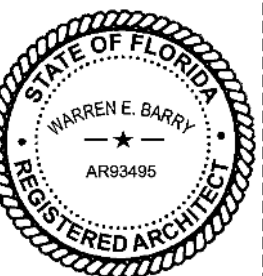




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CONSTRUCTION
DOCUMENTS FOR
CONTRACTOR'S
STANDARD
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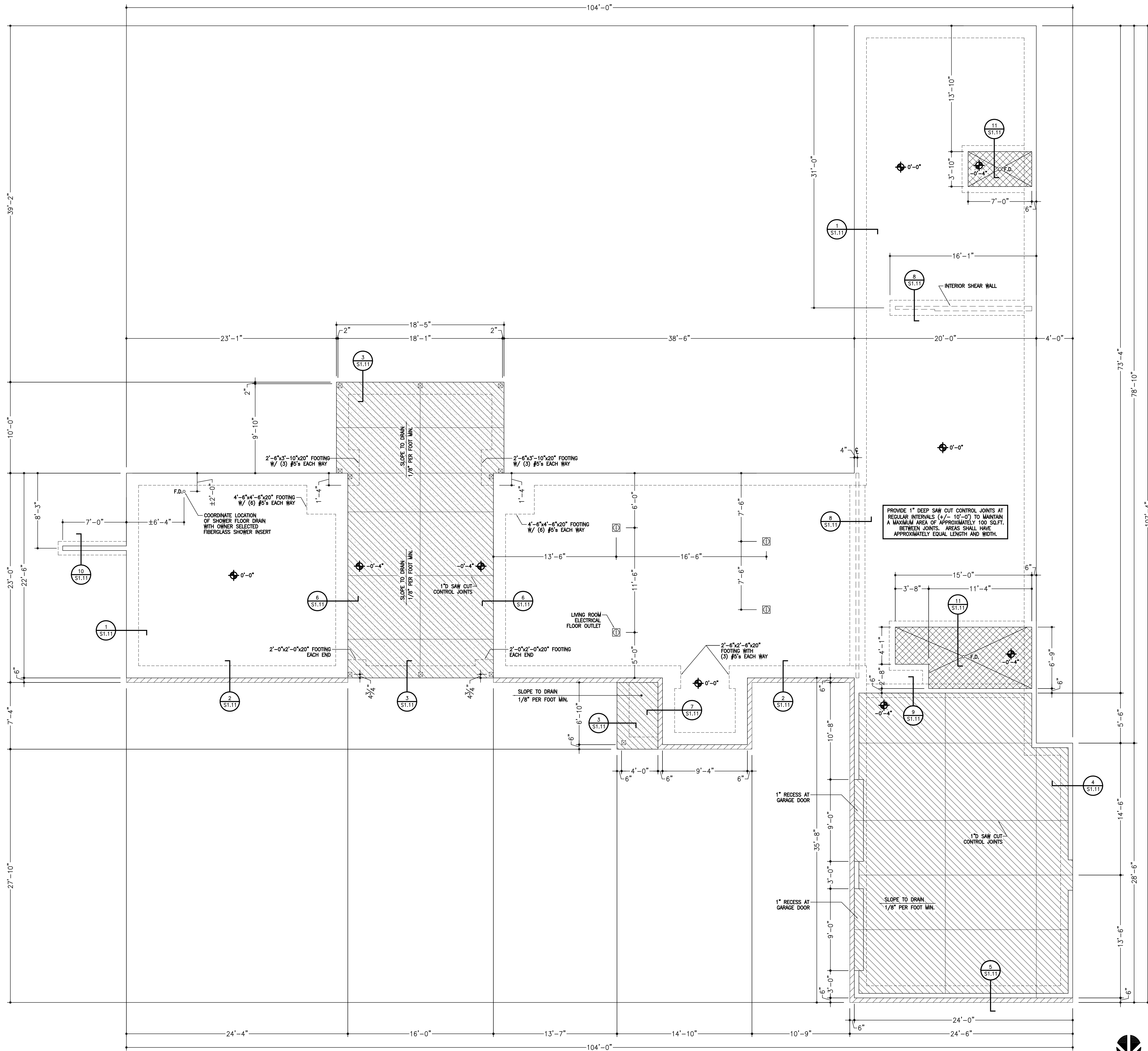
PROJECT NO.: 21020

DATE: JUNE 2, 2022

REVISION DATES:

FOUNDATION PLAN

S1.10



SLAB LEGEND

- F.D. FLOOR DRAIN
- BUILT-UP SLAB, SLOPE TO DRAIN
- EXTERIOR SLAB TO RECEIVE LIGHT BROOM FINISH
- RECESSED SLAB, SEE TAG FOR ELEVATION

NOTES:

- W.E.B. DESIGNS STRONGLY RECOMMENDS THAT A SOIL TEST BE PERFORMED ON SITE TO DETERMINE THE SOIL CLASSIFICATION, SOIL DENSITY, SOIL BEARING CAPACITY, AND THE DEPTH OF THE WATER TABLE AT THE LOCATION(S) OF CONSTRUCTION ACTIVITY. SOIL TEST, IF OBTAINED, SHALL BE PERFORMED BY A LICENSED GEO-TECHNICAL ENGINEER AND SHALL PROVIDE INSTRUCTIONS FOR SITE PREPARATION SHOULD SPECIAL PROCEDURES BE REQUIRED. SOIL TEST RESULTS AND RECOMMENDATIONS, IF OBTAINED, SHALL BE PROVIDED TO THE PROJECT ARCHITECT FOR REVIEW.
- W.E.B. DESIGNS RECOMMENDS THAT TESTS BE PERFORMED TO DETERMINE THE ELEVATION OF THE WATER TABLE ON SITE AND ANY UNDERGROUND WATER MOVEMENT AND THAT ALL NECESSARY PRECAUTIONS ARE TAKEN TO ELIMINATE THE POTENTIAL EXISTENCE OF HYDROSTATIC PRESSURE IF NECESSARY.
- SOIL SHALL BE COMPACTED TO A MINIMUM OF 95% OF MODIFIED PROCTOR AND TESTED IN LIFTS NOT TO EXCEED 12".
- ALL CONCRETE SLABS, SIDEWALKS, AND DRIVEWAYS SHALL RECEIVE 1" DEEP SAWCUT CONTROL JOINTS AT REGULAR INTERVALS TO MAINTAIN A MAXIMUM AREA OF 100 SQ.FT. BETWEEN JOINTS. AREAS SHALL HAVE APPROXIMATELY EQUAL LENGTH AND WIDTH WHEREVER POSSIBLE.

FOUNDATION PLAN

PROJECT
SCALE: 3/16"=1'-0"

ARCHITECT

WARREN E. BARRY

FLORIDA ARCHITECT, LICENSE #AR93495



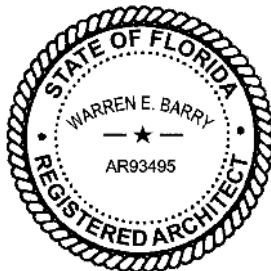
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CONSTRUCTION
DOCUMENTS FOR
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PROJECT NO.: 21020

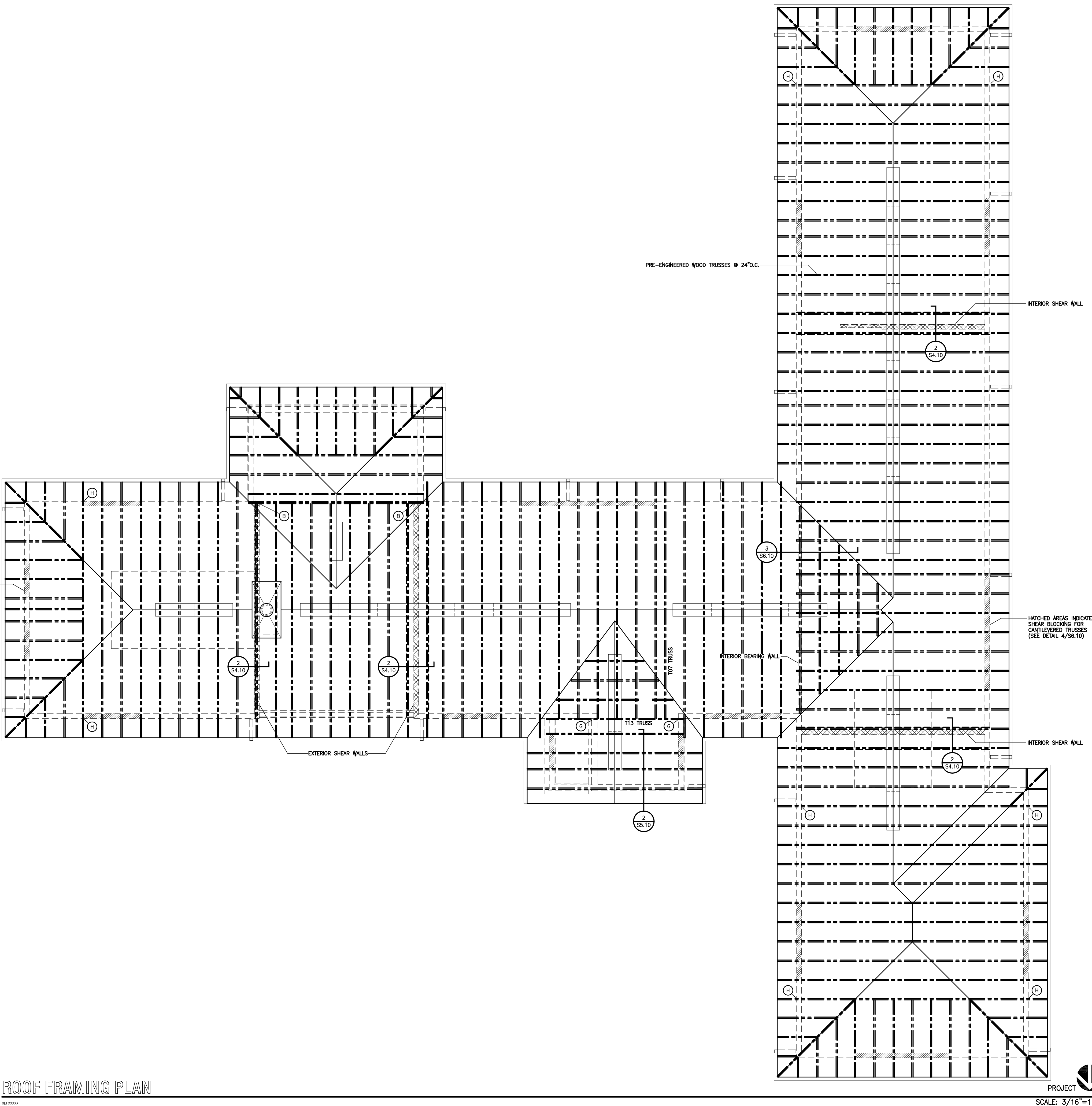
DATE: JUNE 2, 2022

REVISION DATES:

ROOF FRAMING PLAN,

S1.40

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TRUSS ANCHORS: (WELL HOUSE)

1) INSTALL "SIMPSON STRONG-TIE" CONNECTORS WITH "Z-MAX" COATING, NAILS SHALL FILL ALL NAIL HOLES.

(W) (2) "H2.5A" AT EACH TRUSS TO WALL CONNECTION THROUGHOUT WELL HOUSE U.N.O.

TRUSS ANCHORS: (RESIDENCE)

1) INSTALL "SIMPSON STRONG-TIE" CONNECTORS WITH "Z-MAX" COATING, NAILS SHALL FILL ALL NAIL HOLES.

(R) (1) "H10A" EACH TRUSS END TO WALL THROUGHOUT RESIDENCE UNLESS NOTED OTHERWISE

(G) "H10A" TRUSS TO INSIDE OF WALL + (2) "H2.5A" TRUSS TO OUTSIDE OF WALL

(H) "H14" HIP GIRDERS AND T13 TO WALL OVER (1) 2x6 COLUMN CENTERED BELOW TRUSS

(B) (1) "VGT" WITH (1) "HDU4" GIRDER TRUSS TO EXTERIOR WALL OVER (2) 2x6 BUILT-UP COLUMN CENTERED BELOW TRUSS WITH "HTTS-3/4" COLUMN TO FOUNDATION WITH 3/4" SIMPSON THREADED ROD WITH MIN. 12" EMBEDMENT INTO CONCRETE WITH "SIMPSON SET-3G" EPOXY

2) TRUSS TO TRUSS CONNECTORS SHALL BE SPECIFIED BY TRUSS MANUFACTURER.

STRUCTURAL NOTE:

1. SEE TRUSS PACKAGE BY BUILDERS FIRST SOURCE IN LAKE CITY, FLORIDA FOR ADDITIONAL INFORMATION.

TRUSS PACKAGE NOTES:

1. SOME GABLE WALLS ARE INSET PAST THE EXTERIOR WALLS, SEE ROOF PLAN FOR DIMENSIONS.

2. TOP OF GABLE END TRUSSES SHALL BE DROPPED TO ALLOW OVERHANG OUTLOOKS TO EXTEND TO FIRST INTERIOR TRUSS. SEE STRUCTURAL PACKAGE FOR DETAILING.

3) SEE PLANS FOR PORCH COLUMN AND BEAM INSET (INSET OF TRUSS BEARING) FROM OUTSIDE EDGE OF SLAB/FOUNDATION.

4) GABLE END TRUSSES SHALL PROVIDE OPENINGS AS REQUIRED FOR GABLE END WINDOWS WHEN SHOWN ON ELEVATIONS, SEE PLANS FOR LOCATIONS AND SIZING. OPENINGS IN TRUSSES SHALL BE OVERSIZED TO ALLOW FOR SITE FRAMING OF WINDOW ROUGH OPENING.

5) SEE WALL SECTION(S) FOR OVERHANG TYPE.

6) TRUSSES SHALL BE SPACED TO ALLOW FOR HVAC SUPPLY AND RETURN DUCTWORK TO PASS INTO ATTIC. SEE FLOOR PLAN FOR LOCATION(S).

