A	1/2008			uilding Permit on Premises During Co		PERMIT 000027163
APPLICANT	JACOB K		e i i ominenti y i osted	PHONE	344-4817	00002/103
ADDRESS	484	NW TURNER AVE	NUE	LAKE CITY	344-4617	FL 32055
OWNER	-	AST DEVELOPERS	-	PHONE	755-2082	
ADDRESS	226	SW MORNING GLO	ORY DR	LAKE CITY	-	FL 32024
CONTRACTO		OB KIRSCH	-	PHONE	344-4817	
LOCATION O	F PROPER	TY 90W, TL C	ON 247S, TL CALLAHA	AN,TL HOPE HENRY,	TR MORNING	
		GLORY, 6	TH LOT ON RIGHT			
TYPE DEVEL	OPMENT	SFD,UTILITY	ES	ΓΙΜΑΤΕD COST OF C	ONSTRUCTION	118800.00
HEATED FLO	OR AREA	1700.00	TOTAL ARE	EA 2376.00	HEIGHT _	STORIES 1
FOUNDATION	N CONC	WALI	LS FRAMED F	ROOF PITCH 8/12	FI	OOR SLAB
LAND USE &	ZONING	RSF-2		MA	X. HEIGHT	21
Minimum Set l	Back Requir	ments: STREET-	FRONT 25.00	REAR	15.00	SIDE 10.00
NO. EX.D.U.	0	FLOOD ZONE	X PP	DEVELOPMENT PER	RMIT NO.	
PARCEL ID	15-4S-16-	03023-554	SUBDIVISIO	N ROLLING MEAI	oows	
LOT 54	BLOCK	PHASE	UNIT	TOT	AL ACRES 0	.51
000001635	×		CBC1253775			
Culvert Permit	No.	Culvert Waiver C	ontractor's License Nun	nber	Applicant/Owner	-/Contractor
CULVERT		08-0053	BK		JH	Y
Driveway Con	nection	Septic Tank Number	LU & Zonii	ng checked by Ap	proved for Issuan	ce New Resident
COMMENTS:	PLAT RE	QUIRES MFE AT 106	FT., ELEVATION CO	NFIRMATION LETTE	R	
REQUIRED A	TCLAD					
	I SLAB					
	I SLAB				Check # or C	ash 12110
	I SLAB	FOR BU	IILDING & ZONIN	IG DEPARTMEN		
Temporary Pov		FOR BU	F	IG DEPARTMEN	TONLY	(footer/Slab)
		FOR BU	F		TONLY	
	ver	date/app. by	Foundation Slab	date/app. by	T ONLY  Monolithic	(footer/Slab)  date/app. by /Nailing
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Temporary Pov Under slab rou Framing Electrical roug Permanent pow	yergh-in plumb date/apgh-in	date/app. by singdate/ap	Foundation Slab  p. by  Rough-in plumbing at  Heat & Air Duct  C.O. Final	date/app. by  date/app. by  bove slab and below woo  date/app. by  date/app. by	Monolithic _ Sheathing  od floor  Peri. beam (Linte	(footer/Slab)  date/app. by  /Nailing date/app. by  date/app. by
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PERMIT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

# Columbia County Building Permit Application

	g . orimic Application
For Office Use Only Application # 080 -55 Date	Received 1-11-08 By UH Permit # 1635/,27163
Daily Control of the state of t	to 18 OLO 8 Diana Francisco de Carto
Development Permit /V/A 700	ing K ) F = L :
Comments Plat Requires MFE of 106 St. Eleva	tion Confirmation Letter Consider Confirmation Letter Confirmation Letter Confirmation Letter Confirmation Letter Confirmation Letter Confirmation Confirmation Letter Confirmation Confirmation Letter Confirmation
NOC DEH Deed or PA Site Plan OS	itate Road Info   Parent Parcel #   Development Permi
	Fax 386 75 9 - 5047
Name Authorized Person Signing Permit Jacob	1156h Phone 386-344-4817
Address 10 100 101161 due #1101	1 Ct 1111 [] 3305-5-
Owners Name DUTHEUST DEVELOPERS	201 700 3000
THE PARTY OF THE P	1.6. / 17 51
Contractors Name Jacob Kirsch - Compa	SE BUILDER DU DUN MENT
101111111111111111111111111111111111111	12K- 11th 11 2300-0
ree Simple Owner Name & Address	Developers Group loke City II
Bonding Co. Name & Address N/H	Crosp, Lake (114. FZ
Architect/Engineer Name & Address Nicholas Paul	Geisler 1758 NW Brown Id. l.C. F
Mortgage Lenders Name & Address Columbia Bar	ok 173 NW hillshoro ST Lake Cityos
Circle the correct power company - FL Power & Light - Circle the correct power Company - FL Power & Light - Circle the correct power company -	
Subdivision Name Rolling Meadows  Driving Directions from Usaalyuu -	confided Cost of Construction
Driving Directions from Usgo/441 - go usgo W.	Lor Block Unit Phase
Sw callahan ave, the follow to	Palling A Hope Henry
follow (w Thorning Glory dr 10 6th	Totting / Teadows Set. TR.
Type of Construction S. F. D.	
Total Acreage Lot Size Do you need a -Cu	Number of Existing Dwellings on Property
Actual Distance of Structure from Property Lines - Front	or <u>Culvert Walver</u> or <u>Have an Existing Driver</u>
	The state of the s
	Heated Floor Area 1700 Roof Pitch 8/12
Application is hereby made to obtain a permit to do work and installation has commenced prior to the issuance of a permit.	installations as indicated. I certify that no work or
Il laws regulating construction in this jurisdiction.	and that all work be performed to meet the standards of
OWNERS AFFIDAVIT: I hereby certify that all the foregoing info	ormation is accurate and all work will be done in
	on and zoning.
VARNING TO OWNER: YOUR FAILURE TO RECORD A NOTIC WICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU IN ENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE.	E OF COMMENCMENT MAY RESULT IN YOU PAYING
ENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE	OF COMMENCEMENT.
owner Builder or Authorized Person by Notarized Letter	Contractor Signature
TATE OF EL OPIDA WIICHEIle Fischer	Contractors License Number (BC 1753775
OUNTY OF COLUMBIA Commission # DD598374	Competency Card NumberNOTARY STAMP/SEAL
Co., INC.	1
nis	Muchello. Hochop
ersonally known or Produced Identification	Notary Signature (Revised Sept. 2006)
	JW (ELT MISSAY - )-1808
3	0100 (

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

### FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

#### NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

<u>YOU ARE HEREBY NOTIFIED</u> as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit. Contractor's License Number <u>CBC</u> 12 53775 Contractor's Signature (Permitee) **Columbia County** Competency Card Number\_ Affirmed under penalty of perjury to by the <u>Contractor</u> and subscribed before me this 3/ day of 3/Personally known V or Produced Identification Notary Public State of Florida SEAL: Matthew Rocco My Commission DD578349 State of Florida Notary Signature (For the Contractor) Expires 09/17/2010

# **Columbia County Building Department Culvert Permit**

# Culvert Permit No.

000001635

ADDRESS 484 NW TURNER AVENUE LAKE CITY FL 32055 OWNER SOUTHEAST DEVELOPERS PHONE 755-2082	DATE $07/1$	1/2008 PARCEL 1	ID # 15-4S-16-03023-554	
OWNER  SOUTHEAST DEVELOPERS  ADDRESS  226 SW MORNING GLORY DR  LAKE CITY  FL  32024  CONTRACTOR  JACOB KIRSCH  PHONE  344-4817  LOCATION OF PROPERTY  90W, TL ON 247S, TL CALLAHAN, TL HOPE HENRY, TR MORNING  GLORY, 6TH LOT ON RIGHT  SUBDIVISION/LOT/BLOCK/PHASE/UNIT  ROLLING MEADOWS  54  SIGNATURE  INSTALLATION REQUIREMENTS  X  Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4: 1 slope and poured with a 4 inch thick reinforced concrete slab.  INSTALLATION NOTE: Turnouts will be required as follows: a) a majority of the current and existing driveway turnouts are paved, or; b) the driveway to be served will be paved or formed with concrete. Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.  Culvert installation shall conform to the approved site plan standards.  Department of Transportation Permit installation approved standards.  Other	APPLICANT	JACOB KIRSCH	PHONE	344-4817
ADDRESS 226 SW MORNING GLORY DR LAKE CITY FL 32024  CONTRACTOR JACOB KIRSCH PHONE 344-4817  LOCATION OF PROPERTY 90W, TL ON 247S, TL CALLAHAN, TL HOPE HENRY, TR MORNING  GLORY, 6TH LOT ON RIGHT  SUBDIVISION/LOT/BLOCK/PHASE/UNIT ROLLING MEADOWS 54  INSTALLATION REQUIREMENTS  X Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.  INSTALLATION NOTE: Turnouts will be required as follows: a) a majority of the current and existing driveway turnouts are paved, or; b) the driveway to be served will be paved or formed with concrete.  Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.  Culvert installation shall conform to the approved site plan standards.  Department of Transportation Permit installation approved standards.  Other	ADDRESS _	184 NW TURNER AVENUE	LAKE CITY	FL 32055
CONTRACTOR JACOB KIRSCH  PHONE 344-4817  LOCATION OF PROPERTY  90W, TL ON 247S, TL CALLAHAN, TL HOPE HENRY, TR MORNING  GLORY, 6TH LOT ON RIGHT  SUBDIVISION/LOT/BLOCK/PHASE/UNIT ROLLING MEADOWS  SIGNATURE  INSTALLATION REQUIREMENTS  X Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4: 1 slope and poured with a 4 inch thick reinforced concrete slab.  INSTALLATION NOTE: Turnouts will be required as follows: a) a majority of the current and existing driveway turnouts are paved, or; b) the driveway to be served will be paved or formed with concrete.  Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.  Culvert installation shall conform to the approved site plan standards.  Department of Transportation Permit installation approved standards.  Other	OWNER SO	UTHEAST DEVELOPERS	PHONE	755-2082
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Department of Transportation Permit installation approved standards.  Other		<ul> <li>a) a majority of the current and ab) the driveway to be served will Turnouts shall be concrete or concrete or paved driveway, w</li> </ul>	existing driveway turnouts are I be paved or formed with con paved a minimum of 12 feet whichever is greater. The width	crete. wide or the width of the
Other		Culvert installation shall conform	n to the approved site plan star	ndards.
		Department of Transportation Per	rmit installation approved star	ndards.
ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED		Other		
ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED				
	ALL DDODED CA	EETV DECHIDEMENTS SHOULD BE	S EQUI OWED	610

135 NE Hernando Ave., Suite B-21

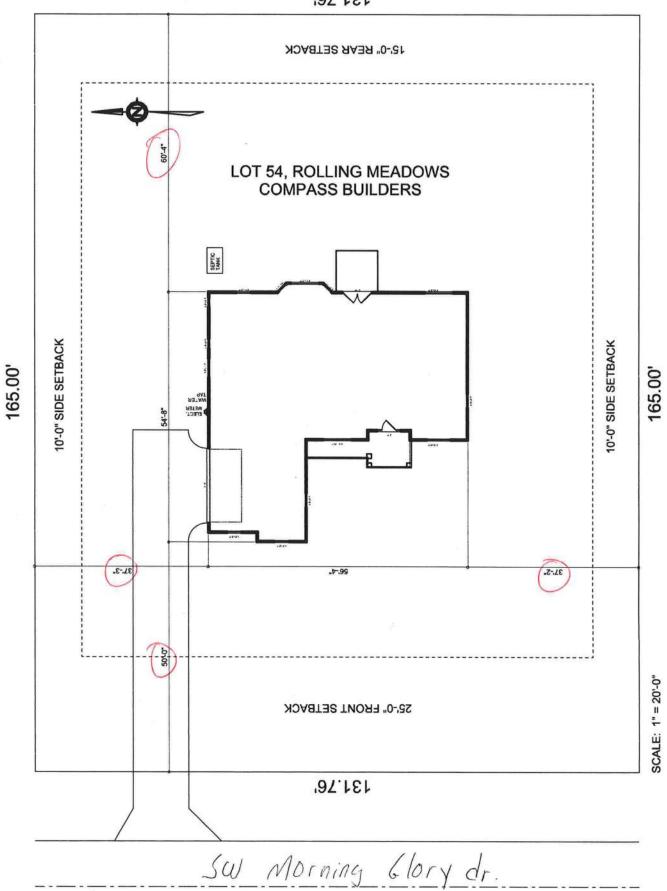
Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

DURING THE INSTALATION OF THE CULVERT.

Amount Paid 25.00





15-4S-16-03023-554 LOT 54 ROLLING MEADOWS S/D. WD 1063-1963. WD 1067-2449. CWD 1104-327

# Corporate Warranty Deed

This Indenture, made this December 4, 2006 A.D.

Between

Burbach Investment Group, LLC, a Florida Limited Liability Company, whose post office address is: 507 W. Duval Street, Lake City, FL 32055; Grantor and Southeast Developers Group, Inc., a Florida Corporation whose post office address is: 197 SW Waterford Court, Lake City, Florida 32025

Witnesseth, that the said Grantor, for and in consideration of the sum of Ten and No/100 Dollars (\$10.00), to it in hand paid by the said Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said Grantee forever, the following described land, situate, lying and being in the County of Columbia, State of Florida, to wit:

Lots 7, 48, 50, 52, and 54, ROLLING MEADOWS, according to the Plat thereof, recorded in Plat Book 8, Pages 45 and 46, of the Public Records of Columbia County, Florida.

Subject to taxes for the current year, covenants, restrictions and easements of record, if any.

Parcel Identification Number: R03023-507, R03023-548, R03023-550, R03023-552, and R03023-554

And the said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2006.

In Witness Whereof, the said Grantor has caused this instrument to be executed in its name by its duly authorized officer and caused its corporate seal to be affixed the day and year first above written.

> **Burbach Investment Group, LLC** a Florida Limited Liability Company

Signed and Sealed in Our Presence:

MELINDA WEAVER

State of County of Florida Columbia

The foregoing instrument was acknowledged before me this 4th day of December, 2006, by Thomas P Cady, Manager of Burbach Investment Group, LLC, a Florida Limited Liability Company, on behalf of the company. as identification.

He is personally known to me or has produced

Notary Public Notary Printed Name:

My Commission Expires::

#06-0389 Prepared by & Return to: Matt Rocco Sierra Title, LLC, 619 SW Baya Drive, Suite 102 Lake City, Florida 32025

Inst:2006028778 Date:12/06/2006 Time:12:42

1400.00 Doc Stamp-Deed :

DC,P.DeWitt Cason,Columbia County B:1104 P:327

Notary Public State of Florida Malthew Rocco

My Commission DD578349 Expires 09/17/2010

# COLUMBIA COUNTY 9-1-1 ADDRESSING / GIS DEPARTMENT

P. O. Box 1787, Lake City, FL 32056-1787
Telephone: (386) 758-1125 \* Fax: (386) 758-1365 \* E-mail: ron\_croft@columbiacountyfla.com

# **ADDRESS ASSIGNMENT DATA**

The Columbia County Board of County Commissioners has passed Ordinance 2001-9, which provides for a uniform numbering system. A copy of this ordinance is available in the Clerk of Court records, located in the courthouse. This new numbering system will increase the efficiency of POLICE, FIRE AND EMERGENCY MEDICAL vehicles responding to calls within Columbia County by immediately identifying the location of the caller.

Residential or Other Structure on Parcel Number: 15-4S-16-03023-554

Address Assignment: 226 SW MORNING GLORY DR, LAKE CITY, FL, 32024

Note: LOT 54 ROLLING MEADOWS S/D

Any questions concerning this information should be referred to the Columbia County 9-1-1 Addressing / GIS Department at the address or telephone number above.

Project Name:

PREPARED BY:

OWNER/AGENT:

with the Florida Energy Code.

DATE:

DATE:

Compass Builders - Kailee

Compass Builders

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

with the Florida Energy Code. Before construction is completed

BUILDING OFFICIAL:

Florida Statutes.

DATE:

this building will be inspected for

compliance with Section 553.908

Address: City, State: Owner: Climate Zone:	Lot: 5,4Sub: Roll Lake City, FL 32 Spec House North	ing Meadows, Pla 1025-6675	at:	Permit	ting Office: Cour Number: 27090 ction Number: 22	
I. New construction		New	_   1	12. Cooling systems	**************************************	*
2. Single family or n	1989 - 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Single family	_ !	<ol> <li>Central Unit</li> </ol>		Cap: 36.0 kBtu/hr
3. Number of units, i		1	_			SEER: 13.00
Number of Bedroo		3	_	b. N/A		_
5. Is this a worst case	7917/	No	_			_
6. Conditioned floor		1700 ft²	_	c. N/A		_
	rea: (Label reqd. by 13-1				05	_
a. U-factor:		Description Area	1	<ol><li>Heating systems</li></ol>		
b. SHGC:	ble DEFAULT) 7a.(DI	ble Default) 259.3 ft <sup>2</sup>	-	a. Electric Heat Pun	np	Cap: 36.0 kBtu/hr HSPF: 7.70
(or Clear or Tint	DEFAULT) 7b.	(Clear) 259.3 ft <sup>2</sup>		b. N/A		
8. Floor types						
a. Slab-On-Grade Ed	lge Insulation	R=5.0, 190.0(p) ft	_	c. N/A		_
b. N/A			_			
c. N/A			_   1	<ol><li>Hot water system:</li></ol>	S	
9. Wall types				<ol> <li>Electric Resistance</li> </ol>	ce	Cap: 50.0 gallons
a. Frame, Wood, Ext		$R=13.0, 1146.7 \text{ ft}^2$	_			EF: 0.90
b. Frame, Wood, Adj	acent	R=13.0, 180.0 ft <sup>2</sup>	_	b. N/A		_
c. N/A			_			-
d. N/A			_	<ul> <li>c. Conservation cred</li> </ul>		_
e. N/A			_	(HR-Heat recover	y, Solar	
<ol><li>Ceiling types</li></ol>			_	DHP-Dedicated I	heat pump)	
a. Under Attic		R=30.0, 1850.0 ft <sup>2</sup>	1	<ol><li>HVAC credits</li></ol>		РТ,
b. N/A			_	(CF-Ceiling fan, C	CV-Cross ventilation,	900 patrimosas:
c. N/A				HF-Whole house	fan,	_
11. Ducts				PT-Programmabl	e Thermostat,	
a. Sup: Unc. Ret: Un	ic. AH: Interior	Sup. R=6.0, 50.0 ft		MZ-C-Multizone	cooling,	=
b. N/A				MZ-H-Multizone	heating)	
			-			
Glass	s/Floor Area: 0.1	5 Total as-bui	•	nts: 21957 nts: 23500	PASS	
I hereby certify that t this calculation are in Code.	he plans and specific	cations covered by Florida Energy		Review of the plans specifications cove calculation indicate	red by this	OF THE STATE

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCPB v4.5.2)

I hereby certify that this building, as designed, is in compliance

# **SUMMER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

E	BASE					AS-	BUI	LT				
GLASS TYPES .18 X Conditions Floor Area		PM = F	oints	Type/SC	Ove Ornt	rhang Len	Hgt	Area X	SPM	1 X S	SOF	= Points
.18 1700.0	1	8.59	5689.0	1.Double, Clear	W	1.5	10.0	75.0	38	.52	0.98	2828.0
			100 (3 et ) (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1	2.Double, Clear	W	1.5	10.0	20.0	38	.52	0.98	754.0
				3.Double, Clear	W	1.5	10.0			.52	0.98	1508.0
				4.Double, Clear	N	1.5	8.0			.20	0.97	278.0
				5.Double, Clear	E	1.5	8.0			.06 .06	0.96	1208.0 297.0
				6.Double, Clear	E	9.5 5.5	10.0			.06	0.69	875.0
				7.Double, Clear 8.Double, Clear	E S		8.0			.87	0.92	529.0
				9.Double, Clear	S		8.0			.87	0.92	662.0
				3.Double, Olcar		1.0	0.0		0.70.00			
				As-Built Total:				259.3			-	8939.0
WALL TYPES	Area X	BSPM	= Points	Туре		R-	Value	e Area	Χ	SPN	1 =	Points
Adjacent	180.0	0.70	126.0	1. Frame, Wood, Exterior			13.0	1146.7		1.50		1720.0
Exterior	1146.7	1.70	1949.4	2. Frame, Wood, Adjacent			13.0	180.0		0.60		108.0
Base Total:	1326.7		2075.4	As-Built Total:				1326.7				1828.0
DOOR TYPES	Area X	BSPM	= Points	Туре			į.	Area	Х	SPN	1 =	Points
Adjacent	20.0	2.40	48.0	1.Exterior Insulated				20.0		4.10		82.0
Exterior	20.0	6.10	122.0	2.Adjacent Insulated				20.0		1.60		32.0
Base Total:	40.0		170.0	As-Built Total:				40.0				114.0
CEILING TYPES	Area X	BSPM	= Points	Туре		R-Val	ue	Area X	SPM	X SC	CM =	Points
Under Attic	1700.0	1.73	2941.0	1. Under Attic			30.0	1850.0	1.73 >	1.00		3200.5
Base Total:	1700.0		2941.0	As-Built Total:		W A		1850.0				3200.5
FLOOR TYPES	Area X	BSPM	= Points	Туре		R	-Valu	e Area	X	SPN	1 =	Points
Slab 19 Raised	90.0(p) 0.0	-37.0 0.00	-7030.0 0.0	Slab-On-Grade Edge Ins.	ulation		5.0	190.0(p		36.20		-6878.0
Base Total:			-7030.0	As-Built Total:				190.0				-6878.0
INFILTRATION	Area X	BSPM	= Points		(4)			Area	X	SPN	Λ =	Points
	1700.0	10.21	17357.0					1700.	0	10.21		17357.0

# **SUMMER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

	BASE		AS-BUILT								
Summer Ba	ase Points: 2	21202.4	Summer As-Built Points:	24560.5							
Total Summer Points	X System Multiplier	= Cooling Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	= Cooling Points							
21202.4	0.3250	6890.8	(sys 1: Central Unit 36000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(II       24561     1.00     (1.09 x 1.147 x 0.91)     0.260     0.950       24560.5     1.00     1.138     0.260     0.950	6901.9 <b>6901.9</b>							

# WINTER CALCULATIONS

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

BASE	AS-BUILT							
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Overhang  Type/SC  Ornt Len Hgt Area X WPM X WOF = Points							
.18 1700.0 20.17 6172.0	1.Double, Clear W 1.5 10.0 75.0 20.73 1.01 1563.0							
3	2.Double, Clear W 1.5 10.0 20.0 20.73 1.01 416.0							
¥1	3.Double, Clear W 1.5 10.0 40.0 20.73 1.01 833.0							
0	4.Double, Clear N 1.5 8.0 15.0 24.58 1.00 368.0 5.Double, Clear E 1.5 8.0 30.0 18.79 1.02 574.0							
	5.Double, Clear E 1.5 8.0 30.0 18.79 1.02 574.0 6.Double, Clear E 9.5 10.0 13.3 18.79 1.27 318.0							
	7.Double, Clear E 5.5 10.0 30.0 18.79 1.14 641.0							
	8.Double, Clear S 1.5 8.0 16.0 13.30 1.04 221.0							
	9.Double, Clear S 1.5 8.0 20.0 13.30 1.04 276.0							
	As-Built Total: 259.3 5210.0							
WALL TYPES Area X BWPM = Points	Type R-Value Area X WPM = Points							
Adjacent 180.0 3.60 648.0	1. Frame, Wood, Exterior 13.0 1146.7 3.40 3898.8							
Exterior 1146.7 3.70 4242.8								
Base Total: 1326.7 4890.8	As-Built Total: 1326.7 4492.8							
DOOR TYPES Area X BWPM = Points	Type Area X WPM = Points							
Adjacent 20.0 11.50 230.0	1.Exterior Insulated 20.0 8.40 168.0							
Exterior 20.0 12.30 246.0	2.Adjacent Insulated 20.0 8.00 160.0							
Base Total: 40.0 476.0	As-Built Total: 40.0 328.0							
CEILING TYPES Area X BWPM = Points	Type R-Value Area X WPM X WCM = Points							
Under Attic 1700.0 2.05 3485.0	1. Under Attic 30.0 1850.0 2.05 X 1.00 3792.5							
Base Total: 1700.0 3485.0	As-Built Total: 1850.0 3792.5							
FLOOR TYPES Area X BWPM = Points	Type R-Value Area X WPM = Points							
Slab         190.0(p)         8.9         1691.0           Raised         0.0         0.00         0.0								
Base Total: 1691.0	As-Built Total: 190.0 1444.0							
INFILTRATION Area X BWPM = Points	Area X WPM = Points							
1700.0 -0.59 -1003.0	1700.0 -0.59 -1003.0							

# **WINTER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

	BASE		AS-BUILT								
Winter Base	Points:	15711.8	Winter As-Built Points: 14	14264.3							
Total Winter X Points	System = Multiplier	Heating Points		eating oints							
15711.8	0.5540	8704.3		6.0 974.5 <b>74.5</b>							

# **WATER HEATING & CODE COMPLIANCE STATUS**

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

	E	BASE			AS-BUILT								
WATER HEA Number of Bedrooms	TING X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier		Credit Aultiplier	
3		2635.00		7905.0	50.0	0.90	3		1.00	2693.56		1.00	8080.7
					As-Built To	otal:							8080.7

	CODE COMPLIANCE STATUS												
	BASE							AS-BUILT					
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
6891		8704		7905		23500	6902		6974		8081		21957

**PASS** 



# **Code Compliance Checklist**

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

PERMIT #:

# **6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST**

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: 3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.  EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

# 6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	1
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

#### ESTIMATED ENERGY PERFORMANCE SCORE\* = 85.6

The higher the score, the more efficient the home.

