

COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2017 EFFECTIVE 1 JANUARY 2018 AND THE NATIONAL ELECTRICAL 2014 EFFECTIVE 1 JANUARY 2018

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.3.1 THRU 1609.3.3.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES **Revised 7/1/18**

	Website: http://www.columbiacountyfla.com/BuildingandZoning.asp GENERAL REQUIREMENTS:		Each	ns to Inclu n Box shal Circled as	l be
	APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Applicable Select From Drop d			
1	Two (2) complete sets of plans containing the following:	_ ✓			
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	√			
3	Condition space (Sq. Ft.) 2266 Total (Sq. Ft.) under roof 3312	· ·	l'es	No	NA

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL 107.1.

Site Plan information including:

4	Dimensions of lot or parcel of land	Yes	
5	Dimensions of all building set backs	Yes	
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	Yes	
7	Provide a full legal description of property.	Yes	

Wind-load Engineering Summary, calculations and any details are required.

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable		l be
8	Plans or specifications must show compliance with FBCR Chapter 3	Yes	No	NA
		Select Fr	om Drop	down
9	Basic wind speed (3-second gust), miles per hour	Yes		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	Yes		
11	Wind importance factor and nature of occupancy	Yes		
12	The applicable internal pressure coefficient, Components and Cladding	Yes		
13	The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component, cladding materials not specifally designed by the registered design professional.	Yes		
Ele	evations Drawing including:			
14	All side views of the structure	Yes		
15	Roofpitch	Yes		
16	Overhang dimensions and detail with attic ventilation	Yes		
17	Location, size and height above roof of chimneys	NA		
18	Location and size of skylights with Florida Product Approval	NA		
19	Number of stories	Yes		
20	Building height from the established grade to the roofs highest peak	Yes		

Floor Pl an Including:

21	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches,	Yes	
	deck, balconies		
22	Raised floor surfaces located more than 30 inches above the floor or grade	NA	
23	All exterior and interior shear walls indicated	Yes	
24	Shear wall opening shown (Windows, Doors and Garage doors)	Yes	
25	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	Yes	
26	Safety glazing of glass where needed	, NA	
27	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)	NA	
28	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	NA	
29	Identify accessibility of bathroom (see FBCR SECTION 320)	Yes	

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be
	Circled as
	Applicable

FBCR 403: Foundation Plans

Select From Drop down

30	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	Yes	
31	All posts and/or column footing including size and reinforcing	Yes	
		NA	
	Assumed load-bearing valve of soil Pound Per Square Foot	NA	
34	Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	NA	

FBCR 506: CONCRETE SLAB ON GRADE

35 Show Varor retarder (6mil. Polyethylene with pints la 6 inches and sealed)	Yes	
36 Show control i oints, synthetic fiber reinforcement or welded fire fabric reinforcement and Sports	Yes	

FBCR 318: PROTECTION AGAINST TERMITES

1				i l		ı
1	Indicate on the foundation plan if soil treatment is used for	or subterranean termite prevention or	l i			1
1			Yes			ı
3,	Submit other approved termite protection methods. Protec	tion shall be provided by registered	res			į
))	Dublint dilet approved to mite protection memorial in a			[i	Ĺ
1	termiticides					i

FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

3	8 Show all materials making up walls, wall height, and Block size, mortar type	Yes		
3	9 Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	Yes		

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

40	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	NA	
41	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers	NA	
42	Girder type, size and spacing to load bearing walls, stem wall and/or priers	NA	
43	Attachment of joist to girder	NA	
44	Wind load requirements where applicable	NA	
45	Show required under-floor crawl space	NA	
46	Show required amount of ventilation opening for under-floor spaces	NA	
47	Show required covering of ventilation opening	NA	
48		NA	
	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges &	la la	
49	intermediate of the areas structural panel sheathing	NA	
50	Show Draftstopping, Fire caulking and Fire blocking	NA NA	
51	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	NA NA	
52	Provide live and dead load rating of floor framing systems (psf).	NA	

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

	Items to include-
GENERAL REQUIREMENTS:	Each Box shall be
APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Circled as
	Applicable

Select from Drop down 53 Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls NA 54 Fastener schedule for structural members per table FBC-R602.3.2 are to be shown NA Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural 55 NA members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or Yes rafter systems Show sizes, type, span lengths and required number of support jack studs, king studs for NA shear wall opening and girder or header per FBC-R602.7. 58 Indicate where pressure treated wood will be placed NA Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural NA panel sheathing edges & intermediate areas 60 A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail NA

FBCR: ROOF SYSTEMS:

61	Truss design drawing shall meet section FBC-R 802.10. 1 Wood trusses	Yes	
62	Include a layout and truss details, signed and sealed by Florida Professional Engineer	Yes	
	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	Yes	
	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	Yes	
	Provide dead load rating of trusses	Yes	

FBCR 802: Conventional Roof Framing Layout

66	Rafter and ridge beams sizes, span, species and spacing		
67	Connectors to wall assemblies' include assemblies' resistance to uplift rating	-	
68	Valley framing and support details	-	
69	Provide dead load rating of rafter system	-	

FBCR 803 ROOF SHEATHING

70	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	Yes	
71	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	Yes	

ROOF ASSEMBLIES FRC Chapter 9

72	Include all materials which will make up the roof assembles covering	Yes	
73	Submit Florida Product Approval numbers for each component of the roof assembles covering	Yes	

FBCR Chapter 11 Energy Efficiency Code for Residential Building

Residential construction shall comply with this code by using the following compliance methods in the FBCR Chapter 11 Residential buildings compliance methods. Two of the required forms are to be submitted, N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable	
	S_{0}	elect from	Drop Down
74	Show the insulation R value for the following areas of the structure	Yes	
75		Yes	
	Exterior wall cavity	NA	
77		NA	
н	VAC information		
	Submit two copies of a Manual J sizing equipment or equivalent computation study	Yes	
79	• · · · · · · · · · · · · · · · · · · ·	1,4	
,,	20 cfm continuous required	Yes	
80	Show clothes dryer route and total run of exhaust duct	Yes	
Ph	umbing Fixture layout shown		
	All fixtures waste water lines shall be shown on the foundationplan	Yes	
82		Yes	
	ivate Potable Water	13.	
	Pump motor horse power	Yes	
	Reservoir pressure tank gallon capacity	Yes	
85	Rating of cycle stop valve if used	NA	
El	ectrical layout shown including		
86	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	Yes	
87	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected	V	
-	by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	Yes	
88		Yes	
89		Yes	
	On the electrical plans identify the electrical service overcurrent protection device for the main	A.	
90	disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an	Yes	
90	disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3	Yes	

Notice Of Commencement:

A notice of commencement form RECORDED in the Columbia County Clerk Office is required to be filed with the Building Department BEFORE ANY INSPECTIONS can be performed.

	Items to Include-
GENERAL REQUIREMENTS:	Each Box shall be
APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Circled as
	Applicable

ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT. Select from Drop down Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. Yes There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed. 94 Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office Yes (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com Environmental Health Permit or Sewer Tap Approval A copy of a approved 95 Yes Columbia County Environmental Health (386) 758-1058 City of Lake City A City Water and/or Sewer letter. Call 386-752-2031 NA 96 Yes 97 Toilet facilities shall be provided for all construction sites Town of Fort White (386) 497-2321 If the parcel in the application for building permit is NA within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit. Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood NA elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municode.com) CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot NA Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required. A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00 NA Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. Yes If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required. 911 Address: An application for a 911 address must be applied for and received through the Columbia Yes County Emergency Management Office of 911 Addressing Department (386) 758-1125.

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, waters, canals, or lots less than ten acres in size within the county.

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	MASONITI	Three Panel Stiding, Impact	225211
B. SLIDING	MASONITO	Three Panel Stiding Impair	22401
C. SECTIONAL/ROLL UP			
D. OTHER	MASONITE	EN PAILS GARAGE ENTRY	7091.1
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	MI	Sigsle Hung, IMPACT, LOWE	21637.1
B. HORIZONTAL SLIDER	771	3,	
C. CASEMENT			
D. FIXED	MI	IMPACT LOWE	211371
E. MULLION		′	
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR			
ENVELOPE PRODUCTS			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

NOTES:	 	



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2137010 - PETER LEV RES.

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: Peter Lev Project Name: Lev Res. Model: Custom

Lot/Block: 2

Address: 468 SE Holly Terrace, N/A

City: Columbia Cty

Subdivision: Creek Run Plantation

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name: License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014

Wind Code: ASCE 7-10

Design Program: MiTek 20/20 8.2

Wind Speed: 130 mph

Truss Name

11/14/19

11/14/19

11/14/19

T14G

T15G

T16

T16G

Roof Load: 37.0 psf

Floor Load: N/A psf

This package includes 26 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

T18661395

T18661396 T18661397

T18661398

Seal#

N 12345678911123456789	Seal# T18661373 T18661374 T18661375 T18661376 T18661377 T18661379 T18661380 T18661381 T18661383 T18661384 T18661385 T18661387 T18661387 T18661388 T18661389 T18661399	Truss Name EJ01 EJ02 EJ02G PB01 PB01G T01 T01G T02 T03 T03G T04 T05 T05G T06 T07 T08 T09 T10 T11	Date 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19 11/14/19	No 23 24 25 26
18 19 20 21 22				



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Albani, Thomas

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 14,2019

Job	Truss	Truss Type	Qty	Ply	PETER LEV RES	
2137010	EJ01	Jack-Partial	16	1 1	T1	8661373
					Job Reference (optional)	

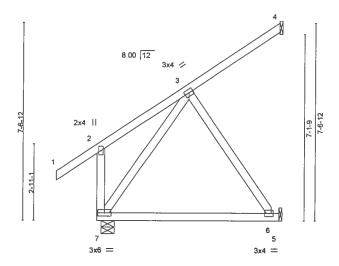
Builders FirstSource,

Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Nov 14 12:23 25 2019 Page 1 ID 8JZdTM?FFUYfvYYhdRcKEYzSpMZ-TIEdw7RCOzdymk7_093o3za98CdVKQu3IWg0VyyJGXW

1-6-8 3-5-12 6-11-8 1-6-8 3-5-12 3-5-12

Scale = 1 42



0-2,0	6-11-8	
0-2-0	6-9-8	

LOADING (psf) SPACING- 2-0-0 TCLL 20.0 Plate Grip DOL 1.25 TCDL 7.0 Lumber DOL 1.25 BCLL 0.0 Rep Stress Incr YES BCDL 10.0 Code FBC2017/TPI2014	CSI. DEFL. in (loc) Vdefl L/d TC 0.39 Vert(LL) -0.10 6-7 >847 240 BC 0.50 Vert(CT) -0.19 6-7 >416 180 WB 0.21 Horz(CT) 0.01 4 n/a n/a Matrix-MS	PLATES GRIP MT20 244/190 Weight: 46 lb FT = 20%
---	---	---

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-

TOP CHORD S

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size)

(lb/size) 4=77/Mechanical, 5=161/Mechanical, 7=352/0-6-0 Max Horz 7=221(LC 12)

Max Horz 7=221(LC 12)

Max Uplift 4=-84(LC 12), 5=-191(LC 12), 7=-12(LC 12) Max Grav 4=90(LC 19), 5=220(LC 19), 7=352(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-7=-289/248

