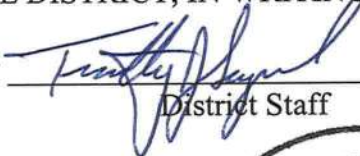
Page 4 of 7

information that is later discovered to be inaccurate.

15. The permittee shall perform all construction authorized in a manner so as to minimize adverse impacts to fish, wildlife, natural environmental values, and water quality. The permittee shall institute necessary measures during construction including riprap, reinforcement, or compaction of any fill materials placed around newly installed structures, to minimize erosion, turbidity, nutrient loading, and sedimentation in the receiving waters.

16. The permit is issued based on the information submitted by the applicant which reasonably demonstrates that adverse off-site water resource impacts will not be caused by the permitted activity. It is the responsibility of the permittee to insure that such adverse impacts do not in fact occur either during or after construction.

WITHIN 30 DAYS AFTER COMPLETION OF THE PROJECT, THE PERMITTEE SHALL NOTIFY THE DISTRICT, IN WRITING, THAT THE FACILITIES ARE COMPLETE.

Approved by  Date Approved 9/21/10
District Staff



NOTICE OF RIGHTS

1. A person whose substantial interests are or may be determined has the right to request an administrative hearing by filing a written petition with the Suwannee River Water Management District (District), or may choose to pursue mediation as an alternative remedy under Section 120.569 and 120.573, Florida Statutes, before the deadline for filing a petition. Choosing mediation will not adversely affect the right to a hearing if mediation does not result in a settlement. The procedures for pursuing mediation are set forth in Sections 120.569 and 120.57 Florida Statutes. Pursuant to Rule 28-106.111, Florida Administrative Code, the petition must be filed at the office of the District Clerk at District Headquarters, 9225 C.R. 49, Live Oak, Florida 32060 within twenty-one (21) days of receipt of written notice of the decision or within twenty-one (21) days of newspaper publication of the notice of District decision (for those persons to whom the District does not mail actual notice). A petition must comply with Chapter 28-106, Florida Administrative Code.
2. If the Governing Board takes action which substantially differs from the notice of District decision to grant or deny the permit application, a person whose substantial interests are or may be determined has the right to request an administrative hearing or may chose to pursue mediation as an alternative remedy as described above. Pursuant to Rule 28-106.111, Florida Administrative Code, the petition must be filed at the office of the District Clerk at District Headquarters, 9225 C.R. 49, Live Oak, Florida 32060 within twenty-one (21) days of receipt of written notice of the decision or within twenty-one (21) days of newspaper publication of the notice of District decision (for those persons to whom the District does not mail actual notice). Such a petition must comply with Chapter 28-106, Florida Administrative Code.
3. A substantially interested person has the right to a formal administrative hearing pursuant to Section 120.569 and 120.57(1), Florida Statutes, where there is a dispute between the District and the party regarding an issue of material fact. A petition for formal hearing must comply with the requirements set forth in Rule 28-106.201, Florida Administrative Code.
4. A substantially interested person has the right to an informal hearing pursuant to Section 120.569 and 120.57(2), Florida Statutes, where no material facts are in dispute. A petition for an informal hearing must comply with the requirements set forth in Rule 28-106.301, Florida Administrative Code.
5. A petition for an administrative hearing is deemed filed upon receipt of the petition by the Office of the District Clerk at the District Headquarters in Live Oak, Florida.
6. Failure to file a petition for an administrative hearing within the requisite time frame shall constitute a waiver of the right to an administrative hearing pursuant to Rule 28-106.111, Florida Administrative Code.

Permit No.: ERP88-0078M

Project: RAMONA PARK CHURCH SECOND ADDITION

Page 6 of 7

7. The right to an administrative hearing and the relevant procedures to be followed is governed by Chapter 120, Florida Statutes, and Chapter 28-106, Florida Administrative Code.

8. Pursuant to Section 120.68, Florida Statutes, a person who is adversely affected by final District action may seek review of the action in the District Court of Appeal by filing a notice of appeal pursuant to the Florida Rules of Appellate Procedure, within 30 days of the rendering of the final District action.

9. A party to the proceeding before the District who claims that a District order is inconsistent with the provisions and purposes of Chapter 373, Florida Statutes, may seek review of the order pursuant to Section 373.114, Florida Statutes, by the Florida Land and Water Adjudicatory Commission, by filing a request for review with the Commission and serving a copy of the Department of Environmental Protection and any person named in the order within 20 days of adoption of a rule or the rendering of the District order.

10. For appeals to the District Courts of Appeal, a District action is considered rendered after it is signed on behalf of the District, and is filed by the District Clerk.

11. Failure to observe the relevant time frames for filing a petition for judicial review, or for Commission review, will result in waiver of the right to review.

CERTIFICATE OF SERVICE

I hereby certify that a copy of the foregoing Notice of Rights has been sent by U.S. Mail to:

RAMONA PARK CHURCH
POST OFFICE BOX 1575
LAKE CITY, FL 32056

At 4:00 p.m. this 21 day of Sept, 2010.


Jon M. Dinges
Deputy Clerk

Suwannee River Water Management District
9225 C.R. 49

Permit No.: ERP88-0078M

Project: RAMONA PARK CHURCH SECOND ADDITION

Page 7 of 7

Live Oak, Florida 32060

386.362.1001 or 800.226.1066 (Florida only)

cc: File Number: ERP88-0078M



**Suwannee
River
Water
Management
District**

9225 CR 49
Live Oak, FL 32060
TELEPHONE: 386-362-1001
TELEPHONE: 800-226-1066

Dear Permittee:

Enclosed is your approved Permit. Based on the activity described in your application, Suwannee River Water Management District (District) staff has reasonable assurance that the proposed construction meets conditions for issuance, provided you follow the permit conditions and your stated activity.

The construction of a surfacewater management system requires filing a Notice of Commencement and as-built certification forms within 30 days of completion of construction. These forms are enclosed with your permit.

Please note: your project may require a Generic Permit for Stormwater Discharge for Large and Small Construction Activities, as part of the Florida NPDES stormwater program regulated by the Florida Department of Environmental Protection (FDEP). This permit is separate from the Environmental Resource Permit and must be obtained through FDEP. Please refer to the FDEP program website, <http://www.dep.state.fl.us/water/stormwater/npdes/>, for information about the NPDES permit.

Be aware of the location of underground utilities before starting excavation.

If you wish, we will visit with you on site to discuss the terms of the permit, review existing pre-construction conditions, and answer any questions you may have prior to beginning work. If you would like to schedule a pre-construction meeting, please contact Resource Management staff at 386.362.1001 or 800.226.1066.

Sincerely,

A handwritten signature in black ink, appearing to read "Jon Dinges".

Jon Dinges, P. E.
Department Director

**SUWANNEE RIVER WATER MANAGEMENT DISTRICT
CONSTRUCTION COMMENCEMENT NOTICE**

Suwannee River Water Management District
Department of Resource Management
9225 County Road 49
Live Oak, Florida 32060

PROJECT: _____ PHASE: _____

I hereby notify the Suwannee River Water Management District that the construction of the surface water management system authorized by Environmental Resource Permit No. _____ has commenced/is expected to commence on _____, 20____, and will require duration of approximately ____months/____ weeks/days to complete. It is understood that should the construction term extend beyond one calendar year from the date of the permit issuance, I am obligated to submit the Annual Status Report for Surface Water Management System Construction form number 40B-1.901(15).

Note: If the construction commencement date is not shown, the District should be so notified in writing in order to satisfy permit conditions.

Type or Print Permittee's or Authorized Agent's Name

Phone

Address

Permittee's or Authorized
Agent's Signature

Title Company

Date

AS-BUILT VERIFICATION

(FOR PROJECTS NOT REQUIRING ENGINEERED PLANS)

I hereby notify the Suwannee River Water Management District that construction of the surfacewater management system authorized by permit number

_____, issued on _____, for

_____, has

been built in substantial conformance with the permitted drawings. I further confirm that operation and maintenance of the system will be performed as needed.

Signature

Name (Please print or type)

Company Name

Mailing Address

City, State, Zip Code

Phone Number

Suwannee River Water Management District
9225 County Road 49
Live Oak, Florida 32060
386.362.1001 or 800.226.1066 (Florida only)

Florida Energy Efficiency Code For Building Construction
Florida Department of Community Affairs
EnergyGauge Summit® Fla/Com-2008, Effective: March 1, 2009 -- Form 400A-2008
Method A: Whole Building Performance Method for Commercial Buildings

PROJECT SUMMARY

Short Desc: New Prj	Description: ADDITION TO RAMONA I
Owner: RAMONA PARK CHURCH	
Address1: -	City: LAKE CITY
Address2: -	State: FL here
	Zip: 0
Type: Religious Building	Class: Addition to existing Building
Jurisdiction: COLUMBIA COUNTY, COLUMBIA COUNTY, FL (221000)	
Conditioned Area: 1330 SF	Conditioned & UnConditioned Area: 1330 SF
No of Stories: 1	Area entered from Plans 1368 SF
Permit No: 0	Max Tonnage 4
	If different, write in: _____



SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 1010-12 CONTRACTOR DWIGHT R. HODGES PHONE 752.4089
 THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
MECHANICAL/ A/C <u>1038</u>	Print Name <u>TIM TAYLOR</u> License #: <u>CAC 57170</u>	Signature <u>[Signature]</u> Phone #: <u>386.364.8525</u>
PLUMBING/ GAS	Print Name _____ License #: _____	Signature _____ Phone #: _____
ROOFING	Print Name _____ License #: _____	Signature _____ Phone #: _____
SHEET METAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
FIRE SYSTEM/ SPRINKLER	Print Name _____ License #: _____	Signature _____ Phone #: _____
SOLAR	Print Name _____ License #: _____	Signature _____ Phone #: _____

Specialty License	Contract Number	Subcontractor's Print and Title	Subcontractor's Signature
MASON			
CONCRETE FINISHER			
FRAMING			
INSULATION			
STUCCO			
DRYWALL			
PLASTER			
CABINET INSTALLER			
PAINTING			
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING			
ALUM/VINYL SIDING			
GARAGE DOOR			
METAL BLDG ERECTOR			

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

Florida Energy Efficiency Code For Building Construction

Florida Department of Community Affairs

EnergyGauge Summit® Fla/Com-2008, Effective: March 1, 2009 -- Form 400A-2008

Method A: Whole Building Performance Method for Commercial Buildings

PROJECT SUMMARY

Short Desc: New Prj
Owner: RAMONA PARK CHURCH
Address1: -
Address2: -
Type: Religious Building
Jurisdiction: COLUMBIA COUNTY, COLUMBIA COUNTY, FL (221000)
Conditioned Area: 1330 SF
No of Stories: 1
Permit No: 0

Description: ADDITION TO RAMONA I
City: LAKE CITY
State: FL here
Zip: 0
Class: Addition to existing Building
Conditioned & UnConditioned Area: 1330 SF
Area entered from Plans: 1368 SF
Max Tonnage: 4
If different, write in: _____

28972



Compliance Summary

Component	Design	Criteria	Result
Gross Energy Cost (in \$)	5,033.0	5,197.0	PASSED
LIGHTING CONTROLS			PASSES
EXTERNAL LIGHTING			PASSES
HVAC SYSTEM			PASSES
PLANT			None Entered
WATER HEATING SYSTEMS			None Entered
PIPING SYSTEMS			None Entered
Met all required compliance from Check List?			Yes/No/NA
 IMPORTANT MESSAGE Info 5009 -- -- -- An input report of this design building must be submitted along with this Compliance Report			

CERTIFICATIONS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code

Prepared By: N. P. GEISLER

Building Official: _____

Date: 20 July 2010

Date: _____

I certify that this building is in compliance with the FLorida Energy Efficiency Code

Owner Agent: _____

Date: _____

If Required by Florida law, I hereby certify (*) that the system design is in compliance with the FLorida Energy Efficiency Code

Architect: N. P. GEISLER

Reg No: 20 July 2010
4127005

Electrical Designer: _____

Reg No: _____

Lighting Designer: _____

Reg No: _____

Mechanical Designer: _____

Reg No: _____

Plumbing Designer: _____

Reg No: _____

(*) Signature is required where Florida Law requires design to be performed by registered design professionals.

CERTIFICATIONS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code

Prepared By: N. P. GEISLER

Building Official: _____

Date: 20 July 2010

Date: _____

I certify that this building is in compliance with the FLorida Energy Efficiency Code

Owner Agent: _____

Date: _____

If Required by Florida law, I hereby certify (*) that the system design is in compliance with the FLorida Energy Efficiency Code

Architect: N. P. GEISLER

Reg No: AR7625

Electrical Designer: _____

Reg No: _____

Lighting Designer: _____

Reg No: _____

Mechanical Designer: _____

Reg No: _____

Plumbing Designer: _____

Reg No: _____

(*) Signature is required where Florida Law requires design to be performed by registered design professionals.

Project: New Prj
 Title: ADDITION TO RAMONA PARK CHURCH
 Type: Religious Building
 (WEA File: FL JACKSONVILLE INTL ARPT.tm3)

Building End Uses

	1) Proposed	2) Baseline
Total	344.30	414.10
	\$5,033	\$6,114
ELECTRICITY (MBtu/kWh/\$)	344.30	414.10
	100853	121301
	\$5,033	\$6,114
AREA LIGHTS	13.20	19.50
	3871	5702
	\$193	\$287
MISC EQUIPMT	13.30	13.30
	3897	3897
	\$194	\$196
PUMPS & MISC	0.10	0.10
	22	24
	\$1	\$1
SPACE COOL	109.80	161.10
	32161	47196
	\$1,605	\$2,379
SPACE HEAT	59.30	44.70
	17363	13094
	\$866	\$660
VENT FANS	148.60	175.40
	43539	51388
	\$2,173	\$2,590

Passing requires Proposed Building cost to be at most 85%
 of Baseline cost. This Proposed Building is at 82.3%

PASSES

Project: New Prj
Title: ADDITION TO RAMONA PARK CHURCH
Type: Religious Building
(WEA File: FL JACKSONVILLE INTL ARPT.tm3)

External Lighting Compliance

Description	Category	Tradable?	Allowance (W/Unit)	Area or Length or No. of Units (Sqft or ft)	ELPA (W)	CLP (W)
Ext Light 2	Other (doors) than main entries	Yes	20.00	3.0	60	60
Tradable Surfaces: 60 (W) Allowance for Tradable: 60 (W) All External Lighting: 60 (W)						PASSES

Project: New Prj
Title: ADDITION TO RAMONA PARK CHURCH
Type: Religious Building
(WEA File: FL JACKSONVILLE INTL ARPT.tm3)

Lighting Controls Compliance

Acronym	Ashrae ID	Description	Area (sq.ft)	Design CP	Min CP	Compli- ance
Pr0Zo1Sp1	14	Classroom/Lecture Hall	1,330	7	1	PASSES
						PASSES

Project: New Prj
 Title: ADDITION TO RAMONA PARK CHURCH
 Type: Religious Building
 (WEA File: FL JACKSONVILLE INTL ARPT.tm3)

System Report Compliance

Pr0Sy1 System 1 Constant Volume Air Cooled No. of Units
 Split System < 65000 Btu/hr 1

Component	Category	Capacity	Design Eff	Eff Criteria	Design IPLV	IPLV Criteria	Compliance
Cooling System	Air Conditioners Air Cooled Split System < 65000 Btu/h		14.00	12.00	8.00		PASSES
Heating System	Cooling Capacity Heat Pumps Air Cooled (Heating Mode) Split System < 65000 Btu/h Cooling Capacity		8.00	7.40			PASSES
Air Handling System - Supply	Air Handler (Supply) - Constant Volume		0.60	0.90			PASSES
Air Handling System - Return	Air Handler (Return) - Constant Volume		0.60	0.90			PASSES
Air Distribution System	ADS System		4.20				PASSES

PASSES

Plant Compliance

Description	Installed No	Size	Design Eff	Min Eff	Design IPLV	Min IPLV	Category	Compliance
None								

Water Heater Compliance

Description	Type	Category	Design Eff	Min Eff	Design Loss	Max Loss	Compliance
None							

Piping System Compliance							
Category	Pipe Dia [inches]	Is Runout?	Operating Temp [F]	Ins Cond [Btu-in/hr .SF.F]	Ins Thick [in]	Req Ins Thick [in]	Compliance
							None

Project: New Prj
Title: ADDITION TO RAMONA PARK CHURCH
Type: Religious Building
(WEA File: FL_JACKSONVILLE_INTL_ARPT.tn3)

Other Required Compliance

Category	Section	Requirement (write N/A in box if not applicable)	Check
Report	13-101	Input Report Print-Out from EnergyGauge FlaCom attached	<input type="checkbox"/>
Operations Manual	13-102.1, 13-410, 13-413	Operations manual provided to owner	<input type="checkbox"/>
Windows & Doors	13-406.AB.1.1	Glazed swinging entrance & revolving doors: max. 1.0 cfm/ft ² ; all other products: 0.4 cfm/ft ²	<input type="checkbox"/>
Joints/Cracks	13-406.AB.1.2	To be caulked, gasketed, weather-stripped or otherwise sealed	<input type="checkbox"/>
Dropped Ceiling Cavity System	13-406.AB.3 13-407	Vented: seal & insulated ceiling. Unvented seal & insulate roof & side walls HVAC Load sizing has been performed	<input type="checkbox"/> <input type="checkbox"/>
Reheat	13-407.B	Electric resistance reheat prohibited	<input type="checkbox"/>
HVAC Efficiency	13-407, 13-408	Minimum efficiencies: Cooling Tables 13-407.AB.3.2.1A-D; Heating Tables 13-407.AB.3.2.1B, 13-407.AB.3.2.1D, 13-408.AB.3.2.1E, 13-408.AB.3.2F	<input type="checkbox"/>
HVAC Controls	13-407.AB.2	Zone controls prevent reheat (exceptions); simultaneous heating and cooling in each zone; combined HAC deadband of at least 5°F (exceptions)	<input type="checkbox"/>
Ventilation Controls	13-409.AB.3	Motorized dampers reqd, except gravity dampers OK in: 1) exhaust systems and 2) systems with design outside air intake or exhaust capacity ≤300 cfm	<input type="checkbox"/>
ADS	13-410	Duct sizing and Design have been performed	<input type="checkbox"/>
HVAC Ducts	13-410.AB	Air ducts, fittings, mechanical equipment & plenum chambers shall be mechanically attached, sealed, insulated & installed per Sec. 13-410 Air Distribution Systems	<input type="checkbox"/>
Balancing	13-410.AB.4	HVAC distribution system(s) tested & balanced. Report in construction documents	<input type="checkbox"/>
Piping Insulation	13-411.AB	In accordance with Table 13-411.AB.2	<input type="checkbox"/>
Water Heaters	13-412.AB	Performance requirements in accordance with Table 13-412.AB.3. Heat trap required	<input type="checkbox"/>
Swimming Pools	13-412.AB.2.6	Cover on heated swimming pools: Time switch (exceptions); Readily accessible on/off switch	<input type="checkbox"/>
Hot Water Pipe Insulation	13-411.AB.3	Table 13-411.AB.2 for circulating systems, first 8 feet of outlet pipe from storage tank and between inlet pipe and heat trap	<input type="checkbox"/>
Water Fixtures	13-412.AB.2.5	Shower hot water flow restricted to 2.5 gpm at 80 psi. Public lavatory fixture hot water flow 0.5 gpm max; if self-closing valve 0.25 gallon recirculating, 0.5 gallon non recirculating	<input type="checkbox"/>
Motors	13-414	Motor efficiency criteria have been met	<input type="checkbox"/>
Lighting Controls	13-415.AB	Automatic control required for interior lighting in buildings >5,000 s.f.; Space control; Exterior photo sensor; Tandem wiring with 1 or 3 linear fluourescent lamps>30W	<input type="checkbox"/>

EnergyGauge Summit® v3.20
INPUT DATA REPORT

Project Information

Project Name: New Pri

Orientation: East

Project Title: ADDITION TO RAMONA PARK CHURCH

Building Type: Religious Building

Address: -

Building Classification: Addition to existing Building

State: FL here

No.of Stories: 1

Zip: 0

GrossArea: 1330

SF

Owner: RAMONA PARK CHURCH

Zones

No	Acronym	Description	Type	Area [sf]	Multiplier	Total Area [sf]
1	PrOZol	Zone 1	CONDITIONED	1330.0	1	1330.0

☐

Spaces

No	Acronym	Description	Type	Depth [ft]	Width [ft]	Height [ft]	Multi plier	Total Area [sf]	Total Volume [cf]
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In Zone: Pr0Zo1		Classroom/Lecture Hall	38.00	35.00	10.67	1	1330.0	14191.1	<input type="checkbox"/>
1	Pr0Zo1Sp1	Zo0Sp1							

Lighting

No	Type	Category	No. of Luminaires	Watts per Luminaire	Power [W]	Control Type	No. of Ctrl pts
In Zone: Pr0Zo1							
In Space: Pr0Zo1Sp1							
1	Compact Fluorescent	General Lighting	8	128	1024	Manual On/Off	5 <input type="checkbox"/>
2	Incandescent	General Lighting	4	60	240	Manual On/Off	2 <input type="checkbox"/>

Walls

No	Description	Type	Width H [ft]	(Effic) [ft]	Multi plier	Area [sf]	Direction	Conductance [Btu/hr. sf. F]	Heat Capacity [Btu/sf.F]	Dens. [lb/cf]	R-Value [h.s.f.F/Btu]
In Zone: Pr0Zo1											
1	Pr0Zo1Wa1	Metal siding/2x4@24"+R1 1Batt/5/8"Gyp	35.00	1,067.00	1	37345.0	East	0.0920	1.072	19.38	10.9 <input type="checkbox"/>
2	Pr0Zo1Wa2	Metal siding/2x4@24"+R1 1Batt/5/8"Gyp	38.00	10.67	1	405.5	North	0.0920	1.072	19.38	10.9 <input type="checkbox"/>
3	Pr0Zo1Wa3	Metal siding/2x4@24"+R1 1Batt/5/8"Gyp	35.00	10.67	1	373.5	West	0.0920	1.072	19.38	10.9 <input type="checkbox"/>
4	Pr0Zo1Wa4	Metal siding/2x4@24"+R1 1Batt/5/8"Gyp	38.00	10.67	1	405.5	South	0.0920	1.072	19.38	10.9 <input type="checkbox"/>

Windows

No	Description	Type	Shaded	U [Btu/hr sf F]	SHGC	Vis. Tra	W [ft]	H (Effec) [ft]	Multi plier	Total Area [sf]
In Zone: Pr0Z01										
In Wall: Pr0Z01Wa1										
1	Pr0Z01Wa1Wi1	User Defined	No	1.2500	0.82	0.76	6.33	5.25	1	33.2
In Wall: Pr0Z01Wa2										
1	Pr0Z01Wa2Wi1	User Defined	No	0.9000	0.50	0.40	3.00	6.67	2	40.0
<input type="checkbox"/>										

Doors

No	Description	Type	Shaded?	Width [ft]	H (Effec) [ft]	Multi plier	Area [sf]	Cond. [Btu/hr. sf. F]	Dens. [lb/cf]	Heat Cap. [Btu/sf. F]	R-Value [h.s.f.F/Btu]
----	-------------	------	---------	---------------	-------------------	----------------	--------------	--------------------------	------------------	--------------------------	--------------------------

In Zone:

In Wall:

☐

Roofs

No	Description	Type	Width [ft]	H (Effec) [ft]	Multi plier	Area [sf]	Tilt [deg]	Cond. [Btu/hr. Sf. F]	Heat Cap [Btu/sf. F]	Dens. [lb/cf]	R-Value [h.s.f.F/Btu]
In Zone: Pr0Z01											
1	Pr0Z01Rf1	Shngl/1/2"WD Deck/WD Truss/9" Batt/Gyp Brd	35.00	21.25	1	743.8	27.00	0.0320	1.50	8.22	31.2
<input type="checkbox"/>											
2	Pr0Z01Rf2	Shngl/1/2"WD Deck/WD Truss/9" Batt/Gyp Brd	35.00	21.25	1	743.8	27.00	0.0320	1.50	8.22	31.2
<input type="checkbox"/>											

Skylights

No	Description	Type	U [Btu/hr sf F]	SHGC	Vis. Trans	W [ft]	H (Effec) [ft]	Multiplier	Area [Sf]	Total Area [Sf]
----	-------------	------	--------------------	------	------------	-----------	-------------------	------------	--------------	--------------------

In Zone:

In Roof:

☐

Floors

No	Description	Type	Width [ft]	H (Effec) [ft]	Multi plier	Area [sf]	Cond. [Btu/hr. sf. F]	Heat Cap. [Btu/sf. F]	Dens. [lb/cf]	R-Value [h.s.f.F/Btu]
----	-------------	------	---------------	-------------------	----------------	--------------	--------------------------	--------------------------	------------------	--------------------------

In Zone: Pr0Z01
1 Pr0Z01FI1

1 ft. soil, concrete
floor, carpet and
rubber pad

35.00

38.00

1

1330.0

0.2681

34.00

113.33

3.73

☐

Systems

Pr0Sy1

System 1

Constant Volume Air Cooled Split
System < 65000 Btu/hr

No. Of Units 1

Component	Category	Capacity	Efficiency	IPLV
1	Cooling System	48000.00	14.00	8.00
2	Heating System	46000.00	8.00	
3	Air Handling System - Supply	1600.00	0.60	
4	Air Handling System - Return	1600.00	0.60	
5	Air Distribution System		4.20	

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☐

Plant

Equipment	Category	Size	Inst.No	Eff.	IPLV
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☐

Water Heaters

W-Heater Description

CapacityCap. Unit

I/P Rt.