Spec House, Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

1.	New construction or existing	New		12.	Cooling systems		
2.	Single family or multi-family	Single family	_	a.	Central Unit	Cap: 36.0 kBtu/hr	
3.	Number of units, if multi-family	1				SEER: 13.00	_
4.	Number of Bedrooms	3	-	b.	N/A		_
5.	Is this a worst case?	No					_
6.	Conditioned floor area (ft²)	1700 ft²		c.	N/A		_
7.	Glass type 1 and area: (Label reqd.	by 13-104.4.5 if not default)					_
a.	U-factor:	Description Area		13.	Heating systems		
	(or Single or Double DEFAULT)	7a. (Dble Default) 259.3 ft <sup>2</sup>		a.	Electric Heat Pump	Cap: 36.0 kBtu/hr	_
b.	SHGC:					HSPF: 7.70	
	(or Clear or Tint DEFAULT)	7b. (Clear) 259.3 ft <sup>2</sup>		ь.	N/A		
8.	Floor types		10 (may 1)				
a.	Slab-On-Grade Edge Insulation	R=5.0, 190.0(p) ft		c.	N/A		-
b.	N/A	11 3870					_
c.	N/A			14.	Hot water systems		
9.	Wall types				Electric Resistance	Cap: 50.0 gallons	
a.	Frame, Wood, Exterior	R=13.0, 1146.7 ft <sup>2</sup>				EF: 0.90	
b.	Frame, Wood, Adjacent	R=13.0, 180.0 ft <sup>2</sup>		b.	N/A		
c.	N/A						
d.	N/A			c.	Conservation credits		
e.	N/A		25.154		(HR-Heat recovery, Solar		_
10.	Ceiling types				DHP-Dedicated heat pump)		
a.	Under Attic	R=30.0, 1850.0 ft2		15.	HVAC credits	PT,	
b.	N/A				(CF-Ceiling fan, CV-Cross ventilation,	75. 74 <b>.</b> 17	
c.	N/A				HF-Whole house fan,		
11.	Ducts		-		PT-Programmable Thermostat,		
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 50.0 ft			MZ-C-Multizone cooling,		
	N/A		-		MZ-H-Multizone heating)		
			_		6/		
I ce	rtify that this home has complie	ed with the Florida Energ	y Effic	iency	Code For Building	OUE CT	
	struction through the above en					OF	Ø.
in th	nis home before final inspection	. Otherwise, a new EPL	Displa	y Car	d will be completed		BE
	ed on installed Code compliant					5	01
	lder Signature:		Date:			B	
Add	lress of New Home:		City/	FL Zi	p:	COD WE TRUST	
*N(	OTE: The home's estimated and	ray performance score is	ouh a	nail~	ble through the FLA/RES compute	The state of the s	
					86 for a US EPA/DOE EnergyStar		
-	Turning Erici & Italii	A TOWN BUDGE IS OU OF	S' Cuici	101	o joi a ob Li mbol Line gybiai	wood gride toriff	

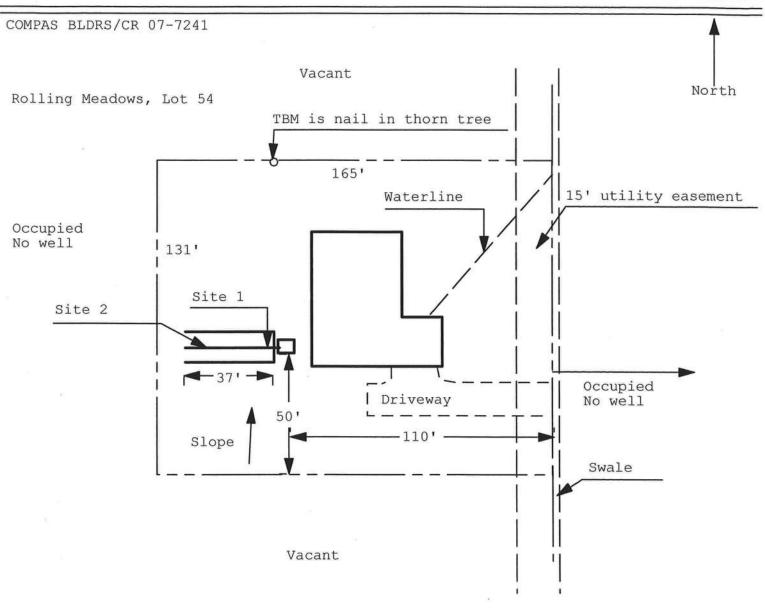
\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation) your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCPB v4.5.2)

08-0053

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: 080-55

# ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



		1 inch = 40 feet
	Plan Submitted By Carl Approved Not Approved Date	Date 1/11/08
Зу	Mon & Zanch	(olumbia CPHU
Notes	:	

# WILLIAM N. KITCHEN

PROFESSIONAL SURVEYOR AND MAPPER 152 N. MARION AVENUE LAKE CITY, FLORIDA 32055 PHONE (386) 755-7786 FAX (386) 755-5506 E-MAIL BSSK@BELLSOUTH.NET



DATE: 8/28/2008

To Whom It May Concern:

RE: COMPASS BUILDER LOT 54 ROLLING MEADOWS

SUBJECT Parcel: 15-4S-16-03023-554 PERMIT NO. 271-63

IS NOT IN A FLOOD ZONE ACCORDING TO FEMA FLOOD INSURANCE RATE MAP NO. 120070 0175 B DATED JANUARY 6, 1988. AND THE TOP OF FORMS IS AT AN ELEVATION OF 104.46 FEET. LOT 52 PER CURTIS KEEN P.E. 23836, LETTER DATED AUG. 28, 2008 STATES MINIMUM FLOOR ELEVATION OF 104.25 FEET, OR A MINUMUN OF 12" ABOVE THE HIGHEST GRADE.

Thank you,

WILLIAM N. KITCHEN PSM # 5490

William N. Either 8-28-2008

Waiting for original

# KEEN ENGINEERING & SURVEYING, INC. 9263 COUNTY ROAD 417 LIVE OAK, FLORIDA 32060 386/362-4787

August 28, 2008

Columbia County Building Department P.O. Drawer 1529 Lake City, FL 32056

RE: LOT 54 ROLLING MEADOWS S/D COLUMBIA COUNTY PERMIT # 27163 FINISH FLOOR ELEVATION

The above lot 54 of Rolling Meadows Subdivision has a minimum finish floor elevation of 106.00 stated on the plans. The lot has elevations that run from 105 on the North to 101.4' on the South.

The finish floor elevation is to be set at 104.25' and be a minimum of 12" above the highest adjacent grade within 15' of the proposed residence.

The finish floor elevation will be at an adequate height to prevent any flooding of the proposed home.

If additional information is required, please advise.

Curtis E. Reen, PE #23836

Eng. Bus. #3761

Copy: Compass Builders



# OCCUPANCY

# **COLUMBIA COUNTY, FLORIDA**

partment of Building and Zoning nspection

and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code. This Certificate of Occupancy is issued to the below named permit holder for the building

Parcel Number 15-4S-16-03023-554

Fire: 64.20

Building permit No. 000027163

Use Classification SFD,UTILITY

Permit Holder JACOB KIRSCH

Waste: 167.50

Owner of Building SOUTHEAST DEVELOPERS

Total: 231.70

Location: 226 SW MORNING GLORY DR, LAKE CITY, FL 32024

Date: 12/18/2008

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

# **Residential System Sizing Calculation**

Summary Project Title:

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Kailee

Lot 54

Code Only Professional Version

Climate: North

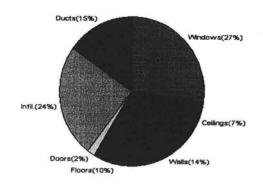
1/10/2008

				1/10/200	8
Location for weather data: Gaine	sville - Def	aults: Lati	tude(29) Altitude(152 ft.) Temp Ran	ige(M)	
Humidity data: Interior RH (50%	Outdoor	wet bulb (	77F) Humidity difference(54gr.)	90(111)	
Winter design temperature	33		Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	30539	Btuh	Total cooling load calculation	46532	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	
Total (Electric Heat Pump)	117.9	36000	Sensible (SHR = 0.75)		27000
Heat Pump + Auxiliary(0.0kW)	117.9	36000	Latent	110.0	
			Total (Electric Heat Pump)		36000

# WINTER CALCULATIONS

Winter Heating Load (for 1700 sqft)

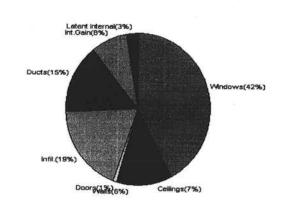
Load component			Load	
Window total	259	sqft	8348	Btuh
Wall total	1327	sqft	4357	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1850	sqft	2180	Btuh
Floor total	190	sqft	3107	Btuh
Infiltration	181	cfm	7345	Btuh
Duct loss			4684	Btuh
Subtotal		1	30539	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS		20070101	30539	Btuh



# **SUMMER CALCULATIONS**

Summer Cooling Load (for 1700 sqft)

Load component			Load	
Window total	259	sqft	19662	Btuh
Wall total	1327	sqft	2663	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1850	sqft	3064	Btuh
Floor total			0	Btuh
Infiltration	159	cfm	2953	Btuh
Internal gain			3780	Btuh
Duct gain		- 1	5838	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			38352	Btuh
Latent gain(ducts)			1181	Btuh
Latent gain(infiltration)			5799	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occu	pants/othe	r)	1200	Btuh
Total latent gain		350)	8180	Btuh
TOTAL HEAT GAIN			46532	Btuh





Version 8 For Florida residences only PREPARED BY: 1-9-08

# System Sizing Calculations - Winter

# Residential Load - Whole House Component Details

Spec House

Project Title: Compass Builders - Kailee

Lake City, FL 32025-

Professional Version

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	75.0	32.2	2414 Btu
2	2, Clear, Metal, 0.87	W	20.0	32.2	644 Btu
3	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btu
4	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btu
5	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btu
6	2, Clear, Metal, 0.87	E	13.3	32.2	429 Btu
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btu
8	2, Clear, Metal, 0.87	S	16.0	32.2	515 Btu
9	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btu
12.	Window Total		259(sqft)	02.2	8348 Btu
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1147	3.3	3766 Btu
2	Frame - Wood - Adj(0.09)	13.0	180	3.3	591 Btu
	Wall Total		1327	3.13	4357 Btu
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btu
2	Insulated - Adjacent		20	12.9	259 Btu
	Door Total		40	000000000	518Btu
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1850	1.2	2180 Btu
	Ceiling Total	0100000000	1850	2.500	2180Btu
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	190.0 ft(p)	16.4	3107 Btu
	Floor Total		190		3107 Btu
			Envelope Sul	ototal:	18510 Btu
Infiltration	Туре	ACH X Volu	ume(cuft) walls(sqft)	CFM=	
	Natural	0.80	13600 1327	181.3	7345 Btu
Ductload			(DL	.M of 0.181)	4684 Btu
All Zones		Sens	sible Subtotal All	Zones	30539 Btu

# **Manual J Winter Calculations**

Residential Load - Component Details (continued)

Spec House

Project Title: Compass Builders - Kailee

Lake City, FL 32025-

Code Only Professional Version

Climate: North

1/10/2008

QUIPMENT	Subtotal Sensible Ventilation Sensible Total Btuh Loss	30539 Btuh 0 Btuh 30539 Btuh
EQUIPMENT		

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8
For Florida residences only

# **System Sizing Calculations - Winter**

# Residential Load - Room by Room Component Details Project Title: Code C

Spec House

Lake City, FL 32025-

Compass Builders - Kailee

Code Only Professional Version

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

# Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	75.0	32.2	2414 Btul
2	2, Clear, Metal, 0.87	W	20.0	32.2	644 Btul
3	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btul
4	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btul
5	2, Clear, Metal, 0.87	Ε	30.0	32.2	966 Btul
6	2, Clear, Metal, 0.87	E	13.3	32.2	429 Btul
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btul
8	2, Clear, Metal, 0.87	S	16.0	32.2	515 Btul
9	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btul
	Window Total		259(sqft)		8348 Btul
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1147	3.3	3766 Btul
2	Frame - Wood - Adj(0.09)	13.0	180	1000 01100	591 Btul
1331	Wall Total		1327		4357 Btul
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btul
2	Insulated - Adjacent		20		259 Btul
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM= Lo  12.9 25  12.9 25  HTM= Lo  12.9 25  12.9 25  HTM= Lo  1.2 218  HTM= Lo  1.2 310  HTM= Lo  1.2 18510  De Subtotal: 18510	Load
1	Vented Attic/D/Shin	30.0	1850	00.00070707070	2180 Btul
	Ceiling Total		1850		2180Btul
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	190.0 ft(p)	16.4	3107 Btul
	Floor Total		190		3107 Btul
		Z	one Envelope Su	ibtotal:	18510 Btuh
Infiltration	Туре	ACH X Volu	ume(cuft) walls(sqf	t) CFM=	
	Natural	0.80	13600 1327	181.3	7345 Btul
Ductload	Average sealed, Supply(R6.	0-Attic), Retur	n(R6.0-Attic) (D	LM of 0.181)	4684 Btuh
Zone #1		Sens	sible Zone Subto	otal	30539 Btul

# **Manual J Winter Calculations**

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Kailee

Code Only Professional Version

Climate: North

1/10/2008

Subtotal Sensible	30539 Btuh
Ventilation Sensible	0 Btuh
Total Btuh Loss	30539 Btuh

	EQUIPMENT	
--	-----------	--

Electric Heat Pump	#	36000 Btuh
1. Electric Fleat Famp	"	36000 Btur

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



Version 8 For Florida residences only

# **System Sizing Calculations - Summer**

# Residential Load - Whole House Component Details Project Title: Code

Spec House

Compass Builders - Kailee

Code Only Professional Version Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/10/2008

### **Component Loads for Whole House**

	Type*		Over	hang	Wine	dow Area	a(saft)	H	ITM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		Unshaded		Unshaded		
1	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	75.0	0.0	75.0	29	80	5964	Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	20.0	0.0	20.0	29	80	1590	Btul
3	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	40.0	0.0	40.0	29	80	3181	Btul
4	2, Clear, 0.87, None, N, N	N	1.5ft	8ft.	15.0	0.0	15.0	29	29	434	Btul
5	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	Btul
6 7	2, Clear, 0.87, None,N,N	E	9.5ft	10ft.	13.3	9.1	4.2	29	80	600	Btul
8	2, Clear, 0.87, None,N,N 2, Clear, 0.87, None,N,N	E S	5.5ft	10ft.	30.0	0.0	30.0	29	80	2385	Btul
9	2, Clear, 0.87, None,N,N	S	1.5ft 1.5ft	8ft. 8ft.	16.0 20.0	16.0	0.0	29	34		Btul
•	Excursion	3	1.511	OIL.	20.0	20.0	0.0	29	34		Btul
	Window Total				250 /	n~#\				2079	
Walls	Type		D.V	ا/مبياد	259 ( I-Value		ooft)		нтм	19662	Btur
1	Frame - Wood - Ext		IX-V			Area(	S. 18.			Load	
2	Frame - Wood - Ext			13.0/		114			2.1	2392	
2	Wall Total			13.0/	0.09	180	1917		1.5		Btuh
Doors					-		7 (sqft)			2663	Btuh
	Туре					Area (			HTM	Load	
1 2	Insulated - Exterior					20.	177		9.8	196	Btuh
2	Insulated - Adjacent					20.			9.8	196	Btuh
A	Door Total						0 (sqft)			392	Btuh
Ceilings	Type/Color/Surface		R-Va	alue		Area(	sqft)		HTM	Load	
1	Vented Attic/DarkShingle			30.0		1850	0.0		1.7	3064	Btuh
	Ceiling Total					1850	0 (sqft)			3064	Btuh
Floors	Туре		R-Va	lue		Siz	e		HTM	Load	
1	Slab On Grade			5.0		19	0 (ft(p))		0.0	0	Btuh
	Floor Total						O (sqft)			1974	Btuh
					- 1	Fn	velope S	Subtotal		25781	Rtub
							толоро (	Jubiotai		20701	Dian
nfiltration	71		Α	CH	Volum		all area	(sqft)	CFM=	Load	
	SensibleNatural			0.70		13600	1327	* * *	181.3	2953	Btuh
Internal		(	Occup	ants		Btuh/occ	cupant	A	ppliance	Load	
gain				6		( 230			2400	THE PARTY OF THE P	Btul
						Se	nsible E	nvelope	Load:	32514	
ouct load							(DGN	/I of 0.18	30)	5838	Btul
						Sen	sible Lo	ad All 2	Zones	38352 E	3tuh

# **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Spec House

Project Title: Compass Builders - Kailee

Code Only **Professional Version** 

Climate: North

1/10/2008

# Lake City, FL 32025-

## WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	32514	Btuh
	Sensible Duct Load	5838	Btuh
	Total Sensible Zone Loads	38352	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	38352	Btuh
<b>Totals for Cooling</b>	Latent infiltration gain (for 54 gr. humidity difference)	5799	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1181	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	8180	Btuh
	TOTAL GAIN	46532	Btuh

EQUIPMENT		
1. Central Unit	#	36000 Btuh

\*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value) (BS - Insect screen: none(N), Full(F) or Half(H)) (Ornt - compass orientation)



Version 8 For Florida residences only

# **System Sizing Calculations - Summer**

# Residential Load - Room by Room Component Details

Spec House

Project Title:

Code Only Professional Version

Lake City, FL 32025-

Compass Builders - Kailee

Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/10/2008

#### Component Loads for Zone #1: Main

	Type*		Over	hang	Win	dow Area	a(sqft)	ł	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	75.0	0.0	75.0	29	80	5964	Btuh
2	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	20.0	0.0	20.0	29	80	1590	Btuh
3	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	40.0	0.0	40.0	29	80	3181	Btuh
4	2, Clear, 0.87, None, N, N	N	1.5ft	8ft.	15.0	0.0	15.0	29	29	434	Btuh
5	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	Btuh
6	2, Clear, 0.87, None, N, N	E	9.5ft	10ft.	13.3	9.1	4.2	29	80	600	Btuh
7	2, Clear, 0.87, None,N,N	Ε	5.5ft	10ft.	30.0	0.0	30.0	29	80	2385	Btuh
8	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	16.0	16.0	0.0	29	34	463	Btuh
9	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	20.0	20.0	0.0	29	34		Btuh
	Window Total				259 (	sqft)				17583	Btuh
Walls	Туре		R-Va	alue/U	-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	114	16.7		2.1	2392	Btuh
2	Frame - Wood - Adj		13.0/0.09		180.0		1.5	272 B	Btuh		
	Wall Total				132	27 (sqft)		26		63 Btuh	
Doors	Туре					The second second	(sqft)		нтм	Load	
1	Insulated - Exterior						0.0		9.8	196	Btuh
2	Insulated - Adjacent						0.0		9.8	196	Btuh
-	Door Total						0 (sqft)			392	Btuh
Ceilings	Type/Color/Surface		R-Va	alue		the second second	(sqft)		нтм	Load	
1	Vented Attic/DarkShingle			30.0			50.0		1.7	3064	Btuh
	Ceiling Total			00.0		7-5-5	0 (sqft)		3.7	3064	
Floors	Type		R-Va	duo			ze		нтм	Load	Dian
			IV-AG							-300,000,000,000	
1	Slab On Grade			5.0		190 (ft(p)) 0.0		0.0	97.5	Btuh	
	Floor Total					190.0 (sqft)				0	Btuh
						Z	one Enve	elope S	ubtotal:	23702	Btuh
nfiltration	Type		Δ	СН	Volum	e(cuft)	wall area	(saft)	CFM=	Load	
	SensibleNatural			0.70		13600	1327	(-4.4)	158.7	2953	Btuh
Internal			Occup	-		and the second second	cupant	17.	Appliance	Load	
gain				6		X 23			2400	3780	Blub
guiii			*****				20. 10. 11.0				
						S	ensible E	nvelop	e Load:	30435	Btuh
Duct load	Average sealed, Supply	(R6.0-	Attic),	Retu	n(R6.0	-Attic)		(DGM	of 0.180)	5465	Btuh
							Sensib	le Zon	e Load	35900	Btuh

# The following window Excursion will be assigned to the system loads.

Windows	July excursion for System 1		2079 Btuh
		Excursion Subtotal:	2079 Btuh

# **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Project Title:
Compass Builders - Kailee

Professional Continued

Spec House

Code Only Professional Version

Climate: North

Lake City, FL 32025-

1/10/2008

	Sensible Excursion Load	2452 Btuh
Duct load		373 Btuh

# **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Spec House

Project Title: Compass Builders - Kailee

Lake City, FL 32025-

Code Only Professional Version Climate: North

1/10/2008

#### WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	32514	Btuh
	Sensible Duct Load	5838	Btuh
	Total Sensible Zone Loads	38352	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	38352	Btuh
<b>Totals for Cooling</b>	Latent infiltration gain (for 54 gr. humidity difference)	5799	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1181	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	8180	Btuh
	TOTAL GAIN	46532	

1 Central Unit	#	20000 Pt. t
Central Unit	#	36000 Btuh

\*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default) (InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value) (BS - Insect screen: none(N), Full(F) or Half(H)) (Ornt - compass orientation)



Version 8 For Florida residences only

# **Residential Window Diversity**

# MidSummer

Spec House

Lake City, FL 32025-

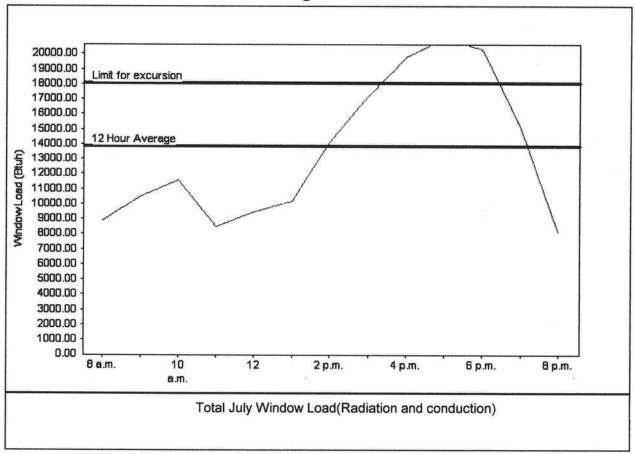
Project Title: Compass Builders - Kailee

Code Only Professional Version Climate: North

1/10/2008

Weather data for: Gainesville - Defa	aults			
Summer design temperature	92	F	Average window load for July	13873 Btu
Summer setpoint	75	F	Peak window load for July	20990 Btu
Summer temperature difference	17	F	Excusion limit(130% of Ave.)	18035 Btu
Latitude	29	North	Window excursion (July)	2955 Btuh

# **WINDOW Average and Peak Loads**



This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only PREPARED BY:

DATE:

EnergyGauge® FLRCPB v4.5.2

MRMURL J

# PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Jeld-Wen	Exterior Swinging door	FL- 49X-R1
B. SLIDING		72.31.3	1 - 110 -
C. SECTIONAL/ROLL UP	ROYMOR	Raynor	FL-4867
D. OTHER	132711111111111111111111111111111111111		7161
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	MI- Products	Sinale Hung Window	FL-5108
B. HORIZONTAL SLIDER			1 2 3 10 8
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	James Hardie	Hardi Plant Siding	FL-889-R1
B. SOFFITS	Kaylan	Aluminum Soffit	FL- 4957
C. STOREFRONTS			- 1137
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS	<del>                                     </del>		
A. ASPHALT SHINGLES	Elk Roofing	Asphalt shingles	FL-586-RZ
B. NON-STRUCT METAL	1	Topical Survey (c)	12 346 KZ
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS		**************************************	
A. WOOD CONNECTORS	SIMPSON Strong to	Truss Strups	FL- 474-RI
B. WOOD ANCHORS		119/01	
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR	<del> </del>		
ENVELOPE PRODUCTS			
A			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE DATE

project 1 - 1

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

# ALL REQUIREMENTS ARE SUBJECT TO CHANGE **EFFECTIVE OCTOBER 1, 2005**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ---
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

# APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENI	CRAL REQUIREM	IENTS: Two (2) complete sets of plans containing the following:
	ant Plans Exami	ner (2) complete sets of plans containing the following:
( e	0	All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square
Q	0	Designers name and signature on document (CDC) and a
ď.	0	Site Plan including:  a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility exceptors.
<b>13-</b>	D	Wind-load Engineering Summary, calculations and any details required  Plans or specifications must state compliance with FBC Section 1609.  The following information must be shown as per section 1603.1.4 FBC  a. Basic wind speed (3-second gust), miles per hour (km/hr).  b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.  c. Wind exposure, if more than one wind exposure is all the second secon
	*	<ul> <li>wind exposure and applicable wind direction shall be indicated.</li> <li>d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient.</li> <li>e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.</li> </ul>
4	0 0	Elevations including:  a) All sides  b) Roof pitch c) Overhang dimensions and detail with attic ventilation

9		d) Location, size and height above roof of chimneys.
		e) Location and size of skylights
6		f) Building height
中		e) Number of stories
		Floor Plan including:
Ø		a) Rooms labeled and dimensioned.
<b>P</b>	0	b) Shear walls identified.
	0	c) Show product approval specification as required by Fla. Statute 553.842 and
1	_	Fla. Administrative Code 9B-72 (see attach forms).
4		d) Show safety glazing of glass, where required by code.
4	0	e) Identify egress windows in bedrooms, and size.
d		f) Fireplace (gas vented), (gas non-vented) or wood burning with
		hearth, (Please circle applicable type).
P	0	g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
西		h) Must show and identify accessibility requirements (accessible bathroom)
\		Foundation Plan including:
ę p		<ul> <li>a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.</li> </ul>
000	0	b) All posts and/or column footing including size and reinforcing
ф	0	c) Any special support required by soil analysis such as piling
4		d) Location of any vertical steel.
1		Roof System;
ф		a) Truss package including:
		<ol> <li>Truss layout and truss details signed and sealed by Fl. Pro. Eng.</li> </ol>
1		<ol><li>Roof assembly (FBC 106.1.1.2) Roofing system, materials.</li></ol>
		manufacturer, fastening requirements and product evaluation with
rav.		wind resistance rating)
Ch.		b) Conventional Framing Layout including:
		<ol> <li>Rafter size, species and spacing</li> <li>Attachment to wall and uplift</li> </ol>
		Ridge beam sized and valley framing and support details
		4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials,
		manufacturer, fastening requirements and product evaluation with
		wind resistance rating)
		Wall Sections including:
		a) Masonry wall
		All materials making up wall
		<ol><li>Block size and mortar type with size and spacing of reinforcement</li></ol>
		Lintel, tie-beam sizes and reinforcement     Gable ends with rake beams showing reinforcement or cable trues.
		and wall bracing details
		<ol><li>All required connectors with uplift rating and required number and</li></ol>
		size of fasteners for continuous tie from roof to foundation shall be
		designed by a Windload engineer using the engineered roof truss
		plans.
		6. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
		7. Fire resistant construction (if required)
		8. Fireproofing requirements
		<ol><li>Shoe type of termite treatment (termiticide or alternative method)</li></ol>
		10. Slab on grade
č:		a. Vapor retarder (6mil. Polyethylene with joints lapped 6

a. Vapor retarder (6mil. Polyemylene with joints tapped 6 inches and sealed)

b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports

11. Indicate where pressure treated wood will be placed

12. Provide insulation R value for the following:

Crawl space (if applicable) D b) Wood frame wall 1. All materials making up wall 2. Size and species of studs 3. Sheathing size, type and nailing schedule 4. Headers sized 5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail 6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans. 7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating) 8. Fire resistant construction (if applicable) Fireproofing requirements 10. Show type of termite treatment (termiticide or alternative method) 11. Slab on grade a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports 12. Indicate where pressure treated wood will be placed 13. Provide insulation R value for the following: a. Attic space Exterior wall cavity Crawl space (if applicable) c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect) Floor Framing System; a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer 0 b) Floor joist size and spacing c) Girder size and spacing d) Attachment of joist to girder e) Wind load requirements where applicable Plumbing Fixture layout Electrical layout including: a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified b) Ceiling fans c) Smoke detectors d) Service panel and sub-panel size and location(s) e) Meter location with type of service entrance (overhead or underground) f) Appliances and HVAC equipment g) Arc Fault Circuits (AFCI) in bedrooms h) Exhaust fans in bathroom **HVAC** information a) Energy Calculations (dimensions shall match plans)

b) Manual J sizing equipment or equivalent computation

Disclosure Statement for Owner Builders

Private Potable Water

c) Gas System Type (LP or Natural) Location and BTU demand of equipment

\*\*\* Notice Of Commencement Required Before Any Inspections Will Be Done

 a. Attic spaceb. Exterior wall cavity



Project Information for:

L265564

Builder:

Compass Builders

Lot:

54-1

Subdivision:

Rolling Meadows

County:

Gravity:

Baker

Truss Count:

Design Program: MiTek 20/20 6.3 **Building Code:** FBC2004/TPI2002 Truss Design Load Information:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Wind:

Contractor of Record, responsible for the Structural Engineering:

Jacob C. Kirsch Florida License No. CBC1253775

Address: 196 Southwest Huntsview Way Lake City, Florida 32024

Florida P.E. License No. 34869 Truss Design Engineer: Julius Lee, PE

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

25

26

27

28

J1925172

J1925173

J1925174

J1925175

T16

T17

T18

T19G

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

> /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08 /10/08

No.	Drwg. #	Truss ID	Date	No.	Drwg. #	Truss ID	Date
1	J1925148	CJ01	1/10/08	29	J1925176	T20	1/10/08
2	J1925149	CJ02	1/10/08	30	J1925177	T21	1/10/08
3	J1925150	CJ03	1/10/08	31	J1925178	T22G	1/10/08
4	J1925151	EJ01	1/10/08	32	J1925179	T23G	1/10/08
5	J1925152	EJ02	1/10/08	33	J1925180	T24	1/10/08
6	J1925153	HJ01	1/10/08	34	J1925181	T25	1/10/08
7	J1925154	P01	1/10/08	35	J1925182	T26	1/10/08
8	J1925155	P02	1/10/08	36	J1925183	T27	1/10/08
9	J1925156	P03	1/10/08	37	J1925184	T28	1/10/08
10	J1925157	T01G	1/10/08	38	J1925185	T30	1/10/08
11	J1925158	T02G	1/10/08	39	J1925186	T31	1/10/08
12	J1925159	T03	1/10/08	40	J1925187	T32	1/10/08
13	J1925160	T04	1/10/08	41	J1925188	T33	1/10/08
14	J1925161	T05	1/10/08	42	J1925189	T34	1/10/08
15	J1925162	T06	1/10/08	43	J1925190	T35	1/10/08
16	J1925163	T07	1/10/08	44	J1925191	T36	1/10/08
17	J1925164	T08G	1/10/08	45	J1925192	T37	1/10/08
18	J1925165	T09	1/10/08	46	J1925193	T38	1/10/08
19	J1925166	T10	1/10/08	47	J1925194	T39	1/10/08
20	J1925167	T11	1/10/08				
21	J1925168	T12	1/10/08				
22	J1925169	T13	1/10/08				
23	J1925170	T14	1/10/08				
24	J1925171	T15	1/10/08				

1/10/08

1/10/08

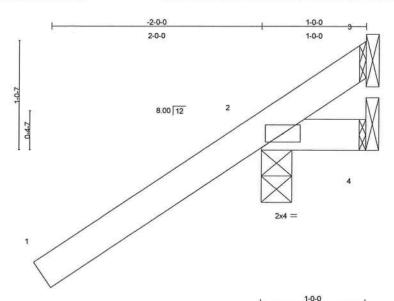
1/10/08

1/10/08

Qty Job Truss Ply 00 Truss Type J1925148 CJ01 **JACK** 2 1 Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

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1-0-0 LOADING (psf) SPACING 2-0-0 CSI DEFL L/d in (loc) I/defl **PLATES** 20.0 Plates Increase 1.25 TC 0.31 Vert(LL) -0.00>999 360 MT20 Lumber Increase 7.0 BC 0.01 -0.002 >999 1.25 Vert(TL)

0.00

WB

(Matrix)

240 n/a

GRIP 244/190

Scale = 1:10.4

Weight: 7 lb

LUMBER

TCLL

TCDL

**BCLL** 

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

10.0

BRACING

Horz(TL)

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

n/a

3

0.00

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=270/0-3-8, 4=5/Mechanical, 3=-100/Mechanical

YES

Max Horz 2=119(load case 6)

Rep Stress Incr

Code FBC2004/TPI2002

Max Uplift 2=-312(load case 6), 3=-100(load case 1)

Max Grav 2=270(load case 1), 4=14(load case 2), 3=161(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-90/111

BOT CHORD 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.32

#### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 Ib uplift at joint 2 and 100 lb uplift at joint 3. Continued on page 2

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building odes. For general guidance regarding storage, delivery, erection and bracing, consult BCSI. or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0.0	
	CJ01	JACK	2	1		J1925148
					Job Reference (optional)	

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LOAD CASE(S) Standard

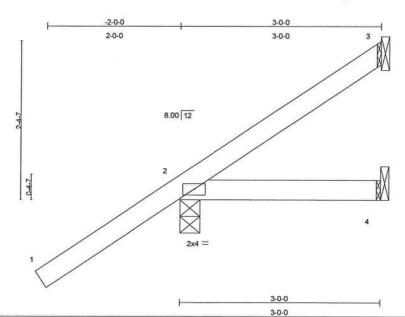


 Job
 Truss
 Truss Type
 Qty
 Ply
 0 0
 J1925149

 CJ02
 JACK
 2
 1
 Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:29:59 2008 Page 1



LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.33 Vert(LL) -0.00>999 360 MT20 2-4 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.06 Vert(TL) -0.012-4 >999 240 **BCLL** 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.00n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 14 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=27/Mechanical, 2=258/0-3-8, 4=14/Mechanical

Max Horz 2=179(load case 6)

Max Uplift 3=-33(load case 7), 2=-207(load case 6)

Max Grav 3=34(load case 4), 2=258(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-72/19

BOT CHORD 2-4=0/0

# JOINT STRESS INDEX

2 = 0.27

## **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 207 lb uplift at joint 2. Continued on page 2

Truse Cesign Engineer Florida PE No. 24889 1100 Chastel Bay Blvd. Boynton Beach, Ft. 33435

January 10,2008

Scale = 1:16.2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	0.100	LL OU				J1925149
	CJ02	JACK	2	1	No district to the team of	
					Job Reference (optional)	

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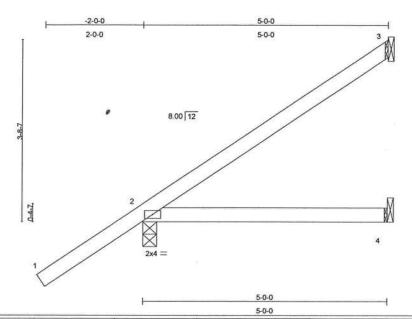
LOAD CASE(S) Standard



Truss Type Job Truss Qty Ply 00 J1925150 **CJ03 JACK** 2 1 Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 20 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=101/Mechanical, 2=302/0-3-8, 4=24/Mechanical

Max Horz 2=240(load case 6)

Max Uplift 3=-103(load case 6), 2=-188(load case 6)

Max Grav 3=101(load case 1), 2=302(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-90/43

**BOT CHORD** 2-4=0/0

# JOINT STRESS INDEX

2 = 0.30

## **NOTES**

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 Ib uplift at joint 3 and 188 lb uplift at joint 2. Continued on page 2

January 10,2008

Scale = 1:22.1

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connec Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erecti and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	CJ03	JACK	2	1		J1925150
	0000	UNION	-		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:00 2008 Page 2

LOAD CASE(S) Standard

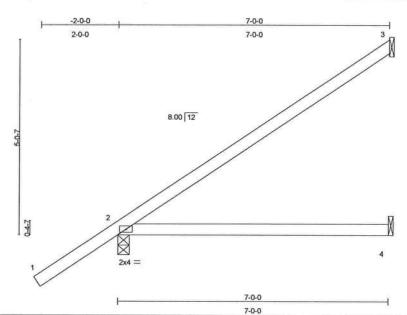


Job Truss Truss Type Qty Ply 00 J1925151 **EJ01** MONO TRUSS 3 1 Job Reference (optional)

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6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:01 2008 Page 1

Scale = 1:28.0



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.11	2-4	>730	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.20	2-4	>417	240	4 (1) 4 (1)	0.000 3 0.00 0.00 0.00
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 27 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=164/Mechanical, 2=357/0-3-8, 4=34/Mechanical

Max Horz 2=218(load case 6)

Max Uplift 3=-110(load case 6), 2=-124(load case 6)

Max Grav 3=164(load case 1), 2=357(load case 1), 4=102(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-138/72

**BOT CHORD** 2-4=0/0

## JOINT STRESS INDEX

2 = 0.34

# NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 3 and 124 lb uplift at joint 2.

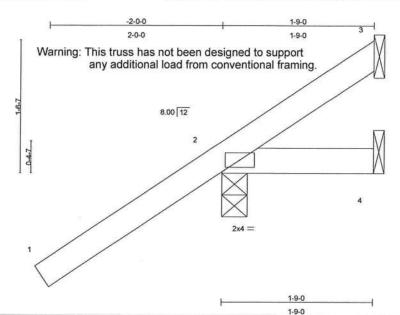
January 10,2008 LOAD CASE(S) Standard

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.02	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 10 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-9-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=239/0-3-8, 4=9/Mechanical, 3=-24/Mechanical

Max Horz 2=142(load case 6)

Max Uplift 2=-233(load case 6), 3=-24(load case 1)

Max Grav 2=239(load case 1), 4=26(load case 2), 3=55(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/61, 2-3=-67/37

BOT CHORD

2-4=0/0

## JOINT STRESS INDEX

2 = 0.27

## NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 2 and 24 lb uplift at joint 3. Continued on page 2

Julius Les Truse Design Engineer Florida PE No. 24869 1109 Coestal Bay Blvd Boynton Beach, Ft. 33435

January 10,2008

Scale = 1:12.6

A Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	######################################
	EJ02	MONO TRUSS	4	1		J1925152
	Looz	More moss	1		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:01 2008 Page 2

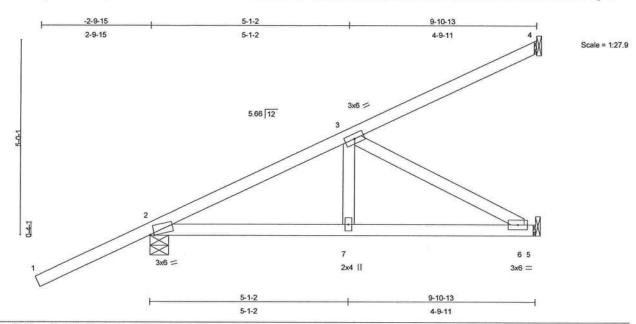
LOAD CASE(S) Standard



Job Truss Truss Type Qty Ply 00 J1925153 **HJ01** MONO TRUSS Job Reference (optional)

Builders First Source, Jacksonville , Florida 32244

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.04	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.06	6-7	>999	240	(3111.00.00.00.00.00.00.00.00.00.00.00.00.	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.22	Horz(TL)	0.01	5	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 46 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 **WEBS** 

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=234/Mechanical, 2=460/0-5-11, 5=251/Mechanical

Max Horz 2=367(load case 5)

Max Uplift 4=-238(load case 5), 2=-242(load case 5), 5=-109(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/66, 2-3=-466/0, 3-4=-134/73 2-7=-243/394, 6-7=-243/394, 5-6=0/0

**BOT CHORD** WEBS

3-6=-447/276, 3-7=0/186

# JOINT STRESS INDEX

2 = 0.85, 3 = 0.13, 6 = 0.12 and 7 = 0.13

#### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 4, 242 lb uplift at joint 2 and 109 lb uplift at joint 5.

Continued on page 2

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	William Company Company
	HJ01	MONO TRUSS	1	1		J1925153
	11001	Mone mode	'		Job Reference (optional)	

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#### **NOTES**

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

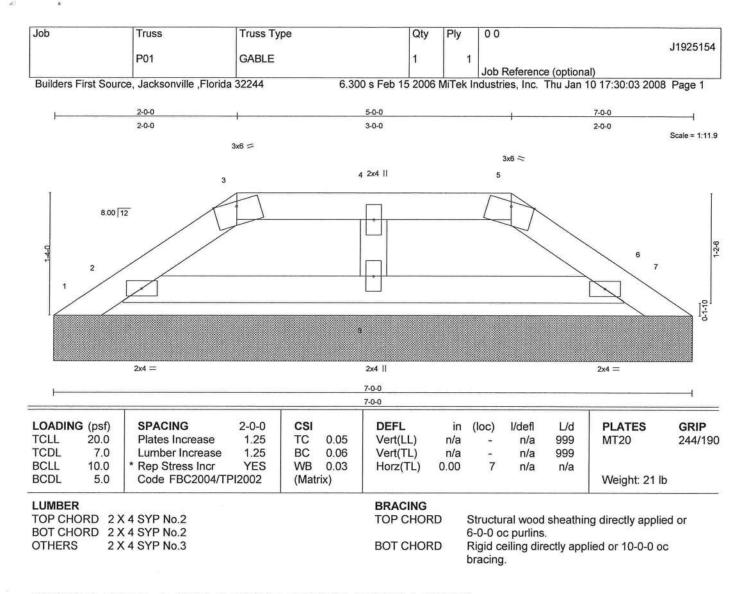
Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)





REACTIONS (lb/size) 1=-9/7-0-0, 7=-9/7-0-0, 2=146/7-0-0, 6=146/7-0-0, 8=142/7-0-0

Max Horz 1=-33(load case 4)

Max Uplift 1=-42(load case 4), 7=-32(load case 11), 2=-90(load case 5), 6=-71(load

case 4), 8=-37(load case 5)

Max Grav 1=53(load case 5), 7=33(load case 4), 2=170(load case 10), 6=170(load

case 11), 8=142(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-54/53, 2-3=-111/91, 3-4=-69/89, 4-5=-69/89, 5-6=-111/91, 6-7=-16/32

BOT CHORD 2-8=-29/69, 6-8=-29/69

WEBS 4-8=-108/124

#### JOINT STRESS INDEX

2 = 0.18, 3 = 0.03, 4 = 0.07, 5 = 0.03, 6 = 0.18 and 8 = 0.07

#### NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail" Continued on page 2

Truse Design Engineer Florida PE No. 24869 1109 Coastal Bay Blvd Boynton Beach, FL 33436

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	00	
	P01	GABLE	1	1		J1925154
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:03 2008 Page 2

#### **NOTES**

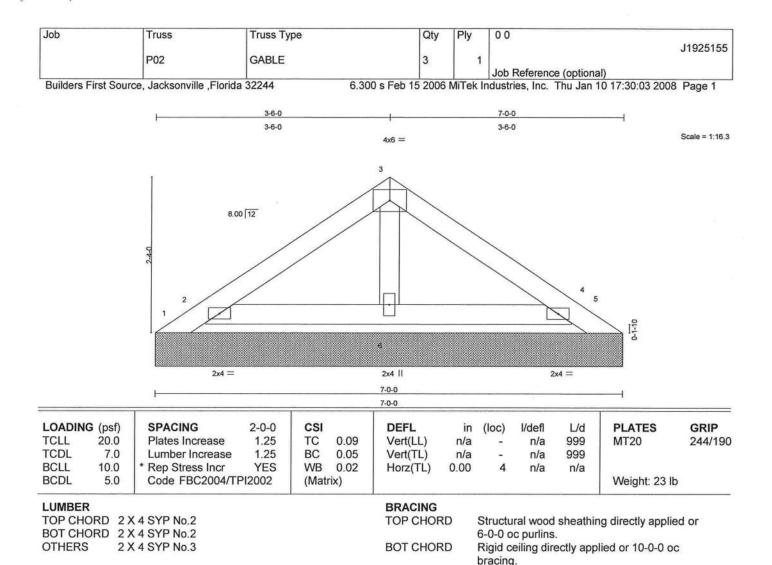
4) Provide adequate drainage to prevent water ponding.

- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1, 32 lb uplift at joint 7, 90 lb uplift at joint 2, 71 lb uplift at joint 6 and 37 lb uplift at joint 8.
- 10) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonds PE No. 24868 1109 Gestel Bay Blvd Boynton Besch, FL 99435





**REACTIONS** (lb/size) 1=-72/7-0-0, 5=-72/7-0-0, 2=214/7-0-0, 4=214/7-0-0, 6=131/7-0-0

Max Horz 1=60(load case 5)

Max Uplift 1=-72(load case 1), 5=-72(load case 1), 2=-118(load case 6), 4=-108(load

case 7)

Max Grav 1=78(load case 5), 5=56(load case 7), 2=214(load case 1), 4=214(load case 1), 6=131(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-90/79, 2-3=-68/46, 3-4=-68/46, 4-5=-46/51

**BOT CHORD** 

2-6=-11/37, 4-6=-11/37

**WEBS** 

3-6=-97/43

## JOINT STRESS INDEX

2 = 0.22, 3 = 0.02, 4 = 0.22 and 6 = 0.03

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail" Continued on page 2

Truse Cesign Engineer Florida PE No. 34969 1109 Coastal Bay Blvd. Boymon Beach, Ft. 93435

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	P02	CARLE	2			J1925155
	P02	GABLE	3	1	Job Reference (optional)	

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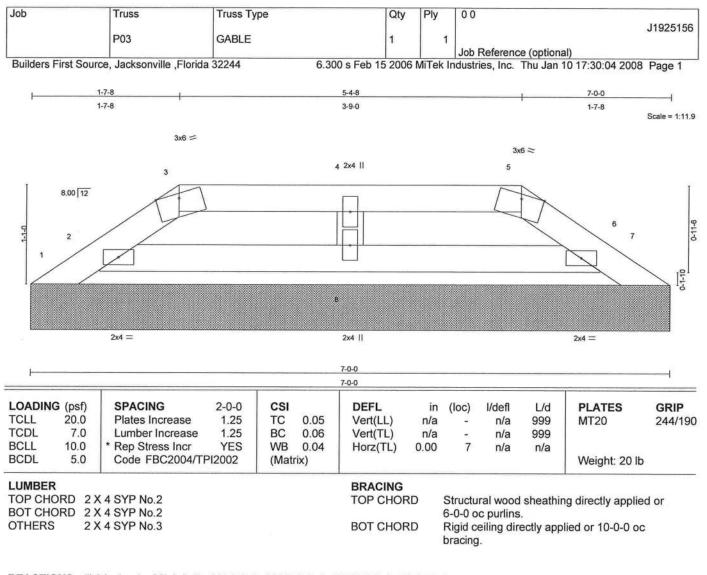
#### **NOTES**

- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 4-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 1, 72 lb uplift at joint 5, 118 lb uplift at joint 2 and 108 lb uplift at joint 4.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 24865 1109 Coastel Bay Blyd





**REACTIONS** (lb/size) 1=-6/7-0-0, 7=-6/7-0-0, 2=132/7-0-0, 6=132/7-0-0, 8=164/7-0-0

Max Horz 1=-26(load case 4)

Max Uplift 1=-22(load case 4), 7=-16(load case 11), 2=-69(load case 5), 6=-52(load

case 4), 8=-49(load case 5)

Max Grav 1=34(load case 5), 7=17(load case 4), 2=143(load case 10), 6=143(load case 11), 8=164(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-39/37, 2-3=-100/90, 3-4=-62/81, 4-5=-62/81, 5-6=-100/90, 6-7=-8/24

**BOT CHORD** 

2-8=-34/62, 6-8=-34/62

WEBS

4-8=-129/151

## **JOINT STRESS INDEX**

2 = 0.15, 3 = 0.03, 4 = 0.08, 5 = 0.03, 6 = 0.15 and 8 = 0.08

#### NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
 Continued on page 2

Julius Lee Truse Design Engineer Florida PE No. 24888 1 100 Coastal Bay Blyd Boynton Beach, FL 33435

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
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**Builders** FirstSource

Job	Truss	Truss Type	Qty	Ply	00	
	P03	GABLE	1	1		J1925156
					Job Reference (optional)	

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#### NOTES

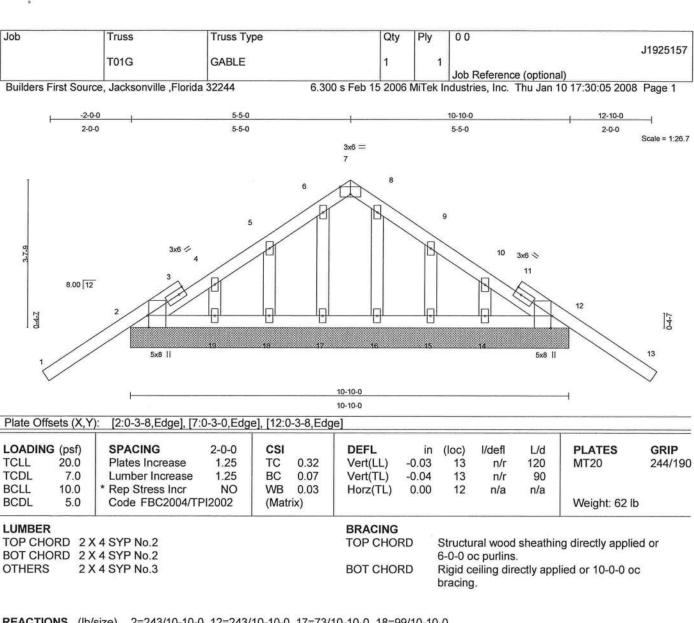
4) Provide adequate drainage to prevent water ponding.

- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1, 16 lb uplift at joint 7, 69 lb uplift at joint 2, 52 lb uplift at joint 6 and 49 lb uplift at joint 8.
- 10) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lew Truse Design Engineer Flonda PE No. 24209 1100 Geastal Rey Blvd Bovnton Besch, Ft. 23436





REACTIONS (lb/size) 2=243/10-10-0, 12=243/10-10-0, 17=73/10-10-0, 18=99/10-10-0,

19=44/10-10-0, 16=73/10-10-0, 15=99/10-10-0, 14=44/10-10-0

Max Horz 2=-122(load case 4)

Max Uplift 2=-221(load case 6), 12=-235(load case 7), 17=-18(load case 5),

18=-111(load case 6), 19=-16(load case 7), 15=-114(load case 7),

14=-12(load case 6)

Max Grav 2=243(load case 1), 12=243(load case 1), 17=73(load case 1), 18=101(load case 10), 19=68(load case 2), 16=73(load case 1), 15=101(load case 11),

14=68(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-74/74, 3-4=-67/84, 4-5=-51/70, 5-6=-37/106, 6-7=-31/100,

7-8=-31/100, 8-9=-37/106, 9-10=-29/39, 10-11=-29/45, 11-12=-56/35, 12-13=0/61

**BOT CHORD** 2-19=-6/132, 18-19=-6/132, 17-18=-6/132, 16-17=-6/132, 15-16=-6/132,

14-15=-6/132, 12-14=-6/132

6-17=-61/27, 5-18=-82/110, 4-19=-43/36, 8-16=-61/1, 9-15=-82/110, 10-14=-43/31

# JOINT STRESS INDEX

**WEBS** 

2 = 0.46, 3 = 0.00, 3 = 0.16, 4 = 0.02, 5 = 0.06, 6 = 0.03, 7 = 0.12, 8 = 0.03, 9 = 0.06, 10 = 0.02, 11 = 0.00, 11 = 0.16, 12 = 0.000.46, 14 = 0.02, 15 = 0.06, 16 = 0.02, 17 = 0.02, 18 = 0.06 and 19 = 0.02

Continued on page 2

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conner Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T01G	GABLE	1	1		J1925157
	1010	O' IDEE			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:05 2008 Page 2

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2, 235 lb uplift at joint 12, 18 lb uplift at joint 17, 111 lb uplift at joint 18, 16 lb uplift at joint 19, 114 lb uplift at joint 15 and 12 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Flonda PE No. 248es 1100 Coasiel Bay Blyd Boynton Beach, Ft. 23435

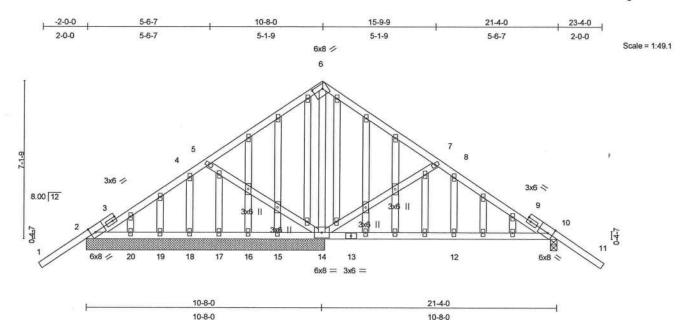


 Job
 Truss
 Truss Type
 Qty
 Ply
 0 0
 J1925158

 T02G
 GABLE
 1
 1
 Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:36:20 2008 Page 1



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	277.5	12-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	0.06	12-14	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.74	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mati	rix)						Weight: 175 lb	

 LUMBER

 TOP CHORD
 2 X 4 SYP No.2

 BOT CHORD
 2 X 4 SYP No.2

 WEBS
 2 X 4 SYP No.3

 OTHERS
 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=567/10-9-8, 14=1392/10-9-8, 15=-4/10-9-8, 16=21/10-9-8, 17=9/10-9-8, 18=293/10-9-8, 19=-16/10-9-8, 20=86/10-9-8, 10=462/0-3-8

Max Horz 2=-243(load case 4)

Max Uplift 2=-387(load case 6), 14=-1071(load case 7), 15=-17(load case 2), 16=-16(load case 5), 18=-204(load case 7), 19=-17(load case 10), 20=-52(load case 7), 10=-445(load case 7)

Max Grav 2=599(load case 10), 14=1392(load case 1), 15=69(load case 5), 16=64(load case 2), 17=32(load case 2), 18=293(load case 1), 19=32(load case 7), 20=90(load case 10), 10=485(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-79/128, 2-3=-323/172, 3-4=-208/106, 4-5=-220/162, 5-6=-237/347, 6-7=-275/378,

7-8=-319/353, 8-9=-378/503, 9-10=-476/474, 10-11=0/62

BOT CHORD 2-20=-177/188, 19-20=-177/188, 18-19=-177/188, 17-18=-177/188, 16-17=-177/188,

15-16=-177/188, 14-15=-177/188, 13-14=-258/356, 12-13=-258/356, 10-12=-258/356

WEBS 5-14=-312/401, 6-14=-900/860, 7-14=-547/783, 4-18=-272/235, 8-12=-275/173

Julius Lee Truse Cesign Engineer Florida PE No. 24868 1100 Ceastal Bay Blvd. Boynton Beach, FL 33435

### JOINT STRESS INDEX

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.

Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T02G	GABLE	1	1		J1925158
	1				Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:36:20 2008 Page 2

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 387 lb uplift at joint 2, 1071 lb uplift at joint 14, 17 lb uplift at joint 15, 16 lb uplift at joint 16, 204 lb uplift at joint 18, 17 lb uplift at joint 19, 52 lb uplift at joint 20 and 445 lb uplift at joint 10.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

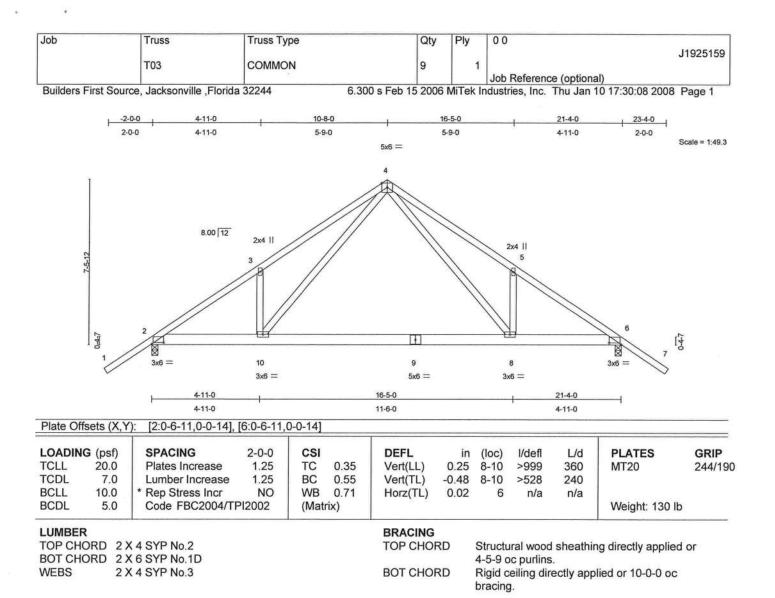
### LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-6=-114(F=-60), 6-7=-141(F=-87), 7-11=-54, 2-10=-10

Julius Les Truss Design Engineer Florida FE No. 34868 1 100 Gestal Bay Blyd





REACTIONS (lb/size) 2=1139/0-3-8, 6=1139/0-3-8

Max Horz 2=-194(load case 4)

Max Uplift 2=-336(load case 6), 6=-336(load case 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-1810/731, 3-4=-1793/903, 4-5=-1793/903, 5-6=-1810/731, 6-7=0/65

BOT CHORD 2-10=-427/1429, 9-10=-116/797, 8-9=-116/797, 6-8=-427/1429

WEBS 3-10=-252/259, 4-10=-493/1023, 4-8=-493/1023, 5-8=-252/259

#### JOINT STRESS INDEX

2 = 0.78, 3 = 0.33, 4 = 0.50, 5 = 0.33, 6 = 0.78, 8 = 0.69, 9 = 0.86 and 10 = 0.69

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

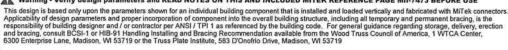
 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Les Truse Cesign Engineer Flonda ME No. 2-1869 1 109 Coastal Bay Blvri Boynton Beach, Ft. 33435

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	00	
	тоз	COMMON	9	1		J1925159
	1.00	- Commerc			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:08 2008 Page 2

### **NOTES**

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 336 lb uplift at joint 2 and 336 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

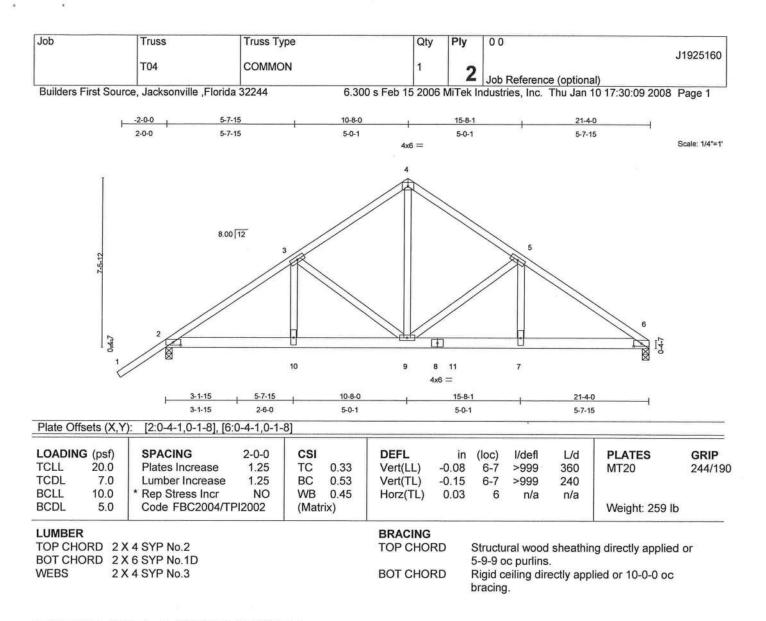
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

Julius Les Truss Design Engineer Florida FE No. 34888 1100 Coastal Bay Blvd





REACTIONS (lb/size) 6=3933/0-3-8, 2=1629/0-3-8

Max Horz 2=223(load case 4)

Max Uplift 6=-1103(load case 6), 2=-487(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/65, 2-3=-2424/606, 3-4=-2197/641, 4-5=-2202/633, 5-6=-4803/1329

**BOT CHORD** 

2-10=-496/1935, 9-10=-496/1935, 8-9=-1036/3941, 8-11=-1036/3941,

7-11=-1036/3941, 6-7=-1036/3941

**WEBS** 

3-10=-18/143, 3-9=-223/109, 4-9=-603/2090, 5-9=-2729/878, 5-7=-784/2784

## JOINT STRESS INDEX

2 = 0.73, 3 = 0.81, 4 = 0.54, 5 = 0.81, 6 = 0.73, 7 = 0.44, 8 = 0.54, 9 = 0.64 and 10 = 0.44

# NOTES

 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads Connected (B), unless otherwise indicated.

Truse Design Engineer Florida PE No. 34868 1100 Coastal Bay Blvd Boynton Besch, FL 33435

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	ACCUSAGE 10. 10 P. L.
	T04	COMMON	1	_		J1925160
	1.2.			2	Job Reference (optional)	

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#### NOTES

- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 3x8 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1103 lb uplift at joint 6 and 487 lb uplift at joint 2.

## LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-11=-10, 6-11=-491(F=-481)

Julius Les Trues Elesian Engineer Flands FE No. 34869 1 109 Coastal Bay Blvri



Builders First Source, Jacksonville ,Florida 32244

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Scale = 1:47.4

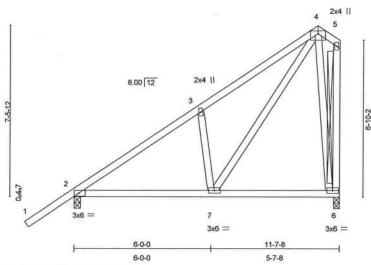


Plate Of	fsets (X, Y	<u>(): [2:0-3-9,0-1-8]</u>										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.02	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	50000000000000000000000000000000000000					Weight: 79 lb	

# LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 \*Except\*

5-6 2 X 4 SYP No.2

## BRACING

**BOT CHORD** 

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 - 5-6

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=495/0-3-8, 6=351/0-3-0

Max Horz 2=286(load case 6)

Max Uplift 2=-141(load case 6), 6=-144(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-439/27, 3-4=-392/176, 4-5=-47/54, 5-6=-88/79

BOT CHORD 2-7=-226/294, 6-7=-41/49

WEBS 3-7=-265/281, 4-7=-270/384, 4-6=-389/350

# JOINT STRESS INDEX

2 = 0.41, 3 = 0.15, 4 = 0.27, 5 = 0.25, 6 = 0.23 and 7 = 0.27

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

Julius Less Truss Design Engineer Flonda FE No. 34869 1109 Coasial Bay Blvd Bovoton Besch, FL 33435



Job	Truss	Truss Type	Qty	Ply	00	000000000000000000000000000000000000000
	T05	COMMON	1	1		J1925161
	100	COMMON		l .	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:10 2008 Page 2

#### **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 2 and 144 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 24869 1160 Ceastal Bay Blvd Bovaton Beach, FL 23425



Job	Truss	Truss Type	Qty	Ply	00	
	T06	MONO HIP	1	1		J1925162
	1.00				Job Reference (optional)	

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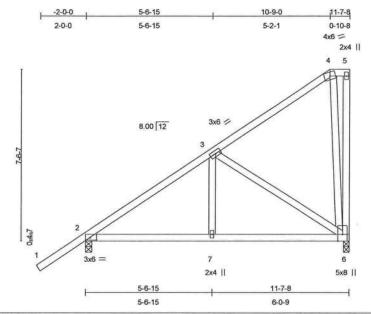


Plate Of	fsets (X,Y	′): [2:0-3-9,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	6-7	>999	240	50500000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.01	6	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 77 lb	

			_	_	_
-	ш	۱л		=	D

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

#### BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

6-0-0 oc punins, except end vertic

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 6=351/0-3-0, 2=495/0-3-8

Max Horz 2=300(load case 6)

Max Uplift 6=-153(load case 6), 2=-133(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-443/0, 3-4=-116/39, 4-5=-8/3, 5-6=-113/87

BOT CHORD 2-7=-226/297, 6-7=-226/297

WEBS 3-7=0/193, 3-6=-319/235, 4-6=-240/275

# JOINT STRESS INDEX

Continued on page 2

2 = 0.32, 3 = 0.14, 4 = 0.44, 5 = 0.21, 6 = 0.11 and 7 = 0.14

# NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Truss Design Engineer Florida PE No. 34849 I 100 Chastal Bay Blvd Boynton Beach, FL 99435

January 10,2008

Scale: 1/4"=1"

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T06	MONO HIP	1	1		J1925162
	100	IMONO TIII			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:10 2008 Page 2

#### **NOTES**

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 6 and 133 lb uplift at joint 2.

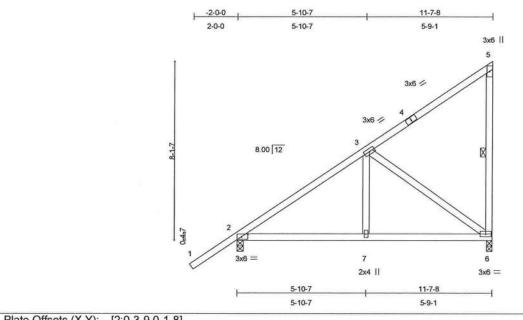
LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
	Т07	MONO TRUSS	8	1		J1925163
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:11 2008 Page 1

1 Row at midpt



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.02	2-7	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.04	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.01	6	n/a	n/a		
BCDL 5.0 Code FBC2004/TPI2002 (Ma		(Mat	rix)	all sold and a second					Weight: 68 lb			
LUMBE	R					BRACING						
TOP CHORD 2 X 4 SYP No.2					TOP CHORD Structural wood sheathing directly applied					d or		
BOT CHORD 2 X 4 SYP No.2					6-0-0 oc purlins, except end verticals.							
WEBS 2 X 4 SYP No.3					вот сно		Rigid ceiling directly applied or 10-0-0 oc bracing.					

WEBS

REACTIONS (lb/size) 6=351/0-3-0, 2=495/0-3-8

Max Horz 2=315(load case 6)

Max Uplift 6=-171(load case 6), 2=-123(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-437/0, 3-4=-109/0, 4-5=-78/59, 5-6=-125/137

BOT CHORD 2-7=-225/291, 6-7=-225/291

WEBS 3-7=0/193, 3-6=-341/262

## JOINT STRESS INDEX

2 = 0.36, 3 = 0.14, 4 = 0.24, 5 = 0.23, 6 = 0.18 and 7 = 0.14

#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Truss Design Engineer Florida FE No. 34989 1 100 Coastel Bay Blvd Boynton Beach, FL 33436

5-6

January 10,2008

Scale = 1:49.4

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	T07	MONO TRUSS	8	1		J1925163
					Job Reference (optional)	

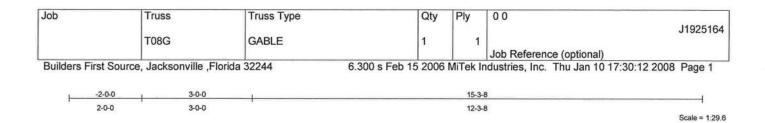
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:11 2008 Page 2

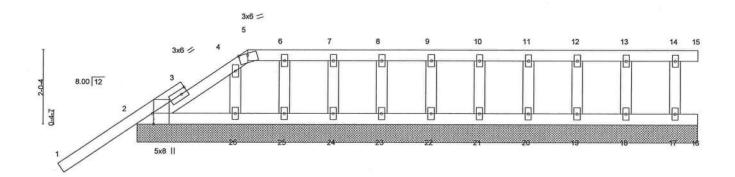
# **NOTES**

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 6 and 123 lb uplift at joint 2.

LOAD CASE(S) Standard







15-3-8 15-3-8

Plate Of	fsets (X, Y	'): [2:0-3-8,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	1	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.02	Horz(TL)	-0.00	15	n/a	n/a		
BCDL			(Mat	rix)						Weight: 73 lb		

LOWIDER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

**BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size)

LIMPED

15=10/15-3-8, 2=239/15-3-8, 16=1/15-3-8, 26=85/15-3-8, 25=88/15-3-8, 24=85/15-3-8, 23=85/15-3-8, 22=85/15-3-8, 21=85/15-3-8, 20=85/15-3-8, 19=85/15-3-8, 18=89/15-3-8, 17=68/15-3-8

Max Horz 2=166(load case 6)

Max Uplift 15=-8(load case 4), 2=-209(load case 6), 26=-29(load case 7), 25=-74(load case 4), 24=-49(load case 4), 23=-54(load case 4), 22=-53(load case 4), 21=-53(load case 4), 20=-53(load case 4), 19=-53(load case 4), 18=-56(load case 4), 17=-41(load case 4)

Max Grav 15=10(load case 11), 2=239(load case 1), 16=4(load case 2), 26=94(load case 2), 25=88(load case 1), 24=86(load case 11), 23=85(load case 1), 22=85(load case 11), 21=85(load case 1), 20=85(load case 11), 19=85(load case 1), 18=89(load case 11), 17=68(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-44/7, 3-4=-29/21, 4-5=-21/7, 5-6=0/0, 6-7=0/0, 7-8=0/0, 8-9=0/0,

9-10=0/0, 10-11=0/0, 11-12=0/0, 12-13=0/0, 13-14=0/0, 14-15=0/0

**BOT CHORD** 2-26=0/0, 25-26=0/0, 24-25=0/0, 23-24=0/0, 22-23=0/0, 21-22=0/0, 20-21=0/0,

19-20=0/0, 18-19=0/0, 17-18=0/0, 16-17=0/0

4-26=-74/49, 6-25=-73/74, 7-24=-72/59, 8-23=-72/61, 9-22=-72/61, 10-21=-72/61,

11-20=-72/61, 12-19=-71/61, 13-18=-76/64, 14-17=-57/48

Continued on page 2

**WEBS** 

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	0.0	
	T08G	GABLE	1	1		J1925164
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:12 2008 Page 2

#### JOINT STRESS INDEX

2 = 0.44, 3 = 0.00, 3 = 0.15, 4 = 0.04, 5 = 0.04, 6 = 0.04, 7 = 0.03, 8 = 0.03, 9 = 0.03, 10 = 0.03, 11 = 0.03, 12 = 0.03, 13 = 0.04, 14 = 0.03, 17 = 0.03, 18 = 0.04, 19 = 0.03, 20 = 0.03, 21 = 0.03, 22 = 0.03, 23 = 0.03, 24 = 0.03, 25 = 0.04 and 26 = 0.03

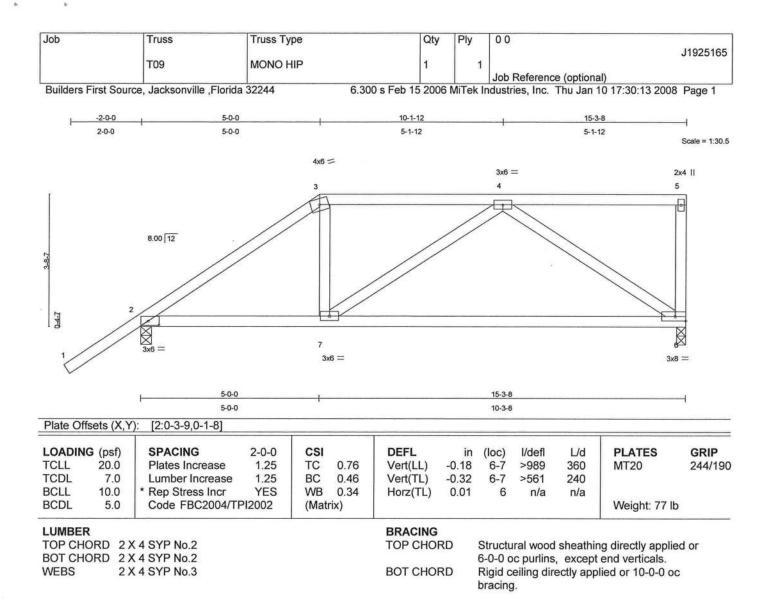
# NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 15, 209 lb uplift at joint 2, 29 lb uplift at joint 26, 74 lb uplift at joint 25, 49 lb uplift at joint 24, 54 lb uplift at joint 23, 53 lb uplift at joint 22, 53 lb uplift at joint 21, 53 lb uplift at joint 20, 53 lb uplift at joint 19, 56 lb uplift at joint 18 and 41 lb uplift at joint 17.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Florida PE No. 24869 1109 Gestal Bay Blvd Boynton Besch, FL 22435





**REACTIONS** (lb/size) 6=471/0-3-0, 2=609/0-3-8

Max Horz 2=177(load case 6)

Max Uplift 6=-141(load case 4), 2=-184(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-667/251, 3-4=-487/263, 4-5=-79/4, 5-6=-127/93

BOT CHORD 2-7=-266/484, 6-7=-298/475

WEBS 3-7=0/193, 4-7=0/132, 4-6=-518/355

#### JOINT STRESS INDEX

2 = 0.43, 3 = 0.38, 4 = 0.16, 5 = 0.79, 6 = 0.62 and 7 = 0.12

### **NOTES**

 Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truss Design Engineer Flonds PE No. 34869 1199 Ceastal Bay Blvd Boynton Beach, FL 33435

January 10,2008

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	Т09	MONO HIP	1	1		J1925165
					Job Reference (optional)	

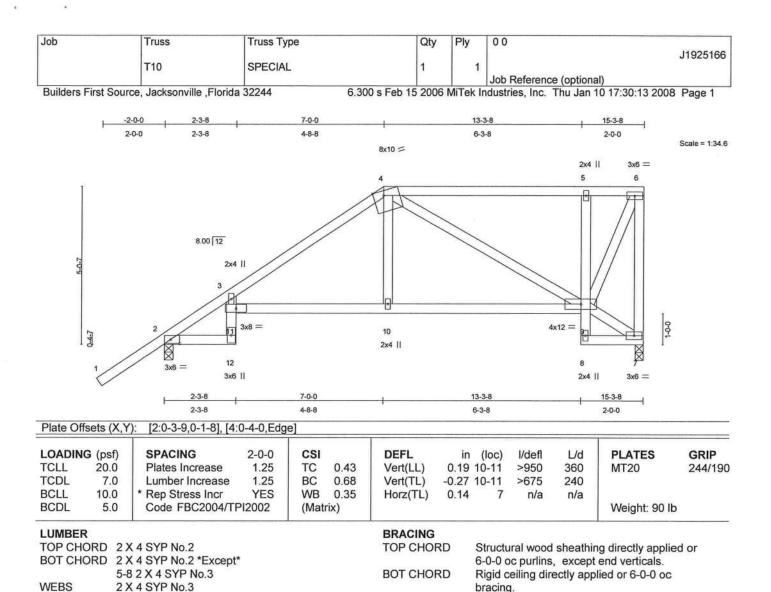
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:13 2008 Page 2

# **NOTES**

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 6 and 184 lb uplift at joint 2.

LOAD CASE(S) Standard





REACTIONS (lb/size) 7=471/0-3-0, 2=609/0-3-8

Max Horz 2=220(load case 6)

Max Uplift 7=-132(load case 5), 2=-186(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-661/162, 3-4=-672/313, 4-5=-226/130, 5-6=-203/118, 6-7=-437/251

**BOT CHORD** 2-12=-247/437, 11-12=-12/84, 3-11=-2/91, 10-11=-348/548, 9-10=-348/554,

8-9=0/19, 5-9=-254/198, 7-8=-59/44

WEBS 4-10=-25/252, 4-9=-380/253, 6-9=-283/484, 7-9=-48/68

# JOINT STRESS INDEX

2 = 0.53, 3 = 0.72, 4 = 0.68, 5 = 0.53, 6 = 0.42, 7 = 0.14, 8 = 0.09, 9 = 0.56, 10 = 0.18, 11 = 0.83 and 12 = 0.36

### NOTES

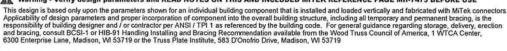
1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colinade page 2

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	00	
	T10	SPECIAL	1	1		J1925166
					Job Reference (optional)	

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# **NOTES**

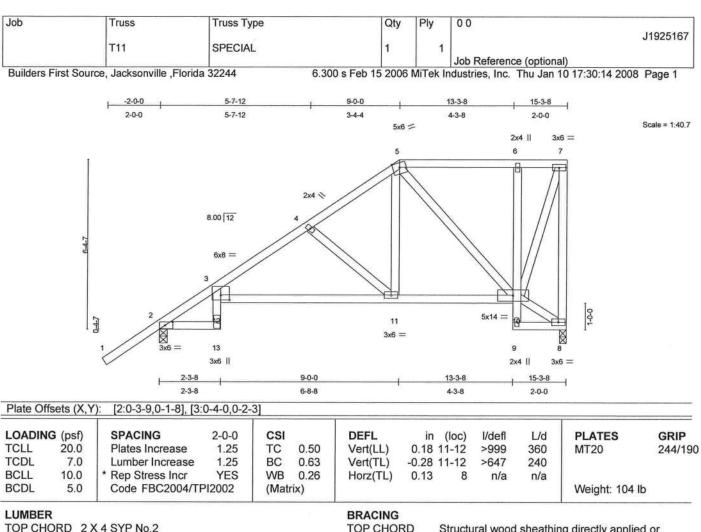
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 7 and 186 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truss Cesign Engineer Flonda PE No. 24869 1100 Coastal Bay Blvd





BOT CHORD 2 X 4 SYP No.2 \*Except\*

6-9 2 X 4 SYP No.3

2 X 4 SYP No.3

**WEBS** 

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

**BOT CHORD** 

Rigid ceiling directly applied or 9-1-8 oc

bracing.

REACTIONS

(lb/size) 8=471/0-3-0, 2=609/0-3-8

Max Horz 2=263(load case 6)

Max Uplift 8=-133(load case 5), 2=-182(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-648/113, 3-4=-699/298, 4-5=-514/245, 5-6=-152/95, 6-7=-142/89,

7-8=-433/276

**BOT CHORD** 2-13=-258/417, 12-13=-14/90, 3-12=0/156, 11-12=-427/595, 10-11=-237/362,

9-10=0/24, 6-10=-200/150, 8-9=-38/50

**WEBS** 4-11=-291/239, 5-11=-152/347, 5-10=-319/216, 7-10=-276/437, 8-10=-55/42

# JOINT STRESS INDEX

2 = 0.47, 3 = 0.82, 4 = 0.13, 5 = 0.20, 6 = 0.30, 7 = 0.41, 8 = 0.14, 9 = 0.09, 10 = 0.35, 11 = 0.22, 12 = 0.00 and 13 = 0.37

#### NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	00	
	T11	SPECIAL	1	1		J1925167
	1/2/2015				Job Reference (optional)	

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#### NOTES

- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 8 and 182 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	So the broad with our money.
	T12	SPECIAL	1	1		J1925168
	1112	or Eon te	"		Job Reference (optional)	

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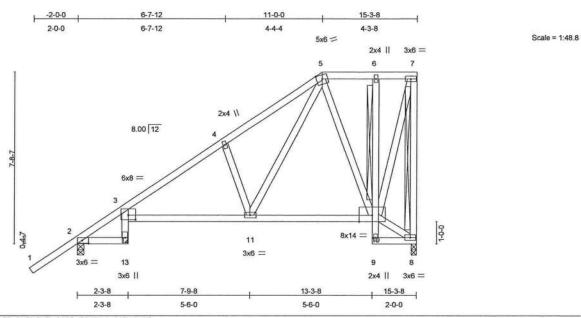


Plate Of	fsets (X,Y	(): [2:0-3-9,0-1-8], [3:	0-4-0,0-2-	7]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.21	11-12	>851	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.29	11-12	>616	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.14	8	n/a	n/a	-	
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 113 lb	

LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 \*Except\* 6-0-0 oc purlins, except end verticals. 6-9 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc **WEBS** 2 X 4 SYP No.3 bracing. Except: T-Brace: 2 X 4 SYP No.3 -6-10

T-Brace:

**WEBS** 2 X 4 SYP No.3 - 7-8 Fasten T and I braces to narrow edge of web

> with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=471/0-3-0, 2=609/0-3-8

Max Horz 2=305(load case 6)

Max Uplift 8=-139(load case 6), 2=-171(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-657/68, 3-4=-661/225, 4-5=-619/317, 5-6=-110/69, 6-7=-107/69,

7-8=-438/289

**BOT CHORD** 2-13=-283/431, 12-13=-14/86, 3-12=0/115, 11-12=-403/545, 10-11=-165/225,

9-10=0/20, 6-10=-97/69, 8-9=-47/36

**WEBS** 4-11=-308/283, 5-11=-324/488, 5-10=-323/267, 7-10=-259/401, 8-10=-38/52

## JOINT STRESS INDEX

2 = 0.48, 3 = 0.94, 4 = 0.15, 5 = 0.26, 6 = 0.12, 7 = 0.39, 8 = 0.15, 9 = 0.06, 10 = 0.12, 11 = 0.39, 12 = 0.00 and 13 = 0.36 January 10,2008 Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	00	
	T12	SPECIAL	1	1		J1925168
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:15 2008 Page 2

#### **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 lb uplift at joint 8 and 171 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Flonds PE No. 24889 1109 Ceastel Bay Blvd Bovnton Besch, FL 22425



Job	Truss	Truss Type	Qty	Ply	00	
	T13	SPECIAL	1	1		J1925169
	113	SPECIAL		1	Job Reference (optional)	

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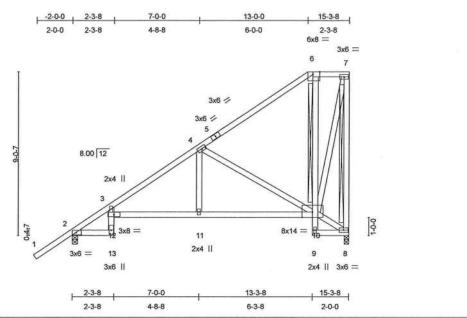


Plate Offsets (X,Y): [2:0-3-9,0-1-8], [6:0-4-8,Edge]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.20	11-12	>894	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.26	11-12	>685	240	A CONTRACTOR OF THE CONTRACTOR	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.14	8	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 110 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
WERS	2 X 4 SYP No 3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc

> bracing. Except: T-Brace:

2 X 4 SYP No.3 -

Scale = 1:60.0

6-10

**WEBS** 

T-Brace:

2 X 4 SYP No.3 - 7-8 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=471/0-3-0, 2=609/0-3-8

Max Horz 2=349(load case 6)

Max Uplift 8=-185(load case 6), 2=-153(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-661/15, 3-4=-678/175, 4-5=-231/0, 5-6=-128/12, 6-7=-96/69,

7-8=-434/296

BOT CHORD 2-13=-299/438, 12-13=-14/83, 3-12=-7/85, 11-12=-414/552, 10-11=-414/552,

9-10=0/18, 6-10=-177/198, 8-9=-75/44

**WEBS** 4-11=-34/255, 4-10=-513/387, 7-10=-317/437, 8-10=-49/86

2 = 0.47, 3 = 0.63, 4 = 0.19, 5 = 0.29, 6 = 0.77, 7 = 0.43, 8 = 0.15, 9 = 0.08, 10 = 0.14, 11 = 0.18, 12 = 0.85 and 13 = 0.36 January 10,2008 Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T13	SPECIAL	1	1		J1925169
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:16 2008 Page 2