ROOF FRAMING PLAN

DESKXXX



PROJECT
SCALE: 3/16" = 1'-0"

APPLICABLE STRUCTURAL CODES:

- 1) THE STRUCTURAL DESIGN, DETAILS, AND SPECIFICATIONS WITHIN THESE PLANS COMPLY WITH THE 2020 FLORIDA BUILDING CODE – RESIDENTIAL.
- 2) COMPLY WITH ONE OR MORE OF THE FOLLOWING CODES PER 2020 FBC R301.1.1, R301.2.1.1, AND R602.3:
- A) 2020 FLORIDA BUILDING CODE – BUILDING
- B) WFCM: WOOD FRAME CONSTRUCTION MANUAL FOR ONE AND TWO-FAMILY DWELLINGS
- C) ASCE-7: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
- D) ANSI AMERICAN WOOD COUNCIL NDS: NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
- 3) EXCEPTIONS PER 2020 FBC R301.2.1.1:
- A) FOUNDATIONS: SEE 2020 FBC RESIDENTIAL CHAPTER 4
- B) EXTERIOR DOORS AND WINDOWS: SEE 2020 FBC RESIDENTIAL R609
- C) ROOF SHEATHING: SEE 2020 FBC RESIDENTIAL R803
- D) CONCRETE: SEE 2020 FBC RESIDENTIAL R608.2
- 4) ADDITIONAL APPLICABLE CODES:
- A) ACI 318 PER 2020 FBC R608.5.1 (ACI 318 DEFERS TO ACI 332 FOR 1 AND 2 FAMILY DWELLINGS)
- B) TMS 402 - BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES PER 2020 FBC R606.1

GENERAL STRUCTURAL NOTES:

- 1) THE STRUCTURAL DESIGN, DETAILS, AND SPECIFICATIONS WITHIN THESE PLANS ARE APPLICABLE ONLY TO PROJECTS WITHIN A 140MPH (OR LESS) WIND SPEED ZONE, AND NOT LOCATED WITHIN WITHIN 1 MILE OF THE COAST. SEE F.B.C. FIGURE R301.2(4) FOR WIND SPEED MAP.
- 2) ROOF TRUSS LAYOUT, UPLIFT LOADS, AND GRAVITY LOADS RELIED UPON FOR DESIGN OF SUPPORTING CONSTRUCTION (WALLS, LINTELS, HEADERS, COLUMNS, FOOTINGS, ETC.) PREPARED BY:
- "BUILDERS FIRST SOURCE" IN LAKE CITY, FL**
- 3) THE ARCHITECT IS NOT RESPONSIBLE FOR CHANGES MADE TO THE STRUCTURAL CONTENT OF THESE DRAWINGS BY OTHERS WITHOUT WRITTEN CONSENT.
- 4) IT IS THE ARCHITECT'S UNDERSTANDING THAT THIS RESIDENCE IS NOT LOCATED WITHIN A 100 YEAR FLOOD PLAIN. THIS STRUCTURE IS NOT DESIGNED FOR HYDROSTATIC OR MOVING WATER LOADS. IF IT IS DISCOVERED THAT THIS RESIDENCE WILL BE CONSTRUCTED WITHIN A FLOOD ZONE OTHER THAN "X" OR "C", CONSULT ARCHITECT PRIOR TO CONSTRUCTION.
- 5) CONTRACTOR SHALL SCHEDULE CONSTRUCTION AND INSPECTIONS TO MINIMIZE THE EXPOSURE OF WOOD SHEATHING TO WEATHER/RAIN TO MINIMIZE THE POTENTIAL OF WATER DAMAGE.

PRE-ENGINEERED WOOD TRUSSES:

- 1) ARCHITECT IS NOT RESPONSIBLE FOR DESIGN AND DETAILING, OR INSTALLATION OF, PRE-ENGINEERED WOOD ROOF OR FLOOR TRUSSES. THESE ARE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND CONTRACTOR.
- 2) TRUSS ENGINEERING DRAWINGS SHALL BE PREPARED AND SIGNED/SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA.
- 3) TRUSSES SHALL BE ENGINEERED TO SUPPORT DEAD, LIVE, AND WIND LOADS PER FLORIDA BUILDING CODE, AND ASCE 7.
- 4) TRUSSES SHALL COMPLY WITH ANSI/TPI 1 "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION.
- 5) INSTALLATION OF TRUSSES SHALL COMPLY WITH BCSI "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING, AND BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- 6) TRUSSES SHALL COMPLY WITH TPI HIB "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING, AND BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- 7) INSTALLATION OF TRUSSES SHALL COMPLY WITH TPI DSB "RECOMMENDED DESIGN SPECIFICATIONS FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- 8) TRUSSES SHALL BE PERMANENTLY BRACED WITH LATERAL AND DIAGONAL BRACING AT LOCATIONS SPECIFIED ON TRUSS ENGINEERING DRAWINGS AND AS OUTLINED IN BCSI AND TPI REFERENCES ABOVE.
- 9) TRUSSES SHALL NOT BE CUT, NOTCHED, BORED, OR MODIFIED IN ANY WAY WITHOUT PRIOR WRITTEN APPROVAL FROM THE TRUSS MANUFACTURER.

METHODS OF CONSTRUCTION:

- 1) ARCHITECT HAS NO RESPONSIBILITY FOR CONSTRUCTION SAFETY, MEANS, METHODS, SCHEDULE, OR SEQUENCING OF CONSTRUCTION. THESE ARE THE RESPONSIBILITY OF THE CONTRACTOR. FINAL CONSTRUCTION SHALL COMPLY WITH CONSTRUCTION DOCUMENTS.
- 2) THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCES TO ENSURE SAFETY OF THE BUILDING, ITS COMPONENTS DURING ERECTION, AND ALL PERSONS ON SITE. THIS INCLUDES THE USE OF NECESSARY TEMPORARY SHORING AND BRACING, AND IMPLEMENTING COMPLIANCE WITH APPLICABLE SAFETY REGULATIONS.

DOORS, WINDOWS, SKYLIGHTS:

- 1) DESIGN: ARCHITECT IS NOT RESPONSIBLE FOR THE DESIGN OR CONSTRUCTION OF WINDOWS, DOORS, OR SKYLIGHTS. THE BUILDING ENVELOPE IS DESIGNED ASSUMING A FULLY ENCLOSED CONDITION, THEREFORE WINDOWS, DOORS, AND SKYLIGHTS MUST BE DESIGNED TO SUPPORT THE SAME WIND PRESSURES THAT WALLS AND ROOFS ARE DESIGNED FOR.
- 2) CERTIFICATION AND FL PRODUCT APPROVAL #'S: WINDOW, DOOR, AND SKYLIGHT MANUFACTURER SHALL SUBMIT FL PRODUCT APPROVAL NUMBER CERTIFICATION INDICATING THAT WINDOW OR DOOR UNITS CAN ADEQUATE SUPPORT DESIGN WIND PRESSURES FOR THE SPECIFIED WIND ZONE.
- 3) FASTENING: WINDOW, DOOR, AND SKYLIGHT MANUFACTURER SHALL PROVIDE FASTENING INFORMATION FOR ATTACHMENT TO SUPPORTING CONSTRUCTION. SEE MANUFACTURER'S FLORIDA PRODUCT APPROVAL INFORMATION.
- 4) PROTECTION: GLAZED OPENINGS IN "WIND BORN DEBRIS REGIONS" (140MPH OR GREATER) SHALL BE PROTECTED BY USE OF IMPACT RESISTANT GLAZING IN COMPLIANCE WITH 2020 FBC R301.2.1.2. ALTERNATIVE: WOOD STRUCTURAL PANELS COMPLYING WITH R301.2.1.2 EXCEPTION.

DESIGN LOADS AND DEFLECTION:

- 1) WIND LOADS (ASCE 7-16, F.B.C. BUILDING 1609, AND F.B.C. RESIDENTIAL CH3):
- A) ENCLOSURE CLASSIFICATION: ENCLOSED
- B) BASIC WIND SPEED: DESIGNED FOR 140 MPH (SEE FBC FIGURE 1609.3(1) FOR MAP)
- C) BUILDING RISK CATEGORY (TABLE 1604.5): II
- D) SURFACE ROUGHNESS (FBC R301.2.1.4.2): C (C=FLAT OPEN COUNTRY AND GRASSLANDS, NOT ADJACENT TO OPEN AREAS/WATER WITH 5,000ft OR MORE OF OPEN SPACE)
- E) EXPOSURE CATEGORY (FBC 301.2.1.4.3): C (C=FLAT OPEN COUNTRY AND GRASSLANDS, NOT ADJACENT TO OPEN AREAS/WATER WITH 5,000ft OR MORE OF OPEN SPACE)
- F) INTERNAL PRESSURE COEFFICIENTS: +/-0.18
- G) TOPOGRAPHIC WIND EFFECT NOT APPLICABLE UNLESS HILL IS OVER 30' TALL IN EXPOSURE C PER FBC R301.2.1.5(2).
- 2) DEAD LOADS (F.B.C. BUILDING 1606): SEE "MATERIAL DEAD LOADS" ON S3.10
- 3) UNIFORM LIVE LOADS (2020 FBC TABLE R301.5):
- A) ATTICS WITHOUT STORAGE: 10 PSF
- B) ATTICS WITH LIMITED STORAGE: 20 PSF
- C) HABITABLE ATTICS: 30 PSF
- D) STAIRS: 40 PSF AND 500lb POINT LOAD CONCENTRATED ON 4 SQUARE INCHES
- E) SLEEPING AREAS: 30 PSF
- F) ROOMS OTHER THAN SLEEPING: 40 PSF
- G) EXTERIOR BALCONIES AND DECKS: 40 PSF
- H) GARAGES: 50 PSF AND 2,000lb POINT LOAD CONCENTRATED ON 20 SQUARE INCHES
- 4) MIN. ROOF LIVE LOADS (Lr): 16lbs MIN. PER 2020 FBC TABLE R301.6
- 20lbs PER FBC TABLE 1607.1
- MODIFIED/REDUCED PER FBC 1607.12.2.1:
- $R_r = L_r \times R_f \times R_d$
- $R_r = 1$ (R_r REDUCTION NOT UTILIZED)
- 12:12 PITCH R_r = 0.6: 42.16 PSF
- 10:12 PITCH R_r = 0.7: 44.16 PSF
- 8:12 PITCH R_r = 0.8: 16 PSF
- 7:12 PITCH R_r = 0.85: 17 PSF
- 6:12 PITCH R_r = 0.9: 18 PSF
- 5:12 PITCH R_r = 0.95: 19 PSF
- 4:12 PITCH R_r = 1: 20 PSF

- 5) COMPONENTS AND CLADDING: DESIGN WIND PRESSURE FOR DOORS, WINDOWS, WALL AND ROOF COVERINGS, AND SKYLIGHTS
- ROOF WIND PRESSURE (140MPH, EXP-C):
- PER TABLE: +19.4/-67.8
- ADJUSTMENT FACTOR FOR EXP C: 1.4
- ADJUSTED:
- +19.4x1.4 = +27.16 lbs/sq.ft.
- 67.8x1.4 = -94.92 lbs/sq.ft.
- WALL WIND PRESSURE (140 MPH, EXP-C):
- PER TABLE: +21.2/-28.3
- ADJUSTMENT FACTOR FOR EXP C: 1.4
- ADJUSTED:
- +21.2x1.4 = +29.68 lbs/sq.ft.
- 28.3x1.4 = -39.62 lbs/sq.ft.

- OVERHANGS (140MPH, EXP-C): 122.35 lbs/sq.ft.
- SEE "WIND LOAD PRESSURE" DETAIL AND CHART ON S4.10 FOR ZONE SPECIFIC PRESSURES

- 6) GARAGE OVERHEAD DOOR WIND LOAD PRESSURE: SEE 2020 FBC TABLE R301.2(4)&(3)
- PER TABLE: +18.5/-20.9
- ADJUSTMENT FACTOR FOR EXP C: 1.4
- ADJUSTED:
- +18.5x1.4 = +25.90 lbs/sq.ft.
- 20.9x1.4 = -29.26 lbs/sq.ft.

- 7) ROOFS, FLOORS, AND WALLS HAVE BEEN DESIGNED USING PRESCRIPTIVE DESIGN AS PROVIDED WITHIN THE BUILDING CODES OR TO ALLOW FOR A MAXIMUM DEFLECTION OF L/360 (SEE 2020 FBC TABLE R301.7) WHILE ACCOUNTING FOR LOAD COMBINATIONS PER 2020 F.B.C. – BUILDING 1605. EXCEPTION: LINTELS SUPPORTING MASONRY VENEER WALLS = L/600

- 8) INTERIOR WALLS AND PARTITIONS (FBC 1607.14): SHALL RESIST A 5 lbs PER SQUARE FOOT LOAD

- 9) HANDRAILS & GUARDS (2020 FBC TABLE R301.5): SHALL RESIST 200 lbs POINT LOAD APPLIED IN ANY DIRECTION ALONG THE TOP

MATERIAL DEAD LOADS:

- 1) SOILS:
- A) SOIL/SAND-DRY: 100 lbs/cu.ft.
- B) SOIL/SAND-WET: 125 lbs/cu.ft.
- 2) CONCRETE AND CMU:
- A) CONCRETE: 150 lbs/cu.ft.
- B) 8"x16"x8" CONCRETE BLOCK: 39 lbs
- C) STEEL REINFORCEMENT
- #3: 0.376 lbs/ft
- #4: 0.668 lbs/ft
- #5: 1.043 lbs/ft
- #6: 1.502 lbs/ft
- 3) WOOD:
- A) WOOD FRAMING (SPF):
- a. 1x2 P.T.: 1.34 lbs./ft
- b. 2x4: 1.49 lbs./ft.
- c. 2x4 P.T.: 2.65 lbs./ft.
- d. 2x6: 2.34 lbs./ft.
- e. 2x6 P.T.: 4.17 lbs./ft.
- f. 2x8: 3.09 lbs./ft.
- g. 2x10: 3.95 lbs./ft.
- h. 2x12: 4.80 lbs./ft.
- i. 4x4: 3.49 lbs./ft.
- j. 4x4 P.T.: 6.21 lbs./ft.
- k. 6x6: 8.61 lbs./ft.
- l. 6x6 P.T.: 15.33 lbs/ft.
- m. 11.25"x1.75" LVL: 5.6 lbs/ft.
- n. 14"x1.75" LVL: 7 lbs/ft.
- o. 16"x1.75" LVL: 8 lbs/ft.
- p. 18"x1.75" LVL: 9 lbs/ft.
- B) ROOF/WALL SHEATHING:
- m. 1/2" PLYWOOD: 1.5 lbs/sq.ft.
- n. 5/8" PLYWOOD: 1.9 lbs/sq.ft.
- o. 3/4" PLYWOOD: 2.3 lbs./sq.ft.
- p. 7/16" OSB: 1.68 lbs./sq.ft.
- q. 19/32" OSB: 2 lbs./sq.ft.
- C) PRE-ENGINEERED ROOF TRUSSES (1 PLY):
- a. TYPICAL: 7 lbs/ft.
- b. GABLE END TRUSS: 9 lbs/ft.
- c. ATTIC STORAGE TRUSS: 9 lbs/ft.
- D) PRE-ENGINEERED FLOOR TRUSSES:
- a. 24"D OPEN WEB (1 PLY): 7.5 lbs/ft.