Efficiency

Loss

☐

Ext-Lighting

Description

Category

No. of
Luminaires

Watts per
Luminaire

Area/Len/No. of units
[sf/ft/No]

Control Type

Wattage
[W]

1 Ext Light 2

Other (doors) than main
entries

1

60

3.00

Photo Sensor control

60.00

☐

Piping

No Type

Operating
Temperature
[F]

Insulation
Conductivity
[Btu-in/h.sf.F]

Nomonal pipe
Diameter
[in]

Insulation
Thickness
[in]

Is Runout?

☐

Fenestration Used

Name

Glass Type

No. of
Panels

Glass
Conductance
[Btu/h.sf.F]

SHGC

VLT

ASHULSglClrAl
IFrm
ASHULDbtTnM
ti-Ofh frm

User Defined

1

1.2500

0.8200

0.7600

☐

User Defined

2

0.9000

0.5000

0.4000

☐

Materials Used

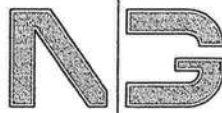
Mat No	Acronym	Description	Only R-Value Used	RValue [h.sf.F/Btu]	Thickness [ft]	Conductivity [Btu/h.ft.F]	Density [lb/cf]	SpecificHea t	
187	Mat187	GYP OR PLAS BOARD, 1/2IN	No	0.4533	0.0417	0.0920	50.00	0.2000	<input type="checkbox"/>
178	Mat178	CARPET W/RUBBER PAD	Yes	1.2300	1.0000	0.5000	100.00	0.2000	<input type="checkbox"/>
265	Mat265	Soil, 1 ft	No	2.0000	0.5000	1.0000	140.00	0.2000	<input type="checkbox"/>
48	Mat48	6 in. Heavyweight concrete	No	0.5000	0.2500	0.0250	2.00	0.2000	<input type="checkbox"/>
12	Mat12	3 in. Insulation	No	10.0000	0.5000	0.0250	5.70	0.2000	<input type="checkbox"/>
23	Mat23	6 in. Insulation	No	20.0000	0.0050	26.0000	480.00	0.1000	<input type="checkbox"/>
4	Mat4	Steel siding	No	0.0002	0.0050	0.0280	7.11	0.2000	<input type="checkbox"/>
271	Mat271	2x4@24" oc + R11 Batt	No	10.4179	0.2917				<input type="checkbox"/>
81	Mat81	ASPHALT-ROOFING, ROLL	Yes	0.1500					<input type="checkbox"/>
244	Mat244	PLYWOOD, 1/2IN	No	0.6318	0.0417	0.0660	34.00	0.2900	<input type="checkbox"/>

Constructs Used

No	Name	Simple Construct	Massless Construct	Conductance [Btu/h.sf.F]	Heat Capacity [Btu/sf.F]	Density [lb/cf]	RValue [h.sf.F/Btu]	
1038	Shngl/1/2"WD Deck/WD Truss/9" Batt/Gyp Brd	No	No	0.03	1.50	8.22	31.2	<input type="checkbox"/>
	Layer	Material No.	Material	Thickness [ft]	Framing Factor			
	1	81	ASPHALT-ROOFING, ROLL		0.000			<input type="checkbox"/>
	2	244	PLYWOOD, 1/2IN	0.0417	0.000			<input type="checkbox"/>
	3	12	3 in. Insulation	0.2500	0.000			<input type="checkbox"/>
	4	23	6 in. Insulation	0.5000	0.000			<input type="checkbox"/>
	5	187	GYP OR PLAS BOARD, 1/2IN	0.0417	0.000			<input type="checkbox"/>

No	Name	Simple Construct	Massless Construct	Conductance [Btu/h.sf.F]	Heat Capacity [Btu/sf.F]	Density [lb/cf]	RValue [h.sf.F/Btu]	
1055	Metal siding/2x4@24"+R11Batt/5/8"Gyp	No	No	0.09	1.07	19.38	10.9	<input type="checkbox"/>
Layer	Material No.	Material	Thickness [ft]	Framing Factor				
1	4	Steel siding	0.0050	0.000	<input type="checkbox"/>			
2	271	2x4@24" oc + R11 Batt	0.2917	0.000	<input type="checkbox"/>			
3	187	GYP OR PLAS BOARD, 1/2IN	0.0417	0.000	<input type="checkbox"/>			
No	Name	Simple Construct	Massless Construct	Conductance [Btu/h.sf.F]	Heat Capacity [Btu/sf.F]	Density [lb/cf]	RValue [h.sf.F/Btu]	
1057	1 ft. soil, concrete floor, carpet and rubber pad	No	No	0.27	34.00	113.33	3.7	<input type="checkbox"/>
Layer	Material No.	Material	Thickness [ft]	Framing Factor				
1	265	Soil, 1 ft	1.0000	0.000	<input type="checkbox"/>			
2	48	6 in. Heavyweight concrete	0.5000	0.000	<input type="checkbox"/>			
3	178	CARPET W/RUBBER PAD		0.000	<input type="checkbox"/>			

28972



NICHOLAS
PAUL
GEISLER
ARCHITECT
N.C.A.R.B. Certified

1758 NW Brown Rd.
Lake City, FL 32055
386/755-6608

22 NOVEMBER 2010

BUILDING OFFICIAL
COLUMBIA COUNTY BUILDING DEPARTMENT
COLUMBIA COUNTY COURTHOUSE ANNEX
LAKE CITY, FLORIDA

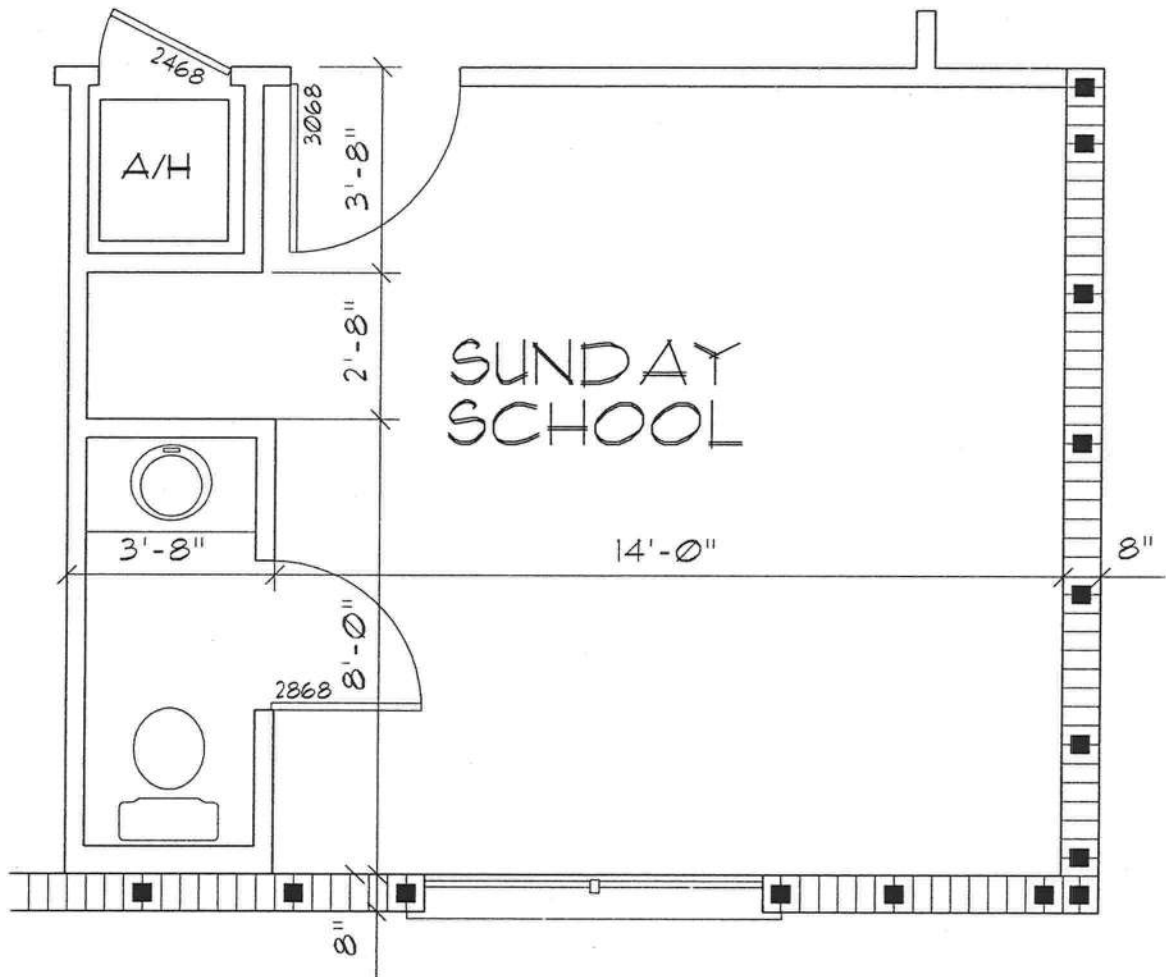
RE: RAMONA PARK CHURCH ADDITION
PERMIT Nr.: 1010-12

DEAR SIR:

PLEASE BE ADVISED THAT THE FLOOR PLAN FOR THE ABOVE
REFERENCED PROJECT HAS BEEN MODIFIED TO INCLUDE A
1/2 BATH IN THE SUNDAY SCHOOL ROOM AS SHOWN ON
ATTACHMENT 'A'.

SHOULD YOU HAVE ANY QUESTIONS, PLEASE DO NOT HESITATE TO
CALL FOR ANY ASSISTANCE.

YOURS TRULY,
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005



New Sunday School Restroom

SCALE : NONE

[Handwritten signature]

AR1005 23NOV 2010

ATTACHMENT 'A'



Columbia County

BUILDING DEPARTMENT

MINIMUM PLAN REQUIREMENTS FOR THE
FLORIDA BUILDING CODE ,FLORIDA PLUMBING CODE,FLORIDA MECHINICAL
CODE,FLORIDA FUEL AND GAS CODE 2007 , NATIONAL ELECTRICAL 2005
ALL REQUIREMENTS ARE SUBJECT TO CHANGE

COMMERCIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST

**ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE
CURRENT FLORIDA BUILDING CODES. ALL PLANS OR DRAWING SHALL
PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND
SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED
IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES,
APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION.**

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FBC FIGURE 1609 STATE OF FLORIDA WIND SPEED MAP

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75
ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ----- 110 MPH
NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS:		Items to Include- Each Box shall be Circled as Applicable		
1	All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void.	YES	NO	N/A
2	If the design professional is an architect or engineer legally registered under the laws of this state regulating the practice of architecture as provided for in Chapter 481, Florida Statutes, Part I, or engineering as provided for in Chapter 471, Florida Statutes, then he or she shall affix his or her official seal to said drawings, specifications and accompanying data, as required by Florida Statute.	YES	NO	N/A
3	The design professional signature shall be affixed to the plans	YES	NO	N/A
4	Two (2) complete sets of plans with the architecture or engineer signature and the date the affix embossed official seal was placed on the plans	YES	NO	N/A

Building Site Plan Requirements										Items to Include- Each Box shall be Circled as Applicable		
4	Parking, including provision FBC chapter 11 for the required accessible parking site									Yes	No	N/A
5	Fire access, showing all drive way which will be accessible for emergency vehicles									Yes	No	N/A
6	Driving/turning radius of parking lots									Yes	No	N/A
7	Vehicle loading include truck dock loading or rail site loading									Yes	No	N/A
8	Nearest or number of onsite Fire hydrant/water supply/post indicator valve (PIV)									Yes	No	N/A
9	Set back of all existing or proposed structures from each structure and property boundaries, Show all separation including assumed property lines									Yes	No	N/A
10	Location of specific tanks(above or under grown ,water lines and sewer lines and septic tank and drain fields									Yes	No	N/A
11	All structures exterior views include finished floor elevation									Yes	No	N/A
12	Total height of structure(s) form established grade									Yes	No	N/A
Occupancy group use circle all uses:		Group A	Group B	Group E	Group F	Group H	Group I	Group M	Group R	Group S	Group U D	
13	Special occupancy requirements.									Yes	No	N/A
14	Incidental use areas (total square footage for each room of use area)									Yes	No	N/A
15	Mixed occupancies									Yes	No	N/A
16	REQUIRED SEPARATION OF OCCUPANCIES IN HOURS FBC TABLE 302.3.2									Yes	No	N/A
Minimum type of permitted construction by code for occupancy use circle the construction type FBC 602												
17	Type I	Type II	Type III	Type IV	Type V							
Fire-resistant construction requirements shall be shown, include the following components												
18	Fire-resistant separations									Yes	No	N/A
19	Fire-resistant protection for type of construction									Yes	No	N/A
20	Protection of openings and penetrations of rated walls									Yes	No	N/A
21	Protection of openings and penetrations of rated walls									Yes	No	N/A
22	Fire blocking and draftstopping and calculated fire resistance									Yes	No	N/A
Fire suppression systems shall be shown include:												
23	Early warning smoke evacuation systems Schematic fire sprinklers Standpipes									Yes	No	N/A
24	Standpipes									Yes	No	N/A
25	Pre-engineered systems									Yes	No	N/A
26	Riser diagram									Yes	No	N/A
Life safety systems shall be shown include the following requirements:												
27	Occupant load and egress capacities									Yes	No	N/A
28	Early warning									Yes	No	N/A
29	Smoke control									Yes	No	N/A
30	Stair pressurization									Yes	No	N/A
31	Systems schematic									Yes	No	N/A
Occupancy load/egress requirements shall be shown include:												
32	Occupancy load									Yes	No	N/A
33	Gross occupancy load									Yes	No	N/A
34	Net occupancy load									Yes	No	N/A
35	Means of egress									Yes	No	N/A
36	Exit access									Yes	No	N/A
37	Exit discharge									Yes	No	N/A
38	Stairs construction/geometry and protection									Yes	No	N/A
39	Doors									Yes	No	N/A
40	Emergency lighting and exit signs									Yes	No	N/A
41	Specific occupancy requirements									Yes	No	N/A
42	Construction requirements									Yes	No	N/A
43	Horizontal exits/exit passageways									Yes	No	N/A

**Items to Include-
Each Box shall
be Circled as
Applicable**

Structural requirements shall be shown include:				
44	Soil conditions/analysis	Yes	No	N/A
45	Termite protection	Yes	No	N/A
46	Design loads	Yes	No	N/A
47	Wind requirements	Yes	No	N/A
48	Building envelope	Yes	No	N/A
49	Structural calculations (if required)	Yes	No	N/A
50	Foundation	Yes	No	N/A
51	Wall systems	Yes	No	N/A
52	Floor systems	Yes	No	N/A
53	Roof systems	Yes	No	N/A
54	Threshold inspection plan	Yes	No	N/A
55	Stair systems	Yes	No	N/A
Materials shall be shown include the following				
56	Wood	Yes	No	N/A
57	Steel	Yes	No	N/A
58	Aluminum	Yes	No	N/A
59	Concrete	Yes	No	N/A
60	Plastic	Yes	No	N/A
61	Glass	Yes	No	N/A
62	Masonry	Yes	No	N/A
63	Gypsum board and plaster	Yes	No	N/A
64	Insulating (mechanical)	Yes	No	N/A
65	Roofing	Yes	No	N/A
66	Insulation	Yes	No	N/A
Accessibility requirements shall be shown include the following				
67	Site requirements	Yes	No	N/A
68	Accessible route	Yes	No	N/A
69	Vertical accessibility	Yes	No	N/A
70	Toilet and bathing facilities	Yes	No	N/A
71	Drinking fountains	Yes	No	N/A
72	Equipment	Yes	No	N/A
73	Special occupancy requirements	Yes	No	N/A
74	Fair housing requirements	Yes	No	N/A
Interior requirements shall include the following				
75	Interior finishes (flame spread/smoke development)	Yes	No	N/A
76	Light and ventilation	Yes	No	N/A
77	Sanitation	Yes	No	N/A
Special systems				
78	Elevators	Yes	No	N/A
79	Escalators	Yes	No	N/A
80	Lifts	Yes	No	N/A
Swimming pools				
81	Barrier requirements	Yes	No	N/A
82	Spas	Yes	No	N/A
83	Wading pools	Yes	No	N/A

Items to Include-Each Box shall be Circled as Applicable

Electrical			
84	Wiring	Yes	No <u>N/A</u>
85	Services	Yes	No <u>N/A</u>
86	Feeders and branch circuits	Yes	No <u>N/A</u>
87	Overcurrent protection	Yes	No <u>N/A</u>
88	Grounding	Yes	No <u>N/A</u>
89	Wiring methods and materials	Yes	No <u>N/A</u>
90	GFCIs	Yes	No <u>N/A</u>
91	Equipment	Yes	No <u>N/A</u>
92	Special occupancies	Yes	No <u>N/A</u>
93	Emergency systems	Yes	No <u>N/A</u>
94	Communication systems	Yes	No <u>N/A</u>
95	Low voltage	Yes	No <u>N/A</u>
96	Load calculations	Yes	No <u>N/A</u>
Plumbing			
97	Minimum plumbing facilities	Yes	No <u>N/A</u>
98	Fixture requirements	Yes	No <u>N/A</u>
99	Water supply piping	Yes	No <u>N/A</u>
100	Sanitary drainage	Yes	No <u>N/A</u>
101	Water heaters	Yes	No <u>N/A</u>
102	Vents	Yes	No <u>N/A</u>
103	Roof drainage	Yes	No <u>N/A</u>
104	Back flow prevention	Yes	No <u>N/A</u>
105	Irrigation	Yes	No <u>N/A</u>
106	Location of water supply line	Yes	No <u>N/A</u>
107	Grease traps	Yes	No <u>N/A</u>
108	Environmental requirements	Yes	No <u>N/A</u>
109	Plumbing riser	Yes	No <u>N/A</u>
Mechanical			
110	Energy calculations	Yes	No <u>N/A</u>
111	Exhaust systems	Yes	No <u>N/A</u>
112	Clothes dryer exhaust	Yes	No <u>N/A</u>
113	Kitchen equipment exhaust	Yes	No <u>N/A</u>
114	Specialty exhaust systems	Yes	No <u>N/A</u>
Equipment location			
115	Make-up air	Yes	No <u>N/A</u>
116	Roof-mounted equipment	Yes	No <u>N/A</u>
117	Duct systems	Yes	No <u>N/A</u>
118	Ventilation	Yes	No <u>N/A</u>
119	Laboratory	Yes	No <u>N/A</u>
120	Combustion air	Yes	No <u>N/A</u>
121	Chimneys, fireplaces and vents	Yes	No <u>N/A</u>
122	Appliances	Yes	No <u>N/A</u>
123	Boilers	Yes	No <u>N/A</u>
124	Refrigeration	Yes	No <u>N/A</u>
125	Bathroom ventilation	Yes	No <u>N/A</u>

Items to Include-Each Box shall be Circled as Applicable

Gas			
126	Gas piping	Yes	No <input checked="" type="radio"/> N/A
127	Venting	Yes	No <input checked="" type="radio"/> N/A
128	Combustion air	Yes	No <input checked="" type="radio"/> N/A
129	Chimneys and vents	Yes	No <input checked="" type="radio"/> N/A
130	Appliances	Yes	No <input checked="" type="radio"/> N/A
131	Type of gas	Yes	No <input checked="" type="radio"/> N/A
132	Fireplaces	Yes	No <input checked="" type="radio"/> N/A
133	LP tank location	Yes	No <input checked="" type="radio"/> N/A
134	Riser diagram/shutoffs	Yes	No <input checked="" type="radio"/> N/A
Notice of Commencement			
135	A recorded (in the Columbia County Clerk Office) notice of commencement is required to be on file with the building department. <i>Before Any Inspections Will Be Done</i>	Yes	No N/A
Disclosure Statement for Owner Builders			
		Yes	No N/A

Private Potable Water			
136	Horse power of pump motor	Yes	No N/A
137	Capacity of pressure tank	Yes	No N/A
138	Cycle stop valve if used	Yes	No N/A

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

139	Building Permit Application	A current Building Permit Application form is to be completed and submitted for all construction projects.	<input checked="" type="radio"/> Yes	No	N/A
140	Parcel Number	The parcel number (Tax ID number) from the Property Appraiser is required. A copy of property deed is also requested. (386) 758-1084 <i>ORB 237-79 CLERKS OFFICE</i>	Yes	No	N/A
141	Environmental Health Permit or Sewer Tap Approval	A copy of an approved Environmental Health (386) 758-1058 waste water disposal permit or an approved City of Lake City (386) 752-2031 sewer tap is required before a building permit can be issued. Toilet facilities shall be provided for construction workers	Yes	No	N/A
142	Driveway Connection	If the property does not have an existing access to a public road, then an application for a culvert permit must be made (\$25.00). Culvert installation for commercial, industrial and other uses shall conform to the approved site plan or to the specifications of a registered engineer. Use or joint use of driveways will comply with Florida Department of Transportation specifications. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	Yes	No	<input checked="" type="radio"/> N/A
143	Suwannee River Water Management District Approval	All commercial projects must have an SRWMD permit issued or an exemption letter, before a building permit will be issued.	<input checked="" type="radio"/> Yes	No	N/A

144	Flood Management	Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of section 8.5.2 of the Columbia County Land Development Regulations. Any project that is located within a flood zone where the base flood elevation (100 year flood) has not been established shall meet the requirements of section 8.5.3 of Columbia County Land Development Regulations. A development permit will also be required. The development permit cost is \$50.00	Yes	No	N/A
145	Flood Management	A CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.	Yes	No	N/A
146	911 Address	If the project is located in an area where a 911 address has not been issued, then application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125	Yes	No	N/A

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

Section 105 of the Florida Building Code defines the:

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became null and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date of issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

When the submitted application is approved for permitting the applicant will be notified by phone as to the date and time a building permit will be prepared and issued by the Columbia County Building & Zoning Department.

