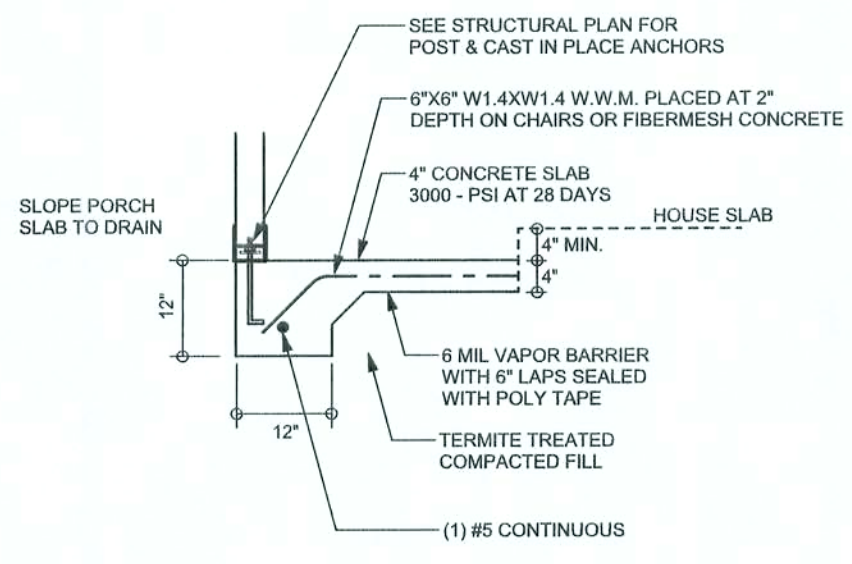
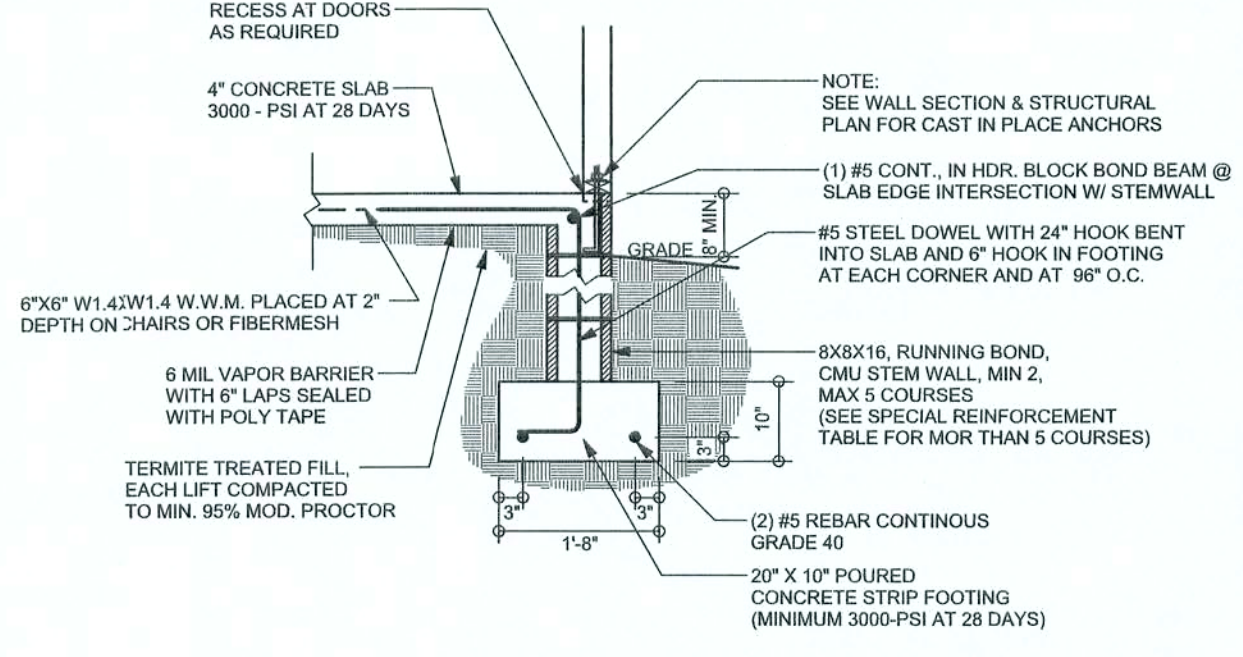


REVISIONS	
15 JAN 07	

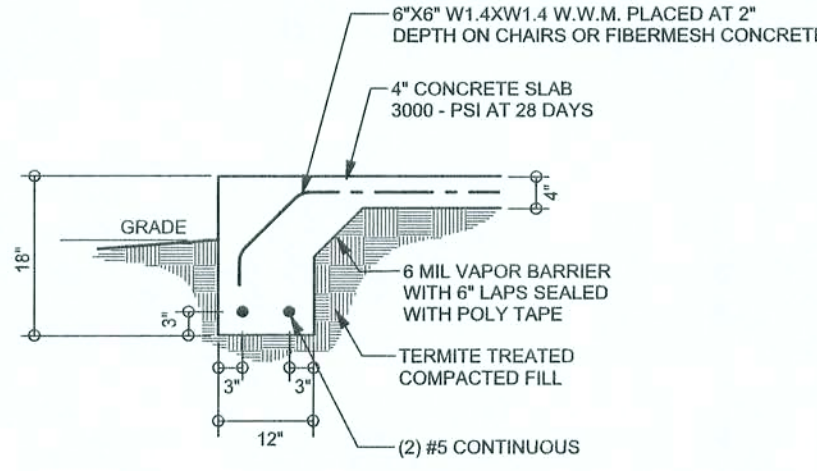
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



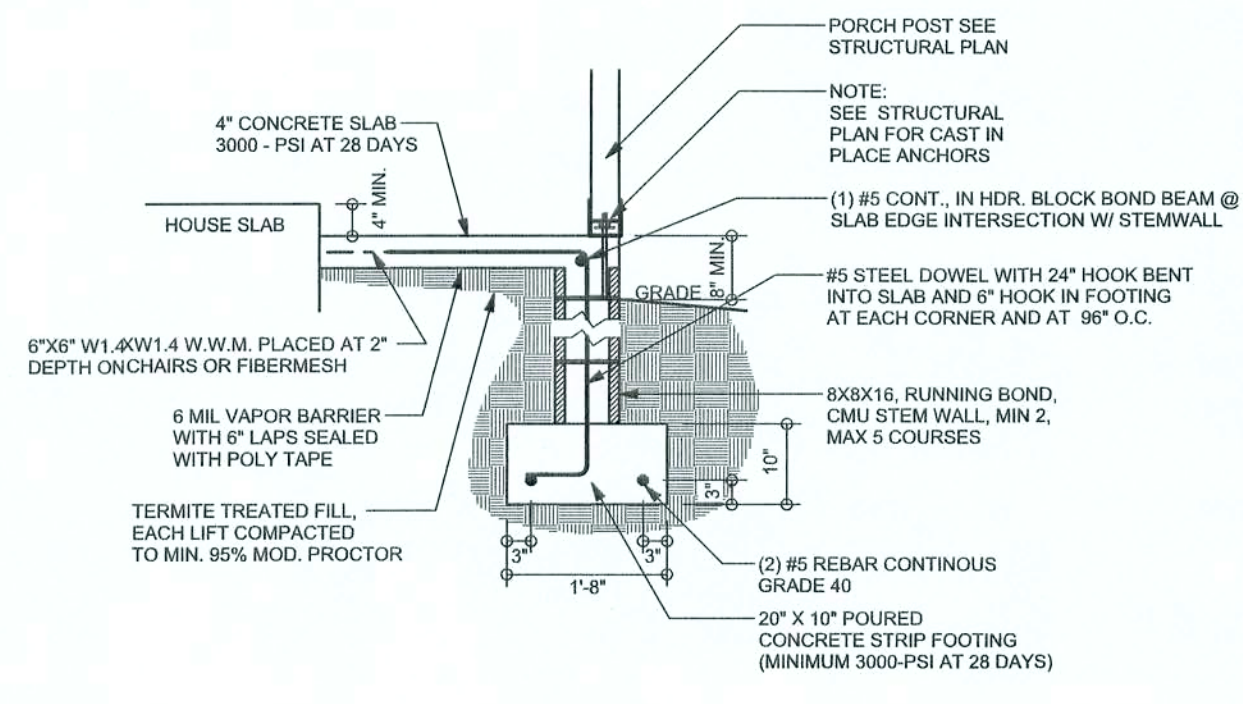
F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



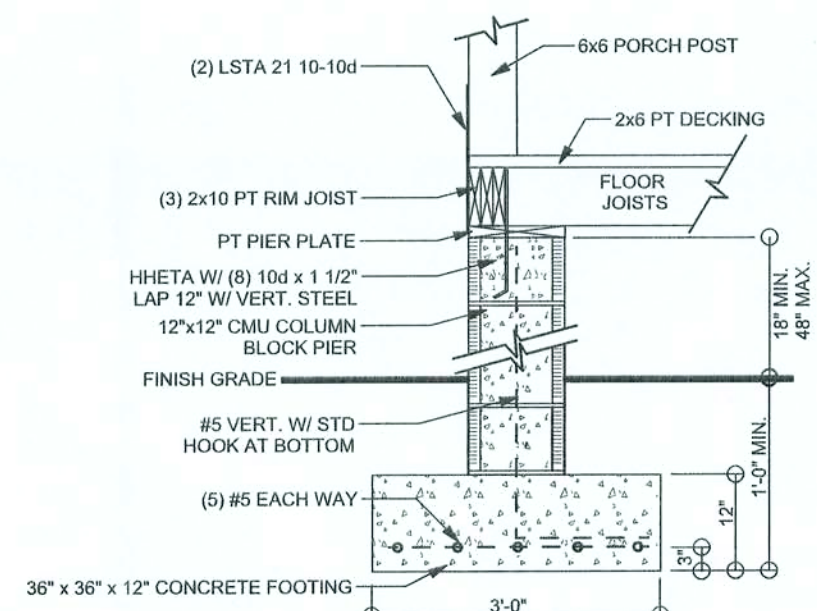
F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



