

DATE 01/16/2008

Columbia County Building Permit

PERMIT

This Permit Must Be Prominently Posted on Premises During Construction

000026623

APPLICANT PATRICK WILSON PHONE 352.206.5459
ADDRESS 6800 SOUTHPOINT PKWY. # 300 JACKSONVILLE FL 32216
OWNER MARONDA HOMES INC. OF FLORIDA PHONE 904.296.1490
ADDRESS 209 SW TIMBER RIDGE DRIVE LAKE CITY FL 32024
CONTRACTOR THEODORE C. BROCK PHONE 407.22.3504
LOCATION OF PROPERTY 90-W TO SR. 247-S TL TO C-252-B,TR TO TIMBER RIDGE,
TL AND IT'S THE 8TH LOT ON R.
TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 153050.00
HEATED FLOOR AREA 2236.00 TOTAL AREA 3061.00 HEIGHT 20.00 STORIES 1
FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC
LAND USE & ZONING RSF-2 MAX. HEIGHT 35
Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO. _____

PARCEL ID 10-4S-16-02856-105 SUBDIVISION TIMBERLAND
LOT 5 BLOCK _____ PHASE _____ UNIT _____ TOTAL ACRES 0.50
000001525 _____ CRC1326913 _____
Culvert Permit No. _____ Culvert Waiver _____ Contractor's License Number _____ Applicant/Owner/Contractor _____
18"X32'MITERED 07-0998 BLK JTH N
Driveway Connection _____ Septic Tank Number _____ LU & Zoning checked by _____ Approved for Issuance _____ New Resident _____

COMMENTS: MFE: 99'- ELEVATION CONFIRMTION LETTER REQUIRED, NOC ON FILE

Check # or Cash 861761

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ Foundation _____ Monolithic _____
date/app. by _____ date/app. by _____ date/app. by _____
Under slab rough-in plumbing _____ Slab _____ Sheathing/Nailing _____
date/app. by _____ date/app. by _____ date/app. by _____
Framing _____ Rough-in plumbing above slab and below wood floor _____
date/app. by _____ date/app. by _____
Electrical rough-in _____ Heat & Air Duct _____ Peri. beam (Lintel) _____
date/app. by _____ date/app. by _____ date/app. by _____
Permanent power _____ C.O. Final _____ Culvert _____
date/app. by _____ date/app. by _____ date/app. by _____
M/H tie downs, blocking, electricity and plumbing _____ Pool _____
date/app. by _____ date/app. by _____
Reconnection _____ Pump pole _____ Utility Pole _____
date/app. by _____ date/app. by _____ date/app. by _____
M/H Pole _____ Travel Trailer _____ Re-roof _____
date/app. by _____ date/app. by _____ date/app. by _____

BUILDING PERMIT FEE \$ 770.00 CERTIFICATION FEE \$ 15.30 SURCHARGE FEE \$ 15.30
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$ _____
FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 900.60
INSPECTORS OFFICE _____ CLERKS OFFICE _____

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

For Office Use Only Application # 0801-08 Date Received 1/3/08 By JW Permit # 26623-1525
Zoning Official BK Date 08.01.08 Flood Zone Xpuplt FEMA Map # N/A Zoning RSF-2
Land Use R2D Elevation N/A MFE 97' pgt River N/A Plans Examiner OKJTH Date 1-7-08
Comments Elevation Confirmation Letter Required

☐ NOC ☐ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel # _____
☐ Dev Permit # _____ ☐ In Floodway ☐ Letter of Authorization from Contractor
☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 07-998 PATRICK WILSON 352.206.5459 Fax (904)-332-6367

Name Authorized Person Signing Permit Theodore C. Brock/Melissa McKague Phone (904)-296-1490

Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Owners Name Maronda Homes Inc. of Florida Phone (904)-296-1490

911 Address 209 SW Timber Ridge Dr., LAKE CITY, FL 32024

Contractors Name Theodore C. Brock Phone (407)-227-3504

Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way Sanford, FL 32771

Mortgage Lenders Name & Address Bank of America 250 Park Ave. S. #400 Winter Park, FL 32789

Circle the correct power company - FL Power & Light - Clay Elec - Suwannee Valley Elec. - Progress Energy

Property ID Number 10-45-10-02850-105 Estimated Cost of Construction 82,669

Subdivision Name Timberlands Lot 5 Block Unit Phase 1

Driving Directions Hwy 90, Left on 247 South; Right on 252B; Left on Timber Ridge 8th lot
ON RIGHT

Number of Existing Dwellings on Property

Construction of Residential Single Family Dwelling Total Acreage .50 Lot Size

Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 20'

Actual Distance of Structure from Property Lines - Front 50 28.1 Side 28.2 Side 28.2 Rear 75.6

Number of Stories 1 Heated Floor Area 2230 Total Floor Area 3061 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

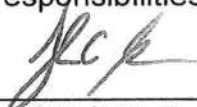
According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature


Theodore C. Brock

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)


Theodore C. Brock

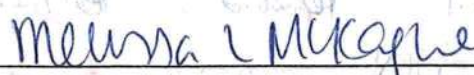
Contractor's License Number CRC 1326913

Columbia County

Competency Card Number _____

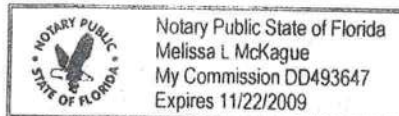
Affirmed under penalty of perjury to by the Contractor and subscribed before me this 10 day of December 2007.

Personally known XXX or Produced Identification _____


State of Florida Notary Signature (For the Contractor)

Melissa L. McKague

SEAL:



Maronda Homes
AUSTIN
 AMERICANA SERIES
 (4 Bedroom)



1st Floor Living:	2236 sq. ft.
Covered Porch:	288 sq. ft.
Garage Area	372 sq. ft.
Covered Entry:	165 sq. ft.
Total Area	3061 sq. ft.

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www.maronda.com

Providing More Home for Less Money . . . It's the Way We Do Business!



Maronda Homes
AUSTIN
AMERICANA SERIES
(4 Bedroom)



Elevation M

Prices, plans, dimensions, features, specifications, materials and availability of Maronda Homes or communities are subject to change without prior notice or obligation. Maronda Homes illustrations are artist's depictions only and may differ from completed improvements. Copyright 2002 © Maronda Homes Inc. of Florida. All Rights Reserved.

www.maronda.com



Providing More Home for Less Money . . . It's the Way We Do Business!

12/06

NOTICE OF COMMENCEMENT

Tax Parcel Identification Number 26623

County Clerk's Office Stamp or Seal

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): 511 Timber lands
a) Street (job) Address: 209 SW Timber Ridge Dr.
2. General description of improvements: Construction of a single family dwelling
3. Owner Information
a) Name and address: Maronda Homes Inc. of FL 6800 Southpoint Pkwy #300 Jax. FL 32216
b) Name and address of fee simple titleholder (if other than owner) _____
c) Interest in property _____
4. Contractor Information
a) Name and address: Theodore C. Brock 6800 Southpoint Pkwy #300 Jax. FL 32216
b) Telephone No.: (407)-227-3504 Fax No. (Opt.) _____
5. Surety Information
a) Name and address: _____
b) Amount of Bond: _____
c) Telephone No.: _____ Fax No. (Opt.) _____
6. Lender
a) Name and address: Bank of America 250 Park Ave S. #400 Winter Park, FL 32789
b) Phone No. _____
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
a) Name and address: Kampf Title and Guaranty Corp. P.O. Box 1359 200 W. 1st St.
b) Telephone No.: _____ Fax No. (Opt.) Winter Park, FL 32771
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(l)(b), Florida Statutes:
a) Name and address: Staff at Maronda Homes Inc. of FL
b) Telephone No.: (904)-296-1490 Fax No. (Opt.) _____
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): _____

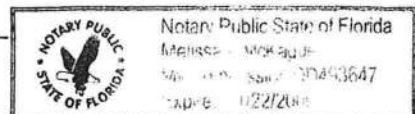
WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [Signature]
Signature of Owner or Owner's Authorized Officer/Partner/Manager
Theodore C. Brock
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 3 day of January, 2008, by:
Theodore C. Brock as V.P. of Construction (type of authority, e.g. officer, trustee, attorney
fact) for Maronda Homes Inc. of FL (name of party on behalf of whom instrument was executed).
Personally Known XX OR Produced Identification _____ Type _____

Notary Signature Melina L. Myer Notary Stamp or Seal:



11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

[Signature]
Signature of Natural Person Signing (in line #10 above.)

**Columbia County Building Department
Culvert Permit**

26623
**Culvert Permit No.
000001525**

DATE 01/16/2008 PARCEL ID # 10-4S-16-02856-105
APPLICANT PATRICK WILSON PHONE 352.206.5459
ADDRESS 6800 SOUTHPPOINT PKWY. # 300 JACKSONVILLE FL 32216
OWNER MARONDA HOME INC. OF FLORIDA PHONE 904.296.1490
ADDRESS 209 SW TIMBER RIDGE DRIVE LAKE CITY FL 32024
CONTRACTOR THEODORE C. BROCK PHONE 407.227.3504
LOCATION OF PROPERTY 90-W TO SR 247-S, TL TO C-252-B, TR TO TIMBER RIDGE, TL AND
IT'S THE 8TH LOT ON R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT TIMBERLANDS 5

SIGNATURE

INSTALLATION REQUIREMENTS

☒ X

Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
- b) the driveway to be served will be paved or formed with concrete.

Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

☐

Culvert installation shall conform to the approved site plan standards.

☐

Department of Transportation Permit installation approved standards.

☐

Other _____

**ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALATION OF THE CULVERT.**

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00





STATE OF FLORIDA
DEPARTMENT OF HEALTH

See attached.

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-0998

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.

Notes: _____

Site Plan submitted by: Jessie Abreu _____

Signature

Agent

Title

Plan Approved ☒ Not Approved ☐ 12-28-07 Date 2007

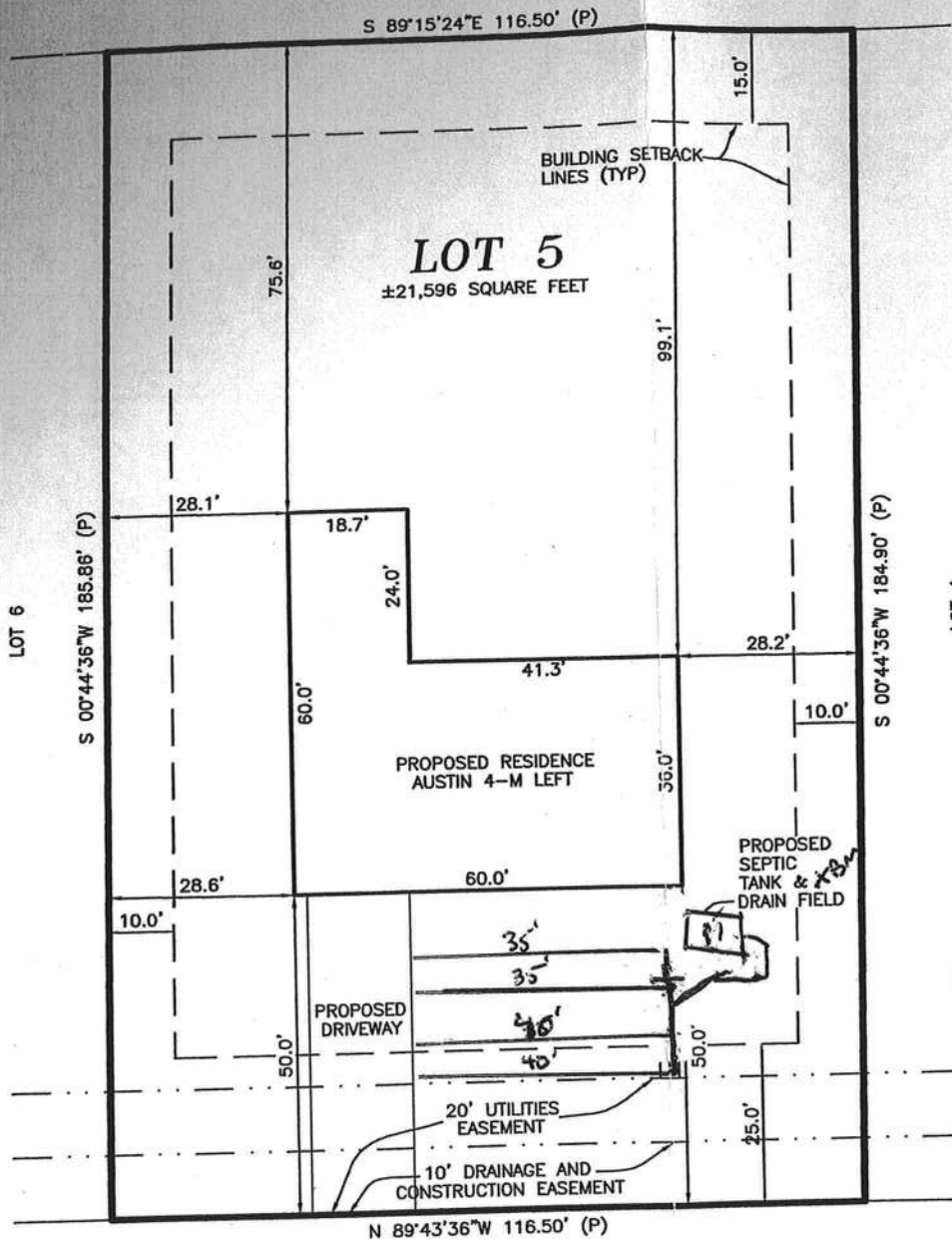
By M J h Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA

30
1" = 30'



LEGEND:

- = FOUND 1/2" REBAR IDENTIFICATION
- = FOUND 1/2" REBAR L.B. 6894
- = SET 1/2" REBAR & L.B. 6894
- = FOUND 3/4" IRON PI
- = FOUND 4" X 4" CONC. NO IDENTIFICATION
- = SET 4" X 4" CONC. P.S.M. 5582
- ✕ = SET NAIL & DISK P.S.
- ✕ = FOUND NAIL & DISK
- ⊠ = FOUND 6" X 6" S.R. R/W MON.
- ⊠ = CATV RISER
- ⊠ = TELEPHONE PEDESTAL
- ⊠ = WOOD POWER POLE

THIS
CERTIFICATE
NOT VALID
A FLOOD
TO THE

I HER
TRUE
MY SI
THE I
BOARD
STATU

BY
JAMES

DATE:



SCALE: 1" =

DATE: 10/1

THIS IS NOT

PREPARED I

SW TIMBER RIDGE DRIVE
60' RIGHT-OF-WAY

LOAD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE
NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE
MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88,
THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA
DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS
CALLED FROM SAID MAP. INFORMATION FROM THE FEDERAL
EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE
RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE
REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE
PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED
ON THE MOST CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE
DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS
SURVEYOR HAS NOT PERFORMED A SEARCH OF THE
PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF
TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY AFFECT
THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH
CLAIMS ARE NOT CERTIFIED HEREON.

Handwritten calculations:
60 x 20 = 1200
55 x 20 = 1100
1200 - 1100 = 100

This instrument prepared by:
William J. Haley, Esquire
Brannon, Brown,
Haley & Bullock, P. A.
P. O. Box 1029
Lake City, FL 32056-1029

Inst:200712024085 Date:10/26/2007 Time:4:45 PM
Doc Stamp-Deed:3276.00
27 DC,P.DeWitt Cason,Columbia County Page 1 of 2

SPECIAL WARRANTY DEED

THIS INDENTURE, made this 10th day of October, 2007, between **RML HOLDINGS, INC.**, a Florida corporation, having a mailing address of 703 NW Blackberry Circle, Lake City, Florida 32055, hereinafter referred to as Grantor, and **MARONDA HOMES, INC. OF FLORIDA**, a Florida corporation, having a mailing address of 3993 West First Street, Sanford, Florida 32771, hereinafter referred to as Grantee.

WITNESSETH: That said Grantor, for and in consideration of the sum of \$10.00 and other good and valuable considerations to said Grantor in hand paid by said Grantee, the receipt and sufficiency of which are hereby acknowledged, have granted, bargained and sold to the said Grantee, and Grantee's successors and assigns forever, the following described land, situate, lying and being in **Columbia** County, Florida, to-wit:

Lot(s) 2, 3, 4, 5, 6, 7, 8, 21, 22, 23, 25, and 26, **TIMBERLANDS, PHASE 1**, according to the plat thereof, as recorded in Plat Book 9, pages 26-27, public records of Columbia County, Florida.

PARCEL NO. Part of 10-4S-16-02856-000 and 15-4S-16-02990-000

SUBJECT TO: Taxes and special assessments for the year 2007 and subsequent years; restrictions, reservations, rights of way for public roads, easements of record, if any; and zoning and any other governmental restrictions regulating the use of the lands.

and said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons claiming by, through or under said Grantor.

IN WITNESS WHEREOF, Grantor has hereunto set its hand and seal the day and year first above written.

Signed, sealed and delivered
in the presence of:

RML HOLDINGS, INC., a Florida
corporation

Linda Brown
Print Name: Linda Brown

By: Margaret Lardizabal
Margaret Lardizabal
Vice President

Debbie G. Moore
Print Name: Debbie G. Moore

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 10th day of October, 2007, by Margaret Lardizabal, as Vice President of RML Holdings, Inc., a Florida corporation, on behalf of said corporation, who is personally known to me.