PRODUCT APPROVAL SPECIFICATION SHEET

Location: _____ **Project Name:** _____

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles			FL 9631.7
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

Contractor or Contractor's Authorized Agent Signature

Print Name

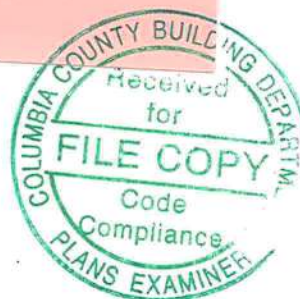
Date

Location

Permit # (FOR STAFF USE ONLY)

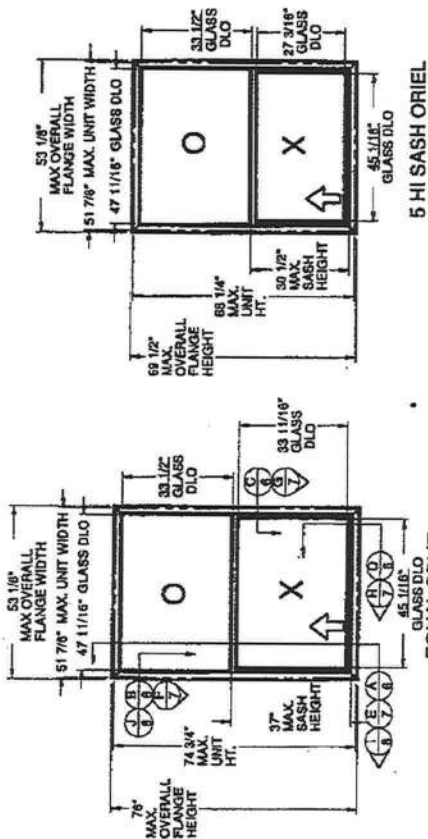
4091.1	8100 Vinyl Flange/Fin Frame Single Hung	8100 Vinyl Flange/Fin Frame Single Hung
Limits of Use Approved for use in HVHZ: No Approved for use outside HVHZ: Yes Impact Resistant: No Design Pressure: +50/-50 Other: 8100 Vinyl Flange/Fin Frame Single Hung H-LC50 (53" x 76")		Installation Instructions FL4091_R4_II_CWS-154B (SH-8100 Non-Impact).pdf Verified By: Roberto Lomas 62514 Created by Independent Third Party: Yes Evaluation Reports FL4091_R4_AE_510856A (Eval Rep_SH-8100 Non-Impact 53 x 76).pdf Created by Independent Third Party: Yes

Shingle
Approval
- FL-9631.7



SINGLE HUNG ELEVATION (NON-IMPACT)

(SHOWN W/ DIFFERENT OPTIONS)



EQUAL SPLIT

5 HI SASH ORIEL

60/40 SPLIT ORIEL

4 HI SASH ORIEL

TABLE OF CONTENTS

GENERAL NOTES & ELEVATIONS	1
ANCHOR SCHEDULE & NOTES	2
CHARTS & GLAZING DETAIL	3
SECTIONAL ELEVATIONS & ALT. FRAME	4
EXTRUSIONS & DETAIL	5
INSTALLATION DETAIL	6-8

PVC NON-IMPACT REQUIREMENTS:

1. THE PRODUCT SHOWN HEREIN IS DESIGNED AND MANUFACTURED TO COMPLY WITH THE FLORIDA BUILDING CODE, CURRENT EDITION.
2. GLAZING OPTIONS: (SEE SHEET 3)
3. CONFIGURATIONS: "OX".
4. DESIGN PRESSURE RATINGS (SEE SHEET 3):
NEGATIVE DESIGN LOADS BASED ON, TESTED PRESSURE AND GLASS TABLES ASTM E 1300.
POSITIVE DESIGN LOADS BASED ON, TESTED PRESSURE, WATER INFILTRATION TEST PRESSURE AND GLASS TABLES ASTM E 1300.
5. ANCHORAGE: THE 33 1/4" STRESS INCREASE HAS NOT BEEN USED IN THE DESIGN OF THIS PRODUCT. SEE SHEETS 6-8 FOR ANCHOR DETAIL.
6. NOT APPROVED FOR IMPACT RESISTANCE. IMPACT PROTECTIVE SYSTEM IS REQUIRED IN WIND BORNE DEBRIS REGION.
7. ALL FRAMES AND VENTS FULLY WELDED. SMALL JOINT SEAM SEALANT USED AT FIXED MEETING RAIL AND JAMB.
8. REFERENCE TEST REPORTS: NCTI-210-3522-1, NCTI-210-3522-1A
9. SERIES / MODEL DESIGNATION SH-8100.
10. WOOD BUCK OR STUD FRAMING TO BE DESIGNED AND ANCHORED TO PROPERLY TRANSFER ALL LOADS TO STRUCTURE. BUCK OR FRAMING IS RESPONSIBILITY OF ARCHITECT OR ENGINEER OF RECORD.
11. THE DESIGNATION X AND O STAND FOR THE FOLLOWING: X - OPERABLE SASH, O - FIXED SASH
12. SECTION CUTOFFS FROM EQUAL SPLIT ELEVATION APPLY TO ALL ELEVATIONS IN A SIMILAR LOCATION.

B	ADDED COMP. ANALYSIS CHART	12/18/08
A	UPDATED PER NEW FBC REQUIREMENTS	07/07/08


Centum WINDOW SYSTEMS, INC.	
1900 SW 44TH AVE.	
MIAMI, FLORIDA 33147	
PROJECT:	8100
MODEL:	PVC SINGLE HUNG
SCALE:	AS SHOWN
DATE:	12/18/2008
REV. NO.:	CWS-154
REV. LETTER:	B
SHEET:	1 OF 8

PROPERTY AND COPYRIGHT
THE INFORMATION CONTAINED IN THIS DRAWING IS THE SOLE PROPERTY OF CUSTOMER WINDOW SYSTEMS, INC. ANY REPRODUCTION IN PART OR AS A WHOLE WITHOUT THE WRITTEN PERMISSION OF CUSTOM WINDOW SYSTEMS, INC. IS PROHIBITED.



1. INSTALL ONE ANCHOR AT EACH INSTALLATION LOCATION.
2. SHIMS BE USED AT EACH SET OF INSTALLATION ANCHORS USING LOAD BEARING SHIMS. MAX. ALLOWABLE SHIM STACK TO BE 1/4". USE SHIMS WHERE SPACE GREATER THAN 1/16" IS PRESENT.
3. LOAD BEARING SHIMS SHALL BE CONSTRUCTED OF HIGH DENSITY PLASTIC OR BETTER. WOOD SHIMS ARE NOT ALLOWED.
4. 3/16" DIA. ELCO TAPCON MASONRY ANCHORS MUST BE OF SUFFICIENT LENGTH TO ACHIEVE A MIN. EMBEDMENT OF 1 1/4" INTO MASONRY THRU FRAME OR THRU 1 BY WOOD BUCK INTO MASONRY. A MINIMUM EDGE DISTANCE OF 2 1/2" SHALL BE ACHIEVED.
5. #10 WOOD SCREW ANCHORS MUST BE OF SUFFICIENT LENGTH TO ACHIEVE A MIN. EMBEDMENT OF 1 3/8" INTO 2 BY WOOD BUCK.
6. #6 WOOD SCREW ANCHORS MUST BE OF SUFFICIENT LENGTH TO ACHIEVE A MIN. EMBEDMENT OF 1 1/2" INTO 2 BY WOOD BUCK FOR FIN INSTALLATION
7. SEAL CORNERS WITH SMALL JOINT SEAM SEALANT.
8. ALL INSTALLATION ANCHORS MUST BE MADE OF A CORROSION RESISTANT MATERIAL OR COATING.
9. INSTALLATION ANCHORS SHALL BE IN ACCORDANCE WITH ANCHOR MANUFACTURER'S INSTALLATION INSTRUCTIONS, AND ANCHORS SHALL NOT BE USED IN SUBSTRATES WITH STRENGTHS LESS THAN THE MINIMUM STRENGTH SPECIFIED BELOW.
10. INSTALLATION ANCHOR CAPACITIES FOR PRODUCTS HEREIN ARE BASED ON SUBSTRATE MATERIAL WITH THE FOLLOWING PROPERTIES:
 - A. WOOD - MINIMUM SPECIFIC GRAVITY OF G=0.42
 - B. CONCRETE - MINIMUM COMPRESSIVE STRENGTH OF 3,200 PSI.
 - C. MASONRY - STRENGTH CONFORMANCE TO ASTM C-90, GRADE N, TYPE 1 (OR GREATER).

B	ADDED COMP. ANALYSIS CHART	12/18/08
A	UPDATED PER NEW FBC REQUIREMENTS	07/07/08



1600 SW 44TH AVE.
DADELA, FLORIDA 33474

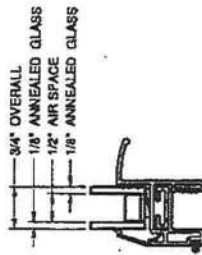
TUBS - ANCHOR SCHEDULE & NOTES - NON IMPACT	
DRAWN BY:	DATE: 12/18/2008
SCALE:	DWG. NO.: CWS-154
REV. LETTER: B	SHEET: 2 OF 8

MODEL: RVC SINGLE HUNG	ANCHOR: 8100
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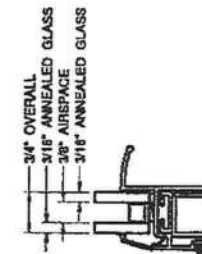
PROPRIETARY AND CONFIDENTIAL

Design pressures for units glazed with IGU 1/8" Annealed (Type A), 3/16" Annealed (Type B), IGU 1/8" Tempered (Type C) or IGU 3/16" Tempered (Type D)

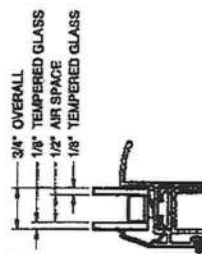
Height (in)	23.5	27.5	28.5	31.5	35.5	39.5	41.5	43.5	47.5	51.5	53.125
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28.5	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75
31.5	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75
35.5	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75
39.5	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75
41.5	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75
43.5	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75
47.5	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75
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71.5	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75
74.75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75	+50/-75
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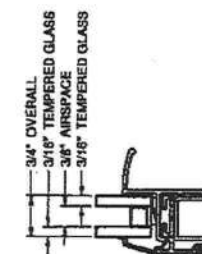
GLASS TYPE A



GLASS TYPE B



GLASS TYPE C



GLASS TYPE D

NOTE:
1. IF SIZE INTENDED IS NOT SHOWN,
USE NEXT LARGER SIZE.

B ADDED COMP. ANALYSIS CHART		12/18/08
A UPDATED PER NEW FBC REQUIREMENTS		07/07/08

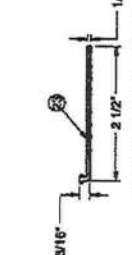
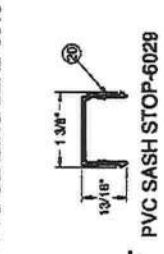
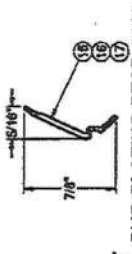
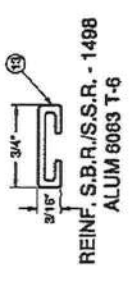
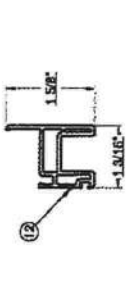
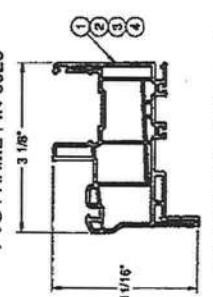
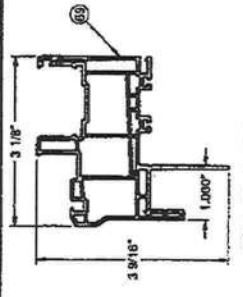
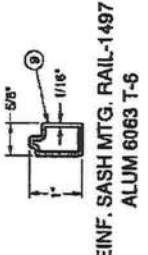
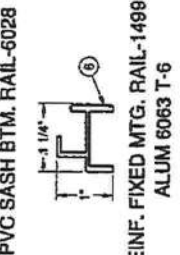
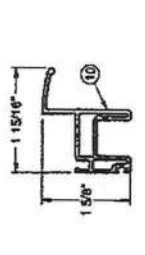
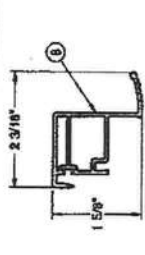
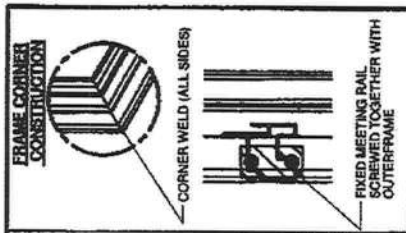
Centum
WINDOW SYSTEMS
1800 SW 44TH AVE.
OCALA, FLORIDA 34474

DESIGN PRESSURE CHART / GLAZING - NON IMPACT
DRAWN BY: 8100
DATE: 12/18/2008
SCALE: NTS
REV. LETTER: B
SHEET: 3 OF 8

PROPRIETARY AND CONFIDENTIAL
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Item #	Part #	Description
1	H-6031	Main Fr. SH, w/fg., PVC Head
2	H-6031	Main Fr. SH, w/fg., PVC Sill
3	H-6031	Main Fr. SH, w/fg., PVC L.Jb.
4	H-6031	Main Fr. SH, w/fg., PVC R. Jb.
5	H-6001	Meat Rail, Fxd.
6	S-1499	Reinf. Fxd. MR.
8	H-6027	Sash Meet Rail, PVC
9	H-1457	Reinf. Sash Meet Rail
10	H-6028	Sash Bottom Rail, PVC
11	S-1498	Reinf. S.B.R./S.S.R.
12	H-6004	Sash Sider/TopBot. Rail
13	S-1498	Reinf. S.B.R./S.S.A
14	S-6016	Glz. Bd. 3/4" Fxd. Horiz.
15	S-6016	Glz. Bd. 3/4" Fxd. Vert.
16	S-6016	Glz. Bd. 3/4" Sash Horiz.
17	S-6016	Glz. Bd. 3/4" Sash Vert.
18	S-6013	Scrm. Adapt. w/whk. PVC, Horiz.
19	S-6014	Scrm. Adapt. PVC, Vert.
20	S-6029	Sash Stop/Filler, PVC, Vert.
23	P-3780	Anchor Clip
24	P-2190	Balancer
25	P-3058	Sash Cam, PVC
26	P-3046	Take Out Clip
27	P-3783	Lock, Impact, PVC
28	P-3784	Keeper, Impact, PVC
30	P-3216	Woop Cover w/Flap
31	P-3893	Reinf. Plate, F.M.R.
32	P-3428	Welp. 0.270 x .187 back FinSeal, Gray
33	P-3428	Welp. 0.270 x .187 back FinSeal, Gray
34	P-3428	Welp. 0.270 x .187 back FinSeal, Gray
35	P-3428	Welp. 0.270 x .187 back FinSeal, Gray
36	P-3428	Welp. 0.270 x .187 back FinSeal, Gray
37	P-3765	#6 x 2.000 Ph PH SMS
38	P-3768	#8 x 0.825 Ph PH SMS
39	P-4051W	#8 x 1.250 Ph PH TEK, Whl.
40	P-3748W	#6 x 0.525 Quad FH TEK, Whl.
44	P-3342	Seam Sealer, SMA5504
45	P-3752	Cap Plug, 1/2" Hole
46	P-3438	Hot Melt, TruSeal, "H"
47	P-3438	Hot Melt, TruSeal, "H"
48	P-3352	Set. Bk., 65 Durs., 1/8" x 5/8" x 2" Lg.
51	P-3881	Temporary Lbl. INFRACOP
52	P-3813	CWS Lbl. (loop)
53		Glass, Fixed
54		Glass, Sash
55	P-3291	Spacer (see sheet 3)
56	P-3281	Spacer (see sheet 3)
62	P-3218	Screen Frame, Horiz.
63	P-3218	Screen Frame, Vert.
64	P-3321	Screen Frame Corner Key
65	P-3881	Screen Cloth, 18 x 16 Fiberglass
66	P-3228	Screen Spine, 154 Dia., Bk.
67	P-3029	Screen Pull Tab
68	P-3033	Screen Spring, SS
69	H-6026	Main Fr. SH, w/fg., PVC
70	P-3113	Screw Support for PVC



NOTES: ITEMS 21, 22, 23, 41, 42, 43, 48, 50, 57-61 ARE NOT USED ON THIS PRODUCT

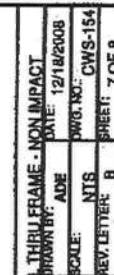
B	ADDED COMP. ANALYSIS CHART	12/18/08
A	UPDATED PER NEW FBC REQUIREMENTS	07/07/08

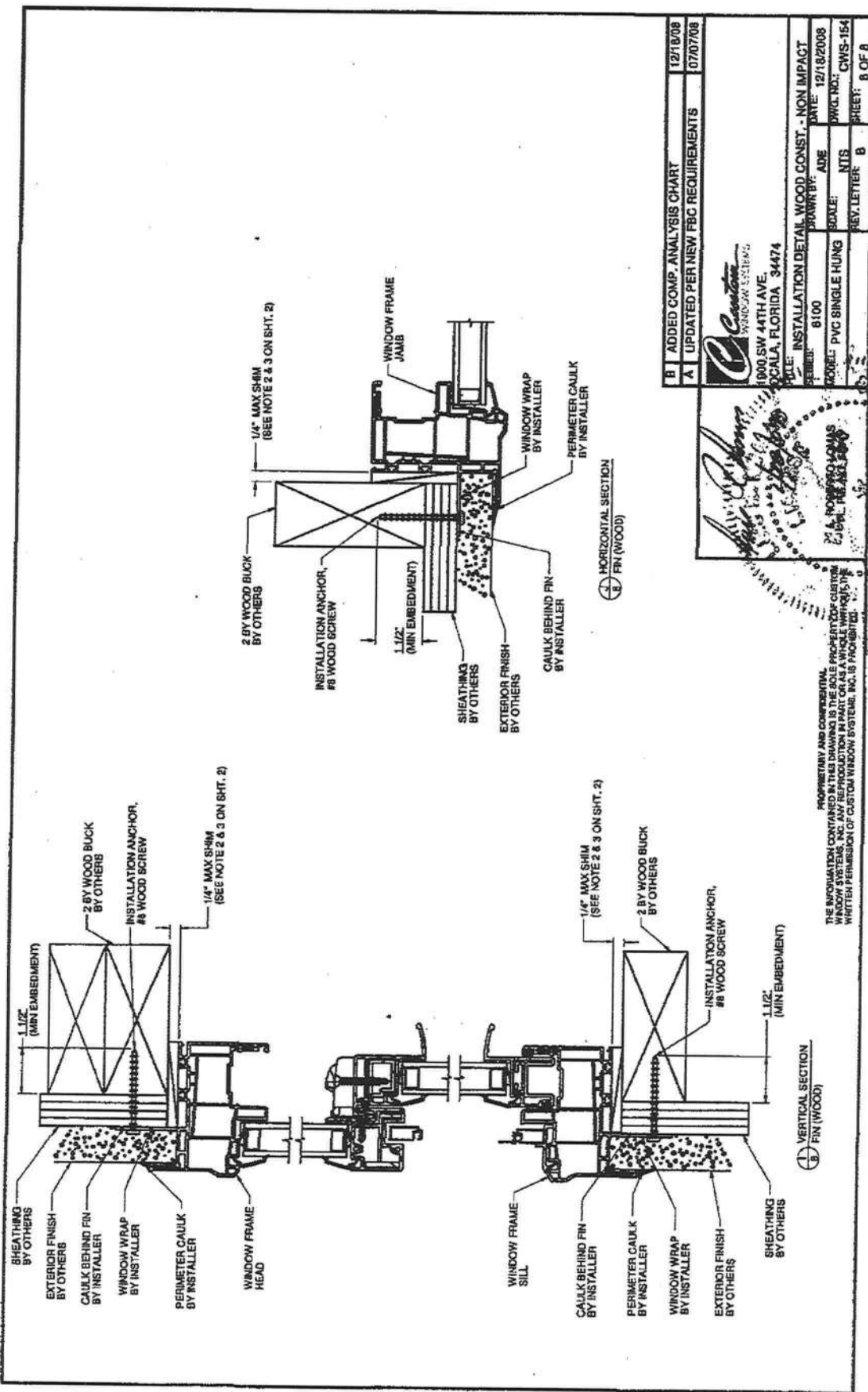
Creston
WINDOW SYSTEMS
1970 SW 44TH AVE.
OCALA, FLORIDA 34474


DATE:	12/18/2008
DRAWN BY:	ADE
CHECKED BY:	NTS
SCALE:	PVC SINGLE HUNG
REV. NO.:	CWS-154
REV. LETTER:	B
SHEET:	5 OF 8

PROPRIETARY AND CONFIDENTIAL
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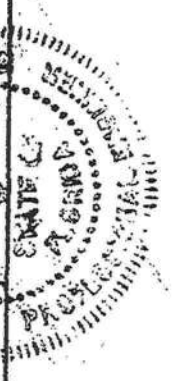






B ADDED COMP. ANALYSIS CHART		12/18/08
A UPDATED PER NEW FBC REQUIREMENTS		07/07/08
 1000 SW 44TH AVE. OCALA, FLORIDA 34474 TITLE: INSTALLATION DETAIL WOOD CONST. - NON IMPACT DRAWN BY: ADE DATE: 12/18/2008 SCALE: PVC SINGLE HUNG SHEET: NTS REV. LETTER: B SHEET: 8 OF 8		

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**GEOTECHNICAL EXPLORATION
RAMONA PARK CHURCH ADDITION
LAKE CITY, COLUMBIA COUNTY, FLORIDA
CTI PROJECT NO. 10-00362-01**

--- Prepared for ---
**Rhodes Brothers
1262 SE Baya Drive
Lake City, Florida 32025**



--- Prepared by ---
**Cal-Tech Testing, Inc.
P. O. Box 1625
Lake City, Florida 32056-1625**

September 21, 2010



Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

P.O. Box 1625 • Lake City, FL 32056

4784 Rosselle Street • Jacksonville, FL 32254

Tel. (386) 755-3633 • Fax (386) 752-5456

Tel. (904) 381-8901 • Fax (904) 381-8902

LABORATORIES

September 21, 2010

Rhodes Brothers

1262 SE Baya Drive

Lake City, Florida 32025

Attention: Mr. Dwight Rhodes

Subject: Report of Geotechnical Exploration
Ramona Park Church Addition
8192 S US Highway 40
Lake City, Columbia County, Florida 32025
CTI project No. 10-00362-01

Dear Mr. Rhodes:

Cal-Tech Testing, Inc. (CTI) has completed the subsurface exploration for the new additions at the existing Ramona Park Church. This report briefly outlines our understanding of the planned construction, describes the field exploration, presents the collected data, and provides our geotechnical engineering evaluation of the subsurface conditions, with respect to the planned construction and estimated structural loading conditions. This exploration was performed in general accordance with our proposal dated September 21, 2010. Authorization for this work and acceptance of the proposal was provided by you on September 16, 2010.

Introduction

The purpose of this exploration was to develop information concerning the site and subsurface conditions in order to evaluate site preparation requirements and foundation support recommendations for the proposed addition at the existing Ramona Park Church. The subject site is located at 8192 South US Highway 41 in Lake City, Columbia County, Florida. Mrs. Josie Gaskin of Rhodes Brothers provided us with a site survey plan prepared by Britt Surveying & Associates, Inc. of Lake City, Florida dated August 24, 2010; and a set of architectural plans prepared by Nicholas Paul Geisler Architect of Lake City, Florida dated August 30, 2010. Based on our review of the furnished documents and our conversation with Mrs. Gaskin, we understand the proposed addition will consist of constructing a 1,368 SF one-story block building addition with an attached 900 SF aluminum structural carport. These additions will be constructed to connect the two existing one-story block buildings.

Detailed structural loading information has not been provided to us; however, we assume that bearing wall and individual column loads will not exceed 4 kips per lineal foot and 50 kips, respectively. We anticipate that soil-supported floor live loads will not exceed 150 psf. We also anticipate the finished floor elevation for the new additions will be at or near the existing ground surface (to match the existing building) with new earthwork fill or cut not to exceed 24 inches to achieve desired finished subgrade elevations.

Field Program

Our field program consisted of performing two (2) Standard Penetration Test (SPT) borings within the proposed additions. The SPT borings were extended 15 feet below the existing ground surface and were performed at the approximate locations shown on the attached Field Exploration Plan. These locations were determined in the field and measured by tape and approximating right angles from existing features (existing building corners). Therefore, the borings location should be considered only as accurate as the means and methods by which they were obtained.

Sampling and penetration procedures of the SPT borings were accomplished in general accordance with ASTM D-1586, "*Penetration Test and Split-Barrel Sampling of Soils*", using a power rotary drill rig (BK-51 drill rig equipped with a manual hammer). The standard penetration tests were performed by driving a standard 1-3/8" I.D. and 2" O.D. split spoon sampler with a 140 pound hammer falling 30 inches. The number of hammer blows required to drive the sampler a total of 18 inches, in 6 inch increments, were recorded. The penetration resistance or "N" value is the summation of the last two 6 inch increments and is illustrated on the attached boring logs adjacent to their corresponding sample depths. The penetration resistance is used as an index to derive soil parameters from various empirical correlations.

The attached record of boring logs presents the descriptions of the subsurface conditions encountered at the time of our field program, and also provide the penetration resistances recorded during the drilling and sampling process. The stratification lines and depth designations on the boring record represent the approximate boundaries between the various soils encountered, as determined in the field by our personnel. In some cases, the transition between the various soils may be gradual.

Subsurface Conditions

The soil profile as disclosed by test borings B-1 and B-2 initially consisted of about 4 to 6 inches of gray fine sand with silt and organics (topsoil). This surface cover is underlain by alternating layers of gray to tan fine sand with silt (SP-SM), yellowish tan to light gray clayey fine sand (SC), or gray with reddish brown mottles sandy clay (CL).

The standard resistance or "N" values of the sandy soils ranged from 6 to exceeding 60 Blows Per Foot (BPF) indicating the granular soils to vary from loose to very dense in relative density. The clayey soils have a hard consistency with "N" value of 31 BPF.