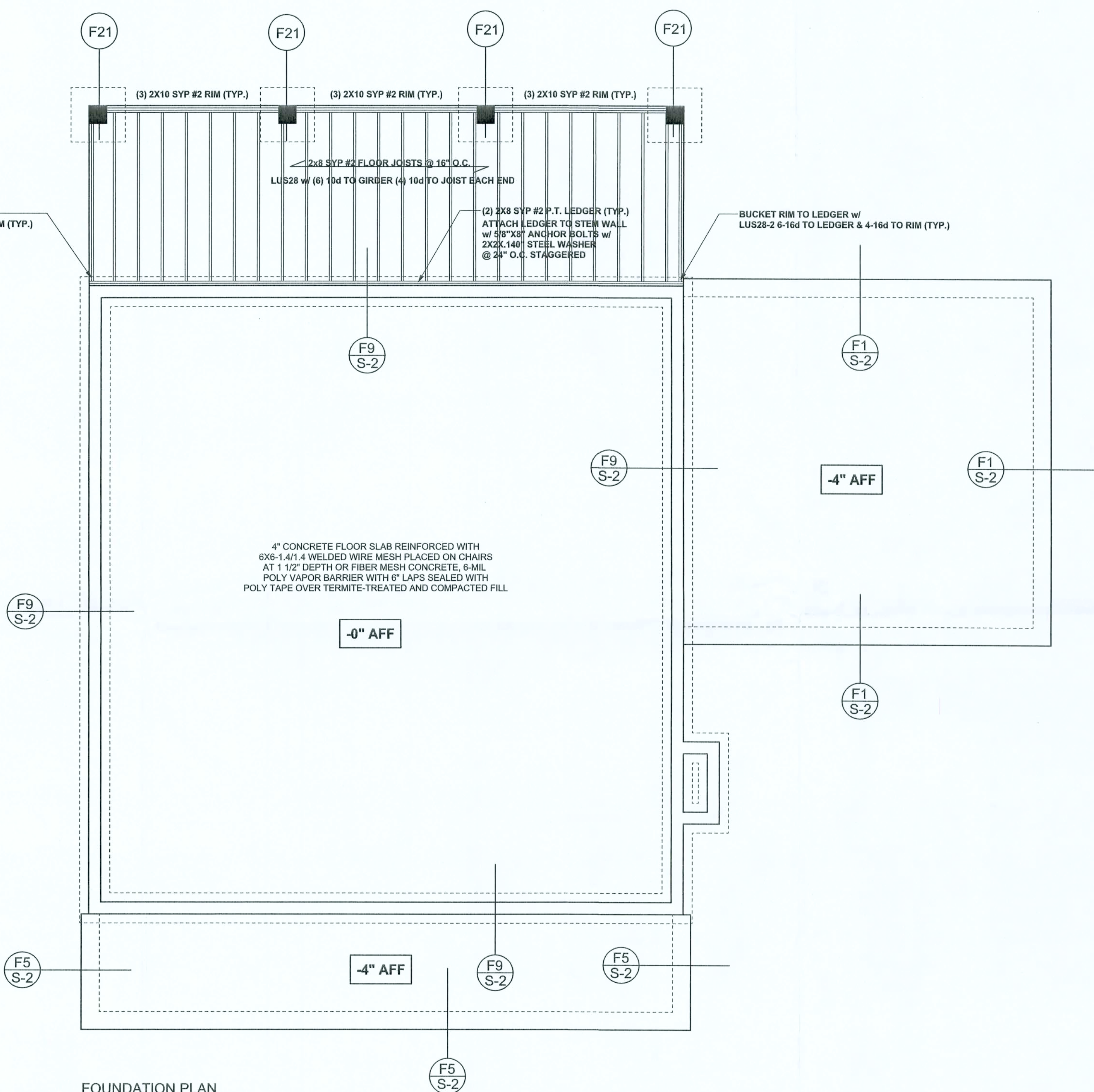
F21 S-2 1' PORCH PIER WITH UPLIFT
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with 165 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

BUCKET RIM TO LEDGER w/ LUS28-2 6-16d TO LEDGER & 4-16d TO RIM (TYP.)



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

ADDENDUM TO PERMIT # 0701-26

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056, (386) 754-0419
DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.
LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
15 JAN 07
SEAL

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PRINTED DATE:
January 15, 2007

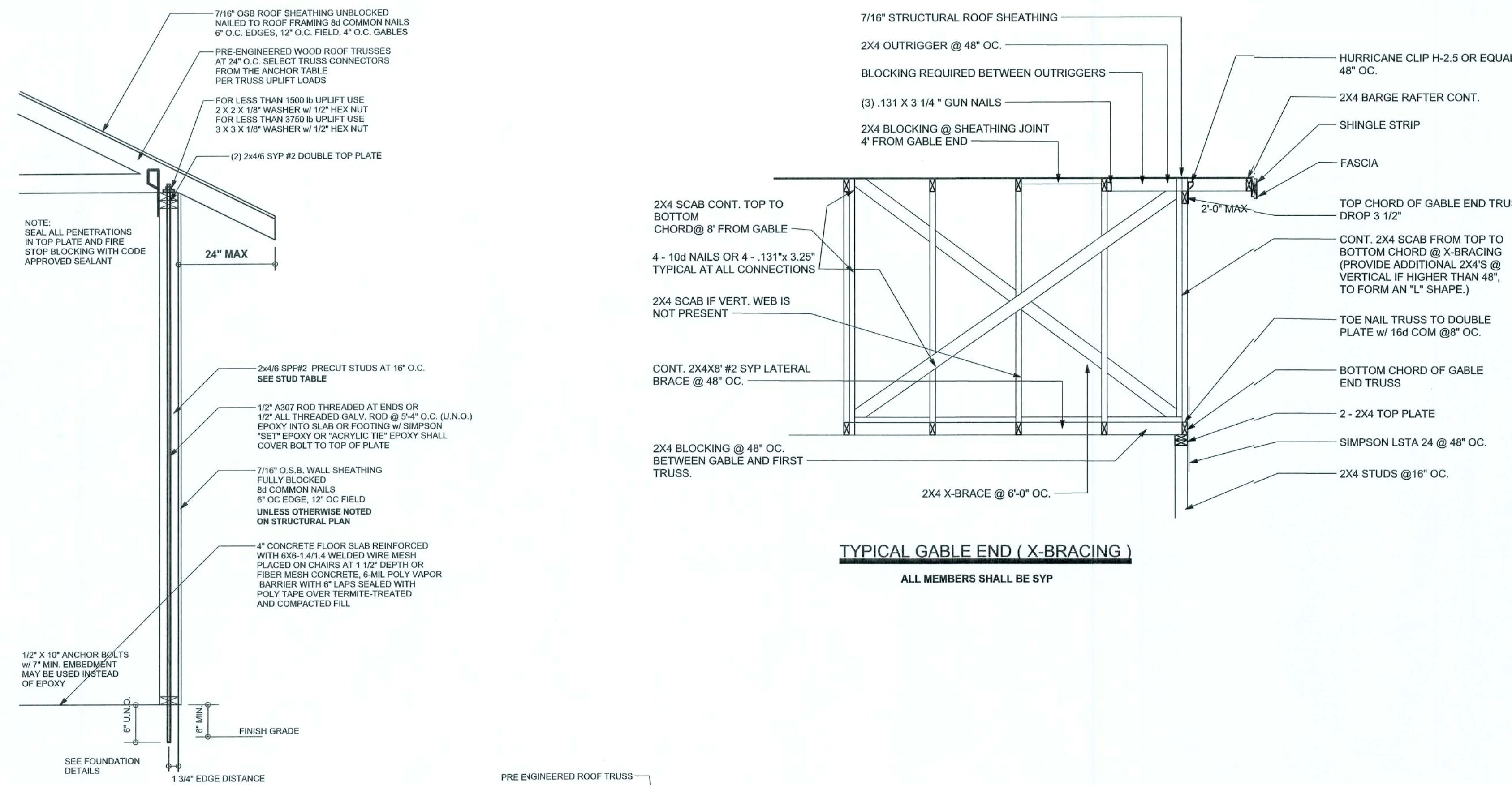
DRAWN BY: STRUCTURAL BY:
David Disoway

FINALS DATE:
02 / Jan / 07

JOB NUMBER:
701025
DRAWING NUMBER

S-2
OF 3 SHEETS

02/10/2007

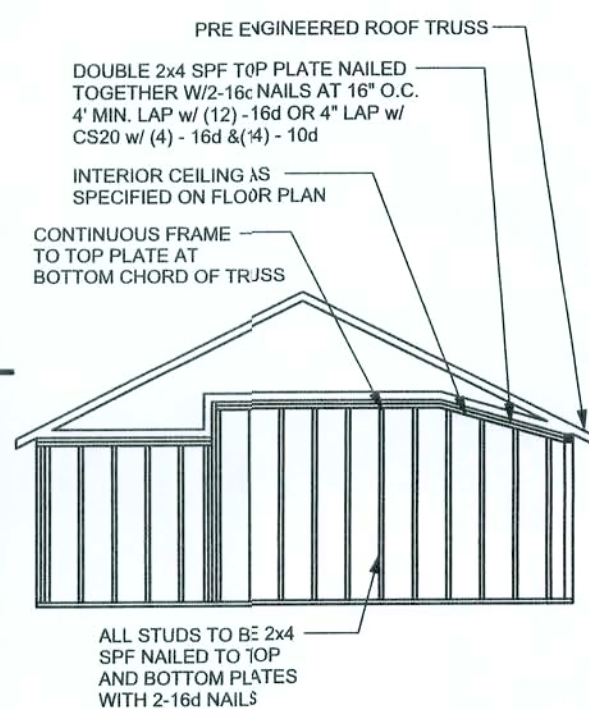


ONE STORY WALL SECTION
SCALE: 3/4\"/>

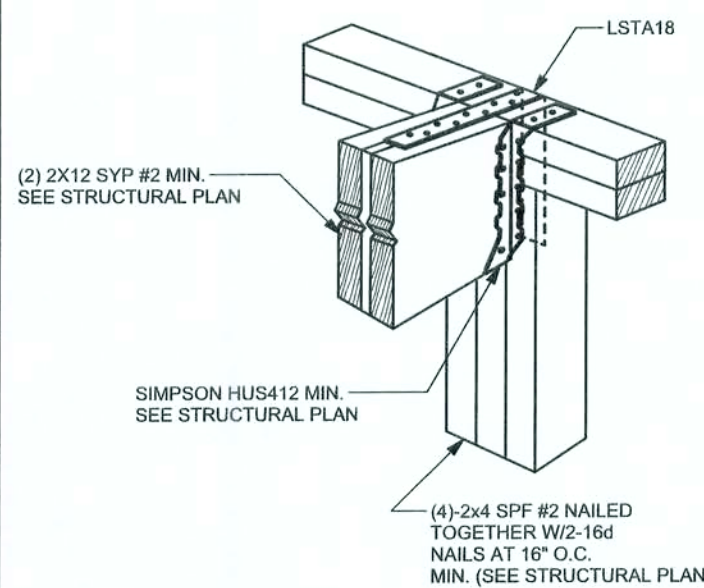
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16\"/>	TO 11'-9\"/>
(1) 2x4 @ 12\"/>	TO 13'-0\"/>
(1) 2x6 @ 16\"/>	TO 18'-10\"/>
(1) 2x6 @ 12\"/>	TO 20'-0\"/>

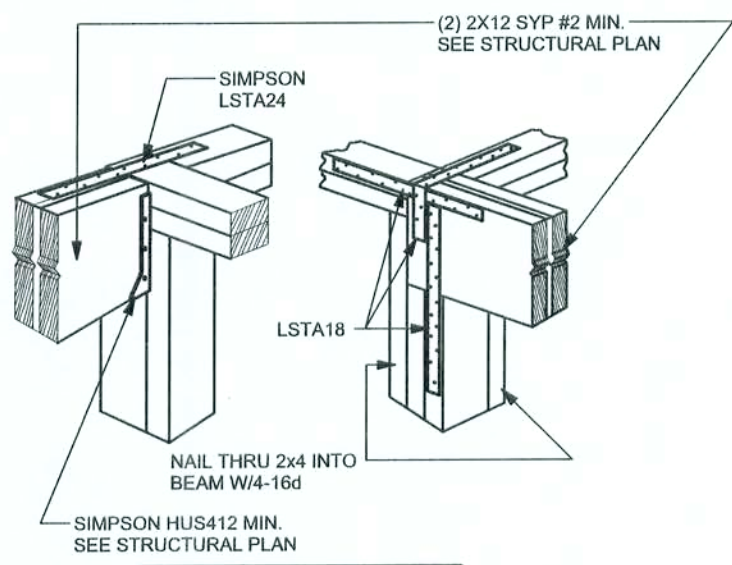
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16\"/>



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.