Debbie G. Moore
Notary Public, State of Florida





Greater Lake City Regional Utility Authority

205 North Marion Avenue
LAKE CITY, FLORIDA 32055-3918
TELEPHONE: 386.719.5778 FAX: 386.719.5837
E-mail: customer.service@ci.lake-city.fl.us

APPLICATION FOR WATER/SEWER/GAS TAP-CAPACITY COMMITMENT

Project Name: Timberlands Date/Time: 12/10/15
Service Address: 209 SW Timber Ridge Dr
Applicant Name: Maronda Homes Telephone Number: (904)-296-1490
Applicant's Agent: Melissa McKague Telephone Number: (904)-296-1490 ext. 117
Business Name: Maronda Homes Inc. of Telephone Number: (904)-296-1490
Parcel ID #'s: _____
Mailing Address: 6800 Southpoint Pkwy #300 Jacksonville, FL 32216

Requested Water Capacity: ☐ No ☒ Yes, in the amount of _____ gpd/gph
Requested Sewer Capacity: ☐ No ☐ Yes, in the amount of _____ gpd/gph
Requested Gas Capacity: ☐ No ☐ Yes, in the amount of _____ btu/unit

Application For: ☒ Water/Tap Size 3/4 ☐ Sewer/Tap Size _____
☐ Irrigation/Tap Size _____ ☐ Gas/Tap Size _____

Meter Size /Quantity: ☒ 3/4" ☐ 1" ☐ 1 1/2" ☐ 2" ☐ 6" ☐ Other Specify _____

Within City limits: ☐ Yes ☒ No

Fire Protection: ☒ No ☐ Yes, Diameter of new mainline _____
Quantity of new Hydrants: _____

Growth Management Zoned: _____ ☒ Residential ☐ Commercial ☐ Industrial

The above named applicant request that an inspection be made by the Greater Lake City Regional Utility Authority for verification of available services based on address, parcel numbers, lot numbers, etc. Upon confirmation of both capacity and or availability, the applicant will be notified and provided a "Cost Estimate/Tap & Impact Fees" summary.

NOTE: This is only an estimate, fees are subject to change.

Applicant: Melissa McKague

Date: 12/10/15

Service Available Date Requested

Date: ASAP

Billing will begin upon completion of tap.

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

5/1 TM

Project Name: AUSTIN 4 BDR GAINESVILLE	Builder: MARONDA HOMES
Address: 209 SW Timber Ridge Dr	Permitting Office: Columbia
City, State: Lake City, FL 32055	Permit Number: _____
Owner: ELECTRIC	Jurisdiction Number: 221000
Climate Zone: North	

1. New construction or existing ☐ New ☐ _____	12. Cooling systems
2. Single family or multi-family ☐ Single family ☐ _____	a. Central Unit ☐ Cap: 42.5 kBtu/hr _____
3. Number of units, if multi-family ☐ 1 ☐ _____	SEER: 14.00 _____
4. Number of Bedrooms ☐ 4 ☐ _____	b. N/A ☐ _____
5. Is this a worst case? ☐ Yes ☐ _____	c. N/A ☐ _____
6. Conditioned floor area (ft²) ☐ 2236 ft² ☐ _____	13. Heating systems
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)	a. Electric Heat Pump ☐ Cap: 42.5 kBtu/hr _____
a. U-factor: _____ Description Area _____	HSPF: 8.70 _____
(or Single or Double DEFAULT) 7a.(Single Default) 177.0 ft² ☐ _____	b. N/A ☐ _____
b. SHGC: _____	c. N/A ☐ _____
(or Clear or Tint DEFAULT) 7b. (Clear) 177.0 ft² ☐ _____	14. Hot water systems
8. Floor types	a. Electric Resistance ☐ Cap: 50.0 gallons _____
a. Slab-On-Grade Edge Insulation ☐ R=0.0, 219.0(p) ft ☐ _____	EF: 0.90 _____
b. N/A ☐ _____	b. N/A ☐ _____
c. N/A ☐ _____	c. Conservation credits
9. Wall types	(HR-Heat recovery, Solar
a. Concrete, Int Insul, Exterior ☐ R=4.1, 1199.0 ft² ☐ _____	DHP-Dedicated heat pump)
b. Frame, Steel, Adjacent ☐ R=11.0, 320.0 ft² ☐ _____	15. HVAC credits ☐ PT, _____
c. N/A ☐ _____	(CF-Ceiling fan, CV-Cross ventilation,
d. N/A ☐ _____	HF-Whole house fan,
e. N/A ☐ _____	PT-Programmable Thermostat,
10. Ceiling types	MZ-C-Multizone cooling,
a. Under Attic ☐ R=19.0, 2450.0 ft² ☐ _____	MZ-H-Multizone heating)
b. N/A ☐ _____	
c. N/A ☐ _____	
11. Ducts	
a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 200.0 ft ☐ _____	
b. N/A ☐ _____	

Glass/Floor Area: 0.08

Total as-built points: 27794

Total base points: 30537

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Gary Carmack
DATE: 12/13/07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Melissa McKague
DATE: 12/13/07

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____
DATE: _____



¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT								
GLASS TYPES												
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points					
.18	2236.0	18.59	7482.0	1.Single, Clear	W	1.0	6.0	16.0	43.84	0.97	680.0	
				2.Single, Clear	N	1.0	6.0	16.0	21.73	0.97	338.0	
				3.Single, Clear	W	1.0	3.0	5.0	43.84	0.85	186.0	
				4.Single, Clear	W	1.0	7.0	30.0	43.84	0.98	1294.0	
				5.Single, Clear	W	1.0	8.0	40.0	43.84	0.99	1734.0	
				6.Single, Clear	E	1.0	6.0	20.0	47.92	0.97	927.0	
				7.Single, Clear	E	1.0	6.0	20.0	47.92	0.97	927.0	
				8.Single, Clear	E	1.0	6.0	30.0	47.92	0.97	1394.0	
				As-Built Total:				177.0	7480.0			
WALL TYPES				Area X BSPM = Points		Type	R-Value	Area X SPM = Points				
Adjacent	320.0	0.70	224.0	1. Concrete, Int Insul, Exterior		4.1	1199.0	1.13		1360.9		
Exterior	1199.0	1.70	2038.3	2. Frame, Steel, Adjacent		11.0	320.0	1.00		320.0		
Base Total:		1519.0	2262.3	As-Built Total:		1519.0		1680.9				
DOOR TYPES				Area X BSPM = Points		Type		Area X SPM = Points				
Adjacent	18.0	2.40	43.2	1.Adjacent Wood			18.0	2.40		43.2		
Exterior	20.0	6.10	122.0	2.Exterior Insulated			20.0	4.10		82.0		
Base Total:		38.0	165.2	As-Built Total:		38.0		125.2				
CEILING TYPES				Area X BSPM = Points		Type	R-Value	Area X SPM X SCM = Points				
Under Attic	2236.0	1.73	3868.3	1. Under Attic		19.0	2450.0	2.34 X 1.00		5733.0		
Base Total:		2236.0	3868.3	As-Built Total:		2450.0		5733.0				
FLOOR TYPES				Area X BSPM = Points		Type	R-Value	Area X SPM = Points				
Slab	219.0(p)	-37.0	-8103.0	1. Slab-On-Grade Edge Insulation		0.0	219.0(p)	-41.20		-9022.8		
Raised	0.0	0.00	0.0									
Base Total:		-8103.0		As-Built Total:		219.0		-9022.8				
INFILTRATION				Area X BSPM = Points				Area X SPM = Points				
		2236.0	10.21	22829.6				2236.0		10.21		22829.6

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT									
Summer Base Points: 28504.3				Summer As-Built Points: 28825.8									
Total Summer Points	X	System Multiplier	= Cooling Points	Total Component (System - Points)	X	Cap Ratio (DM x DSM x AHU)	X	Duct Multiplier	X	System Multiplier	X	Credit Multiplier	= Cooling Points
28504.3		0.3250	9263.9	(sys 1: Central Unit 42500btuh ,SEER/EFF(14.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 28826 1.00 (1.08 x 1.147 x 0.86) 0.244 0.950 7156.0 28825.8 1.00 1.072 0.244 0.950 7156.0									

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	2236.0	20.17	8118.0	1.Single, Clear	W	1.0	6.0	16.0	28.84	1.01	465.0
				2.Single, Clear	N	1.0	6.0	16.0	33.22	1.00	531.0
				3.Single, Clear	W	1.0	3.0	5.0	28.84	1.04	150.0
				4.Single, Clear	W	1.0	7.0	30.0	28.84	1.00	869.0
				5.Single, Clear	W	1.0	8.0	40.0	28.84	1.00	1157.0
				6.Single, Clear	E	1.0	6.0	20.0	26.41	1.02	536.0
				7.Single, Clear	E	1.0	6.0	20.0	26.41	1.02	536.0
				8.Single, Clear	E	1.0	6.0	30.0	26.41	1.02	804.0
				As-Built Total:				177.0		5048.0	
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	320.0	3.60	1152.0	1. Concrete, Int Insul, Exterior	4.1		1199.0	6.42		7697.6	
Exterior	1199.0	3.70	4436.3	2. Frame, Steel, Adjacent	11.0		320.0	5.20		1664.0	
Base Total:				As-Built Total:				1519.0		9361.6	
DOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	18.0	11.50	207.0	1.Adjacent Wood			18.0	11.50		207.0	
Exterior	20.0	12.30	246.0	2.Exterior Insulated			20.0	8.40		168.0	
Base Total:				As-Built Total:				38.0		375.0	
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	2236.0	2.05	4583.8	1. Under Attic	19.0		2450.0	2.70 X 1.00		6615.0	
Base Total:				As-Built Total:				2450.0		6615.0	
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	219.0(p)	8.9	1949.1	1. Slab-On-Grade Edge Insulation	0.0		219.0(p)	18.80		4117.2	
Raised	0.0	0.00	0.0								
Base Total:				As-Built Total:				219.0		4117.2	
INFILTRATION Area X BWPM = Points								Area X WPM = Points			
2236.0 -0.59 -1319.2								2236.0 -0.59 -1319.2			

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE			AS-BUILT					
Winter Base Points: 19373.0			Winter As-Built Points: 24197.5					
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
19373.0	0.5540	10732.6	(sys 1: Electric Heat Pump 42500 btuh ,EFF(8.7) Ducts:Unc(S),Con(R),Int(AH),R6.0 24197.5	1.000	(1.060 x 1.169 x 0.88)	0.392	0.950	9864.1
19373.0	0.5540	10732.6	24197.5	1.00	1.095	0.392	0.950	9864.1

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE					AS-BUILT					
WATER HEATING										
Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X Credit = Total Multiplier
4		2635.00		10540.0	50.0	0.90	4		1.00	2693.56
					As-Built Total:					10774.2

CODE COMPLIANCE STATUS											
BASE						AS-BUILT					
Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points
9264		10733		10540	30537	7156		9864		10774	27794

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.6

The higher the score, the more efficient the home.

ELECTRIC, , , ,

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 42.5 kBtu/hr
3. Number of units, if multi-family	1		SEER: 14.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft ²)	2236 ft ²		
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 42.5 kBtu/hr
(or Single or Double DEFAULT) 7a(Sngle Default)	177.0 ft ²		HSPF: 8.70
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT) 7b. (Clear)	177.0 ft ²		
8. Floor types		c. N/A	
a. Slab-On-Grade Edge Insulation	R=0.0, 219.0(p) ft		
b. N/A		14. Hot water systems	
c. N/A		a. Electric Resistance	Cap: 50.0 gallons
9. Wall types			EF: 0.90
a. Concrete, Int Insul, Exterior	R=4.1, 1199.0 ft ²	b. N/A	
b. Frame, Steel, Adjacent	R=11.0, 320.0 ft ²		
c. N/A		c. Conservation credits	
d. N/A		(HR-Heat recovery, Solar	
e. N/A		DHP-Dedicated heat pump)	
10. Ceiling types		15. HVAC credits	PT,
a. Under Attic	R=19.0, 2450.0 ft ²	(CF-Ceiling fan, CV-Cross ventilation,	
b. N/A		HF-Whole house fan,	
c. N/A		PT-Programmable Thermostat,	
11. Ducts		MZ-C-Multizone cooling,	
a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 200.0 ft		MZ-H-Multizone heating)	
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Melina Mercaghe

Date: 12/13/07

Address of New Home: 209 SW Timber Ridge Dr City/FL Zip: Lake City, FL 32055



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.5)



Project Summary
Entire House
MARONDA HOMES

Job: AUSTIN 4 BDR
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

For: AUSTIN 4BDR

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	32841 Btuh
Ducts	4522 Btuh
Central vent (50 cfm)	2024 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	39387 Btuh

Sensible Cooling Equipment Load Sizing

Structure	19727 Btuh
Ducts	9690 Btuh
Central vent (50 cfm)	930 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	29437 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft ²)	2236	2236
Volume (ft ³)	17888	17888
Air changes/hour	0.32	0.16
Equiv. AVF (cfm)	95	48

Latent Cooling Equipment Load Sizing

Structure	4477 Btuh
Ducts	2266 Btuh
Central vent (50 cfm)	1758 Btuh
Equipment latent load	8501 Btuh
Equipment total load	37937 Btuh
Req. total capacity at 0.76 SHR	3.2 ton

Heating Equipment Summary

Make	TEMPSTAR
Trade	HEAT PUMP
Model	N4H342AKA
Efficiency	8.7 HSPF
Heating input	0 Btuh @ 47°F
Heating output	0 °F
Temperature rise	0 cfm
Actual air flow	0.000 cfm/Btuh
Air flow factor	0.60 in H2O
Static pressure	
Space thermostat	

Cooling Equipment Summary

Make	TEMPSTAR
Trade	HEAT PUMP
Cond	N4H342AKA
Coil	FEM4X4800
Efficiency	14 SEER
Sensible cooling	32300 Btuh
Latent cooling	10200 Btuh
Total cooling	42500 Btuh
Actual air flow	1270 cfm
Air flow factor	0.043 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.78

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



wrightsoft Right-Suite Residential 6.0.90 RSR21115

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2007-Sep-12 12:51:49

Page 1

Project Information

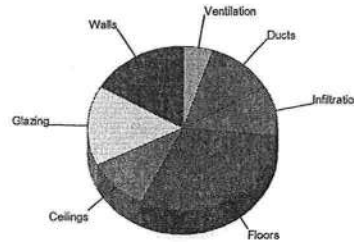
For: AUSTIN 4BDR

Design Conditions

Location:		Indoor:		Heating	Cooling
Gainesville, FL, US		Indoor temperature (°F)		70	75
Elevation: 151 ft		Design TD (°F)		37	17
Latitude: 30°N		Relative humidity (%)		30	50
		Moisture difference (gr/lb)		10.6	52.0
Outdoor:		Heating	Cooling		
Dry bulb (°F)	33	92			
Daily range (°F)	-	19 (M)			
Wet bulb (°F)	-	77			
Wind speed (mph)	15.0	7.5			
		Infiltration:			
		Method	Simplified		
		Construction quality	Average		
		Fireplaces	0		

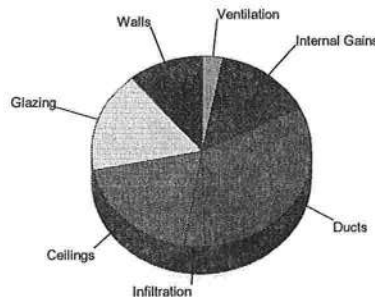
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.5	6541	16.6
Glazing	47.0	6203	15.7
Doors	0.0	0	0.0
Ceilings	1.8	4126	10.5
Floors	10.0	12109	30.7
Infiltration	2.4	3862	9.8
Ducts		4522	11.5
Piping		0	0.0
Humidification		0	0.0
Ventilation		2024	5.1
Adjustments		0	0.0
Total		39387	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.3	3347	11.0
Glazing	39.4	5201	17.1
Doors	0.0	0	0.0
Ceilings	2.6	5872	19.3
Floors	0.0	0	0.0
Infiltration	0.6	887	2.9
Ducts		9690	31.9
Ventilation		930	3.1
Internal gains		4420	14.6
Blower		0	0.0
Adjustments		0	0.0
Total		30347	100.0



Overall U-value = 0.154 Btuh/ft²-°F

Data entries checked.

Duct System Summary

Entire House

MARONDA HOMES

Job: AUSTIN 4 BDR
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

For: AUSTIN 4BDR

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.48 / 0.12 in H2O	0.48 / 0.12 in H2O
Lowest friction rate	1.935 in/100ft	1.935 in/100ft
Actual air flow	1270 cfm	1270 cfm
Total effective length (TEL)	31 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
MASTER BATH	h 258 4	25	25	1.93 5	4	x0	VIFx	25.0	0.0	st4
MASTER BDR	h 532 1	175	175	2.41 9	7	x0	VIFx	20.0	0.0	st4
FAMILY	c 425 0	175	175	3.22 6	7	x0	VIFx	15.0	0.0	st4
LIVING	c 355 6	175	175	3.22 6	7	x0	VIFx	15.0	0.0	st4
KITCHEN	c 319 4	100	100	4.83 9	6	x0	VIFx	10.0	0.0	ST2
DINING	h 468 8	175	175	2.41 9	7	x0	VIFx	20.0	0.0	ST2
BDR #4	h 301 1	80	80	4.83 9	6	x0	VIFx	10.0	0.0	st1
BDR #3	h 323 6	80	80	4.83 9	6	x0	VIFx	10.0	0.0	st1
BDR #2	h 310 4	80	80	2.41 9	6	x0	VIFx	20.0	0.0	st1
MASTER TOILET	c 308	25	25	1.93 5	4	x0	VIFx	25.0	0.0	st4
NOOK	c 301 7	135	135	9.67 7	6	x0	VIFx	5.0	0.0	ST2
LAUNDRY	c 868	25	25	4.83 9	4	x0	VIFx	10.0	0.0	ST3
BATH	c 136 6	20	20	2.41 9	4	x0	VIFx	20.0	0.0	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	260	260	2.419	488	10	0 x 0	VinlFlx	st2
ST2	Peak AVF	985	985	1.935	684	16	0 x 0	VinlFlx	
ST3	Peak AVF	25	25	4.839	416	4	0 x 0	VinlFlx	
st4	Peak AVF	575	575	1.935	524	12	0 x 0	VinlFlx	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1270	1270	6.0	1.935	582	18	x0 0		VIFx	

Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing

CLIENT MARIONDA HOMES LAKE CITY DATE 22 FEB 08
PROJECT NAME TIMBERLAHOS LOT #5 PROJECT NO. _____
EARTH CONTRACTOR 209 SKI TIMBER RIDGE DR. PERMIT NO. #0000 26623
COMPACTION REQUIREMENT (%) 95% ☐ Standard Proctor TESTED BY JWL
☒ Modified Proctor _____ FIELD CONTACT
TOTAL ON-SITE TIME _____ MILES FROM OFFICE _____
☐ Limerock ☐ Subgrade ☐ Pipe Backfill ☒ Building Pad ☒ Building Footing ☐ Other

REMARKS _____

- * Density failed to meet minimum project requirement
- ** Retest indicates minimum density requirement was obtained.
- () Client is aware of unsatisfactory test results.