WEBS 3-7=-304/241, 3-6=-310/320

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 7 except (jt=lb) 5=191.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 14,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	PETER LEV RES
				,	T18661375
2137010	EJ02G	GABLE	2	1	
					Job Reference (optional)
Builders FirstSource,	Jacksonville, FL - 32244,		8	240 s Jul	1 14 2019 MiTek Industries, Inc. Thu Nov 14 12 23 26 2019 Page 1
			ID 8JZdTM?FFUYfvY	YhdRcKE'	YzSpMZ-xUo07SRq9HlpOulAZsa1cB7LZc0O3vgDXAQa1OyJGXV
L	-1-6-0		8-0-0		
ı	1-6-0		8-0-0		

Scale = 1:17.6

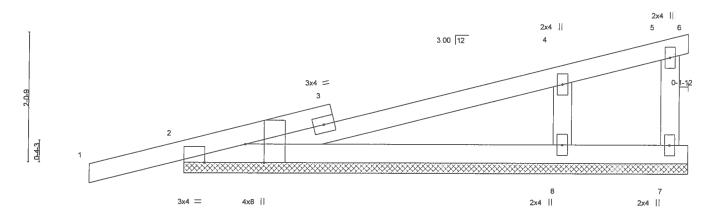


Plate Offsets (X,Y)-	2:0-3-8,Edge], [2:0-7-12,Edge]		8-0-0		
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.34 BC 0.26 WB 0.11 Matrix-S	DEFL. in (loc) I/defl Vert(LL) -0.00 1 n/r Vert(CT) 0.01 1 n/r Horz(CT) -0.01 6 n/a	L/d 120 120 n/a	PLATES GRIP MT20 244/190 Weight: 32 lb FT = 20%

 LUMBER

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

8-0-0

Structural wood sheathing directly applied or 8-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 8-0-0.

(lb) - Max Horz 2=106(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 6 except 2=-169(LC 8), 7=-149(LC 1), 8=-232(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 6, 7 except 2=269(LC 1), 8=485(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 4-8=-338/363

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 2=169, 7=149, 8=232.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 14,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and or properly incoporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ______ANSI/TP/10 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty PETER LEV RES T18661377 2137010 PB01G GABLE 2 Job Reference (optional) Jacksonville, FL - 32244 Builders FirstSource 8 240 s Jul 14 2019 MiTek Industries, Inc., Thu Nov 14 12 23 27 2019 Page 1 ID.8JZdTM?FFUYfvYYhdRcKEYzSpMZ-PhMOLoSSwbtg?2HM7a5G9Ogak0PZoNHMmq97ZqyJGXU 4x4 = Scale = 1:15.3 3 8 00 12 7-9 0-1-0 2x4 = 2x4 || 2x4 = LOADING (psf) SPACING-CSL 2-0-0 DEEL in l/def L/d **PLATES** GRIP (loc) TCLL 20.0 Plate Grip DOL 0.12 1.25 TC Vert(LL) 0.00 n/r 120 MT20 244/190 TCDL 7.0 Lumber DOL 1 25 BC. 0.07 Vert(CT) 0.00 5 n/r 120 BCLL 0.0 Rep Stress Incr YES WB 0.02 Horz(CT) 0.00 4 n/a n/a BCDL Code FBC2017/TPI2014 10.0 Matrix-P Weight: 23 lb FT = 20% LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=136/5-5-2, 4=136/5-5-2, 6=182/5-5-2

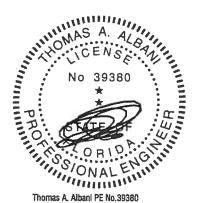
Max Horz 2=-66(LC 10)

Max Uplift 2=-72(LC 12), 4=-80(LC 13), 6=-34(LC 12)

FORCES. (lb).- Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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November 14,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, cerction and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type PETER LEV RES Ply Qty T18661379 2137010 IT01G Common Supported Gable Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8 240 s Jul 14 2019 MiTek Industries, Inc. Thu Nov 14 12 23 30 2019 Page 1 ID 8JZdTM?FFUYfvYYhdRcKEYzSpMZ-pG2WzqUKCWFFsV0xoiezm1I3bDQI?iwoSoOnA9yJGXR 11-11-8 11-11-8 11-11-8

Scale: 3/16"=1"

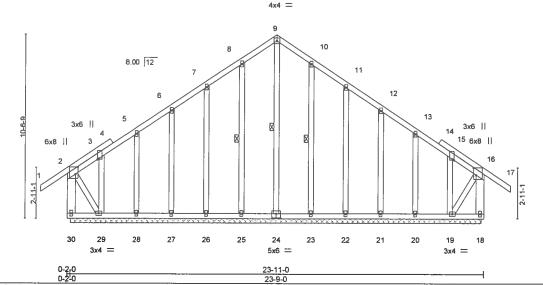


Plate Offsets (X,Y)-[2:0-5-0,0-1-12], [3:0-0-9,0-1-0], [15:0-0-9,0-1-0], [16:0-5-0,0-1-12], [24:0-3-0,0-3-0] LOADING (psf) SPACING-2-0-0 CSI. DEFL. **PLATES** in l/defl L/d GRIP (loc) TCLL 20.0 Plate Grip DOL 1.25 TC 0.21 Vert(LL) -0.01 17 120 244/190 MT20 n/r TCDL 7.0 Lumber DOL 1.25 BC 0.07 -0.02 17 Vert(CT) n/r 120 **BCLL** 0.0 * Rep Stress Incr YES WB 0.16 Horz(CT) 0.01 18 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-S FT = 20% Weight: 216 lb

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 P **WEBS**

2x6 SP No.2 *Except*

2-29,16-19; 2x4 SP No.3

OTHERS 2x4 SP No.3

REACTIONS. All bearings 23-7-0 (lb) - Max Horz 30=387(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) except 30=-295(LC 8), 18=-156(LC 9),

25=-106(LC 12), 26=-116(LC 12), 27=-112(LC 12), 28=-107(LC 12), 29=-370(LC

9), 23=-105(LC 13), 22=-117(LC 13), 21=-112(LC 13), 20=-108(LC 13),

19=-322(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 24, 25, 26, 27, 28, 23, 22, 21, 20

except 30=392(LC 9), 18=280(LC 19), 29=398(LC 10), 19=293(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-30=-379/295, 8-9=-247/293, 9-10=-247/293, 16-18=-263/157

BOT CHORD 29-30=-362/336

2-29=-354/393, 16-19=-245/290 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.,
- GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Lumber designated with a "P" is pressure-treated with preservatives. Plate lateral resistance values have been reduced 20% where used in this lumber. Plates should be protected from corrosion per the recommendation of the treatment company. Borate or other suitable treatment may be used if it does not corrode the plates. If ACQ, CBA, or CA-B treated lumber is used, improved corrosion protection is required, and G185 galvanized plates may be used with this design. Incising factors have not been considered for this design. Building designer to verify suitability of this product for its intended use.

10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

9-24, 8-25, 10-23

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

1 Row at midpt

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

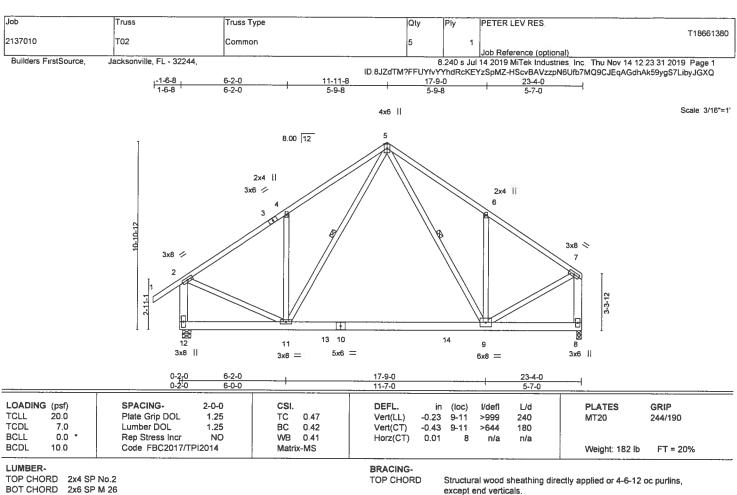
November 14,2019

Continued on page 2

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610



BOT CHORD

WEBS

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt

BOT CHORD 2x6 SP M 26

WEBS 2x4 SP No.3 *Except*

2-12,7-8: 2x6 SP No.2

(lb/size) 12=1284/0-6-0, 8=1199/0-3-8 Max Horz 12=267(LC 9)

Max Uplift 12=-500(LC 12), 8=-450(LC 13) Max Grav 12=1317(LC 19), 8=1236(LC 20)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

2-4=-1338/565, 4-5=-1414/790, 5-6=-1354/754, 6-7=-1272/527, 2-12=-1420/655, TOP CHORD

7-8=-1369/565

BOT CHORD 11-12=-278/256, 9-11=-250/802 **WEBS**

2-11=-336/1205, 4-11=-431/392, 5-11=-481/840, 5-9=-429/747, 6-9=-428/392,

7-9=-393/1203

NOTES-

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 500 lb uplift at joint 12 and 450 lb uplift
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Vert: 1-2=54, 2-5=54, 5-7=54, 11-12=-20, 9-11=-80(F=-60), 8-9=-20



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

November 14,2019

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ANSITPH Quality Criterie, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty PETER LEV RES. Ply T18661382 2137010 T03G Common Supported Gable 1 Job Reference (optional) Builders FirstSource. Jacksonville, FL - 32244, 8 240 s Jul 14 2019 MiTek Industries, Inc. Thu Nov 14 12 23 33 2019 Page 1 ID 8JZdTM?FFUYfvYYhdRcKEYzSpMZ-EqjfcsXDVRdqjzkWUqCgOfwarQS3C38F8mcRnUyJGXO 19-5-8 17,11,0 1-6-8 Scale ≈ 1.51.7 4x4 = 7 8 8 00 12 9 5 3x4 🥠 3x4 < 10 34 3x4 11 3x4 N X 6x8 || 6x8 II 12 13 2-11-1 25 24 23 22 21 20 19 16 18 17 15 14 3x6 = 3x4 = 3x4 = 0-2-0 17-11-0 17-9-0 Plate Offsets (X,Y)-[2:0-5-0,0-1-12], [12:0-5-0,0-1-12] LOADING (psf) SPACING-2-0-0 CSL DEFL. l/defl I /d **PLATES** GRIP in (loc) TCLL 20 0 Plate Grip DOL 1.25 TC 0.21 Vert(LL) -0.01 13 120 MT20 244/190 n/r TCDL 7.0 Lumber DOL 1.25 BC 0.07 Vert(CT) -0.02 13 n/r 120 **BCLL** 0.0 Rep Stress Incr WB YES 0.13 Horz(CT) 0.00 14 n/a n/a Code FBC2017/TPI2014

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2

10.0

BOT CHORD 2x4 SP No.2 P

2x6 SP No.2 *Except* WEBS

2-23,12-16; 2x4 SP No.3

OTHERS 2x4 SP No.3

TOP CHORD

WEBS

BRACING-

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals

Weight: 159 lb

FT = 20%

Rigid ceiling directly applied or 6-0-0 oc bracing. 7-19

1 Row at midpt

REACTIONS. All bearings 17-7-0.