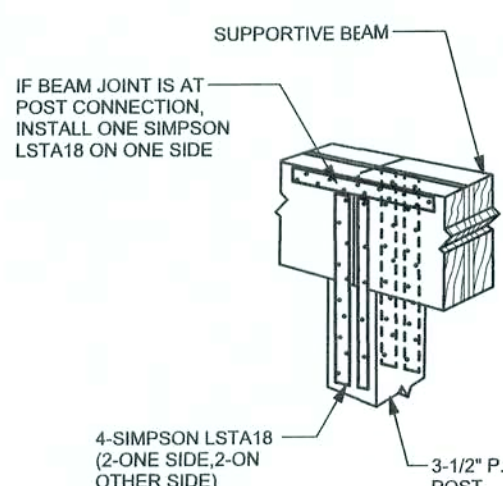


BEAM MID-WALL CONNECTION DETAIL
SCALE: N.T.S.

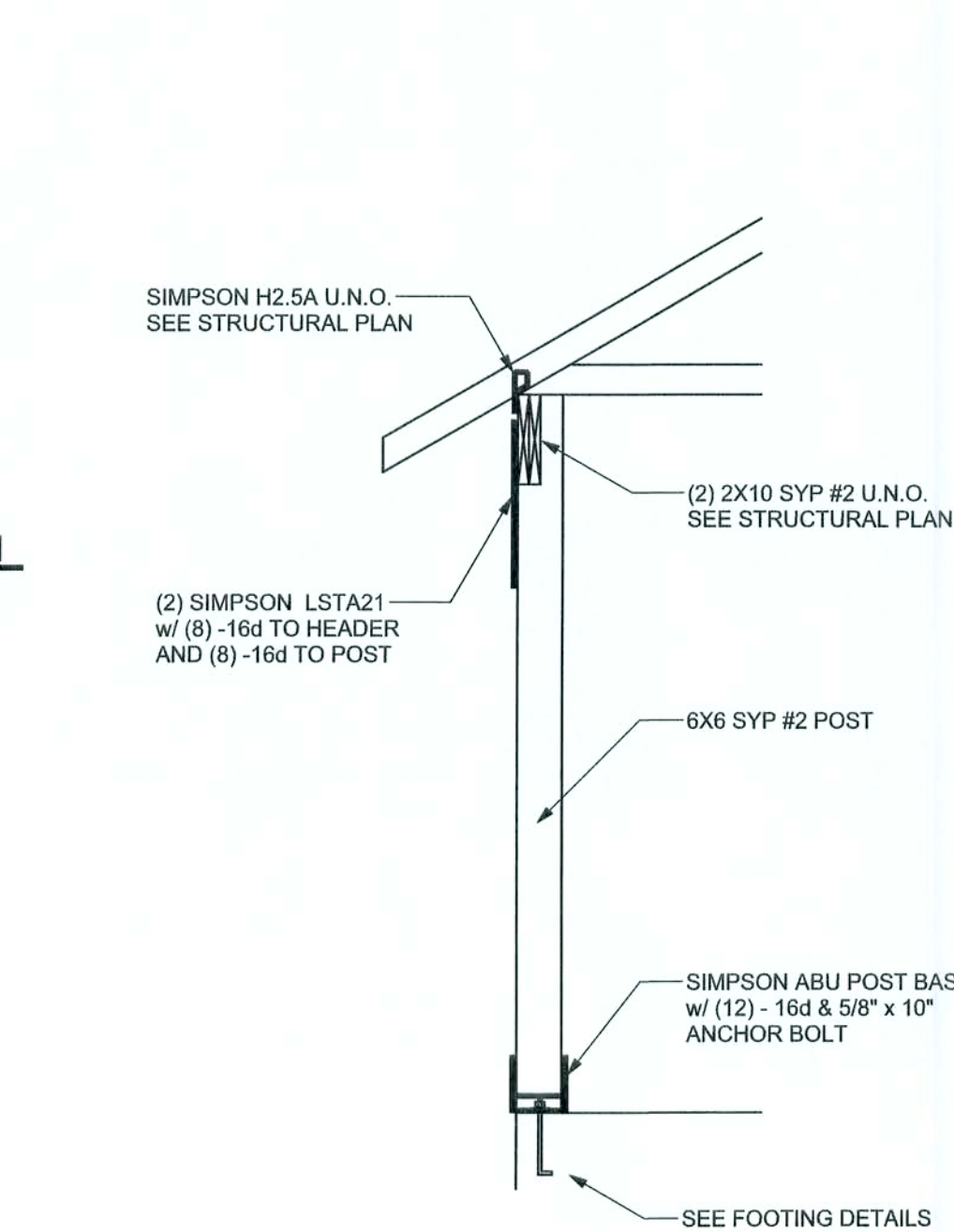


BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.

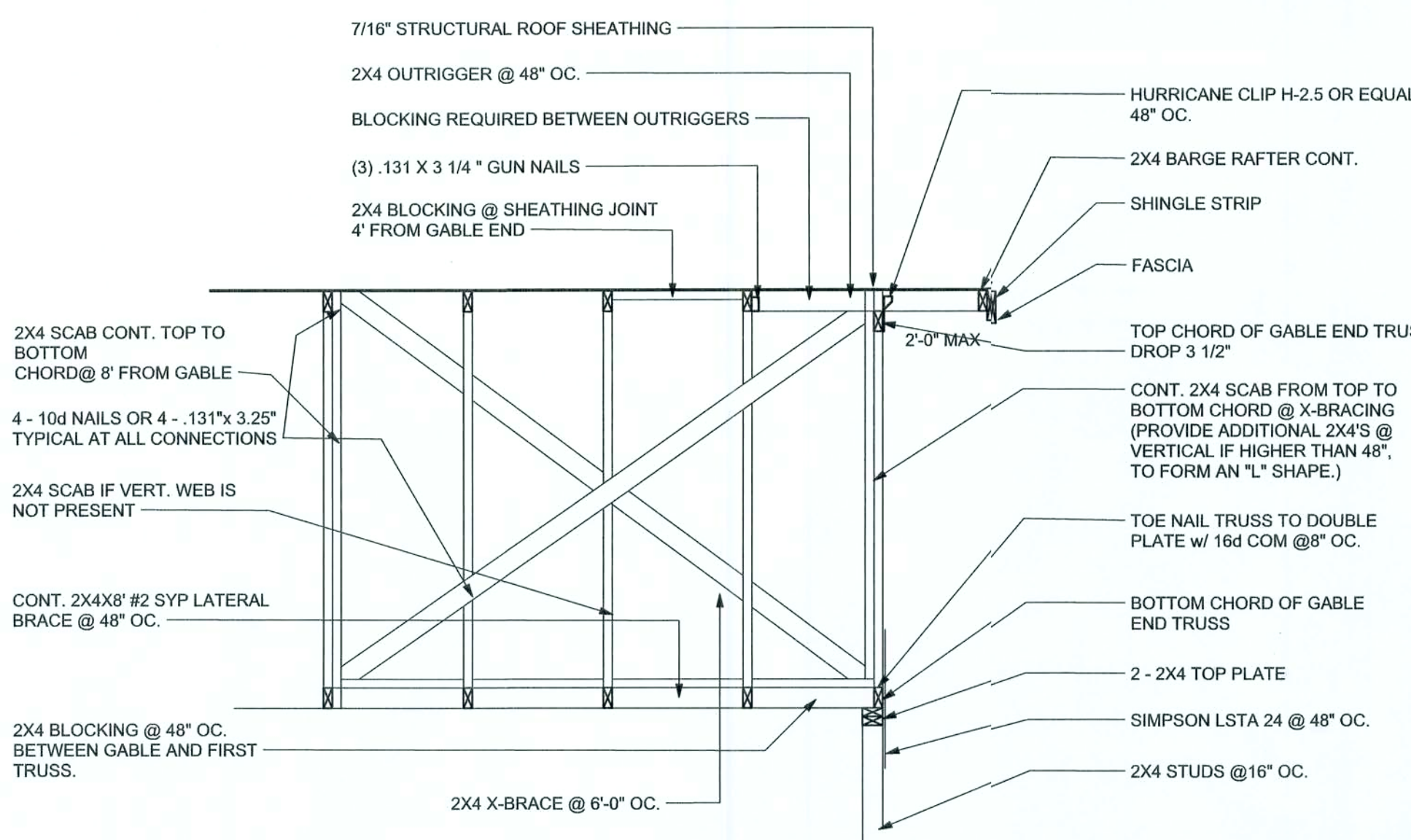
SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM
SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.

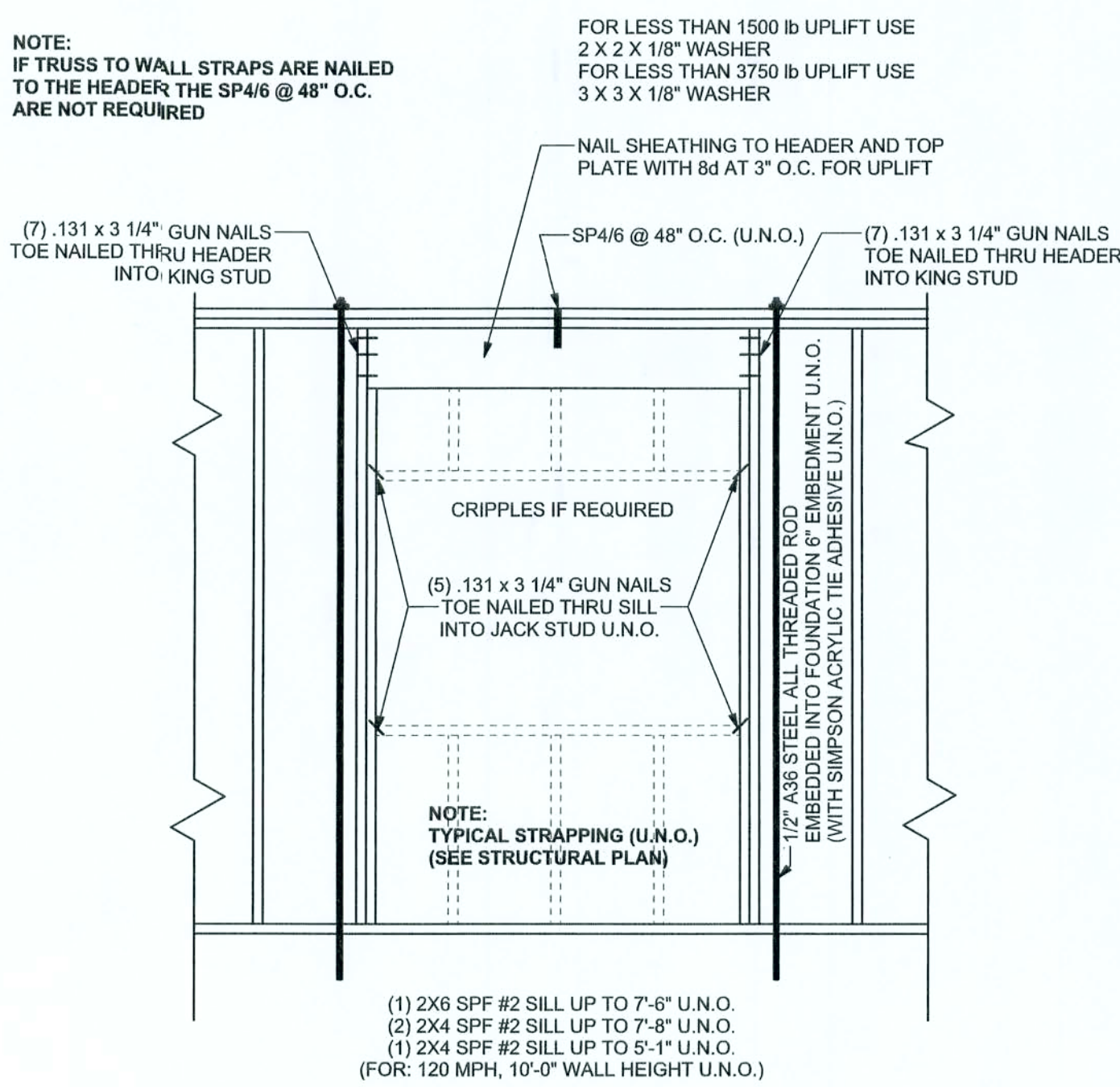


TYPICAL PORCH POST DETAIL
SCALE: 1/2\"/>



TYPICAL GABLE END (X-BRACING)
ALL MEMBERS SHALL BE SYP

NOTE:
IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER, THE SP4/6 @ 48\"/>