CERTIFICATE OF OCCUPANCY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-105

Building permit No. 000026623

Use Classification SFD/UTILITY

Fire: 25.68

Permit Holder THEODORE C. BROCK

Waste: 67.00

Owner of Building MARONDA HOMES INC. OF FLORIDA

Total: 92.68

Location: 209 SW TIMBER RIDGE DRIVE, LAKE CITY, FL

Date: 06/11/2008

Wayne H. Russ

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



Maronda Systems	4005 Maronda Way	Sanford FL 32771	(407) 321-0064	Fax (407) 321-3913
Engineer of Record:	Tomas Ponce, P.E.	367 Medallion PL.	Chuluota, FL 32766	FL PE # 50068,
Design Criteria: TPI	Design: Matrix Analysis	TEE-LOK software		

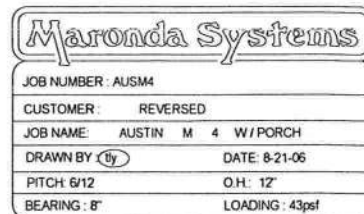
PLAN JOB #	LOT	ADDRESS	DIV/SUB	MODEL
1TM00501	5-1		ORA-1TM	AUSM4 LEFT

AUSTIN M 4

This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone.

Truss loading is in accordance with ASCE 7-02 These trusses are designed for an enclosed building.

[illegible]



[Signature]
OCT 12 2007

WARNING! BCSI-B1 SUMMARY SHEET - GUIDE FOR HANDLING

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more information. All other permanent bracing design is the responsibility of the Building Designer.

NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arrioste diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arrioste temporal de los trusses. Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arrioste de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restricción/Arrioste Permanente de Cuerdas y Miembros Secundarios para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.

- ⚠ The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, levantamiento, instalación, restricción y arrioste incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

- ⚠ Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

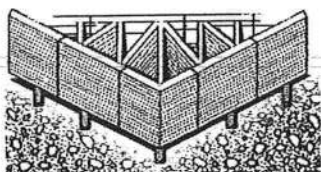
HANDLING — MANEJO

- ⚠ Avoid lateral bending. — Evite la flexión lateral.



- ⚠ The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



- ✓ If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

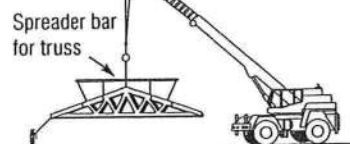
Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostres de los Trusses de Madera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenado de los trusses en área de trabajo.

- ⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



- ✓ Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.



- ⚠ Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.



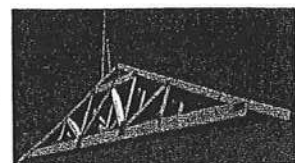
- ⚠ Do not store on uneven ground.

No almacene en tierra desigual.



HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

- ⚠ Warning! Don't overload the crane.
¡Advertencia! ¡No sobrecarga la grúa!
- ⚠ Never use banding alone to lift a bundle. Do not lift a group of individually banded bundles.
Nunca use sólo los empaques para levantar un paquete. No levante un grupo de empaques individuales.



- ✓ A single lift point may be used for bundles with trusses up to 45'.
Two lift points may be used for bundles with trusses up to 60'.
Use at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies.

Puede usar dos puntos de levantar para paquetes más de 60 pies.

Use por lo menos tres puntos de levantar para paquetes más de 60 pies.

- ⚠ Warning! Do not over load supporting structure with truss bundle.
¡Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.

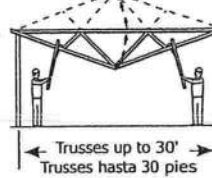
- ✓ Place truss bundles in stable position.
Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

- ✓ Trusses 20' or less, support at peak.
Levante del pico los trusses de 20 pies o menos.



- ✓ Trusses 30' or less, support at quarter points.
Levante de los cuartos de tramo los trusses de 30 pies o menos.



HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUAL

- ✓ Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

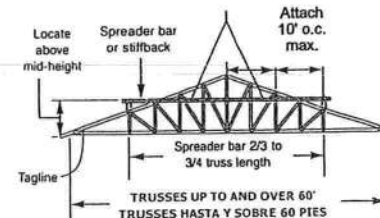
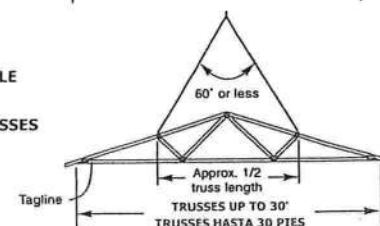
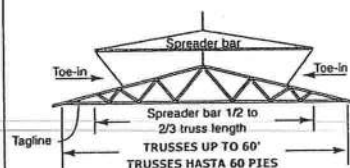
- ⚠ Warning! Using a single pick-point at the peak can damage the truss.

¡Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES

RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES



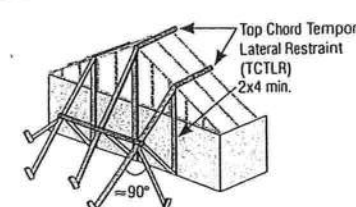
TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

- ⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

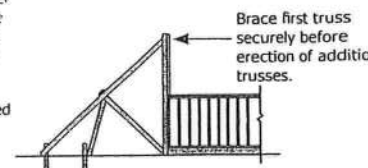
Vea el resumen BCSI B2 - Restricción/Arrioste Temporal y Instalación de los Trusses para más información.

- ✓ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).



- ⚠ Do not walk on unbraced trusses.
No camine en trusses sueltos.



This document summarizes the information provided in Section B1 of the 2006 Edition of Building Component Safety Information BCST - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Joints.

B1WARN11x17 20061115

Maronda Systems

MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

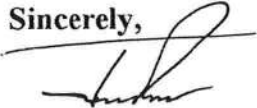
From: Maronda Systems
Tomas Ponce
Professional Engineer
State of Florida #0050068

Subject: Valley Trusses

All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

If you have any questions please feel free to call at 407-321-0064.

Sincerely,



Tomas Ponce, P.E.

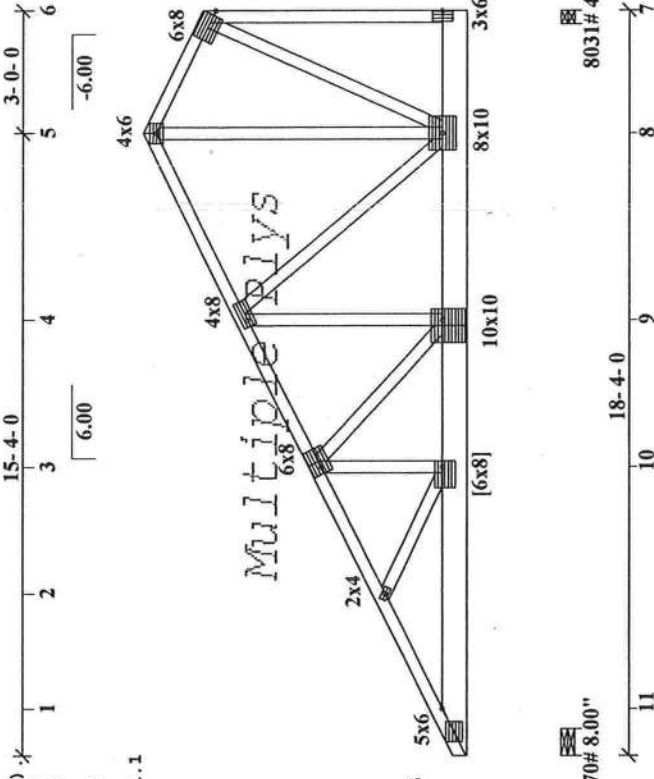
Date: 11/1/06

1000

9)	10-
0)	7-
1)	0-

16 18-
10 7-
10 18-
1
3
CQ= 1
:(s) :
Tensi
11372/33
-2982/
-2738
2/4313,

1-800-7030/



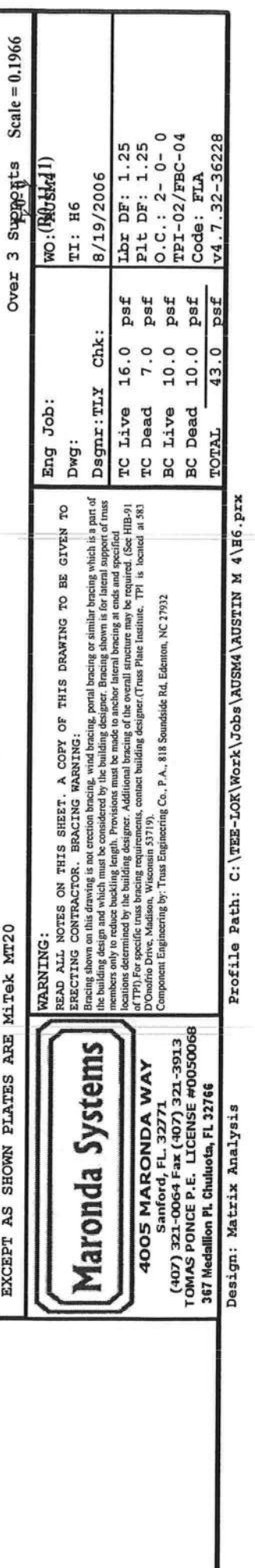
Scale:

100

1

25

04 26



Job: AUSTIN M 4

Customer: WO: AUSM4

TI: HGRD1

Qty: 1

TC: 2x 4 SP #2

2x 6 SP #2

BC: 2x 6 SP #2

WB: 2x 4 SP #3

MULTIPLE LOADS -- This design is the composite result of multiple loads. This 1-PHY Hip Master designed to carry 7' 0" open jacks (no webs) across center and Hip Jack Truss framed to BC

WindLoad per ASCE 7-02, C6C, V= 125mph, B= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0 Bld Type= part I= 60.0 ft W= 30.7 ft Truss in INT zone, TCCL= 4.2 psf, BCDL= 6.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance with Cq= 1 In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for Joint(s): 11, 12, 14, 15

TC: 2x 4 SP #2

2x 6 SP #2

BC: 2x 6 SP #2

WB: 2x 4 SP #3

MULTIPLE LOADS -- This design is the composite result of multiple loads. This 1-PHY Hip Master designed to carry 7' 0" open jacks (no webs) across center and Hip Jack Truss framed to BC

WindLoad per ASCE 7-02, C6C, V= 125mph, B= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0 Bld Type= part I= 60.0 ft W= 30.7 ft Truss in INT zone, TCCL= 4.2 psf, BCDL= 6.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance with Cq= 1 In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for Joint(s): 11, 12, 14, 15

1) 0-0-0 7) 23-8-0 13) 15-4-0

2) 3-11-4 8) 26-8-12 14) 12-4-0

3) 7-0-0 9) 30-8-0 15) 7-0-0

4) 12-2-3 10) 30-8-0 16) 0-0-0

5) 15-2-4 11) 23-8-0

6) 18-4-1 12) 18-4-1

TOTAL DESIGN LOADS

Uniform PLF From PLF To

TC Vert L+D -46 -1-0-0 -46 7-0-0

TC Vert L+D -100 7-0-0 -100 23-8-0

BC Vert L+D -46 23-8-0 -46 31-8-0

BC Vert L+D -40 0-0-0 -40 7-0-0

BC Vert L+D -87 7-0-0 -87 23-8-0

BC Vert L+D -40 23-8-0 -40 30-8-0

Concentrated LBS Location

BC Vert L+D -559 7-0-0 -559 23-8-0

BC Vert L+D -559 23-8-0

MAX. REACTIONS PER BEARING LOCATION----

X-Loc BSet Vert Horiz Uplift Y Type

0-4-0 1 581 0 -418 B Pin

12-4-0 1 3645 0 -1758 B H Roll

30-4-0 1 1310 0 -1770 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

WEB...FORCE...CSI...CF...WEB...FORCE...CSI...CF

2-15 223 0.07 C 5-12 1755 0.56

15-3 959 0.31 C 6-12 -541 0.15

3-14 -1907 0.47 C 12-7 -628 0.48

14-4 -642 0.17 C 11-7 938 0.30

14-5 -2005 0.85 C 11-8 198 0.06

13-5 98 0.03 C

1) Plate(s) OFFSET from joint center. The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

Bracing Schedule:

Web: Brace Pt Joint to Joint 1/2 3-14

Bracing shown is for visual purposes only.

1x4 continuous lateral bracing attached with 2-11d nails each member where shown. Bracing MUST be positioned to provide equal unbraced segments OR 2x4 "n" brace may be nailed flat to edge of web with 12d nails spaced 8" o.c. Brace must extend at least 90% of web length

2x6 Brace required on any web exceeding 14'.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-692/455, 2-3=-566/251, 3-4=-558/1135, 4-5=-558/1135, 5-6=-1367/649, 6-7=-1360/644, 7-8=-2126/1003, 8-9=-2226/1195, BOT CHORD 16-15=-376/594, 15-14=-188/494, 14-13=-95/193, 13-12=-95/193, 12-11=-864/1895, 11-10=-1031/1951, 2-15=-120/223, 15-3=-322/959, 3-14=-1907/873, 14-4=-642/384, 14-5=-2005/986, 13-5=-45/98, 5-12=-828/1755, 6-12=-541/335, 12-7=-628/257, 11-7=-313/938, 11-8=-69/198,

11-10=-1031/1951, 2-15=-120/223, 15-3=-322/959, 3-14=-1907/873, 14-4=-642/384, 14-5=-2005/986, 13-5=-45/98, 5-12=-828/1755, 6-12=-541/335, 12-7=-628/257, 11-7=-313/938, 11-8=-69/198,

in./Ratio in./Ratio Jnt(s)

I. Span -0.07/999 -0.14/999 7-6

Horiz. 0.02 0.04 NA

Max DL Deflection L/999 = -0.07

Long term deflection factor = 0.4050

1 2 3 4 5 6 7 8 9

4-3-2 0-4-3 3-10-3

581# 8.00" 3645# 8.00" 1310# 8.00"

1-0-0 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100

EXCEPT AS SHOWN PLATES ARE MITEK MT20 559.8# C/L: 12-4-0

559.8# Over 3 Supports Scale = 0.1952

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications or design data furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility for any errors or omissions in the design or fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSUTPI 1-1995 and NDS-97 to be incorporated as part of the building design by the Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by shabbling unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. Trusses shall be fabricated and erected in accordance with the information on Quality Control refer to ANSUTPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Erection instructions shall be more detailed than the design loads shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed.

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 O'Connor Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27032

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: (R) AUSM4

TI: HGRD1

8/19/2006

Lbr DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FLA

v4.7.32-36232

Profile Path: C:\TEE-LOK\Work\Jobs\AUSM4\AUSTIN M 4\HGRD1.prx

DESIGN INFORMATION
This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or creates no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records, for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied roof purlin, they should be braced at a maximum spacing of 10'-0". Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES
Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knots, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3/4" plate is 5" wide x 4" long. A 1/2" plate is 4" wide x 4" long. Slot (holes) run parallel to plate length specified. Double cuts on web members shall extend the full length of the web unless otherwise shown. Connector plates are minimum sizes based on the forces shown and need not to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES
All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for moving trusses in a straight and plumb position is required. Careful handling is essential and erection bracing is shown. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. This 1-PLY Hip Master designed to carry 5' 0" open jacks (no webs) across center and Hip Jack Truss framed to BC. This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O. Bracing Schedule: Webs Brace Pt Joint to Joint 1/2 3-10, 4-8. Bracing shown is for visual purposes only. Continuous lateral bracing attached to either edge of web(s) shown OR a T-brace, nailed flat to edge of web with 3"x 0.120" nails spaced 8" o.c. T-brace must extend at least 90% of web length and be same size, species, and grade as web. 2x6 T-Brace required on any web over 14'-0".

REACTANTS PER BEARING LOCATION ---
X-Loc BSet Vert Horiz Uplift I Type
0- 1- 4 1 1993 0 -1257 B Pin
27-4-12 1 1993 0 -1257 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

ALL COMPRESSION CHORDS are assumed to be continuously braced unless noted otherwise. WindLoad per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

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Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

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Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

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Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

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Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

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Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10

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WINDLOAD per ASCE 7-02, Exp. Cat. B, V=125mph, B=15.0 ft, I=1.00, Kzt=1.0
Bld Type= part L=27.5 ft W=27.5 ft Truss in INT zone, TCDF=0.0 psf, BCDL=0.0 psf Supported span based on int. wind zone. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 3, 8, 10



Joint Locations	PLF	From	To
1) 0-0-0	5	22-6-0	9) 13-9-0
2) 5-0-0	6	27-6-0	10) 5-0-0
3) 10-10-0	7	27-6-0	11) 0-0-0
4) 16-8-0	8	22-6-0	

Uniform	PLF	From	To
TC Vert L+D	-46	1-0-0	-46 5-0-0
TC Vert L+D	-77	5-0-0	-77 22-6-0
TC Vert L+D	-46	22-6-0	-46 28-6-0
BC Vert L+D	-40	0-0-0	-40 5-0-0
BC Vert L+D	-67	5-0-0	-67 22-6-0
BC Vert L+D	-40	22-6-0	-40 27-6-0

Concentrated	PLF	From	To
BC Vert L+D	-255	5-0-0	-255 5-0-0
BC Vert L+D	-255	22-6-0	-255 22-6-0

LL	in./Ratio	in./Ratio	Int(s)
I-Span	-0.32/999	-0.63/519	9-10
Horiz.	0.08	0.17	NA

Long term deflection factor = 1.50

GLOBAL MAX DEFLECTIONS

Scale = 0.2222

Eng Job: WO: AUSM4

Dwg: TI: HGRD3

Dsgnr: TLY Chk: 6/15/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

FBC-04/TPI-02/NDS-01

Code: FTA

v5.5.6-8163

Profile Path: T:\TEE-LOK\Work\Jobs\AUSM4\AUSTIN M 4\HGRD3.prx

Design: Matrix Analysis

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

387 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

EXCEPT AS SHOWN PLATES ARE MITEK MT20

WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing as shown and specified (See HTP-01 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Sandside Rd. Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

387 Medallion Pl. Chuluota, FL 32766

Job: AUSTIN M 4

Customer: WO:AUSM4

TI:HSIA

Qty:1

TC: 2x 4 SP #2

BC: 2x 6 SP #2

WB: 2x 4 SP #3

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

WindLoad per ASCE 7-02, CcC, V= 125mph, H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl I= 60.0 ft W= 36.0 ft Truss

INT zone, TCCL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

TC... FORCE... CSI... Cr... BC... FORCE... CSI...