(lb) -Max Horz 25=-325(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 14, 24 except 25=-163(LC 8), 21=-111(LC 12), 22=-116(LC 12),

Matrix-S

23=-241(LC 12), 18=-111(LC 13), 17=-116(LC 13), 16=-236(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 14, 19, 21, 22, 24, 18, 17, 16, 15 except 25=268(LC 20),

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-25=-255/147

BOT CHORD 24-25=-300/274, 23-24=-300/274

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Lumber designated with a "P" is pressure-treated with preservatives. Plate lateral resistance values have been reduced 20% where used in this lumber. Plates should be protected from corrosion per the recommendation of the treatment company. Borate or other suitable treatment may be used if it does not corrode the plates. If ACQ, CBA, or CA-B treated lumber is used, improved corrosion protection is required, and G185 galvanized plates may be used with this design. Incising factors have not been considered for this design. Building designer to verify suitability of this product for its intended use.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 24 except (jt=lb) 25=163, 21=111, 22=116, 23=241, 18=111, 17=116, 16=236.
- 12) Non Standard bearing condition. Review required.



Thomas A. Albani PE No 39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

November 14,2019

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters and individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated its to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucaliapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems. see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type Qty Ply PETER LEV RES	
2137010 T04 Common Girder 1 2 Job Reference (optional)	T18661383

Builders FirstSource,

Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Nov 14 12 23 35 2019 Page 2 ID.8JZdTM?FFUYfvYYhdRcKEYzSpMZ-ADrP0XYT12tXzHuvbFE8T4?uyE4ygpGXb45YrNyJGXM

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-54, 3-5=-54, 6-10=-20

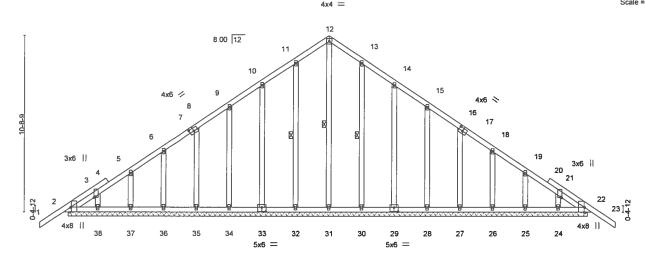
Concentrated Loads (lb)

Vert: 8=-1147(F) 11=-2509(F) 12=-1147(F) 13=-986(F) 14=-986(F) 15=-990(F)



Job PETER LEV RES Truss Truss Type Qty Ply T18661385 2137010 TO5G Common Supported Gable Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244 8 240 s Jul 14 2019 MiTek Industries, Inc. Thu Nov 14 12 23 38 2019 Page 1 ID 8JZdTM?FFUYfvYYhdRcKEYzSpMZ-aoXYfZbMKzG6qkdTGOnr5jdRSRATtJn_H2KCShyJGXJ 33-6-0 16-0-0 16-0-0

Scale = 1:67.3



32-0-0 [2:0-3-8,Edge], [3:0-0-9,0-1-0], [7:0-0-0,0-1-12], [7:0-3-0,0-2-4], [8:0-2-2,0-0-0], [16:0-2-2,0-0-0], [17:0-0-0,0-1-12], [17:0-3-0,0-2-4], [21:0-0-9,0-1-0],

Plate Offsets (X,Y)-[22:0-3-8,Edge], [29:0-3-0,0-3-0], [33:0-3-0,0-3-0] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) l/defl L/d **PLATES** GRIP Plate Grip DOL **TCLL** 20.0 1.25 TC 0.16 Vert(LL) -0.01 23 n/r 120 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 вс 0.05 Vert(CT) -0.01 23 n/r 120 BCLL 0.0 Rep Stress Incr YES WB 0.16 Horz(CT) 0.01 22 n/a n/a Code FBC2017/TPI2014 FT = 20% **BCDL** 10.0 Matrix-S Weight: 234 lb

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 P **OTHERS** 2x4 SP No.3

BRACING-

TOP CHORD **BOT CHORD** WEB\$

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing. 12-31, 11-32, 13-30

REACTIONS.

All bearings 31-7-0.

(lb) - Max Horz 2=-352(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 31, 37, 38, 24, 22 except 32=-106(LC 12), 33=-117(LC 12)

34=-111(LC 12), 35=-111(LC 12), 36=-115(LC 12), 30=-100(LC 13), 29=-119(LC 13), 28=-110(LC 13),

27=-112(LC 13), 26=-114(LC 13), 25=-101(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 32, 33, 34, 35, 36, 37, 38, 30, 29, 28, 27, 26, 25, 24, 22 except 31=271(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-312/248, 3-5=-261/234, 10-11=-214/273, 11-12=-269/321, 12-13=-269/321,

13-14=-214/255 BOT CHORD

2-38=-178/267, 37-38=-180/267, 36-37=-180/267, 35-36=-180/267, 34-35=-180/267,

33-34=-180/267, 32-33=-180/267, 31-32=-180/267, 30-31=-180/267, 29-30=-180/267, 28-29=-180/267, 27-28=-180/267, 26-27=-180/267, 25-26=-180/267, 24-25=-180/267,

22-24=-179/266

12-31=-264/179 WEBS

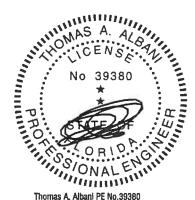
NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Lumber designated with a "P" is pressure-treated with preservatives. Plate lateral resistance values have been reduced 20% where used in this lumber. Plates should be protected from corrosion per the recommendation of the treatment company. Borate or other suitable treatment may be used if it does not corrode the plates. If ACQ, CBA, or CA-B treated lumber is used, improved corrosion protection is required, and G185 galvanized plates may be used with this design. Incising factors have not been considered for this design. Building designer to verify suitability of this product for its intended use.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

Continued on page 2

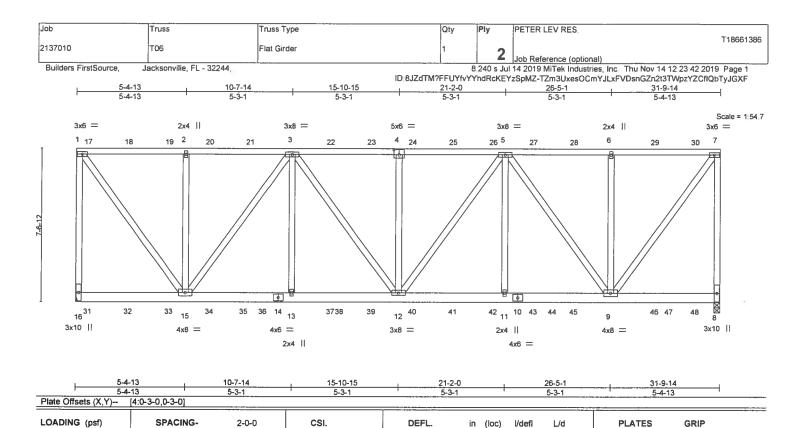
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer, Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 14,2019





LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x6 SP No.2

WEBS 2x4 SP No 3

20.0

7.0

0.0

10.0

BRACING-

Vert(LL)

Vert(CT)

Horz(CT)

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

MT20

Weight: 536 lb

244/190

FT = 20%

except end verticals

I/defl

>999

>999

n/a

(loc)

8

0.14 12-13

-0.12 12-13

-0.03

Rigid ceiling directly applied or 10-0-0 oc bracing.

L/d

240

180

n/a

REACTIONS. (lb/size) 16=2529/Mechanical, 8=2455/0-3-8 Max Uplift 16=-2512(LC 4), 8=-2426(LC 4)

Max Grav 16=2851(LC 29), 8=2757(LC 29)

Plate Grip DOL

Rep Stress Incr

Code FBC2017/TPI2014

Lumber DOL

FORCES. (lb) - Max, Comp./Max, Ten, - All forces 250 (lb) or less except when shown.

TOP CHORD 1-16=-2488/2248, 1-2=-1732/1538, 2-3=-1732/1538, 3-4=-3081/2725, 4-5=-3081/2725.

1.25

1.25

NO

TC

BC

WB 0.82

Matrix-MS

0.45

0.30

5-6-1736/1542, 6-7-1736/1542, 7-8-2465/2231 **BOT CHORD**

13-15=2423/2752, 12-13=2423/2752, 11-12=2426/2757, 9-11=2426/2757 **WEBS** 1-15=-2584/2913, 2-15=-374/354, 3-15=-1727/1500, 3-13=-475/691, 3-12=-511/558,

4-12-344/326, 5-12-506/550, 5-11-478/698, 5-9-1729/1499, 6-9-373/354,

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 16=2512, 8=2426.



Thomas A. Albani PF No 39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 14,2019

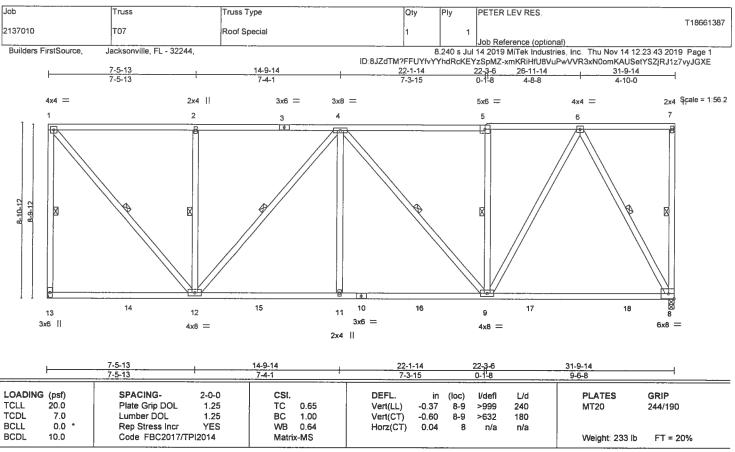
Continued on page 2

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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandra, VA 22314.





LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 **WEBS** 2x4 SP No.3 **BRACING-**

TOP CHORD

Structural wood sheathing directly applied or 5-3-15 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

WEBS

1-13, 7-8, 1-12, 2-12, 4-12, 4-9, 6-8, 5-9

REACTIONS. (lb/size) 13=1167/Mechanical, 8=1167/0-3-8

Max Horz 13=4(LC 8)

Max Uplift 13=-539(LC 8), 8=-541(LC 8) Max Grav 13=1273(LC 2), 8=1255(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

1-13=-1130/574, 1-2=-868/389, 2-4=-868/389, 4-5=-1022/441, 5-6=-1017/439

BOT CHORD 11-12=-536/1217, 9-11=-536/1217, 8-9=-273/569

1-12=-587/1309, 2-12=-431/332, 4-12=-535/219, 4-11=0/352, 4-9=-297/140, WEBS

6-9=-348/935, 6-8=-1157/569, 5-9=-340/260

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 13=539, 8=541.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 14,2019

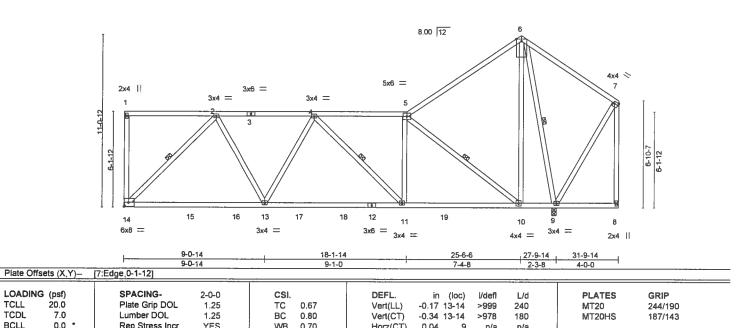
🕰 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property personal entry and property personal injury and property personal inju



Job	Truss	Truss Type		Qty	Ply	PETER LEV RES	
2137010	T09	Roof Special		1	1		T18661389
						Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,				3 240 s Jul	14 2019 MiTek Industries, Inc. Thu	Nov 14 12 23 45 2019 Page 1
			ID:8JZdTM?	FFUYfvY	hdRcKEY:	zSpMZ-t8SB7yglg787ApfqBMQUtBF	WcGNN0M60udW4CnyJGXC
	5-11-2	12-2-11	18-1-14	1	25-6-6	31-9-14	1
	5-11-2	6-3-9	5-11-2		7-4-8	6-3-8	-

8x14 MT20HS II

Scale = 1.71.2



Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

WEBS

0.04

n/a

except end verticals

1 Row at midpt

n/a

Rigid ceiling directly applied or 7-6-8 oc bracing.

Structural wood sheathing directly applied or 5-4-2 oc purlins,

Weight: 224 lb

2-14, 4-11, 5-10, 6-9

FT = 20%

LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

2x4 SP No.3 WEBS

10.0

REACTIONS.

(lb/size) 14=998/Mechanical, 9=1335/0-3-8 Max Horz 14=149(LC 9)

Rep Stress Incr

Code FBC2017/TPI2014

Max Uplift 14=-431(LC 12), 9=-520(LC 12)

Max Grav 14=1009(LC 25), 9=1335(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1087/446, 4-5=-1119/465, 5-6=-320/157

BOT CHORD 13-14=-480/811, 11-13=-607/1192, 10-11=-520/1111

WEBS 2-14=-1127/545, 2-13=-127/584, 4-13=-282/196, 5-11=-76/364, 5-10=-1200/585,

YES

WB 0.70

Matrix-MS

6-10=-315/838, 6-9=-1143/498

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. It; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=431, 9=520.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

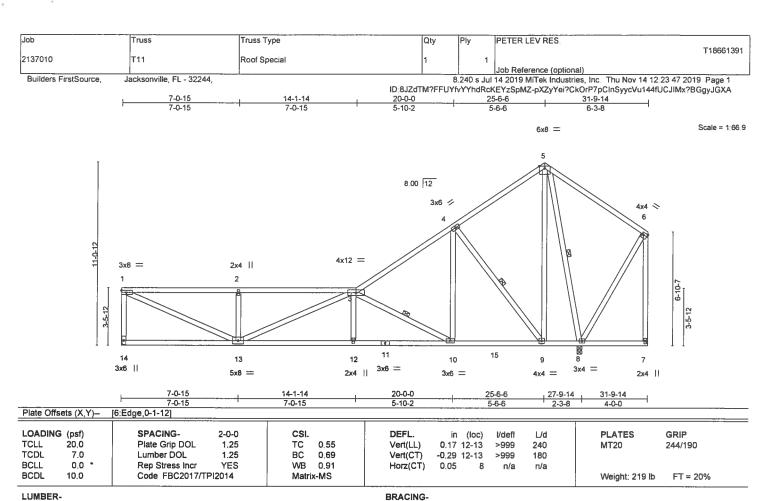
November 14,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TPH Quality Criteria, DSB-89 and BCSI Building Comp Safety Information

available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 14=998/Mechanical, 8=1335/0-3-8

Max Horz 14=254(LC 12)

Max Uplift 14=-409(LC 12), 8=-542(LC 12) Max Grav 14=1006(LC 23), 8=1335(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 1-14=942/453, 1-2=1741/739, 2-3=1741/739, 3-4=1037/383, 4-5=293/193

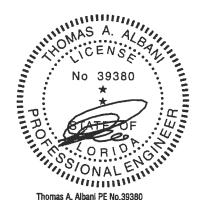
BOT CHORD 13-14=-272/190, 12-13=-1015/2189, 10-12=-1016/2184, 9-10=-425/799

1-13=-794/1872, 2-13=-432/332, 3-13=-511/121, 3-10=-1577/711, 4-10=-310/883, WEBS

4-9=-1030/576, 5-9=-410/850, 5-8=-1114/503

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|t=|b) 14=409, 8=542.



Structural wood sheathing directly applied or 3-7-7 oc purlins,

3-10, 4-9, 5-8

Rigid ceiling directly applied or 5-10-7 oc bracing.

except end verticals

1 Row at midpt

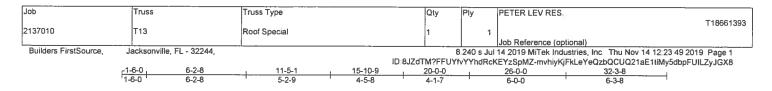
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

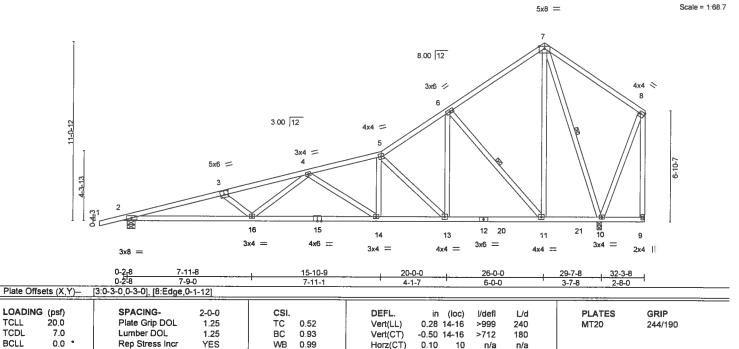
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A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTeMo connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucliapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see.

ANSITH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.







BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

10.0

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 10=1293/0-3-8, 2=1167/0-6-0

Max Horz 2=423(LC 12)

Max Uplift 10=-533(LC 12), 2=-489(LC 12)

FORCES. (lb) - Max. Comp./Max, Ten, - All forces 250 (lb) or less except when shown.

Code FBC2017/TPI2014

TOP CHORD 2-3=-3439/1609, 3-4=-3192/1454, 4-5=-2087/922, 5-6=-1297/585, 6-7=-463/279

2-16=-1842/3313, 14-16=-1431/2596, 13-14=-1028/1973, 11-13=-553/1045, BOT CHORD

10-11=-136/310

WEBS 3-16=-339/300, 4-16=-248/615, 4-14=-729/476, 5-14=-229/532, 5-13=-1298/664,

6-13=-438/1004, 6-11=-1115/649, 7-11=-444/939, 7-10=-1132/521

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MS

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=533, 2=489.



Weight: 209 lb

Structural wood sheathing directly applied or 2-7-15 oc purlins,

6-11, 7-10

Rigid ceiling directly applied or 2-2-0 oc bracing.

except end verticals.

1 Row at midpt

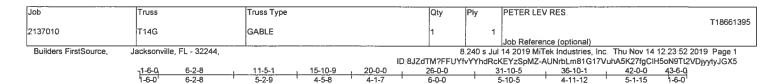
FT = 20%

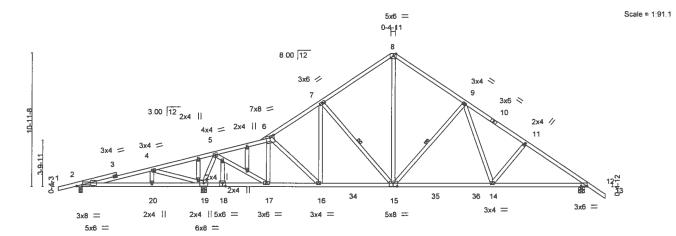
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

November 14,2019

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		0-2-8 6-2-8	10-5-8		20-0-0	26-0-0	34-2-4		41-9-8 42-6)-0
		0-2-8 6-0-0	4-3-0	0-3-0 5-2-1	4-1-7	6-0-0	8-2-4	1	7-7-4 0-2	-8
Plate Offse	ets (X,Y)-	[2:1-1-12,0-0-7], [2:0-5-12	,0-1-1], [6:0-4-	0,0-1-12], [8:0-2-9,	,Edge], [12:0-6-0	0,0-0-4], [15:0-4-0	0,0-3-0], [18:0-3-0	,0-3-0]		<u> </u>
LOADING	(psf)	SPACING-	2-0-0	CSI.	DE	EFL. in	(loc) I/defi	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.45	Ve	ert(LL) -0.16	14-15 >999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.65	Ve	ert(CT) -0.29	14-15 >999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.89	l Ho	orz(CT) 0.04	12 n/a	n/a		
BCDL	10.0	Code FBC2017/TF	PI2014	Matrix-MS					Weight: 252 lb	FT = 20%

BRACING-

WERS

TOP CHORD

BOT CHORD

LUMBER-

OTHERS REACTIONS.

TOP CHORD 2x4 SP No 2 **BOT CHORD** 2x4 SP No.2 WERS

2x4 SP No 3 2x4 SP No.3

12=1181/0-6-0, 2=276/0-3-8, 19=1813/0-5-8 (lb/size)

Max Horz 2=365(LC 11) Max Uplift 12=-465(LC 13), 2=-342(LC 8), 19=-733(LC 12) Max Grav 12=1216(LC 20), 2=301(LC 23), 19=1813(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-126/472, 4-5=-663/945, 5-6=-1091/421, 6-7=-1280/530, 7-8=-1093/564,

8-9=-1097/567, 9-11=-1593/697, 11-12=-1729/692

2-20=-436/240, 19-20=-436/240, 17-19=-337/468, 16-17=-313/1078, 15-16=-305/1143, **BOT CHORD**

14-15=-258/1122, 12-14=-447/1363

4-19-789/958, 5-17-687/1468, 6-17-672/399, 7-15-388/289, 8-15-401/888,

9-15=-598/404, 9-14=-155/462, 11-14=-301/245, 5-19=-1579/872

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=465, 2=342, 19=733.



Structural wood sheathing directly applied or 4-4-3 oc purlins.