TYPICAL 1 STORY HEADER STRAPPING DETAIL
SCALE: 1/2\"/>

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

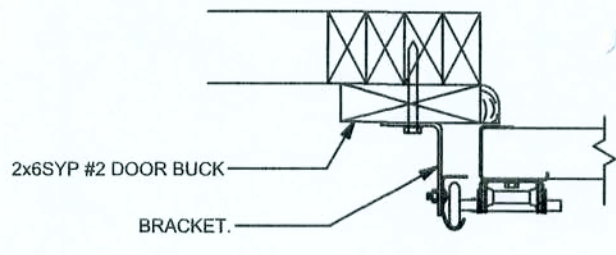
UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-6d	3-6d	
< 455	< 265	H5	4-6d	4-6d	
< 360	< 235	H4	4-6d	4-6d	
< 455	< 320	H3	4-6d	4-6d	
< 415	< 365	H2.5	5-6d	5-6d	
< 600	< 535	H2.5A	5-6d	5-6d	
< 950	< 820	H8	8-6d	8-6d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-6d	12-6d, 1 1/2"	
< 1465	< 1050	H14-2	15-6d	12-6d, 1 1/2"	
< 990	< 850	H10-1	8-6d, 1 1/2"	8-6d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS324	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS324			
< 2050	< 1785	LGT2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MG1		22-10d	1-5/8\"/>
< 10980	< 6485	HGT-2		16-10d	2-5/8\"/>
< 10530	< 9035	HGT-3		16-10d	2-5/8\"/>
< 9250	< 9250	HGT-4		16-10d	2-5/8\"/>
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-6d		
< 1705	< 1705	CS16	28-6d		
STUD ANCHORS*					
< 1350	< 1305	LTT18	8-16d		1/2\"/>
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2\"/>
< 2775	< 2570	HD2A	2-5/8\"/>		5/8\"/>
< 4175	< 3695	HTT16	18-16d		5/8\"/>
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2\"/>
< 2300	< 2300	ABU86	12-16d		1/2\"/>
< 2320	< 2320	ABU88	18-16d		2-5/8\"/>

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8\"/>

DOOR WIDTH	3/8\"/>	16d STAGGER	(2) ROWS OF .131 x 3 1/4\"/>
8' - 10'	24\"/>	5\"/>	5\"/>
11' - 15'	18\"/>	4\"/>	4\"/>
16' - 18'	16\"/>	3\"/>	3\"/>



GARAGE DOOR BUCK INSTALLATION DETAIL
SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE DESIGNED AND SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 416LB EACH END, 2X6 RAFTERS 700LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS ASSUMING 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOLS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSI

WELDED WIRE REINFORCED SLAB: 6\"/>

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF CURE PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 1FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40\"/>

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, FB = 2,400 PSI, E = 1,800,000 PSI. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS: 7/16\"/>

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\"/>

WASHERS: WASHERS USED WITH 1/2\"/>

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = <30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

	Zone	Effective Wind Area (ft ²)	
	1	19.9 -21.8	18.1 -18.1
	2	19.9 -25.5	18.1 -21.8
2 Orhg			40.6 -40.6
	3	19.9 -25.5	18.1 -21.8
3 Orhg		68.3	42.4
	4	21.8 -23.6	18.5 -20.4
	5	21.8 -29.1	18.5 -22.6
Doors & Windows Worst Case (Zone 5, 10 ft ²)		21.8	29.1
8x7 Garage Door		19.5	22.9
16x7 Garage Door		18.5	21.0

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPLAN
ARCHITECTURAL POWER SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,
PE No. 53915, POB 968, Lake City, FL
32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
02/10/07
SEAL

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PRINTED DATE:
January 02, 2007

DRAWN BY: STRUCTURAL BY:
David Disoway

FINALS DATE:
02 / Jan / 07

JOB NUMBER:
701025

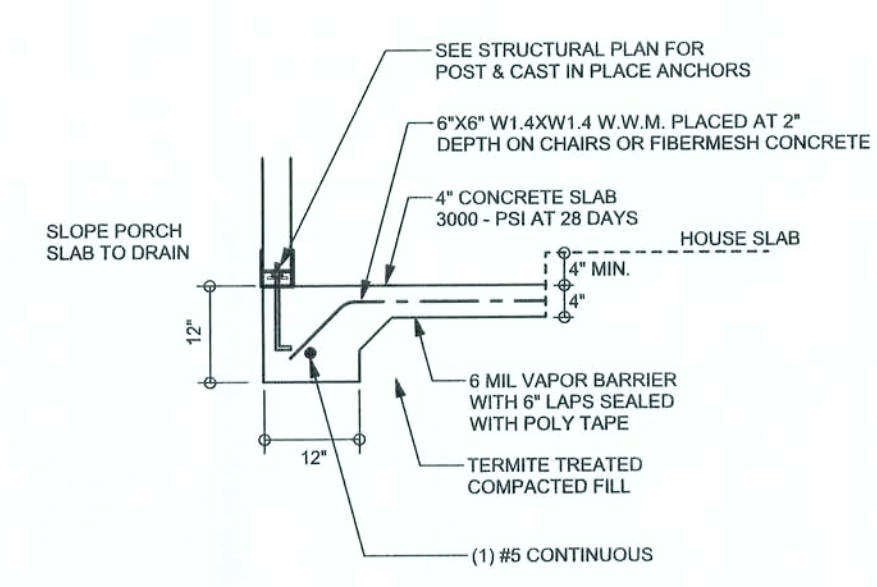
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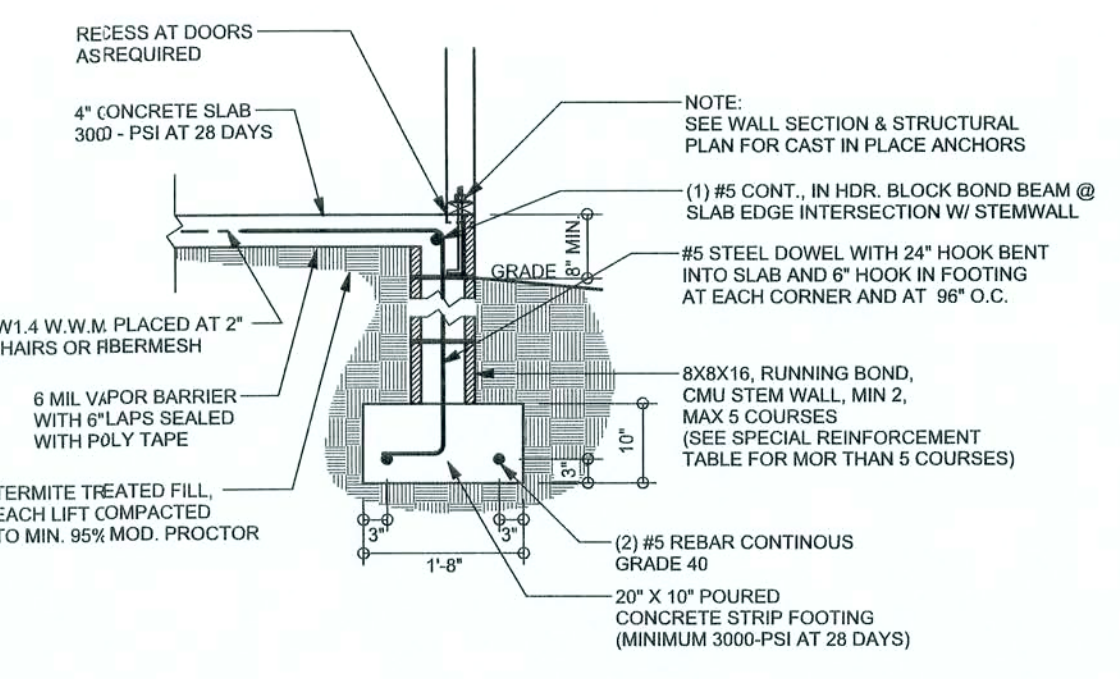
OF 3 SHEETS

REVISIONS	

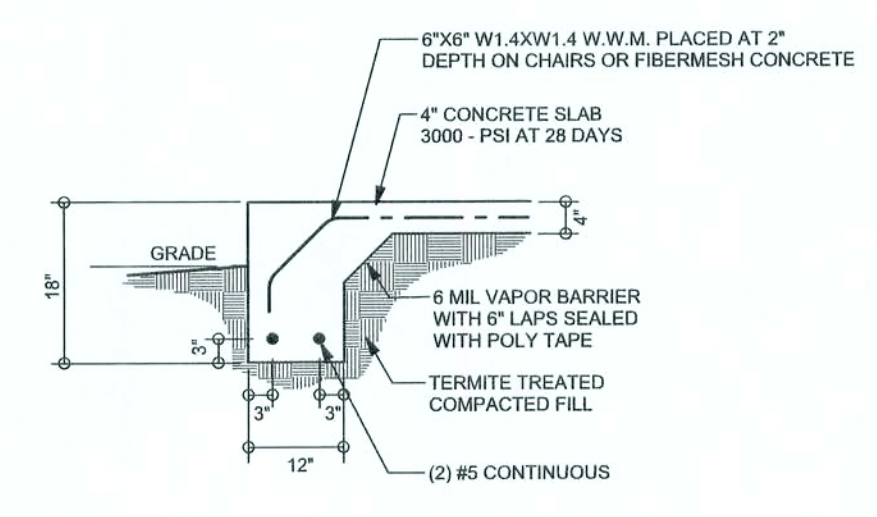
SOFTPLAN
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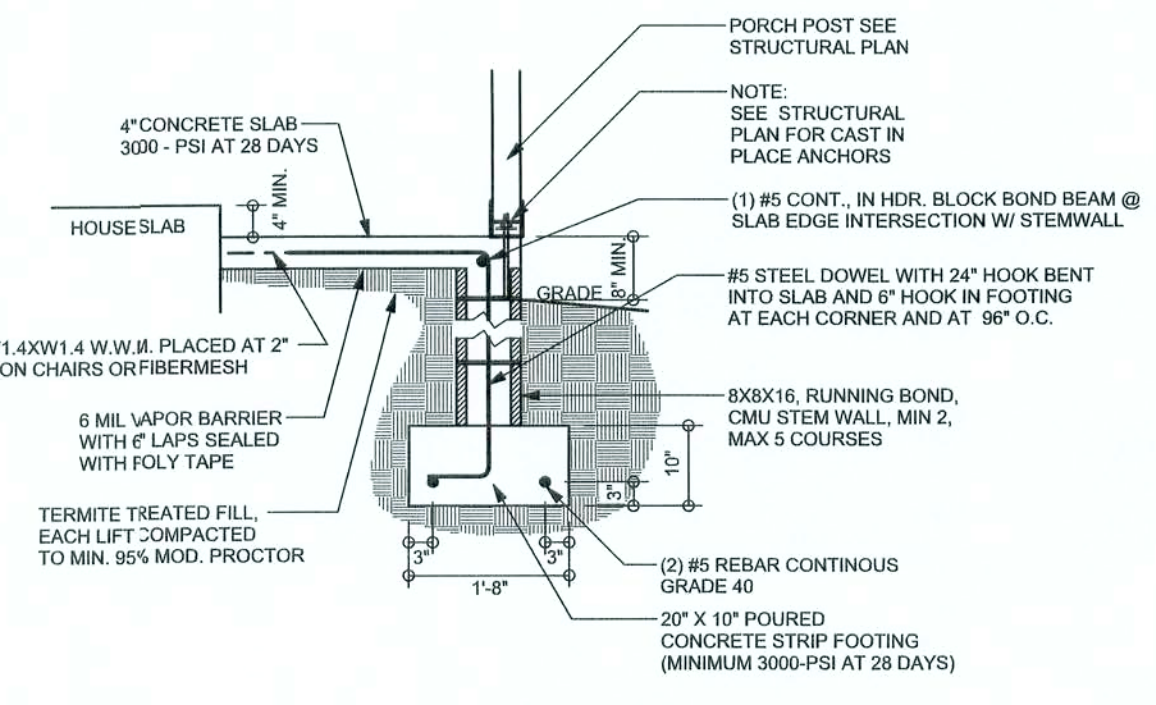
F5
S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