1- 2 -4901 0.53 A 13-14 4476 0.85

2- 3 -4656 0.69 A 14-13 3978 0.71

3- 4 -3551 0.48 A 13-12 3160 0.58

4- 5 -3257 0.37 A 12-11 3160 0.58

5- 6 -3551 0.48 A 11-10 3978 0.71

6- 7 -4656 0.69 A 10- 9 4476 0.85

7- 8 -4901 0.53 A

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0- 4- 0 1 1594 -10 -767 B Pin

35- 8- 0 1 1548 0 -671 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

WEB... FORCE... CSI... Cr... WEBS... FORCE... CSI... Cr

2-14 228 0.07 C 12- 5 608 0.19

14- 3 539 0.17 C 5-11 816 0.26

3-13 -726 0.45 C 11- 6 -726 0.45

13- 4 816 0.26 C 6-10 539 0.17

4-12 608 0.19 C 10- 7 241 0.07

---GLOBAL MAX DEFLECTIONS---

LL

I Span in./Ratio in./Ratio Jnt(s)

Horiz. -0.49/845 -0.97/427 13-14

Max DL Deflection L/866 = -0.48

Long term deflection factor = 1.50

Joint Locations

1) 0- 0- 0 6) 24- 3- 8 11) 19- 0- 0

2) 6- 5- 1 7) 29- 6-15 12) 18- 0- 0

3) 11- 8- 8 8) 36- 0- 0 13) 17- 0- 0

4) 17- 0- 0 9) 36- 0- 0 14) 9- 0-13

5) 19- 0- 0 10) 26-11- 3 15) 0- 0- 0

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-4901/2001,2-3=-4656/1881,

3-4=-3551/1492,4-5=-3257/1457,5-6=-3551/1492,

6-7=-4656/1881,7-8=-4901/2001,

BOT CHORD 15-14=-1720/4476,14-13=-1445/3978,

C13-12=-981/3160,12-11=-981/3160,

C11-10=-1445/3978,10-9=-1720/4476,

WEBS 2-14=-205/228,14-3=-109/539,

C3-13=-726/440,13-4=-260/816,4-12=-226/608,

C12-5=-226/608,5-11=-260/816,11-6=-726/440,

6-10=-126/539,10-7=-205/241,

Design: Matrix Analysis

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl, Chuluota, FL 32766

Maronda Systems

EXCEPT AS SHOWN PLATES ARE MITEK MT20

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See Hibb-91 et al) For specific bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroville Drive, Madison, WI 53717)

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenport, NC 27932

Eng Job: WO: AUSM4

Dwg: TI: HSIA

Dsgnr: TLY Chk: 8/19/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-65713

Profile Path: C:\TEE-LOK\Work\Jobs\AUSM4\AUSTIN M 4\HSIA.prx

DESIGN INFORMATION

This design is for an individual building component and shall not be used for any other building component. The design is based on information provided by the client. The client is responsible for providing accurate information and for verifying the design. The design is based on the assumption that the building is a single-story building. The design is based on the assumption that the building is a single-story building. The design is based on the assumption that the building is a single-story building.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed before cutting or fabrication. Plates shall not be installed before cutting or fabrication. Plates shall not be installed before cutting or fabrication.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

Job: AUSTIN M 4

Customer: WO: AUSM4

TI: HS2A

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, loading, shipment and installation of trusses.

This truss has been designed with the following assumptions:

- 1- Truss is in accordance with AS/NZS 1171:1995 and NDS-97.
- 2- Truss is a part of the building design by the Building Designer. When required for approval by the Building Designer, the design records shall be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chord (top or bottom) are continuously braced by shabbling unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/TP1 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 6 SP #2

WB: 2x 4 SP #3

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

WindLoad per ASCE 7-02, Exp. Cat. B, Kzt= 1.0

H= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0

Bl'd type= encl I= 60.0 ft W= 36.0 ft Truss

in INT zone, TC/L= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

..TC.. FORCE.. CSI.. Cr ..BC.. FORCE.. CSI..

1- 2 -4851 0.54 A 15-14 4431 0.83

2- 3 -4637 0.52 A 14-13 4260 0.66

3- 4 -3862 0.53 A 13-12 3496 0.50

4- 5 -4289 0.51 A 12-11 3495 0.50

5- 6 -3861 0.53 A 11-10 4259 0.66

6- 7 -4636 0.52 A 10- 9 4430 0.83

7- 8 -4851 0.54 A

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cg= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 1594 -10 -796 B Pin

35- 8- 0 1 1548 0 -699 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

..WEB.. FORCE.. CSI.. Cr ..WEB.. FORCE.. CSI.. Cr

1- 2 -4851 0.54 C 12- 5 1251 0.40

2- 3 -4637 0.52 C 11- 4 1151 0.39

3- 4 -3862 0.53 C 10- 3 1151 0.39

4- 5 -4289 0.51 C 9- 2 1151 0.39

5- 6 -3861 0.53 C 8- 1 1151 0.39

6- 7 -4636 0.52 C 7- 0 1151 0.39

7- 8 -4851 0.54 C 6- 0 1151 0.39

8- 9 -4289 0.51 C 5- 0 1151 0.39

9- 10 -3861 0.53 C 4- 0 1151 0.39

10- 11 -4636 0.52 C 3- 0 1151 0.39

11- 12 -4851 0.54 C 2- 0 1151 0.39

12- 13 -4289 0.51 C 1- 0 1151 0.39

13- 14 -3861 0.53 C 0- 0 1151 0.39

14- 15 -4636 0.52 C -1- 0 1151 0.39

15- 16 -4851 0.54 C -2- 0 1151 0.39

16- 17 -4289 0.51 C -3- 0 1151 0.39

17- 18 -3861 0.53 C -4- 0 1151 0.39

18- 19 -4636 0.52 C -5- 0 1151 0.39

19- 20 -4851 0.54 C -6- 0 1151 0.39

20- 21 -4289 0.51 C -7- 0 1151 0.39

21- 22 -3861 0.53 C -8- 0 1151 0.39

22- 23 -4636 0.52 C -9- 0 1151 0.39

23- 24 -4851 0.54 C -10- 0 1151 0.39

24- 25 -4289 0.51 C -11- 0 1151 0.39

25- 26 -3861 0.53 C -12- 0 1151 0.39

26- 27 -4636 0.52 C -13- 0 1151 0.39

27- 28 -4851 0.54 C -14- 0 1151 0.39

28- 29 -4289 0.51 C -15- 0 1151 0.39

29- 30 -3861 0.53 C -16- 0 1151 0.39

30- 31 -4636 0.52 C -17- 0 1151 0.39

31- 32 -4851 0.54 C -18- 0 1151 0.39

32- 33 -4289 0.51 C -19- 0 1151 0.39

33- 34 -3861 0.53 C -20- 0 1151 0.39

34- 35 -4636 0.52 C -21- 0 1151 0.39

35- 36 -4851 0.54 C -22- 0 1151 0.39

36- 37 -4289 0.51 C -23- 0 1151 0.39

37- 38 -3861 0.53 C -24- 0 1151 0.39

38- 39 -4636 0.52 C -25- 0 1151 0.39

39- 40 -4851 0.54 C -26- 0 1151 0.39

40- 41 -4289 0.51 C -27- 0 1151 0.39

41- 42 -3861 0.53 C -28- 0 1151 0.39

42- 43 -4636 0.52 C -29- 0 1151 0.39

43- 44 -4851 0.54 C -30- 0 1151 0.39

44- 45 -4289 0.51 C -31- 0 1151 0.39

45- 46 -3861 0.53 C -32- 0 1151 0.39

46- 47 -4636 0.52 C -33- 0 1151 0.39

47- 48 -4851 0.54 C -34- 0 1151 0.39

48- 49 -4289 0.51 C -35- 0 1151 0.39

49- 50 -3861 0.53 C -36- 0 1151 0.39

50- 51 -4636 0.52 C -37- 0 1151 0.39

51- 52 -4851 0.54 C -38- 0 1151 0.39

52- 53 -4289 0.51 C -39- 0 1151 0.39

53- 54 -3861 0.53 C -40- 0 1151 0.39

54- 55 -4636 0.52 C -41- 0 1151 0.39

55- 56 -4851 0.54 C -42- 0 1151 0.39

56- 57 -4289 0.51 C -43- 0 1151 0.39

57- 58 -3861 0.53 C -44- 0 1151 0.39

58- 59 -4636 0.52 C -45- 0 1151 0.39

59- 60 -4851 0.54 C -46- 0 1151 0.39

60- 61 -4289 0.51 C -47- 0 1151 0.39

61- 62 -3861 0.53 C -48- 0 1151 0.39

62- 63 -4636 0.52 C -49- 0 1151 0.39

63- 64 -4851 0.54 C -50- 0 1151 0.39

64- 65 -4289 0.51 C -51- 0 1151 0.39

65- 66 -3861 0.53 C -52- 0 1151 0.39

66- 67 -4636 0.52 C -53- 0 1151 0.39

67- 68 -4851 0.54 C -54- 0 1151 0.39

68- 69 -4289 0.51 C -55- 0 1151 0.39

69- 70 -3861 0.53 C -56- 0 1151 0.39

70- 71 -4636 0.52 C -57- 0 1151 0.39

71- 72 -4851 0.54 C -58- 0 1151 0.39

72- 73 -4289 0.51 C -59- 0 1151 0.39

73- 74 -3861 0.53 C -60- 0 1151 0.39

74- 75 -4636 0.52 C -61- 0 1151 0.39

75- 76 -4851 0.54 C -62- 0 1151 0.39

76- 77 -4289 0.51 C -63- 0 1151 0.39

77- 78 -3861 0.53 C -64- 0 1151 0.39

78- 79 -4636 0.52 C -65- 0 1151 0.39

79- 80 -4851 0.54 C -66- 0 1151 0.39

80- 81 -4289 0.51 C -67- 0 1151 0.39

81- 82 -3861 0.53 C -68- 0 1151 0.39

82- 83 -4636 0.52 C -69- 0 1151 0.39

83- 84 -4851 0.54 C -70- 0 1151 0.39

84- 85 -4289 0.51 C -71- 0 1151 0.39

85- 86 -3861 0.53 C -72- 0 1151 0.39

86- 87 -4636 0.52 C -73- 0 1151 0.39

87- 88 -4851 0.54 C -74- 0 1151 0.39

88- 89 -4289 0.51 C -75- 0 1151 0.39

89- 90 -3861 0.53 C -76- 0 1151 0.39

90- 91 -4636 0.52 C -77- 0 1151 0.39

91- 92 -4851 0.54 C -78- 0 1151 0.39

92- 93 -4289 0.51 C -79- 0 1151 0.39

93- 94 -3861 0.53 C -80- 0 1151 0.39

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Job: AUSTIN M 4

Customer:

WO: AUSM4

TI: HSGRD1

Qty: 3

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0". Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/8" plate is 5" wide x 4" long. (6x6 plate is 6" wide x 8" long. (8x8 plate is 8" wide x 8" long. (10x10 plate is 10" wide x 10" long. (12x12 plate is 12" wide x 12" long. (14x14 plate is 14" wide x 14" long. (16x16 plate is 16" wide x 16" long. (18x18 plate is 18" wide x 18" long. (20x20 plate is 20" wide x 20" long. (22x22 plate is 22" wide x 22" long. (24x24 plate is 24" wide x 24" long. (26x26 plate is 26" wide x 26" long. (28x28 plate is 28" wide x 28" long. (30x30 plate is 30" wide x 30" long. (32x32 plate is 32" wide x 32" long. (34x34 plate is 34" wide x 34" long. (36x36 plate is 36" wide x 36" long. (38x38 plate is 38" wide x 38" long. (40x40 plate is 40" wide x 40" long. (42x42 plate is 42" wide x 42" long. (44x44 plate is 44" wide x 44" long. (46x46 plate is 46" wide x 46" long. (48x48 plate is 48" wide x 48" long. (50x50 plate is 50" wide x 50" long. (52x52 plate is 52" wide x 52" long. (54x54 plate is 54" wide x 54" long. (56x56 plate is 56" wide x 56" long. (58x58 plate is 58" wide x 58" long. (60x60 plate is 60" wide x 60" long. (62x62 plate is 62" wide x 62" long. (64x64 plate is 64" wide x 64" long. (66x66 plate is 66" wide x 66" long. (68x68 plate is 68" wide x 68" long. (70x70 plate is 70" wide x 70" long. (72x72 plate is 72" wide x 72" long. (74x74 plate is 74" wide x 74" long. (76x76 plate is 76" wide x 76" long. (78x78 plate is 78" wide x 78" long. (80x80 plate is 80" wide x 80" long. (82x82 plate is 82" wide x 82" long. (84x84 plate is 84" wide x 84" long. (86x86 plate is 86" wide x 86" long. (88x88 plate is 88" wide x 88" long. (90x90 plate is 90" wide x 90" long. (92x92 plate is 92" wide x 92" long. (94x94 plate is 94" wide x 94" long. (96x96 plate is 96" wide x 96" long. (98x98 plate is 98" wide x 98" long. (100x100 plate is 100" wide x 100" long. (102x102 plate is 102" wide x 102" long. (104x104 plate is 104" wide x 104" long. (106x106 plate is 106" wide x 106" long. (108x108 plate is 108" wide x 108" long. (110x110 plate is 110" wide x 110" long. (112x112 plate is 112" wide x 112" long. (114x114 plate is 114" wide x 114" long. (116x116 plate is 116" wide x 116" long. (118x118 plate is 118" wide x 118" long. (120x120 plate is 120" wide x 120" long. (122x122 plate is 122" wide x 122" long. (124x124 plate is 124" wide x 124" long. (126x126 plate is 126" wide x 126" long. (128x128 plate is 128" wide x 128" long. (130x130 plate is 130" wide x 130" long. 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(996x996 plate is 996" wide x 996" long. (998x998 plate is 998" wide x 998" long. (1000x1000 plate is 1000" wide x 1000" long. (1002x1002 plate is 1002" wide x 1002" long. (1004x1004 plate is 1004" wide x 1004" long. (1006x1006 plate is 1006" wide x 1006" long. (1008x1008 plate is 100

[illegible]

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the manufacturer's plans and to realize a continuing responsibility for each verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be fabricated over knuckles, bends or distorted grain. Members shall be cut for fitting wood to wood bearing.

Connector plates shall be located on both faces of the truss chord. The width of the plates shall be 5" wide x 4" long, unless otherwise fully indicated and shall be symmetrical about the plate longitudinal axis. A 5x6 plate is 5" wide x 6" long. A 6x6 plate is 6" wide x 6" long. Double (holes) on members in the plate length is 16" wide x 16" thick. Stubs (holes) on members shall meet at the centroid of the web; unless otherwise shown.

Connector plate sizes are minimum sizes based on the assumption that any member need not be increased for certain loading and fastener spacing. The design of the truss in the drawing shall be fabricated with fastener treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSIS/TPI 1-1993.

Following tracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during unloading and handling, delivery and installation to avoid damage. Temporary permanent trussing for holding trusses in a straight and plumb position and for resuming work on the design and installed by others. Careful handling is required to avoid damage to the trusses. Normal prescriptive action for trusses requires such temporary trussing during installation between trusses at each toppling and dismantling. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the design loads shall be applied to trusses at any time after the lifting and bracing is completed.

TC: 2x 4 SP #2
SC: 2x 4 SC #2
[] or []: Plate(s) OFFSET from joint center.
The Joint Detail Report must be included
with any submittal, inspection, and/or
fabrication documentation.
C&C, V= 125mph,
W= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0
Bld Type= encl L 36.0 ft W= 5.0 ft Truss
INT zone, TCDL= 4.2 psf, BCDL= 6.0 psf
Impact Resistant Covering and/or glazing Req

I. Span	in./Ratio	in./Ratio	Jnt(s).
Horiz.	-0.04/999	-0.08/599	3-1
	0.00	0.00	NA
Long term deflection factor =			1.50

Analysis based on Simplified Analog Model.
 402R1E LONCAGES -- This design as the
 composite result of multiple loadcases.
 ALL COMPRESSION chords are assumed to be
 continuously braced unless noted otherwise
 This Truss is designed in accordance with
 the 2004 Florida Building Code section
 3306.1 and referenced standards, U.N.O.
 3306.1-Plant Quality Assurance with Cq = 1.00
 .TC. .FORCE. CSI. Cr .BC. .FORCE. CSI
 1-2 -52 0.28 A 1-3 .0 0.21

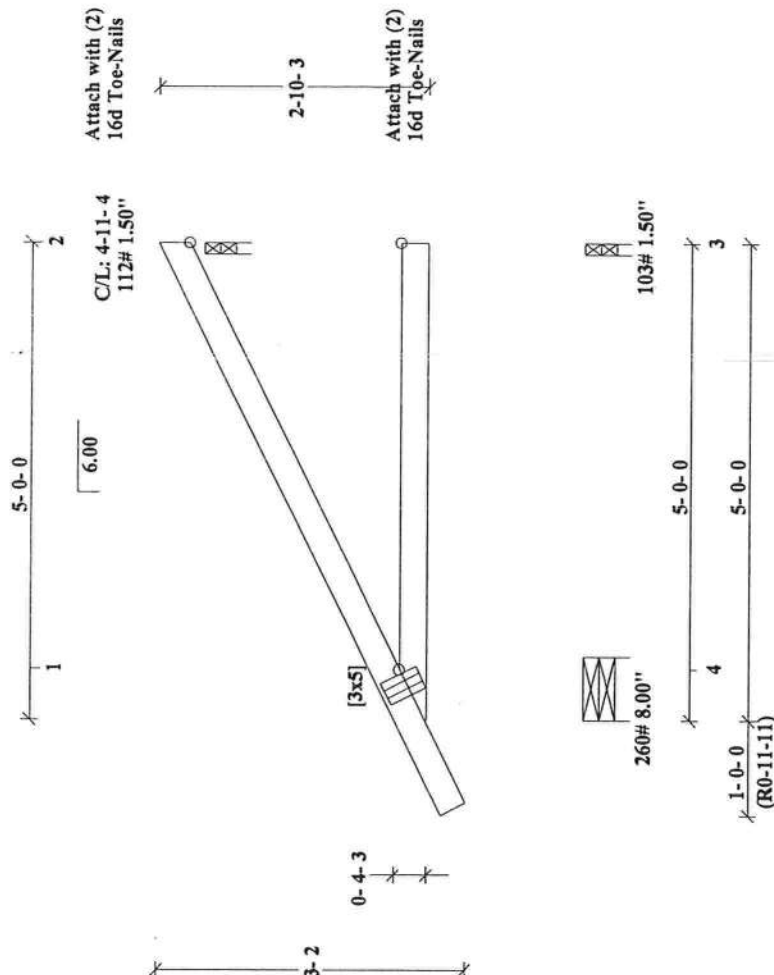
0.444

Unbalanced load case(s) have been checked.