Groundwater Conditions

The depth to the groundwater was measured immediately following completion of the drilling operations, the groundwater table was not encountered in any of the test borings. It must be noted that due to the relatively short time frame of the field exploration and clayey nature of some of the site soils, the groundwater may not have had sufficient time to stabilize. For a true "stabilized" groundwater level reading, piezometers may be required. In any event, fluctuation in groundwater levels should be anticipated due to seasonal climatic conditions, construction activities, rainfall variations, surface water runoff, and other site-specific factors.

General Area Geology/Sinkholes Potential

Published information regarding the geology in this area of Columbia County indicates the site is underlain by the Undifferentiated Quaternary Sediments (**Qu**) of the Pleistocene and Holocene epochs. Typically, the undifferentiated Quaternary sediments consist of Siliciclastics, organics and freshwater carbonates. The Siliciclastics are light gray, tan, brown to dark, unconsolidated to poorly consolidated, clean to clayey, silty, fossiliferous, variably organic-bearing sands to blue green to olive green, poorly to moderately consolidated, sandy, silty, clays. Freshwater carbonates "*marls*" are buff colored to tan, unconsolidated to poorly consolidated, fossiliferous (mollusks) carbonate muds containing organics.

Sinkhole Potential

Sinkholes are primarily caused by an advanced state of internal soil erosion or raveling action, which under certain circumstances can lead to ground subsidences. This internal soil erosion is a very slow process by which soil particles usually migrate under the influence of a hydraulic gradient to underlying karsted and/or fractured limestone formation. There are several indicators generally associated with an advanced state of long term internal soil erosion such as noticeable surface depressions and very loose to soft soil zones just above the limestone rock formation. The USGS Map Series No. 110, Sinkhole Type, Development, and Distribution in Florida dated 1985 identifies the site within Area III. This area consists of ground with 30 to 200 feet of cohesive clayey sediments of low permeability cover to limestone. Cover-collapse sinkholes of varying size abruptly develop and are numerous.

Based on our evaluation of the test borings and the subsoil conditions encountered, it is our opinion the explored area has no greater risk of damage due to sinkhole activity than development of structures in other areas within the immediate vicinity of the subject site. Due to the nature of sinkholes and the limitations of the current detection methods, it is incorrect to assume that this exploration has detected all existing or future sinkholes. It is possible that existing sinkholes or subsurface conditions which could be associated with future sinkholes were not detected or predicted by this exploration. Also, it must be understood that this exploration was not intended to predict or preclude future sinkholes from occurring within the limits of the subject site.

Foundation Recommendations

The test borings indicated the presence of loose soils within the upper strata. These soils are considered suitable for reuse as structural fill, however, they are not considered acceptable for the support of the proposed foundation system in their current conditions. To improve the density of the supporting soils, the upper 2 feet of the existing soils will need overexcavation and recompaction as indicated herein (including 5 feet outside the building perimeter, where possible).

Provided the foundation and site soils are prepared in accordance with the guidelines presented in this report, it is our opinion the proposed addition may be supported on a conventional shallow foundation system. The shallow foundation may be designed for an allowable bearing pressure of 2,500 pounds per square foot (psf) or less on recompacted soils or newly placed structural fill.

Settlement Analyses

Actual magnitude of settlement that will occur beneath foundations will depend upon variations within the subsurface soil profile, actual structural loading conditions, embedment depth of the footings, actual thickness of compacted fill or cut, and the quality of the earthwork operations. Assuming the foundation related site work and foundation design is completed in accordance with the enclosed recommendations, we estimate the total settlement of the structure will be on the order of 1 inch or less. Differential settlements (between adjacent columns or along the length of a continuous wall footing) should be approximately one-half of the total settlement. This settlement is primarily the result of elastic compression of the upper looser sands, and should occur almost immediately following the application of the structural dead load during construction.

Uplift Resistance

Under wind loading conditions, portions of the proposed structure and carport will likely be subjected to uplift forces. In order to resist these uplift forces, it may be necessary to increase the footing size (thus increasing the dead weight) or lower the footing to mobilize additional soil weight above the footing. Uplift resistance from the soil may be evaluated as the weight of the soil directly above the footing, plus the shearing resistance along the vertical face of the soil prism. Alternately, the available soil uplift resistance may be calculated as the weight of the soil prism defined by the diagonal line drawn from the top of the footing to the ground surface at an angle of 30 degrees with the vertical. A moist unit weight of 100 pcf (compacted to a minimum of 95% of the modified Proctor's maximum dry density) may be used for well-compacted, suitable fill. Should the bottom of any structure bear at depths below the groundwater level, these structures must be properly designed to resist the resulting uplift forces due to hydrostatic pressures.

Lateral Resistance

Lateral loads created by wind loads may be resisted by the passive pressure of the soil acting against the side of the individual footings and/or the friction developed between the base of the foundation system and the underlying soils. For compacted backfill and/or in-situ material, the passive pressure may be taken as an equivalent to the pressure exerted by a fluid weighing 300 pcf for above the ground-water table and 113 pcf below water level. A coefficient of friction equal to 0.35 may be used for calculating the frictional resistance at the base of the shallow footings. The resistance values discussed herein are based on the assumption that the foundations can withstand horizontal movements on the order of 1/4 inch. Lateral resistance determined in accordance with the recommendations provided should be considered the total available resistance. Consequently, the design should include a minimum factor of safety of 1.5.

Site Clearing/Grading

Initial site preparation should consist of relocating existing utilities that fall within the new construction area. This should be followed by the removal of all topsoil, organics and any other deleterious materials that falls within the construction area. The perimeter areas may need grading to help direct surface water runoff away from the planned construction area. Also, the site preparations should include the removal of the existing septic tank and associated drain lines that fall within the new building addition and backfilled accordingly.

Foundation Size and Bearing Depth

In using net pressures, the weight of the footing and backfill over the footing need not be considered. Hence, only loads applied at or above final grade need to be used for dimensioning footings. However, wall bearing footings should be designed with a minimum width of 18 inches, while the individual column footings should have minimum dimensions of 2 feet by 2 feet. All exterior footings should bear at a depth of at least 18 inches below the exterior final grades. Interior footings should bear at a depth of at least 18 inches below the interior floor slab. These recommended minimum-bearing depths should provide the necessary confinement for the foundation bearing level soils.

To alleviate differential settlement of the existing structure due to new imposed loads of the new building addition, the footing bearing elevation of the new building should be made to bear at the elevation of the existing footing system.

Bearing Material

The foundations should bear in either natural soils, or in compacted structural fill/backfill. Sandy soils should be compacted to densities equivalent to 95 percent of the modified Proctor maximum dry density (ASTM D 1557). Compaction should not be attempted on clayey soils at the footing bearing level (if any encountered). Rather they should be excavated using a smooth bucket/shovel, and replaced with a working platform of 10 to 12-inches of coarse concrete aggregate or two to three inches of a lean concrete mud mat.

Ground Floor Slab Support

The ground floor slab for the proposed additions may be constructed directly on a re-compacted fine sand subgrade. Structural fill soils placed directly beneath the slab should be compacted to a minimum of 95 percent of the modified Proctor maximum dry density (ASTM D-1557) to a depth of at least 12 inches.

Site & Fill Compaction

We recommend that exposed and underlying soils be compacted to densities equivalent to 95 percent of the modified Proctor maximum dry density (ASTM D-1557). To compact the exposed and underlying soils, we recommend using a roller that has a static at-drum weight on the order of four to five tons and a drum diameter on the order of four feet (the roller should operate with the vibratory action disengaged to avoid damage to the existing foundation system). The initial compaction operations should also consist of at least eight overlapping passes of the roller in each direction. This compaction effort should help improve the overall uniformity and bearing conditions of the near-surface soils.

Caution should be exercised during the compaction of soils adjacent to the existing building. Compaction of fill placed adjacent to the existing footing system may be accomplished using a plate tamper. Generally, the SPT borings indicated the presence of loose sandy soils within the upper 5 feet of the existing ground surface. The majority of these soils (with the exception of the topsoil) are considered suitable for reuse as structural fill, however, they are not considered acceptable for the support of the proposed building and carport additions in their current conditions. To improve the density of these supporting soils, the upper 2 feet of the site soils within the new building and carport additions (including 5 feet outside the perimeter of the building where not conflicting with the

existing building) should be overexcavated and recompactd as indicated herein. All excavations should be properly backfilled the same day they are opened.

When removing the existing septic tank and associated drain lines, a 4 to 5 foot open hole may be created. Backfill placed in this hole to reestablish the subgrade should be placed in thin loose lifts not exceeding 12 inches in thickness and compacted accordingly.

Using a roller meeting the above requirements, structural fill required to raise the site to the planned finish grades may then be placed in loose lifts not exceeding 12 inches in thickness, and should then be compacted to densities similar to those recommended above. For ease of construction and compaction, we recommend that structural fill consist of a non-plastic, inorganic, granular soil containing less than 12 percent material passing the 200 mesh sieve (i.e., relatively clean sand). The majority of the upper fine sands encountered during this exploration should meet this criteria.

Construction Monitoring and Testing Guidelines

Prior to initiating compaction operations, we recommend that representative samples of the on-site and any off-site materials be used as structural fill be tested to determine their compaction and classification characteristics. These tests are needed for compaction quality control of the structural fill/backfill and existing soils and to determine if the fill/backfill material is acceptable. A representative number of in-place field density tests should be performed in the compacted soils and in each lift of structural fill or backfill to verify the required degree of compaction has been achieved. In-place density tests should also be performed at representative locations in the bearing level soils in the footing excavation bottoms. The following minimum density testing frequencies are recommended:

Area	Recommended Minimum Density Test Frequency
Concrete slab-on-grade	1 test per 1,000 ft ² in compacted existing soils and in each lift of structural fill
Footing Bearing Level Soils	
-Spread Footings	1 test per 100 ft ² of bearing surface
-Continuous/Strip Footings	1 test per 100 lineal feet of bearing surface
Pavement Areas	1 test per 2,000 ft ²


Report Limitations


This report has been prepared for the exclusive use of **Rhodes Brothers of Lake City, Florida** for the specific application to the project discussed herein. Our conclusions and recommendations have been rendered using generally accepted standards of geotechnical engineering practice in the State of Florida, no other warranty is expressed or implied. CTI is not responsible for the interpretations, conclusions, opinions, or recommendations of others based on the data contained herein. Field observations, monitoring, and quality assurance testing during earthwork and foundation installation are an extension of the geotechnical design. We recommend that the owner retain these services and that CTI be allowed to continue our involvement in the project through these phases of construction. During construction, we accept no responsibility for job site safety; which is the sole responsibility of the contractor.

Closing

We appreciate the opportunity to work with you on this project, and look forward to serving as your geotechnical and construction materials testing consultant for the remainder of this and future projects. Should you have any questions and/or comments concerning this report, please contact our office at 386-755-3633.

Very truly yours,
Cal-Tech Testing, Inc.


David B. Brown
Executive Vice President


Nabil O. Hmeidi, P.E. 9/21/2010
Senior Geotechnical Engineer
Licensed, Florida No. 57842

Distribution: File (1 copy)
Addressee (3 copies)

Attachments: Vicinity Map (1 page)
Field Exploration Plan (1 page)
Record of SPT Borings (2 pages)
Unified Soil Classification System (1 page)
Key To Test Data (1 page)

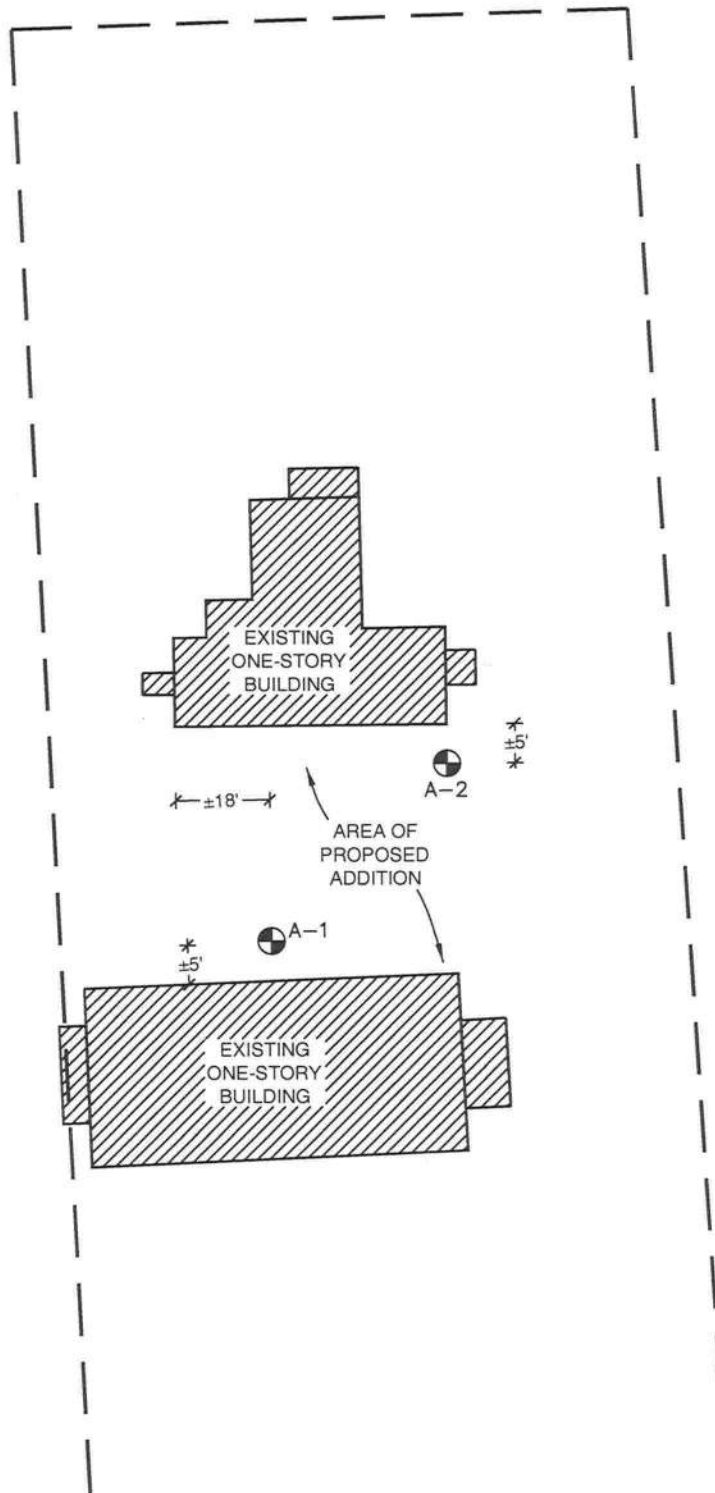
ATTACHMENTS



CAL-TECH TESTING, INC.
P.O. Box 1625
Lake City, Florida 32056-1625
Phone: (386) 755-3633
Fax: (386) 752-5456

VICINITY MAP
Ramona Park Church Addition
8192 S. US Highway 41
Lake City, Columbia County, Florida
Cal-Tech Testing Project No. 10-00362-01

Figure 1



FOR ILLUSTRATION ONLY
NOT TO SCALE
NOT FOR CONSTRUCTION

STANDARD PENETRATION TEST BORINGS PERFORMED BY CTI ON SEPTEMBER 17, 2010

SUBSURFACE EXPLORATION
RAMONA PARK CHURCH ADDITION
8192 S. US HIGHWAY 41
LAKE CITY, COLUMBIA COUNTY, FLORIDA

CAL-TECH TESTING, INC.
P.O. Box 1625
Lake City, Florida 32056-1625
Phone: (386) 755-3633
Fax: (386) 752-5456

FIELD EXPLORATION PLAN

Project No. 10-00362-01		DATE: 09/21/2010	FIGURE: 2
DRAWN:	APPROVED:	SCALE: As Shown	SHEET: 1/1



Cal-Tech Testing, Inc.
P.O. Box 1625
Lake City, Florida 32024
Telephone: 386-755-3633
Fax: 386-752-5456

BORING NUMBER B-1

PAGE 1 OF 1

CLIENT Rhodes Brothers PROJECT NAME Ramona Park Church Addition
PROJECT NUMBER 10-00362-01 PROJECT LOCATION 8192 S. US Hwy 41, Lake City, Columbia County, FL
DATE STARTED 09/17/10 COMPLETED 09/17/10 GROUND ELEVATION _____ HOLE SIZE 4"
DRILLING CONTRACTOR Cal-Tech Testing, Inc. GROUND WATER LEVELS:
DRILLING METHOD Continuous Flight Auger AT TIME OF DRILLING ---
LOGGED BY N.H. CHECKED BY _____ AT END OF DRILLING --- Not Encountered
NOTES BK-51 (manual hammer) AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	▲ SPT N VALUE ▲			
								20	40	60	80
0		Gray, fine sand with silt and organics (TOPSOIL)						PL	MC	LL	
		LOOSE, gray to tan, fine sand with silt (SP-SM)						20	40	60	80
								□ FINES CONTENT (%) □			
								20	40	60	80
5		MEDIUM DENSE, yellowish tan, clayey fine sand (SC)	SPT 1	100	4-4-5 (9)						
			SPT 2	100	4-4-3 (7)						
			SPT 3	100	3-4-4 (8)						
		HARD, gray with reddish brown mottles, sandy clay (CL)	SPT 4	100	6-6-7 (13)						
			SPT 5	100	11-13-18 (31)						
10		VERY DENSE, light gray, clayey fine sand (SC)	SPT 18	100	19-23-27 (50)						
			SPT 18	100	29-35-37 (72)						
15											

Bottom of borehole at 15.0 feet.

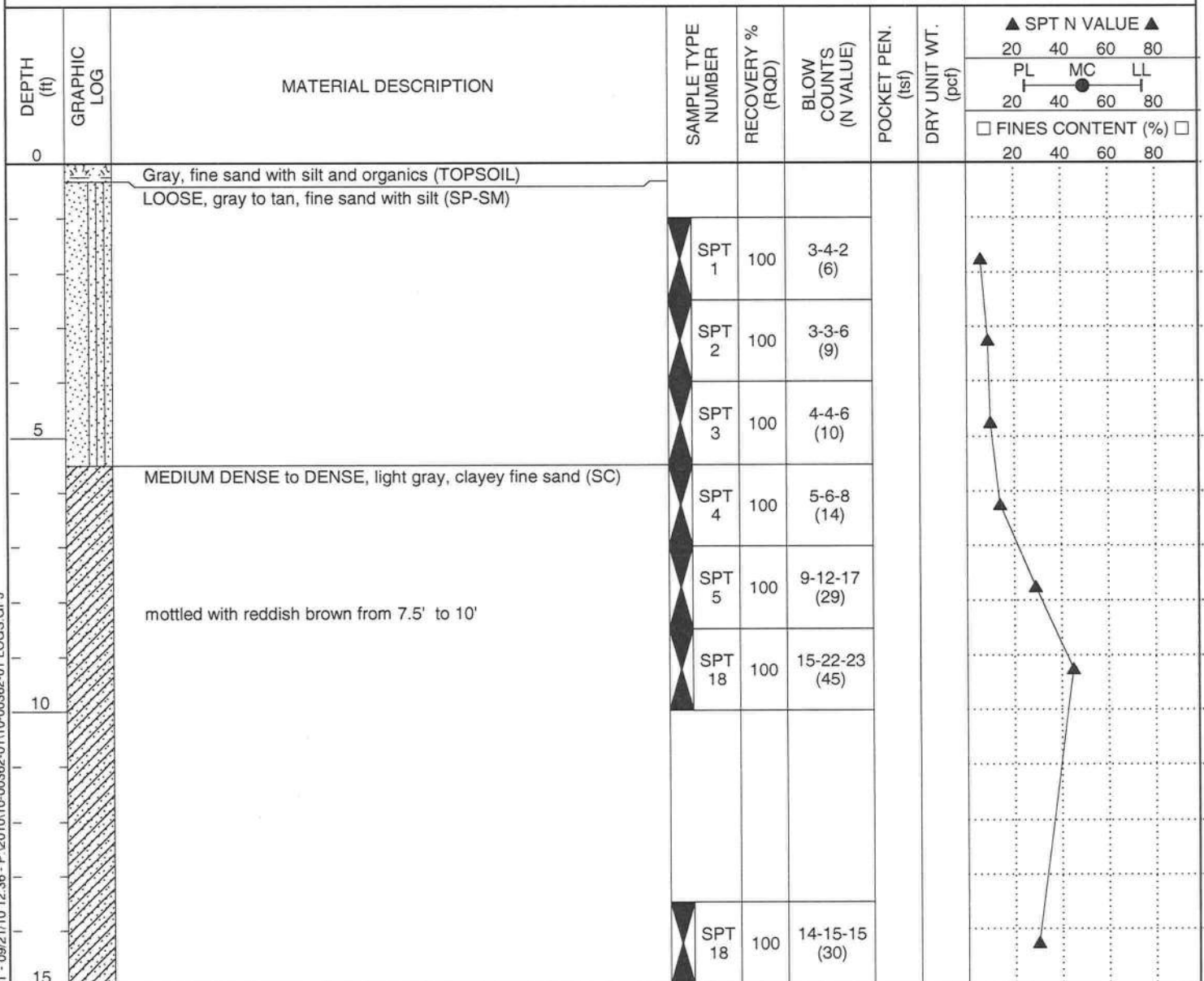


Cal-Tech Testing, Inc.
P.O. Box 1625
Lake City, Florida 32024
Telephone: 386-755-3633
Fax: 386-752-5456

BORING NUMBER B-2

PAGE 1 OF 1

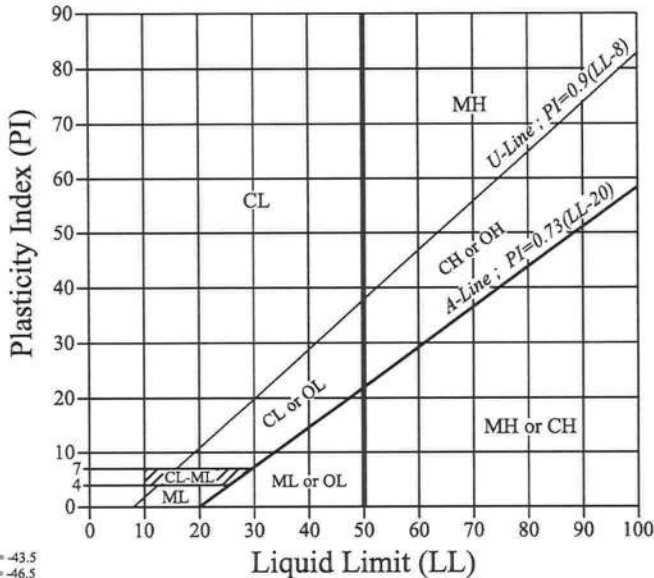
CLIENT Rhodes Brothers PROJECT NAME Ramona Park Church Addition
PROJECT NUMBER 10-00362-01 PROJECT LOCATION 8192 S. US Hwy 41, Lake City, Columbia County, FL
DATE STARTED 09/17/10 COMPLETED 09/17/10 GROUND ELEVATION _____ HOLE SIZE 4"
DRILLING CONTRACTOR Cal-Tech Testing, Inc. GROUND WATER LEVELS:
DRILLING METHOD Continuous Flight Auger AT TIME OF DRILLING ---
LOGGED BY N.H. CHECKED BY _____ AT END OF DRILLING --- Not Encountered
NOTES BK-51 (manual hammer) AFTER DRILLING ---



Bottom of borehole at 15.0 feet.