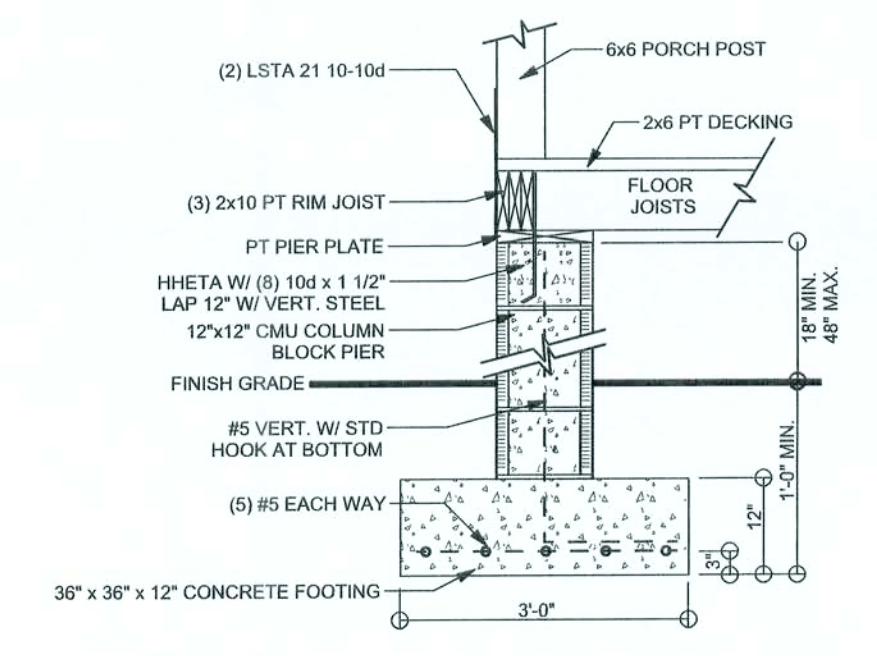
F9
S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F1
S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



F12
S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

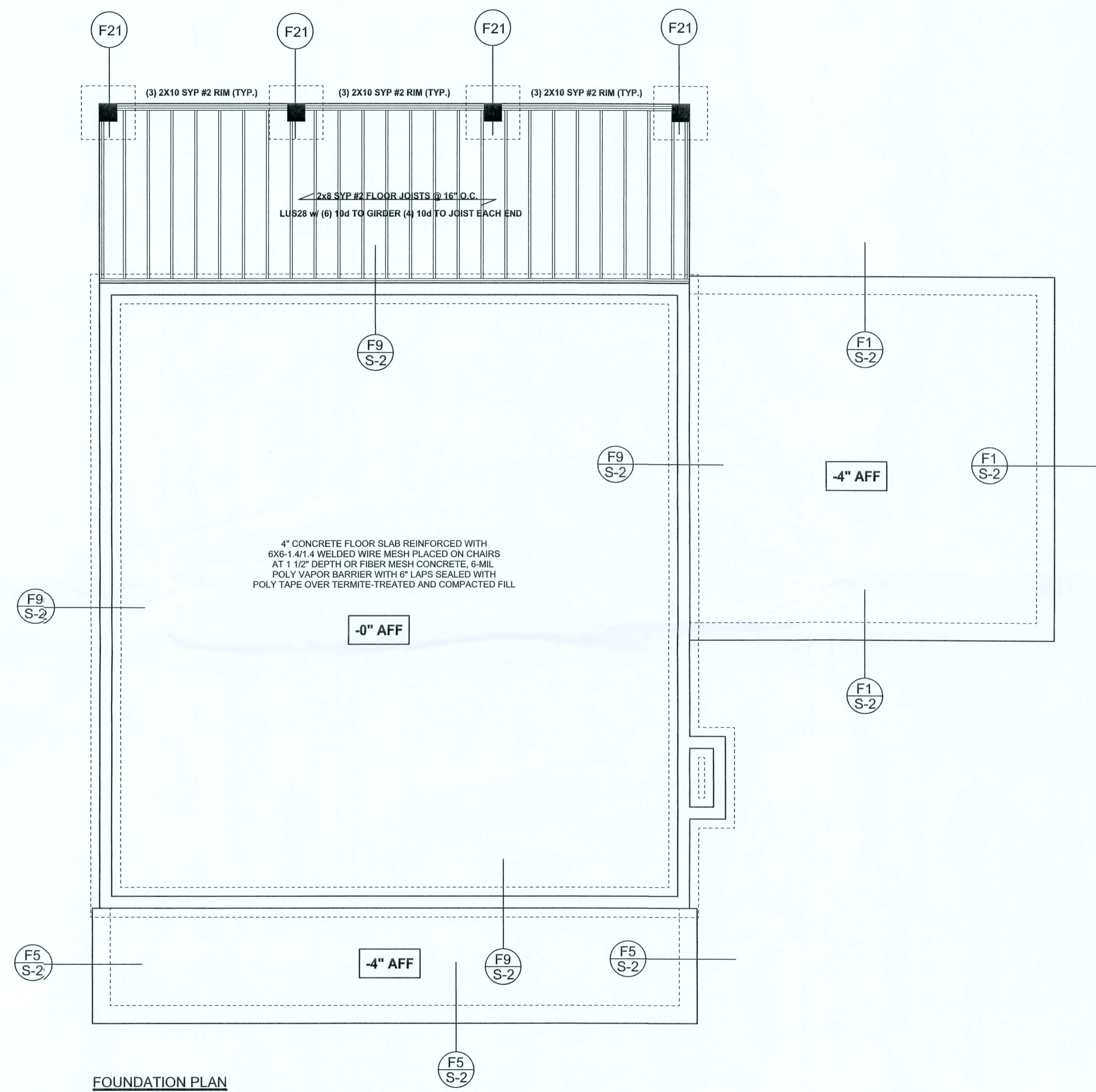


F21
S-2 1' PORCH PIER WITH UPLIFT
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with 1#6 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disoway,
P.E. No. 53915, P.O. Box 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
02 JAN 07
SEAL

Bryan Zecher
Construction

Jim & Karen
Lewis Residence

ADDRESS:
Carl Wilson St.
Columbia County, Florida

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
January 02, 2007

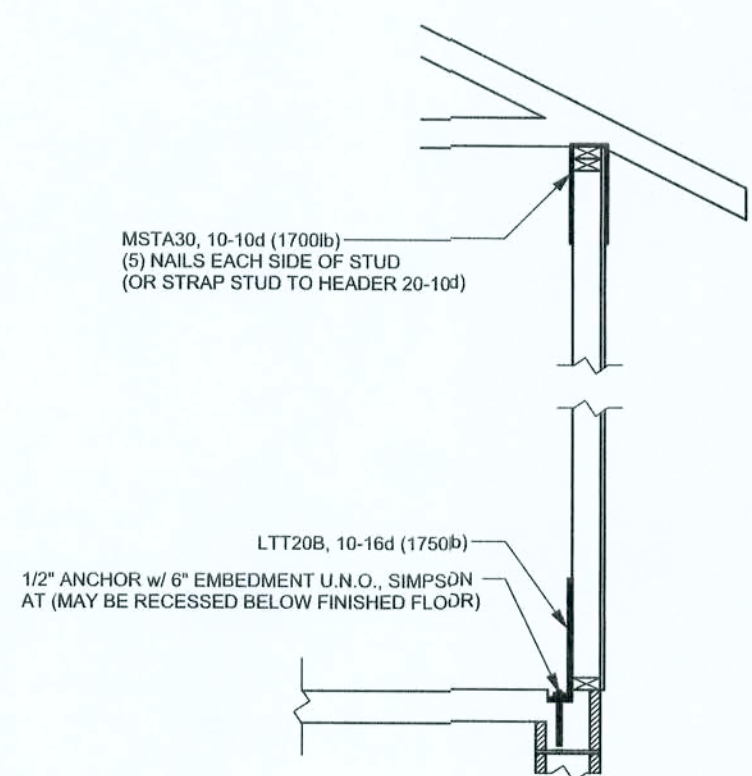
DRAWN BY: STRUCTURAL BY:
David Disoway

FINALS DATE:
02 / Jan / 07

JOB NUMBER:
701025

DRAWING NUMBER
S-2
OF 3 SHEETS

REVISIONS	



**ALTERNATE WALL TIE CONNECTION WHERE
THREADED ROD CANNOT BE PLACED IN WALL**
SCALE: 1/2" = 1'-0"

WINDLOAD ENGINEER: Mark Disoway,
PE No.53915, PGB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section RS01.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915