	Joint Locations		
1) 0-6-7	3)	5-0-0	0
2) 5-0-0	4)	0-6-7	7

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

	MAX. REACTIONS PER BEARING LOCATION----					
	Bset	Vart	Horiz	Uplift	Y	Type
0-0-4	0	1	260	108	-248	B Pin
4-11-4	1	112	83	-143	T	Pin
4-11-4	1	103	0	-20	B	H Roll



EXCEPT AS SHOWN PLATES ARE MITAK MT20

Over 3 supports
Scale = 0.5771

WARNING:

READ ALL NOTES ON THIS SHEET, A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl, Chuluoth, FL 32766

Position: Matrix Analyst

Profile Path: T:\TEE-LOK\Work\Jobs\AUSM4\AUSTIN M 4\JL.prx

This design is for an individual building component and has not been based on information provided by the client. The designer disclaims any responsibility for damages as a result of failure or incorrect information, specifications and/or design intended to be used by the engineer by the client and the engineer is not responsible for the design or the failure of any specific project and accepts no responsibility or liability for the design or the failure of any specific project or the design or the failure of any specific project or the design or the failure of any specific project.

Prior to fabrication, the fabricator shall review this drawing to verify that the drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be mottled over, knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood joints. Cornerplate plates shall be located on both faces of the truss and with nails fully embedded and shall be sym. about the joint line unless otherwise shown. A 5/8" plate is 5" wide x 4' long. A 3/4" (6/8) plate is 4' wide x 8' long. Double (solid) run parallel in the plate length specified. Stools (solid run parallel) shall be cut at the centroid of the web unless otherwise shown. The minimum depth of the minimum end for each of the faces shown and may need to be increased if the truss is sheared with fire resistant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/UPU 1-1-1995

All tracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during unloading and handling, delivery and installation to avoid damage. Temporary permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces that be designed and installed by others. Required bracing shall be designed and installed by others. Normal precautionary action for trusses requires that temporary bracing during installation between trusses to avoid tipping and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of administration forces greater than the design loads that will be applied to the trusses shall be avoided. The weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

```

IC: 2x 4 SP #2
EC: 2x 4 SP #2
{|||} or {}: Plate(s) OFFSET from joint center.
The Joint Detail Report must be included
with any submittal, inspection, and/or
fabrication documentation.

```

This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.

	LL	TL
I.Span	in./Ratio -0.01/999	in./Ratio -0.01/999
Horiz.	0.00	0.00
	Long term deflection factor = 1.50	NA

Analysis based on Simplified Analog Model.
MULTIPLE LOADCASES -- This design is the
composite result of multiple loadcases

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise
C&C. V=125mph
WndLd per ASCE 7-02.

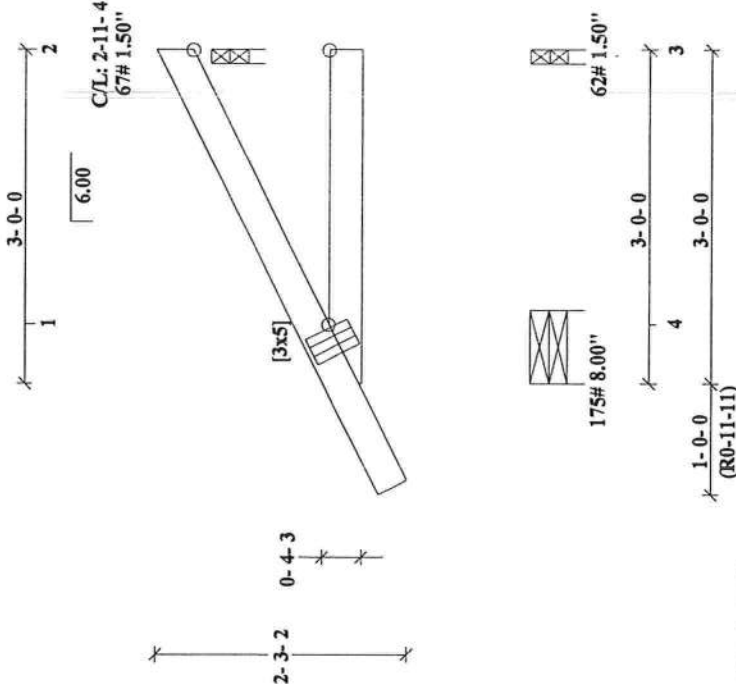
in END zone, TCCL= 4.2 psf, BCDL= 6.0 psf
Impact Resistant Covering and/or Glazing R
..TC. ..FORCE..CSI. Cr ..BC. ..FORCE..CSI

Joint Locations

1)	0-6-7	3)	3-0-0
2)	3-0-0	4)	0-6-7

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

MAX.	REACTI	PER BEAR	LOCAT	TYPE
0-4	0	1	175	Y
2-11	4	1	67	T
2-11	4	1	92	T
2-11	4	1	62	T
2-11	4	1	0	T



EXCEPT AS SHOWN PLATES ARE MITEX MT20

Over 3 Supports Scale = 0.6093

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE CONTRACTOR FOR HIS RECORD.

Maronda Systems

4005 MARONDA WAY
Sanford, FL. 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluoth, FL 32766

Design: Matrix Analysis

Eng vob:
Dwa:

Dsgnr: TLY Chk:

TC Live	16.0
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TC Dead 7.0

BC Live 10.0

BC Dead	10.0
TOTAL	43.0

Profile Path: T:\TEE-LOK\Work\Jobs\AUSM4\AUSTIN M 4\J2.prx

This design is for an individual building component and has not been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the use of this information as it may relate to a specific project. The trust designer does not exercise an opinion with regard to fabrication, handling, shipment and installation of trusses. This trust has been designed as an individual building component in accordance with ANSI/APA-1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When the building design is complete, the Building Designer should show drawings should not be checked to be sure that the data obtained are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special approval for struts or occupancy loads. This trust has not been designed for storage or occupancy loads. The trust designer disclaims any responsibility for damages continuously caused by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid coupling, they should be braced at a maximum spacing of 10' o.c. n.e. Consistent plates that are not meeting ASTM A 653, 48 gal, zinc coated galvalume, should be replaced with galvalume, otherwise shown.

PREPARATION NOTES

Prior to fabricating, the fabricator shall review this drawing and the applicable specifications and standards in conformance with the applicable codes, laws and regulations, and to ensure compliance with all applicable codes, laws and regulations. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be oriented before cutting or fabrication, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing.

Connector plates shall be located on both faces of the truss members with nails fully embedded and shall be sym. about the joint line. The length of the connector plates shall be 4" long, unless otherwise shown. A 3x4 plate is 3" wide & 4" long. A 4x6 plate is 4" wide & 6" long. A 6x8 plate is 6" wide & 8" long. Slots (holes) run parallel to the plate length specified. Double holes on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the member shown and may need to be increased for certain loading and/or erection stresses. This truss is not to be fabricated from a single piece of material. The truss is to be fabricated from two (2) pieces of material. For additional information on Quality Assurance refer to: ANSI/CEC-1198.

[illegible]

TC: 2x 4 SP #2
BC: 2x 4 SP #2
() or {}: Plate(s) OFFSET from joint center.
The Joint Detail Report must be included
with any submittal, inspection, and/or
fabrication documentation. C&C, V= 125mph,
H= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0
Bld Zone= encl I= 36.0 ft W= 1.0 ft Truss
in INT zone, TCDL= 4.2 psf, ECDL= 6.0 psf
Impact Resistant covering and/or Glazing Req
-----GLOBAL MAY BE DIFFERENT-----

	IN			TL		
	in./Ratio	in./Ratio	in./Ratio	TL	in./Ratio	Jnt(s)
I.Span	0.00/999	0.00/999	0.00/999			2-1
Horiz.	0.00	0.00	0.00			NA
Long term deflection factor =						1.50

This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O. In-Plant Quality Assurance with Cq = 1.00

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Qty: 8

[] or {}: Plate(s) OFFSET from joint center.
The Joint Detail Report must be included
with any submittal, inspection, and/or

fabrication documentation. C&C, V= 125mph,
 Windload per ASCE 7-02, C&C, V= 125mph,
 H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0
 Bld Type= encl L= 36.0 ft W= 1.0 ft Truss
 in INT zone, TCDL= 4.2 psf, ECDL= 6.0 psf
 Impact Resistant Covering and/or Glazing Re-
 -----CLIMATE SYSTEMS

	IN			TL		
	in./Ratio	in./Ratio	in./Ratio	TL	in./Ratio	Jnt(s)
I.Span	0.00/999	0.00/999	0.00/999			2-1
Horiz.	0.00	0.00	0.00			NA
Long term deflection factor =						1.50

This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O. In-Plant Quality Assurance with Cq = 1.00