- 4) EXTERIOR FINISHES:
- A) HOUSEWRAP: 0.06 lbs./sq.ft.
- B) FIBER CEMENT
- a. F.C. LAP SIDING: 5 lbs/sq.ft.
- b. F.C. PANEL: 2.4 lbs./sq.ft.
- c. F.C. SHINGLE SIDING: 2 lbs/sq.ft.
- d. F.C. 3.5" TRIM: 5.04 lbs/sq.ft.
- C) CULTURED STONE: UP TO 15 lbs/sq.ft.
- D) BRICK (REAL): 39 lbs/sq.ft.
- E) SYNTHETIC STUCCO: 3 lbs/sq.ft.
- F) 3 COAT STUCCO: 9 lbs/sq.ft.
- 5) INTERIOR FINISHES:
- A) 1/2" GWB: 1.6 lbs/sq.ft.
- B) 5/8" GWB: 2.2 lbs/sq.ft.
- C) PLASTER, 1" = 8 lbs/sq.ft.
- D) PORCELAIN WALL TILE: 10 lbs./sq.ft.
- E) CEMENT TILE BACKER: 2.6 lbs./sq.ft.

- 6) INSULATION:
- A) BATT INSULATION: 0.4 lbs/sq.ft.
- B) BLOWN-IN INSULATION: 2.35 lbs/sq.ft.
- C) SPRAY FOAM INSULATION: 0.5 lbs/cu.ft.

- 7) ROOF/CEILING:
- A) ROOFING UNDERLAYMENT:
- a. 15# FELT: 0.2 lbs./sq.ft.
- b. 30# FELT: 0.3 lbs./sq.ft.
- c. SYNTHETIC UNDERLAYMENT: 0.4 lbs/sq.ft.
- d. SELF-ADHERING: 3 lbs/sq.ft.
- B) ASPHALT SHINGLES:
- a. 3 TAB: 2 lbs/sq.ft.
- b. ARCHITECTURAL: 3.05 lbs/sq.ft.
- c. LUXURY: 4.25 lbs/sq.ft.
- C) CLAY TILE WITH MORTAR: 19 lbs./sq.ft.
- D) METAL ROOF: 1.5 lbs/sq.ft.
- E) WATER: 62.4 lbs/cubic ft (8.4 lbs/gallon)
- D) ICE: 57.3 lbs/cubic ft

- 8) TYPICAL ASSEMBLIES:
- A) ROOF WITH ARCH. SHINGLES = 21 lbs/sf.ft.
- B) 2x6x8" WALL W/ LAP SIDING = 114 lbs/ft.
- C) 2x6x10" WALL W/ LAP SIDING = 141 lbs/ft.
- D) 8"x8" CMU WITH STUCCO = 449 lbs/ft.
- E) 8"x10" CMU WITH STUCCO = 562 lbs/ft.

GEOTECHNICAL / BEARING SOIL:

- 1) A GEOTECHNICAL REPORT (STRUCTURAL EVALUATION OF SOIL) HAS NOT BEEN PERFORMED FOR THIS SITE. IT IS STRONGLY RECOMMENDED THAT THE OWNER OR CONTRACTOR EMPLOY THE SERVICES OF A GEOTECHNICAL ENGINEER TO PERFORM SOIL BORINGS, EVALUATE BORING DATA, CONFIRM THE SOIL TYPE ASSUMED IN THIS SPECIFICATION, AND PROVIDE RECOMMENDATIONS FOR PREPARATION OF THE SOILS SPECIFIC TO THIS BUILDING SITE. CONSULT GEOTECHNICAL ENGINEER FOR RECOMMENDATIONS ON TYPE, QUANTITY, AND DEPTH OF BORINGS. THE ARCHITECT HAS NO KNOWLEDGE OF THE ON-SITE SOILS, AND THEREFORE ACCEPTS NO RESPONSIBILITY FOR THEIR BEARING CAPACITY OR PERFORMANCE.
- 2) BEARING SOIL IS PRESUMED TO BE SANDY SOIL WITH NO ORGANICS, PEAT, CLAY, EXPANSIVE CLAYS, OR BOULDERS.
- A) IN COMPLIANCE WITH 2020 FBC TABLE R401.4.1, FOUNDATIONS BEARING ON SANDY SOIL ARE DESIGNED FOR AN ALLOWABLE VERTICAL SOIL BEARING PRESSURE OF 2,000 PSF.
- 3) IT IS ASSUMED THAT THE SEASONAL HIGH GROUNDWATER TABLE IS DEEP ENOUGH ON SITE TO BE INCONSEQUENTIAL AND IRRELEVANT TO THE DESIGN OF THE BUILDING AND ITS FOUNDATIONS.
- 4) IF THE CONTRACTOR, OWNER, OR BUILDING INSPECTOR ENCOUNTER ORGANICS, CLAYS, SILTS, BOULDERS, OR HIGH GROUNDWATER LEVELS DURING FOUNDATION EXCAVATION, A GEOTECHNICAL ENGINEER SHALL BE CONTACTED AND EMPLOYED TO ASSESS CONDITIONS FIRST HAND AND GIVE DIRECTION FOR ADDITIONAL CORRECTIVE WORK OR MODIFICATIONS TO THE DESIGN THAT MAY NEED TO BE PERFORMED.

SITework:

- 1) STRIP ALL TREES, ROOTS, STUMPS, GRASSES, TOPSOIL, AND OTHER ORGANICS FROM THE BUILDING FOOTPRINT, AND WITHIN A MINIMUM OF 10'-0" OF THE BUILDING FOOTPRINT. USE ROOT RAKE OR SIMILAR EQUIPMENT.
- 2) PROOFCOMPACT EXISTING GRADE WITH A LOADED DUMP TRUCK OR COMPACTOR TO DENSITY SOILS AND IDENTIFY SOFT AREAS. IF SOFT SOILS ARE ENCOUNTERED, OVERCUT UNSUITABLE MATERIAL AND REPLACE WITH COMPACTED BACKFILL IN LIFTS AS SPECIFIED BELOW.
- 3) EXCAVATIONS SHALL BE PERFORMED IN ACCORDANCE WITH OSHA STANDARDS. CONTRACTOR IS SOLELY RESPONSIBLE FOR EXCAVATION SAFETY.
- 4) COMPACT ALL EXCAVATION BOTTOMS TO FIRM UNYIELDING CONDITIONS. FOUNDATIONS TO BEAR ON COMPACTED SANDY SOIL OR COMPACTED FILL AS DESCRIBED BELOW.
- 5) EXCAVATION AND BACKFILL OPERATIONS ARE TO BE MAINTAINED IN A DRY CONDITION.
- 6) SLOPE OR CROWN BUILDING SUBGRADES, FILL, AND SURROUNDING ON-SITE GRADE TO PROMOTE RUN-OFF AND PREVENT PONDING AND EROSION. WATER SHALL NOT DRAIN AGAINST THE STRUCTURE OR ITS FOUNDATIONS. FINAL GRADE SHALL SLOPE AWAY FROM THE BUILDING TO PROMOTE DRAINAGE.
- 7) SURFACE AND INFILTRATING WATER ARE TO BE REMOVED BY GRADING AND PUMPING FROM SUMPS IF REQUIRED. SEE "GEOTECHNICAL/BEARING SOIL" NOTES.
- 8) BACKFILL SHALL BE ONLY CLEAN, WELL GRADED SAND, FREE OF VEGETATION AND FOREIGN MATERIAL, WITH NO MORE THAN 10% PASSING #200 SIEVE FOR BACKFILL WITHIN THE BUILDING FOOTPRINT.
- 9) MECHANICALLY COMPACT ALL BACKFILL WITHIN BUILDING FOOTPRINT IN MAXIMUM 12" LOOSE LIFTS TO FIRM UNYIELDING CONSISTENCY. COMPACTION OF FILL AND BEARING SOIL UNDER ALL FOUNDATIONS SHALL BE TO 95% OF MAXIMUM DRY DENSITY PER MODIFIED PROCTOR TEST, ASTM D-1557.
- 10) TREAT ALL SLAB SUBGRADES FOR TERMITES IN ACCORDANCE WITH THE FLORIDA BUILDING CODE AND LOCAL ORDINANCES AND REGULATIONS.
- 11) EXTERIOR GRADE (AFTER PLACEMENT OF SOD, LANDSCAPING MULCH/ROCKS, ETC.) SHALL BE 6" MIN. BELOW WOOD SIDING AND FOAM INSULATION.

CONCRETE:

- 1) COMPLY WITH 2020 FBC R402.2 AND ACI 332-14 CODE REQUIREMENTS FOR RESIDENTIAL CONCRETE.
- 2) CONCRETE (ACI 332 4.1): CEMENT: ASTM C150, TYPE I PORTLAND CEMENT AGGREGATE: ASTM C33, MAX. SIZE = 1" WATER: ASTM C1602, CLEAN AND POTABLE WATER/CEMENT RATIO: 0.5 MAX. SLUMP: 4" (±1") AIR ENTRAINING (FBC TABLE R402.2): CONCRETE EXPOSED TO WEATHER: ASTM C260, MILD EXPOSURE 5 TO 6% COMPRESSIVE STRENGTH (ACI 322 TABLE 5.3.2): MIN. AT 28 DAYS PER ASTM C39 TEST FOOTINGS, SLAB ON GRADE: 3,000psi
- 3) REINFORCING (ACI 332 4.2.1.1): ASTM A615, GRADE 40 (40,000 psi)
- CONCRETE – LAPS, BENDS, HOOKS
- 2020 FBC TABLE R608.5.4(1) AND FIGURE R608.5.4(3)
- | BAR SIZE | LAP LENGTH | BEND DIAMETER | HOOK LENGTH |
|----------|------------|---------------|-------------|
| #3 | 16" | 2 1/4" | 6" |
| #4 | 20" | 3" | 8" |
| #5 | 25" | 3 3/4" | 10" |
| #6 | 30" | 4 1/2" | 12" |
- BAR COVER (2020 FBC R404.1.3.3.7.4 AND ACI 332 5.6)
- | EXPOSURE CONDITION | MIN. COVER |
|--|------------|
| CAST AGAINST AND EXPOSED TO EARTH | 3" |
| EXPOSED TO EARTH OR WEATHER #5 AND SMALLER | 1 1/2" |
| #6 AND LARGER | 2" |
| NOT EXPOSED TO WEATHER OR EARTH SLABS, WALLS, JOISTS | 3/4" |
| BEAMS, COLUMNS (STIRRUPS, TIES) | 1 1/2" |
| MAX. TOLERANCE | ±3/8" |
- 4) SLABS-ON-GRADE:
- A) THICKNESS: 4" UNLESS NOTED OTHERWISE
- B) VAPOR RETARDER: 6 MIL. POLYETHYLENE, LAP EDGES 6"
- C) WIRE REINFORCING (REQUIRED): WELDED WIRE REINFORCING (WWR) ASTM A185, 6x6-w1.4xw1.4 (6x6-10/10) LAP EDGES MIN. 10" SUPPORT ON CHAIRS AT 36"O.C. EACH WAY
- D) MICROFIBER REINFORCING (OPTIONAL ADDITIVE): ASTM C 1116, FIBERESH POLYPROPYLENE FIBERS BY SI CONCRETE SYSTEMS OR EQUAL ADD 1.5 POUNDS PER CUBIC YARD
- E) PROTECTION: CURE ALL SLABS FOR 7 DAYS USING SPRAYED-ON CURING COMPOUND OR CONTINUOUS WATER SPRINKLING. CURING COMPOUND SHALL BE COMPATIBLE WITH FLOORING ADHESIVES.
- F) SLAB JOINTS: AS CONCRETE SLABS CURE AND DRY OUT, THEY WILL SHRINK, CAUSING CRACKS TO FORM IN SURFACE OF SLAB. SLAB REINFORCEMENT IS PLACED IN SLAB TO HELP LIMIT WIDTH OF CRACKS THAT DO FORM. ALL SLABS SHOULD BE SAW CUT IN 10'-0"x10'-0" MAX. SQUARES.
- G) TO PREVENT RUST STAINS, RECYCLED CONCRETE SHALL NOT BE USED.
- 5) CONSTRUCTION JOINTS (2020 FBC R404.1.3.3.7.8): #4 REINFORCEMENT AT 24"O.C. WITH 12" EMBEDMENT EACH SIDE
- 6) TEMPERATURE PROTECTION:
- a) INSTITUTE HOT WEATHER PROTECTION PROCEDURES WHEN AIR TEMPERATURES EXCEEDS 90° F.
- b) INSTITUTE COLD WEATHER PROJECTION PROCEDURES WHEN AIR TEMPERATURES ARE BELOW 40° F.
- 7) CONCRETE ANCHORS: SEE "WOOD FRAMING" NOTES ON S3.10.