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 8 and 153 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truss Design Endineer Flonda PE No. 34866 1 100 Coastel Bay Blvd Bovaton Beach, FL 33436



Job	Truss	Truss Type	Qty	Ply	00	
		175 CAN TO 175 CAN TO 175 CAN				J1925170
	T14	MONO HIP	1	1		
	1				Job Reference (optional)	

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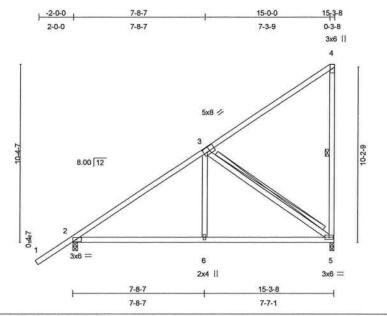


Plate Of	ffsets (X, Y	<u>(): [2:0-3-9,0-1-8], [3:</u>	0-2-0,0-3-	0]							T	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.06	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.13	2-6	>999	240	1000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 89 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS

1 Row at midpt

4-5

T-Brace:

2 X 4 SYP No.3 - 3-5

Scale: 3/16"=1"

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 5=471/0-3-0, 2=609/0-3-8

Max Horz 2=393(load case 6)

Max Uplift 5=-233(load case 6), 2=-126(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-592/0, 3-4=-141/75, 4-5=-157/165

BOT CHORD 2-6=-299/398, 5-6=-301/395

WEBS 3-6=0/261, 3-5=-472/359

# JOINT STRESS INDEX

2 = 0.45, 3 = 0.79, 4 = 0.32, 5 = 0.27 and 6 = 0.18

Julius Les Truse Design Engineer Florida PE No. 34868 1169 Coastal Bay Blvd Boynton Besch, FL 93435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	v "zazvanyan artificani sa
	T14	MONO HIP	1	1		J1925170
					Job Reference (optional)	

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#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 5 and 126 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34868 1 100 Ceestel Bey Blyd Boynton Besch, FL 93435



Job	Truss	Truss Type	Qty	Ply	00	900 0 00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
	T15	MONO HIP	1	1		J1925171
	113	WICHO THE			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:17 2008 Page 1

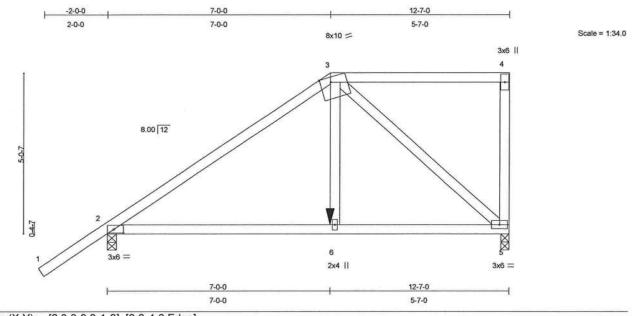


Plate Of	fsets (X, Y	(): [2:0-3-9,0-1-8], [3:	0-4-0,Edg	ej								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.05	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.10	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.76	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 67 lb	

LUMBER	L	U	N	1	В	E	R
--------	---	---	---	---	---	---	---

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

#### BRACING

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 5=937/0-3-0, 2=792/0-3-8

Max Horz 2=221(load case 5)

Max Uplift 5=-458(load case 4), 2=-340(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-937/354, 3-4=-53/38, 4-5=-300/245

**BOT CHORD** 2-6=-340/694, 5-6=-346/710

**WEBS** 3-6=-189/504, 3-5=-872/431

# JOINT STRESS INDEX

2 = 0.55, 3 = 0.82, 4 = 0.48, 5 = 0.41 and 6 = 0.36

# NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 458 Coltinate shiping and 340 lb uplift at joint 2.

January 10,2008

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Job	Truss	Truss Type	Qty	Ply	00	
	T15	MONO HIP	1	1		J1925171
					Job Reference (optional)	

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### **NOTES**

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-118(F=-64), 2-6=-10, 5-6=-22(F=-12)

Concentrated Loads (lb)

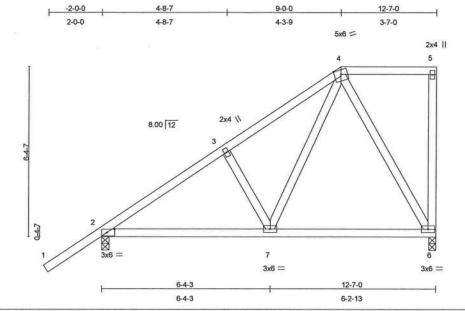
Vert: 6=-411(F)

Julius Lee Truse Design Engineer Flonda PE No. 24869 1109 Coestal Bay Blvd Boynton Beach, FL 22426



Job	Truss	Truss Type	Qty	Ply	00	
	T16	MONO HIP	2	1		J1925172
			-		Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.05	2-7	>999	240	1000 C 1000 C 1000 C	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.01	6	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 77 lb	

2 X 4 SYP No.2
2 X 4 SYP No.2
2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

**REACTIONS** (lb/size) 6=382/0-3-0, 2=524/0-3-8

Max Horz 2=263(load case 6)

Max Uplift 6=-110(load case 6), 2=-161(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-503/102, 3-4=-383/142, 4-5=-11/1, 5-6=-76/57

BOT CHORD 2-7=-271/354, 6-7=-128/164

WEBS 3-7=-189/195, 4-7=-124/280, 4-6=-318/256

#### JOINT STRESS INDEX

2 = 0.52, 3 = 0.11, 4 = 0.32, 5 = 0.22, 6 = 0.21 and 7 = 0.23

# NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truss Design Engineer Flonda PE No. 24869 1169 Coestal Bay Blyd Boynton Beach, Ft. 33436

January 10,2008

Scale = 1:40.7

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T16	MONO HIP	2	1		J1925172
	110	IMONO TIII	-		Job Reference (optional)	

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# **NOTES**

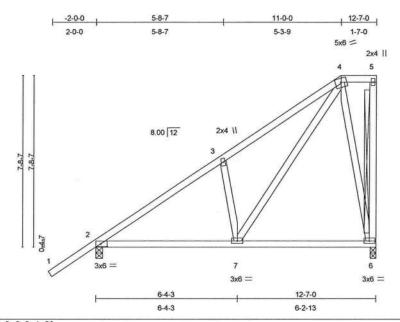
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 6 and 161 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
			1700			J1925173
	T17	MONO HIP	2	1		
	1.000 1100	- Constant Anna C			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:18 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.05	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.00	6	n/a	n/a		
BCDL 5.0 Code FBC2004/TPI2002		(Matrix)		8 8					Weight: 84 lb			

LUMBER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS T-Brace:

2 X 4 SYP No.3 - 5-6

Scale = 1:48.8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 6=382/0-3-0, 2=524/0-3-8

Max Horz 2=305(load case 6)

Max Uplift 6=-152(load case 6), 2=-142(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-487/27, 3-4=-428/167, 4-5=-8/0, 5-6=-44/28

BOT CHORD 2-7=-257/332, 6-7=-66/77

WEBS 3-7=-269/282, 4-7=-267/392, 4-6=-386/352

### JOINT STRESS INDEX

2 = 0.44, 3 = 0.15, 4 = 0.31, 5 = 0.18, 6 = 0.27 and 7 = 0.29

Julius Lee Truss Cesian Engineer Florida PE No. 24898 1109 Ceastal Bay Blvd Boynton Beach, FL 22435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T17	MONO HIP	2	1		J1925173
	1 10 1000				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:18 2008 Page 2

# **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 152 lb uplift at joint 6 and 142 lb uplift at joint 2.

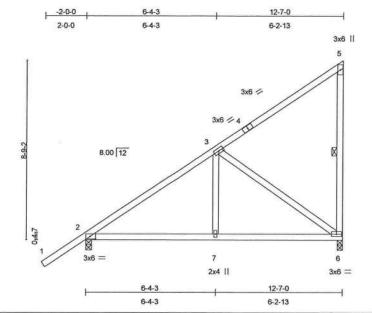
LOAD CASE(S) Standard

Julius Lee Trues Cesian Engineer Florida PE No. 24869 1109 Coestal Bay Blvd. Boynton Geach, Ft. 23435



Job	Truss	Truss Type	Qty	Ply	00	
	T18	MONO TRUSS	6	1		J1925174
	100000				Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.06	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mati	rix)	n 15					Weight: 74 lb	
LUMBE	R					BRACING						
	UMBER OP CHORD 2 X 4 SYP No.2							Structu	ral wood	sheathir	ng directly applie	d or

BOT CHORD 2 X 4 SYP No.2

**WEBS** 

2 X 4 SYP No.3

**BOT CHORD** 

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

**WEBS** 1 Row at midpt 5-6

REACTIONS (lb/size) 6=382/0-3-0, 2=524/0-3-8

Max Horz 2=335(load case 6)

Max Uplift 6=-187(load case 6), 2=-124(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-479/0, 3-4=-118/0, 4-5=-84/64, 5-6=-135/146

**BOT CHORD** 2-7=-247/321, 6-7=-247/321

WEBS 3-7=0/210, 3-6=-376/288

### JOINT STRESS INDEX

2 = 0.38, 3 = 0.16, 4 = 0.09, 5 = 0.26, 6 = 0.19 and 7 = 0.15

# **NOTES**

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

Scale = 1:53.1

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T18	MONO TRUSS	6	1		J1925174
		A CONTRACTOR AND A CONT			Job Reference (optional)	

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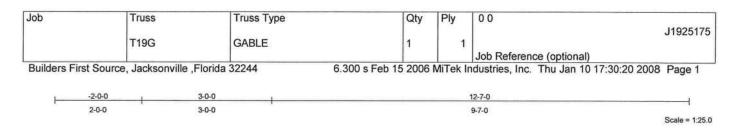
# **NOTES**

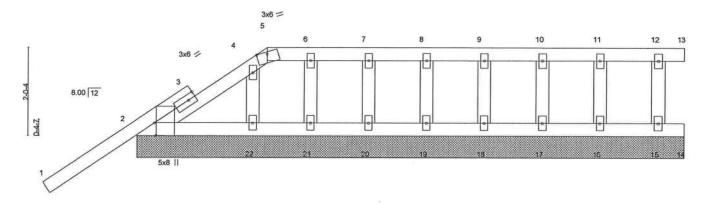
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 6 and 124 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Florida PE No. 24859 1109 Coestal Bay Blyd Boynton Beach, Ft. 22435







12-7-0 12-7-0

Plate Of	ffsets (X, Y	/): [2:0-3-8,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	1	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.02	Horz(TL)	-0.00	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)					1,1000	Weight: 60 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

6-0-0 oc purlin

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size)

13=8/12-7-0, 2=239/12-7-0, 14=1/12-7-0, 22=85/12-7-0, 21=88/12-7-0, 20=85/12-7-0, 19=85/12-7-0, 18=85/12-7-0, 17=85/12-7-0, 16=90/12-7-0, 15=67/12-7-0

Max Horz 2=166(load case 6)

Max Uplift 13=-7(load case 4), 2=-209(load case 6), 22=-29(load case 7), 21=-74(load case 4), 20=-49(load case 4), 19=-54(load case 4), 18=-53(load case 4), 17=-53(load case 4), 15=-41(load case 4)

Max Grav 13=8(load case 11), 2=239(load case 1), 14=3(load case 2), 22=94(load case 2), 21=88(load case 1), 20=86(load case 11), 19=85(load case 1), 18=86(load case 11), 17=85(load case 1), 16=90(load case 11), 15=67(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-44/7, 3-4=-29/21, 4-5=-21/7, 5-6=0/0, 6-7=0/0, 7-8=0/0, 8-9=0/0,

9-10=0/0, 10-11=0/0, 11-12=0/0, 12-13=0/0

BOT CHORD 2-22=0/0, 21-22=0/0, 20-21=0/0, 19-20=0/0, 18-19=0/0, 17-18=0/0, 16-17=0/0,

15-16=0/0, 14-15=0/0

4-22=-74/49, 6-21=-73/79, 7-20=-72/59, 8-19=-72/62, 9-18=-72/61, 10-17=-71/61,

11-16=-76/65, 12-15=-56/47

Julius Lee Trues Design Engineer Florida FE No. 24869 1199 Coasial Bay Blvd Boynton Besch, FL 93435

Continued on page 2

**WEBS** 



Job	Truss	Truss Type	Qty	Ply	00	000000000000000000000000000000000000000
	T19G	GABLE	1	1		J1925175
	1250.80837				Job Reference (optional)	

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### JOINT STRESS INDEX

2 = 0.45, 3 = 0.00, 3 = 0.15, 4 = 0.04, 5 = 0.04, 6 = 0.04, 7 = 0.03, 8 = 0.03, 9 = 0.03, 10 = 0.03, 11 = 0.04, 12 = 0.03, 15 = 0.03, 16 = 0.04, 17 = 0.03, 18 = 0.03, 19 = 0.03, 20 = 0.03, 21 = 0.04 and 22 = 0.03

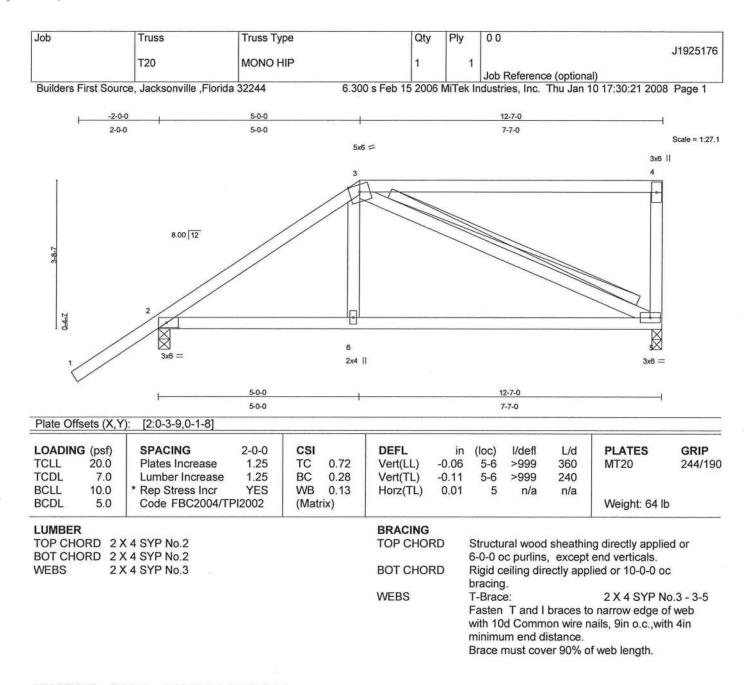
#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 13, 209 lb uplift at joint 2, 29 lb uplift at joint 22, 74 lb uplift at joint 21, 49 lb uplift at joint 20, 54 lb uplift at joint 19, 53 lb uplift at joint 18, 53 lb uplift at joint 17, 56 lb uplift at joint 16 and 41 lb uplift at joint 15.

LOAD CASE(S) Standard

Julius Law Truse Design Engineer Florida PE No. 34869 1109 Coastal Bay Blvd Boynton Beach, FL 33435





REACTIONS (lb/size) 5=382/0-3-0, 2=524/0-3-8

Max Horz 2=177(load case 6)

Max Uplift 5=-108(load case 5), 2=-173(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-526/202, 3-4=-80/50, 4-5=-200/167

BOT CHORD 2-6=-238/381, 5-6=-242/379

WEBS 3-6=0/207, 3-5=-330/211

# JOINT STRESS INDEX

2 = 0.61, 3 = 0.69, 4 = 0.54, 5 = 0.54 and 6 = 0.15

Trues Design Engineer Florida FE No. 34888 1 1109 Cassial Bay Blvd Boynton Besch, FL 33435

January 10,2008

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	00	
	T20	MONO HIP	1	1		J1925176
					Job Reference (optional)	

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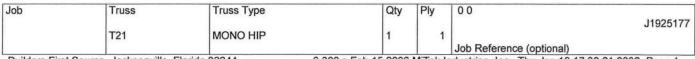
## **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 5 and 173 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 24869 1109 Coestal Bay Blvd Boynton Besch, Ft. 23436





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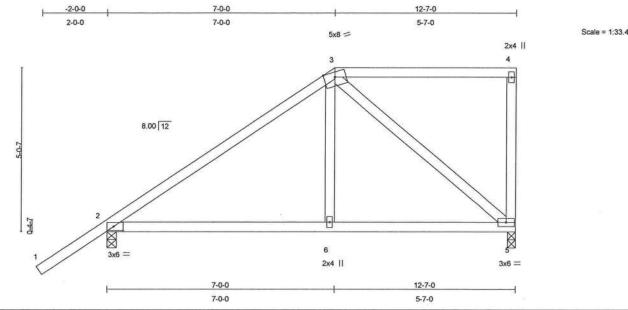


Plate Of	fsets (X,Y	'): [2:0-3-9,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.04	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.09	2-6	>999	240	CAMPICTIPOWAL .	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.34	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 67 lb	

		_	
	M		

**WEBS** 

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3

#### BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 5=382/0-3-0, 2=524/0-3-8

Max Horz 2=220(load case 6)

Max Uplift 5=-108(load case 5), 2=-171(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-460/134, 3-4=-17/15, 4-5=-119/96

BOT CHORD 2-6=-202/295, 5-6=-203/292

WEBS 3-6=0/213, 3-5=-366/257

## JOINT STRESS INDEX

2 = 0.41, 3 = 0.51, 4 = 0.56, 5 = 0.16 and 6 = 0.15

# NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Truse Cesign Engineer Florida PE No. 34868 1100 Caastel Bay Blvd. Boynton Beach, FL 33436

January 10,2008

🚵 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	T21	MONO HIP	1	1		J1925177
					Job Reference (optional)	

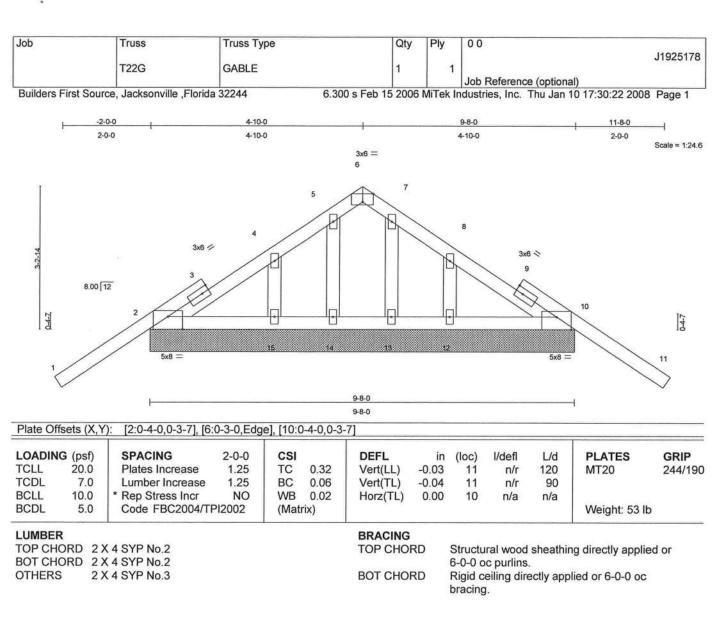
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:21 2008 Page 2

# **NOTES**

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 5 and 171 lb uplift at joint 2.

LOAD CASE(S) Standard





REACTIONS (lb/size) 2=245/9-8-0, 10=245/9-8-0, 14=76/9-8-0, 15=101/9-8-0, 13=76/9-8-0,

12=101/9-8-0

Max Horz 2=108(load case 5)

Max Uplift 2=-217(load case 6), 10=-231(load case 7), 14=-36(load case 6),

15=-58(load case 7), 13=-16(load case 7), 12=-51(load case 6)

Max Grav 2=245(load case 1), 10=245(load case 1), 14=76(load case 1), 15=101(load case 2), 13=76(load case 1), 12=101(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-55/68, 3-4=-45/80, 4-5=-32/98, 5-6=-27/100, 6-7=-27/100,

7-8=-32/98, 8-9=-12/55, 9-10=-54/36, 10-11=0/61

**BOT CHORD** 2-15=-7/122, 14-15=-7/122, 13-14=-7/122, 12-13=-7/122, 10-12=-7/122

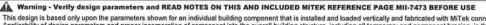
**WEBS** 5-14=-62/30, 4-15=-87/76, 7-13=-62/9, 8-12=-87/69

## JOINT STRESS INDEX

2 = 0.70, 3 = 0.00, 3 = 0.16, 4 = 0.04, 5 = 0.03, 6 = 0.12, 7 = 0.03, 8 = 0.04, 9 = 0.00, 9 = 0.16, 10 = 0.70, 12 = 0.04, 13 = 0.0214 = 0.02 and 15 = 0.04

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2







Job	Truss	Truss Type	Qty	Ply	00	
	T22G	GABLE	1	1		J1925178
	1220	ONDEE			Job Reference (optional)	

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#### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 2, 231 lb uplift at joint 10, 36 lb uplift at joint 14, 58 lb uplift at joint 15, 16 lb uplift at joint 13 and 51 lb uplift at joint 12.

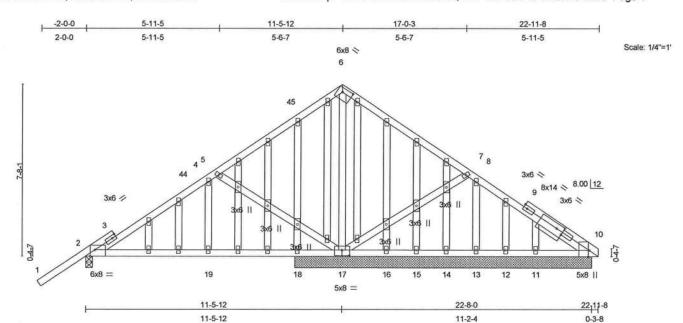
LOAD CASE(S) Standard

Julius Les Trues Design Engineer Flonda PE No. 34868 1100 Coastal Bay Blvd Boynton Beach, FL 22425



Job	Truss	Truss Type	Qty	Ply	00	10°54-32004-3-00-0
	T23G	GABLE	1	1		J1925179
					Job Reference (optional)	

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LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.69	Vert(LL)	0.06	2-19	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.04	2-19	>999	240	200 g to 4400 C 4400	
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.87	Horz(TL)	-0.01	2	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Mati	rix)						Weight: 196 lb	

LOINDLIN	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

LUMBER

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0

oc purlins.

**BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing,

Except:

10-0-0 oc bracing: 2-19,18-19 9-7-6 oc bracing: 17-18.

REACTIONS (lb/size) 10=249/13-3-8, 17=1373/13-3-8, 16=20/13-3-8, 15=12/13-3-8, 14=10/13-3-8, 13=488/13-3-8, 12=-48/13-3-8, 11=133/13-3-8, 18=35/13-3-8, 2=526/0-3-8

Max Horz 18=286(load case 5)

Max Uplift 10=-147(load case 7), 17=-1062(load case 7), 13=-351(load case 7), 12=-48(load

case 1), 11=-87(load case 7), 18=-49(load case 4), 2=-476(load case 6)

Max Grav 10=289(load case 11), 17=1373(load case 1), 16=57(load case 2), 15=35(load case 2), 14=39(load case 2), 13=494(load case 11), 12=58(load case 7), 11=133(load case 1), 18=95(load case 2), 2=540(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 6-7=-269/370, 7-8=-173/88, 8-9=-163/180, 9-10=-247/59, 1-2=0/62, 2-3=-555/528,

3-44=-430/542, 4-44=-421/547, 4-5=-346/384, 5-45=-152/178, 6-45=-286/384

**BOT CHORD** 2-19=-306/420, 18-19=-306/420, 17-18=-430/420, 16-17=-51/107, 15-16=-51/107,

14-15=-51/107, 13-14=-51/107, 12-13=-51/107, 11-12=-51/107, 10-11=-51/107

WEBS 7-17=-213/300, 6-17=-914/877, 5-17=-656/893, 8-13=-464/440, 4-19=-236/153 em lesign Engineer PE No. 34869 mastal Bay Blvd n Besch, ft 33406

### JOINT STRESS INDEX

2 = 0.53, 3 = 0.00, 3 = 0.23, 4 = 0.34, 5 = 0.48, 6 = 0.94, 7 = 0.34, 8 = 0.34, 9 = 0.00, 9 = 0.58, 9 = 0.58, 10 = 0.29, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.55, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.62, 24 = 0.34, 25 = 0.62, 26 = 0.34, 27 = 0.62, 28 = 0.34, 29 = 0.68, 30 = 0.34, 31 = 0.34, 32 = 0.62, 33 = 0.34, 34 = 0.62, 35 = 0.34, 36 = 0.34, 37 and 29 = 0.62, 31 = 0.34, 31 = 0.34, 32 = 0.62, 33 = 0.34, 34 = 0.62, 35 = 0.34, 36 = 0.34, 37 and 39 = 0.34, 31 = 0.34, 31 = 0.34, 32 = 0.62, 33 = 0.34, 34 = 0.62, 35 = 0.34, 36 = 0.34, 37 and 39 = 0.34, 31 = 0.34, 31 = 0.34, 32 = 0.62, 33 = 0.34, 34 = 0.62, 35 = 0.34, 36 = 0.34, 37 and 39 = 0.34, 31 = 0.3Continued on page 20.34, 40 = 0.34, 41 = 0.34, 42 = 0.34 and 43 = 0.34

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T23G	GABLE	1	1		J1925179
					Job Reference (optional)	

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## NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable studs spaced at 1-4-0 oc.

7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint 10, 1062 lb uplift at joint 17, 351 lb uplift at joint 13, 48 lb uplift at joint 12, 87 lb uplift at joint 11, 49 lb uplift at joint 18 and 476 lb uplift at joint 2.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

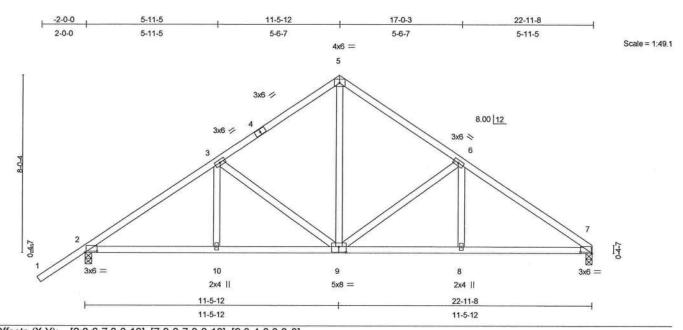
Vert: 6-10=-114(F=-60), 1-44=-54, 44-45=-141(F=-87), 6-45=-114(F=-60), 2-10=-10

Julius Lee Trues Design Engineer Flonda PE No. 34869 1109 Ceastal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00	
	T24	COMMON	1	1		J1925180
					Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.13	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	0.11	7-8	>999	240	1	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.90	Horz(TL)	0.03	2	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	PI2002	(Mati	rix)	7.1020×1000					Weight: 121 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD

Structural wood sheathing directly applied or 5-9-13

oc purlins.