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Unbalanced load case(s) have been checked.
Joint Locations
1) 0-6-7 3) 1-0-0
2) 1-0-0 4) 0-6-7
This truss is designed to bear on multiple
supports. Interior bearing locations should
be marked on truss. Shim or wedge if
unnecessary to achieve full bearing
-----MAX. REACTIONS PER BEARING LOCATION-----
X-Loc Bset Vert Horiz Uplift Y Type
0-4-0 1 89 22 -128 B Pin
0-11-4 1 23 18 -30 T Pin
0-11-4 1 21 0 -3 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE

```

**Attach with (2)
16d Toe-Nails**

0-4-3

1-3-2

0-10-3
Attach w

1-0-0
1
2
6.00
C/L: 0-1
73# 15

89# 8.00" 21# 1.50"

$$\begin{array}{c} \text{1-0-0} \\ \hline \text{4} \\ \text{1-0-0} \end{array} \quad \begin{array}{c} \text{3} \\ \hline \text{1-0-0} \end{array}$$

EXCEPT AS SHOWN PLATES ARE MITAK MT20

Over 3 supports Scale = 0.8059

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
THOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl, Chuluota, FL 32768

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR, BRACING WARNING:

Bracing shown in this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 or TPI). For specific truss truss bracing requirements, consult building designer (Truss Plate Institute, TPI is located at 383 D'Arcy Drive, Madison, Wisconsin 53719).

Truss Engineering Co., P.A., 418 Sandstone Rd., Edenport, NC 27032
Commercial Engineering Inc., Truss Engineering Co., P.A., 418 Sandstone Rd., Edenport, NC 27032

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd., Edenboro, NC 27932

Eng Job:
Dwg:

Dsgmr: TLY Chk:

TTC Live	16.0	psf	Lbr DF: 1.25
TTC Dead	7.0	psf	Plt DF: 1.25
			O.C.: 2-0-0

BC Live	10.0	psf
BC Dead	10.0	psf
TOTAL	43.0	psf

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\AUSM4\AUSTIN M 4\J3.PFX

Job: AUSTIN M 4

Customer: WO:AUSM4

TI: J5SA

Qty: 4

TC: 2x 4 SP #2

BC: 2x 6 SP #2

MULTIPLE JOINTS -- This design is the composite result of multiple loadcases. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

..TC.. FORCE.. CSI.. Cr.. BC.. ..FORCE.. CSI..

1- 2 54 0.51 A 1- 3 4 0.33

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s).

I. Span 0.16/362 0.16/362 2-1

Horiz. 0.00 0.00 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windload per ASCE 7-02, $C_{fc} = 1.0$, $C_{fc} = 1.0$, $H = 15.0$ ft, $I = 1.00$, $Exp.Cat. B$, $K_{zt} = 1.0$

Bld Type= part I= 60.0 ft W= 5.0 ft Truss

in INT zone, $TCDI = 0.0$ psf, $BCDI = 0.0$ psf

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=0/54,

BOT CHORD 1-3=-3/4,

Unbalanced load case(s) have been checked.

1) 0- 2- 9 3) 5- 0- 0

2) 5- 0- 0 4) 0- 2- 9

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 0-12 1 211 220 -392 B Pin

4-11- 4 1 101 147 -279 T Pin

4-11- 4 1 118 0 -151 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

PROVIDE HORIZONTAL CONNECTION PER SCHEDULE

DESIGN INFORMATION

This design is for an individual building component and is not to be used for any other purpose without the approval of the designer. The design is based on the information provided by the client. The design is based on the information provided by the client. The design is based on the information provided by the client.

FABRICATION NOTES

For fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood joining. Connector plates shall be located on both faces of the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x6 plate is 6" wide x 6" long. Slots (holes) in parallel to the plate length specified. Double cut on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for field connections. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI-1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and lateral movement. The supervision of erection of trusses shall be under the direct supervision of professional engineer or architect. Concentration of construction loads greater than he design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.5596

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 381 Oronoto Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: AUSM4

TI: J5SA

8/21/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FTA

v4.7.32-66185

Profile Path: C:\TEE-LOK\Work\Jobs\AUSM4\AUSTIN M 4\J5SA.prx

Job: AUSTIN M 4

Customer: WO:AUSM4

TI: JGRD3

Qty: 2

DESIGN INFORMATION

This design is for an individual building component and as been based on information provided by the client. The engineer disclaims any responsibility for damages as a result of faulty or incomplete information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and 4DS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown otherwise, all trusses have been designed for compression chords (top or bottom). The design is based on the assumption that the truss is continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a rigidly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

For fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be noted and corrected before fabrication. Plates are to be welded to the truss members and shall be installed over webholes, bolts or distorted grain. Members shall be cut for tight fitting wood to wood cutting. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/8" plate is 5" wide x 1" long. A 6/8" plate is 6" wide x 8" long. Slots (holes) in parallel to the plate length specified. Double cut on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry practices. Trusses are to be handled with particular care during banding and handling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and loming. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. Right End vertical designed for wind. Windload per ASCE 7-02, C6C, V= 125mph, H= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0 Bld Type= Part L= 60.0 ft W= 7.0 ft Truss in END zone, TCDF= 0.0 psf, BCDL= 0.0 psf Supported span based on int. wind zone. FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=0/99, BOT CHORD 4-3=0/82, Webs 3-2=-120/23, ---GLOBAL MAX DEFLECTIONS--- in./Ratio in./Ratio Jnt(s) I. Span -0.07/999 -0.13/607 2-1 Horiz. 0.01 0.02 NA Max DL Deflection L/999 = -0.07 Long term deflection factor = 1.50

TC: 2x 4 SP #2

BC: 2x 6 SP #2

WB: 2x 4 SP #3

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. This hip jack truss has been designed to carry open-ended jacks.

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0-1-1 1 127 -165 -210 B Pin

6-11-6 1 249 0 -98 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Multiple Loadcases

1) 0-0-0 3) 7-0-2

2) 7-0-2 4) 0-0-0

Uniform PLF From PLF To

TC Vert L+D -11 -1-5-0 0 0-0-0

TC Vert L+D 0 0-0-0 -56 7-0-2

BC Vert L+D 0 0-0-0 -48 7-0-2

..TC ..FORCE..CSI..CF..BC..FORCE..CSI..

1-2 99 0.27 C 4-3 82 0.18

..WEB.. ..FORCE..CSI..CF..WEB.. ..FORCE..CSI..CF

3-2 -120 0.17 C

Diagram

7-0-2

4.24

2x4

6x8

2x4

3-3-1

0-7-14

Attach with (2) 16d Toe-Nails

3-1-10

Attach with (2) 16d Toe-Nails

127# 2.12"

249# 1.50"

7-0-2

7-0-2

1-5-0

(R1-4-12)

EXCEPT AS SHOWN PLATES ARE MiTek MT20

Scale = 0.3666

WARNING:

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Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #00500668

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\AUSM4\AUSTIN M 4\JGRD3.pfx

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: AUSM4

TI: JGRD3

8/19/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FLA

v4.7.32-65744

Job: AUSTIN M 4

Customer:

WO: AUSM4

TI: JS3

Qty: 2

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 89 26 -123 B Pin

0-11- 4 1 23 21 -30 T Pin

0-11- 4 1 21 0 -2 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 89 26 -123 B Pin

0-11- 4 1 23 21 -30 T Pin

0-11- 4 1 21 0 -2 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

1) 0- 5- 5 3) 1- 0- 0

2) 1- 0- 0 4) 0- 5- 5

WndLoad per ASCE 7-02, C&C, V= 125mph, H= 15.0 ft, I= 1.00, Exp.Cat. B, K&t= 1.0

Bld Type= encl L= 36.0 ft W= 1.0 ft Truss in INT zone, TCDL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..

1- 2 -12 0.01 A 1- 3 3 0.01

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)

I. Span 0.00/999 0.00/999 3-1

Horiz. 0.00 0.00 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

DESIGN INFORMATION

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This truss has been designed as an individual building component in accordance with ANSI/TPI-1:1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by shoring unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased to prevent tearing or splitting of the wood members. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI-1:1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary bracing for trusses requires such temporary bracing during installation to avoid buckling and twisting of trusses shall be done during the erection of the trusses. The supervision of erection of trusses shall be under the control of a Professional Engineer. If needed, Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

EXCEPT AS SHOWN PLATES ARE MITEK MT20

WARNING:

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Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenon, NC 27932

Maronda Systems

4005 MARONDA WAY

Saunder, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

Over 3 Supports Scale = 0.7294

Eng Job:

Dwg:

Dsgnr: TLY Chk:

WO: AUSM4

TI: JS3

8/19/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-65657

1-0-0 1 2

6.00

C/L: 0-11-4

23# 1.50"

3x4

0-1-0 0-1-0

3.00

0-4-3

1-3-2

0-9-3

Attach with (2)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

89# 8.00"

21# 1.50"

1-0-0 4 3

(R0-11-11)

Job: AUSTIN M 4

Customer:

WO: AUSM4

TI: MH3

Qty: 1

DESIGN INFORMATION

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FABRICATION NOTES

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This truss is not to be fabricated with fire resistant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

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TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

2x 4 SP #2

4-11, 5-10, 5-9

6-8

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

WindLoad per ASCE 7-02, C&C, V= 125mph, H= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0

Bld Type= encl I= 60.0 ft W= 36.0 ft Truss in INT zone, TCDL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-2767/1162, 2-3=-2407/1047, 3-4=-2117/1010, 4-5=-2437/1051, 5-6=-1771/775, 6-7=-94/107, 7-8=-1238/2412, 12-11=-1239/2409, 11-10=-1103/2440, 10-9=-954/2227, 9-8=-599/1402,

Web 12-2=-5/122, 2-11=-343/265, 3-11=-208/791, 11-4=-436/171, 4-10=-75/215, 10-5=-141/393, 5-9=-763/404, 9-6=-315/1032, 6-8=-1900/813, 8-7=-118/111,

Multiple Loads -- This design is the composite result of multiple loads. Right End vertical designed for wind. Bracing Schedule: Webs Brace Pt Joint to Joint 1/2 4-11, 6-8

Bracing shown is for visual purposes only. 1x4 continuous lateral bracing attached with 2- 11d nails each member where shown. Bracing MUST be positioned to provide equal unbraced segments OR 2x4 "T" brace may be nailed flat to edge of web with 12d nails spaced 8" o.c. Brace must extend at least 90% of web length. 2x6 Brace required on any web exceeding 14'. ..TC.. ..FORCE.. ..CSI.. Cr ..BC.. ..FORCE.. ..CSI..

1- 2 -2767 0.40 A 13-12 2412 0.86

2- 3 -2407 0.38 A 12-11 2409 0.83

3- 4 -2117 0.37 A 11-10 2440 0.84

4- 5 -2437 0.37 A 10-9 2227 0.85

5- 6 -1771 0.36 A 9-8 1402 0.71

6- 7 107 0.32 A

.....GLOBAL MAX DEFLECTIONS.....

LI TL

I. Span in./Ratio in./Ratio Jnt(s)

Horiz. -0.29/999 -0.57/742 10-11

0.07 0.13 NA

Max DL Deflection L/999 = -0.28

Long term deflection factor = 1.50

Joint Locations

1) 0-0-0 6) 29-9-0 11) 11-0-0

2) 5-9-4 7) 36-0-0 12) 5-9-4

3) 11-0-0 8) 36-0-0 13) 0-0-0

4) 17-3-0 9) 27-8-0

5) 23-6-0 10) 20-0-0

In-Plant Quality Assurance with Cq= 1

In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s):

6, 8, 9

MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0-4-0 1 1602 295 -768 B Pin

35-10-12 1 1540 0 -648 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

WEBS.. ..FORCE.. ..CSI.. Cr ..WEB.. ..FORCE.. ..CSI.. Cr

12- 2 122 0.04 C 10- 5 393 0.10

2-11 -343 0.24 C 5- 9 -763 0.61

3-11 791 0.25 C 9- 6 1032 0.33

11- 4 -436 0.30 C 6- 8 -1900 0.82

4-10 215 0.06 C 8- 7 -118 0.46

WARNING:

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Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32711

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

387 Medallion Pl. Chuluota, FL 32766

EXCEPT AS SHOWN PLATES ARE

MiTek MT20

Scale = 0.1727

Eng Job:

WO: AUSM4

Dwg:

TI: MH3

Dsgnr: TLY

Chk:

8/19/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FLA

v4.7.32-36217

Profile Path: C:\TEB-LOK\Work\Jobs\AUSM4\AUSTIN M 4\MH3.prx

Job: AUSTIN M 4

Customer: WO: AUSM4

TI: MHGRD1

Qty: 2

TC: 2x 4 SP #2

2x 6 SP #2

BC: 2x 6 SP #2

WB: 2x 4 SP #3

2x 4 SP #2

3-11, 4-10, 6-10

3-11, 4-10, 6-10

7-9

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer assumes no responsibility for damages as a result of failure or other consequences of the design and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANS/ITPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

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PRECAUTIONARY NOTES

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Multiple Plys

Diagram showing truss structure with dimensions and member sizes.

3244# 8.00"

3394# 3.50"

559.8#

Scale = 0.1784

Customer: WO: AUSM4