7-15, 9-15

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt

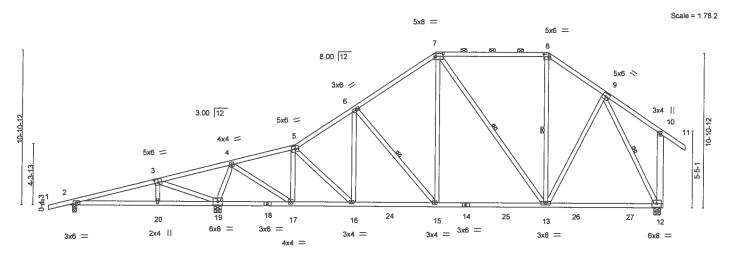
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL. Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 14,2019

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MTek® connectors. This design is based only upon parameters and property and individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Comp. Safety Information: available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss		Truss Type		Qty	Ply	PETER LEV RES			
2137010	T16		Piggyback Base		11	1				T18661397
			303				Job Reference (option	nal)		
Builders FirstSource,	Jacksonville, FL - 3	32244,					14 2019 MiTek Industr			
					ID BJZdTM?FFUYfv	YYhdRcKE	YzSpMZ-a32zDNo0KE	PiMLQImSbqHI	q9_lpdMvTUC	BxcZCyJGX2
₇ 1-6-0	6-2-8	11-5-1	, 15-10-9	20-0-0	25-9-0	4	33-9-0	37-8-8	41-11-8	43-6-0
1-6-0	6-2-8	5-2-9	4-5-8	4-1-7	5-9-0		8-0-0	3-11-8	430	1-6-B



0-2 <u>18</u> 0-2-8 Plate Offsets (X,Y)-	6-2-8 6-0-0 [3:0-3-0,0-3-0], [7:0-6-4	10-5-8 4-3-0 0-2-41 [8:0-4-4	15-10-9 5-5-1 0-2-41	20-0-0 4-1-7	25-9-0 5-9-0		33-9-0 8-0-0	41-9-8 8-0-8	41-11-8 0-2-0
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/	2-0-0 1.25 1.25 YES	CSI. TC 0 BC 0	.94 .68 .52	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.14 12-13 -0.25 12-13 0.03 12	Vdefl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 279 lb	GRIP 244/190 FT = 20%

LUMBER-

WEBS

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.3 *Except*

7-13: 2x4 SP No.2, 10-12: 2x6 SP No.2

BRACING-

WEBS

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-2-9 oc purlins, except end verticals, and 2-0-0 oc purlins (2-2-0 max.): 7-8.

Rigid ceiling directly applied or 6-0-0 oc bracing 1 Row at midpt 6-15, 7-13, 8-13, 9-12

REACTIONS. (lb/size) 19=1801/0-6-0, 12=1189/0-6-0, 2=275/0-3-8

Max Horz 2=452(LC 11)

Max Uplift 19=-739(LC 12), 12=-381(LC 13), 2=-341(LC 8) Max Grav 19=1801(LC 1), 12=1226(LC 2), 2=280(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-88/423, 3-4=-694/809, 4-5=-958/361, 5-6=-1211/566, 6-7=-1046/594, TOP CHORD

7-8=-728/514, 8-9=-834/545, 9-10=-250/262, 10-12=-319/275 BOT CHORD 2-20=-372/8, 19-20=-371/8, 17-19=-297/343, 16-17=-349/992, 15-16=-420/1089,

13-15=-314/861, 12-13=-221/497

WEBS 3-19=-779/927, 4-19=-1509/868, 4-17=-672/1359, 5-17=-674/414, 6-15=-407/281,

7-15=-171/528, 7-13=-344/164, 9-13=-183/415, 9-12=-1015/408

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 19=739, 12=381, 2=341.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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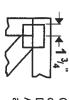
November 14,2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 10/03/2015 BEFORE USE. Design valid for use only with MTEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a runss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandra, VA 22314.

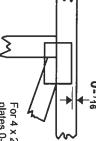


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- ¹/16" from outside edge of truss.

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This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MITek 20/20 software or upon request.

PLATE SIZE



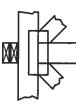
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

Industry Standards: ANSI/TPI1: National I

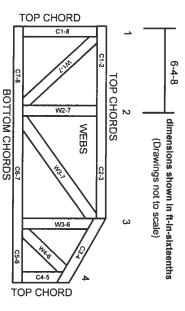
National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.

DSB-89: Desi BCSI: Build

Design Standard for Bracing.

Building Component Safety Information
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

/stem

General Safety Notes

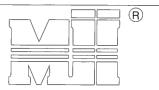
Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated fumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

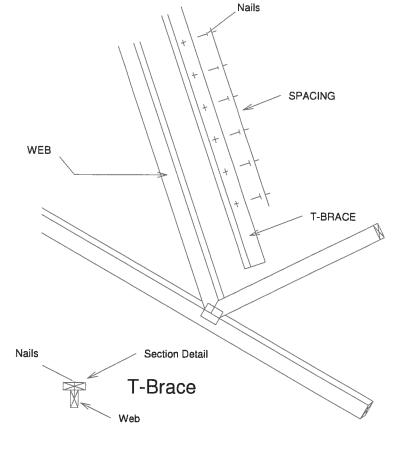
Nailing Pattern								
T-Brace size	Nail Size	Nail Spacing						
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.						

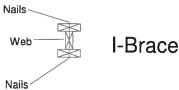
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

		Brace Size for One-Ply Truss				
	Specified Rows of La	Continuous iteral Bracing				
Web Size	1	2				
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace				
2x6	2x6 T-Brace	2x6 I-Brace				
2x8	2x8 T-Brace	2x8 I-Brace				

		e Size -Ply Truss
		Continuous iteral Bracing
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.







Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

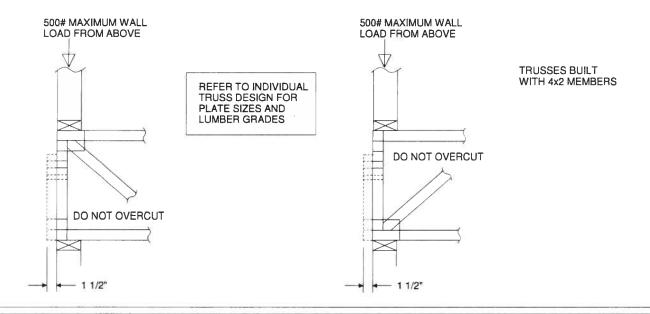
MiTek USA, Inc. Page 1 of 1

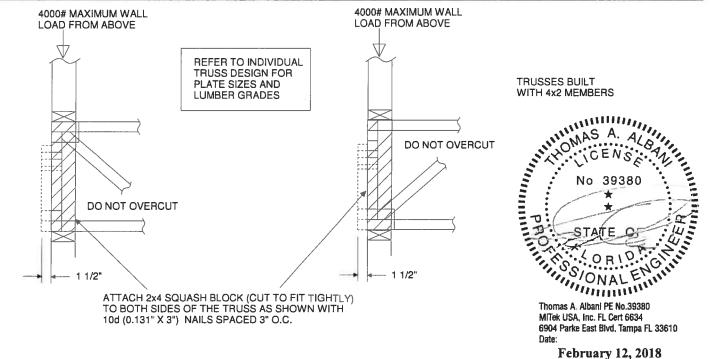


MiTek USA, Inc.

ENGINEERED BY

- 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION, THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE
 APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE
 SUCH AS TO AVOID SPLITTING OF THE WOOD.
 LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
 THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ORIENTATION ONLY.
 CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





Standard Gable End Detail

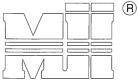
MII-GE130-SP

Page 1 of 2

10d NAILS

@ 24" o.c.

MiTek USA, Inc.



MiTek USA, Inc.

ENGINEERED BY

DIAGONAL BRACE 4'-0" O.C. MAX

Typical _x4 L-Brace Nailed To 2x_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

Varies to Common Truss

SEE INDIVIDUAL MITEK ENGINEERING

3x4 =

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

DRAWINGS FOR DESIGN CRITERIA

SHEATHING TO 2x4 STD SPF BLOCK

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD

- Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

В

NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6* O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Roof S	Sheath	ing		
24" Max	1'-3" Max.	(2) - 10d NAILS		
Diag. Brace at 1/3 points if needed		NAIL	DIAGONAL ACHED TO S AND ATT	BRACE SPA VERTICAL FACHED WITH (5) - 1

End Wall

ACED 48" O.C. WITH (4) -16d 10d NAILS.

HORIZONTAL BRACE (SEE SECTION A-A)

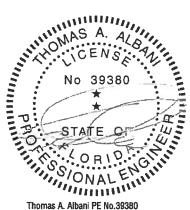
Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
and Grade			Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6		
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1		
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH

DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



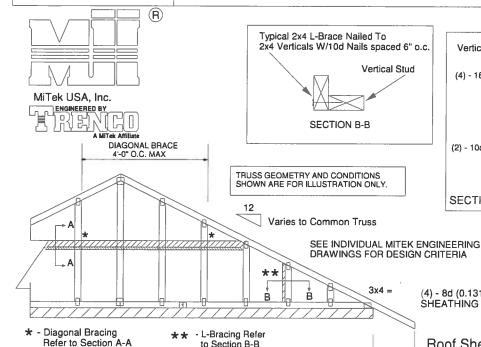
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Standard Gable End Detail

MII-GE170-D-SP



Page 1 of 2



Vertical Stud 2X6 SP OR SPF No. 2 DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails SECTION A-A 2X4 SP OR SPF No. 2

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD, SHEATHING TO 2x4 STD SPF BLOCK

(2) - 10d

NAILS

NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT. 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C BIAPHHAM AT 4-0 C.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4.

(REFER TO SECTION A-A) GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

		X	<u> </u>	Г
		1'-(Ma	O" X. ▶	
at	ag. Brad 1/3 poir needed	ce nts		
	End V	Vall	XXX	
NAL			\rightarrow	

24" Max

Roof Sheathing

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

(2) - 10d NAILS

∕Trusses @ 24" o.c.

HORIZONTAL BRACE (SEE SECTION A-A)

Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
	Maximum Stud Length						
12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4			
16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3			
24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13			
12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7			
16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5			
24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14			
	12" O.C. 16" O.C. 24" O.C. 12" O.C.	Spacing Brace 12" O.C. 3-9-7 16" O.C. 3-4-12 24" O.C. 2-9-4 12" O.C. 3-11-13 16" O.C. 3-7-7	Spacing Brace L-Brace Maximum St 12" O.C. 3-9-7 5-8-8 16" O.C. 3-4-12 4-11-15 24" O.C. 2-9-4 4-0-7 12" O.C. 3-11-13 5-8-8 16" O.C. 3-7-7 4-11-5	Stud Spacing L-Brace BRACE Maximum Stud Length 12" O.C. 3-9-7 5-8-8 6-11-1 16" O.C. 3-4-12 4-11-15 6-9-8 24" O.C. 2-9-4 4-0-7 5-6-8 12" O.C. 3-11-13 5-8-8 6-11-1 16" O.C. 3-7-7 4-11-5 6-11-1			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH

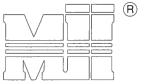
DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY 몴 5 MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10 DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

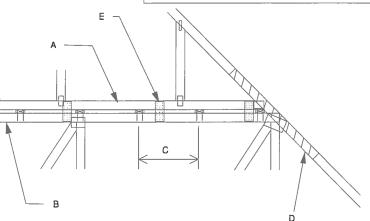
A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) (0.131" X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X ___ X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED.
ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

DIRECTIONS AIVS:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

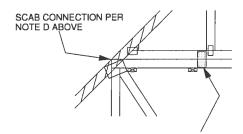
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
PIGGYBACK SPAN OF 12 ft.
FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH

MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72° O.C. W/ (4) (0.131° X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

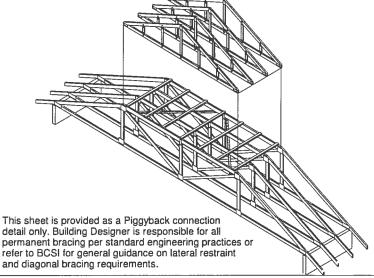


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

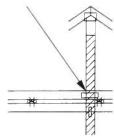
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-ON PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS
- MUST MATCH IN SIZE, GRADE, AND MUST LINE UP
 AS SHOWN IN DETAIL.