**BOT CHORD** Rigid ceiling directly applied or 6-3-6 oc bracing.

REACTIONS (lb/size) 7=719/0-3-8, 2=851/0-3-8

Max Horz 7=239(load case 5)

Max Uplift 7=-435(load case 7), 2=-536(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

5-6=-740/962, 6-7=-1080/1295, 1-2=0/62, 2-3=-1057/1260, 3-4=-739/937, 4-5=-655/960

**BOT CHORD** 

2-10=-851/797, 9-10=-851/797, 8-9=-889/822, 7-8=-889/822

**WEBS** 

6-9=-359/551, 5-9=-864/433, 3-9=-327/505, 3-10=-276/182, 6-8=-305/187

### JOINT STRESS INDEX

2 = 0.71, 3 = 0.43, 4 = 0.40, 5 = 0.50, 6 = 0.43, 7 = 0.71, 8 = 0.34, 9 = 0.35 and 10 = 0.34

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 435 lb uplift at Coinintia and 536 buplift at joint 2.

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connec Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erecti and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	0.000 (0.
	T24	COMMON	1	1		J1925180
li,		COMMON			Job Reference (optional)	

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LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
	T25	SPECIAL	1	1		J1925181
				17.	Job Reference (optional)	

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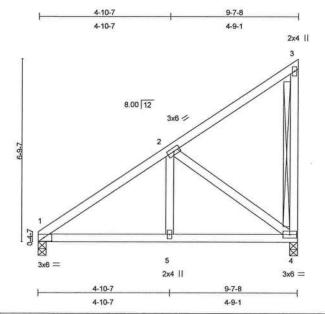


Plate Of	fsets (X,Y	(): [1:0-3-9,0-1-8]				-						
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	0.05	1-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	0.04	1-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.17	Horz(TL)	-0.00	1	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	7					Weight: 53 lb	

	-	-	_	2
u	JIV	ıĸ	-	к

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 **WEBS** 

# BRACING

TOP CHORD

**BOT CHORD** 

**WEBS** 

Rigid ceiling directly applied or 10-0-0 oc

bracing. T-Brace:

2 X 4 SYP No.3 - 3-4

Scale = 1:40.3

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=299/0-3-8, 4=299/0-3-8

Max Horz 4=204(load case 6)

Max Uplift 1=-130(load case 6), 4=-269(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-377/298, 2-3=-90/48, 3-4=-100/107

**BOT CHORD** 1-5=-170/257, 4-5=-170/257

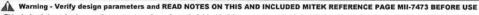
**WEBS** 2-5=-307/163, 2-4=-305/560

# JOINT STRESS INDEX

1 = 0.33, 2 = 0.29, 3 = 0.58, 4 = 0.38 and 5 = 0.12

January 10,2008

Continued on page 2



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	2 <b>4</b> 000000000000000000000000000000000000
	T25	SPECIAL	1	1		J1925181
		Control of Control			Job Reference (optional)	

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## **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 1 and 269 lb uplift at joint 4.

LOAD CASE(S) Standard

Julius Les Trues Design Engineer Florida PE No. 34899 1169 Coastal Bay Blyd





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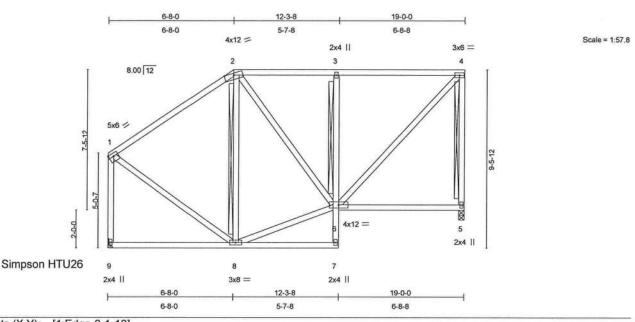


Plate Of	fsets (X, Y	(): [1:Edge,0-1-12]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.05	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.08	5-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.57	Horz(TL)	-0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 149 lb	

11	JIV	ID	<b>D</b>
	JIV	10	•

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

3-7 2 X 4 SYP No.3

**WEBS** 

2 X 4 SYP No.3 \*Except\* 1-9 2 X 4 SYP No.2

# BRACING

**WEBS** 

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing, Except:

6-0-0 oc bracing: 7-8.

T-Brace: T-Brace: 2 X 4 SYP No.3 - 3-6 2 X 4 SYP No.3 - 4-5,

2-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

**REACTIONS** (lb/size) 5=599/0-3-8, 9=599/Mechanical

Max Horz 5=140(load case 6)

Max Uplift 5=-181(load case 5), 9=-88(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 4-5=-559/367, 2-3=-379/236, 3-4=-389/242, 1-2=-458/211, 1-9=-561/248

BOT CHORD 5-6=-13/179, 6-7=0/64, 3-6=-338/234, 8-9=-31/39, 7-8=-22/16

WEBS 4-6=-337/537, 6-8=-50/301, 2-6=-106/132, 2-8=-239/98, 1-8=-68/324

#### JOINT STRESS INDEX

1 = 0.74, 2 = 0.92, 3 = 0.48, 4 = 0.65, 5 = 0.68, 6 = 0.54, 7 = 0.36, 8 = 0.56 and 9 = 0.76

Julius Lee Truse Design Engineer Flonds PE No. 34866 1109 Coasial Bay Blyd Goynton Beach, FL 33435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T26	SPECIAL	1	1		J1925182
		00			Job Reference (optional)	

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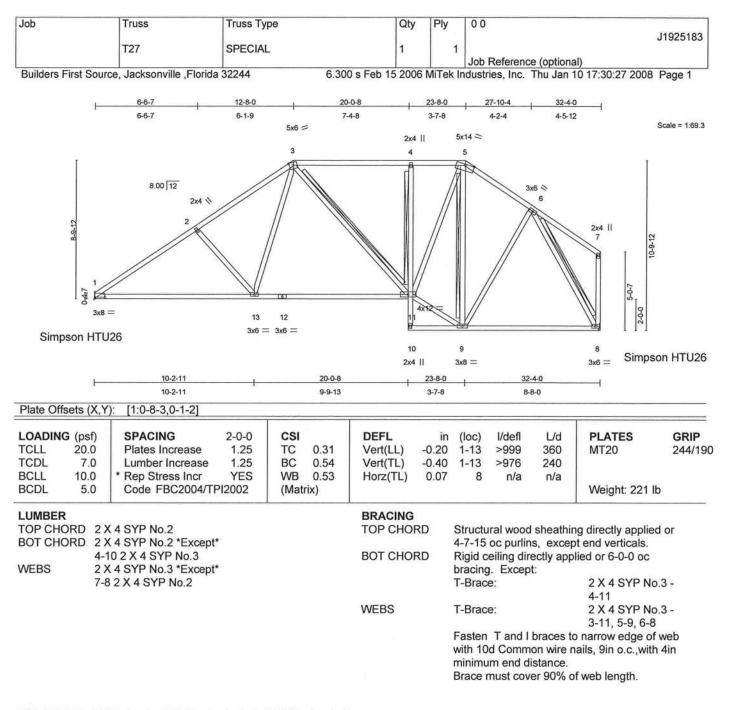
#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 5 and 88 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 24868 1189 Coastal Bay Blvd Boviton Beach Et 23436





REACTIONS (lb/size) 1=1028/Mechanical, 8=1028/Mechanical

Max Horz 1=233(load case 5)

Max Uplift 1=-198(load case 6), 8=-173(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1565/752, 2-3=-1355/745, 3-4=-938/626, 4-5=-925/620, 5-6=-821/524,

6-7=-90/82, 7-8=-125/105

BOT CHORD 1-13=-643/1241, 12-13=-400/952, 11-12=-400/952, 10-11=-16/0, 4-11=-337/220,

9-10=-90/0, 8-9=-236/503

WEBS 2-13=-291/294, 3-13=-177/419, 3-11=-146/128, 5-11=-365/810, 5-9=-507/204,

9-11=-224/722, 6-9=-113/289, 6-8=-983/474

Trues Cesign Engineer Florida PE No. 24868 1100 Ceastal Bay Blvd. Boynton Beach, FL 33425

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T27	SPECIAL	1	1		J1925183
	121	SPECIAL			Job Reference (optional)	

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### JOINT STRESS INDEX

1 = 0.67, 2 = 0.33, 3 = 0.71, 4 = 0.72, 5 = 0.38, 6 = 0.35, 7 = 0.60, 8 = 0.64, 9 = 0.67, 10 = 0.47, 11 = 0.96, 12 = 0.37 and 13 = 0.49

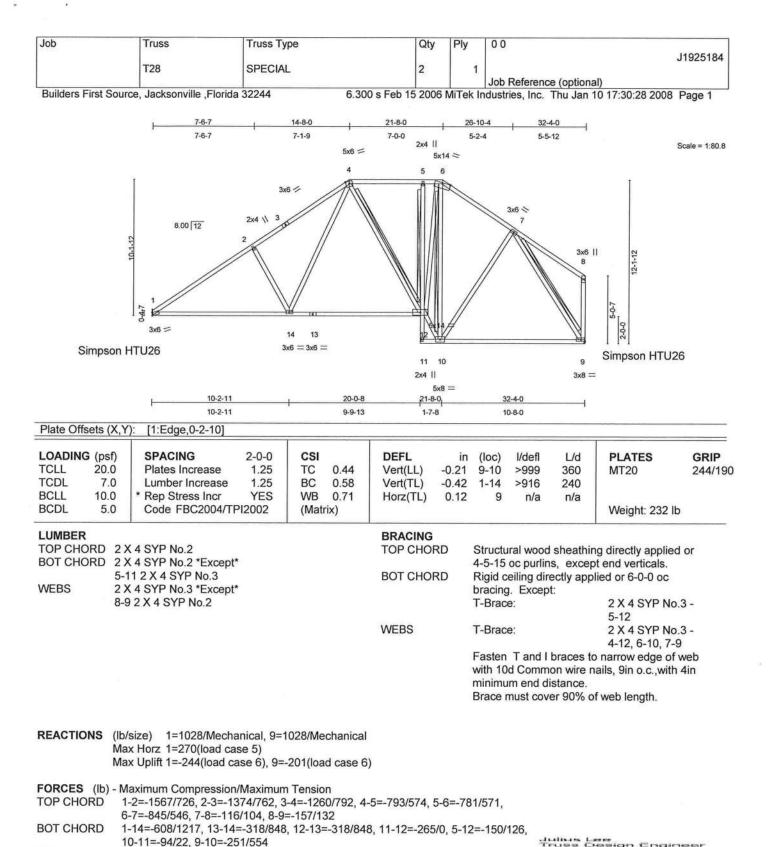
# NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 198 lb uplift at joint 1 and 173 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34868 1109 Coestel Bey Blyd Boynton Besch El 33435





Continued on page 2

**WEBS** 

January 10,2008



6-10=-878/273, 7-10=-70/216, 7-9=-972/456

2-14=-354/355, 4-14=-291/484, 4-12=-190/156, 6-12=-397/1068, 10-12=-272/1082,

Job	Truss	Truss Type	Qty	Ply	0.0	The appendix work year
	T28	SPECIAL	2	1		J1925184
	2000		1000	- 1	Job Reference (optional)	

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#### JOINT STRESS INDEX

1 = 0.81, 2 = 0.33, 3 = 0.39, 4 = 0.69, 5 = 0.52, 6 = 0.79, 7 = 0.34, 8 = 0.26, 9 = 0.62, 10 = 0.55, 11 = 0.73, 12 = 0.85, 13 = 0.35 and 14 = 0.46

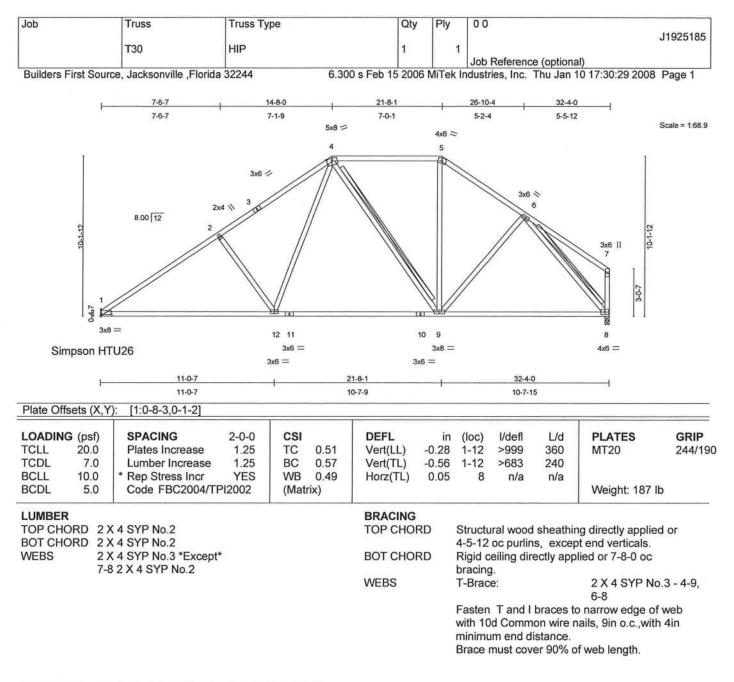
#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 244 lb uplift at joint 1 and 201 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 24869 1100 Coastal Bay Blvd





**REACTIONS** (lb/size) 1=1028/Mechanical, 8=1028/0-3-8

Max Horz 1=270(load case 5)

Max Uplift 1=-208(load case 6), 8=-185(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1557/728, 2-3=-1323/721, 3-4=-1128/752, 4-5=-768/562, 5-6=-993/594,

6-7=-172/122, 7-8=-195/147

1-12=-609/1210, 11-12=-319/854, 10-11=-319/854, 9-10=-319/854, 8-9=-326/723

WEBS 2-12=-350/351, 4-12=-250/465, 4-9=-243/175, 5-9=-103/267, 6-9=-103/171,

6-8=-1011/480

# JOINT STRESS INDEX

**BOT CHORD** 

1 = 0.76, 2 = 0.33, 3 = 0.38, 4 = 0.64, 5 = 0.62, 6 = 0.35, 7 = 0.34, 8 = 0.74, 9 = 0.56, 10 = 0.45, 11 = 0.65 and 12 = 0.48

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T30	HIP	1	1		J1925185
	100				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:29 2008 Page 2

# NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

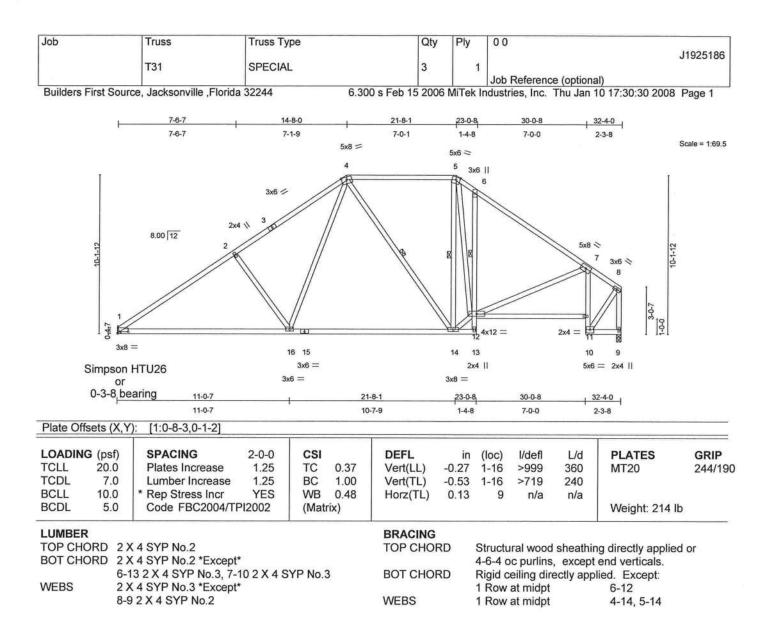
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 208 lb uplift at joint 1 and 185 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida FE No. 34869 1109 Coastal Bay Blyd





REACTIONS (lb/size) 1=1025/Mechanical, 9=1025/0-3-8

Max Horz 1=316(load case 5)

Max Uplift 1=-246(load case 6), 9=-197(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1545/730, 2-3=-1313/724, 3-4=-1119/754, 4-5=-765/570, 5-6=-1019/745,

6-7=-1164/607, 7-8=-658/355, 8-9=-1143/532

BOT CHORD 1-16=-504/1196, 15-16=-220/849, 14-15=-220/849, 13-14=-157/16, 12-13=-188/0,

6-12=-289/255, 11-12=-245/673, 10-11=-610/301, 7-11=-585/325, 9-10=-24/59

WEBS 2-16=-341/345, 4-16=-242/465, 4-14=-233/165, 5-14=-320/89, 5-12=-355/637,

12-14=-94/931, 7-12=-93/256, 8-10=-416/917

Julius Lee Truss Design Engineer Flonda FE No. 24889 1109 Chastal Bay Blyd Governor Beach, Et. 23425

# JOINT STRESS INDEX

1 = 0.73, 2 = 0.33, 3 = 0.38, 4 = 0.61, 5 = 0.49, 6 = 0.25, 7 = 0.52, 8 = 0.69, 9 = 0.42, 10 = 0.56, 11 = 0.74, 12 = 0.70, 13 = 0.62, 14 = 0.88, 15 = 0.67 and 16 = 0.48

# NOTES

1) Unbalanced roof live loads have been considered for this design. Continued on page 2

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.

Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T31	SPECIAL	2	4	J192518	36
	131	SPECIAL	3		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:30 2008 Page 2

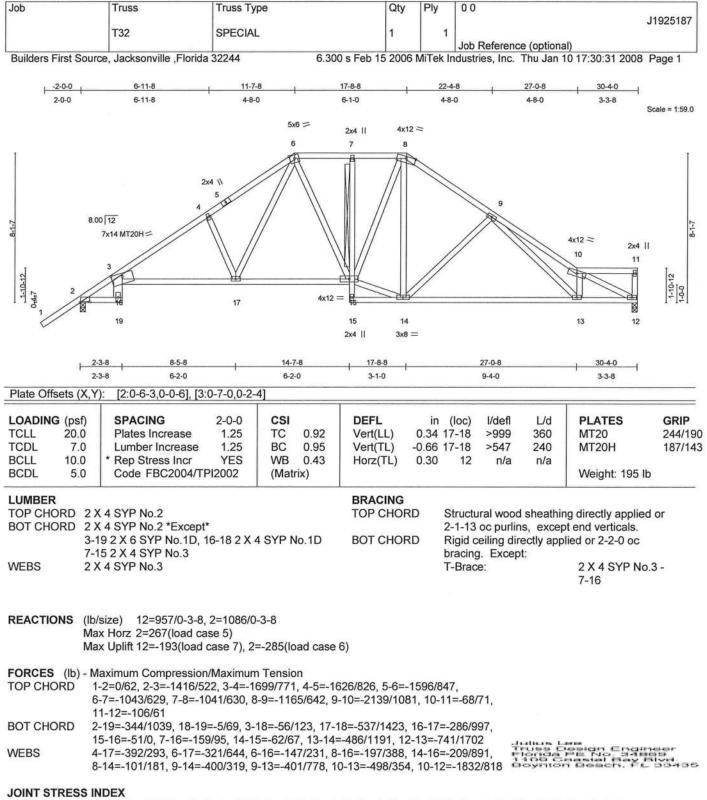
## NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 1 and 197 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Florida PE No. 34869 1109 Coastel Bay Blvd Boynton Beach, FL 33435





2 = 0.66, 3 = 0.91, 4 = 0.33, 5 = 0.42, 6 = 0.38, 7 = 0.33, 8 = 0.66, 9 = 0.51, 10 = 0.66, 11 = 0.67, 12 = 0.51, 13 = 0.47, 14 = 0.80, 15 = 0.41, 16 = 0.77, 17 = 0.53, 18 = 0.00 and 19 = 0.56

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	William Control
	T32	SPECIAL	1	1		J1925187
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:31 2008 Page 2

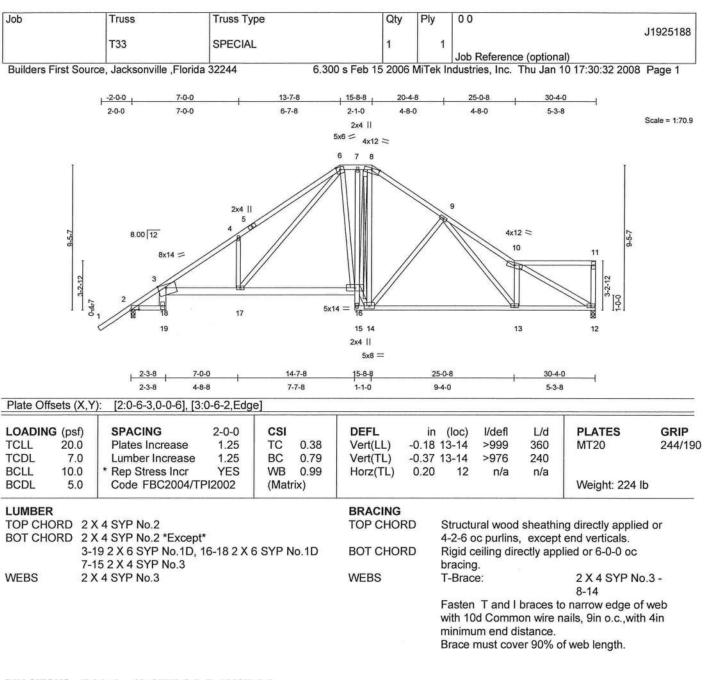
# NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 3x6 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 12 and 285 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda PE No. 24869 1100 Caestel Bay Blvd Boynton Besch, FL 23436





**REACTIONS** (lb/size) 12=957/0-3-8, 2=1086/0-3-8

Max Horz 2=325(load case 5)

Max Uplift 12=-202(load case 7), 2=-295(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1424/512, 3-4=-1809/788, 4-5=-1885/986, 5-6=-1784/1014,

6-7=-872/574, 7-8=-860/570, 8-9=-1032/601, 9-10=-1807/960, 10-11=-50/83.

11-12=-144/100

BOT CHORD 2-19=-346/1051, 18-19=-5/65, 3-18=-191/159, 17-18=-547/1485, 16-17=-214/888,

15-16=-337/0, 7-16=-249/321, 14-15=-101/166, 13-14=-387/1057, 12-13=-600/1429

WEBS 4-17=-378/351, 6-17=-525/939, 6-16=-274/233, 8-16=-242/607, 14-16=-191/1059,

8-14=-377/194, 9-14=-418/341, 9-13=-369/629, 10-13=-434/346, 10-12=-1586/721

Trues Design Engineer Florida PE No. 34888 1100 Crastal Bay Blvd Boynton Beach, Et. 33435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T33	SPECIAL	1	1		J1925188
				· ·	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:32 2008 Page 2

# JOINT STRESS INDEX

2 = 0.66, 3 = 0.84, 4 = 0.33, 5 = 0.39, 6 = 0.69, 7 = 0.33, 8 = 0.62, 9 = 0.42, 10 = 0.57, 11 = 0.29, 12 = 0.45, 13 = 0.43, 14 = 0.54, 15 = 0.49, 16 = 0.57, 17 = 0.63, 18 = 0.00 and 19 = 0.56

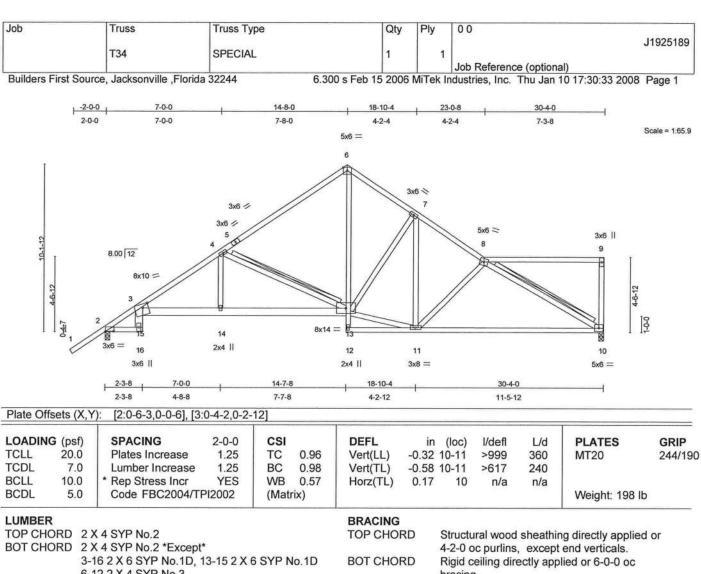
# NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 12 and 295 lb uplift at joint 2.