TI: MHGRD1

Qty: 2

Multiple Loads -- This design is the composite result of multiple loads. Right End vertical designed for wind. 2-PLY TRUSS! fasten w/3"x0.120" nails in staggered pattern per nailing schedule: TC- 2 per ft. BC- 2 per ft. WEBS- 2 per ft. Distribute loads equally to each ply. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 1.7 FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=-6306/1970, 2-3=-6275/1803, 3-4=-8648/2539, 4-5=-8767/2600, 5-6=-8767/2600, 6-7=-6270/1863, BOT CHORD 13-12=-1709/5563, 12-11=-1582/5618, 11-10=-2592/8729, 10-9=-1962/6497, 9-8=-80/65, Webs 2-12=0/150, 12-3=-15/895, 3-11=-1056/3343, 11-4=-768/496, 4-10=-10/42, 10-5=-596/373, 10-6=-716/2546, 6-9=-2019/880, 9-7=-2032/6837, 8-7=-3091/1036, 11-10=-2592/8729, 10-9=-1962/6497, 9-8=-80/65, 8-7=-3091/1036, 7-6=-3091/1036, 6-5=-8767/2600, 5-4=-8767/2600, 4-3=-8648/2539, 3-2=-6306/1970, 2-1=-6275/1803, 1-0=-6306/1970, 0-1=-6275/1803, 1-2=-6306/1970, 2-3=-6275/1803, 3-4=-8648/2539, 4-5=-8767/2600, 5-6=-8767/2600, 6-7=-6270/1863, 7-8=-3091/1036, 8-9=-2019/880, 9-10=-10/42, 10-11=-1582/5618, 11-12=-1709/5563, 12-13=-1056/3343, 13-14=-768/496, 14-15=-10/42, 15-16=-596/373, 16-17=-716/2546, 17-18=-2019/880, 18-19=-2032/6837, 19-20=-3091/1036, 20-21=-3091/1036, 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DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may affect a specific project with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANS/PTP 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be 5" about the joint unless otherwise shown. A 5/4" plate is 5" wide x 4" long. A 6/8" plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain loading and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/PTP 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bundling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No other than the design loads of the truss shall be applied to trusses until after all fastening and bracing is completed.

TC: 2x 4 SP #2
BC: 2x 6 SP #2
WB: 2x 4 SP #3
All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
C6C, V= 125mph, WndLoad per ASCE 7-02, C6C, V= 125mph, E= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0
Bld Type= encl L= 60.0 ft W= 36.0 ft Truss in INT zone, TC6L= 4.2 psf, BC6L= 6.0 psf Impact Resistant Covering and/or Glazing Req
TC, FORCE, CSI, Cr, BC, FORCE, CSI
1-2 -4904 0.54 A 12-11 4483 0.85
2-3 -4627 0.77 A 11-10 3878 0.73
3-4 -3339 0.44 A 10-9 3878 0.73
4-5 -3339 0.45 A 9-8 4484 0.85
5-6 -4627 0.77 A
6-7 -4904 0.54 A

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.
In-Plant Quality Assurance with Cq= 1
sec. 3.2.4 of TPI-1-02, for joint(s):
4
FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=-4904/1986, 2-3=-4627/1834, 3-4=-3339/1371, 4-5=-3339/1371, 5-6=-4627/1805, 6-7=-4904/1944, 7-8=-4627/1805, 8-9=-4904/1944, 9-10=-4627/1805, 10-11=-4904/1944, 11-12=-4627/1805, 12-13=-4904/1944, 13-14=-4627/1805, 14-15=-4904/1944, 15-16=-4627/1805, 16-17=-4904/1944, 17-18=-4627/1805, 18-19=-4904/1944, 19-20=-4627/1805, 20-21=-4904/1944, 21-22=-4627/1805, 22-23=-4904/1944, 23-24=-4627/1805, 24-25=-4904/1944, 25-26=-4627/1805, 26-27=-4904/1944, 27-28=-4627/1805, 28-29=-4904/1944, 29-30=-4627/1805, 30-31=-4904/1944, 31-32=-4627/1805, 32-33=-4904/1944, 33-34=-4627/1805, 34-35=-4904/1944, 35-36=-4627/1805, 36-37=-4904/1944, 37-38=-4627/1805, 38-39=-4904/1944, 39-40=-4627/1805, 40-41=-4904/1944, 41-42=-4627/1805, 42-43=-4904/1944, 43-44=-4627/1805, 44-45=-4904/1944, 45-46=-4627/1805, 46-47=-4904/1944, 47-48=-4627/1805, 48-49=-4904/1944, 49-50=-4627/1805, 50-51=-4904/1944, 51-52=-4627/1805, 52-53=-4904/1944, 53-54=-4627/1805, 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DESIGN INFORMATION

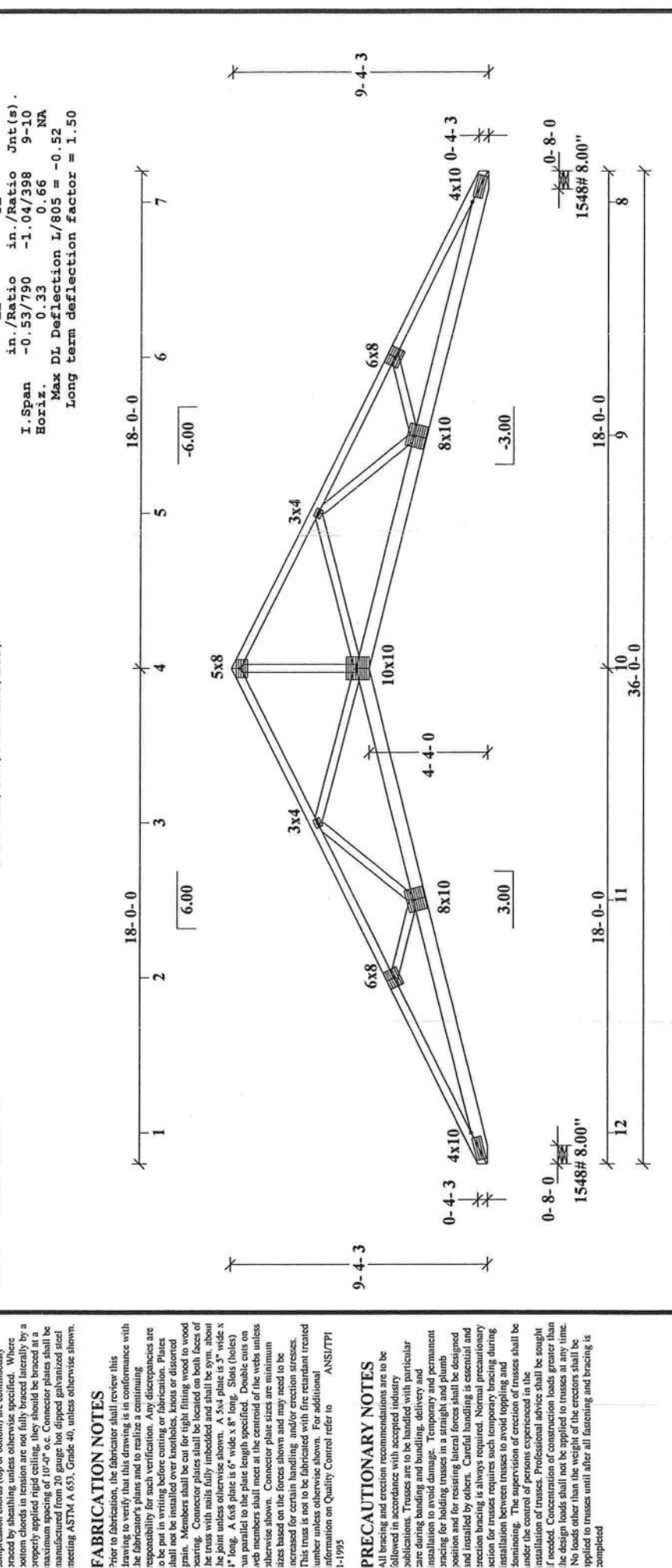
This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the client. The client is responsible for the accuracy of the information as it relates to the design and for the consequences of any errors or omissions in the design. The client is responsible for the accuracy of the information as it relates to the design and for the consequences of any errors or omissions in the design.

FABRICATION NOTES

For fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for right fitting wood to wood joint. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) in parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain loading and/or erection stresses. This truss is not to be fabricated with fire retardant treated wood unless otherwise specified. For additional information on Quality Control refer to ANS/ITP 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bundling and handling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during erection. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed.



Job: AUSTIN M 4

Customer: WO:AUSM4

TT:T2

Qty: 4

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and the design furnished to the truss designer by the client and the truss designer's agency. This information is for the use of the truss designer only and is not to be used for any other purpose. The truss designer shall be responsible for the design, fabrication, handling, shipment and installation of the truss. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by shabbling unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for right fitting wood to wood bearing. Connector plates shall be located on both faces of the truss unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for corner loading and/or erection stresses. The truss shall be fabricated with all members treated with preservative unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid tipping and crushing. The supervision of erection of trusses shall be under the direct supervision of the truss designer or the installer of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed

Customer: WO:AUSM4

TT:T2

Qty: 4

MULTIPLE LOADS -- This design is the composite result of multiple loads. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing. In-Plant Quality Assurance with Cq= 1 FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=-344/235, 2-3=-77/393, 3-4=-111/270, 4-5=-111/270, 5-6=-423/368, 6-7=-990/620, 7-8=-111/270, 8-9=-423/368, 9-10=-111/270, 10-11=-111/270, 11-12=-111/270, 12-13=-111/270, 13-14=-111/270, 14-15=-111/270, 15-16=-111/270, 16-17=-111/270, 17-18=-111/270, 18-19=-111/270, 19-20=-111/270, 20-21=-111/270, 21-22=-111/270, 22-23=-111/270, 23-24=-111/270, 24-25=-111/270, 25-26=-111/270, 26-27=-111/270, 27-28=-111/270, 28-29=-111/270, 29-30=-111/270, 30-31=-111/270, 31-32=-111/270, 32-33=-111/270, 33-34=-111/270, 34-35=-111/270, 35-36=-111/270, 36-37=-111/270, 37-38=-111/270, 38-39=-111/270, 39-40=-111/270, 40-41=-111/270, 41-42=-111/270, 42-43=-111/270, 43-44=-111/270, 44-45=-111/270, 45-46=-111/270, 46-47=-111/270, 47-48=-111/270, 48-49=-111/270, 49-50=-111/270, 50-51=-111/270, 51-52=-111/270, 52-53=-111/270, 53-54=-111/270, 54-55=-111/270, 55-56=-111/270, 56-57=-111/270, 57-58=-111/270, 58-59=-111/270, 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=====Joint Locations=====

1)	0- 7- 0	5) 10- 0-12	9) 4- 0-12
2)	3-11-15	6) 10- 0-12	10) 0- 7- 0
3)	5-11-15	7) 8- 0-12	
4)	7-11-15	8) 6- 0-12	

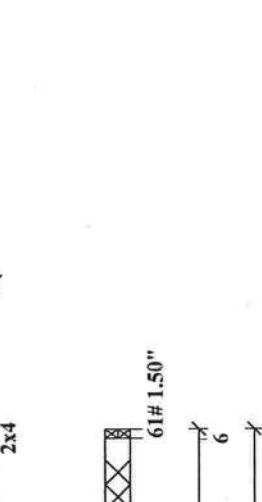
In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-273/89, 2-3=-175/52,
3-4=-116/38, 4-5=-35/12,
BOT CHORD 0-1=-263/87, 1-9=-8/35, 9-8=0/0,
8-7=0/0, 7-6=0/0,
Wabs 9-2=-163/210, 8-3=-62/122,
7-4=-108/170, 6-5=-38/62,
-----GLOBAL MAX DEFLECTIONS-----

	LL	TL	
I.Span	in./Ratio	in./Ratio	Jnt(s)
	-0.01/999	-0.02/999	2-1
Horiz.	0.00	0.01	NA
	Max DL Deflection L/999 = -0.01		
	Long term deflection factor = 1.50		

The diagram shows a rectangular frame. A horizontal line segment extends from the left side of the frame, labeled with the letter 'x'. A vertical line segment extends from the bottom side of the frame, labeled with the letter 'y'.



Scale = 0.3805

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing. Wind bracing, portal bracing or similar bracing which is a part of
the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss
members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified
locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91
of TPI). For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583
D'Orofino Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenston, NC 27932

Eng Job:	WO: AUSM4	
Dwg:	TI: V11	
Dsgnr: TLY	Chk:	8/19/2006
TC Live	16.0	psf
TC Dead	7.0	psf
BC Live	10.0	psf
BC Dead	2.0	psf
TOTAL	35.0	psf

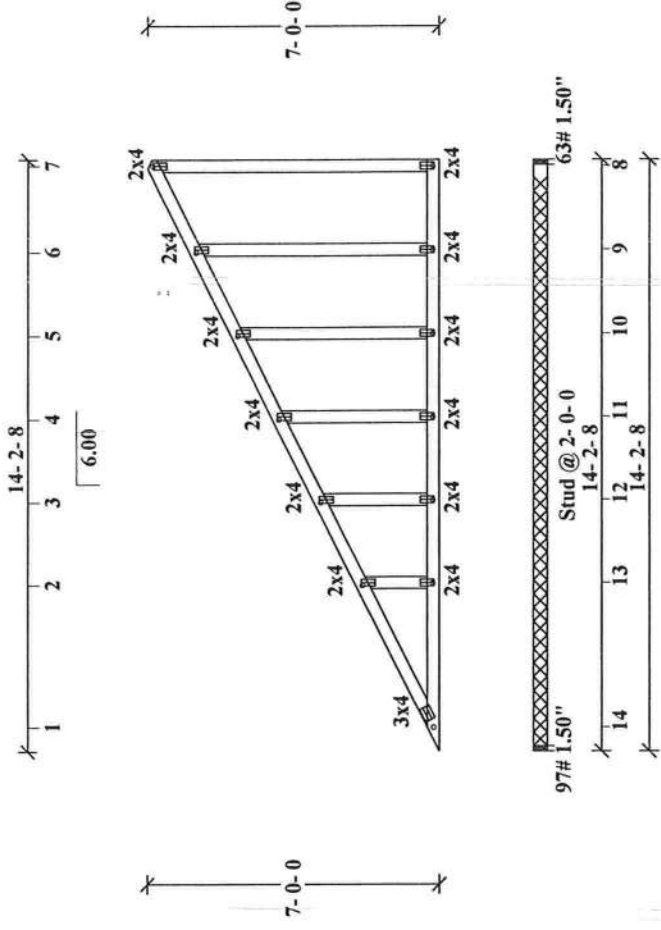
DESIGN INFORMATION
This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of design or construction information, specifications and/or design furnished to the truss designer by the client and the correctness of accuracy of this information as it may be used in a specific project and is not to be used for fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1993 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES
Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for right fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) in the joint unless otherwise shown. A 3x4 plate is 3" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) in web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain loading and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993.

PRECAUTIONARY NOTES
All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than 100 psf shall not be applied to trusses at any time. No design loads shall be applied to trusses until they are applied to trusses until after all fastening and bracing is completed.

Analysis based on Simplified Analog Model.
All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
Windload per ASCE 7-02, C&C, V= 125mph, E= 15.0 ft, I= 1.00, Exp. Cat. B, Kst= 1.0
Bld Type= encl L= 60.0 ft W= 14.2 ft Truss in INT zone, TCDL= 4.2 psf, BCDL= 1.2 psf
Impact Resistant Covering and/or Glazing Req
..TC.. FORCE.. CSI.. Cr ..BC.. ..FORCE.. CSI..
1- 2 -328 0.10 A 0- 1 -298 0.09
2- 3 -242 0.10 A 1-13 46 0.09
3- 4 -193 0.03 A 13-12 0 0.08
4- 5 -130 0.03 A 12-11 0 0.02
5- 6 -99 0.04 A 11-10 0 0.02
6- 7 -33 0.04 A 10- 9 0 0.02
9- 8 0 0.02
-----GLOBAL MAX DEFLECTIONS-----
LL TL
I. Span in./Ratio in./Ratio Jnt(s)
Horiz. 0.01 -0.01/999 -0.02/999 2-1
Max DL Deflection L/999 = -0.01
Long term deflection factor = 1.50

- Joint Locations**
- | | | |
|------------|--------------|-------------|
| 1) 0- 7- 0 | 6) 11-11-15 | 11) 8- 0-12 |
| 2) 3-11-15 | 7) 14- 0-12 | 12) 6- 0-12 |
| 3) 5-11-15 | 8) 14- 0-12 | 13) 4- 0-12 |
| 4) 7-11-15 | 9) 12- 0-12 | 14) 0- 7- 0 |
| 5) 9-11-15 | 10) 10- 0-12 | |
- In-Plant Quality Assurance with Cq= 1**
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-328/95, 2-3=-242/60, 3-4=-193/44, 4-5=-130/21, 5-6=-99/38, 6-7=-33/13,
BOT CHORD 0-1=-298/92, 1-13=-9/46, 13-12=0/0, 12-11=0/0, 11-10=0/0, 10-9=0/0, 9-8=0/0,
Webs 13-2=-163/182, 12-3=-64/98, 11-4=-96/128, 10-5=-89/131, 9-6=-103/150, 8-7=-39/57,



Scale = 0.2153

EXCEPT AS SHOWN PLATES ARE MITEK MT20

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, consult building designer. (Truss Plate Institute, TPI is located at 381 D'Ondra Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenon, NC 27932

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Eng Job:	WO: AUSM4
Dwg:	TI: V13
Dsgnr: TLY Chk:	8/19/2006
TC Live 16.0 psf	Lbr DF: 1.25
TC Dead 7.0 psf	Plt DF: 1.25
BC Live 10.0 psf	O.C.: 2- 0- 0
BC Dead 2.0 psf	TPI-02/FBC-04
TOTAL 35.0 psf	Code: FLA
	v4.7.32-36254

Job: AUSTIN M 4

Customer: WO:AUSM4

TT:V15

Qty:2

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2-84/106,2-3-85/107,

BOT CHORD 5-5-159/293,5-4-63/180,

WEBS 5-2-235/455,

.WEB. .FORCE..CSI. Cr 2- 3 107 0.15 A 5- 4 180 0.19

5- 2 455 0.11 C

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.

Windload per ASCE 7-02, C&C, V= 125mph, E= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl L= 8.0 ft W= 8.0 ft Truss

in END zone, TCDI= 0.0 psf, BCDI= 0.0 psf

Impact Resistant Covering and/or Glazing Req

.TC. .FORCE..CSI. Cr .BC. .FORCE..CSI.

1- 2 106 0.14 A 6- 5 293 0.19

.WEB. .FORCE..CSI. Cr 2- 3 107 0.15 A 5- 4 180 0.19

5- 2 455 0.11 C

Joint Locations

1) 0- 0- 0 3) 8- 0- 0 5) 4- 0- 0

2) 4- 0- 0 4) 8- 0- 0 6) 0- 0- 0

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 132 -113 -188 B Pin

7- 8- 0 1 129 0 -185 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)

I.Span 0.02/999 0.02/999 3-2

Horiz. 0.00 -0.01 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of design errors or omissions of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid tipping and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed

EXCEPT AS SHOWN PLATES ARE MITEK MT20

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building designed and which is to be provided by the building designer. Bracing shown is for lateral support of truss members only and is not to be relied upon for the stability of the building. For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050069

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Scale = 0.6902

132# 8.00"

8-0-0

5

6

4

2-0-0

3x4

4x6

2x4

3

4-0-0

2

1

4-0-0

6.00

-6.00

2-0-0

WO: AUSM4

TI: V15

8/19/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FLA

V4.7.32-65761

DESIGN INFORMATION