 2) ATTACH 2 x ___ x 4-0" SCAB TO EACH FACE OF
 TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
 SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
 VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
- (MINIMUM 2X4)
 THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
 CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
 BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS. FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
- NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

Page 1 of 1

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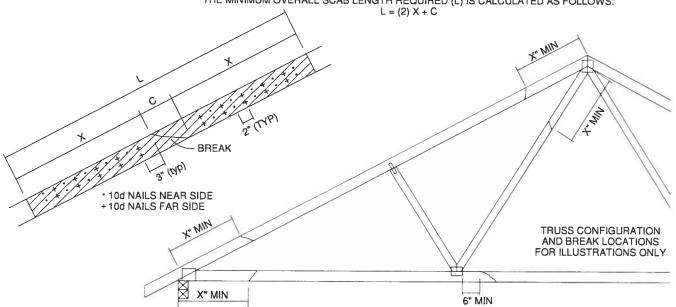
n < 몴

TOTAL NUMBER OF			MAXIMUM FORCE (ibs) 15% LOAD DURATION							
OI BITEAK	X INCHES	S	SP)F	s	PF	ŀ	IF.	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES

- 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR
 AND HELD IN PLACE DURING APPLICATION OF REPAIR.

- 3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

 4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.

 6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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January 19, 2018

TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

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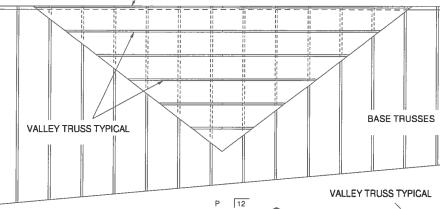
MiTek USA, Inc.

A MITCH APPLICATION

GABLE END, COMMON TRUSS OR GIRDER TRUSS

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3")
- 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
- INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



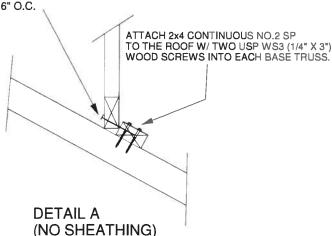
VALLEY TRUSS TYPICAL

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SEE DETAIL
A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.

N.T.S.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C
WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

No 39380

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Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 19, 2018

MiTek USA, Inc.

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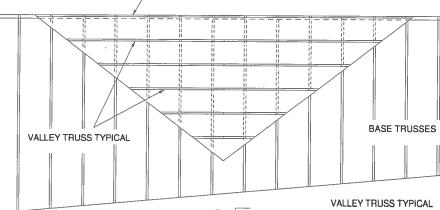
MiTek USA, Inc.

ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

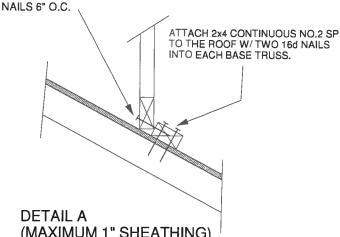
GENERAL SPECIFICATIONS

- 1. NAIL SIZE 16d (0.131" X 3.5")
 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.



GABLE END, COMMON TRUSS P 12 OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 16d



DETAIL A (MAXIMUM 1" SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET MAX MEAN HOOF REIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE : 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF

ON THE TRUSSES

No 39380

STATE OF ALEMAN

Thomas A. Albani PE No.39380

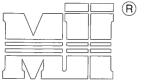
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Standard Gable End Detail

MII-GE146-001

MiTek USA, Inc.

Page 1 of 2



MiTek USA, Inc.

ENGINEERED BY

DIAGONAL BRACE 4'-0" O.C. MAX

Typical _x4 L-Brace Nailed To 2x_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

Varies to Common Truss

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

Roof Sheathing

24" Max

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 =- Diagonal Bracing - L-Bracing Refer Refer to Section A-A to Section B-B

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES:

2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

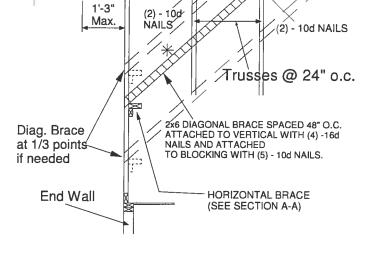
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
and Grade		Maxin	num Stud L	ength	
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 !-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS,





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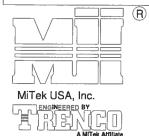
January 19, 2018

LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

MiTek USA, Inc.

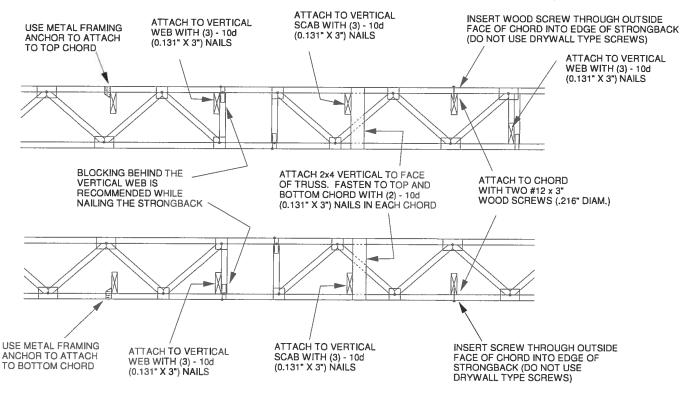
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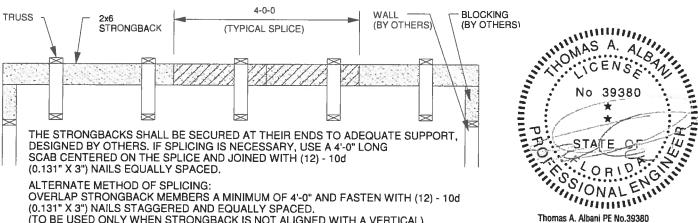


TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

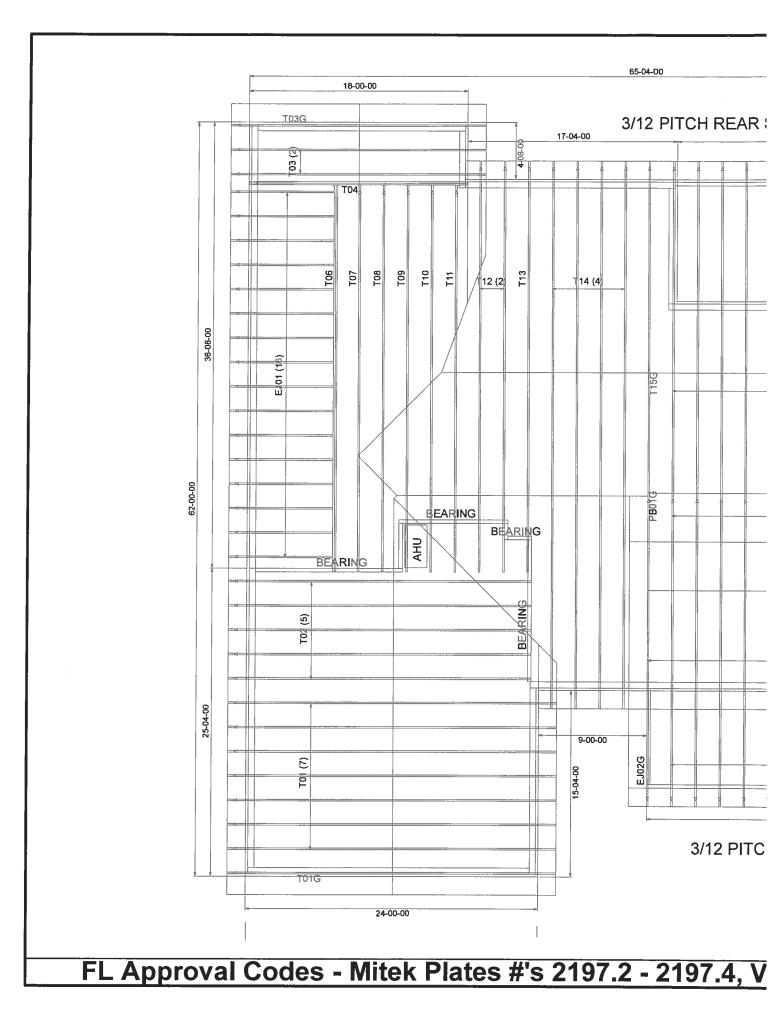
NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END. CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.

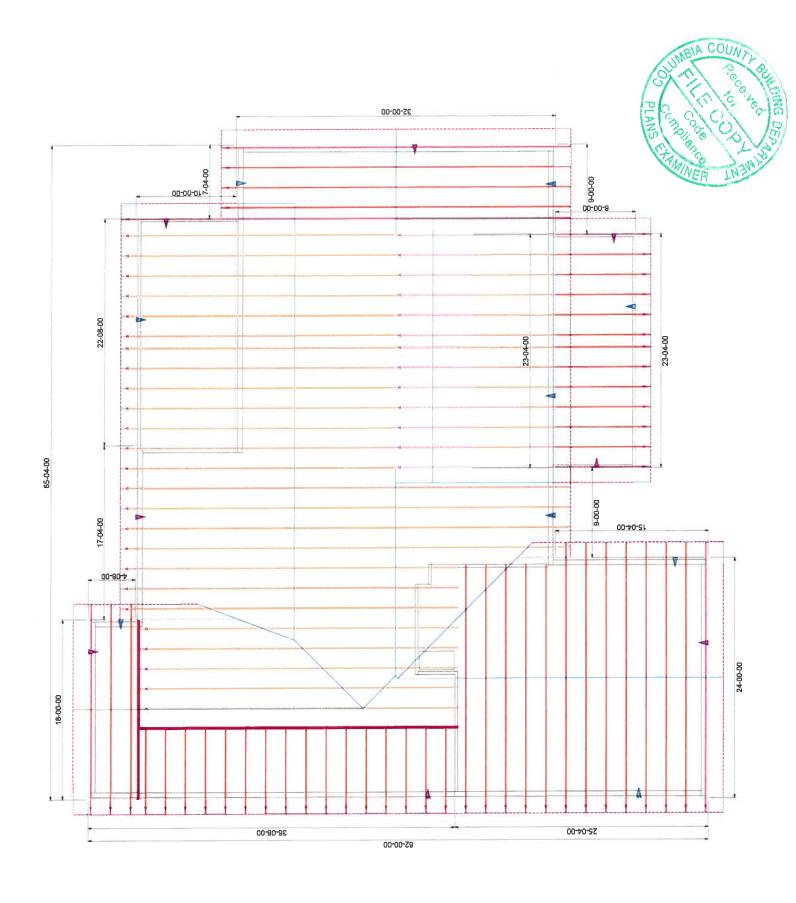




(0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED. (TO BE USÉD ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

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Residential System Sizing Calculation

Summary Project Title:

Peter & Anna Lev

Lev Residence

Lake City, FL 32055

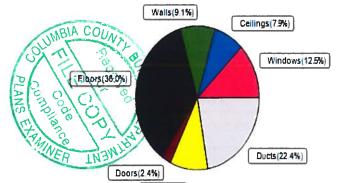
9/11/2019

Location for weather data: Gainesville, FL - Defaults: Latitude(29.7) Altitude(152 ft.) Temp Range(M)							
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)							
Winter design temperature(TMY3 99%) 30 F Summer design temperature(TMY3 99%) 94 F					F		
Winter setpoint	70	F	Summer setpoint	75	F		
Winter temperature difference	40	F	Summer temperature difference	19	F		
Total heating load calculation	30393	Btuh	Total cooling load calculation	22584	Btuh		
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh		
Total (Electric Heat Pump)	100.0	30393	Sensible (SHR = 0.70)	90.0	15809		
Heat Pump + Auxiliary(0.0kW)	100.0	30393	Latent	135.2	6775		
			Total (Electric Heat Pump)	100.0	22584		

WINTER CALCULATIONS

Winter Heating Load (for 2266 sqft)

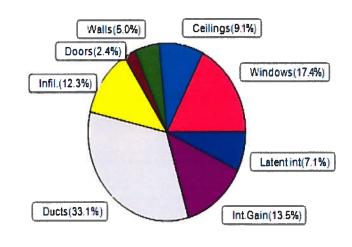
Load component			Load	
Window total	263	sqft	3792	Btuh
Wall total	1812	sqft	2776	Btuh
Door total	40	sqft	736	Btuh
Ceiling total	2379	sqft	2415	Btuh
Floor total	2266	sqft	10950	Btuh
Infiltration	67	cfm	2928	Btuh
Duct loss			6795	Btuh
Subtotal			30393	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			30393	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 2266 sqft)

Load component			Load	
Window total	263	sqft	3940	Btuh
Wall total	1812	sqft	1139	Btuh
Door total	40	sqft	552	Btuh
Ceiling total	2379	sqft	2053	Btuh
Floor total			0	Btuh
Infiltration	50	cfm	1043	Btuh
Internal gain			3040	Btuh
Duct gain			5807	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Blower Load			0	Btuh
Total sensible gain			17574	Btuh
Latent gain(ducts)			1680	Btuh
Latent gain(infiltration)			1731	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1600	Btuh
Total latent gain			5010	Btuh
TOTAL HEAT GAIN			22584	Btuh





EnergyGauge® S	ystem S	izin			
PREPARED BY:		17		١	L
DATE:	9	/11	1001		
					Г

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Peter & Anna Lev

Project Title: Lev Residence Building Type: User

Lake City, FL 32055

9/11/2019

Reference City: Gainesville, FL (Defaults) Winter Temperature Difference: 40.0 F (TMY3 99%)

Component Loads for Whole House

Window	Panes/Type	Frame U	Orientation	Area(sqft) X	HTM=	Load
1	2, NFRC 0.25	Vinyl 0.36	S	15.0	14.4	216 Btuh
2	2, NFRC 0.25	Vinyl 0.36		30.0	14.4	432 Btuh
2 3	2, NFRC 0.25	Vinyl 0.36	S	13.3	14.4	192 Btuh
4	2, NFRC 0.25	Vinyl 0.36	S S S	15.0	14.4	216 Btuh
5	2, NFRC 0.25	Vinyl 0.36	E	4.0	14.4	58 Btuh
6	2, NFRC 0.25	Vinyl 0.36	N	15.0	14.4	216 Btuh
7	2, NFRC 0.25	Vinyl 0.36	N	15.0	14.4	216 Btuh
8	2, NFRC 0.25	Vinyl 0.36	N	60.0	14.4	864 Btuh
9	2, NFRC 0.25	Vinyl 0.36	N	30.0	14.4	432 Btuh
10	2, NFRC 0.25	Vinyl 0.36	N	30.0	14.4	432 Btuh
11	2, NFRC 0.25	Vinyl 0.36	W	20.0	14.4	288 Btuh
12	2, NFRC 0.25	Vinyl 0.36	W	16.0	14.4	230 Btuh
	Window Total			263.3(sqft)		3792 Btuh
Walls	Туре	Ornt. Ueff.	R-Value	Area X	HTM=	Load
			(Cav/Sh)			
1	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	69	1.10	76 Btuh
2	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	154	1.10	169 Btuh
3	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	69	1.10	76 Btuh
4	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	295	1.10	323 Btuh
5	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	53	1.10	59 Btuh
6	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	137	1.10	150 Btuh
7	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	93	1.10	102 Btuh
8	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	132	1.10	144 Btuh
9	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	44	1.10	48 Btuh
10	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	306	1.10	335 Btuh
11	Frame - Wood	- Adj (0.089)	13.0/0.0	204	3.55	724 Btuh
12	Frame - Wood	- Adj (0.089)	13.0/0.0	118	3.55	420 Btuh
13	Ins. Conc. Form	- Ext (0.027)	30.0/0.0	138	1.10	151 Btuh
	Wall Total			1812(sqft)		2776 Btuh
Doors	Туре	Storm Ueff.		Area X	HTM=	Load
1	Insulated - Exterio			20	18.4	368 Btuh
2	Insulated - Garag	e, n (0.460)		20	18.4	368 Btuh
	Door Total			40(sqft)		736Btuh
Ceilings	Type/Color/Surface		R-Value	Area X	HTM=	Load
1	Vented Attic/L/Sh	ing (0.025)	38.0/0.0	2379	1.0	2415 Btuh
	Ceiling Total			2379(sqft)		2415Btuh
Floors	Туре	Ueff.	R-Value	Size X	HTM=	Load
1	Slab On Grade	(1.180	0.0	232.0 ft(per	im.) 47.2	10950 Btuh
	Floor Total			2266 sqft		10950 Btuh
			ı	Envelope Subto	otal:	20669 Btuh
			·	<u>.</u>		

Manual J Winter Calculations

Residential Load - Component Details (continued) Project Title:

Peter & Anna Lev

Lake City, FL 32055

Project Title: Lev Residence Building Type: User

9/11/2019

Infiltration	Type Natural	Wholehouse ACH 0.19	Volume(cuft) 21142	Wall Ratio 1.00	CFM= 66.9	2928 Btuh
Duct load	Average seale	ed, R6.0, Supply(Att), Re	eturn(Att)	(DLM of	0.288)	6795 Btuh
All Zones		•	Sensible Subt	total All Zor	ies	30393 Btuh

WHOLE HOUSE TOTALS

Totals for Heating Subtotal Sensible Heat Loss Ventilation Sensible Heat Loss Total Heat Loss 30393 Btul 0 Btul 30393 Btul
--

EQUIPMENT

1. Electric Heat Pump	#	30393 Btuh

Key: Window types - NFRC (Requires U-Factor and Shading coefficient(SHGC) of glass as numerical values) or - Glass as 'Clear' or 'Tint' (Uses U-Factor and SHGC defaults)
U - (Window U-Factor)

HTM - (ManualJ Heat Transfer Multiplier)



Version 8

System Sizing Calculations - Summer

Residential Load - Whole House Component Details
Project Title:

Peter & Anna Lev

Lev Residence

Lake City, FL 32055

9/11/2019

Reference City: Gainesville, FL

Temperature Difference: 19.0F(TMY3 99%) Humidity difference: 51gr.

Component Loads for Whole House

		Туре	e*			Over	hang	Wind	ow Are	a(sqft)	Н	TM	Load	
Window	Panes	SHGC U		IS	Ornt	Len	Hgt	Gross		Unshaded	Shaded	Unshaded		
1	2 NFRC	0.25, 0.36	No	No	S	1.5ft.	2.0ft.	15.0	15.0	0.0	12	14	181	Btuh
2		0.25, 0.36	No	No	S	9.5ft.	2.0ft.	30.0	30.0	0.0	12	14	363	Btuh
3	2 NFRC	0.25, 0.36	No	No	S	9.5ft.	2.0ft.	13.3	13.3	0.0	12	14	161	Btuh
4	2 NFRC	0.25, 0.36	No	No	S	1.5ft.	2.0ft.	15.0	15.0	0.0	12	14	181	Btuh
5	2 NFRC	0.25, 0.36	No	No	Ε	1.0ft.	8.0ft.	4.0	0.0	4.0	12	31	124	Btuh
6	2 NFRC	0.25, 0.36	No	No	N	1.5ft.	2.0ft.	15.0	0.0	15.0	12	12	181	Btuł
7		0.25, 0.36	No	No	N	11.5f	2.0ft.	15.0	0.0	15.0	12	12	181	Btuh
8	2 NFRC	0.25, 0.36	No	No	N	11.5f	2.0ft.	60.0	0.0	60.0	12	12	726	Btuh
9	2 NFRC	0.25, 0.36	No	No	N	1.5ft.	2.0ft.	30.0	0.0	30.0	12	12	363	Btuh
10	2 NFRC	0.25, 0.36	No	No	N	1.5ft.	4.5ft.	30.0	0.0	30.0	12	12	363	Btuh
11	2 NFRC	0.25, 0.36	No	No	W	1.5ft.	4.5ft.	20.0	0.0	20.0	12	31	619	Btuh
12	2 NFRC	0.25, 0.36	No	No	W	1.5ft.	4.5ft.	16.0	0.0	16.0	12	31	495	Btuh
	Windov	w Total						263 (s	qft)				3940	Btuh
Walls	Type				U	-Value	- R-\	/alue	Area	a(sqft)		НТМ	Load	
	, ,						Cay/S	heath		` ' '				
1	Insulated	d Concrete I	Form-	Ext	-	0.03	30.0		6	9.0		0.4	28	Btuh
2		Concrete I				0.03	30.0		_	54.4		0.4	62	Btuh
3		d Concrete I				0.03	30.0			9.0		0.4	28	Btuh
4		Concrete I				0.03	30.0			94.7		0.4	118	Btuh
5	Insulated	Concrete I	Form-	Ext		0.03	30.0		5	3.4		0.4	21	Btuh
6	Insulated	Concrete I	Form-	Ext		0.03	30.0		13	36.6		0.4	55	Btuh
7	Insulated	d Concrete i	Form-	Ext	(0.03	30.0	/0.0	9	3.3		0.4	37	Btuh
8	Insulated	d Concrete I	Form- I	Ext	(0.03	30.0	/0.0	13	31.8		0.4	53	Btuh
9	Insulated	d Concrete i	Form- I	Ext	(0.03	30.0	/0.0	4	3.6		0.4	17	Btuh
10	Insulated	d Concrete F	Form- i	Ext	(0.03	30.0	VO.0	30	06.2		0.4	122	Btuh
11	Frame - 1	Wood - Adj			(0.09	13.0	/0.0	20	04.0		1.7	344	Btuh
12	Frame - 1	Wood - Adj			(0.09	13.0	/0.0	11	18.2		1.7	199	Btuh
13	Insulated	d Concrete F	Form- I	Ext	(0.03	30.0	VO.0	13	38.0		0.4	55	Btuh
	Wall To	otal							18	12 (sqft)			1139	Btuh
Doors	Туре								Area	(sqft)		НТМ	Load	
1	Insulated	d - Exterior							2	0.0		13.8	276	Btuh
2	Insulated	d - Garage							2	0.0		13.8	276	Btuh
	Door To	otal							4	40 (sqft)			552	Btuh
Ceilings	Type/C	Color/Surf	ace		U	-Value)	R-Value	Area	a(sqft)		НТМ	Load	
1	Vented A	Attic/Light/Sh	ninale/F	₹В		0.025	;	38.0/0.0	23	79.0		0.86	2053	Btuh
	Ceiling	_							23	79 (sqft)			2053	
Floors	Туре						R-V	/alue		ize		НТМ	Load	
1	Slab On	Grade						0.0	22	266 (ft-perin	neter)	0.0	0	Btuh
-	Floor T									i.0 (sqft)	,		_	Btuh
	1 1001 1	Olai							2200	.o (sqit)			U	Dial
									E	nvelope	Subtota	I:	7684	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title: Climate:FL_GAINESVILLE_REGIONAL_A
Lev Residence