UNIFIED SOIL CLASSIFICATION SYSTEM

ASTM DESIGNATION D-2487

MAJOR DIVISIONS			GROUP SYMBOL	TYPICAL NAMES	LABORATORY CLASSIFICATION CRITERIA						
COARSE GRAINED SOILS (More than half of the material is larger than No. 200 sieve)	Gravels (more than half of the coarse fraction is larger than No. 4 sieve)	Clean gravels	GW	Well-graded gravels, gravel-sand mixtures, little or no fines.	Determine percentage of sand and gravel from grain size curve Depending on percentage of fines (fraction smaller than No. 200 Sieve size), coarse grained soils are classified as follows: Less than 5% GW, GP, SW, SP More than 12% ... GM, GC, SM, SC 5 to 12% Borderline cases requiring dual symbols	$C_u = \frac{D_{60}}{D_{10}} > 4 \quad ; \quad 1 < C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} < 3$					
			GP	Poorly graded gravels, gravel-sand mixture, little or no fines.		Not meeting all gradation requirements of GW					
		Gravel with fines	GM	Silty gravels, gravel-sand-silt mixtures.		Atterberg Limits below A-Line or PI less than 4	Above A-Line with PI between 4 and 7 are borderline cases requiring the use of dual symbols.				
			GC	Clayey gravels, gravel-sand-clay mixtures.		Atterberg Limits above A-Line or PI greater than 7					
	Sands (more than half of the coarse fraction is smaller than No. 4 sieve)	Clean sands	SW	Well-graded sands, gravelly sands, little or no fines.		$C_u = \frac{D_{60}}{D_{10}} > 6 \quad ; \quad 1 < C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} < 3$					
			SP	Poorly graded sands, gravelly sands, little or no fines.		Not meeting all gradation requirements of SW					
		Sands with fine	SM	Silty sands, sand-silt mixtures.		Atterberg Limits below A-Line or PI less than 4	Limits plotting in hatched zone with PI between 4 and 7 are borderline cases requiring the use of dual symbols.				
			SC	Clayey sands, sand-clay mixtures.		Atterberg Limits above A-Line or PI greater than 7					
FINE GRAINED SOILS (More than half of the material is finer than No. 200 sieve)	Silts and Clays (LL less than 50)	ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity.	PLASTICITY CHART 1. Plot intersection of PI as determined by the Atterberg Limits tests. 2. Points plotted above the A-Line indicate clay soils. 3. Points plotted below the A-Line indicate silt. 							
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clay.								
		OL	Organic silts and organic silty clays of low plasticity.								
	Silts and Clays (LL greater than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.								
		CH	Inorganic clays of high plasticity, fat clay.								
		OH	Organic clays of medium to high plasticity, organic silts.								
	Highly Organic Soils	Pt	Peat and other highly organic soils.								
	CAL-TECH TESTING, INC. P.O. Box 1625 Lake City, Florida 32056-1625 Phone: 386-755-3633 Fax: 386-752-5456						5% Max. Passing the U.S. No. 200 Sieve SP 5% - 12% Passing the U.S. No. 200 Sieve SP-SM 12% - 50% Passing the U.S. No. 200 Sieve SM/SC				

CAL-TECH TESTING, INC.

P.O. Box 1625

Lake City, Florida 32056-1625

Phone: 386-755-3633 Fax: 386-752-5456

5% Max. Passing the U.S. No. 200 Sieve SP

5% - 12% Passing the U.S. No. 200 Sieve SP-SM

12% - 50% Passing the U.S. No. 200 Sieve SM/SC

KEY TO TEST DATA

STANDARD PENETRATION TEST:

Soil sampling and penetration testing is performed in accordance with ASTM D-1586. The standard penetration resistance ("N") is the number of blows of a 140-pound hammer falling 30 inches to drive a 2-inch O.D., 1.4-inch I.D. split spoon sampler one foot.

ROCK CORE DRILLING:

Rock sampling and core drilling is performed in accordance with ASTM D-2113. The rock quality designation percentage (RQD) is determined by summing only pieces of core that are at least 4 inches long, and dividing by the "run" length.

Relation of RQD and In-situ Rock Quality	
RQD (%)	Rock Quality
90 - 100	Excellent
75 - 90	Good
50 - 75	Fair
25 - 50	Poor
0 - 25	Very Poor

RELATIVE DENSITY (SANDS):

Very loose - less than 4 blows/ft.

Loose - 5 to 10 blows/ft.

Medium - 11 to 30 blows/ft.

Dense - 31 to 50 blows/ft.

Very dense - over 50 blows/ft.

CONSISTENCY (SILTS & CLAYS):

Very soft - less than 2 blows/ft.

Soft - 3 to 4 blows/ft.

Medium stiff - 5 to 8 blows/ft.

Stiff - 9 to 15 blows/ft.

Very stiff - 16 to 30 blows/ft.

Hard - 31 to 50 blows/ft.

Very hard - over 50 blows/ft.

HARDNESS (ROCKS):

Soft - Rock core crumbles when handled.

Medium - Can break core with hands.

Moderately hard - Thin edges of rock core can be broken with fingers.

Hard - Thin edges of core can not be broken with fingers.

Very hard - Can not be scratched with knife.

GROUNDWATER:

Water levels shown on boring logs are taken immediately upon completion of boring, and are intended for general information. The apparent level may have been altered by the drilling process. Groundwater levels, if desired, can be monitored over a long time interval.

CAL-TECH TESTING, INC.

P.O. Box 1625

Lake City, Florida 32056-1625

Phone: 386-755-3633 Fax: 386-752-5456

5% Max. Passing the U.S. No. 200 Sieve SP

5% - 12% Passing the U.S. No. 200 Sieve SP-SM

12% - 50% Passing the U.S. No. 200 Sieve SM/SC



14 DECEMBER 2010

BUILDING OFFICIAL
COLUMBIA COUNTY BUILDING DEPARTMENT
COLUMBIA COUNTY COURTHOUSE ANNEX
LAKE CITY, FLORIDA

RE: RAMONA PARK CHURCH ADDITION
PERMIT Nr.: 28972

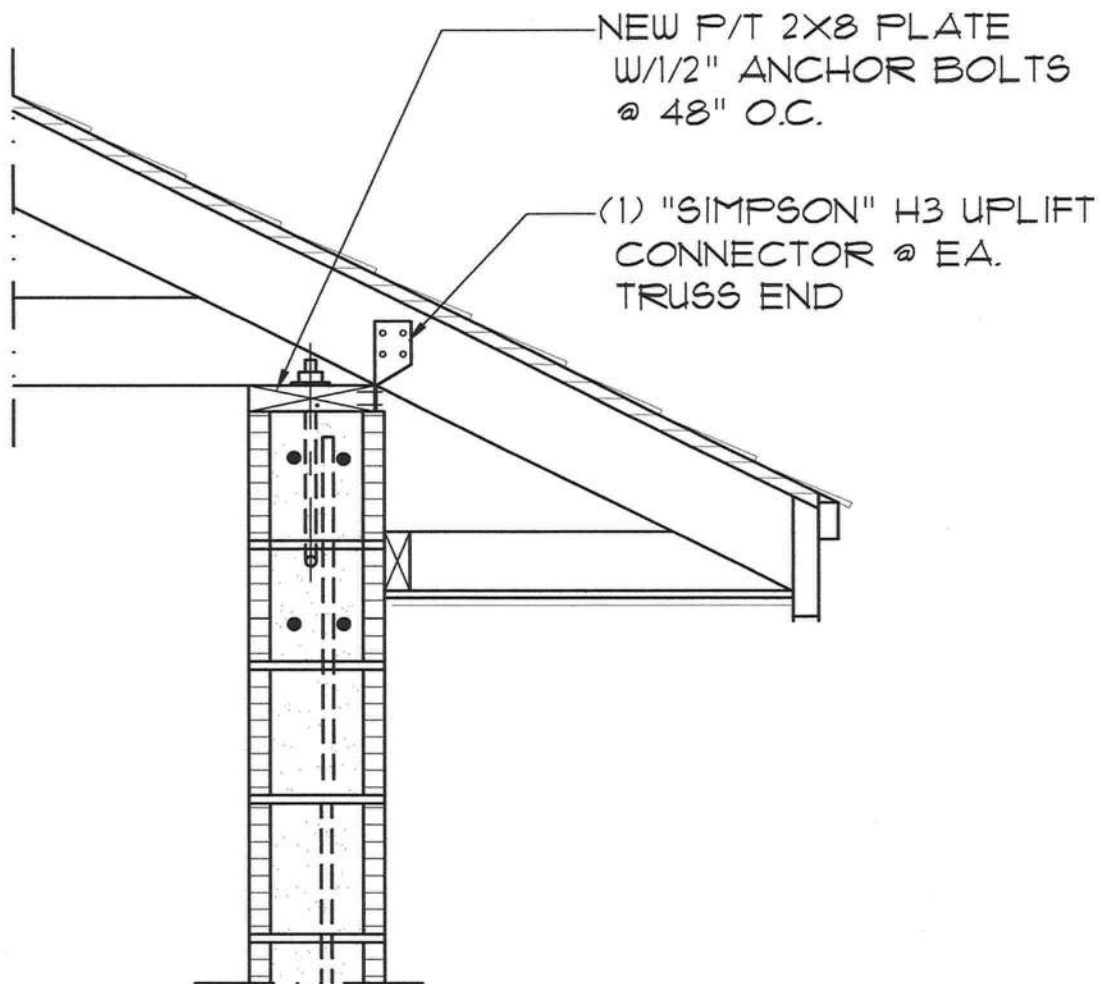
DEAR SIR:

PLEASE BE ADVISED THAT THE "SIMPSON" METAL W/SS GALVANIZED
SADDLES @ EA. TRUSS TO WALL CONNECTION HAS BEEN REVISED
TO INCLUDE A NEW P/T 2X8 TOP PLATE W/1/2" ANCHOR BOLTS AT
48" O.C. AND (1) "SIMPSON" H3 UPLIFT CONNECTOR AT EA. TRUSS END.
SEE ATTACHMENT "A" FOR DETAILS.

SHOULD YOU HAVE ANY QUESTIONS, PLEASE DO NOT HESITATE TO
CALL FOR ANY ASSISTANCE.

YOURS TRULY,
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005

A handwritten signature in black ink, appearing to be 'NPG', written over the typed name.



New C.M.U. Wall Top Plate and Truss Attachment Detail

SCALE : NONE

AR
AR 7005 15 Dec 2010

ATTACHMENT 'A'

COLUMBIA COUNTY FLORIDA DEPARTMENT OF BUILDING AND ZONING INSPECTION

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 03-5S-17-09103-000

Building permit No. 000028972

Use Classification CHURCH ADDITION

Fire: 0.00

Permit Holder DWIGHT RHODES

Waste:

Owner of Building RAMONA PARK CHURCH

Total: 0.00

Location: 8912 S US HWY 441, LAKE CITY, FL 32025

Date: 05/09/2011

Nancy Bricker

Building Inspector



POST IN A CONSPICUOUS PLACE
(Business Places Only)

Notice of Treatment

ADD TO 7509

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address: 5365E BAYVIEW AVE

City: LAKE CITY Phone: 402 1703

Site Location: Subdivision Ramona Park Church

Lot # Block# Permit # 28972

Address 8912 S US Hwy 491

Product used	Active Ingredient	% Concentration
<input checked="" type="checkbox"/> Premise	Imidacloprid	0.1%
<input type="checkbox"/> Termidor	Fipronil	0.12%
<input type="checkbox"/> Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☒ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
ADDITION to Church	1368	110	100

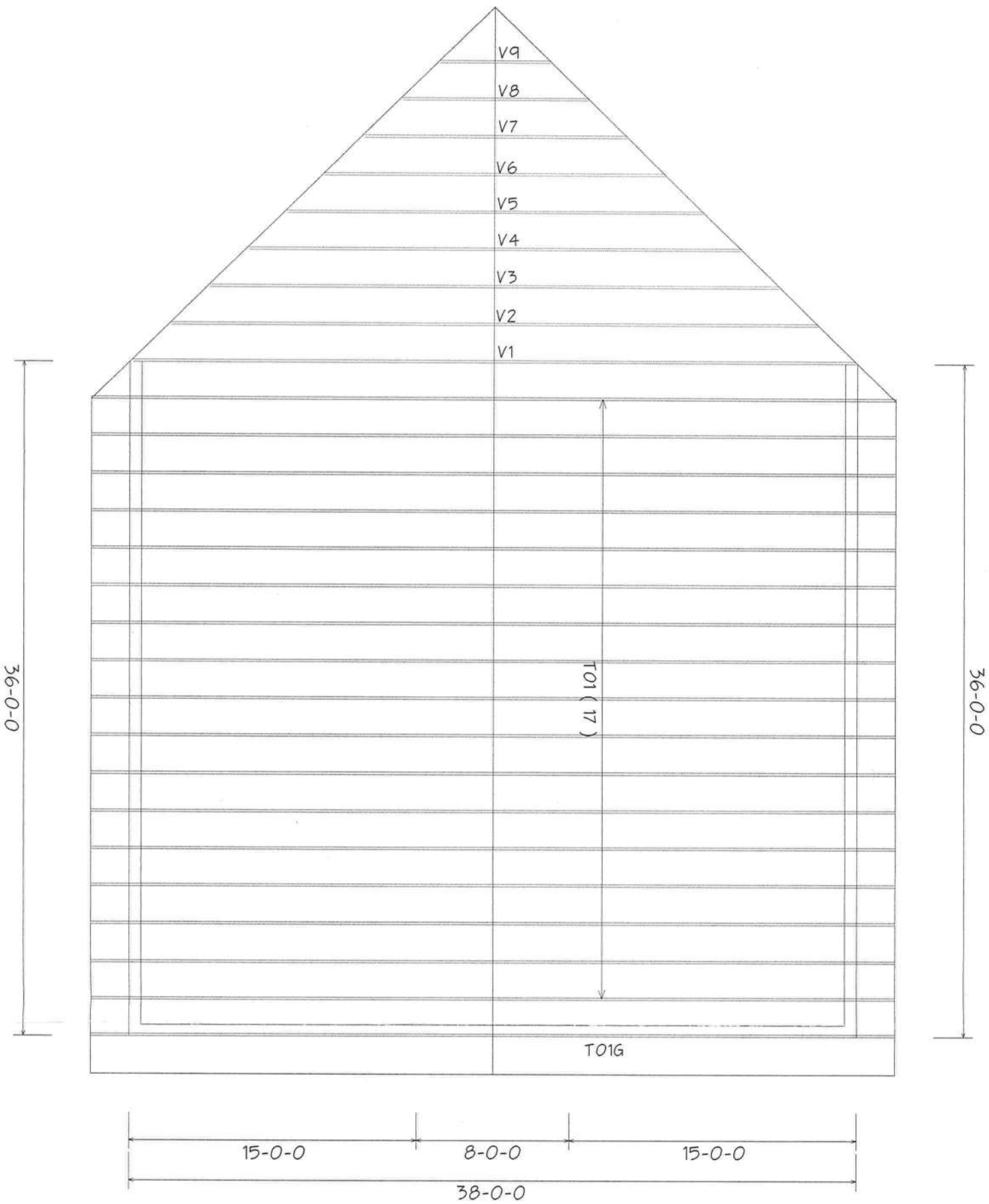
As per Florida Building Code 104.2.6 - If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____

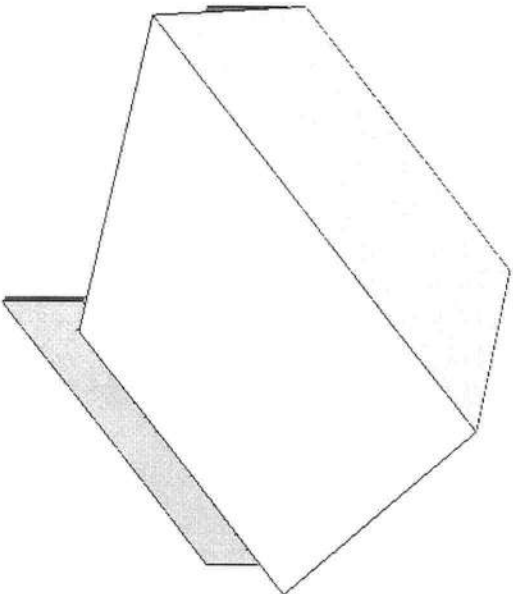
Date 11/30/10 Time 0800 Print Technician's Name James Barker

Remarks: _____

6/12 PITCH - 24" O/H



ALL FLAT CEILINGS



BEARING HEIGHT SCHEDULE

10' 8"

NOTES:

- 1) REFER TO HB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V05 FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 2 O.K. MAXIMUM SPACING UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING UNLESS OTHERWISE NOTED.
- 6) 5/4x2 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL ROOF TRUSS HANGERS TO BE SAMPSON H1026 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SAMPSON TH4422 UNLESS OTHERWISE NOTED.
- 8) BEAM/DECK/JUNCTION (B/D/J) TO BE FINISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND JOISTS. ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS, REVISION AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO REPAIR AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Return Date: _____

Approved By: _____ Date: _____



Bunnell
PHONE: 904-437-3344 FAX: 904-437-3494

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 386-755-6894 FAX: 386-755-7973

Sanford
PHONE: 407-322-0094 FAX: 407-322-9993

OWNER

RHODES BROTHERS
RAMONA PARK CHURCH

DATE: 9-14-10 **SCALE:** NTS **PROJECT:** 345573

Julius Lee

RE: 345573 - RHODES BROTHERS - RAMONA PARK CHURCH

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: RHODES BROTHERS Project Name: 345573 Model: RAMONA PARK CH. ADD.
Lot/Block: Subdivision:
Address: 8192 S HWY 41
City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: DWIGHT RHODES License #: CBC057549
Address: 1262 SE BAYA DR.
City: LAKE CITY, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 40.0 psf

This package includes 11 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date
1	I4487876	T01	9/24/010
2	I4487877	T01G	9/24/010
3	I4487878	V1	9/24/010
4	I4487879	V2	9/24/010
5	I4487880	V3	9/24/010
6	I4487881	V4	9/24/010
7	I4487882	V5	9/24/010
8	I4487883	V6	9/24/010
9	I4487884	V7	9/24/010
10	I4487885	V8	9/24/010
11	I4487886	V9	9/24/010



The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



September 24, 2010

Job 345573	Truss T01	Truss Type COMMON	Qty 17	Ply 1	RHODES BROTHERS - RAMONA PARK CHURCH	14487876
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Sep 24 08:27:34 2010 Page 1

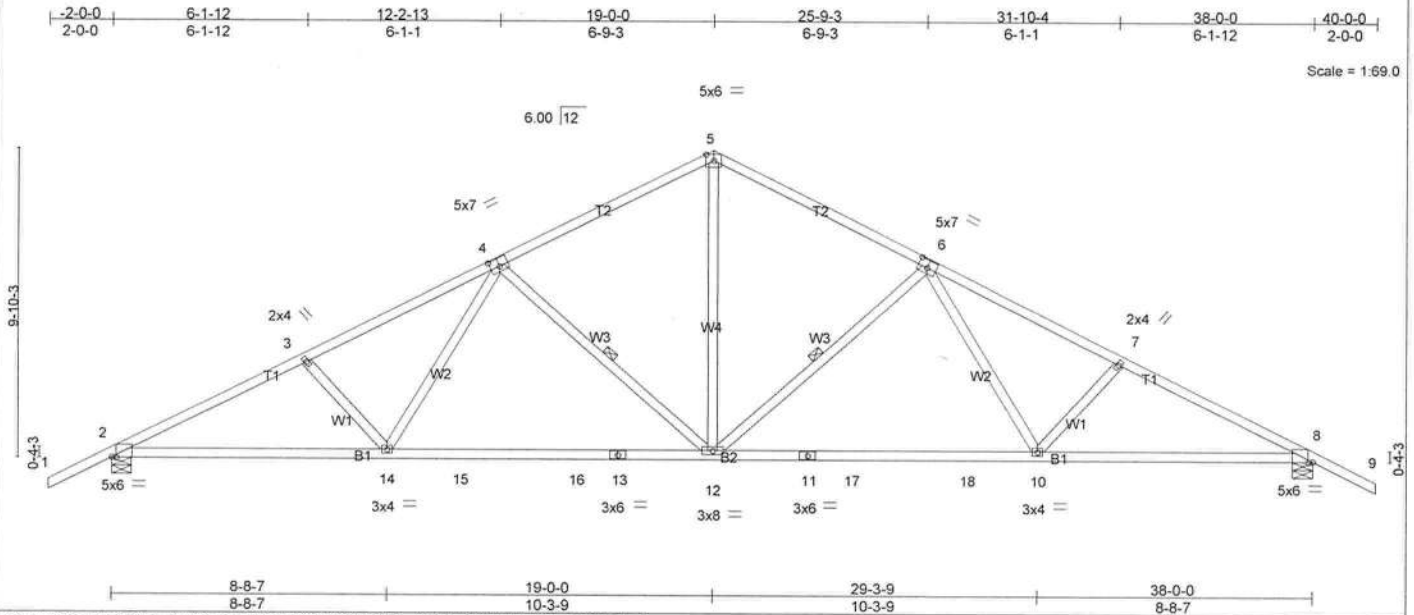


Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-3-8,0-3-0], [6:0-3-8,0-3-0], [8:0-1-11,Edge]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.38	Vert(LL)	-0.38 12-14	>999	360	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.92	Vert(TL)	-0.77 12-14	>580	240		
BCLL 0.0	Lumber Increase 1.25	WB 0.88	Horz(TL)	0.16 8	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Wind(LL)	0.17 12-14	>999	240		
	Code FBC2007/TPI2002						Weight: 198 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 3-4-12 oc purlins.
Rigid ceiling directly applied or 2-2-0 oc bracing.
1 Row at midpt 6-12, 4-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1781/0-7-8, 8=1781/0-7-8
Max Horz 2=148(LC 6)
Max Uplift 2=362(LC 6), 8=362(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3082/1208, 3-4=-2866/1174, 4-5=-1994/922, 5-6=-1994/922, 6-7=-2866/1174, 7-8=-3082/1208

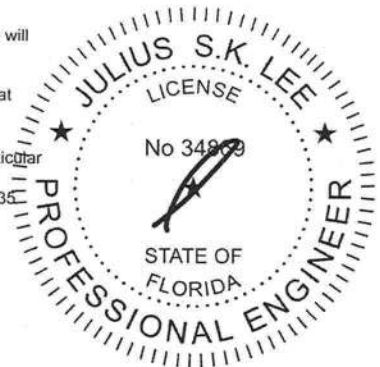
BOT CHORD 2-14=-892/2646, 14-15=-669/2234, 15-16=-669/2234, 13-16=-669/2234, 12-13=-669/2234, 11-12=-669/2234, 11-17=-669/2234, 17-18=-669/2234, 10-18=-669/2234, 8-10=-892/2646

WEBS 5-12=-548/1315, 6-12=-738/455, 6-10=-132/541, 4-12=-738/455, 4-14=-132/541

NOTES (8-9)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone:C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 362 lb uplift at joint 2 and 362 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



September 24, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RHODES BROTHERS - RAMONA PARK CHURCH	I4487877
345573	T01G	GABLE	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Sep 24 08:27:36 2010 Page 2

13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-13=-120(F=-60), 13-25=-120(F=-60), 2-24=-20



September 24, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



Weight: 151 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

(lb) - Max Horz 1=104(LC 5)

(lb) - Max Horz 1=104(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 1, 19, 13 except 18=113(LC 6), 20=122(LC 6), 14=113(LC 7),
12=122(LC 7)
Max Grav All reactions 250 lb or less at joint(s) 1, 11 except 16=426(LC 1), 18=516(LC 10), 19=379(LC 1),
20=369(LC 10), 14=516(LC 11), 13=379(LC 1), 12=369(LC 11)

TOP CHORD 5-6=-85/272, 6-7=-85/272

WEBS 5-18=-263/235, 2-20=-278/251, 7-14=-263/235, 10-12=-278/251

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 110mph (3-second gust); TCDF=4.2psf; BCDF=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

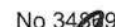
7) All bearings are assumed to be SYP No.2

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 19, 13 except (i) lb 18=113, 20=122, 14=113, 12=122.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



LOAD CASE(S) Standard

September 24, 2010



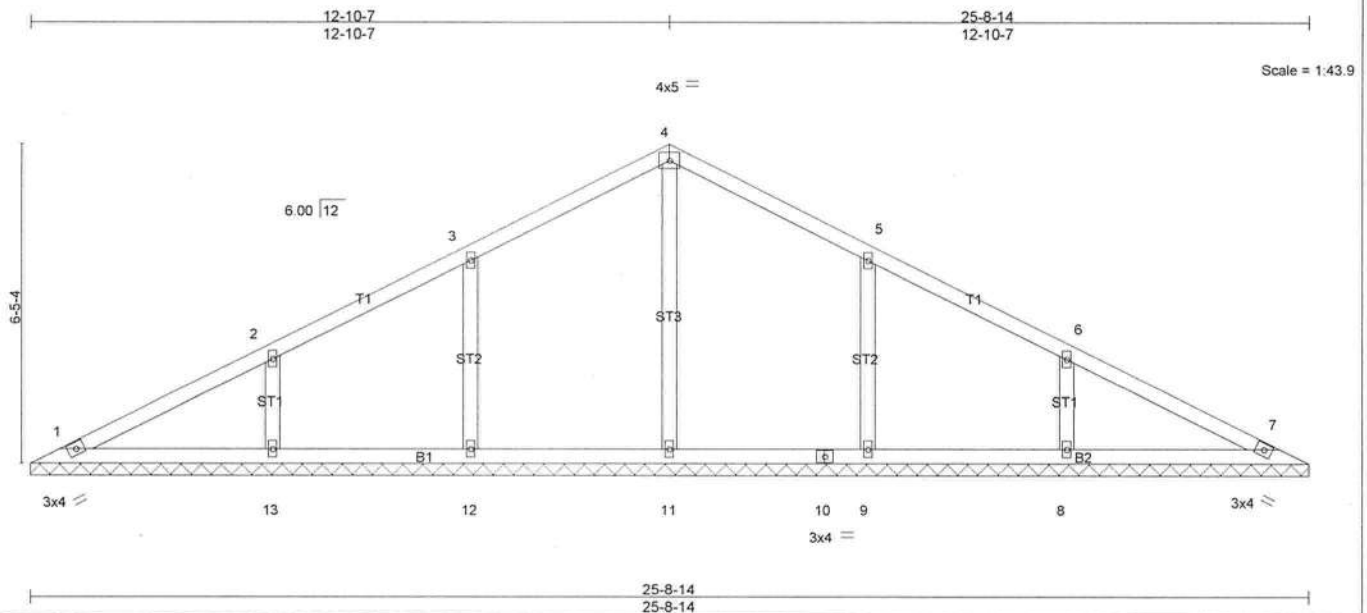
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. The design of the design of the upper incorporation of a component is the responsibility of the building designer - not Truss designer. Bracing shown is for lateral support of individual web members and is not intended to insure stability of the entire truss system during construction. The responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53712.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 345573	Truss V4	Truss Type GABLE	Qty 1	Ply 1	RHODES BROTHERS - RAMONA PARK CHURCH	I4487881
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Sep 24 08:27:38 2010 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.13	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.14	Vert(TL)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.13	Horz(TL)	0.00	7	n/a	n/a		
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)							
									Weight: 105 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 25-8-14.