This design is for an individual building component and the trust designer is not responsible for the overall building. The trust designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness and accuracy of this information as it may relate to a specific project and accepts no responsibility for any excessive cost or control with regard to fabrication, handling, shipment and installation of trusses.

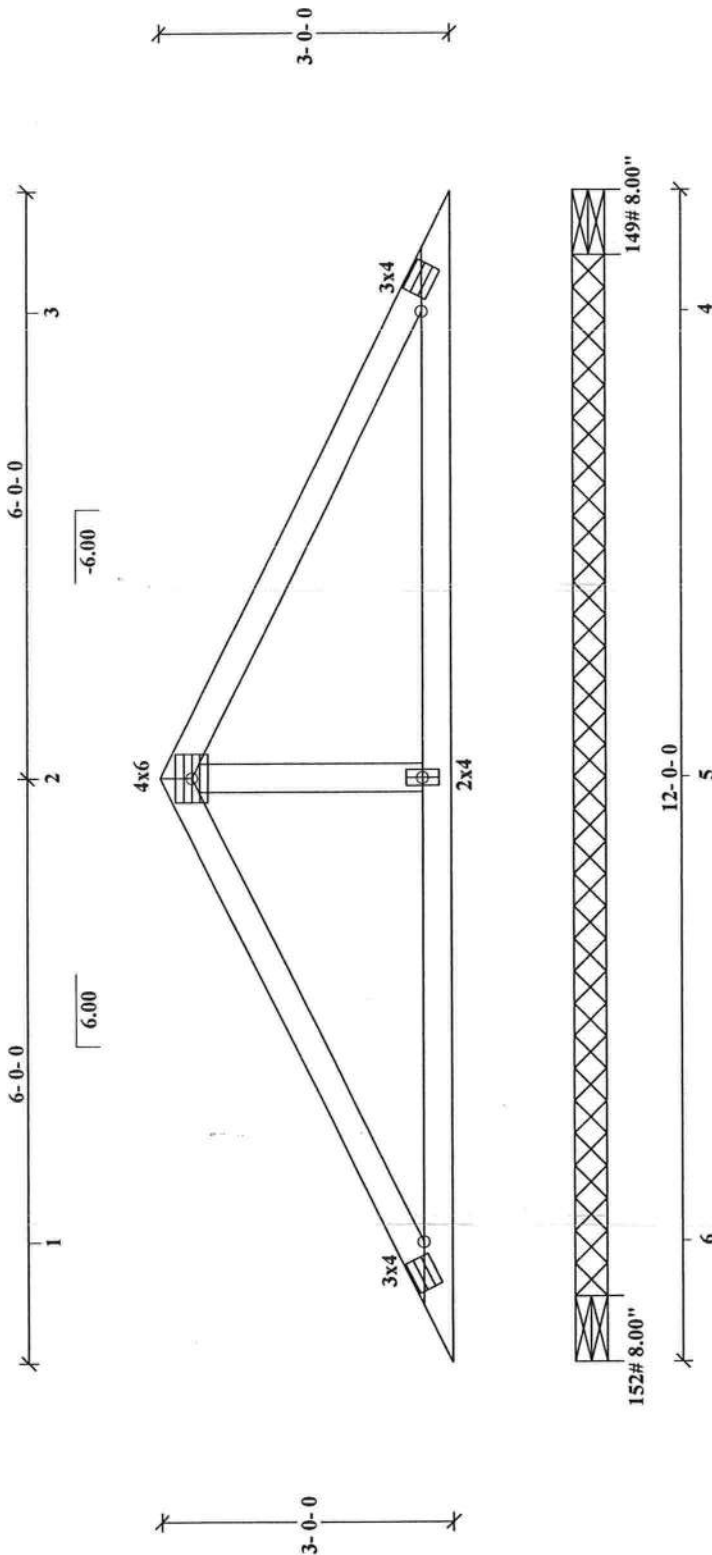
This trust has been designed as an individual building component in accordance with AS/NZS 4608:1998 and is not intended to be incorporated into the design of a building. The Building Designer, who has been used for approval by the building designer, the design loadings shown must be checked to be sure that the data shown is in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special structural loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously connected by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10m. The design is based on the assumption that the truss is fully braced in accordance with AS/NZS 4608:1998. Truss is not to be used as a beam.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to verify a continuing responsibility for each realization. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over toolholes, knots or distorted members. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the joint with male fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Double cut on joint parallel to the plate length specified. Sloste cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum values based on the forces shown and may need to be increased for certain handling and/or erection stresses. This note is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than those specified shall be avoided. No bracing other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.



EXCEPT AS SHOWN PLATES ARE MiTek MT20

Scale = 0.5037

WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oonofrio Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932	Eng Job:	WO: AUSM4	
	Dwg:	TI: V16	
	Dsgnr: TLY	Chk: 8/19/2006	
	TC Live	16.0 psf	Lbr DF: 1.25
	TC Dead	7.0 psf	Plt DF: 1.25
BC Live	10.0 psf	O.C.: 2- 0- 0	
BC Dead	10.0 psf	TPI-02/FBC-04	
TOTAL	43.0 psf	Code: FLA	
		v4.7.32-65769	

DESIGN INFORMATION

TC: 2x 4 SP #2
BC: 2x 4 SP #2
GB: 2x 4 SP #3

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.

WindLoad per ASCE 7-02, C50, V= 125mph,
H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0
BlD Type= encl I= 16.0 ft W= 16.0 ft Truss
in END zone, TCDF= 0.0 psf, BCDL= 0.0 psf
Impact Resistant Covering and/or Glazing Req

Analysis based on Simplified Analog Model.
ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
Wind loads have been checked perpendicular to the face of the truss.
This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.
-----MAX. REACTIONS PER BEARING LOCATION-----
X-Loc BSst Vert Horiz Uplift Y Ty
0-4-0 0 1 139 103 -94 B
15-8-0 0 1 139 -103 -94 B
PROVIDE UPLIFT CONNECTION PER SCHEDULE

Joint Locations	
1) 0 - 0 - 0	7) 10 - 0 - 0
2) 4 - 0 - 0	8) 12 - 0 - 0
3) 6 - 0 - 0	9) 16 - 0 - 0
4) 6 - 4 - 14	10) 16 - 0 - 0
5) 8 - 0 - 0	11) 12 - 0 - 0
6) 9 - 7 - 2	12) 10 - 0 - 0

In-Plant Quality Assurance with $Cq = 1.00$

I Span	in./Ratio	in./Ratio	TL	Jnt(s).
Horiz.	-0.01/999	-0.01/999	0.00	2-1
			0.00	NA
				Long term deflection factor = 1.50

X-Loc	BSet	Vert	Horiz	Uplift	Y	Type
0- 4- 0	1	139	103	-94	B	Pin
15- 8- 0	1	139	-103	-94	B	Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Impact Resistant Covering and/or Glazing Req

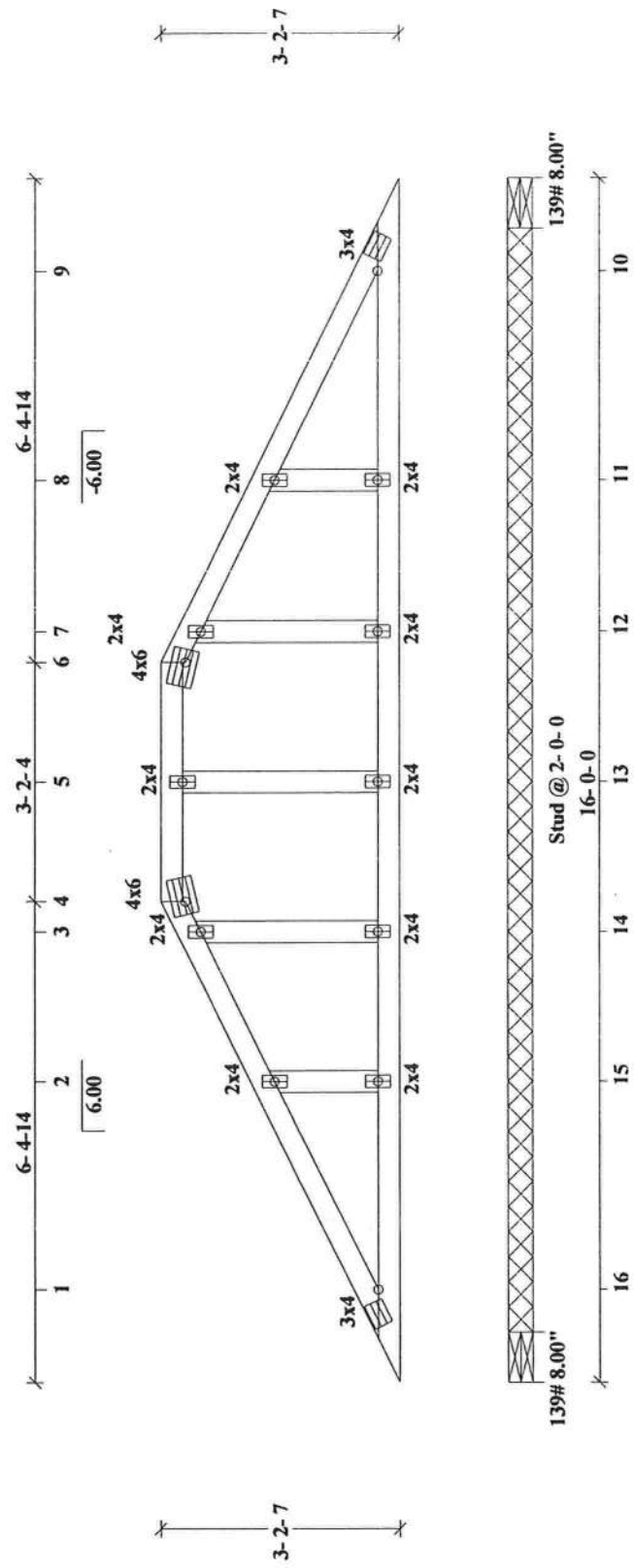
wind or snow loads, project specifications or special stipulated loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a sheathing or other bracing system, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40. Unless otherwise shown,

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing obligation to verify and report any discrepancies to the architect in writing before cutting or fabrication. Plates shall not be installed over knuckles, knots or distorted members. Members shall be cut for tight fitting to wood framing. Connector plates shall be located on both sides of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x6 plate is 6" wide x 8" long. Slats (holes) shall run parallel to the plate length specified. Double cut on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the force shown and may need to be increased for certain handling and/or erection stresses. Fabricator shall verify that all members are properly treated with preservative unless otherwise shown. For additional information on Quality Control refer to ANSII/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with applicable industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful lifting, hoisting and moving of trusses is required. Normal handling and erection of trusses requires such temporary bracing during installation between trusses to avoid toppling and jamming. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.



EXCEPT AS SHOWN PLATES ARE MiTek MT20

Scale = 0.3988

Maronda Systems
4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #00050068
367 Medallion Pl. Chuluota, FL 32766

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Component Engineering by Truss Engineering Co., P.A., 818 Southside Rd. Edenburg, NC 27932

Engineering Job:	WO: AUSM4
Dwg:	TI: V17
Issuedn: TLY Chk:	6/15/2007
TRC Live	Lbr DF: 1.1 psf
TRC Dead	Plt DF: 1.1 psf
BGC Live	O.C.: 2-0 psf
BGC Dead	FBC-04/TPI
TOTAL	Code: FLA
	v5.5.6-816

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\AUSM4\AUSTIN M 4\VI7.prx

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DESIGN INFORMATION

This design is for an individual building component and has been prepared for information only for the client. The designer disclaims any responsibility for the design as a result of failure or incorrect information, specifications and/or designs furnished to the client by the client and/or the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

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PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and action for bracing is always required. Normal precautionary installation below requires such temporary bracing during erection. The supervisor of erection and the truss shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads at any time No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed

Customer:

Name: AUSTIN M 4

TC: 2x 4 SP #2
BC: 2x 4 SP #2
GB: 2x 4 SP #3

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.
WindLoad per ASCE 7-02, Cat. B, V=125mph,
E=15.0 ft, I=1.00, Exp. Cat. B, Kzt=1.0
Bld Type= encl L= 28.0 ft W= 27.5 ft Truss
in END zone, TCDL= 0.0 psf, BCDL= 0.0 psf
Impact Resistant Covering and/or Glazing Req
-----GLOBAL MAX DEFLECTIONS-----
LL in./Ratio in./Ratio Unt(s).
I. Span 0.00/999 -0.01/999 2-1
Horiz. 0.00 0.01 NA
Long term deflection factor = 1.50

Analysis based on Simplified Analog Model.
ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
Wind loads have been checked perpendicular to the face of the truss.
This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.
In-Plant Quality Assurance with Cg = 1.00
-----MAX. REACTIONS PER BEARING LOCATION-----
X-Loc Bset Vert Horiz Uplift Y Type
0- 4- 0 1 126 129 -69 B Pin
27- 2- 0 1 64 -135 -7 B Pin
PROVIDE UPLIFT CONNECTION PER SCHEDULE

Customer: TI: V20

WO: AUSM4

Qty: 1

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126# 8.00"

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64# 8.00"

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6-1-14

126# 8.00"

27-6-0

64# 8.00"

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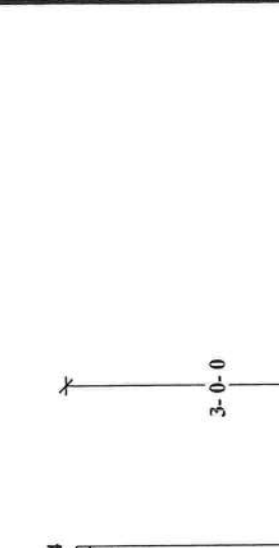
26</

Joint Locations					
1)	0-7-0	3)	6-0-12	5)	4-0-12
2)	3-11-15	4)	6-0-12	6)	0-7-0

In-Plant Quality Assurance with Cq= 1

-----MAX. REACTIONS PER BEARING LOCATION-----

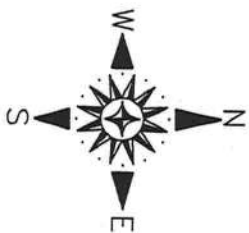
Bld Type= encl L= 60.0 ft W= 6.2 ft Truss
in INT zone, TCUL= 4.2 psf, BCDU= 1.2 psf
Impact Resistant Covering and/or Glazing Re-
...TC...FORCE.CSI. Cr ..EC...FORCE.CSI.
1- 2 -156 0.11 A 0- 1 -186 0.09
2- 3 -33 0.11 A 1- 5 26 0.08
5- 4 51 0.08



Scale = 0.5275

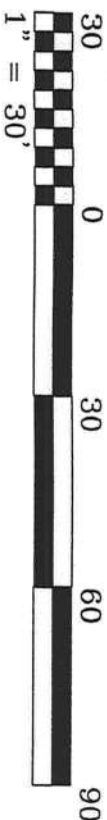
Eng Job:	WO: AUSM4		
Dwg:	TI: V9		
Dsgnr: TLY	Chk:	8/19/2006	
TC Live	16.0	psf	Lbr DF: 1.25
TC Dead	7.0	psf	Plt DF: 1.25
BC Live	10.0	psf	O.C.: 2- 0- 0
BC Dead	2.0	psf	TPI-02/FBC-04
TOTAL	35.0	psf	Code: FLA
			v4.7.32-36250

LEGAL DESCRIPTION:
LOT FIVE (5) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK "D", PAGE 2 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



CERTIFIED TO:

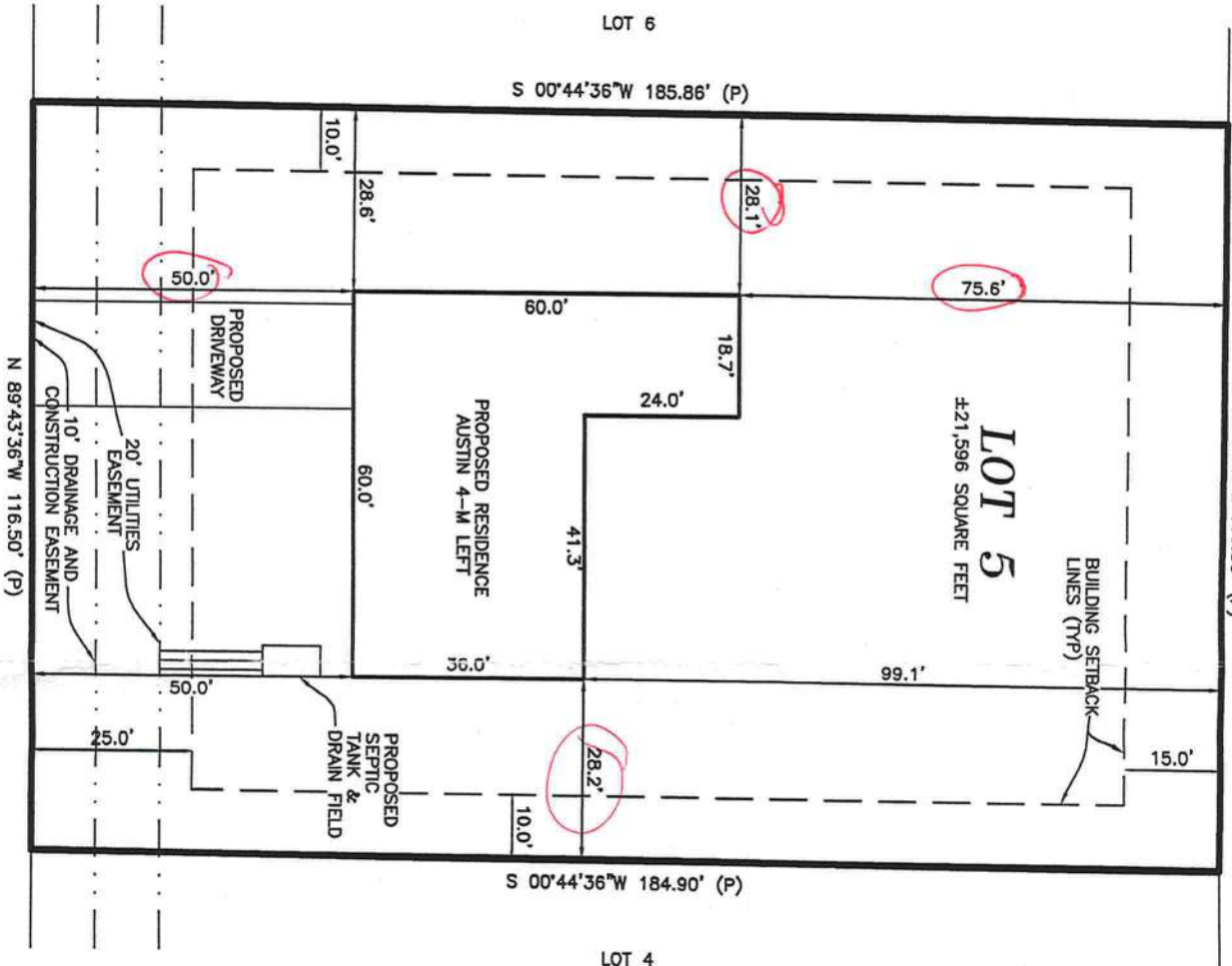
1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.



SW TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
- = FOUND 1/2" REBAR & CAP L.B. 6894
- = SET 1/2" REBAR & CAP L.B. 6894
- = FOUND 3/4" IRON PIPE
- = FOUND 4" X 4" CONC. MON. NO IDENTIFICATION
- = SET 4" X 4" CONC. MON. P.S.M. 5582
- ✕ = SET NAIL & DISK P.S.M. 5582
- ✕ = FOUND NAIL & DISK
- ⊠ = FOUND 6" X 6" S.R.D. R/W MON.
- ⊞ = CATV RISER
- ⊞ = TELEPHONE PEDESTAL
- ⊞ = WOOD POWER POLE

ABBREVIATIONS:

- A = ARC LENGTH
- A/C = AIR CONDITIONER
- ALUM = ALUMINUM
- ASPH = ASPHALT
- AVE = AVENUE
- BLK = BLOCK
- BLVD = BOULEVARD
- BM = BENCHMARK
- C = CALCULATED FROM MEASURED
- CATV = CABLE TELEVISION
- C/B = CONCRETE BLOCK
- CL = CENTER LINE
- CLF = CHAIN LINK FENCE
- CM = CONCRETE MONUMENT
- CMP = CORRUGATED METAL PIPE
- CONC = CONCRETE
- DOT = DEPARTMENT OF TRANSPORTATION
- ELEC = ELECTRIC
- ELEV = ELEVATION
- FND = FOUND
- FNC = FENCE
- (L) = PER LEGAL DESCRIPTION
- LB = LICENSED SURVEYOR BUSINESS
- (M) = FIELD MEASURED
- MH = MANHOLE
- P = PLAT
- PB = PLAT BOOK
- P.U.E. = PUBLIC UTILITIES EASEMENT
- R/W = RIGHT OF WAY

THIS IS NOT A BOUNDARY SURVEY CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE.

BY *James E. Brinkman*
JAMES E. BRINKMAN, PSY - FLA. CERT# 5582

DATE: 10/31/07

BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL. 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

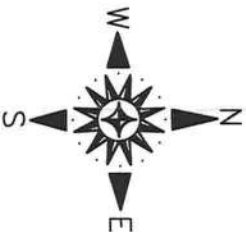
SCALE: 1" = 30' DRAWN BY: ZL

DATE: 10/15/07 "THE BENCHMARK IN QUALITY SERVICE" CHECKED BY: J.B.

THIS IS NOT A BOUNDARY SURVEY PREPARED FOR: MARONDA HOMES DRAWING NUMBER 301-07

LEGAL DESCRIPTION:

LOT FIVE (5) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK "D", PAGE 2 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



FNC END IS 0.4'S & 0.1'W OF PROPERTY CORNER

CERTIFIED TO:

1) MARONDA HOMES

FENCE NOTE:

SOME FENCE TIES AS SHOWN HEREON, (IF APPLICABLE) HAVE BEEN DRAWN EXAGGERATED FOR SAKE OF GRAPHIC REPRESENTATION.

BEARING NOTE:

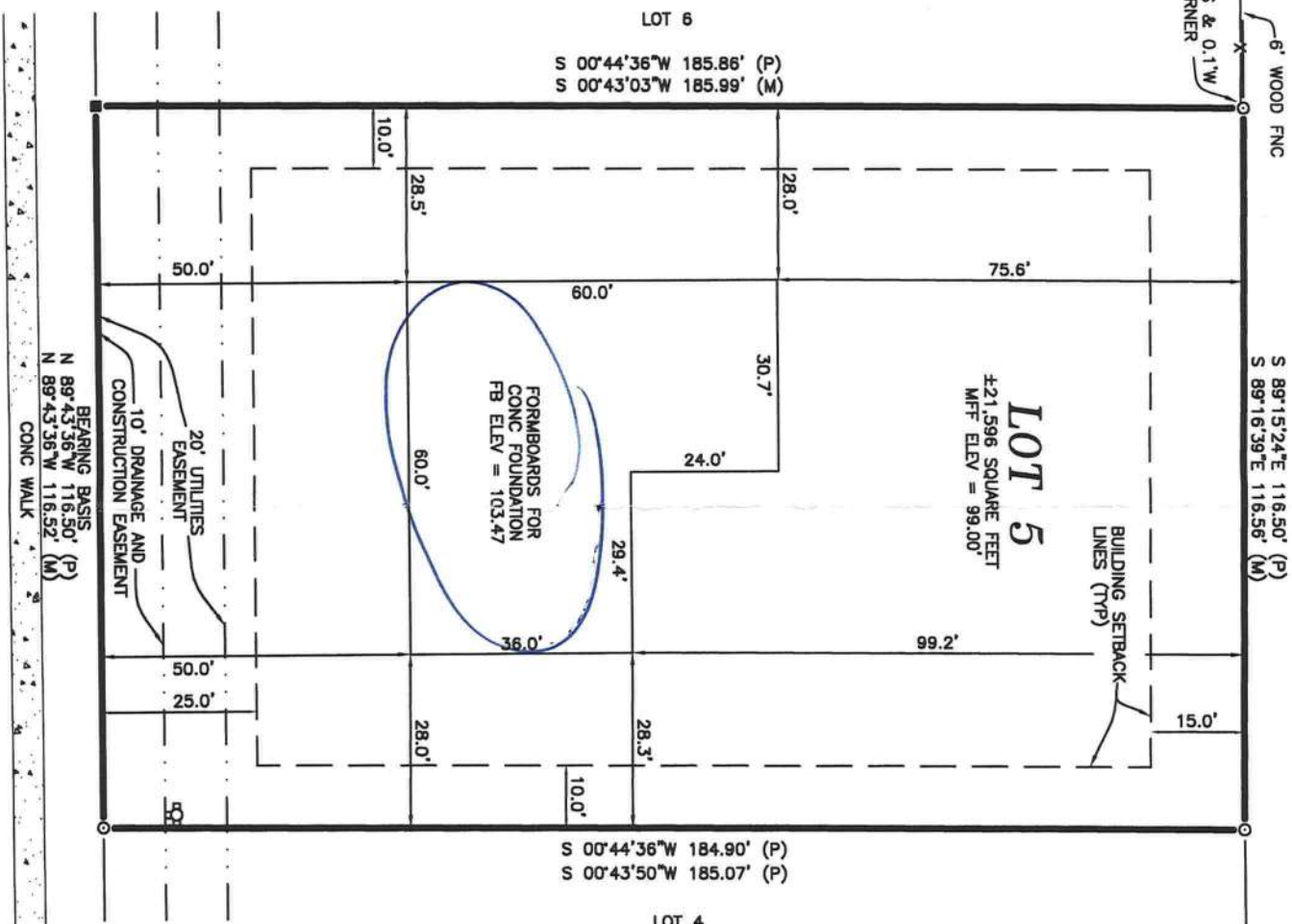
BEARINGS SHOWN HEREON ARE REFERRED TO AN ASSUMED VALUE OF N. 89°43'36" W., FOR THE NORTH RIGHT-OF-WAY OF S.W. TIMBER RIDGE DRIVE.

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.



FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-8-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

EDGE OF PAVEMENT SW TIMBER RIDGE DRIVE 60' RIGHT-OF-WAY ±20' ASPHALT ROAD

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

BOUNDARY SURVEY

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



26623

LEGEND:

- = FOUND 1/2" REBAR & CAP
- LB. 6894
- = FOUND 4" X 4" CONC. MON.
- = NO IDENTIFICATION
- = FIRE HYDRANT
- = TELEPHONE PEDESTAL
- = WOOD POWER POLE

ABBREVIATIONS:

- A = ARC LENGTH
- A/C = AIR CONDITIONER
- ALUM = ALUMINUM
- ASPH = ASPHALT
- AVE = AVENUE
- BLK = BLOCK
- BLVD = BOULEVARD
- BM = BENCHMARK
- C = CALCULATED FROM MEASURED
- CATV = CABLE TELEVISION
- C/B = CONCRETE BLOCK
- CL = CENTER LINE
- CLF = CHAIN LINK FENCE
- CM = CONCRETE MONUMENT
- CMP = CORRUGATED METAL PIPE
- CONC = CONCRETE
- DOT = DEPARTMENT OF TRANSPORTATION
- ELEC = ELECTRIC
- ELEV = ELEVATION
- FB = FORMBORD
- FND = FOUND
- FNC = FENCE
- (L) = PER LEGAL DESCRIPTION
- LB = LICENSED SURVEYOR BUSINESS
- (M) = FIELD MEASURED
- MFT = MINIMUM FINISHED FLOOR
- MH = MANHOLE
- P = PLAT
- PB = PLAT BOOK
- P.U.E. = PUBLIC UTILITIES EASEMENT
- R/W = RIGHT OF WAY

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 494.06, FLORIDA ADMINISTRATIVE CODE.

BY: JAMES E. BRINKMAN, PSM, F.L.A. CERT# 5582
DATE: 2/24/08

BRINKMAN SURVEYING & MAPPING INC.



4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

"THE BENCHMARK IN QUALITY SERVICE"

DRAWN BY: ZL

DATE: 2/22/08

CHECKED BY: J.B.

FIELDWORK COMPLETED 2/15/08

PAGES 71, 75

PREPARED FOR: MARONDA HOMES

DRAWING NUMBER 301-07

**HOMETEAM****PEST DEFENSE®****TREATMENT WORKORDER**☐ Termite Baiting System w/Tubes-under-the slab☐ Treat Only☒ Tubes-under-the slab and Treat☐ Bora-Care

DATE CALLED IN:	2-20	DATE OF SCHEDULE:	2-21
TIME CALLED IN:		TIME SCHEDULE:	7:30

JOB NAME:	MARONDA	SUBDIVISION:	Timberland
JOB ADDRESS:	209 SW Timber Ridge Dr Lake City, FL		
BILLING NAME:		BILLING PHONE:	
BILLING ADDRESS:	26623		
CALLER BY:	Patrick	PHONE:	352-206-5459
		PERMIT NUMBER:	

LOT & MODEL NUMBER: Lot 5

DATE & TIME COMPLETED: 2-21-08

SQUARE FOOT: 3061 LINEAR FOOT: BLOCKVOIDS:

SLAB TYPE: Mono TYPE OF FILL: Dirt

APPROX. DEPTH OF FOOTING: Outside: 20 Inside: 0

☐ Addition ☐ Spot Treat ☐ Pool Addition ☐ Driveway☐ Final/Completion ☐ Other

PESTICIDE USED: Probuld TC TOTAL APPLIED: 165 gals of mix

PERCENT (%) USED: 65% STICKER POSTED: Permit Box

PRICE PER SQ. FT. =	TOTAL FOR P.T.		
	ADDITIONAL		
	TAX:		
	TOTAL AMOUNT	\$	

X

X TECHNICIAN:

I hereby acknowledge the satisfactory completion of the above described work.