LOAD CASE(S) Standard

dalius Lee Truse Design Engineer Honda PE No. 24869 1109 Coestal Bay Blvd Boynton Beach, FL 23436





6-12 2 X 4 SYP No.3

**WEBS** 2 X 4 SYP No.3 bracing.

**WEBS** 

T-Brace: 2 X 4 SYP No.3 -4-13, 8-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 10=957/0-3-8, 2=1086/0-3-8

Max Horz 2=363(load case 5)

Max Uplift 10=-207(load case 7), 2=-299(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1423/506, 3-4=-1823/799, 4-5=-1119/545, 5-6=-1002/577,

6-7=-1044/606, 7-8=-1247/648, 8-9=-84/99, 9-10=-193/137

2-16=-348/1050, 15-16=-6/66, 3-15=-187/156, 14-15=-571/1504, 13-14=-571/1504, 12-13=-17/7, 6-13=-407/756, 11-12=-82/83, 10-11=-529/1249

4-14=-55/411, 4-13=-739/437, 7-13=-298/227, 11-13=-226/918, 7-11=-106/216, **WEBS** 

8-11=-398/326, 8-10=-1373/687

# JOINT STRESS INDEX

**BOT CHORD** 

NT STRESS INDEX
2 = 0.66, 3 = 0.85, 4 = 0.41, 5 = 0.31, 6 = 0.88, 7 = 0.41, 8 = 0.66, 9 = 0.51, 10 = 0.57, 11 = 0.81, 12 = 0.44, 13 = 0.66, 14 =

January 10,2008 Conthadd 55 5 2002 and 16 = 0.56

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T34	SPECIAL	1	1		J1925189
		0. =0=			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:33 2008 Page 2

# NOTES

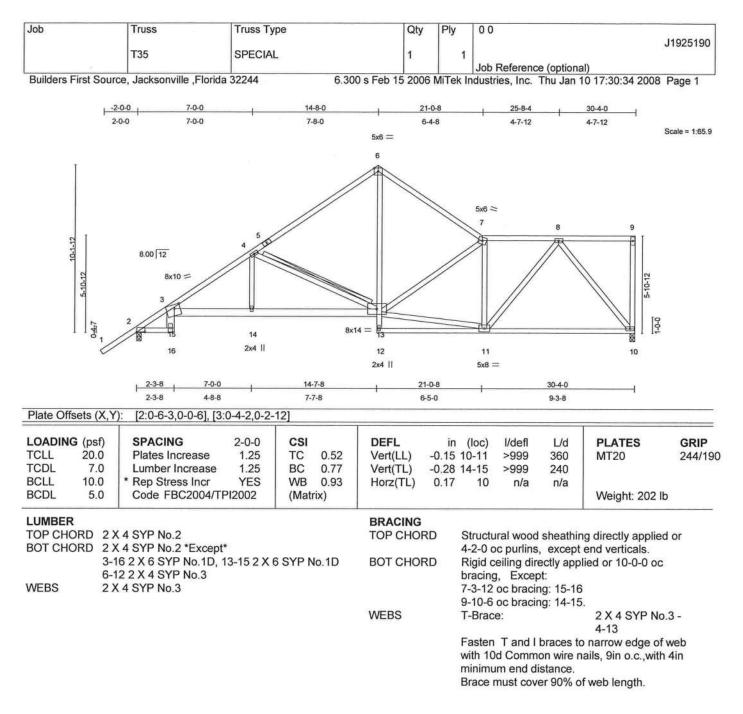
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 207 lb uplift at joint 10 and 299 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34889 1100 Coesial Bay Blvd Boyston Bassel PE 188438





REACTIONS (lb/size) 10=957/0-3-8, 2=1086/0-3-8

Max Horz 2=384(load case 5)

Max Uplift 10=-216(load case 5), 2=-300(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1423/508, 3-4=-1825/804, 4-5=-1115/547, 5-6=-1001/579,

6-7=-1090/591, 7-8=-1104/625, 8-9=-50/111, 9-10=-116/99

BOT CHORD 2-16=-380/1050, 15-16=-9/65, 3-15=-191/160, 14-15=-582/1504, 13-14=-582/1504,

12-13=0/78, 6-13=-366/712, 11-12=-63/144, 10-11=-217/661

WEBS 4-14=-62/420, 4-13=-741/437, 7-13=-383/293, 11-13=-366/990, 7-11=-642/355,

8-11=-316/707, 8-10=-1021/505

Truss Design Engineer Florida FE No. 34865 1 100 Geastal Bay Blvd Boynton Beach, FL 33435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T35	SPECIAL	1	1		J1925190
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:34 2008 Page 2

# JOINT STRESS INDEX

2 = 0.66, 3 = 0.85, 4 = 0.41, 5 = 0.31, 6 = 0.98, 7 = 0.57, 8 = 0.48, 9 = 0.28, 10 = 0.63, 11 = 0.44, 12 = 0.80, 13 = 0.58, 14 = 0.33, 15 = 0.00 and 16 = 0.56

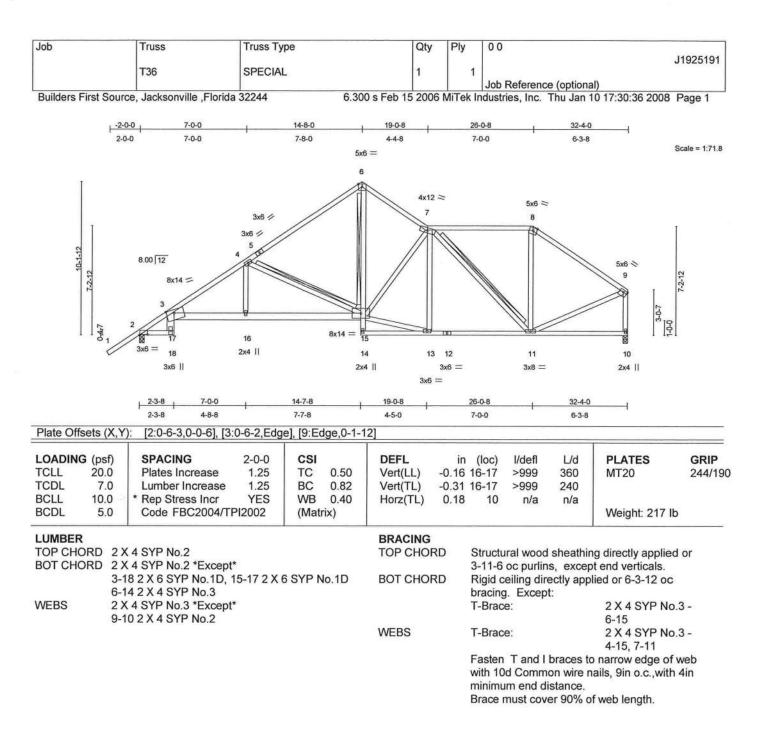
# NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216 lb uplift at joint 10 and 300 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truze Design Engineer Florida PE No. 34889 1109 Cassial Bay Blvd Boynton Beach, FL 33435





REACTIONS (lb/size) 2=1150/0-3-8, 10=1021/0-3-8

Max Horz 2=341(load case 5)

Max Uplift 2=-308(load case 6), 10=-227(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1530/564, 3-4=-1972/876, 4-5=-1243/612, 5-6=-1125/644,

6-7=-1185/678, 7-8=-779/520, 8-9=-1015/526, 9-10=-988/494

BOT CHORD 2-18=-385/1132, 17-18=-8/69, 3-17=-199/161, 16-17=-628/1630, 15-16=-628/1630,

14-15=0/57, 6-15=-492/919, 13-14=-77/133, 12-13=-406/1163, 11-12=-406/1163,

10-11=0/56

4-16=-63/429, 4-15=-766/449, 7-15=-429/324, 13-15=-342/1064, 7-13=-205/122,

7-11=-546/242, 8-11=-19/257, 9-11=-265/788

Truse Design Engineer Florida PE No. 24869 1 100 Chastel Bay Blvd. Boynton Beach, FL 33435

Continued on page 2

**WEBS** 



Job	Truss	Truss Type	Qty	Ply	00	
	T36	SPECIAL		1		J1925191
	136	SPECIAL		1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:36 2008 Page 2

# JOINT STRESS INDEX

2 = 0.71, 3 = 0.88, 4 = 0.41, 5 = 0.34, 6 = 0.83, 7 = 0.73, 8 = 0.72, 9 = 0.70, 10 = 0.74, 11 = 0.71, 12 = 0.39, 13 = 0.57, 14 = 0.52, 15 = 0.74, 16 = 0.33, 17 = 0.00 and 18 = 0.61

## NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2 and 227 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34869 1 109 Coestal Bay Blvd Boynton Beach, Ft. 33435



Job Truss Truss Type Qty Ply 00 J1925192 T37 SPECIAL 1 1 Job Reference (optional) Builders First Source, Jacksonville ,Florida 32244 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:37 2008 Page 1 -2-0-0 7-0-0 12-3-8 18-10-0 24-0-8 30-0-8 32-4-0 6-6-8 2-3-8 Scale = 1:65.8 2x4 || 5x6 = 5x6 = 2x4 II 3x8 = 8 10 2x4 II 8.00 12 8x14 = 3 21 19 17 13 2x4 || 3x8 = 2x4 || 2x4 || 5x6 = 7-0-0 14-7-8 18-10-0 23-0-8 30-0-8 32-4-0 2-3-8 2-3-8 4-8-8 7-7-8 4-2-8 4-2-8 7-0-0 2-3-8 Plate Offsets (X,Y): [2:0-6-3,0-0-6], [3:0-6-2,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL GRIP I/defl L/d **PLATES** in (loc) TCLL 20.0 Plates Increase 1.25 TC 0.38 Vert(LL) 0.18 21-22 >999 360 MT20 244/190 TCDL 1.25 BC 0.96 >999 240 7.0 Lumber Increase Vert(TL) -0.33 21-22 **BCLL** 10.0 Rep Stress Incr YES WB 0.67 0.25 Horz(TL) n/a 13 n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 257 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 \*Except\* 3-11-15 oc purlins, except end verticals. Rigid ceiling directly applied or 2-2-0 oc 3-23 2 X 6 SYP No.1D, 20-22 2 X 6 SYP No.1D **BOT CHORD** 7-19 2 X 4 SYP No.3, 9-17 2 X 4 SYP No.3 bracing. Except: 11-14 2 X 4 SYP No.3 T-Brace: 2 X 4 SYP No.3 -**WEBS** 2 X 4 SYP No.3 \*Except\* 7-20, 9-16 12-13 2 X 4 SYP No.2 **WEBS** T-Brace: 2 X 4 SYP No.3 -8-18, 8-16, 10-15 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1150/0-3-8, 13=1021/0-3-8

Max Horz 2=297(load case 5)

Max Uplift 2=-297(load case 6), 13=-173(load case 4)

FORCES (Ib) - Maximum Compression/Maximum Tension

**WEBS** 

TOP CHORD 1-2=0/62, 2-3=-1531/583, 3-4=-1953/885, 4-5=-2013/1063, 5-6=-1927/1085,

6-7=-1117/685, 7-8=-1118/688, 8-9=-933/602, 9-10=-939/603, 10-11=-865/579,

11-12=-656/358, 12-13=-1137/533

BOT CHORD 2-23=-449/1134, 22-23=-12/69, 3-22=-208/174, 21-22=-623/1601, 20-21=-393/1074

, 19-20=0/60, 7-20=-161/105, 18-19=-53/101, 17-18=-67/0, 16-17=0/59,

9-16=-80/101, 15-16=-247/878, 14-15=-588/283, 11-15=-288/275, 13-14=-23/54

4-21=-349/316, 6-21=-488/888, 6-20=-186/273, 8-20=-145/320, 18-20=-268/900,

Continued on page 18=-445/173, 8-16=-152/99, 16-18=-294/1007, 10-16=-231/506, 10-15=-411/204,

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors, Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, Wi 53719 or the Truss Plate Institute, 883 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T37	SPECIAL	1	1		J1925192
		0. 20			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:37 2008 Page 2

# JOINT STRESS INDEX

2 = 0.71, 3 = 0.91, 4 = 0.33, 5 = 0.39, 6 = 0.46, 7 = 0.33, 8 = 0.59, 9 = 0.33, 10 = 0.62, 11 = 0.50, 12 = 0.67, 13 = 0.40, 14 = 0.55, 15 = 0.69, 16 = 0.82, 17 = 0.46, 18 = 0.89, 19 = 0.50, 20 = 0.65, 21 = 0.62, 22 = 0.00 and 23 = 0.61

## NOTES

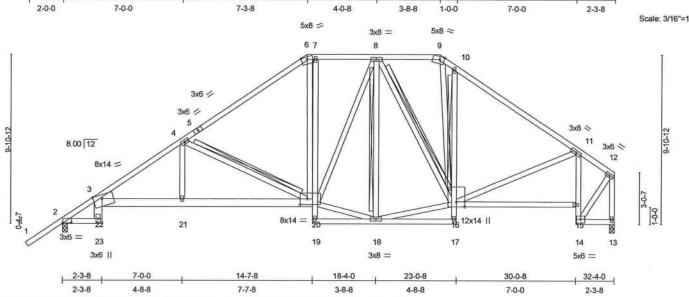
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 3.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 173 lb uplift at joint 13.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 24869 1109 Ceasial Bay Blyd Boynton Basco E. 19445



Ply Job Truss Truss Type Qty 00 J1925193 T38 SPECIAL Job Reference (optional) Builders First Source, Jacksonville ,Florida 32244 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:42:47 2008 Page 1 -2-0-0 7-0-0 14-3-8 18-4-0 22-0-8 23-0-€ 30-0-8 32-4-0 2-0-0 7-0-0 7-3-8 4-0-8 1-0-0 3-8-8 7-0-0 2-3-8 Scale: 3/16"=1" 5x8 =



Tiate Of	fsets (X,Y)	: [2:0-6-3,0-0-6], [3:	3-0-2,Eugej	, 10.0-0-0	, Lugej, įz	1					
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (lo	c) I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.16 21-2	2 >999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.30 21-2	2 >999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.25 1	3 n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/7	PI2002	(Mat	rix)					Weight: 267 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2

**WEBS** 

BOT CHORD 2 X 4 SYP No.2 \*Except\*

3-23 2 X 6 SYP No.1D, 20-22 2 X 6 SYP No.1D 7-19 2 X 4 SYP No.3, 10-17 2 X 4 SYP No.3

11-14 2 X 4 SYP No.3 2 X 4 SYP No.3 \*Except\* 12-13 2 X 4 SYP No.2

BRACING

TOP CHORD

**BOT CHORD** 

**WEBS** 

oc purlins, except end verticals. Rigid ceiling directly applied or 1-7-8 oc bracing.

Except:

T-Brace: T-Brace: 2 X 4 SYP No.3 - 10-16 2 X 4 SYP No.3 - 4-20,

8-18, 8-16

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Structural wood sheathing directly applied or 3-11-8

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1150/0-3-8, 13=1021/0-3-8

Max Horz 2=334(load case 5)

Max Uplift 2=-306(load case 6), 13=-179(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1530/572, 3-4=-1970/885, 4-5=-1245/623, 5-6=-1135/654, 6-7=-958/640,

7-8=-946/629, 8-9=-808/567, 9-10=-1078/772, 10-11=-1160/601, 11-12=-655/351,

12-13=-1138/526

**BOT CHORD** 2-23=-426/1132, 22-23=-11/69, 3-22=-202/165, 21-22=-633/1625, 20-21=-633/1625,

19-20=0/48, 7-20=-238/247, 18-19=-56/98, 17-18=-46/25, 16-17=0/69, 10-16=-287/315,

15-16=-241/668, 14-15=-607/298, 11-15=-584/323, 13-14=-24/59

**WEBS** 4-21=-68/428, 4-20=-752/438, 8-18=-374/137, 18-20=-192/774, 8-20=-153/308,

16-18=-225/834, 8-16=-174/124, 9-16=-398/497, 11-16=-131/262, 12-14=-411/912,

6-20=-129/254

January 10,2008

Continued on page 2

Warming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	71616111
	T38	SPECIAL	1	1		J1925193
	1.00				Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:42:47 2008 Page 2

## JOINT STRESS INDEX

2 = 0.73, 3 = 0.90, 4 = 0.43, 5 = 0.32, 6 = 0.64, 7 = 0.34, 8 = 0.65, 9 = 0.35, 10 = 0.75, 11 = 0.95, 12 = 0.71, 13 = 0.43, 14 = 0.57, 15 = 0.77, 16 = 0.30, 17 = 0.61, 18 = 0.75, 19 = 0.40, 20 = 0.89, 21 = 0.34, 22 = 0.00 and 23 = 0.63

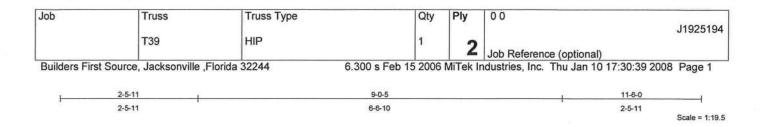
## NOTES

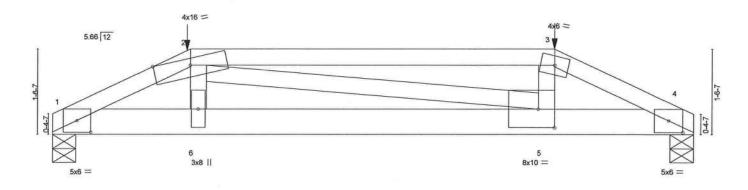
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 306 lb uplift at joint 2 and 179 lb uplift at joint 13.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda FE No. 34869 1100 Ceestel Bay Blvd Boynton Besch, Ft, 33435







1	2-5-11	9-0-5	11-6-0
	2-5-11	6-6-10	2-5-11

Plate Offsets (X,Y)	[1:0-3-0,0-2-9],	[4:0-3-0,0-2-9], [5:0-3-8,0-4-0]
---------------------	------------------	----------------------------------

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.12	5-6	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.24	5-6	>563	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.42	Horz(TL)	0.03	4	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TI	212002	(Mat	rix)	000000000000000000000000000000000000000					Weight: 113 lb	

LUMBE	D

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D WEBS 2 X 4 SYP No.3

# BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-3-6 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=3052/0-4-15, 4=3052/0-4-15

Max Horz 1=14(load case 5)

Max Uplift 1=-832(load case 4), 4=-832(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-6331/1744, 2-3=-5815/1616, 3-4=-6176/1700

BOT CHORD

1-6=-1558/5636, 5-6=-1641/5961, 4-5=-1509/5500

WEBS

2-6=-677/2613, 2-5=-192/57, 3-5=-653/2517

# JOINT STRESS INDEX

1 = 0.75, 2 = 0.74, 3 = 0.63, 4 = 0.73, 5 = 0.26 and 6 = 0.41

# NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

8) Hinhalanged ager live loads have been considered for this design.

Julius Les Truse Cesign Engineer Flonda PE No. 34868 1109 Cassial Bay Blvd Bovnton Besch. Ft. 33435

January 10,2008

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	Name of the last o
	T39	HIP	1			J1925194
	100	7.00		2	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:39 2008 Page 2

# NOTES

- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) Provide adequate drainage to prevent water ponding.
- 6) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 832 lb uplift at joint 1 and 832 lb uplift at joint 4.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

# LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-2=-54, 2-3=-56(F=-2), 3-4=-54, 1-4=-491(F=-481)

Concentrated Loads (lb)

Vert: 2=-22(F) 3=-22(F)

Julius Lee Truss Design Engineer Florida PE No. 34899 1100 Ceastel Bay Blyd Boyston Beach, FL 93435

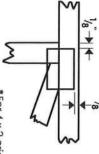


# Symbols

# PLATE LOCATION AND ORIENTATION



 Center plate on joint unless securely seat. plates to both sides of truss and Dimensions are in inches. Apply dimensions indicate otherwise.



\*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



\*This symbol indicates the required direction of slots in connector plates.

# PLATE SIZE

4 × 4

to slots. dimension is the length parallel perpendicular to slots. Second The first dimension is the width

# LATERAL BRACING



continuous lateral bracing. Indicates location of required

NER NER

561

WISC/DILHR

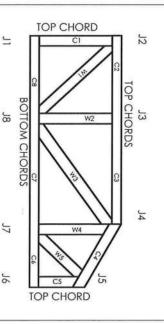
960022-W, 970036-N

# BEARING



Indicates location of joints at

# Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

# CONNECTOR PLATE CODE APPROVALS

BOCA 96-31, 96-67

ICBO

SBCCI

3907, 4922

9667, 9432A



MiTek Engineering Reference Sheet: MII-7473

# General Safety Notes

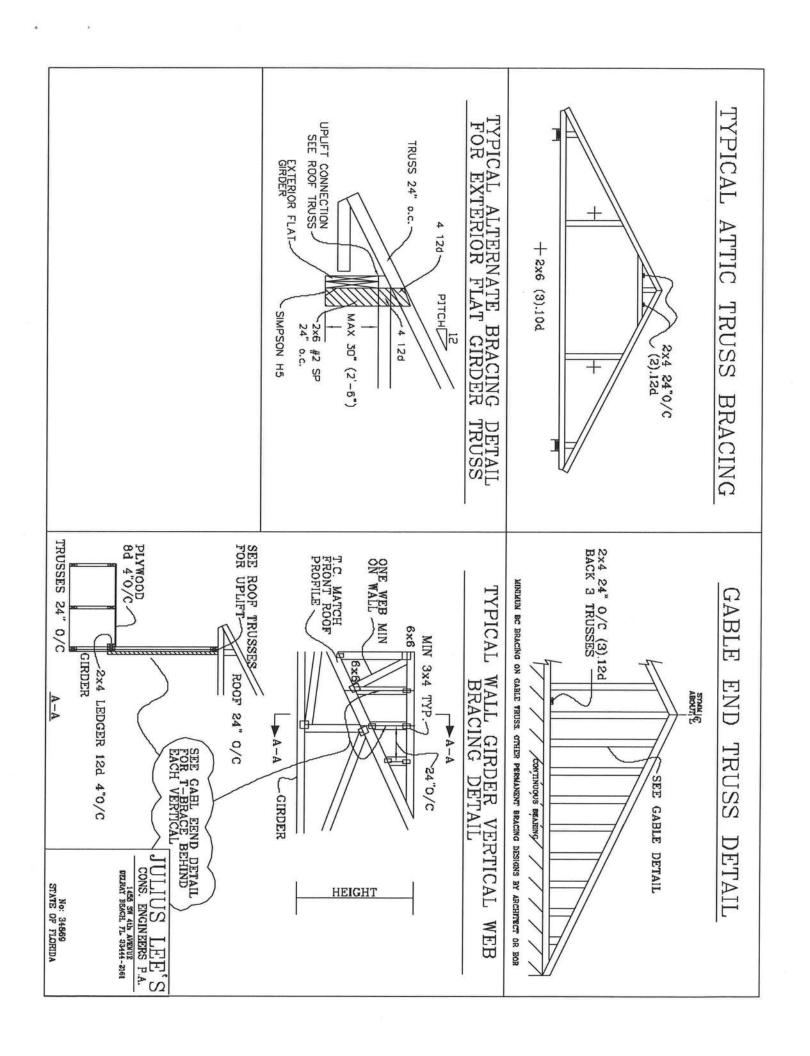
# Damage or Personal Injury Failure to Follow Could Cause Property

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 2 Cut members to bear tightly against each
- ω Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- 4 Unless otherwise noted, locate chord splices at  $\frac{1}{4}$  panel length ( $\pm$  6" from adjacent joint.)
- 5 Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 6 Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection
- 00 Plate type, size and location dimensions shown indicate minimum plating requirements
- 9 Lumber shall be of the species and size, and grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- 11. Bottom chords require lateral bracing at 10 unless otherwise noted. ft. spacing, or less, if no ceiling is installed.
- Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- Do not overload roof or floor trusses with stacks of construction materials.
- 14. Do not cut or after truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling erection and installation of trusses.
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## DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WIRN DIAGONAL BRACE IS USED. CONNECT DIACONAL BRACE FOR SAGE AT EACH YED. MAY REB TOTAL LENGTH IS 14\*. **GABLE VERTICAL** LENGTH MAX VERTICAL LENGTH SHOWN IN TABLE ABOVE. SPACING SPECIES GRADE 16" 24" O.C. O.C. O.C. CONNECT GABLE VERTICAL DFI SPF DFL SPF DFL SPF T DIAGONAL AT SP SP SP H H ASCE #3 STUD STANDARD STANDARD STANDARD STANDARD STANDARD STANDARD #2 #2 #3 #3 STUD WE'S BRACE 7-02 \*\*\*MARNING\*\*\* TRUSSES REQUIRE EXTRENE CARE IN FARRICATING, MANDLING, SAIPPING, INSTALLING AND BRADING, RETER TO JESS 1-30 ERULING COMPONENT SAFETY INFORMATION, PUBLISED BY THY ITRUSS PLANE (INSTITUTE, 383 IDDITECTED BR. SUITE 200, MANDLEN, V. I. 2019) AND VITEA (VODO TRUSS COLUCTIO FAIREA, 6300 ENTERPRISE UN, MADISON, V.) 30719) FOR SAFETY PRACTICES PRIZE TO PERFORMING THESE TAKETIONS UNICES OFFERVISE WIGHLING CONTROLLED CONTROLLED AND FORTER TAKET PROPERTY ATTACHED STRUCTURAL PANELS AND IDITION CHORD SMALL HAVE A PROPERTY ATTACHED RIGHT CELLING. GABLE TRUSS BRACES 3 3 3 8 130 SPF #1/#Z, DF-L #Z, SPF #1/#Z, OR BETTER DIAGONAL BRACE; SINGLE OR DOUBLE CUT (AS SHIDWN) AT UPPER END. GROUP A (1) 1X4 "L" BRACE \* MPH GROUP B WIND GROUP A (1) 2X4 "L" BRACE • (2) 2X4 SPEED, GROUP B REFER TO 15 GROUP A GROUP B 00000 10. MEAN CHART ABOVE FOR MAX GABLE VERTICAL LENGTH "L" BRACE \*\* (1) 2X6 "L" BRACE \* (2) ZXB "L" EX4 MEN OR BETTER CONTINUOUS BEARING 10' 5' 11' 2° 11' 2° HEIGHT, **(** 0 CONS. ENGINEERS P.A. GROUP A 10 4 10 3 10 3 10 10 10 13 13 8 DELRAY BEACH, PL 33444-2161 No: 34869 STATE OF FLORIDA 0 ENCLOSED, GROUP B GROUP A 12 4 12 4 13 5 13 5 12 6 14. 0" 12, 0, Н MAX. MAX. BRACE GROUP B II 14, D 14' D" 14' 0" 4 TOT. 1.00, SPACING 5 ATTACH EACH 'L' BRACE WITH 104 NAILS. 4 FOR (1) 'L' BRACE: SPACE NAILS AT 2° O.C. 4 FOR (2) 'L' BRACES: SPACE NAILS AT 3° O.C. 10 18° END ZONES AND 6° O.C. BETWEEN ZONES. 11 IS END ZONES AND 6° O.C. BETWEEN ZONES. CABLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLIFT CONNECTIONS FOR 136 FLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF BOX OF WEB LIVE LOAD DEPLECTION CRITERIA IS L/240. DOUGLAS FIR-LARCH #3 STUD STANDARD SPRUCE-PINE-FIR 11 / 42 STANDARD 13 STUD PLYWOOD OVERHANG. BRACING GROUP SPECIES VERTICAL LENGTH LESS THAN 1. 0. BUT LESS THAN 1. 0. BUT LESS THAN 11. 0. BUT GREATER THAN 11. 0. EXPOSURE CABLE TRUSS SOUTHERN PINE 60 24.0 REFER TO COMMON TRUES DESIGN FOR PEAK, SPLICE, AND HEEL PLATES. GABLE VERTICAL PLATE SIZES PSF DRWG DATE REF HEM-FIR FI & BIR GROUP B: GROUP DETAIL NOTES: C DOUGLAS FIR-LARCH NITEX STD CABLE 15 E HT 1 SOUTHERN PINE ASCE7-02-CAB13015 A: 12 STUD NO SPLICE AND /26/03 2.5X4 STANDARD HEM-PIR 224 GRADES:

# ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

	DIAGONAL VERTICAL DOUBLED HRACE IS HACENAL AT EACH TOTAL LET TOTAL LET IN	MAX GABLE VERTICAL LENGT	
	DIAGONAL BRACE OFTION: VERTICAL LENGTH MAY BE DOUBLED WIED, DIAGONAL BRACE IS USED, CONNEC BRACE MID, ALX WEB TOTAL LENGTH IS 14*.  VERTICAL LENGTH IS 14*.  CONNECT DIAG MEDPOINT OF V MEDPOINT OF V	12" O.C. 16" O.C. 24" O.C.	GABI
	TH THE MEST NATION:	12" O.C. 16" O.C. 24" O.C. SPF	GABLE VERTICAL
	SHOWN SHOWN	# B B B B B B B B B B B B B B B B B B B	F 1
BRACING PLATE 11 PLATE 17 ANER THESE TO STRUCTU		#1 #2 #3 #1 #3 #1 #1 #1 #1 #1 #1 #1 #1 #1 #1 #1 #1 #1	BRACE
HAVARANGIN TRUSSES REBURE EXTREME CARE IN FARRENTING, HANDLING, SUPPING, INSTALLING MO BRACING. REPER TO BESS 1-03 CRULLING COMPONENT SAFETY (HTDRANTIDA), PUBLISHED BY FPY CIRALS FARTE INSTITUTE, 583 PONOPHIO DE, SUITE 280, MANISON, ME 53757), MOD VICEA (MODD TRUSS CONCIL OF AMERICA, SOB ENTERPRISE LM, MOLSON, ME 53757), MOD VICEA (MODD TRUSS CONCIL THESE FUNCTIONS. UNICESS OTHERWISE INDICATED, TOP CHEM SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAMELS AND BETTOM CHEM SHALL HAVE A PROPERLY ATTACHED REGIO CELLING.	GABLE TRUGS		NO
3 8C31 REBUIJ 3 8C31 1-03 83 D'ONJER WITERPEISE UNLESS DIT 5 AND BOTT	4		(1) 1X4
RE EXTREME RID JR., SUIL LN, MADISC DH CHORD ;	ZX4 SP OR DF-L #Z OR BEYTEN DIAGONAL HRACE; SINGLE OR (AS SHOWN) AT UPPER END		► F,
CORPE IN I COMPONENT TE 200, NA, VI 537) DICATED, TI SHALL HAVY	7		BRACE *
- SAFETCATING - SAFETCATING INSON, VIC. 9) FOR SAFE - CHORD 12 - CA PROPER	to		(1) 2X4 "
,, HANDLING EDBHATIDH SJ719) AND SJ719) AND SJ719) AND SJ719) AND SJ719) AND LY ATTACHE	PEP REF		(1) 2X4 "L" BRACE • (2) 2X4 "L" BRACE •• (1) 2X6 "L"
T Supplied C	ABOUT LE SYAM FE ABOUT LE STAN		B • (2)
INSTALLIN D BY TPI ( DP TRUSS IC TO PERFOR ATTACHED	HART A		(2) 2X4 "L" I
HANGE C	EXA AEN OR BETTER  ABOVE FOR MAX  ABOVE OR BETTER		BRACE **
CONS. ENG DELRAY BEACH.  No: 34 STATE OF	EXA #EN OR BETTER  ** CONTINUOUS FEARING  CHART ABOVE FOR MAX CABLE		(1) 2X6 T
ENGINEERS  6 8M 4th AVEUE  BEACH PL 33444-8  BEACH PL 33444-8  BEACH PL 33444-8	CABLE V		
MINGINEERS P. J. M. 4th AVENUE ACH. PL. 33444-2161 OF FLORIDA	VERTICAL LENGTH		BRACE • (2) 2X8 "L" BRACE •
FU	LENGT		,7, 8X2 (
MAX. TO	# <b>1</b>	12 3 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	BRACE
TOT. LD.	## ## ## ## ## ## ## ## ## ## ##		3 8
80	DUTLIDARES NITH E O OVERHANG, DR 12" PLYWOOD OVERHANG.  \$ POR (1) "L" BRACE WITH 10 ANILS.  \$ POR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONE:  \$ \$ POR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONE:  "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.   GABLE VERTICAL LENGTH NO SPLICE  LESS THAN 4 "O"  LESS THAN 4 "O"  GREATER THAN 11" 6"  GREATER THAN 11" 6"  GREATER THAN 11" 6"  PEAN, SPLICE, AND HEEL PLATES.	BRACING GROUP SPECIES AND GRADES:  GROUP A:  SPRUCE-PIRE—FIR  11 1 12 STANDARD  DOUGLAS FIR-LARCH  STUD  DOUGLAS FIR-LARCH  STUD  GROUP B:  HEM-FIR  13 STANDARD  STANDARD  GROUP B:  HEM-FIR  14 BTE  15 STANDARD  GROUP B:  HEM-FIR  14 BTE  15 STANDARD  GROUP B:  HEM-FIR  14 BTE  14 BTE  14 BTE  14 BTE  14 BTE  14 BTE  SOUTHERN PINE  14 BTE  15 STANDARD  STANDARD  STANDARD  GROUP B:  HEM-FIR  15 STANDARD  STONDARD  STANDARD  STANDARD  STANDARD  STONDARD  STANDARD  STONDARD  STANDARD  STONDARD  STONDARD  STANDARD  STONDARD  STANDARD  STONDARD  STONDARD  STANDARD  STONDARD  STANDARD  STANDARD  STANDARD  STONDARD  STANDARD  STONDARD  STANDARD  STONDARD  STANDARD  STANDARD  STANDARD  STONDARD  STANDARD  STAND	
	UTLOGGERS WITH 2" OF UNEXHANG, DR 12" (LYWOOD OVERHANG, DR 12" (LYWOOD OVERHANG, DR 12" (LYWOOD OVERHANG)  IN 18" END ZONES AND 4" O.C. BETWEEN  IN 18" END ZONES AND 6" O.C. BETWEEN  BRACING MUST BE A MINIMUM OF 80% OF  BEACH LENGTH.  CABLE VERTICAL LENGTH.  LESS THAN 4" O"  LESS THAN 4" O"  LESS THAN 11" 6"  234  GREATER THAN 11" 6"  2554  GREATER THAN 11" 6"  2554  GREATER THAN 11" 6"  2654  GREATER THAN 11" 6"  2764  GREATER THAN 11" 6"  2765  GREATER THAN 11" 6"  2766  GREATER THAN 11" 6"  2767  GR	GROUP SPECIES AND GRADE  GROUP A:  SPRUCE-PINE-PIR  A1 / 42 STANDARD  B3 STUD  GROUP A:  SPRUCE-PINE-PIR  A2 STUD  A3 STUD  GROUP B:  HEM-PIR  FI & BIE  FI & BIE  STANDARD  SOUTHERN PINE  FI & BIE  GROUP B:  HEM-PIR  FI & BIE  FI & BIE  GROUP B:  HEM-PIR  FI & BIE  FI & BIE  GROUP B:  HEM-PIR  FI & BIE  GROUP B:  HEM-PINE  FI & BIE  FI &	
	O' DVER O' DVER O' DVER O' DVER O' DVER O' D' D' BUT B' D' DON TRUSS ND HEEL ON TRUSS ND HEEL ON TRUSS ND HEEL O' D' BUT B' BUT B'	GROUP A:  THE SOLUTIONS FOR CASE IN THE SOLUTION CASE IN	
726/03	HANC, OR 12.  AN MILS, AT E.  AND SPICE  IN OR STICE  IN	CIES AND GRADES  A:  HEM-FIR  A:  STUD  A:  ST	
11/26/03 2K std aale 30' e hi	DUILDOWERS WITH 2 O OVERHANC, DR 12" PLYNOOD OVERHANG.  \$ POR (1) "L" BRACES SPACE WITH 104 NAILS, \$ POR (1) "L" BRACES: SPACE WAILS AT 2" O.C. IN 18" EYD ZONES AND 4" O.C. BETWEEN ZONES, \$ \$ POR (2) "L" BRACES: SPACE WAILS AT 3" O.C.  1" BRACING MUST BE A MINIMUM OF BOX OF WEB MEMBER LENGTH.    CABLE VERTICAL LENGTH   ND SPLICE   LESS THAM 4 0"   124 DR 243   GREATER THAM 11" 6"   224   COREATER THAM 11" 6"   2.534   + REFER TO COMMON TRUSS DESIGN FOR   PEAK, SPLICE, AND HEEL PLATES.	GRADES: PIR STUD STUD STANDARD  BY PINE BY OTES: FOTES: FOTES: FOTES:	
	30, 20		



BOT CHORD 2X4 2X4 \$ 15 to 222 BETTER BETTER BETTER

# PIGGYBACK DETAIL

TYPE

SPANS

Η̈́

4

88 2

52

2.5X4

2.6X4

**5X8** 

5X6

**5**X6 3X5 REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED FURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS: 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C. WIND TC DL=5 PSF, WIND BC DL=5 PSF 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

P H

> 5**X**4 .5X3 4X8 284 30

5X5 .5X4

4X8 OR 3X6 TRULOX AT 4'
HOTATED VERTICALLY

50

C H >

1.5X4 5X6

1.5X4

FRONT FACE (B.\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. LOCATION IS ACCEPTABLE 20' FLAT TOP CHORD MAX SPAN TYP. M A SPLICE B H2 OR BETTER B C-TYP. 要 Ш D-SPLICE No.

ATTACH TRULOX EQUAL PER FAC BE CONNECTED. INFORMATION.
PLATES WITH (8) 0.120" X 1.375". E PER PLY. (4) NAILS IN EACH REFER TO DRAWING 160 TL FOR
(8) 0.120° X (4) NAILS IN RAWING 160° 7
1.375 NAILS, OR EACH MEMBER TO TL FOR TRULOX

	WEB BRACING CHART
WEB LENGTH	REQUIRED BRACING
o' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "I" BRACE. SAME GRADE, SPECIES AS MEMBER. OR BETTER, AND 80% LENGTH OF MEMBER. ATTACH WITH 8d NAILS AT 4" O
10' TO 14'	2x4 "I" BRACE. SAME GRADE SPECIES AS WEMBER. OR BETTER, AND 80% LENGTH OF WEMBER. ATTACH WITH 164 NAILS AT 4" OC.

\* PIGGYBACK SPECIAL PLATE

SPACE ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF PABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120° X 1375° NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND OC OR LESS

N

Z X

\*ATTACH

PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

SIHT
DRAWING
REPLACES
DRAWINGS
634,016
634,017
& 847,04
-

		NAVARNINGOM TRUSSES REQUIRE EXTREME CAME IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BACING REFER TO BEST 1-00 GUILLING COMPONENT SAFETY INFORMATION, PUBLICATED BY TPI CREMES PLAIT INSTITUTE, 550 (TOMORRO) PR. SUITE 200, MANISON V.). 357599 AND LYCLA COODD TRUSS COUNCIL OF AMERICA, SOID CHERRISE LIN, HANDISON, VI 33759 FER SAFETY PRACTICES PRIOR TO PERFORMING THE SEF FUNCTIONS. UNLESS OTHERNISE DIRICATED, ITP CHERD SAML HAVE PREPERLY ATTACHED STRICTURAL PANCES AND BOTTOM CHERD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING.					
STATE OF FLORIDA	:	JULIUS LEE, S cons. engineers p.a. ddray beach tl. 3344-2161					
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	MAX LOADING 55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC.					
		MAX LOADING REF PIGGYBACK  55 PSF AT DATE 09/12/07  1.33 DUR. FAC. DRWG MITEK STD PIGGY  1.25 DUR. FAC. —ENG JL					

# VALLEYTRUSS DETAIL

TOP CHORD CHORD WEBS 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. 2X4 SP #3 OR BETTER.

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- \* ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCI BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. OR (3) 16d I FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"—BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON SR

HENEATH THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

4-0-0 MAX

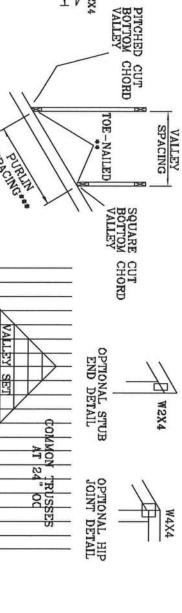
12 NAX. WZX4

W2X4

W2X4

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



W1X3

W1X3

W1X3

(MAX SPACING

6-0-0

W1X3

WZX4

12 MAX.

12

W4X4

8-0-0 MAX

12 MAX.

12

W1X3 W1X3

SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING W1X3 W5X4/SPL 20-0-0 MAX (++)-16-0-0 MAX-W4X4 (MAX SPACING) 6-0-0 FXIM WIXE W1X3 SPACING

COMMON

TRUSSES

00

PARTIAL FRAMING PLAN

24

000

MANARHIMGAM TRUSTEES REQUIRE CYTROME (MARBICATING, MANDILING, SMIPPING, INST.
BACING. REFER TO BEST 1-03 GBULDING COMPORTION FACETY INFORMATION, PUBLISHED BY
PLATE INSTITUTE. SEO CHOOTRED BX. SUITE 280, MANISON V.). SE3799 AND WICK AVOIDS TRA
THESE FINCTIONS. UNLESS GITHEMYSES UNDICATED, TO ENOR OWALL MAVE REPORTED. A THAN
STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED RIGHD CEILING

			is.	PERFORMING	TALLING AND		
STATE OF FLORIDA	No: 34869		2	DELRAY BEACH, IL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S	
SP,	DUR	TO	BC	BC	TC	TC	
SPACING	UR.FAC. 1.25	TOT. LD.	F	DL	DL	H	
	Ċñ	32 40	0	U	~2	20	HI
24"	1.25	40	0	5	15	20	IS DR
		PSF	PSF	PSF	PSF	PSF	THIS DRAWING
			PSF -ENG JL	DRWG	DATE	PSF REF	REPLACES
			JL	VAL	11/	VAL	ES D
				VALTRUSS1103	11/26/03	VALLEY DETAIL	DRAWING
				SS11	33	DET	NG A
				03		AIL	A105

# TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

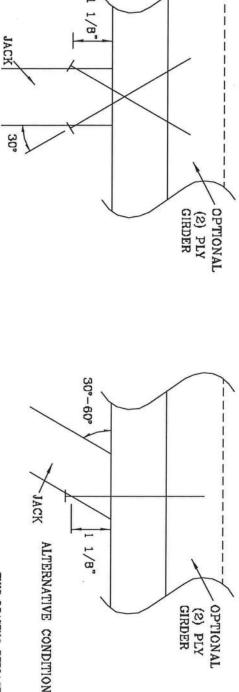
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

# MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

ALL WILLIAM THE WILLIAM THE PROPERTY OF THE PR	5	4.	3	N	TOE-NAILS	NUMBER OF
מו עונו מו	493#	394#	296#	187#	1 PLY	SOUTHERN PINE
TA BELLEVILLE	639#	511#	383#	256#	2 PLIES 1 PLY	RN PINE
77	452#	361#	271#	181#	1 PLY	DOUGLAS
70771	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
110	390#	312#	234#	156#	1 PLY	HEM-FIR
1	507#	406#	304#	203#	2 PLIES	-FIR
2000	384#	307#	230#	154#	1 PLY	SPRUCE
	496#	397#	298#	189#	2 PLIES	SPRUCE PINE FIR

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



SIHT
DRAWING
REPLACES
DRAWING
784040

		e	STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED RIGID DELLING	, 383 PUNDFRID DR., SUITE 200, MADISDN, WI. 53719) AND VICA (W ENTERPRISE LN, MADISDN, VI 53719) FOR SAFETY PRACTICES PRID LINE FSC THE SOURCE INSTINCT TO THE CHIEF SWILL WAVE DESCRIPTION	HHVARNING H TRUSSES REDURE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 85S1 1-03 COULDING COMPONENT SAFETY (MFDOWATION), SHIPPING, INSTALLING AND	
STATE OF FLORIDA	No: 34869			DELRAY BEACH, FL 83444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	1.00	PSF	PSF	PSF	PSF	PSF
			-ENG JL	DRWG	DATE	REF
			JL	CNTONAIL1103	09/12/07	TOE-NAIL

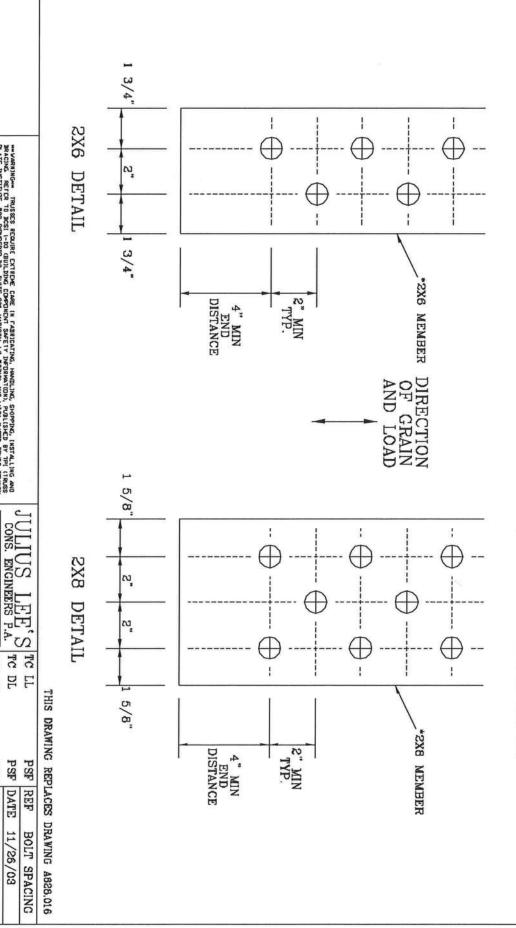
# DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

\* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" OF 1/16" LARGER THAN BOLT DIAMETER. TO A MAXIMUM

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PAITERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



DELRAY BEACH, FL 33444-2161

BC BC DL

PSF PSF PSF PSF

DATE DRWG

CNBOLTSP1103 11/26/03

No: 34869 STATE OF FLORIDA

SPACING DUR. FAC. TOT. LD.

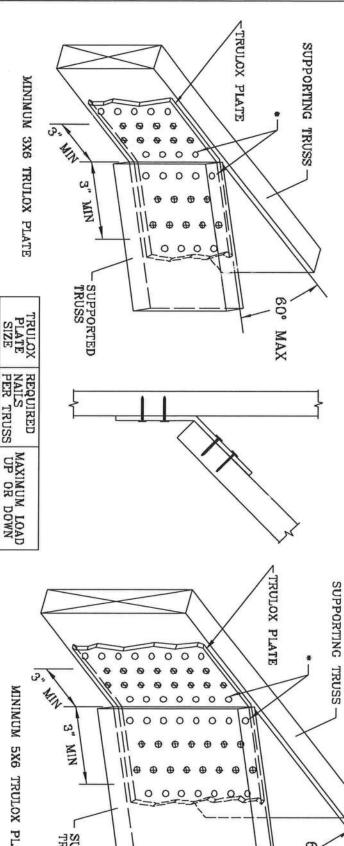
# TRULOX CONNECTION

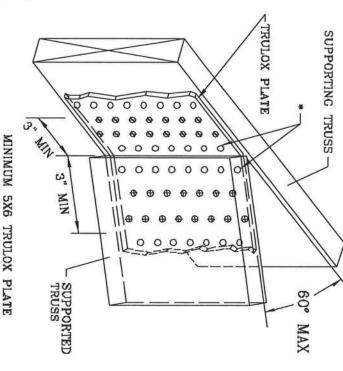
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (+).

NAILS MAY BE OMITTED FROM THESE ROWS THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST DURATION OF LOAD. CHORD SIZE EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.





THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CAME IN FABRICATING, HANGLING, SHIPPING, BRACING REFER TO BEST 1-00 (BUILING COMPONENT SAFETY INFORMATION, PUBLISSED PLATE INSTITUT, 590 INFORTED OR, SUITE 200, AUGUSTON, VI. 337199 AND VICE A VOID OF AMERICA, SOO CHIERRAISE LM, MAISCH, VI. 337199 FOR SAFETY RACFITCES, SHOPPING, VI. 537199 FOR SAFETY RACFITCES, VI. 53719 FOR SAFETY RACFI

MINIMUM 3X6 TRULOX PLATE

3X6

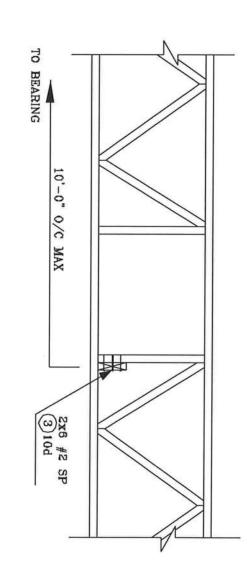
15 Q

#066 350# PER TRUSS

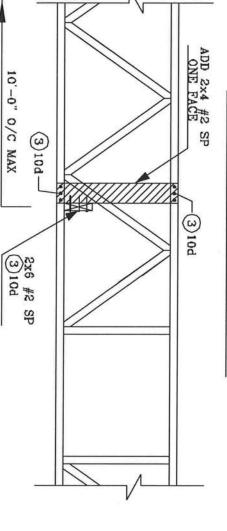
MAXIMUM LOAD UP OR DOWN

	HSTALLING AND D BY TPJ (TRUSS ODI FRUSS COUNCIL TO PERFORMING ATTACHED			
No: 34869 STATE OF FLORIDA		DELRAY BEACH, IL 33444-2351	CONS. ENGINEERS P.A.	
	-ENG JL	DRWG	DATE	REF

# STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



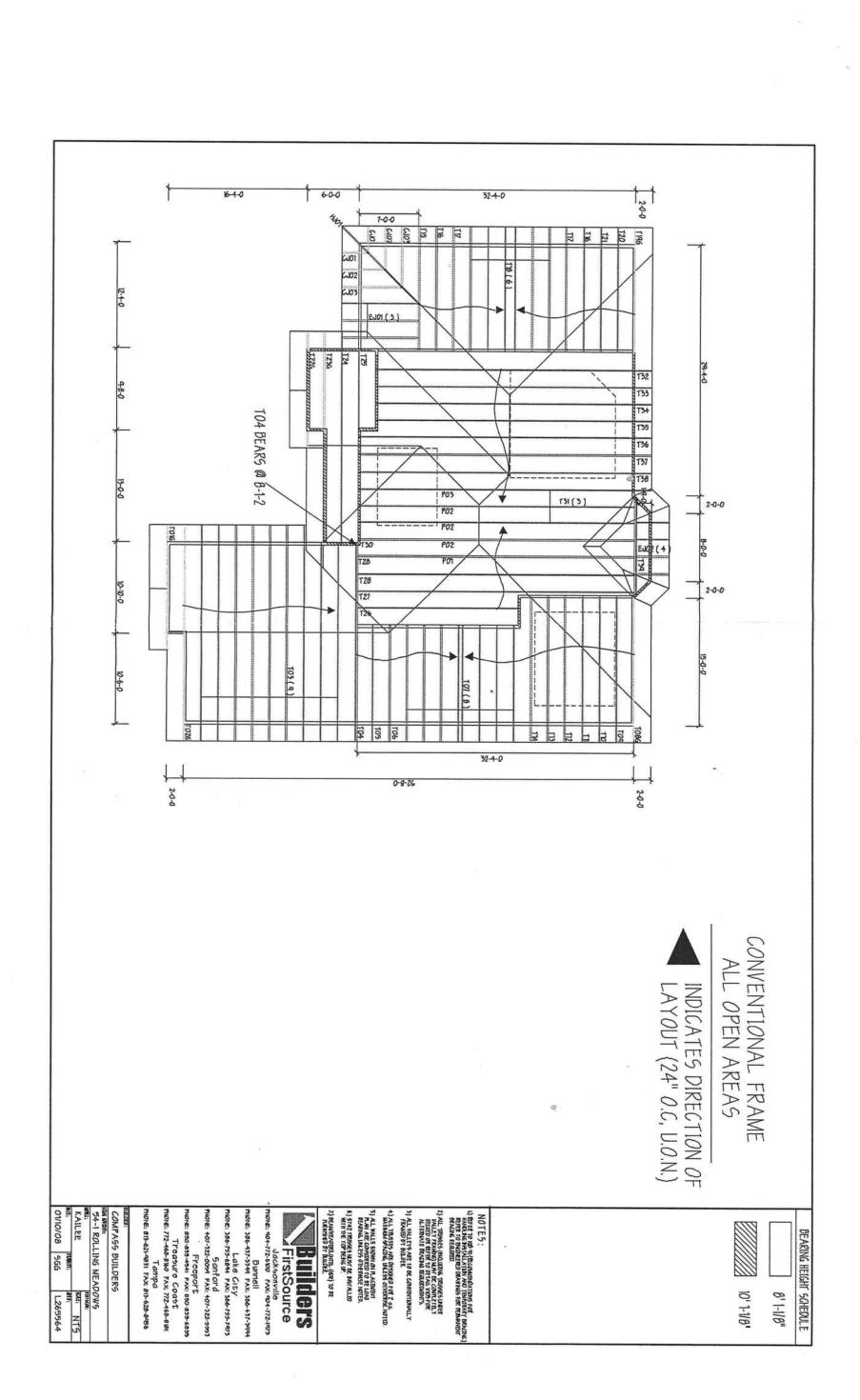
# ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP

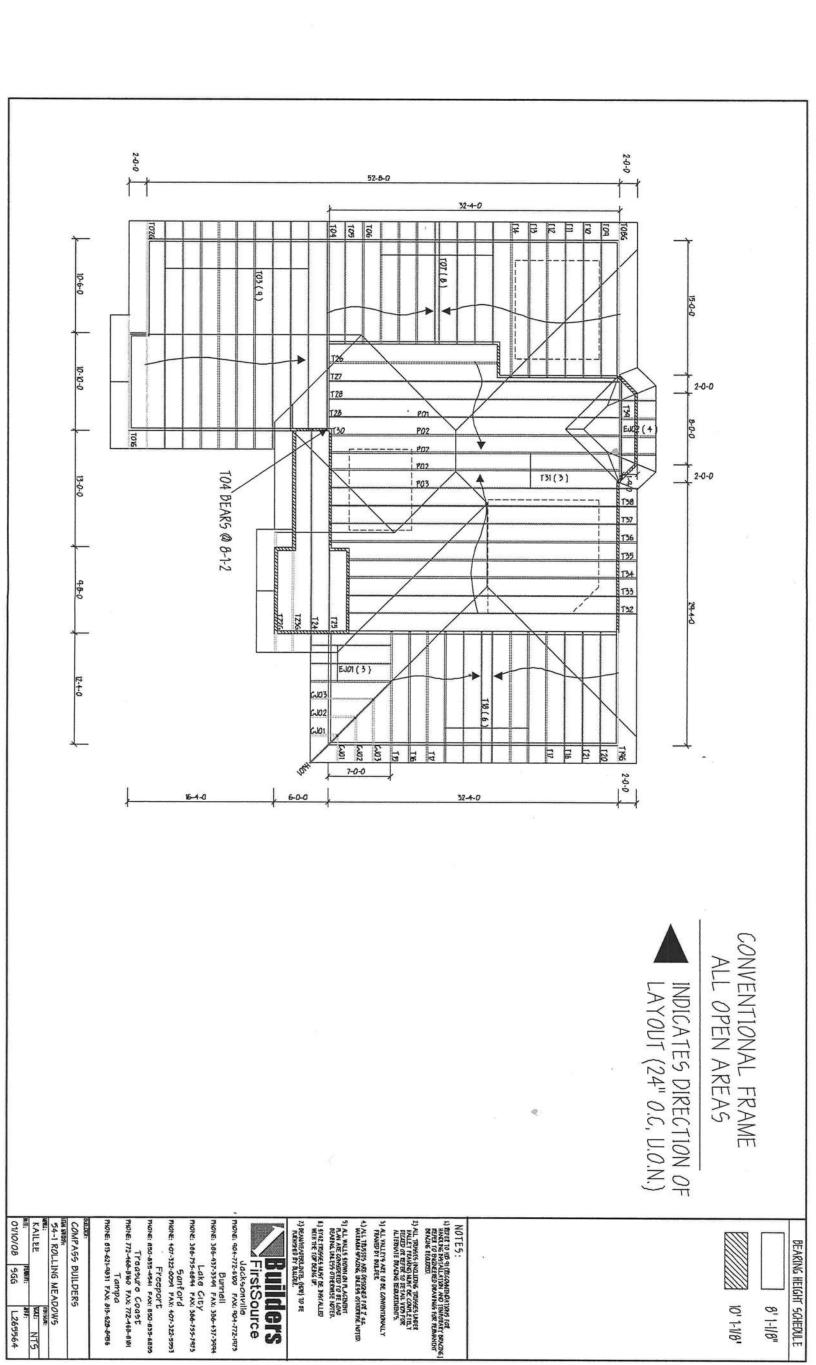


JULIUS LEE'S CONS. ENGINEERS P.A.

TO BEARING

No: 34869 STATE OF FLORIDA







8' 1-1/8"

10' 1-1/8"