SEAL

**Bryan Zecher
Construction**

**Jim & Karen
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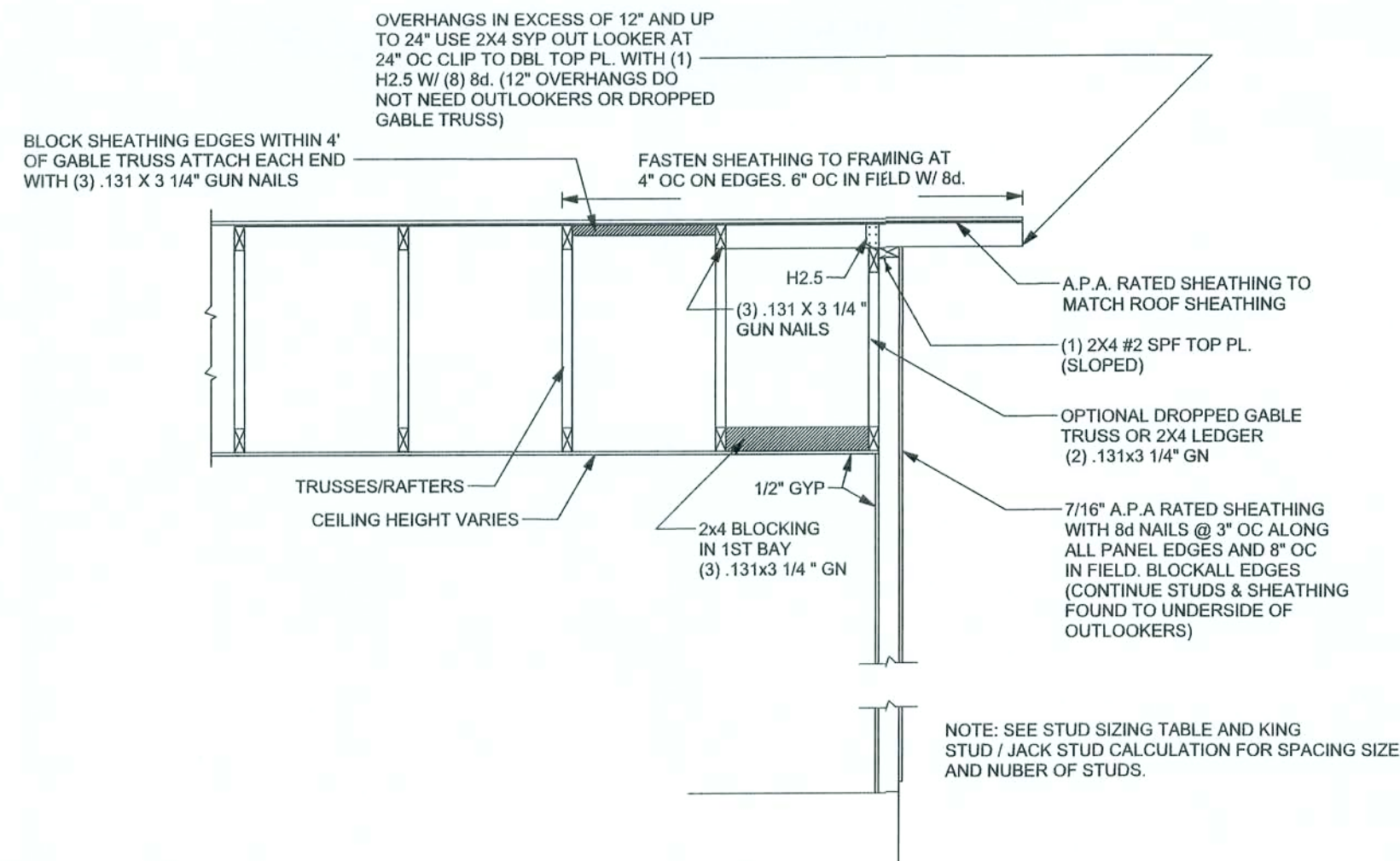
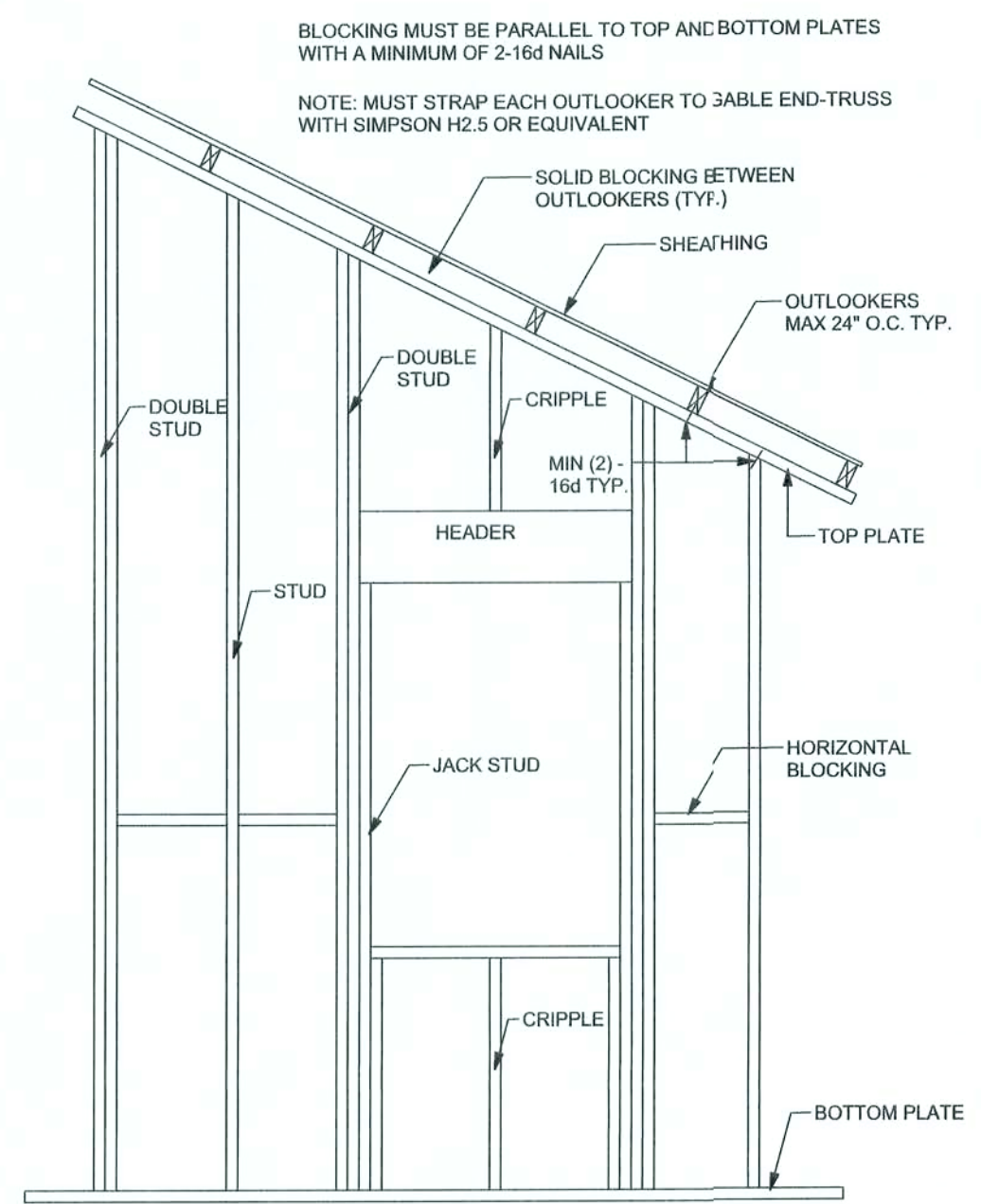
DRAWN BY: David Disoway

FINALS DATE:
02 / Jan / 07

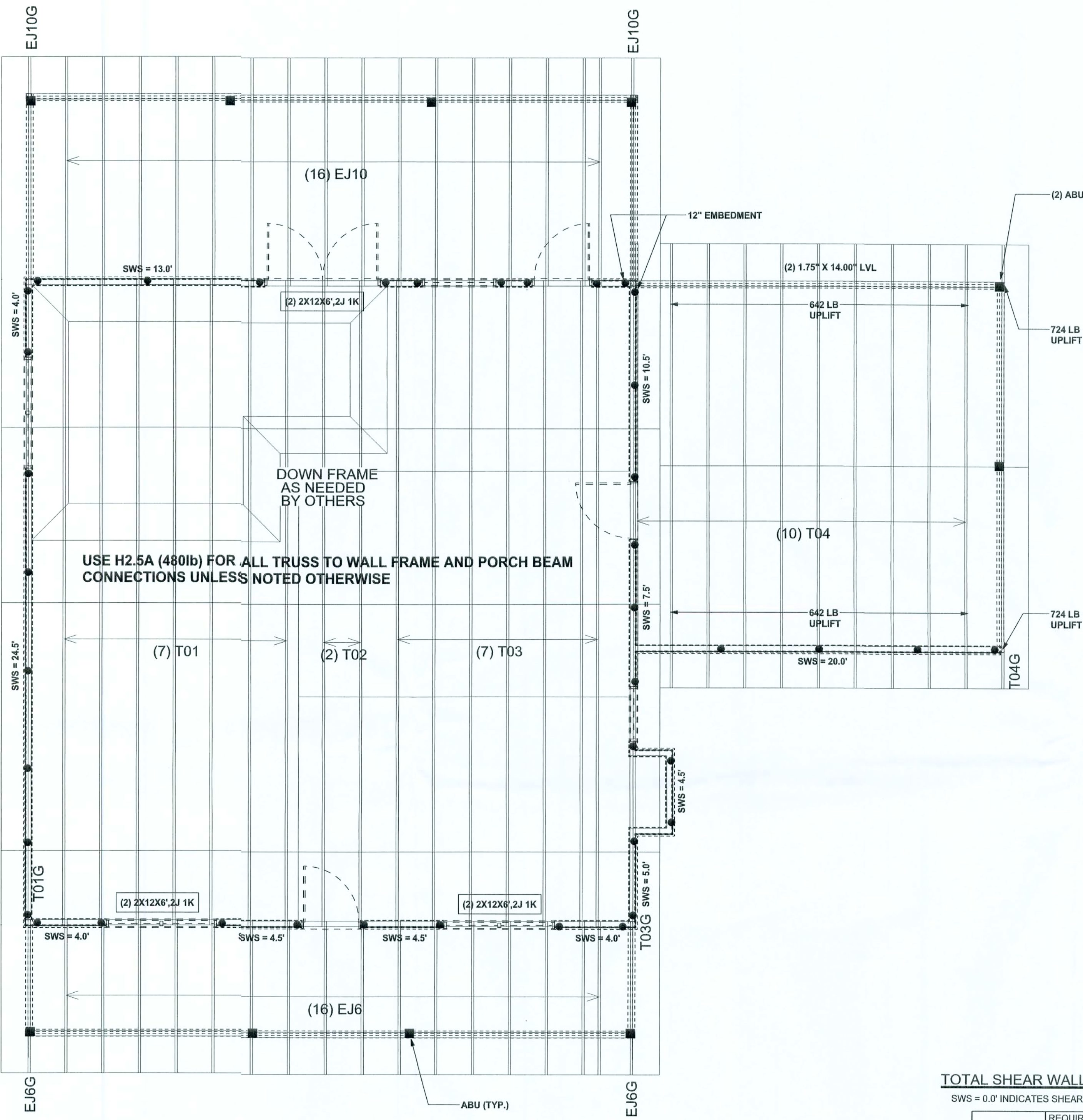
JOB NUMBER:
701025

DRAWING NUMBER
S-3

OF 3 SHEETS



GABLE END WALL BALLOON FRAMING DETAIL
SCALE: 1/2" = 1'-0"



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	32.5'	56.0'
LONGITUDINAL	28.5'	50.0'

THREADED ROD LEGEND

- INDICATES LOCATION OF:
1ST FLOOR 1/2" A307 ALL THREADED ROD
- INDICATES LOCATION OF:
2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