WOOD FRAMING:

- 1) COMPLY WITH 2020 FBC RESIDENTIAL, WFCM, AND FBC CHAPTER 23.
- 2) STUDS:
- | WALL HEIGHT | MEMBER SIZE | STUD SPACING | LUMBER GRADE | WOOD SPECIES |
|--------------|-------------|--------------|-----------------|-----------------------|
| UP TO 10'-0" | 2x4 OR 2x6 | 16" O.C. | NO. 2 OR BETTER | SPRUCE-PINE-FIR (SPF) |
- 3) HEADERS, JOISTS, BEAMS, POSTS/COLUMNS: NO. 2 SOUTHERN PINE (SP)
- 4) LAMINATED VENEER LUMBER (LVL): ASTM D5456 PER NDS 8.1.1 F_y=2,850psi MIN. (FOR 12"DEPTH), F_v=285psi, E=2,000,000psi F_{em}=2,510psi MIN., F_{em}=750psi MIN.,
- 5) FASCIA: NO. 2. SPRUCE-PINE-FIR (SPF)
- 6) TREATMENT: PROVIDE PRESERVATIVE TREATED WOOD FOR ALL LOCATION WHERE WOOD IS IN CONTACT WITH SOIL, CONCRETE, OR MASONRY, OR IS PERMANENTLY EXPOSED TO EXTERIOR WEATHER. DO NOT USE PRESERVATIVES CONTAINING CCA PRODUCTS.
- 7) SHEATHING:
- A) ROOF SHEATHING: APA RATED MIN. SPECIFIC GRAVITY: 0.49 DOC SP1 OR SP2 COMPLIANT SHEATHING GRADE EXPOSURE 1 THICKNESS, TYPE, AND FASTENERS PER TYPICAL ROOF SHEATHING DETAIL
- B) WALL SHEATHING: APA RATED MIN. SPECIFIC GRAVITY: 0.49 DOC SP1 OR SP2 COMPLIANT SHEATHING GRADE EXPOSURE 1 THICKNESS, TYPE, AND FASTENERS PER TYPICAL EXTERIOR WALL DETAIL
- 8) FASTENERS:
- A) NAILS:
- 1) COMPLY WITH 2020 FBC 2303.6 AND TABLE 2304.10.1 "FASTENING SCHEDULE" AND WFCM TABLE 3.1 "NAILING SCHEDULE" UNLESS NOTED OTHERWISE.
- 2) SHALL COMPLY ASTM F1667 PER NDS 12.1.6.1.
- B) CONCRETE ANCHORS:
- 1) L-BOLTS: ASTM F1554 OR ASTM A307 GRADE 36, 1 EXTENSION SHALL BE MIN. 3x DIAMETER
- 2) EPOXY SET, "SIMPSON SET 3G" EPOXY ADHESIVE WITH GRADE 36 "SIMPSON THREADED ROD". FOLLOW MANUFACTURER'S INSTALLATION INSTRUCTIONS EXACTLY. SHALL NOT BE INSTALLED PRIOR TO CONCRETE CURING TIME OF 21 DAYS PER ACI 318.
- 3) SCREWS: "SIMPSON TITAN HD", FOLLOW MANUFACTURER'S INSTALLATION INSTRUCTIONS EXACTLY. SHALL NOT BE INSTALLED PRIOR TO CONCRETE CURING TIME OF 21 DAYS PER ACI 318.
- 4) PER ACI 318 17.1.3, THE REMOVAL AND RESETING OF POST-INSTALLED MECHANICAL ANCHORS IS PROHIBITED.
- 5) DUST AND WATER SHALL BE BLOWN OUT OF ANCHOR HOLES PRIOR TO ANCHOR OR EPOXY INSTALLATION PER "SIMPSON" INSTALLATION INSTRUCTIONS.
- C) BOLTS: ASTM A307, HOT-DIP GALVANIZED SEE PLAN FOR SIZE AND QUANTITY.
- D) UPLIFT ANCHORS & TIES: USE "SIMPSON STRONG-TIE", ZMAX NAILS SHALL FILL ALL NAIL HOLES
- E) CORROSION PROTECTION: ALL FASTENERS AND CONNECTORS EXPOSED TO WEATHER OR IN CONTACT WITH PRESERVATIVE TREATED WOOD (AND AS RECOMMENDED BY MANUFACTURER) ARE TO HAVE MIN. G185 HOT-DIP GALVANIZED COATING PROTECTION.
- 9) NOTCHING AND BORING NOTES
- A) COLUMNS, HEADERS, BEAMS, GIRDERS, AND TRUSSES SHALL NOT BE NOTCHED OR BORED.
- B) WALL STUDS
- 1) STUDS SHALL NOT BE NOTCHED
- 2) BORED HOLES IN STUDS (WFCM 3.4.1.1.1, 3.4.2.1.1, AND 3.4.3.1.1):
- a) EXTERIOR AND LOAD BEARING WALLS: BORED HOLES SHALL NOT EXCEED 40% OF THE ACTUAL STUD DEPTH (2X4=1.25" MAX, 2X6=2" MAX) AND SHALL NOT BE WITHIN 5/8" OF THE EDGE OF THE STUD.
- b) INTERIOR NON-BEARING WALLS: BORED HOLES SHALL NOT EXCEED 60% OF THE ACTUAL STUD DEPTH (2X4=2" MAX, 2X6=3.25") AND SHALL NOT BE WITHIN 5/8" OF THE EDGE OF THE STUD.
- c) BORED HOLES CONTAINING UTILITIES (ELECTRICAL, PLUMBING, ETC.) SHALL HAVE NAILING PLATES INSTALLED ON THE FACE OF STUD TO PROTECT THE APPLICABLE UTILITY FROM NAIL PENETRATION.

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CONSTRUCTION
DOCUMENTS FOR
CONTRACTOR'S
STANDARD
CONSTRUCTION

PROJECT NO.: 21020

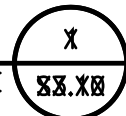
DATE: JUNE 2, 2022

REVISION DATES:

STRUCTURAL SPECIFICATIONS

S3.10

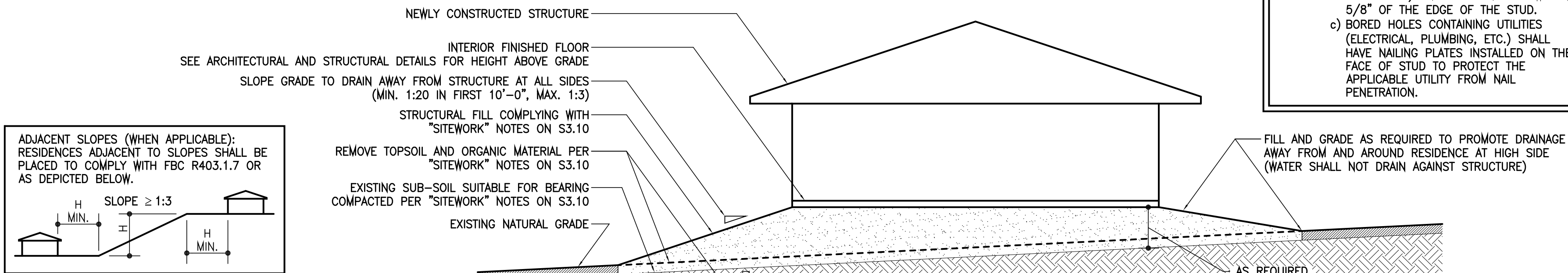
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TYPICAL GRADING DETAIL