(lb) - Max Horz 1=-79(LC 4)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 12=-107(LC 6), 13=-119(LC 6), 9=-107(LC 7), 8=-119(LC 7)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=462(LC 1), 12=401(LC 10), 13=365(LC 1), 9=401(LC 11), 8=365(LC 1)

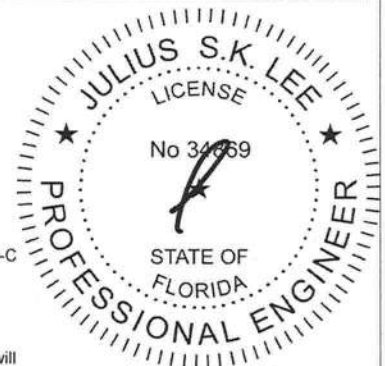
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 3-12=-250/225, 2-13=-273/247, 5-9=-250/225, 6-8=-273/247

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 12=107, 13=119, 9=107, 8=119.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



September 24, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

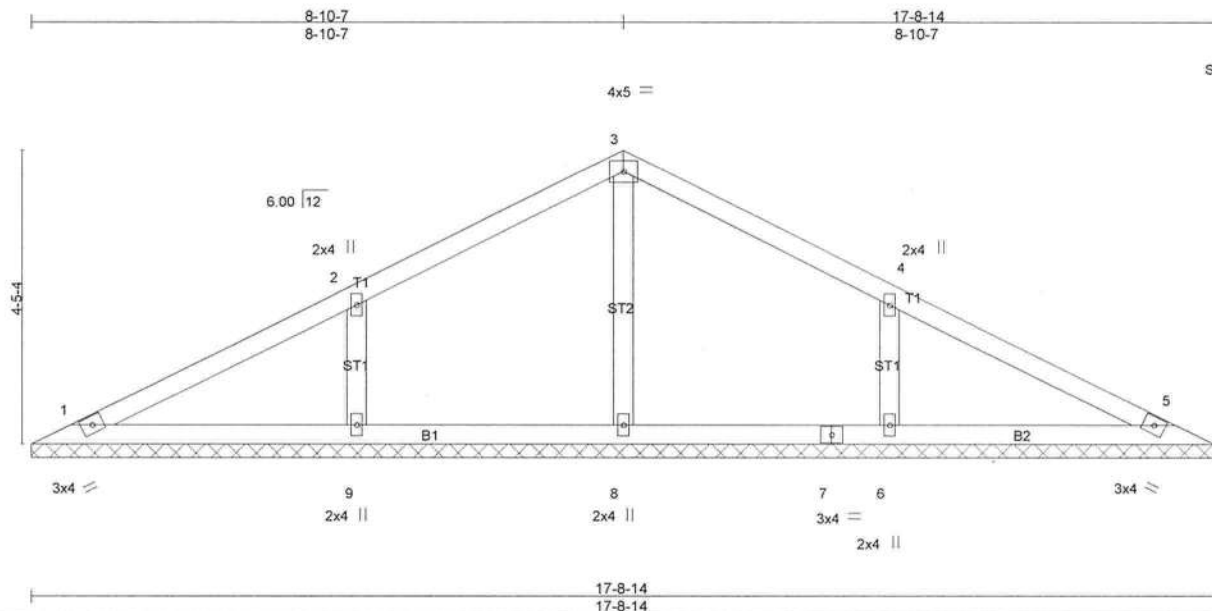
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and 8CSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RHODES BROTHERS - RAMONA PARK CHURCH	I4487883
345573	V6	GABLE	1	1		

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Sep 24 08:27:39 2010 Page 1



Scale = 1:32.7

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.15	Vert(LL) n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.10	Vert(TL) n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr YES	WB 0.07	Horz(TL) 0.00	5	n/a	n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)						
							Weight: 65 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 17-8-14.
(lb) - Max Horz 1=53(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=130(LC 6), 6=130(LC 7)
Max Grav All reactions 250 lb or less at joint(s) 1, 5, 8 except 9=397(LC 10), 6=397(LC 11)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS

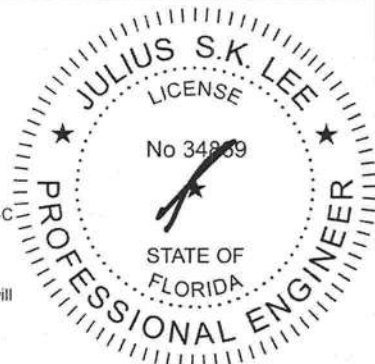
2-9=-295/263, 4-6=-295/263

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 9=130, 6=130.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S)

Standard



September 24, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

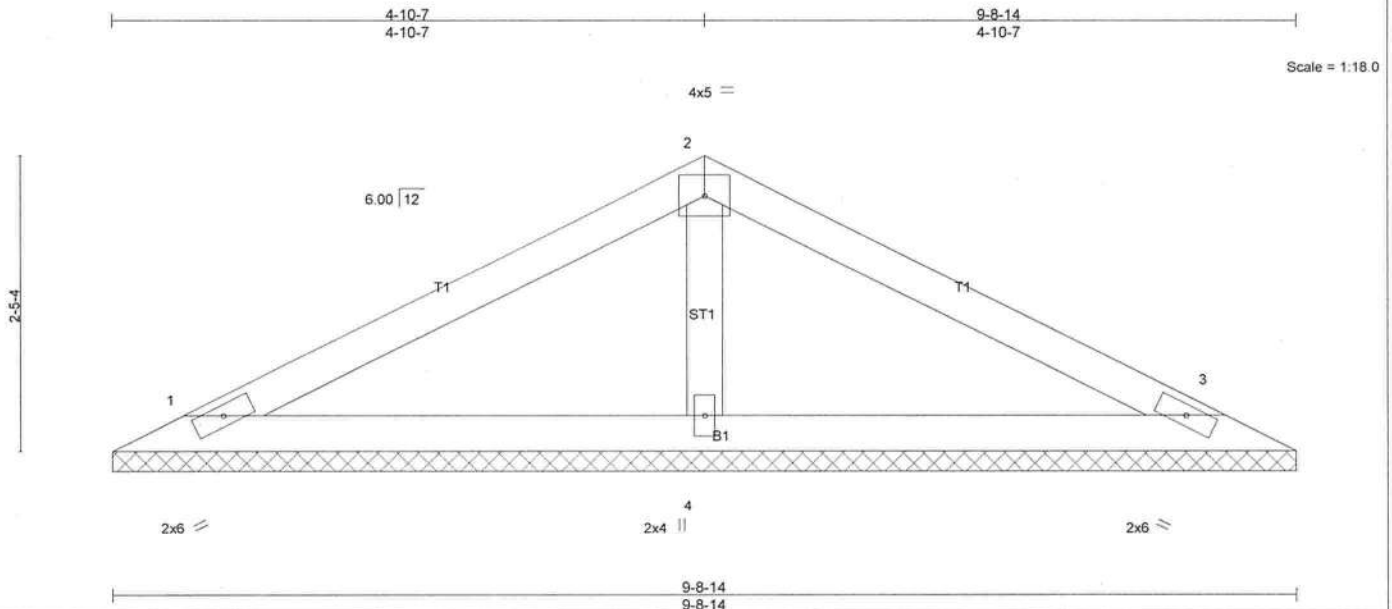
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and 8CSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RHODES BROTHERS - RAMONA PARK CHURCH	14487885
345573	V8	VALLEY	1	1		

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Sep 24 08:27:39 2010 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.13	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.12	Vert(TL)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.05	Horz(TL)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)							
									Weight: 31 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=156/9-8-14, 3=156/9-8-14, 4=369/9-8-14
Max Horz 1=27(LC 4)
Max Uplift 1=37(LC 6), 3=42(LC 7), 4=45(LC 6)
Max Grav 1=157(LC 10), 3=157(LC 11), 4=369(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



September 24, 2010



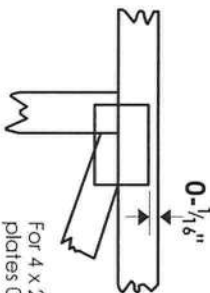
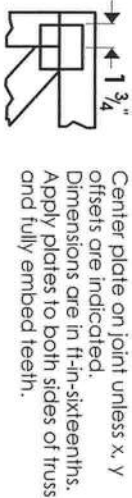
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 X 4

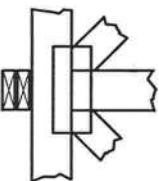
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

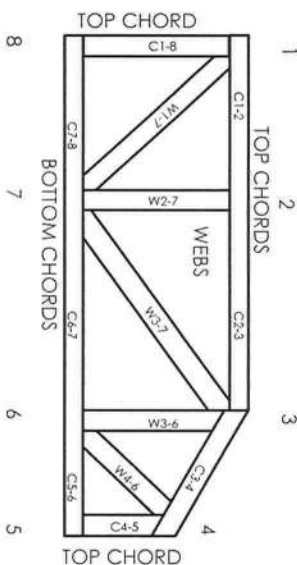


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

STEPPDOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
BOT CHORD 2X4 SO. PINE #2 or Better
WEBS 2X4 SO. PINE #3 or Better

120 MPH MAX

Setback 7' or Less

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less

BRG LOC:

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE

UPLIFT: 400# or Less

BRG LOC:

UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

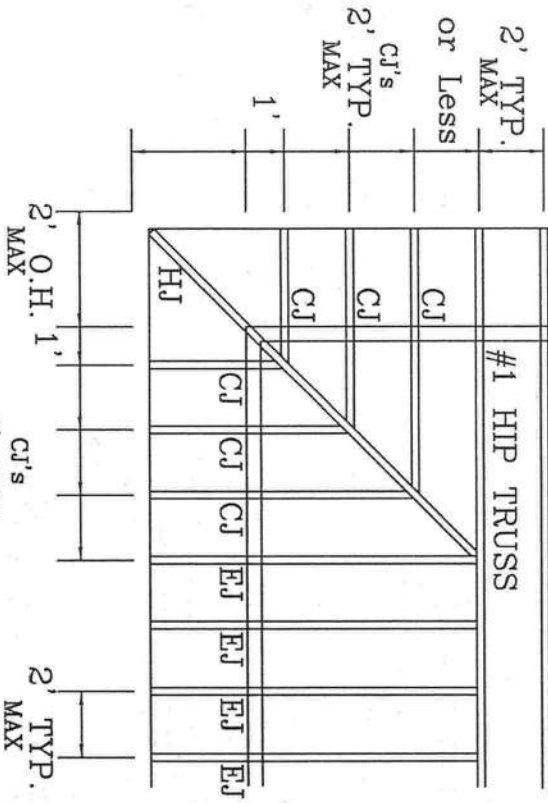
UPLIFT: 400# or Less

BRG LOC:

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

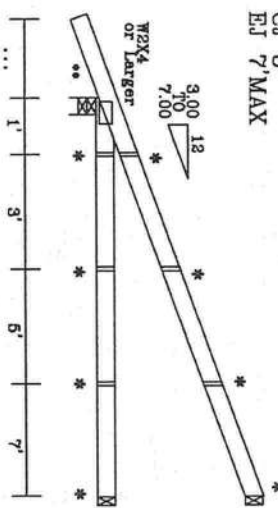
#2 HIP OR COMMON TRUSS

#1 HIP TRUSS



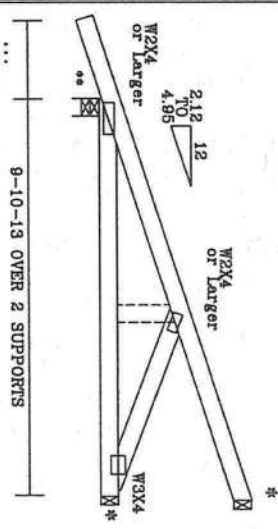
ALL HEELS TO BE STANDBEAR WITH NO CANTILEVER

CJ 1'
CJ 3'
CJ 5'
EJ 7'MAX



ALL HEELS TO BE STANDBEAR WITH NO CANTILEVER

HJ



HIPJACK

(3) 16d TOENAILS

SEE FOR FOR TIE DOWN

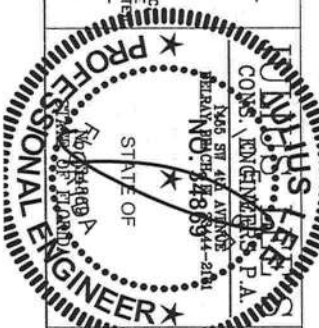
UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET
SETBACK

7'0" MAX

MANUFACTURER TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOSTON BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 583 DOWNEY DR., SUITE 200, MADISON, VT 55719 AND VITA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LN, MADISON, VT 55719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID GELING.

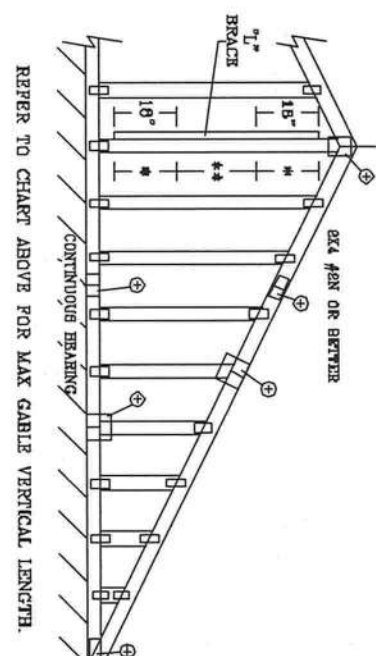
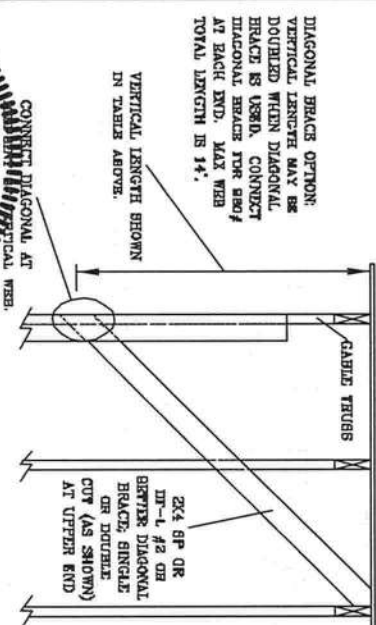
IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN OR FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO THE PERSONS OR PROPERTY. PROVIDE PROPERLY ATTACHED RIGID GELING TO THE TRUSS. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION. THE DESIGNER ASSUMES NO LIABILITY FOR THE TRUSS OR FOR THE BUILDING OR FOR THE PERSONS OR PROPERTY. ANY DEVIATION FROM THIS DESIGN OR FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO THE PERSONS OR PROPERTY SHALL BE THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



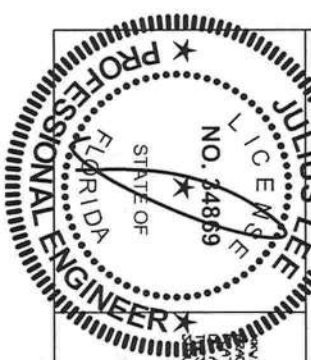
MEMBER	SIZE	MAX PSF	REF
TOP CHORD	2X4	20	7'MAX STBK CS
BOT CHORD	2X4	20	
WEBS	2X4	10*	
DIAGONALS	2X4	20	
VERTICALS	2X4	20	
HORIZONTALS	2X4	20	
SPACING	2'	MAX	
DUR. FAC.	1.25		
REVIEWED	By Julius Lee at 10:52 am, Jun 27, 2008		

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO BRACES		(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE *		(1) 2X6 "L" BRACE *		(2) 2X8 "L" BRACE **	
GABLE VERTICAL SPACING	SPECIES	GRADE	BRACE	NO	GROUP	A	B	A	B	A	B	A	B	A	B
12" O.C.	SPF	#1 / #2	STUD	3	2	5' 6"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
						4' 5"	4' 5"	6' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
						3' 1"	4' 6"	6' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
						2' 11"	3' 8"	6' 0"	6' 0"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
16" O.C.	SPF	#1 / #2	STUD	3	8	5' 6"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
						4' 5"	4' 5"	6' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
						3' 1"	4' 6"	6' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
						2' 11"	3' 8"	6' 0"	6' 0"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
24" O.C.	SPF	#1 / #2	STUD	3	11	5' 6"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
						4' 5"	4' 5"	6' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
						3' 1"	4' 6"	6' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
						2' 11"	3' 8"	6' 0"	6' 0"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEPLETION CRITERIA IS 1/240.	
PROVIDE UPLIFT CONNECTIONS FOR 160 PSF OVER CONTINUOUS BEARING (6 PSF TO DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4' 0" OUTDOCKERS WITH 8' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.	
ATTACH EACH "L" BRACE WITH 10d NAILS.	
* FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) "L" BRACES, SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.	
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	



REVIEWED

By Julius Lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S

CONS. ENGINEERS P.A.

1466 SW 4th Avenue

DELRAY BEACH, FL 33444-2611

No. 34869

STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-GMB130390

DATE 11/26/03

DWG WTRC 97D GABLE 30' E WP

ENG

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

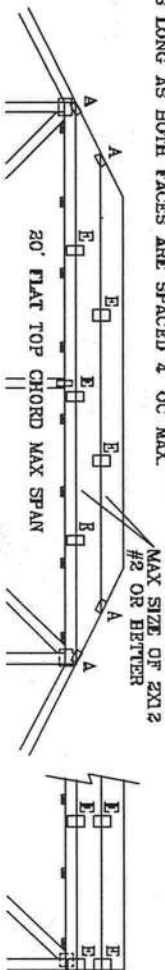
CAT 1, EXP C, WIND TC DL=6 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, FBG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

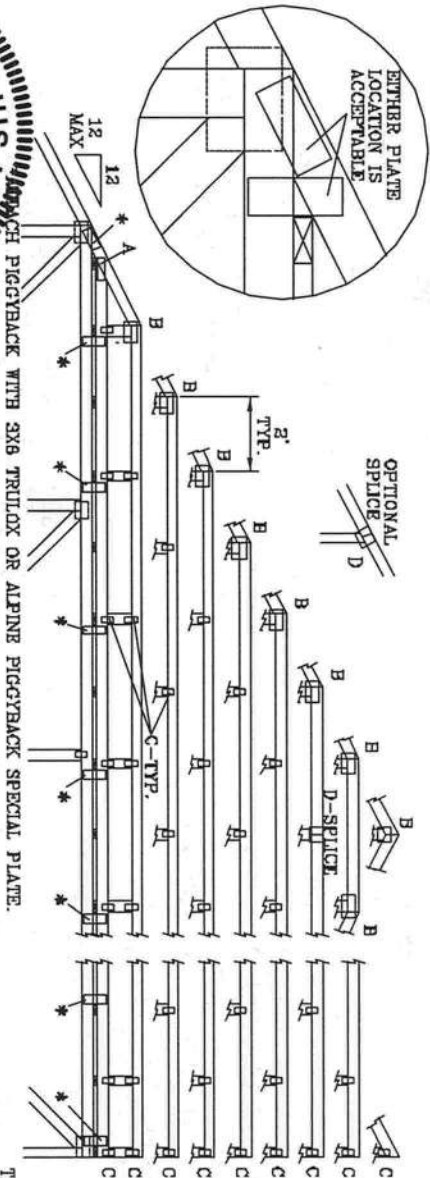
WIND TC DL=6 PSF, WIND BC DL=5 PSF

FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT 1, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF



OPTIONAL SPLICE



THIS DRAWING REPLACES DRAWINGS 634.019 634.017 & 647.045

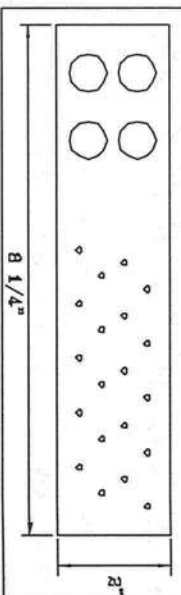
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X6
B	4X6	6X6	6X6	6X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	6X6	6X6	6X6
E	4X3 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRUSS PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

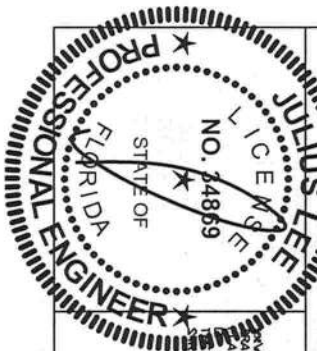
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "I" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH, OC.
10' TO 14'	2X4 "I" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



8 1/4"



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4TH AVENUE
DEERBEE BEACH, FL 33444-2181

No. 34869
STATE OF FLORIDA

MAX LOADING	55 PSF AT	REF	PIGGYBACK
	1.33 DUR. FAC.	DATE	09/12/07
50 PSF AT	1.25 DUR. FAC.	DRWG/MTK STD PIGGY	
	47 PSF AT	-ENG JL	
1.15 DUR. FAC.			
SPACING 24.0"			

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AP&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

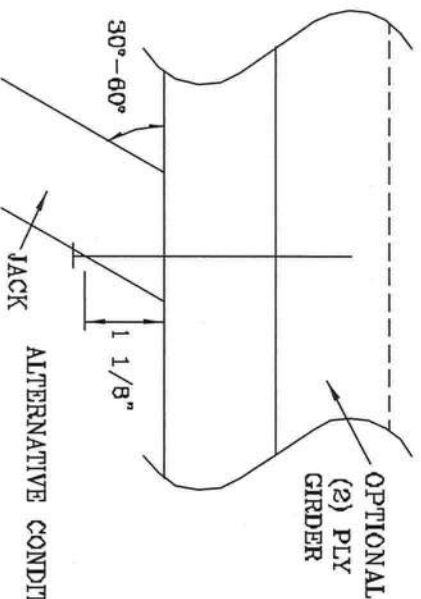
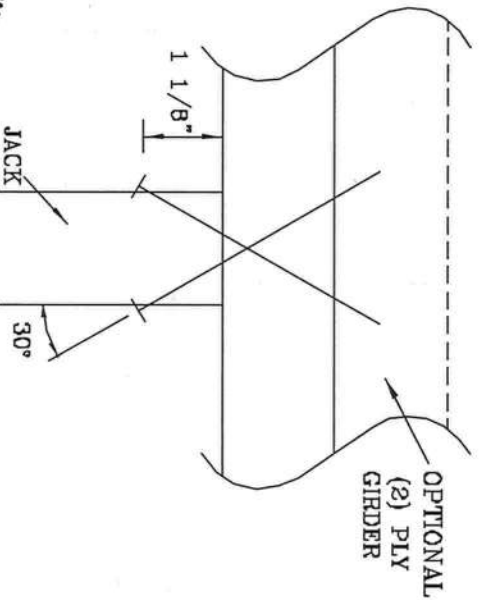
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

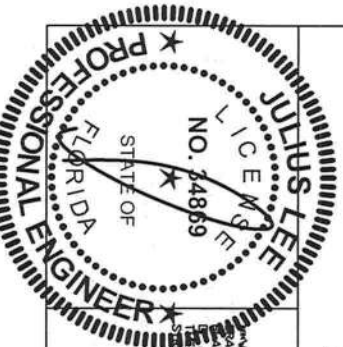
NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOST 1-03 CALLING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS BRACING INSTITUTE, 288 TONGERIDGE RD., SUITE 200, NANTUCKET, VT 05719 AND VITA WOOD TRUSS COUNCIL, 6680 ENTERPRISE LN, NORTON, MA 01945 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED SUBSTRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.