Peter & Anna Lev

Lake City, FL 32055

9/11/2019

Infiltration		Average ACH		. ,	Vall Ratio	CFM=	Load	
	Natural	0.14	2	1142	1	50.1	1043	Btu
Internal		Occupants	Btu	ıh/occu	ıpant	Appliance	Load	
gain			X	230	+	1200	3040	Btu
				Sens	sible Envel	ope Load:	11767	Btuł
Duct load	Average sealed,Supp	ly(R6.0-Attic), Return(R6.0-Attic)			(DGM of	0.494)	5807	Btu
				Sensil	ble Load <i>A</i>	All Zones	17574	Btuł

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title: Climate:FL_GAINESVILLE_REGIONAL_A

Peter & Anna Lev

Lev Residence

Lake City, FL 32055

9/11/2019

WHOLE HOUSE TOTALS

	William Control of the Control of th		
5	Sensible Envelope Load All Zones	11767	Btuh
	Sensible Duct Load	5807	Btuh
	Total Sensible Zone Loads	17574	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	17574	Btuh
Totals for Cooling	Latent infiltration gain (for 51 gr. humidity difference)	1731	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1680	Btuh
	Latent occupant gain (8.0 people @ 200 Btuh per person)	1600	Btuh
	Latent other gain	0	Btuh
	Latent total gain	5010	Btuh
	TOTAL GAIN	22584	Btuh

EQUIPMENT		
1. Central Unit	#	22584 Btuh

*Key: Window types (Panes - Number and type of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value)

(U - Window U-Factor)

(InSh - Interior shading device: none(No), Blinds(B), Draperies(D) or Roller Shades(R))

- For Blinds: Assume medium color, half closed For Draperies: Assume medium weave, half closed

For Roller shades: Assume translucent, half closed

(IS - Insect screen: none(N), Full(F) or Half(1/2))

(Ornt - compass orientation)



Version 8

FORM R405-2017

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Business and Professional Regulation - Residential Performance Method

Project Name Lev Residence Street City State Zip Lake City FL : 320: Owner Peter & Anna Lev Design Location FL Gainesville	55	Builder Name Permit Office Columbia County Permit Number Jurisdiction County Columbia (Florida Chimat	eZone 2 ;
1 New construction or existing 2 Single family or multiple family 3 Number of units, it multiple family 4. Number of Bedrooms 5 Is this a worst case? 6 Conditioned floor area above grade (ft²) Conditioned floor area below grade (ft²) 7 Windows (263.3 sqft.) Description a U-Factor Obl. U=0.36 SHGC SHGC=0.25 b U-Factor N/A SHGC c U-Factor N/A SHGC d U-Factor N/A SHGC d U-Factor N/A SHGC 8 Floor Types (2266.0 sqft.) a Stab-On-Grade Edge Insulation b N/A c. N/A	New (From Plans) Single-family 1 4 No. 2266 0 Area 263.33 ft f	a Central Unit 13. Heating systems a Electric Heat Pump 14. Hot water systems	Insulation Area R=30 0 1773 30 ft² R=13 0 342 22 ft² R= ft² R= ft² Insulation Area R=38 0 2379 00 ft² R= ft² R ft² R ft² 6 566 CQByftr Efficiency 22 A SEER:14 00 GBruthr Efficiency 22 A SEER:14 00 GBruthr Efficiency CV Pstat
Glass/Floor Area: 0.116	Total Proposed Modified Total Baseline i		PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code

PREPARED BY: DATE

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT DATE

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553,908 Flonda Statutes

BUILDING OFFICIAL DATE



⁻ Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with R403.3.2.1.

⁻ Compliance requires an Air Barrier and Insulation Inspection Checklist in accordance with R402.4.1.1 and this project requires an envelope leakage test report with envelope leakage no greater than 5.00 ACH50 (R402.4.1.2).

ORM R405-2017

INPUT SUMMARY CHECKLIST REPORT

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Talig Suite of Evan Office Value = 30 trap Builder Value Ferna (III) e Anisanding Carnin Type Movies and ng (I) mmedi	Common type Society & Anna Society & Anna Society & Anna New Anna type Society & Anna Society & Anna		Bedroom Linuthor Jobel Sto Worst Go Foste Ar Prose Va Whole Ho					AS De la Buda		v. Hildemanica san Pyranga sharing kashin 12066	
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				CEILING							
<i>E</i>	Ceiling Type Under Attics Vente		pare tair	R-Value	ns Type Double Batt		real 79 ft-	Framing 1831		Truss Type	-

FORM R405-2017 INPUT SUMMARY CHECKLIST REPORT

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. INPUT SUMMARY CHECKLIST REPORT ORM R405-2017 **GARAGE INFILTRATION HEATING SYSTEM** BOL for early meat Fund 57521 COOLING SYSTEM Suptype Efficiency in cit. SEER IN IN Cast HOT WATER SYSTEM S.MT.:* 1.316 4 - 11 4 4 artje SOLAR HOT WATER SYSTEM Let F CommoNine System Model # Note: Note:

DUCTS

Leakage Type

Default eakaga

Air CEM 25 CEM25

Garage Default ci Default is

Hearter (107 - 500) - QV+ - RLF - Hear - Cook

--- Heturn

Eccation Area

Atti0 153/3 ff

Located R-Salve Area

40AC #

FORM R405-2017 INPUT SUMMARY CHECKLIST REPORT

						TEM	PERATUR	RES					
KI MIT THE REAL PROPERTY.	emasi				Ü	alimy - arm							
meating			A Mar						(A) Auto Auto Auto			I I-No. (A) Nerv (X) Ne.	
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ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD ESTIMATED ENERGY PERFORMANCE INDEX* = 83

The lower the Energy Performance Index, the more efficient the home.

* New home or addition	New (From Plans)	12. Ducts, location & insulation level
2. Single family or multiple-family	Single-famey	a) Supply ducts R 6.0 b) Return ducts R 6.0
3. No lat units (if multiple-family)	3 *	ci AHU location Garage
4 Number of bedrooms.	4 4	13 Cooling system Gapacay 22 6
files this a worst chaeff (yes no.	5 No	a: Split system SEER bi Single package SLER
6. Conditioned floor area (sq. ft.)	6 2266	cl Ground/water source SEER/COP_ c) Room unit/PTAC EER
Windows Type and area ar U-factor (weighted average) by Solar Heat Gain Coefficient (SHGC) or Area	7a <u>0.360</u> 7b <u>0.250</u> 7b <u>263.3</u>	14 D 14 Heating system Capacity 30 4 a) Split system heat pump HSPF
8 Skylights a 10-factor (weighted average) b Splar Heat Gain Coefficient (SHGC)	8a NA 8b NA	b) Single prickage heat pump HSPF c) Electric resistance — COP d) Gas furnace natural gas — AFUE e) Gas furnace LPG — AFUE f) Other — 8 20
9 Floor type insulation level		
a) Slap-on-grade (R-value) b) Wood, raised (R-value)	9a 00_	
c) Concrete Taised (R-Value)	90	15. Water neating system
16 Wall type and insulation A Exterior S Wood frame (Insulation R-value) B Masonry (Insulation R-value) B Adjacent 1 Wood frame (Insulation R-value) 2 Masonry (Insulation R-value)	10A1 10A2 30 C 1081 13 0 1082	a) Electric resistance EF 0.92 b) Gas fired inatural gas EF c) Gas fired LPG EF c) Solar system with tank EF e) Dedicated heat pump with tank EF f) rieat recovery unit HeatRec% g) Other
11 Ceiling type and insulation level		15 HVAC credits claimed (Performance Method)
a. Under attic b. Single assembly c. Knee walls/skylight walls d. Radiant barrier installed	11a _ 38.0 11b 11c 11dYes	a) Ceiling fans b) Cross ventilation Yes c) Whole nause fan Ne d) Multizone cooling credit e) Multizone heating credit f) Programmable thermostal Yes
* and required to Section 0.303 to the court	lands Ulidana Cara C	
Lanel required by Section R303-1-3 of the F Leertify that this home has complied with the saving features which will be installed for exc display card will be completed based on installed	Florida Building Code End leeded) in this home before	ergy Conservation, through the above energy e final inspection. Otherwise, a new FPI
Builder Signature	all solutions of	Date
Address of New Horne		Ony/Fi Zip Lake <u>Ony, Ft. 32055</u>

Envelope Leakage Test Report (Blower Door Test)

Residential Prescriptive, Performance or ERI Method Compliance 2017 Florida Building Code, Energy Conservation, 6th Edition

Jurisdiction	Perm	nt #
Job Information		
Sulder	Community	Lot 3
Address	and the second s	Address of the second s
City Lake City	State FL	Zip 32055
Air Leakage Test Result	S Passing results must meet either the F	Performance, Prescriptive, or ERI Method
PRESCRIPTIVE METHOD-The changes per hour at a pressure	building or dwelling unit shall be tested and verific of 0.2 inch w.g. (50 Pascals) in Climate Zones 1 a	ed as having an air leakage rate of not exceeding 7 air and 2.
	HOD-The building or dwelling unit shall be tested as on Form R405-2017 (Performance) or R406-2017 fied on Form R405-2017-Energy Calc (Performance)	nd verified as having an air leakage rate of not exceeding 7 (ERI), section labeled as infiltration, sub-section ACH50, section R406-2017 (ERI): 5 000
× 50 - 211 CFM(50) — Build	42 = Ing Volume ACH(50)	Method for calculating building volume
PASS	MGTROOT	Retrieved from architectural plans
PASS		Code software calculated
When ACH(50) is less to mast be verified by built	han 3. Mechanical Veribiation installation ding department	Field measured and calculated
1481 105(2), (*) In the or an approved opposed to the rate of an approved opposed to the rate official Testing shall buring testing. Exterior windows and dixors freplace opposed to the rate windows and dixors freplace opposed to the approximation of the approximation of the time of the approximation o	inviolate as derified in Section 553,993(5) or "Th/or third party." A written report of the results of the test the performed ar any time after creation of a riperior that sealed if and stove opens shall be closed, but not sealed if takeup air, back draft and flue dampers shall be called any time.	reyand the intended weatherstopping or other infiltration used but not sealed buyong intended infiltration control
Testing Company		
charact vents marine amise Will F	eakage results are in accordance with the 2 saccording to the compliance method selec	Phone
Signature of Tester		Date of Test
Printed Name of Tester		
icense/Certification #	Issuing A	uthority