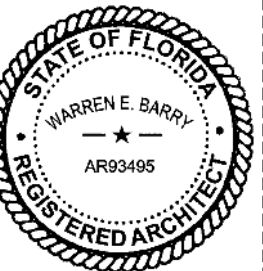
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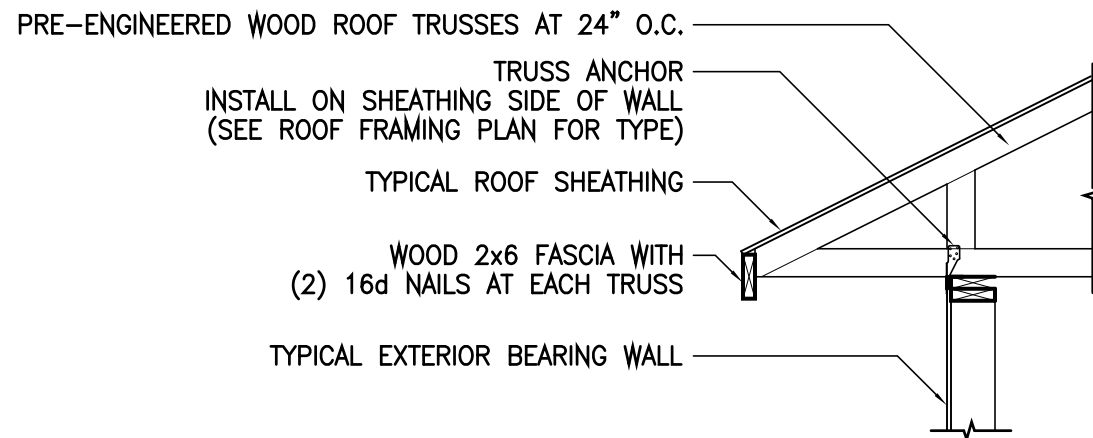
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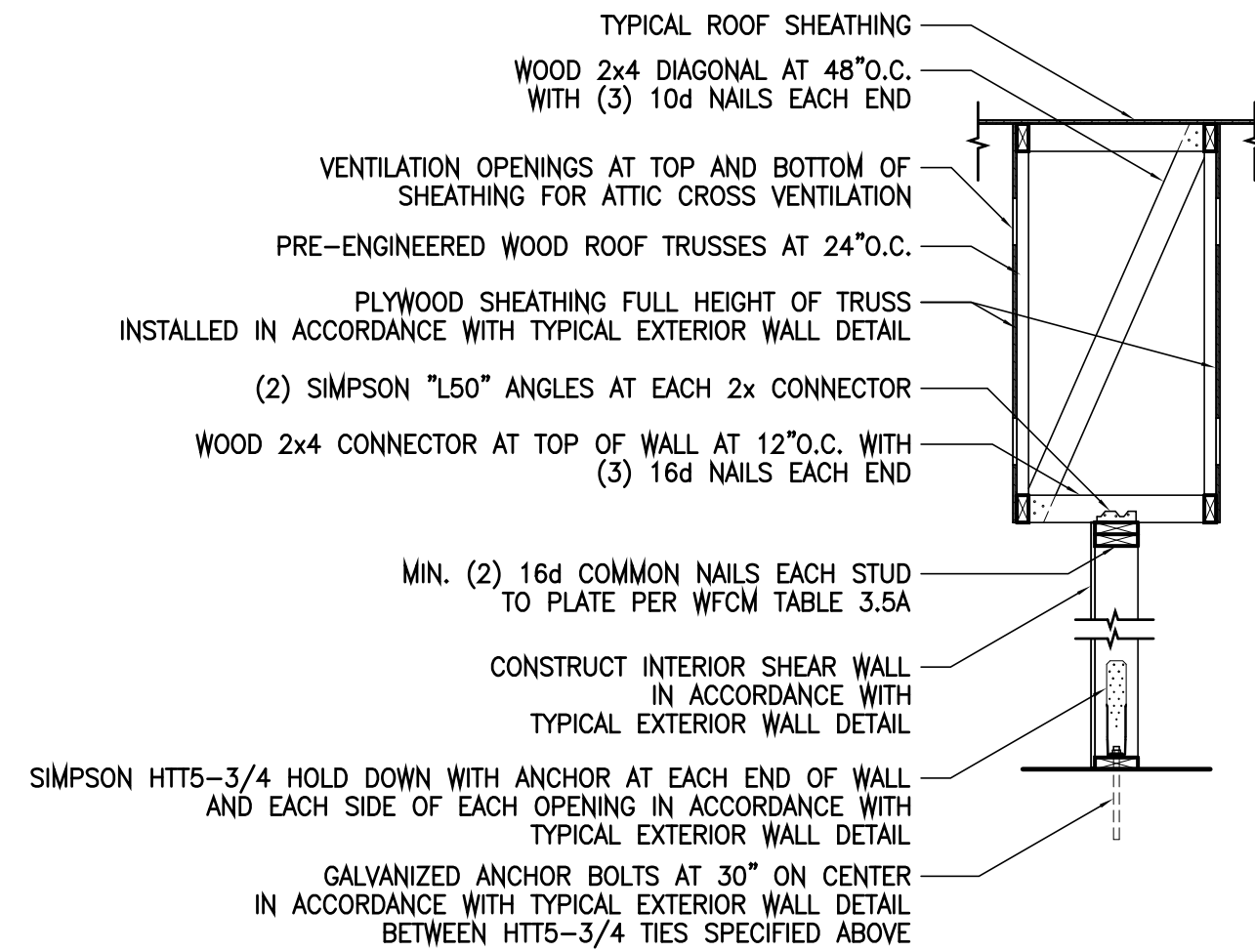
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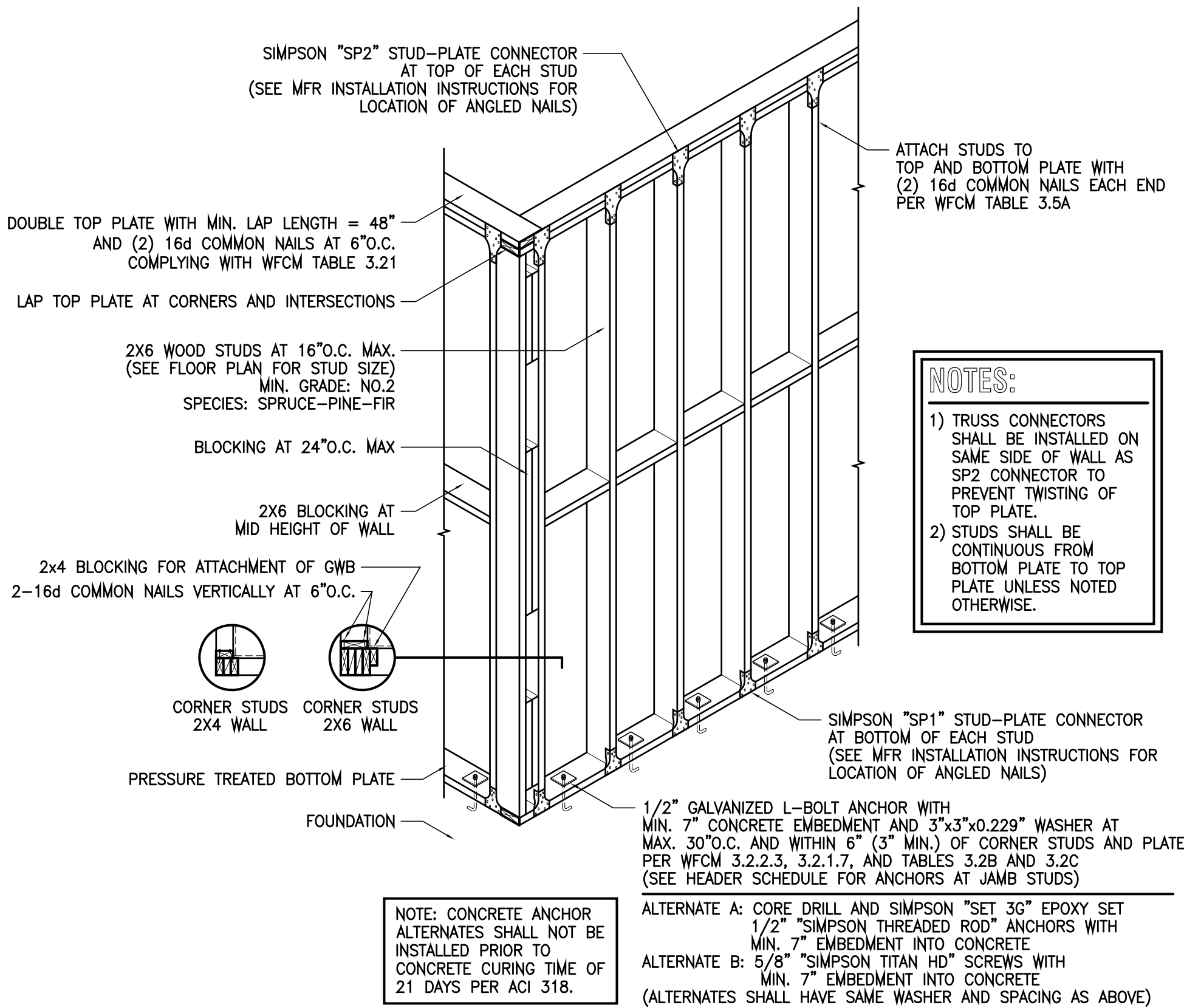
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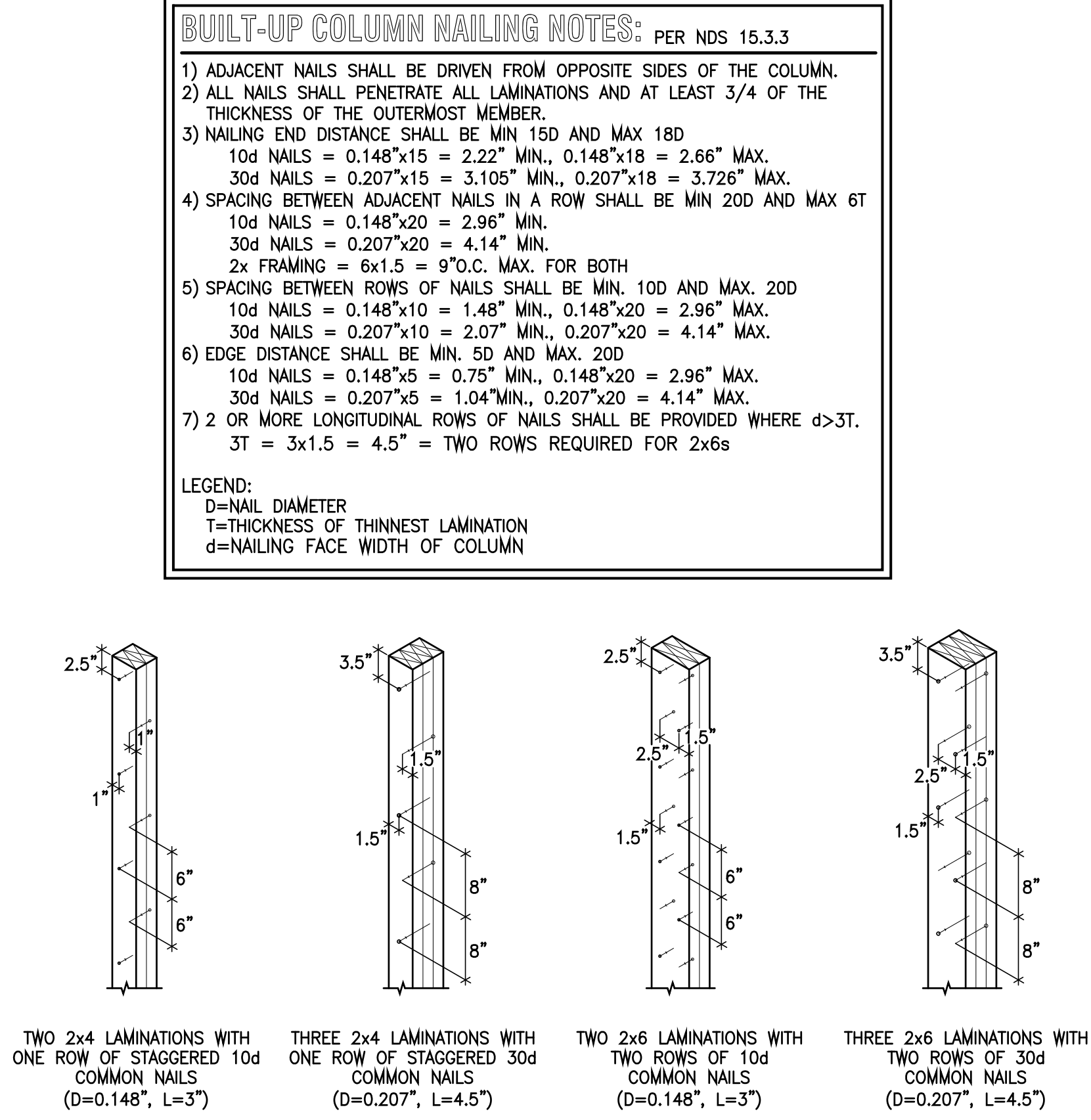
TRUSS BEARING DETAIL (WOOD FRAMED EXTERIOR WALL) 1
D-STRUCTURAL SCALE: NOT TO SCALE S4.10



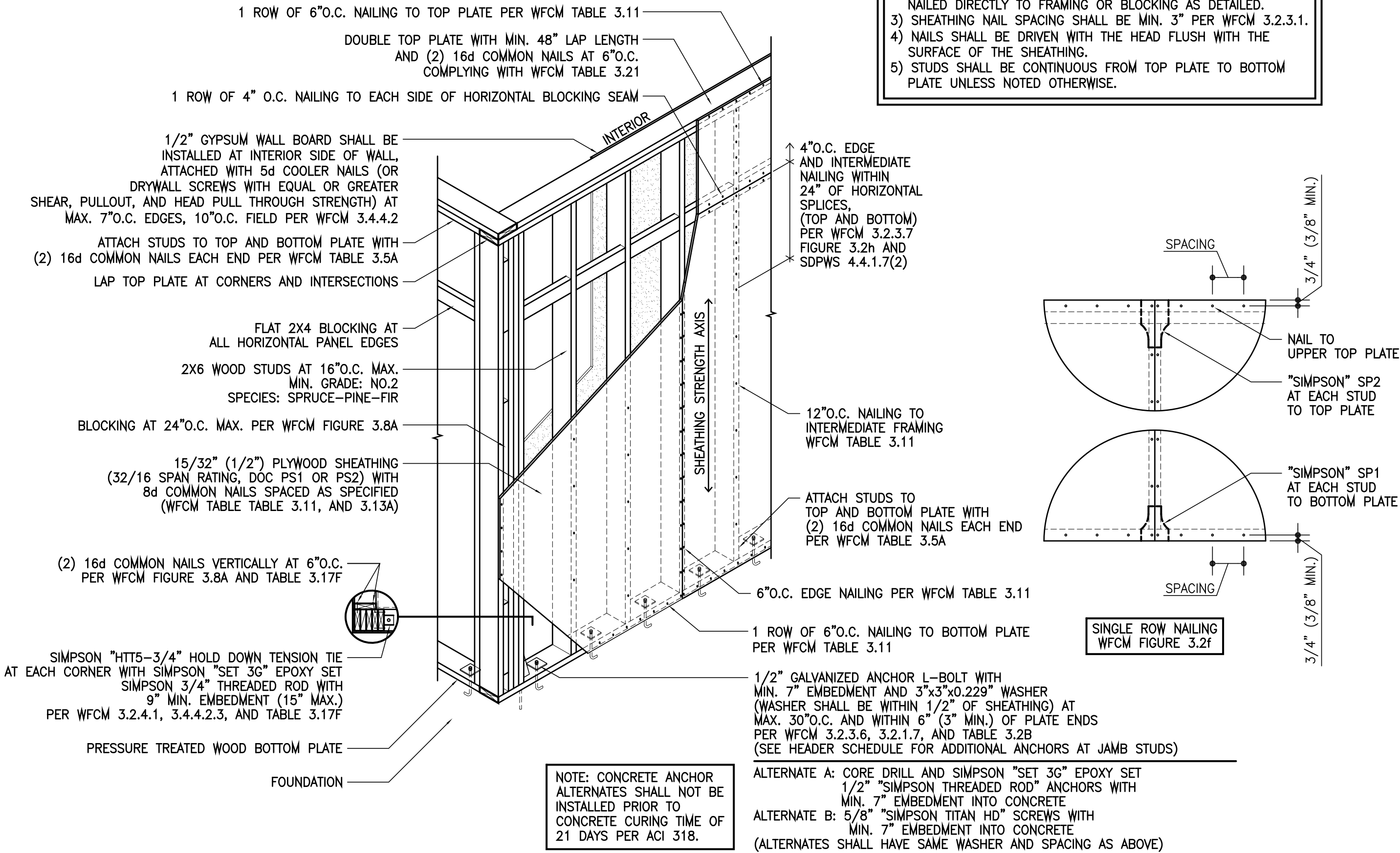
INTERIOR SHEAR WALL DETAIL 2
D-STRUCTURAL SCALE: NOT TO SCALE S4.10



TYPICAL INTERIOR BEARING WALL DETAIL (MAX. 20'-0" TRIBUTARY SPAN LOADING) 3
D-STRUCTURAL SCALE: NOT TO SCALE S4.10



TYPICAL BUILT-UP COLUMN NAILING DETAIL 4
D-STRUCTURAL SCALE: NOT TO SCALE S4.10



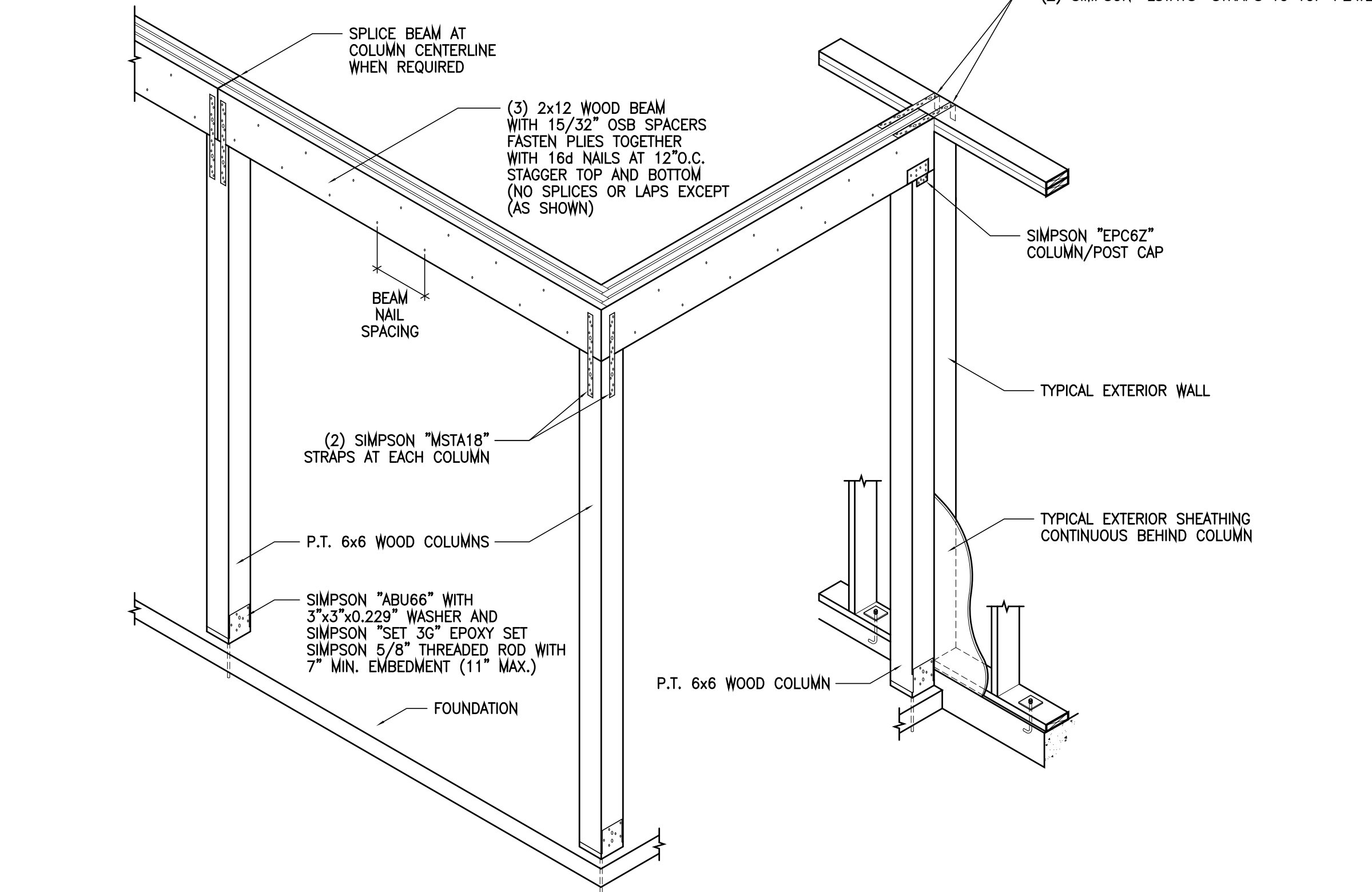
TYPICAL EXTERIOR WALL DETAIL AND INTERIOR SHEAR WALLS WHEN UTILIZED (SHEATHING RESISTING SHEAR ONLY, CONNECTIONS RESISTING UPLIFT)