1490 ST. ALD. AVENUE
DELRAY BEACH, FL 33444-2161

No. 34869
STATE OF FLORIDA

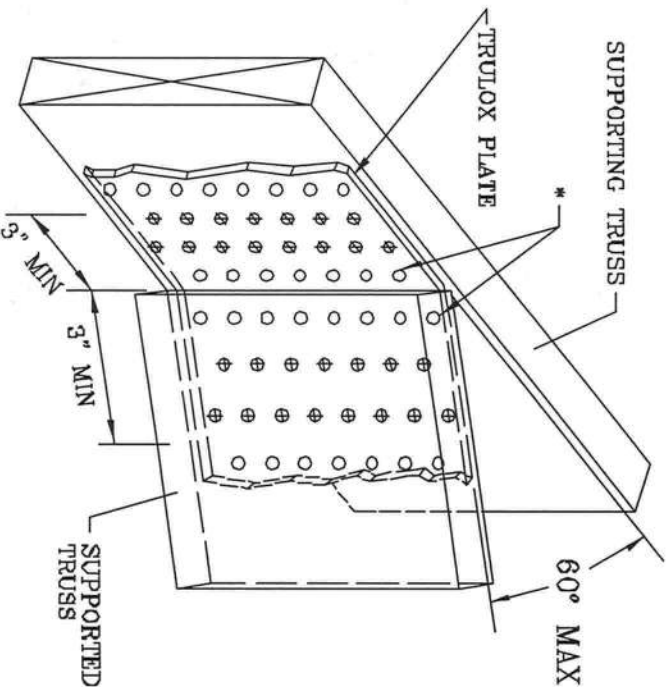
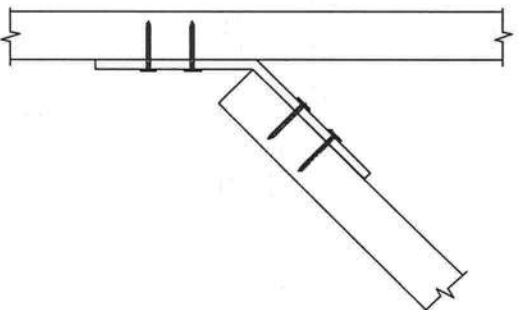
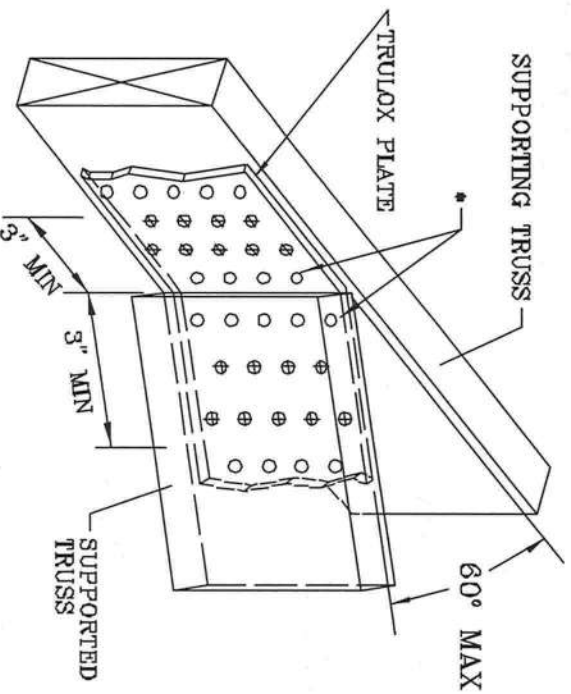
TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3x6	9	350.#
6x6	15	990.#

MINIMUM 5X6 TRULOX PLATE

REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

THIS DRAWING REPLACES DRAWINGS 1.158,989 1.158,989/R
1.154,844 1.152,217 1.152,017 1.159,154 & 1.151,524

JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

[illegible]

REF	TRULOX
-----	--------

DATE 11/26/03

DRWG CINTRULOX1103

-ENG JL

No: 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
			3 1/2\" 2-ply	5 1/4\" 3-ply	5 1/4\" 2-ply	7\" 3-ply	7\" 2-ply	7\" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 1/8" maximum.

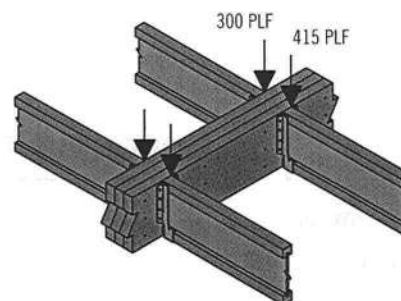
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



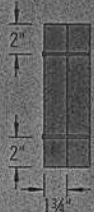


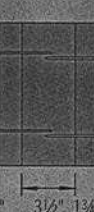


First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
							
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WSG ⁽¹⁾	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
3 3/8" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽³⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽³⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽³⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

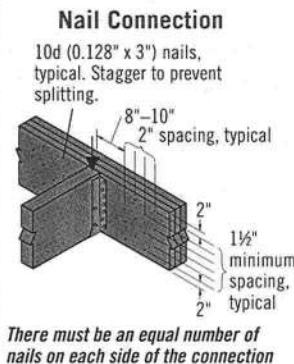
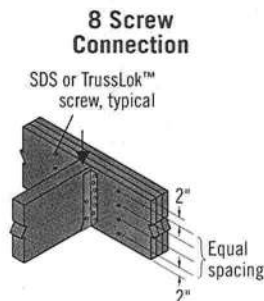
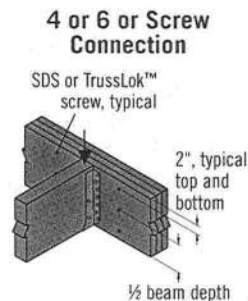
(2) 6" long screws required.

(3) 5" long screws required.

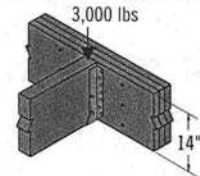
(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

See General Notes on page 38

Connections



Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

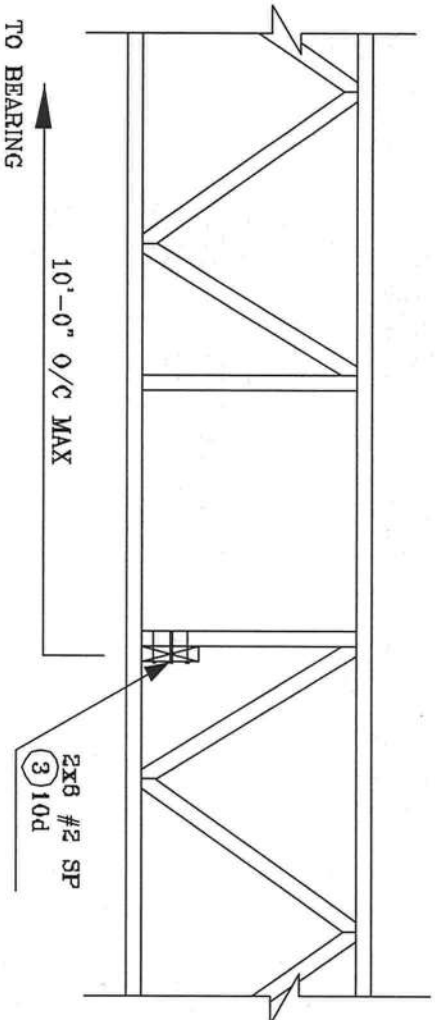
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



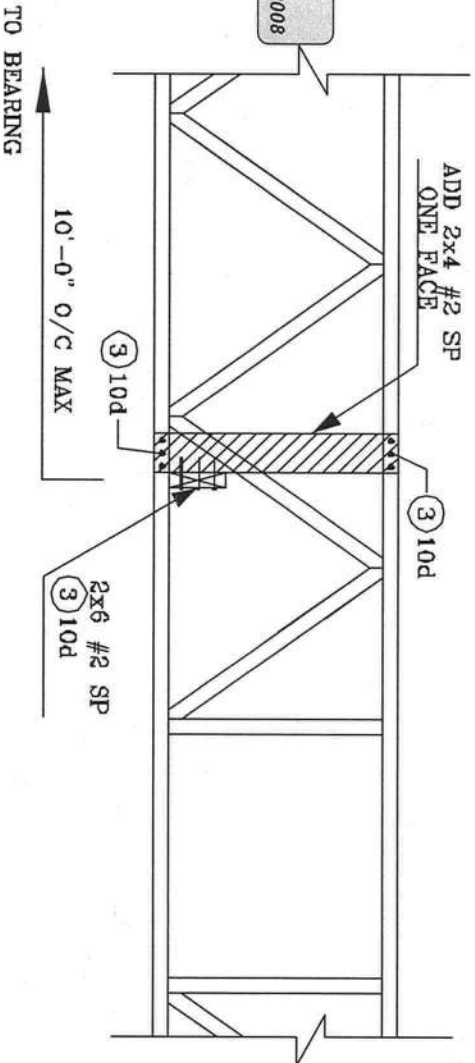
Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6

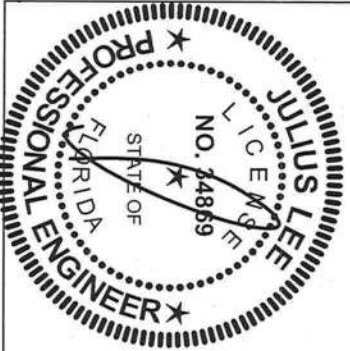
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008



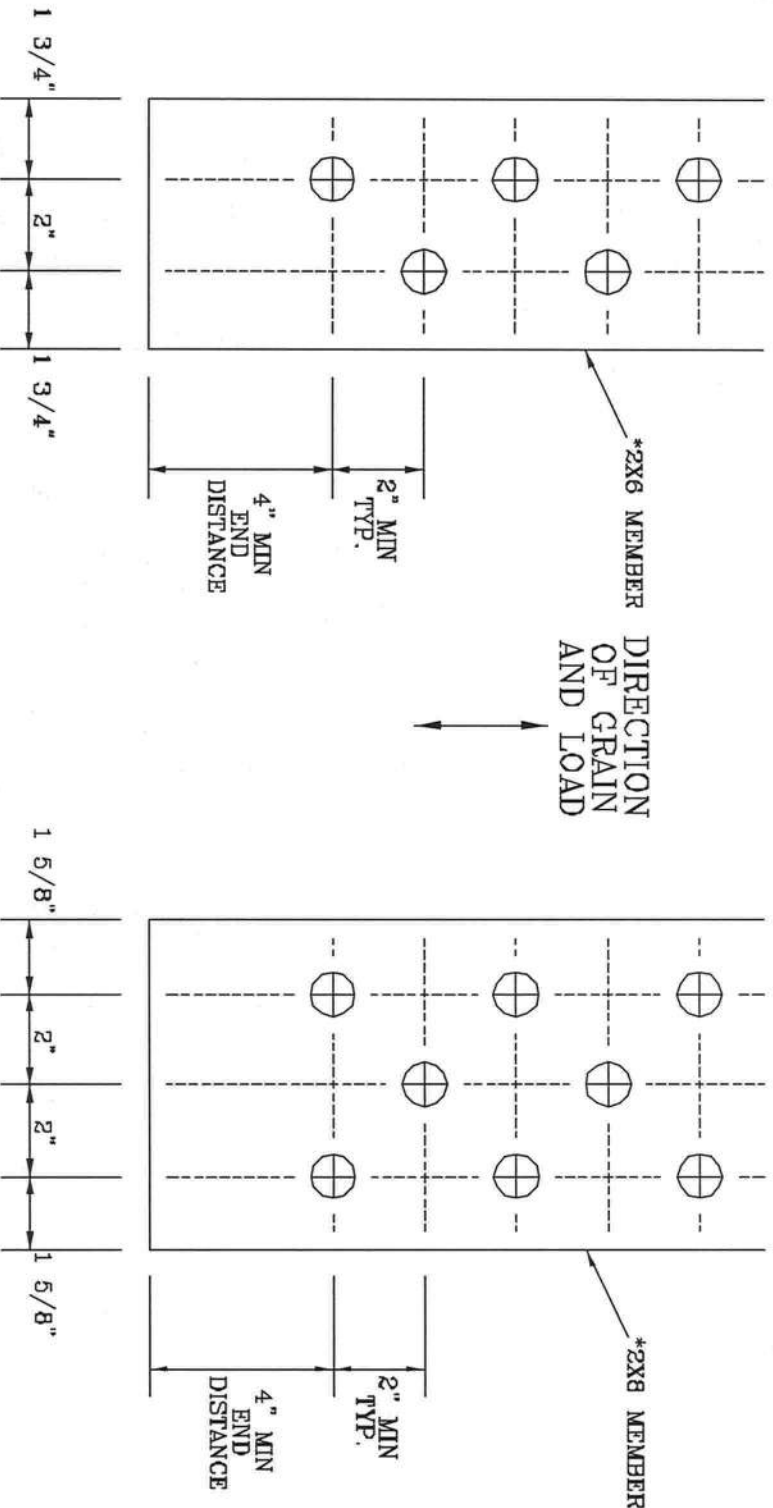
JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 45th AVENUE
DEERAT BEACH, FL 33444-2191

No: 34869
STATE OF FLORIDA

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

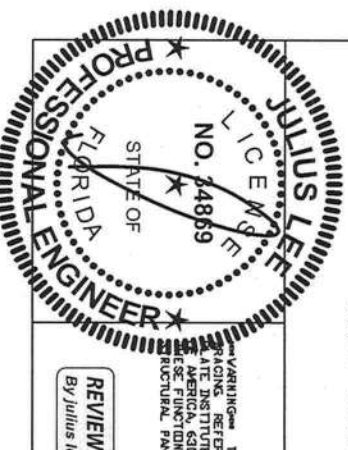
TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016



WARNING: TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SUPPORTING, INSTALLING AND BRACING. REFER TO THE DESIGN DRAWINGS FOR ALL SPECIFICATIONS. THE FOLLOWING INFORMATION IS FOR YOUR INFORMATION ONLY. IT IS NOT A SUBSTITUTE FOR THE DESIGN DRAWINGS. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. THE TRUSS SHALL BE DESIGNED TO RESIST ALL LOADS AND STRESSES INDICATED. THE TRUSS SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1405 IV 4th AVENUE
DELEET BEACH, FL 33441-2161

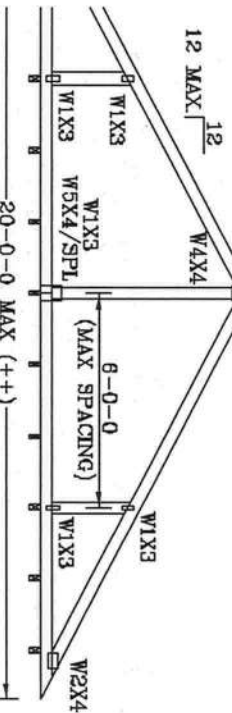
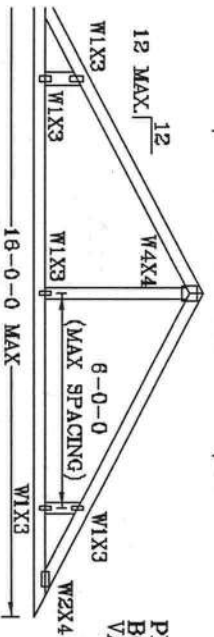
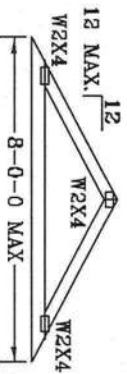
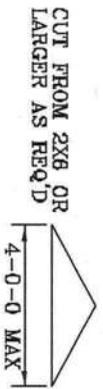
No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

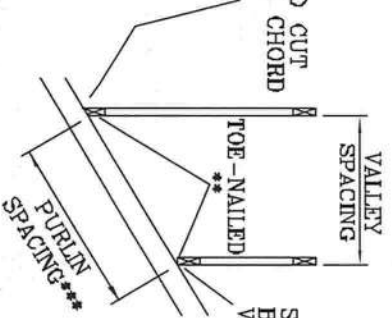
VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

- * 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- ** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF.



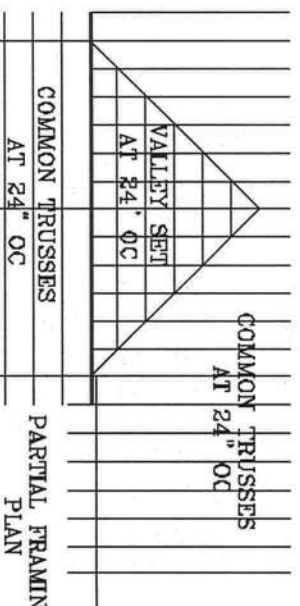
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SQUARE CUT BOTTOM CHORD VALLEY

OPTIONAL STUB END DETAIL

OPTIONAL HIP JOINT DETAIL

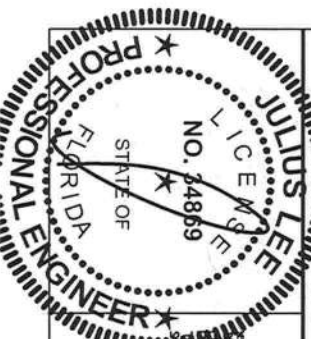


COMMON TRUSSES AT 24" OC

PARTIAL FRAMING PLAN

THIS DRAWING REPLACES DRAWING A105

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

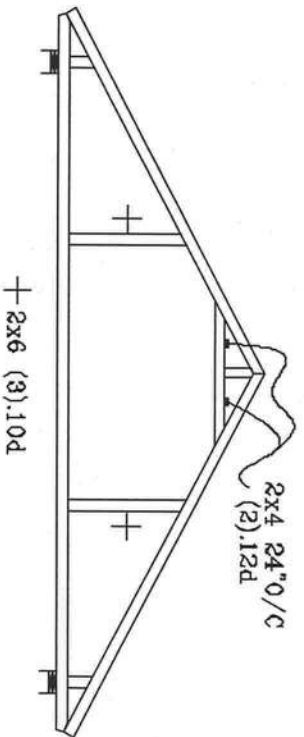
JULIUS LEE'S
CONS. ENGINEERS P.A.

1655 SW 4th Avenue
DeBary Beach, FL 32444-5101

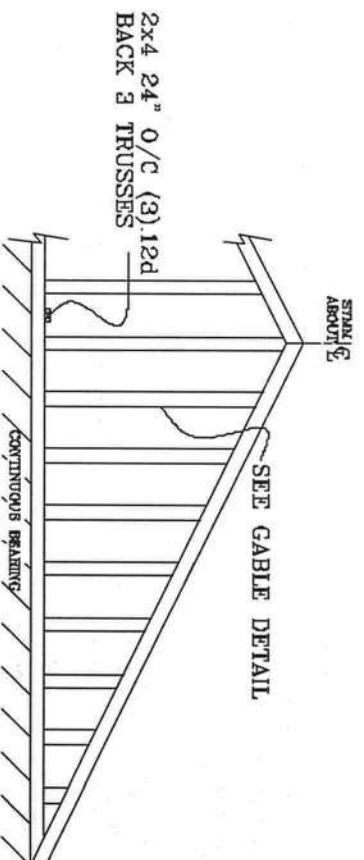
No. 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		
DURFAC	1.25	1.25			
SPACING	24"				

TYPICAL ATTIC TRUSS BRACING

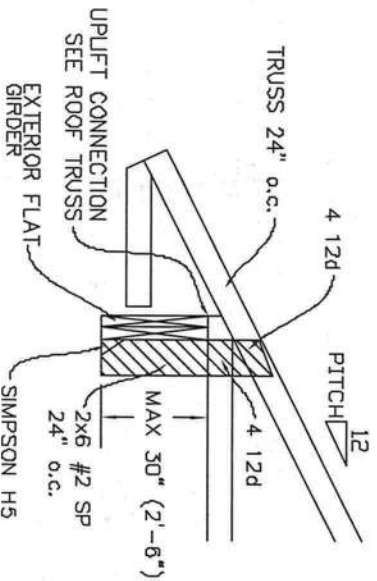


CABLE END TRUSS DETAIL

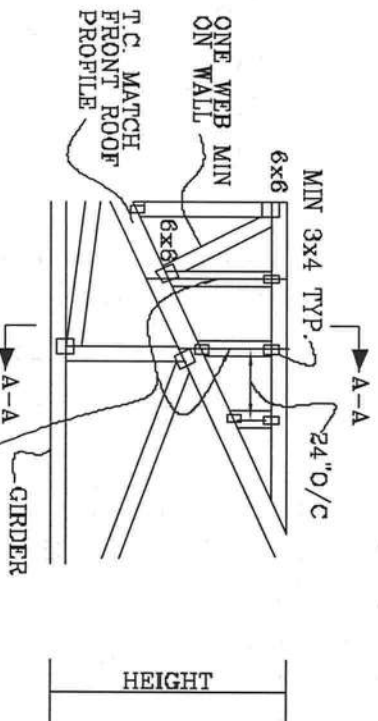


MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNED BY ARCHITECT OR BOR

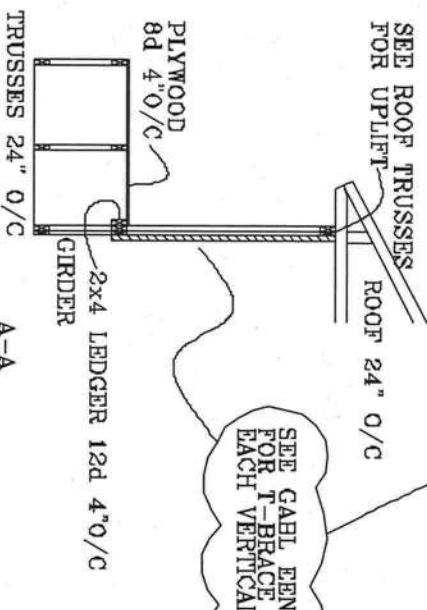
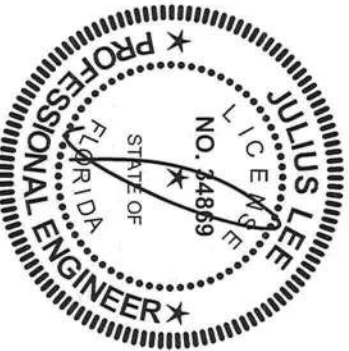
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



JULIUS LEE'S
CONS. ENGINEERS P.A.
1465 SW 4th AVENUE
DUNN BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

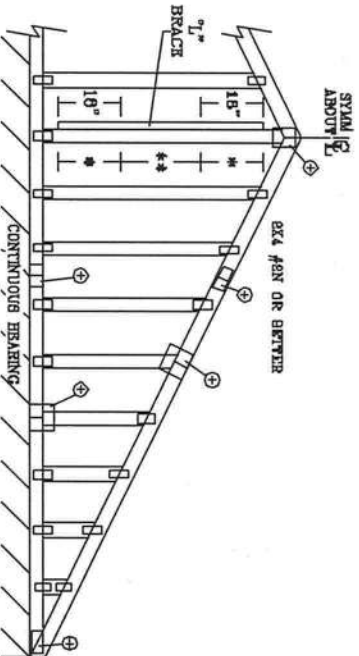
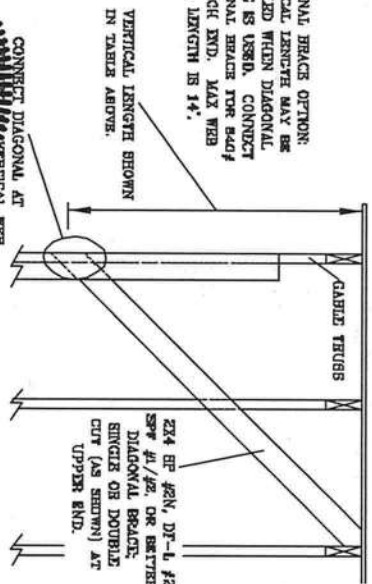
MAX GABLE VERTICAL LENGTH													
CABLE VERTICAL SPACING / SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(1) 2X6 "L" BRACE *		(2) 2X8 "L" BRACE *				
			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B			
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	9' 8"	9' 8"	11' 8"	11' 8"
		#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
	SP DFL	#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 7"
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 6"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 6"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	9' 10"	9' 10"	12' 0"	12' 0"
		#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
16" O.C.	SPF HF	#3	3' 8"	8' 0"	8' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	6' 2"	6' 2"	7' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
		#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
	SP DFL	#3	4' 0"	8' 1"	6' 1"	7' 11"	8' 2"	9' 6"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	8' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
		#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 6"	13' 8"	13' 8"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
12" O.C.	SPF HF SP DFL	STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		#1	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		#2	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCE-PINE-FIR		HDK-FIR	
#1 / #2	STANDARD	#1 & BTR	
#3	STUD	#1	
DOUGLAS FIR-LARCH		HDK-FIR	
#2	STUD	#1 & BTR	
STANDARD		#1	
SOUTHERN PINE		HDK-FIR	
#3	STUD	#1 & BTR	
STANDARD		#1	
SOUTHERN PINE		HDK-FIR	
#1 / #2	STANDARD	#1 & BTR	
#3	STUD	#1	
DOUGLAS FIR-LARCH		HDK-FIR	
#2	STUD	#1 & BTR	
STANDARD		#1	