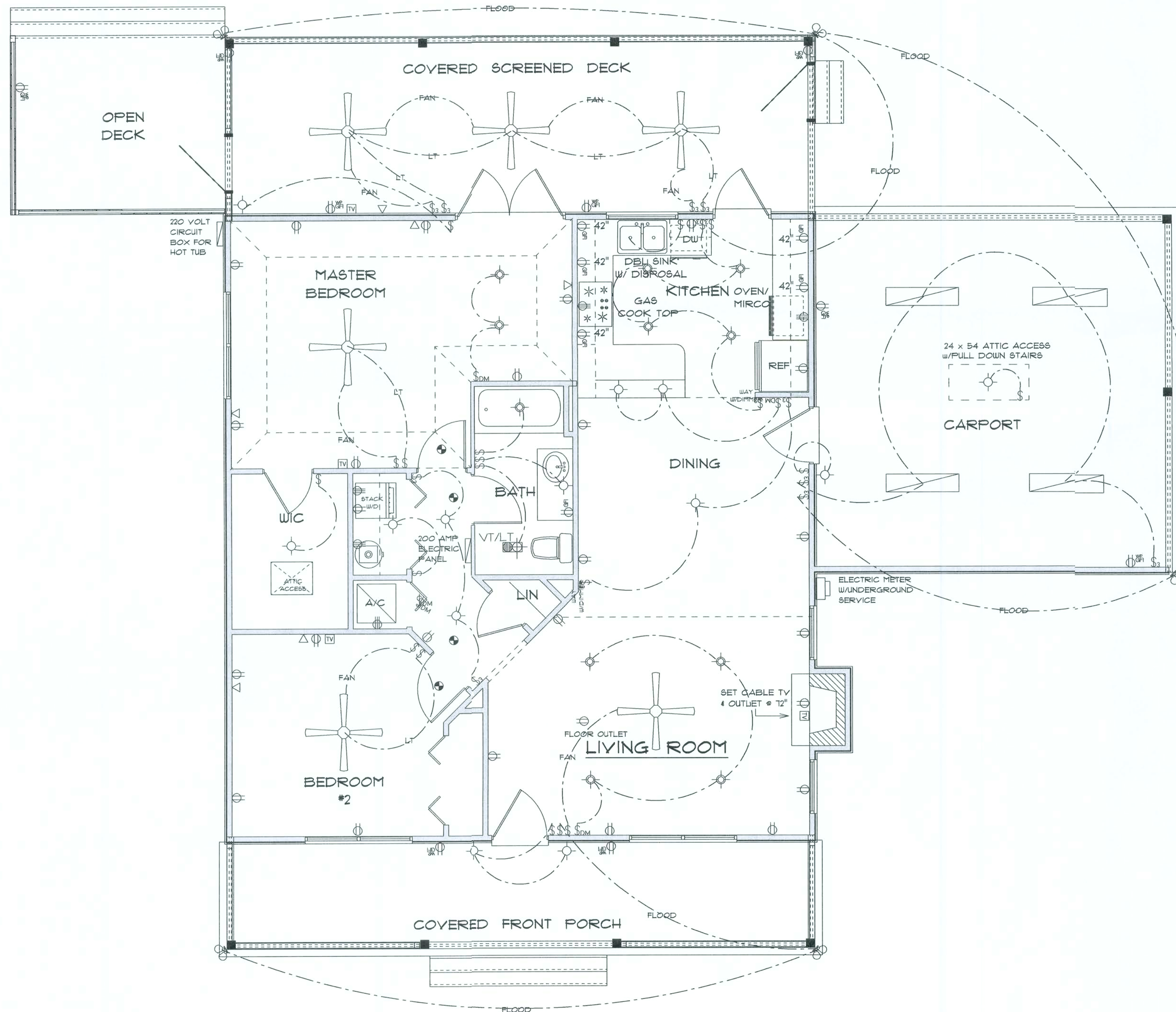
- (2) 2X12X0', 1J 1K ← HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L221634

ELECTRICAL PLAN NOTES

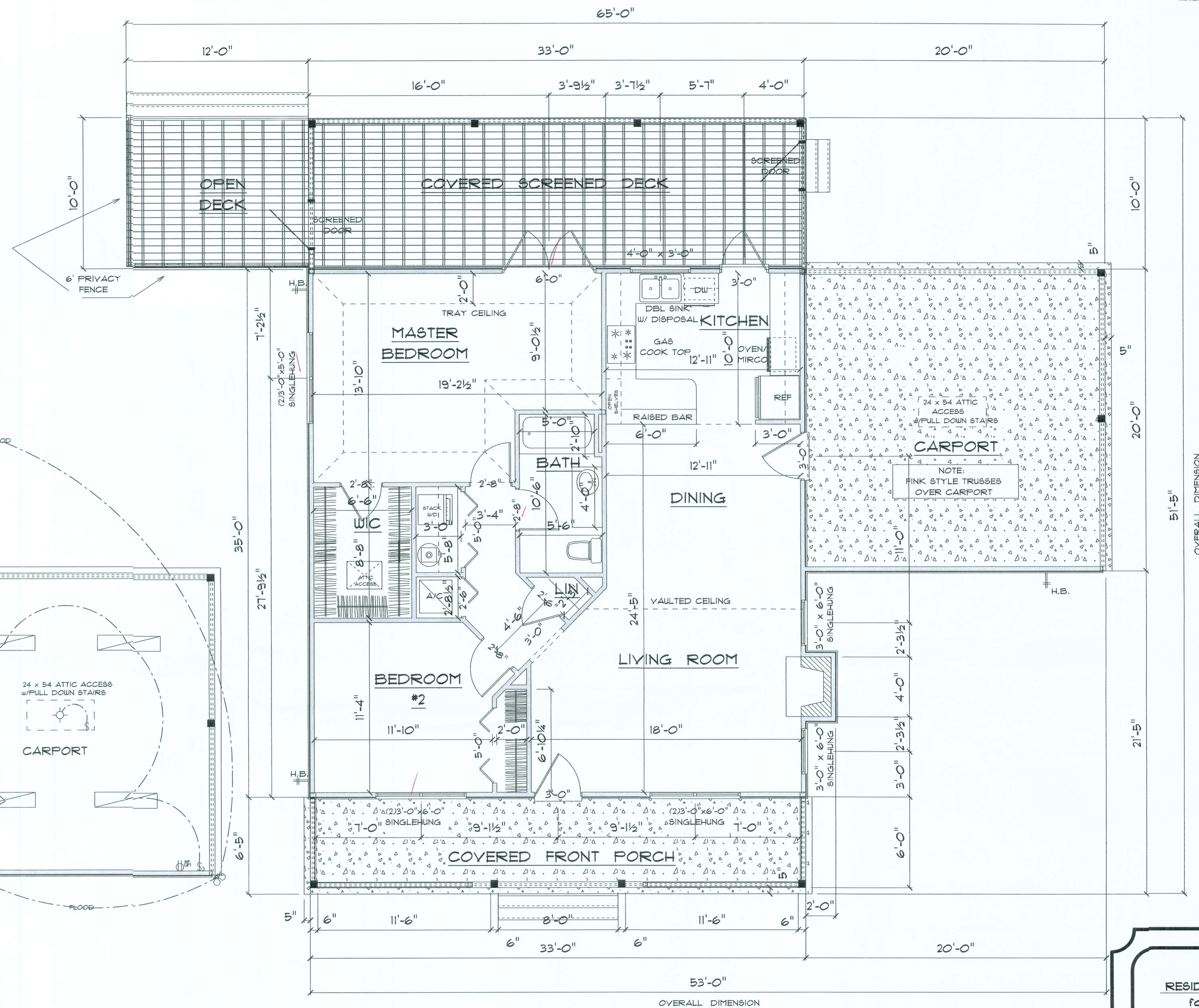
- E-1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUFACTURER'S SPECIFICATIONS.
- E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E-3 ALL INSTALLATIONS SHALL BE PER NATIONAL ELECTRIC CODE.
- E-4 ALL SMOKE DETECTORS SHALL BE 120V W/BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E-5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTION AND IN ACCORDANCE WITH APPLICABLE SECTIONS OF NATIONAL ELCT. CODE LATEST EDITION.
- E-6 ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E-7 ENTRY OF SERVICE UNDERGROUND OR OVERHEAD) IS TO BE DETERMINED BY THE POWER COMPANY.
- E-8 ALL BEDROOM RECEPTALS ARE TO BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

ELECTRICAL	COUNT	SYMBOL
200 amp service panel	1	
3 way switch with dimmer	2	
ceiling fan	6	
double flood lights	5	
dimmer switch	5	
electric meter	1	
floor outlet	1	
fluorescent lt fixture	4	
gfi waterproof outlet	3	
light	13	
outlet 220v	25	
outlet 220v	5	
outlet gfi	6	
recessed can light	11	
smoke detector	4	
switch	16	
switch 3 way	12	
telephone	6	
vent light combo	1	



* ELECTRICAL PLAN *

SCALE : 1/4" = 1'-0"



* FLOOR PLAN *

SCALE : 1/4" = 1'-0"

A/C LIVING AREA	1155 = S.F.
CARPORT	402 = S.F.
COVERED FRONT PORCH	138 = S.F.
COVERED SCREENED DECK	330 = S.F.
OPEN DECK	151 = S.F.
TOTAL AREA UNDER ROOF	2234 = S.F.

IMPORTANT NOTE:

- IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY ALL GEOMETRY AND DIMENSIONS BEFORE ORDERING ANY MATERIALS OR BEGINNING CONSTRUCTION.
- THESE PLANS ARE FOR LAYOUT AND DESIGN PURPOSES ONLY ALL STRUCTURAL NOTES AND DETAILS TO BE PROVIDED BY AN ENGINEER.

RESIDENCE
for
JIM & KAREN
LEWIS

Teresa M. Ruffo
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PRINTED DATE:
January 04, 2007

DRAWN BY: Teresa M. Ruffo

CHECKED BY:

BUILDING CONTRACTOR
ZECHER CONSTRUCTION

FINALS DATE:
DEC 2006

JOB NUMBER:

DRAWING NUMBER
A-1

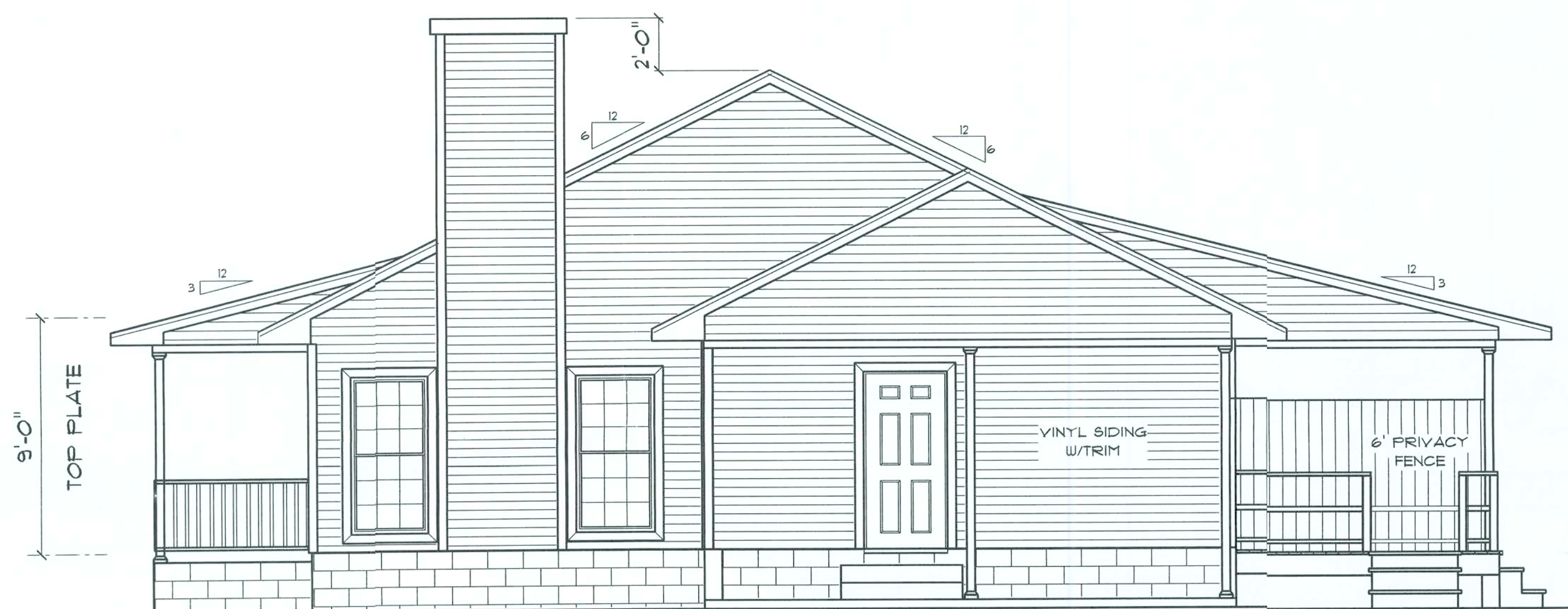
OF 3 SHEETS



* FRONT ELEVATION *

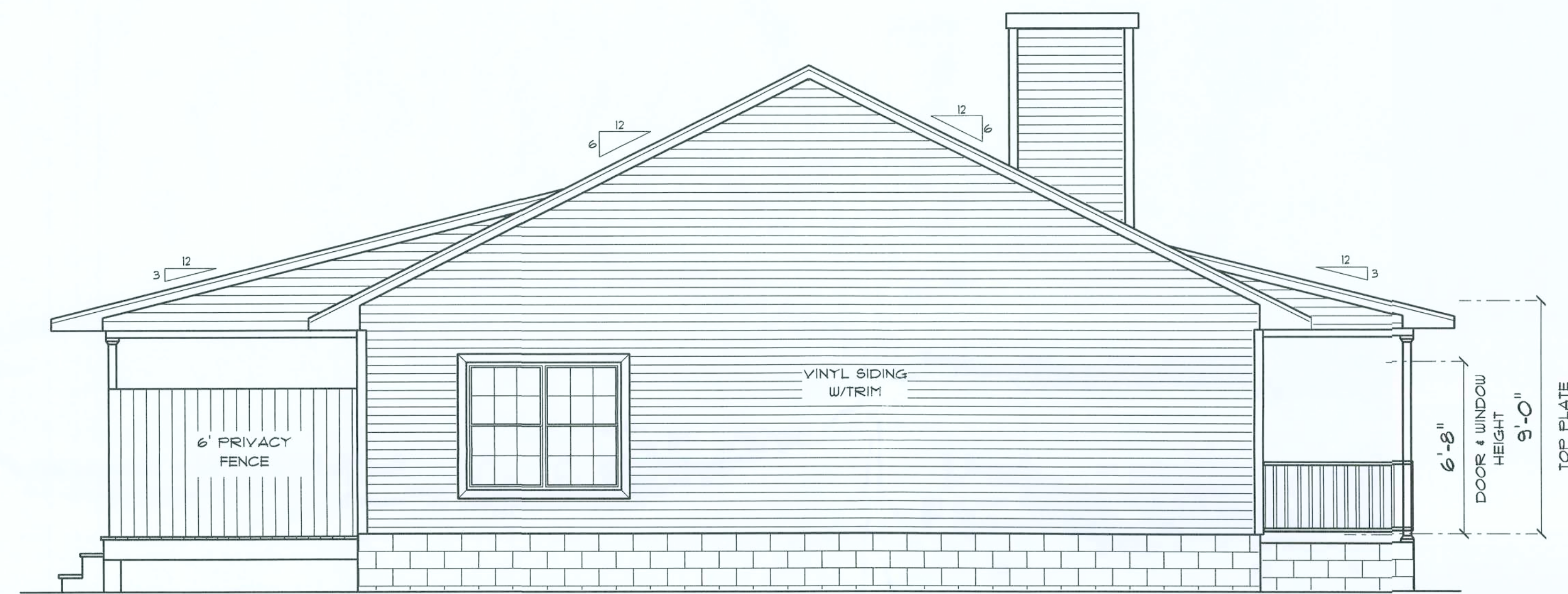
SCALE : 1/4" = 1'-0"

NOTE: 24" OVERHANG ON EAVES
12" OVERHANG ON GABLE ENDS



* RIGHT SIDE ELEVATION *

SCALE : 1/4" = 1'-0"



* LEFT SIDE ELEVATION *

SCALE : 1/4" = 1'-0"



* REAR ELEVATION *

SCALE : 1/4" = 1'-0"

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for
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January 04, 2007

DRAWN BY: CHECKED BY:
Teena M. Ruffo

BUILDING CONTRACTOR:
ZECHER CONSTRUCTION

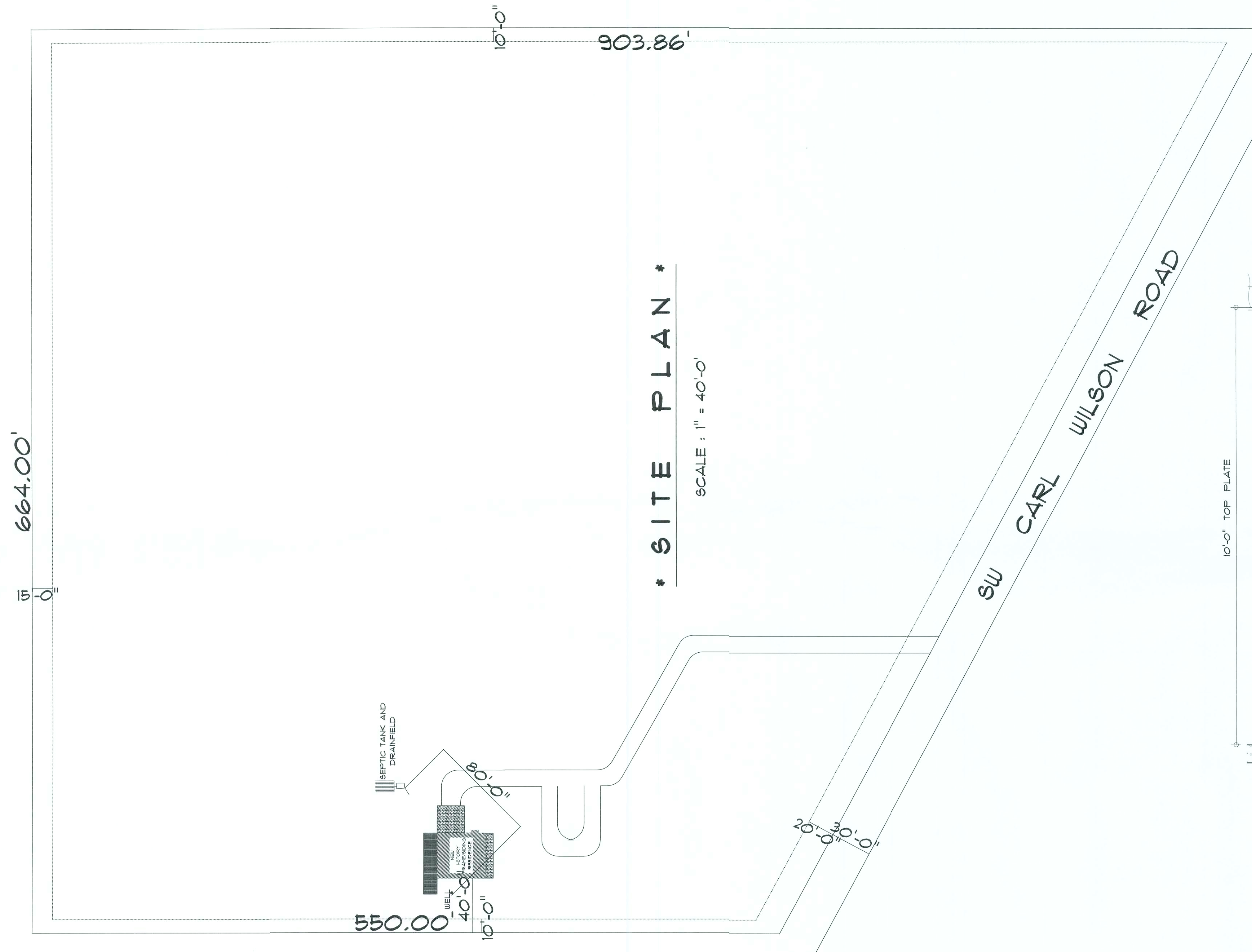
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DEC 2006

JOB NUMBER:

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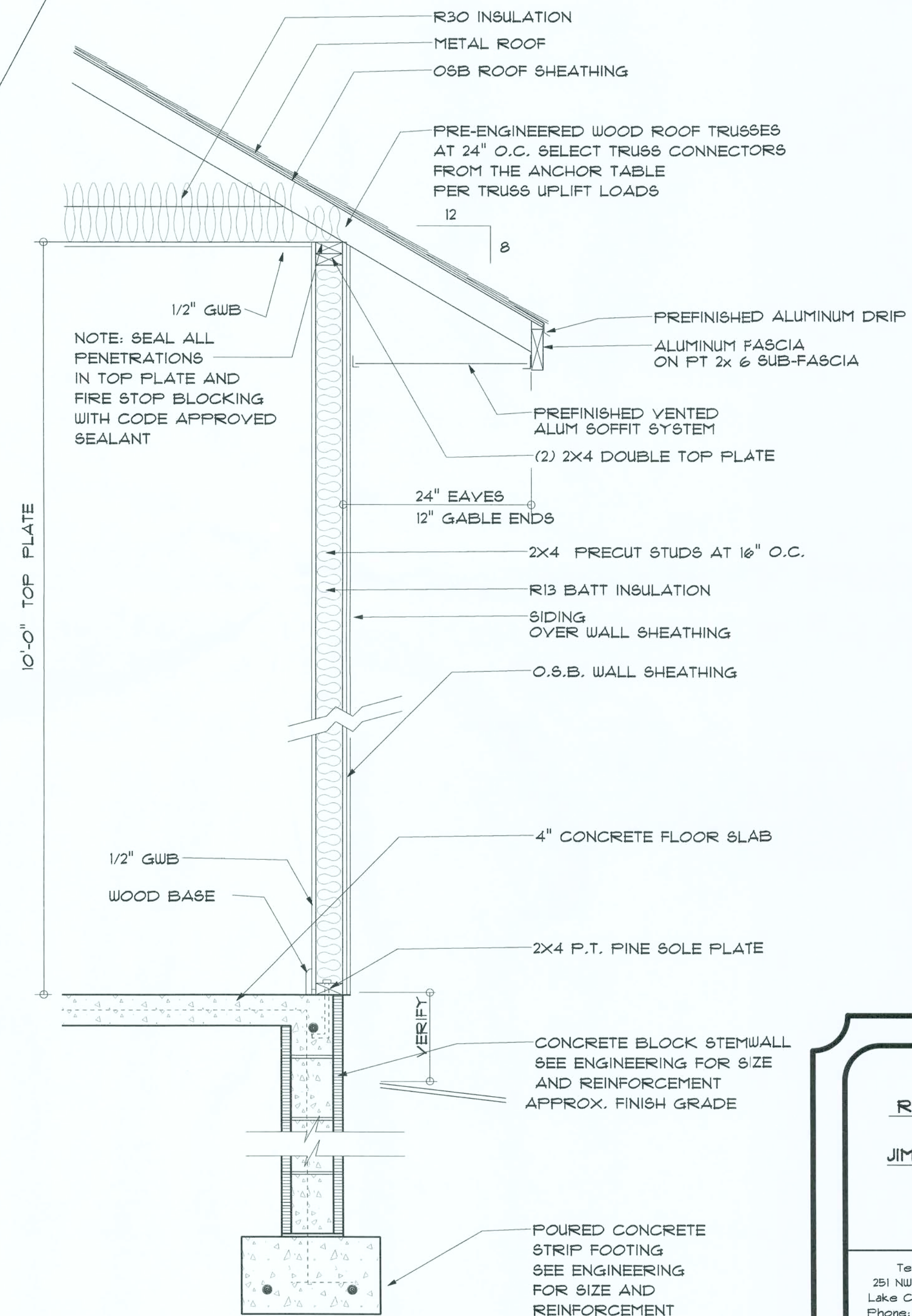
A-2

OF 3 SHEETS



* SITE PLAN *

SCALE : 1" = 40'-0"



TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA

NOT TO SCALE

IMPORTANT NOTE:

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for
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PRINTED DATE:
January 04, 2007

DRAWN BY: Teena M. Ruffo

CHECKED BY:

BUILDING CONTRACTOR

ZECHER CONSTRUCTION

FINALS DATE:

DEC 2006

JOB NUMBER:

DRAWING NUMBER

A-3

OF 3 SHEETS