D-STRUCTURAL

SCALE: NOT TO SCALE

1

S5.10



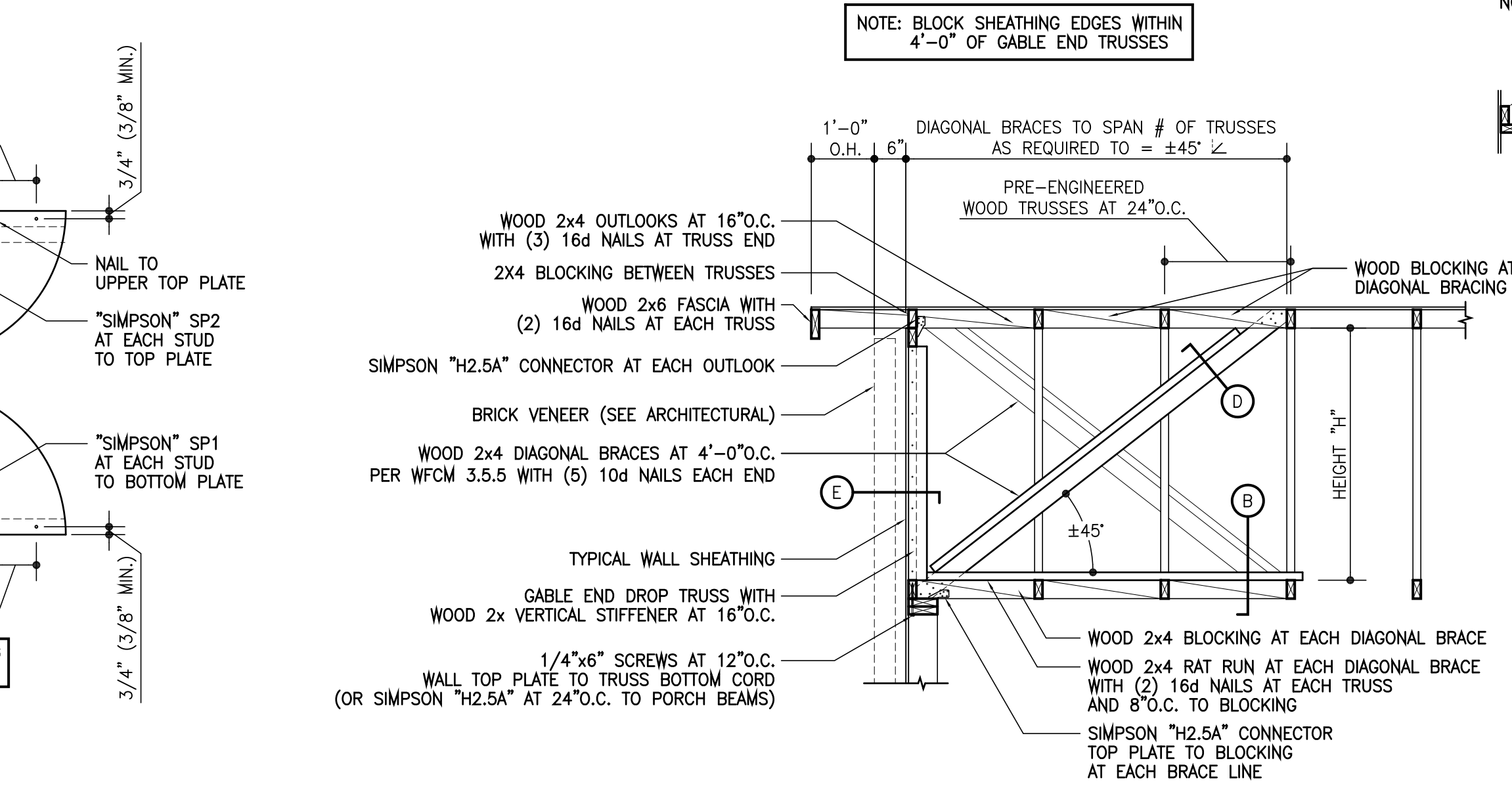
TYPICAL COLUMN AND BEAM DETAIL (WOOD FRAMED EXTERIOR WALL)

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SCALE: NOT TO SCALE

3

S5.10



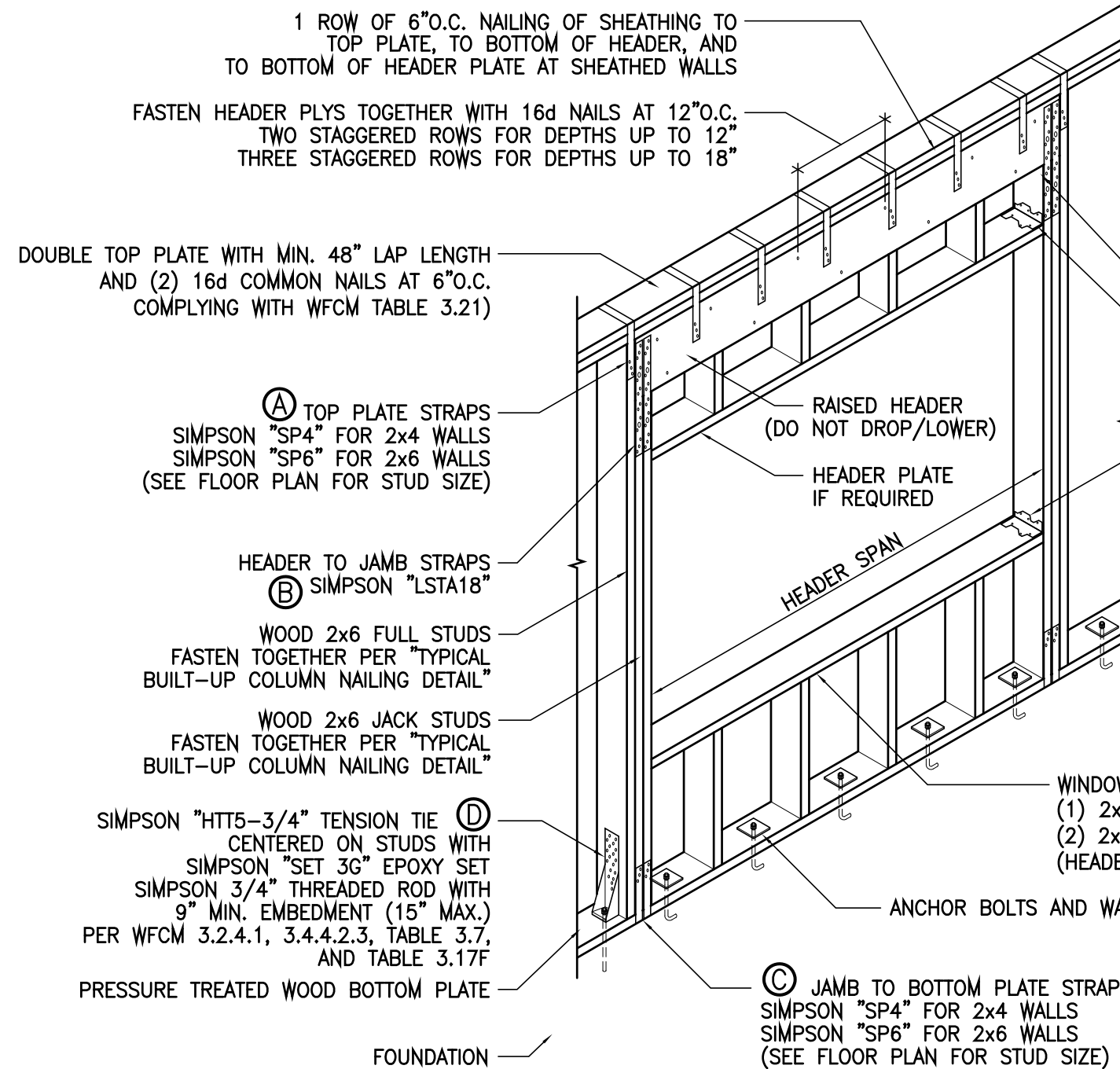
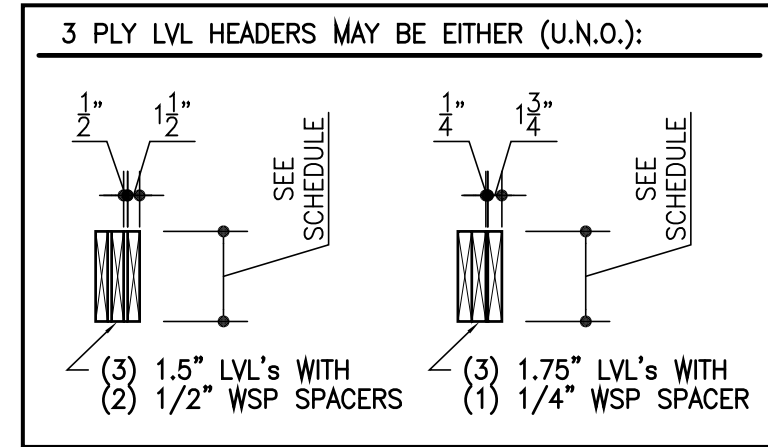
TYPICAL GABLE END BRACING DETAIL (WOOD FRAMED EXTERIOR WALL)

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S5.10



TYPICAL DOOR/WINDOW HEADER DETAIL

D-STRUCTURAL

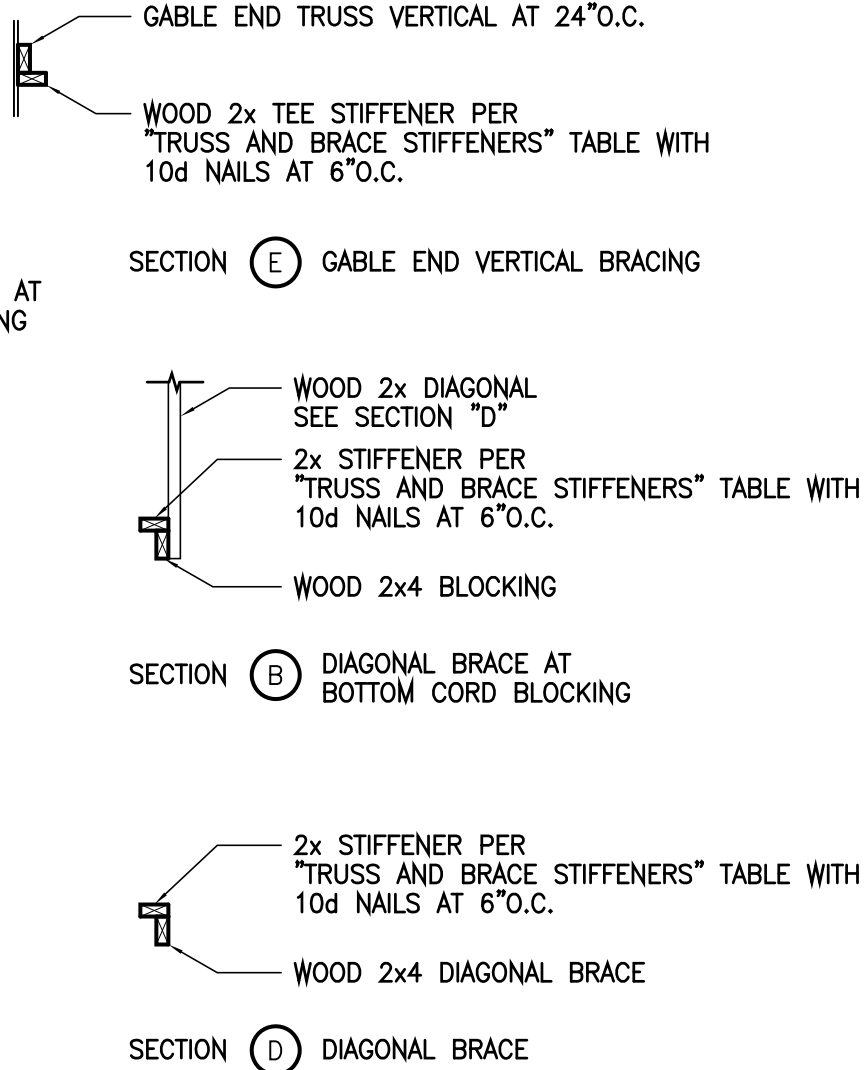
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TRUSS AND BRACE STIFFENERS	
MAX. HEIGHT "H" OR LENGTH	STIFFER REQUIRED
4'-0" OR LESS	NONE
10'-0" OR LESS	2x4 TEE
16'-0" OR LESS	2x6 TEE