LIVE LOAD DEPLETION CRITERIA IS C/240.
PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER
CONTINUOUS BEARING (6 PSF WC DEAD LOAD).
LIFT FROM EMBEDDED 10IN. DEEP 4" Ø"

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRICKS
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2,5X4

+ REFER TO COMMON THRESH DESIGN FOR
PEAK, SPLICE, AND BEEL PLATES.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

MANUFACTURING. THESE RESINULE EXTENSIVE CARE FABRICATING, HANDING, SHAPING, INSTALLING AND REMOVING. REFER TO BEST 1-40 (BUILDING COMPONENT SAFETY IN MANUFACTURING), PULLING, FOR THESE PLATE INSTITUTE, 883 DUNCAN RD., SUITE 200, MANASSA, VA 20108 AND VITA (VITA) TRUSS COUNCIL OF AMERICA, 6300 EIGHTH AVE., WASHINGTON, VA 22204 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP BOARD SHALL HAVE PROPERLY ATTACHED RIGID PANELS AND BOTTOM BOARD SHALL HAVE A PROPERLY ATTACHED CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVE/NOE
DELRAY BEACH, FL 33444-2161

REF	ASCE7-02-CAB13015
DATE	11/26/03
DRWG	MITK STD CABLE 15 T HT
-ENG	

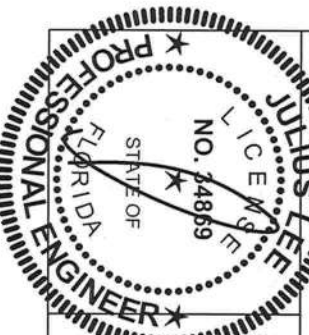
-ENG

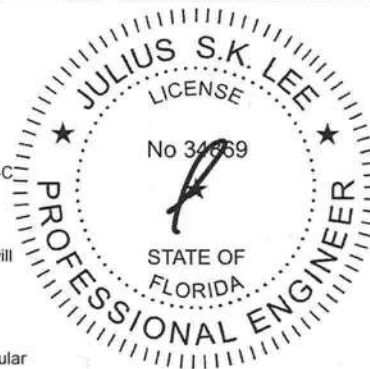
REVIEWED

By Julius Lee at 12:00 pm, Jun 11, 2008

No: 34869
STATE OF FLORIDA

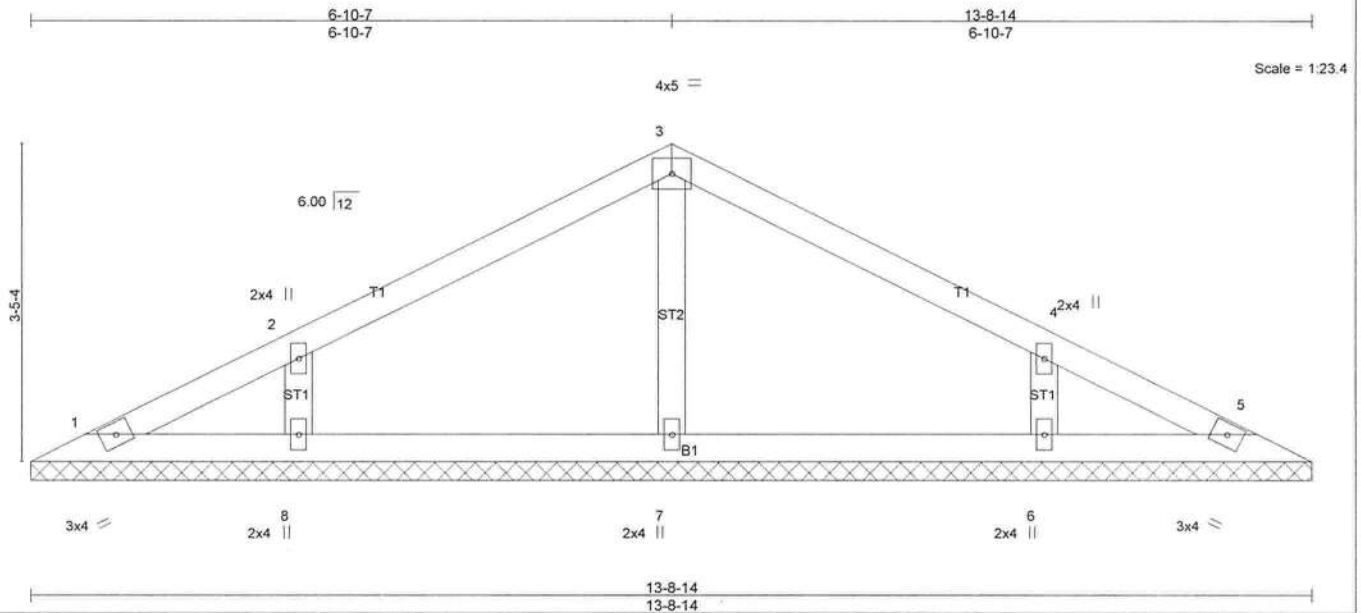
MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"





September 24, 2010

Job 345573	Truss V7	Truss Type GABLE	Qty 1	Ply 1	RHODES BROTHERS - RAMONA PARK CHURCH	14487884
Builders FrstSource, Lake City, FL 32055					Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Sep 24 08:27:39 2010 Page 1	



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.15	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.08	Vert(TL)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.06	Horz(TL)	0.00	5	n/a	n/a		
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)							
										Weight: 48 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

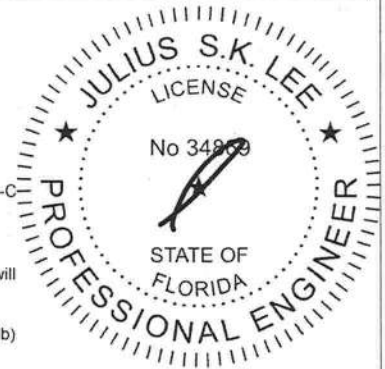
All bearings 13-8-14.
(lb) - Max Horz 1=40(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 7 except 8=102(LC 6), 6=102(LC 7)
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=285(LC 1), 8=305(LC 10), 6=305(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 7 except (jt=lb) 8=102, 6=102.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



September 24, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

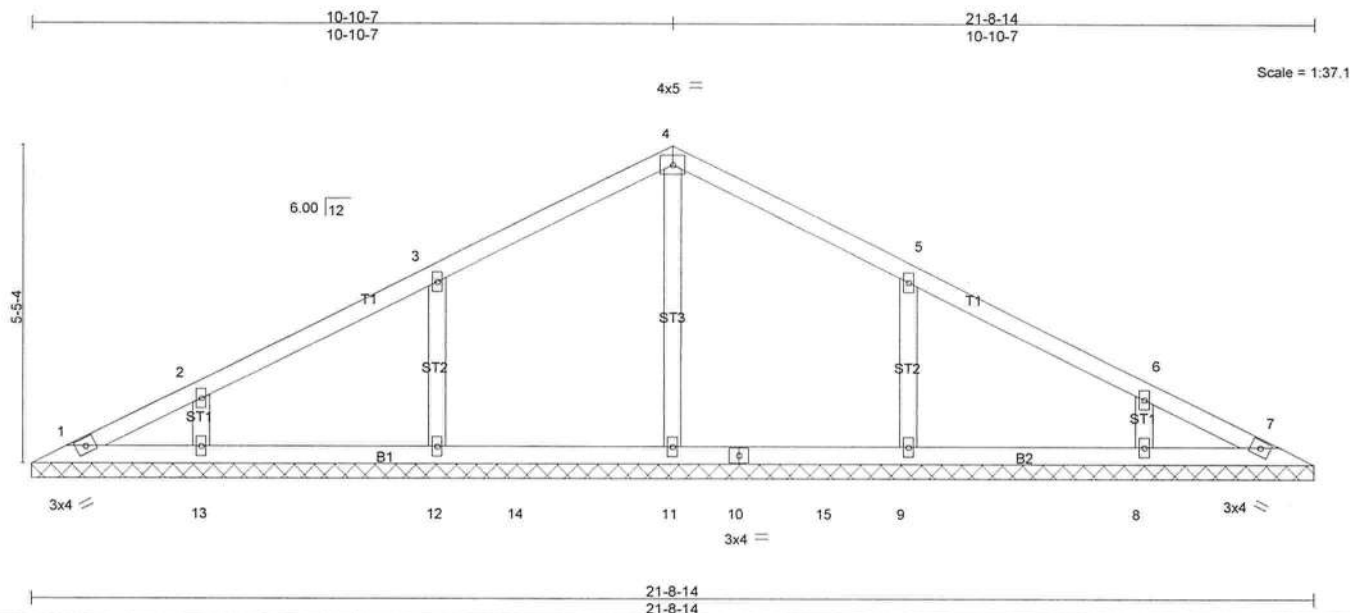
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 345573	Truss V5	Truss Type GABLE	Qty 1	Ply 1	RHODES BROTHERS - RAMONA PARK CHURCH	I4487882
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Sep 24 08:27:38 2010 Page 1



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.13	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.11	Vert(LL) n/a - n/a 999		
BCLL 0.0	Lumber Increase 1.25	WB 0.09	Vert(TL) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 7 n/a n/a		
	Code FBC2007/TPI2002			Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

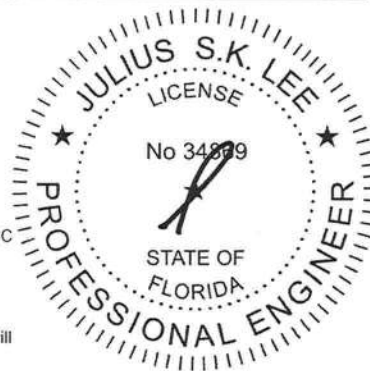
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 21-8-14.
(lb) - Max Horz 1=-66(LC 4)
Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 8 except 12=-115(LC 6), 9=-115(LC 7)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=431(LC 1), 12=379(LC 10), 13=269(LC 1), 9=379(LC 11), 8=269(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-12=-267/240, 5-9=-267/240

- NOTES** (10-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 8 except (jt=lb) 12=115, 9=115.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



September 24, 2010

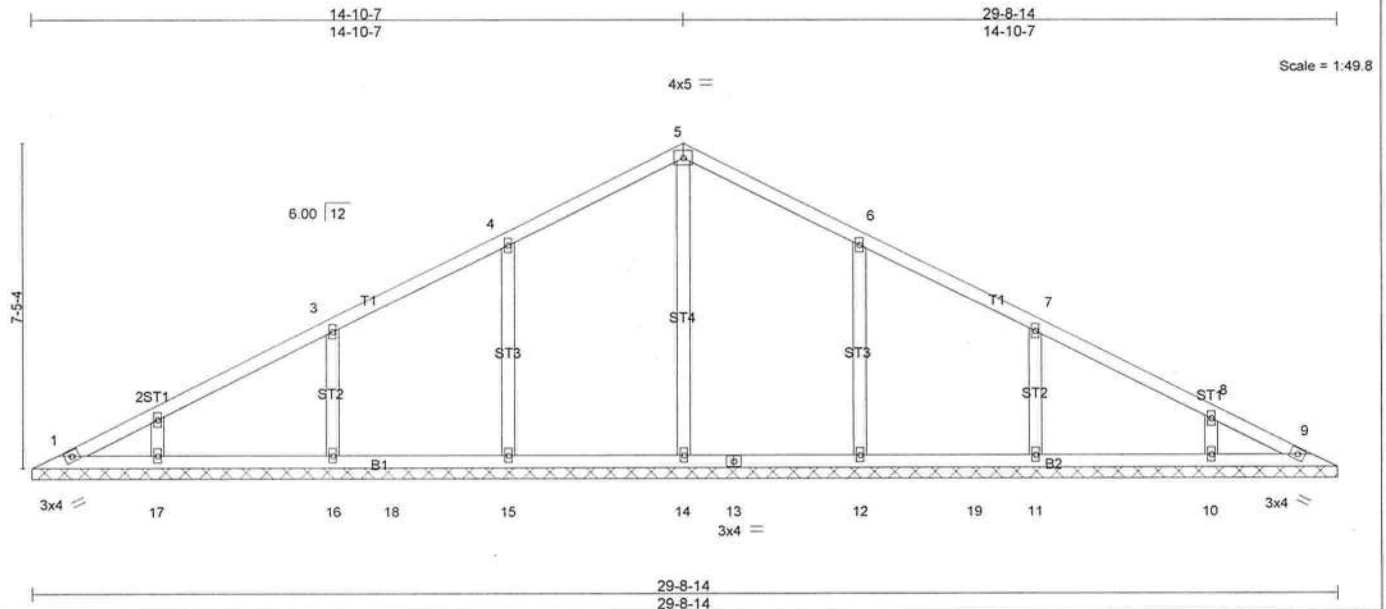


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 345573	Truss V3	Truss Type GABLE	Qty 1	Ply 1	RHODES BROTHERS - RAMONA PARK CHURCH	I4487880
Builders FrstSource, Lake City, FL 32055						Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Sep 24 08:27:38 2010 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.12	Vert(LL) n/a	-	n/a 999	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.11	Vert(TL) n/a	-	n/a 999		
BCLL 0.0	Rep Stress Incr YES	WB 0.17	Horz(TL) 0.00	9	n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)					
						Weight: 128 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 29-8-14.
(lb) - Max Horz 1=-91(LC 4)
Max Uplift All uplift 100 lb or less at joint(s) 1, 17, 10 except 15=-111(LC 6), 16=-103(LC 6), 12=-111(LC 7), 11=-104(LC 7)
Max Grav All reactions 250 lb or less at joint(s) 1, 9 except 14=433(LC 1), 15=499(LC 10), 16=355(LC 1), 17=275(LC 10), 12=499(LC 11), 11=355(LC 1), 10=275(LC 11)

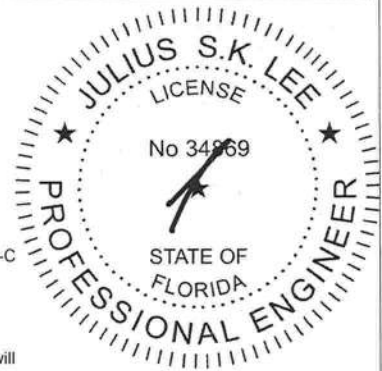
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 4-15=-258/231, 6-12=-258/231

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are to be SYP No.2
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 17, 10 except (jt=lb) 15=111, 16=103, 12=111, 11=104.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



September 24, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

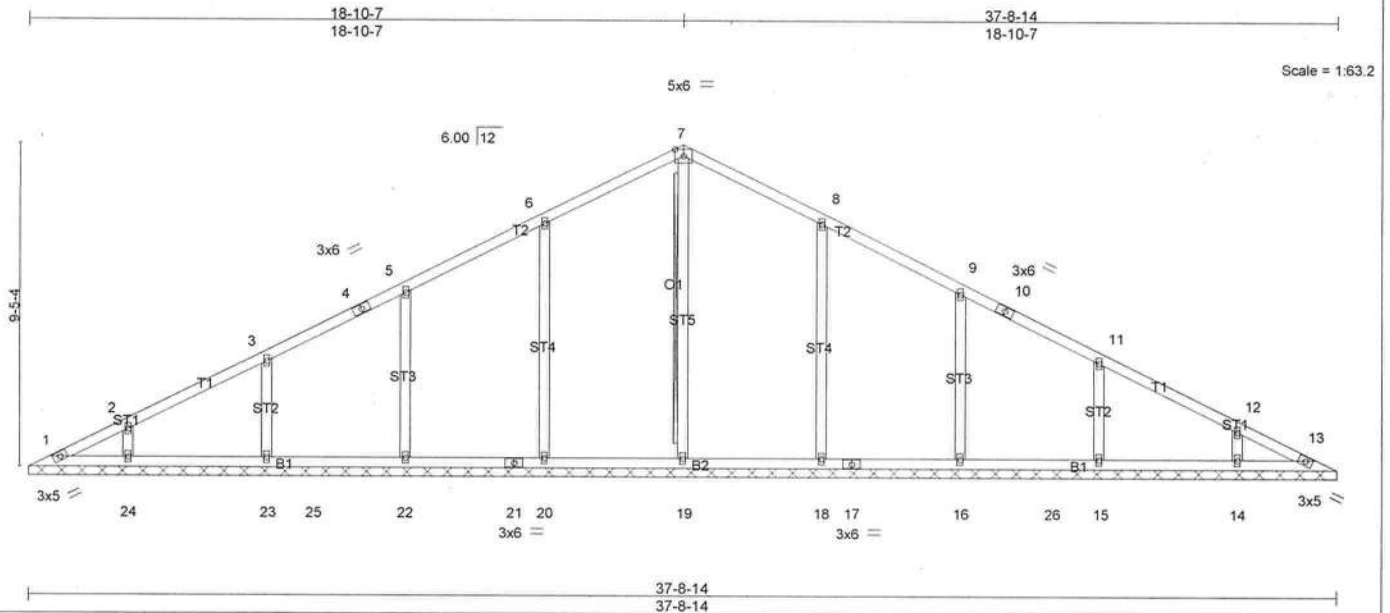
Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 345573	Truss V1	Truss Type GABLE	Qty 1	Ply 1	RHODES BROTHERS - RAMONA PARK CHURCH	I4487878
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Sep 24 08:27:36 2010 Page 1



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.12	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.11	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.23	Vert(TL) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.01 13 n/a n/a		
	Code FBC2007/TPI2002			Weight: 176 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.
T-Brace: 2 X 4 SYP No.3 - 7-19
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 37-8-14.
(lb) - Max Horz 1=-117(LC 4)
Max Uplift All uplift 100 lb or less at joint(s) 1, 24, 14 except 20=-112(LC 6), 22=-100(LC 6), 23=-106(LC 6), 18=-111(LC 7), 16=-100(LC 7), 15=-106(LC 7)
Max Grav All reactions 250 lb or less at joint(s) 1, 13 except 19=433(LC 1), 20=500(LC 10), 22=475(LC 1), 23=360(LC 10), 24=274(LC 1), 18=500(LC 11), 16=475(LC 1), 15=360(LC 11), 14=274(LC 1)

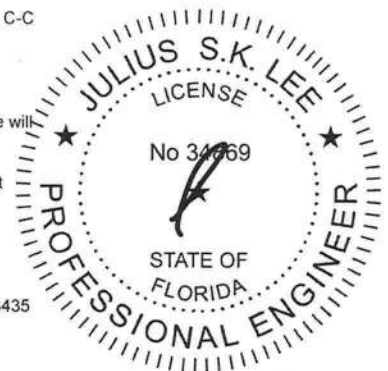
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 6-7=-85/308, 7-8=-85/308
WEBS 6-20=-260/232, 8-18=-260/232

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SYP No.2
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 24, 14 except (jt=lb) 20=112, 22=100, 23=106, 18=111, 16=100, 15=106.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



September 24, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MU 7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 345573	Truss T01G	Truss Type GABLE	Qty 1	Ply 1	RHODES BROTHERS - RAMONA PARK CHURCH	14487877
Builders FrstSource, Lake City, FL 32055			Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Sep 24 08:27:36 2010 Page 1			

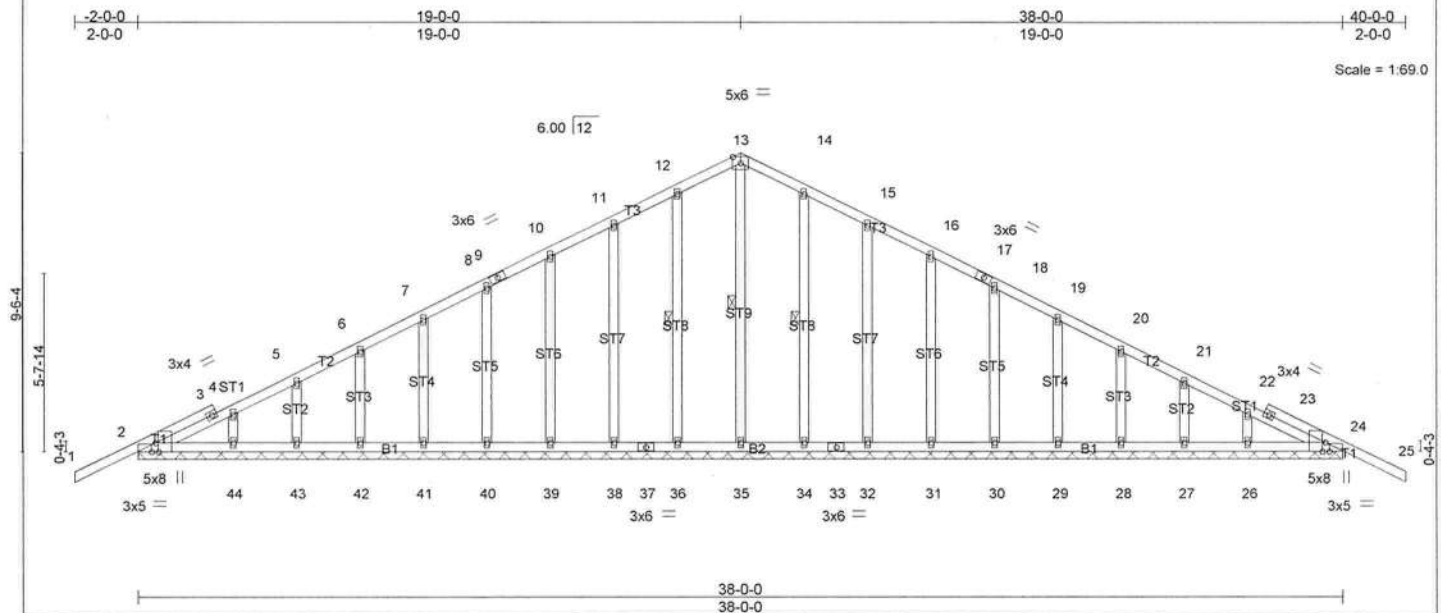


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-1-8,Edge], [24:0-3-8,Edge], [24:0-1-8,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.52	Vert(LL)	-0.04	25	n/r	120	MT20
TCDL 10.0	Lumber Increase	1.25	BC 0.06	Vert(TL)	-0.08	25	n/r	90	244/190
BCLL 0.0	Rep Stress Incr	NO	WB 0.22	Horz(TL)	0.01	24	n/a	n/a	
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)						Weight: 252 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.
1 Row at midpt 13-35, 12-36, 14-34

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 38-0-0.
(lb) - Max Horz 2=-167(LC 7)

Max Uplift All uplift 100 lb or less at joint(s) 44, 26 except 2=-236(LC 6), 24=-262(LC 7), 36=-131(LC 6), 38=-138(LC 6), 39=-133(LC 6), 40=-134(LC 6), 41=-135(LC 6), 42=-130(LC 6), 43=-149(LC 6), 34=-127(LC 7), 32=-139(LC 7), 31=-133(LC 7), 30=-134(LC 7), 29=-135(LC 7), 28=-130(LC 7), 27=-149(LC 7)
Max Grav All reactions 250 lb or less at joint(s) 35 except 2=525(LC 1), 24=525(LC 1), 36=293(LC 10), 38=278(LC 10), 39=280(LC 1), 40=280(LC 10), 41=280(LC 10), 42=279(LC 1), 43=283(LC 10), 44=284(LC 1), 34=293(LC 11), 32=278(LC 11), 31=280(LC 1), 30=280(LC 11), 29=280(LC 11), 28=279(LC 1), 27=283(LC 11), 26=284(LC 1)

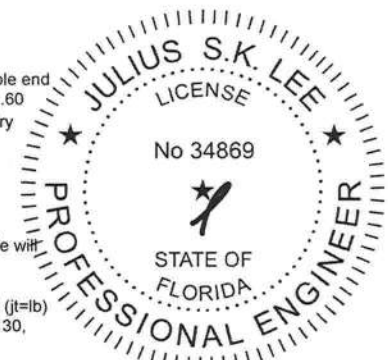
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 10-11=-79/251, 11-12=-78/311, 12-13=-83/364, 13-14=-83/364, 14-15=-78/311, 15-16=-79/251
WEBS 12-36=-253/156, 14-34=-253/156

NOTES (13-14)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 44, 26 except (jt=lb) 2=236, 24=262, 36=131, 38=138, 39=133, 40=134, 41=135, 42=130, 43=149, 34=127, 32=139, 31=133, 30=134, 29=135, 28=130, 27=149.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

Continued on page 2
In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).



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