NOTE: TRUSS MANUFACTURER INSTALLATION INSTRUCTIONS, IF MORE STRINGENT, SHALL SUPERSEDE



HEADER SCHEDULE								
MAX. HEADER SPAN	HEADER LOCATION	WOOD HEADER	STRAP Ⓐ SPACING	QUANTITY REQUIRED AT EACH END				
				JACK STUDS	FULL STUDS	STRAP Ⓑ	STRAP Ⓒ	ANCHOR Ⓓ
4'-0"	NO G.T.*	(3) 2x8's	NOTE 5)	2	2	1	1	1
	HIP G.T.**	(3) 2x10's	NOTE 5)	2	2	1	1	1
6'-0"	NO G.T.*	(3) 2x12's	NOTE 5)	2	3	1	1	1
	HIP G.T.**	(3) 11.25'LVL's	NOTE 5)	3	3	1	1	1
8'-0"	NO G.T.*	(3) 11.25'LVL's	NOTE 5)	3	3	2	2	1
	HIP G.T.**	(3) 11.25'LVL's	NOTE 5)	3	3	2	2	1
10'-0"	NO G.T.*	(3) 11.25'LVL's	NOTE 5)	3	4	2	2	1
	HIP G.T.**	(3) 11.25'LVL's	NOTE 5)	3	4	2	2	1

- 1) NUMBER OF JACK AND FULL STUDS PER WFCM TABLES 3.23C AND 3.22F AND CALCULATIONS.
- 2) NO SPLICES OR LAPS PERMITTED IN HEADERS.
- 3) FULL STUDS SHALL BE CONTINUOUS FROM BOTTOM PLATE TO TOP PLATE.
- 4) JACK STUDS SHALL BE CONTINUOUS FROM BOTTOM PLATE TO BOTTOM OF HEADER.
- 5) INSTALL (1) STRAP (A) AT EACH END OF HEADER AND MAX. 18" O.C. THROUGHOUT SPAN
- 6) *NO G.T. = HEADER MAY NOT HAVE A GIRDER TRUSS OVER OPENING OR OVER JACK STUDS
- 7) **HIP G.T. = HEADERS WITH A HIP ROOF GIRDER TRUSS BEARING OVER OPENING, OR OVER JACK STUDS

GENERAL HEADER NOTES:

- 1) ALL CONNECTORS ARE BY SIMPSON STRONG-TIE.
- 2) PROVIDE 1/2" OSB SPACER BETWEEN HEADER PLYS FOR DIMENSIONAL LUMBER.
- 3) DIMENSIONAL LUMBER HEADERS: GRADE=NO.2, SPECIES=SOUTHERN PINE (SP).
- 4) ENGINEERED LUMBER (LVL): $F_b=2,850$ psi, $F_v=285$ psi, $E=2,000,000$ psi, $F_{tH}=2,510$, $F_{cH}=750$.

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CONSTRUCTION DOCUMENTS FOR CONTRACTOR'S STANDARD CONSTRUCTION

PROJECT NO.: 21020

DATE: JUNE 2, 2022

REVISION DATES:

STRUCTURAL DETAILS

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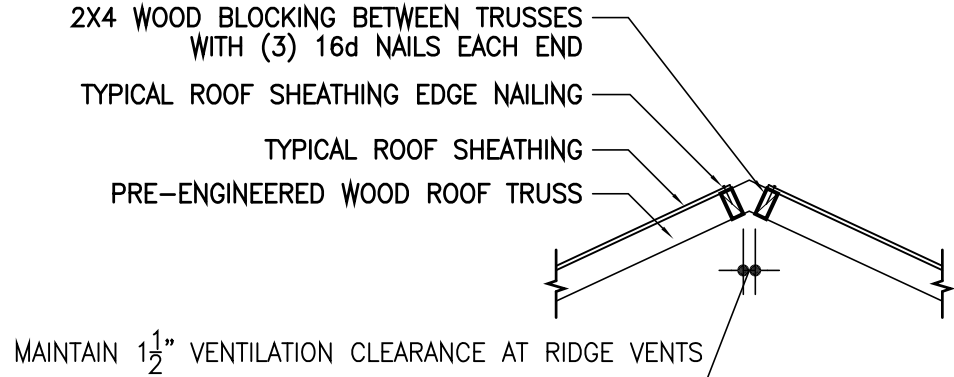
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FBC-R WIND PRESSURE SCHEDULE FOR COMPONENTS AND CLADDING - 140 MPH, EXPOSURE C

AREA (SQ.FT.)	ROOF ZONES												WALL ZONES					
	ZONES 1		ZONES 2e		ZONES 2n – GABLE ONLY		ZONES 2r		ZONES 3 AND 3e		ZONE 3r – GABLE ONLY		ZONE 4		ZONE 5			
	+	–	+	–	+	–	+	–	+	–	+	–	+	–	+	–		
	(PSF) ↓	(PSF) ↑	(PSF) ↓	(PSF) ↑	(PSF) ↓	(PSF) ↑	(PSF) ↓	(PSF) ↑	(PSF) ↓	(PSF) ↑	(PSF) ↓	(PSF) ↑	(PSF) ↓	(PSF) ↑	(PSF) ↓	(PSF) ↑		
< 10	+19.4x1.4=+27.16	–39.1x1.4=–54.74	+19.4x1.4=+27.16	–49.8x1.4=–69.72	+19.4x1.4=+27.16	–57.0x1.4=–79.80	+19.4x1.4=+27.16	–57.0x1.4=–79.80	+19.4x1.4=+27.16	–57.0x1.4=–79.80	+19.4x1.4=+27.16	–67.8x1.4=–94.92	+21.2x1.4=+29.68	–22.9x1.4=–32.06	+21.2x1.4=+29.68	–28.3x1.4=–39.62		
10 ≤ A ≤ 20	+17.2x1.4=+24.08	–39.1x1.4=–54.74	+17.2x1.4=+24.08	–44.8x1.4=–62.72	+17.2x1.4=+24.08	–49.2x1.4=–68.88	+17.2x1.4=+24.08	–49.2x1.4=–68.88	+17.2x1.4=+24.08	–49.2x1.4=–68.88	+17.2x1.4=+24.08	–58.1x1.4=–81.34	+20.2x1.4=+28.28	–22.0x1.4=–30.80	+20.2x1.4=+28.28	–26.4x1.4=–36.96		
20 ≤ A ≤ 50	+14.3x1.4=+20.02	–27.4x1.4=–38.36	+14.3x1.4=+20.02	–38.2x1.4=–53.48	+14.3x1.4=+20.02	–38.8x1.4=–54.32	+14.3x1.4=+20.02	–38.8x1.4=–54.32	+14.3x1.4=+20.02	–38.8x1.4=–54.32	+14.3x1.4=+20.02	–45.2x1.4=–63.28	+19.0x1.4=+26.60	–20.7x1.4=–28.98	+19.0x1.4=+26.60	–23.9x1.4=–33.46		
50 ≤ A ≤ 100	+12.2x1.4=+17.08	–22.1x1.4=–30.94	+12.2x1.4=+17.08	–33.2x1.4=–46.48	+12.2x1.4=+17.08	–31.0x1.4=–43.40	+12.2x1.4=+17.08	–31.0x1.4=–43.40	+12.2x1.4=+17.08	–31.0x1.4=–43.40	+12.2x1.4=+17.08	–35.5x1.4=–49.70	+18.0x1.4=+25.20	–19.8x1.4=–27.72	+18.0x1.4=+25.20	–22.0x1.4=–30.80		
100 ≤ A ≤ 500	SEE ABOVE	SEE ABOVE	SEE ABOVE	SEE ABOVE	SEE ABOVE	SEE ABOVE	SEE ABOVE	SEE ABOVE	SEE ABOVE	SEE ABOVE	SEE ABOVE	SEE ABOVE	+15.8x1.4=+22.12	–17.6x1.4=–24.64	+15.8x1.4=+22.12	–17.6x1.4=–24.64		

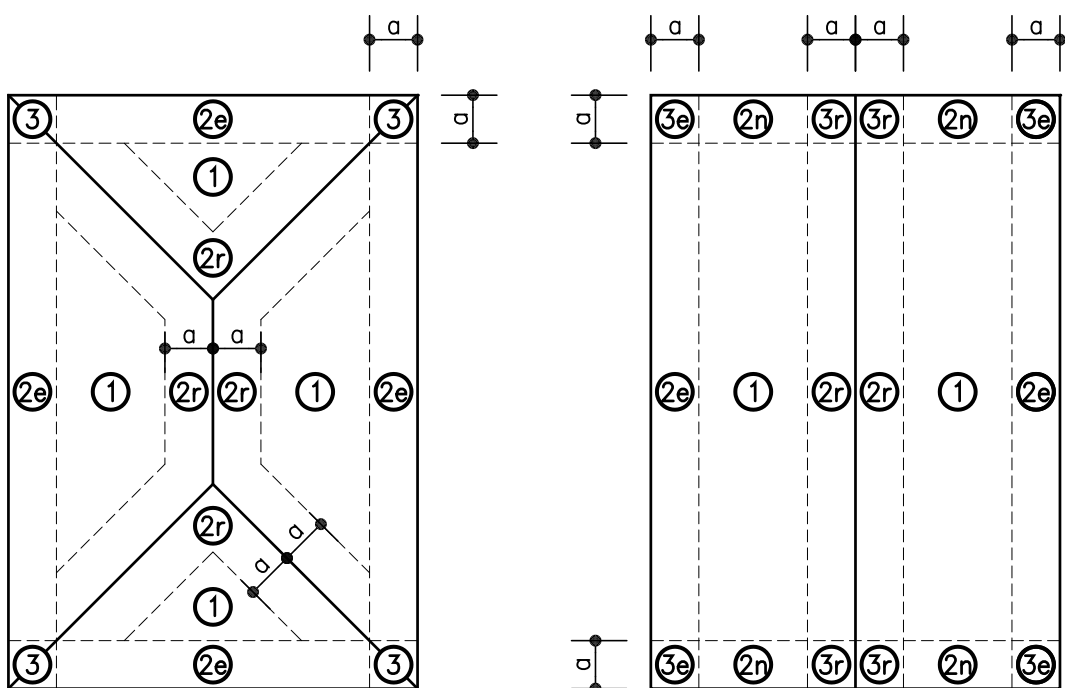


TYPICAL VENTED RIDGE DETAIL

D-STRUCTURAL

SCALE: NOT TO SCALE

1
S6.10



HIP ROOF
ROOF SLOPE OF 7° TO 45°
(3:12=14°, 12:12=45°)

GABLE ROOF
ROOF SLOPE OF 7° TO 45°
(3:12=14°, 12:12=45°)

WIND LOAD PRESSURE - COMPONENT AND CLADDING PRESSURE ZONES

D-STRUCTURAL

SCALE: NOT TO SCALE

2
S6.10

OVERHANG UPLIFT: 122.35 lbs./SQ.FT ↑

- PER CALCULATIONS FROM ASCE 7-16 CHAPTER 30 PART 1, USING HIGHEST PRESSURES FROM HIP AND GABLE ROOF TYPES FOR ROOF SLOPES OF 3:12 TO 12:12.
- VALUE IS FOR EXPOSURE CATEGORY C, WITH MAX. MEAN ROOF HEIGHT OF 30'-0".
- VALUE HAS ALREADY BEEN MULTIPLIED BY 0.6 TO CONVERT PRESSURES TO ASD (APPLIED STRENGTH DESIGN).

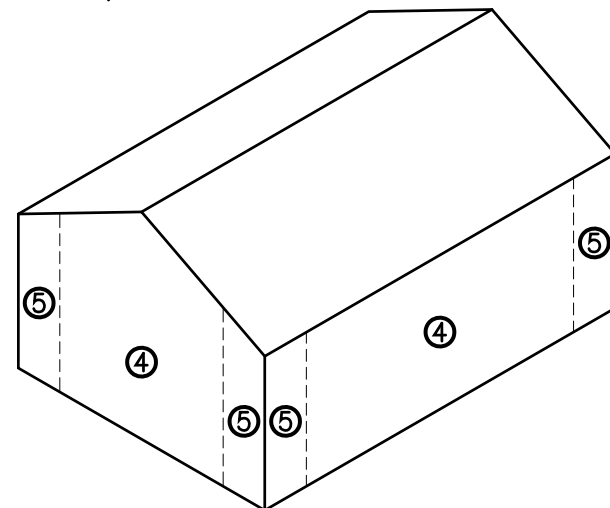
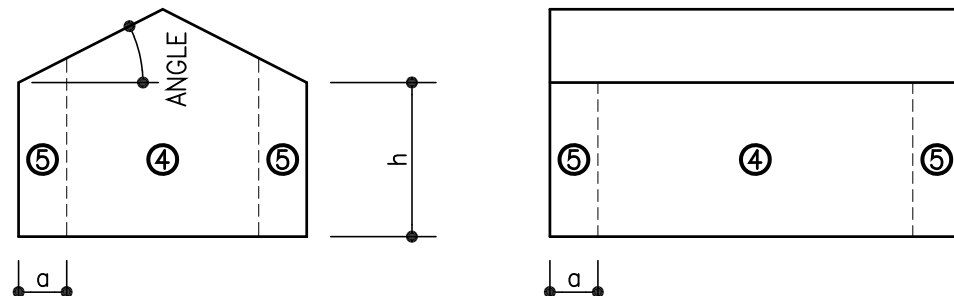


TABLE NOTES:

- TABLE BASE VALUES ARE FROM 2020 FBC TABLE R301.2(2), USING HIGHEST PRESSURES FROM HIP AND GABLE ROOF TYPES FOR ROOF SLOPES OF 3:12 TO 12:12. VALUES ALSO COMPLY WITH ASCE 7-16 CH30 PART 1.
- TABLE BASE VALUES ARE FOR EXPOSURE CATEGORY B. VALUES ARE MULTIPLIED BY AN ADJUSTMENT COEFFICIENT OF 1.4 TO GIVE EXPOSURE CATEGORY C VALUES PER 2020 FBC TABLE R301.2(3).
- FBC HAS MULTIPLIED TABLE VALUES BY 0.6 TO CONVERT PRESSURES TO ASD (APPLIED STRENGTH DESIGN).
- WIND PRESSURE ZONE FIGURES ARE PER 2020 FBC FIGURE R301.2(7).
- TABLE VALUES ALSO COMPLY WITH ASCE 7-16 CHAPTER 30.



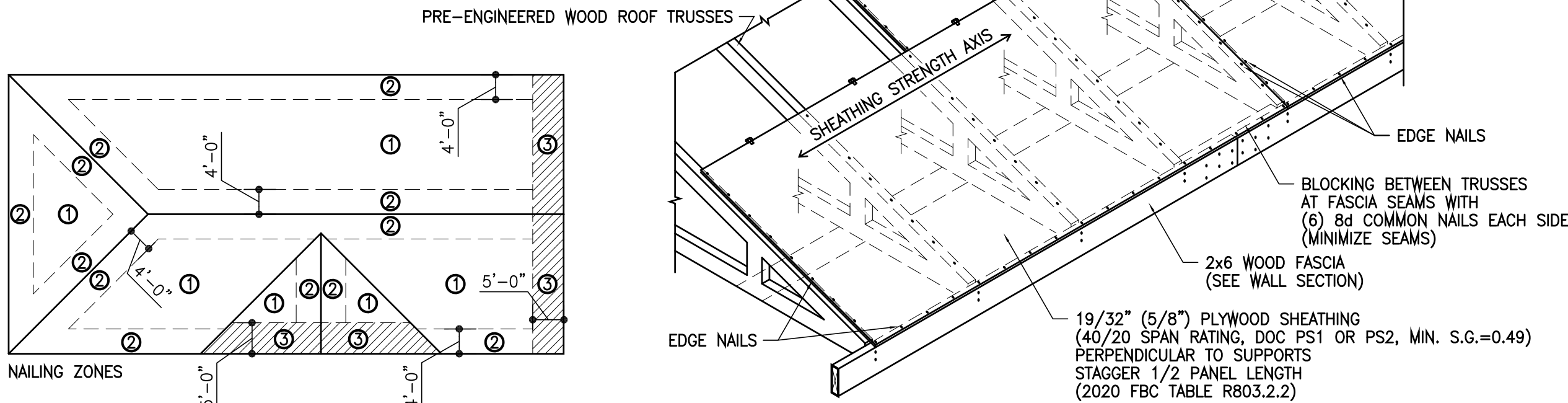
a = 4'-0" IN ALL CASES

h = EAVE HEIGHT SHALL BE USED FOR ROOF SLOPES OF 10° AND GREATER (2.125:12).

ROOF NAILING SCHEDULE		
NAILING ZONE	MAX. NAIL SPACING	
	UP TO 140 MPH	
	EDGE	FIELD
ZONE 1	6"	6"
ZONE 2	6"	6"
ZONE 3	6"	6"

ROOF SHEATHING NOTES:

- SHEATHING SHALL BE MIN. 4'x8' EXCEPT AT BOUNDARIES AND CHANGES IN DIRECTION, MINIMIZE CUTS.
- MIN. SHEATHING DIMENSION ON CUT PANELS SHALL BE 24" UNLESS ALL EDGES OF ARE SUPPORTED AND FASTENED TO FRAMING OR BLOCKING.
- SUPPORTED ROOF SHEATHING EDGES SHALL BE SUPPORTED BY AND NAILED DIRECTLY TO FRAMING OR BLOCKING AS DETAILED.
- MIN. NAIL SPACING SHALL BE 3".
- NAILS SHALL BE DRIVEN WITH THE HEAD FLUSH WITH THE SURFACE OF THE SHEATHING.



TYPICAL ROOF SHEATHING DETAIL

D-STRUCTURAL

SCALE: NOT TO SCALE

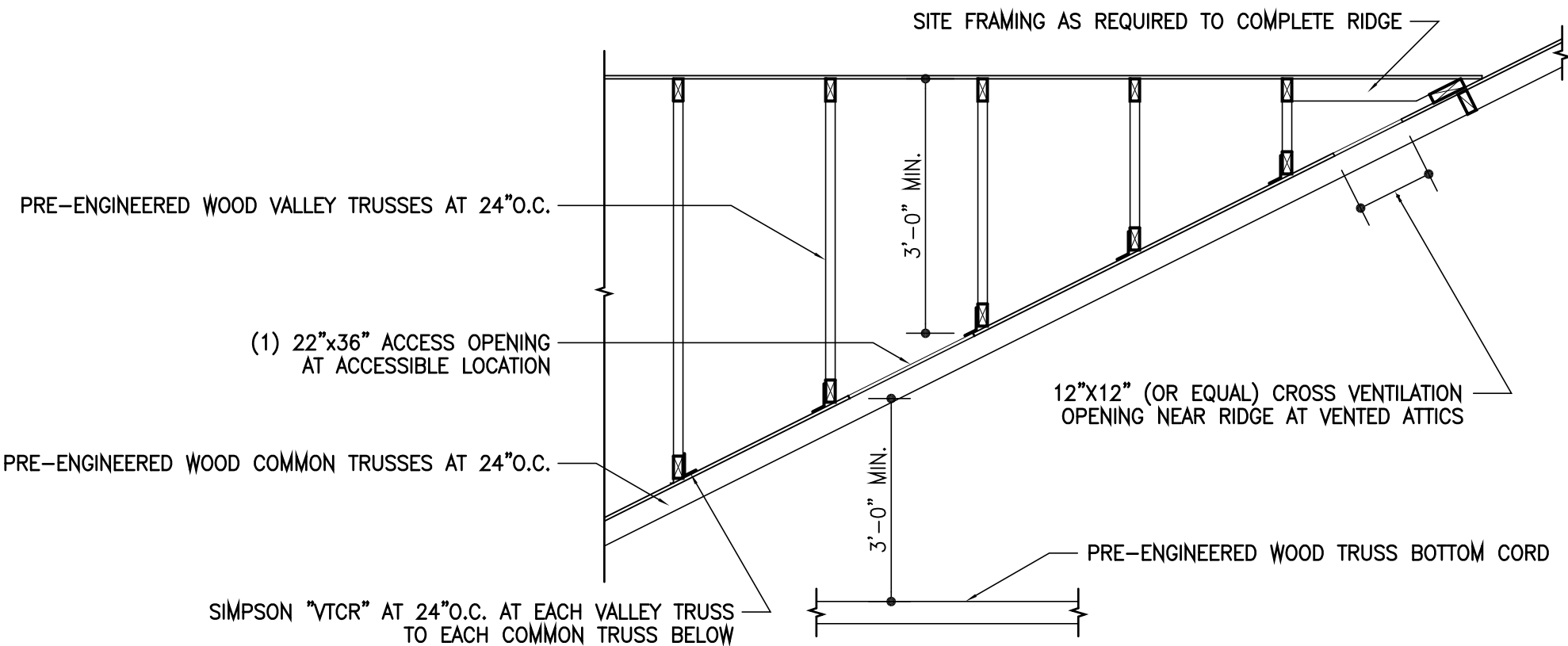
3
S6.10

SHEAR BLOCKING DETAIL FOR CANTILEVERED TRUSSES

D-STRUCTURAL

SCALE: NOT TO SCALE

4
S6.10

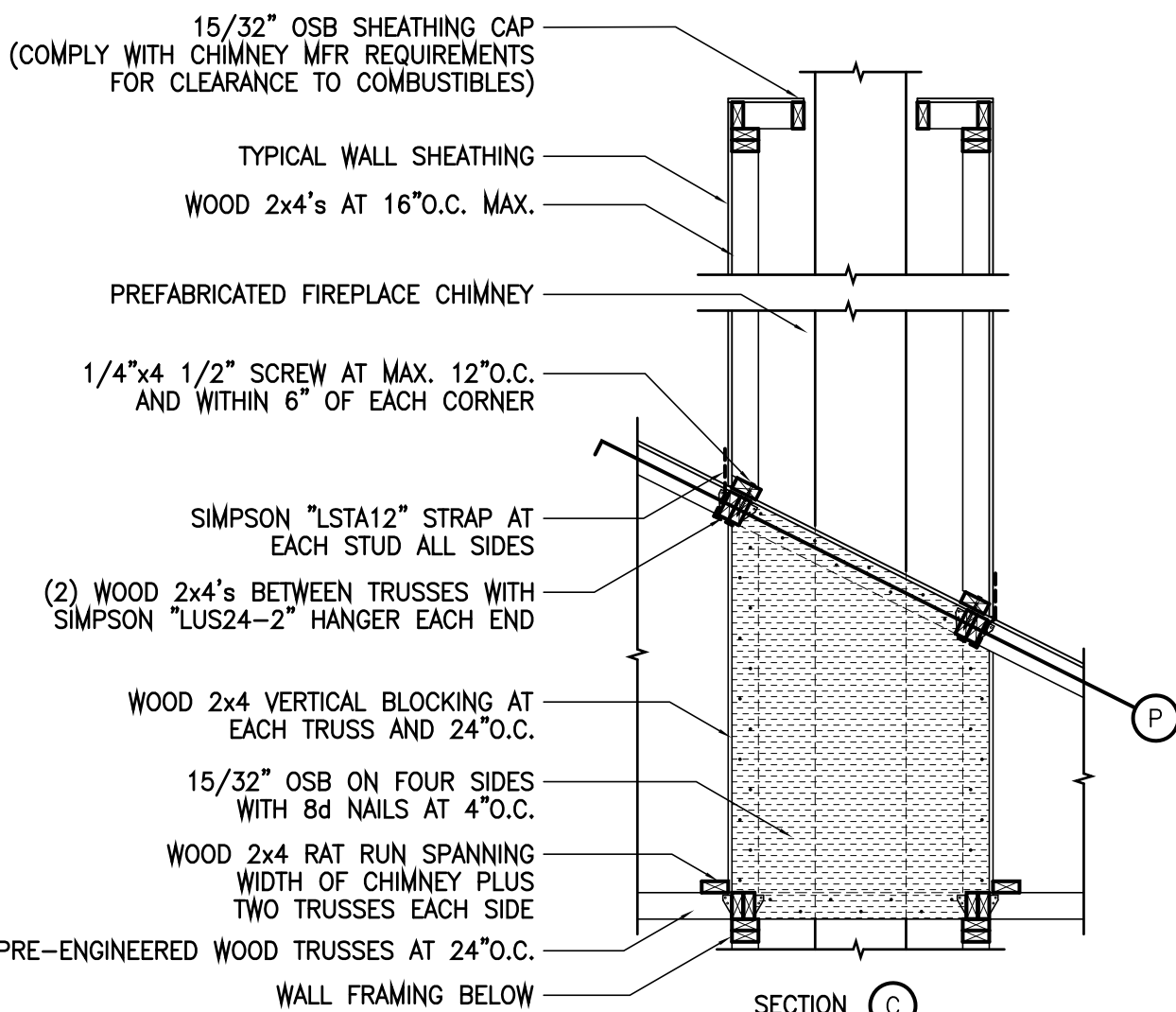


HIP ROOF VALLEY TRUSS OVERBUILD DETAIL

D-STRUCTURAL

SCALE: NOT TO SCALE

5
S6.10

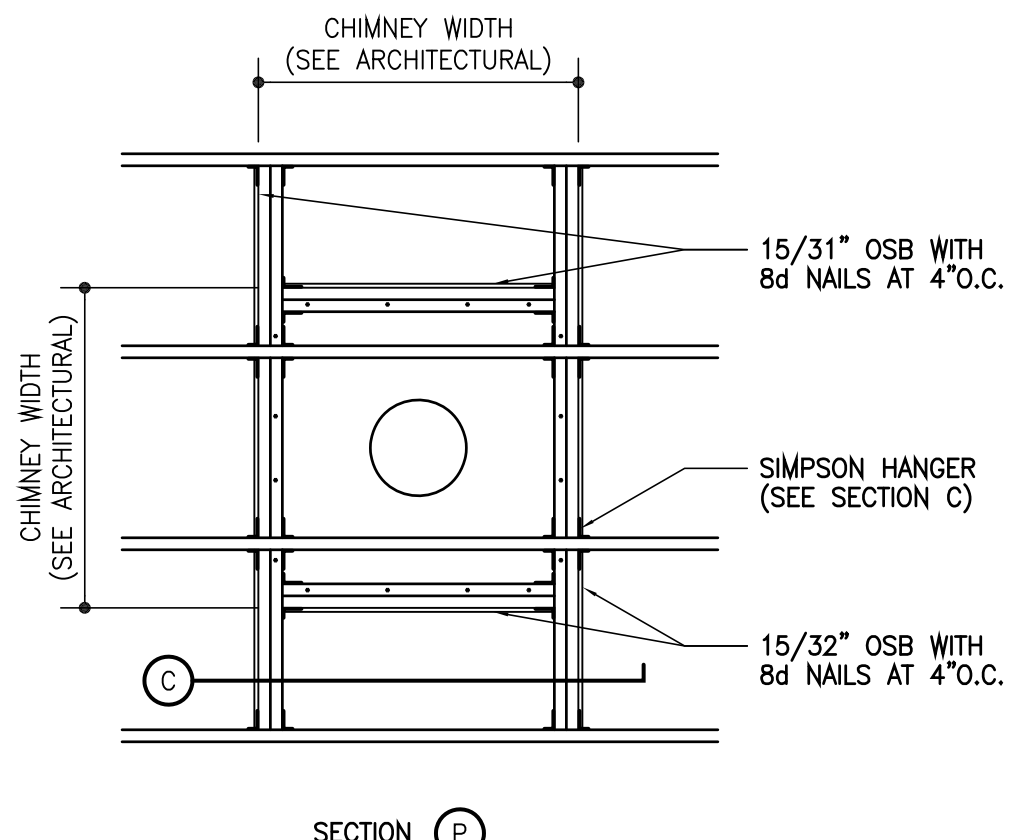


CHIMNEY FRAMING DETAIL

D-STRUCTURAL

SCALE: NOT TO SCALE

6
S6.10



SECTION C

SECTION P

■ ARCHITECT ■

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ROBERTS
RESIDENCE

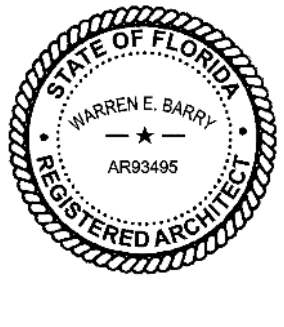
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Barry

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STANDARD
CONSTRUCTION

PROJECT NO.: 21020

DATE: JUNE 2, 2022

REVISION DATES:

STRUCTURAL DETAILS

S6.10

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