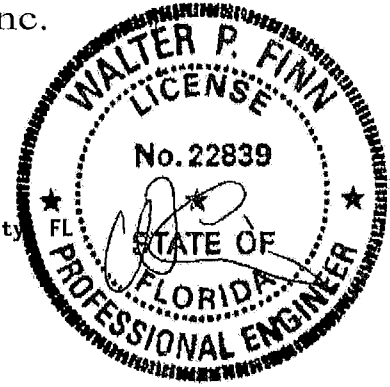


ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID IVIJ487-Z0122093524



11/22/2013

Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

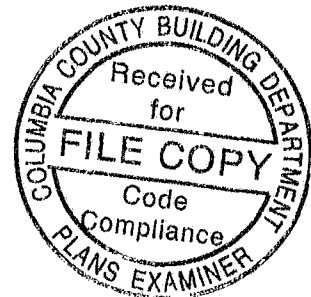
Truss Fabricator **Anderson Truss Company**
Job Identification **13-281--BRYAN ZECHER /Columbia County Builders -- Lake City**
Truss Count **36**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Versions 12.03, 13.01.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61615-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: 14015EC1-GBLLETIN-BRCLBSUB-PB16010-

#	Ref	Description	Drawing#	Date
1	28444-A	38'9"8 Stepdow	13326001	11/22/13
2	28445-A1	38'9"8 Stepdo	13326002	11/22/13
3	28446-B	19'7"8 Common	13326003	11/22/13
4	28447-B1	19'7"8 Common	13326004	11/22/13
5	28448-BG1	11'7" Common	13326035	11/22/13
6	28449-BGE	19'7"8 Gable	13326022	11/22/13
7	28450-CJ1	1' Jack	13326023	11/22/13
8	28451-CJ3	3' End Jack	13326024	11/22/13
9	28452-CJ5	5' Jack	13326025	11/22/13
10	28453-EJ3	3' End Jack	13326026	11/22/13
11	28454-EJ7	7' End Jack	13326027	11/22/13
12	28455-EJ7A	7' End Jack	13326005	11/22/13
13	28456-EJ7B	7' End Jack	13326006	11/22/13
14	28457-EJ7B	7' End Jack	13326007	11/22/13
15	28458-EJ7C	7' End Jack	13326008	11/22/13
16	28459-H11	38'9"8 Stepd	13326009	11/22/13
17	28460-H11A	26'4" Stepd	13326010	11/22/13
18	28461-H13	38'9"8 Stepd	13326011	11/22/13
19	28462-H13A	26'4" Stepd	13326012	11/22/13
20	28463-H15	38'9"8 Stepd	13326013	11/22/13
21	28464-H15A	26'4" Stepd	13326014	11/22/13
22	28465-H17	38'9"8 Stepd	13326015	11/22/13
23	28466-H17A	26'4" Stepd	13326016	11/22/13
24	28467-H19	38'9"8 Stepd	13326017	11/22/13
25	28468-H19A	26'4" Stepd	13326018	11/22/13
26	28469-H3	11'7" Stepdow	13326019	11/22/13
27	28470-H5	11'7" Stepdow	13326020	11/22/13
28	28471-H7	38'9"8 Stepdo	13326021	11/22/13
29	28472-H7A	26'4" Stepdo	13326036	11/22/13
30	28473-H9	38'9"8 Stepdo	13326028	11/22/13
31	28474-H9A	26'4" Stepdo	13326029	11/22/13
32	28475-HJ3	4'2"15 Hip J	13326030	11/22/13
33	28476-HJ7	9'10"13 Hip	13326031	11/22/13
34	28477-HJ7A	9'10"13 Hip	13326032	11/22/13
35	28478-PBA	4'9"6 Common	13326033	11/22/13
36	28479-PBA1	4'9"6 Stepd	13326034	11/22/13



ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837

Page 1 of 1 Document ID 1V1J487-Z0122093524

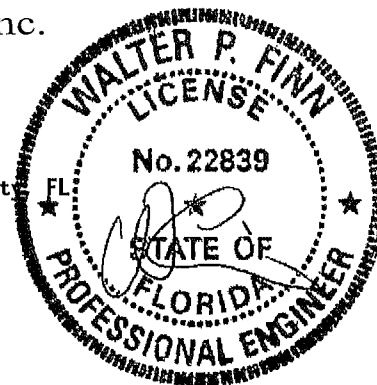
Truss Fabricator **Anderson Truss Company**
Job Identification **13-281--BRYAN ZECHER /Columbia County Builders -- Lake City FL**
Truss Count **1**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Versions 12.03, 13.01.**
Structural Engineer of Record
Address
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Revised Trusses

#	Ref	Description	Drawing#	Date
1	28472-H7A	26'4" Stepdo	13326036	11/22/13



11/22/2013

-Truss Design Engineer-
Walter P Finn

1950 Marley Drive
Haines City, FL 33844

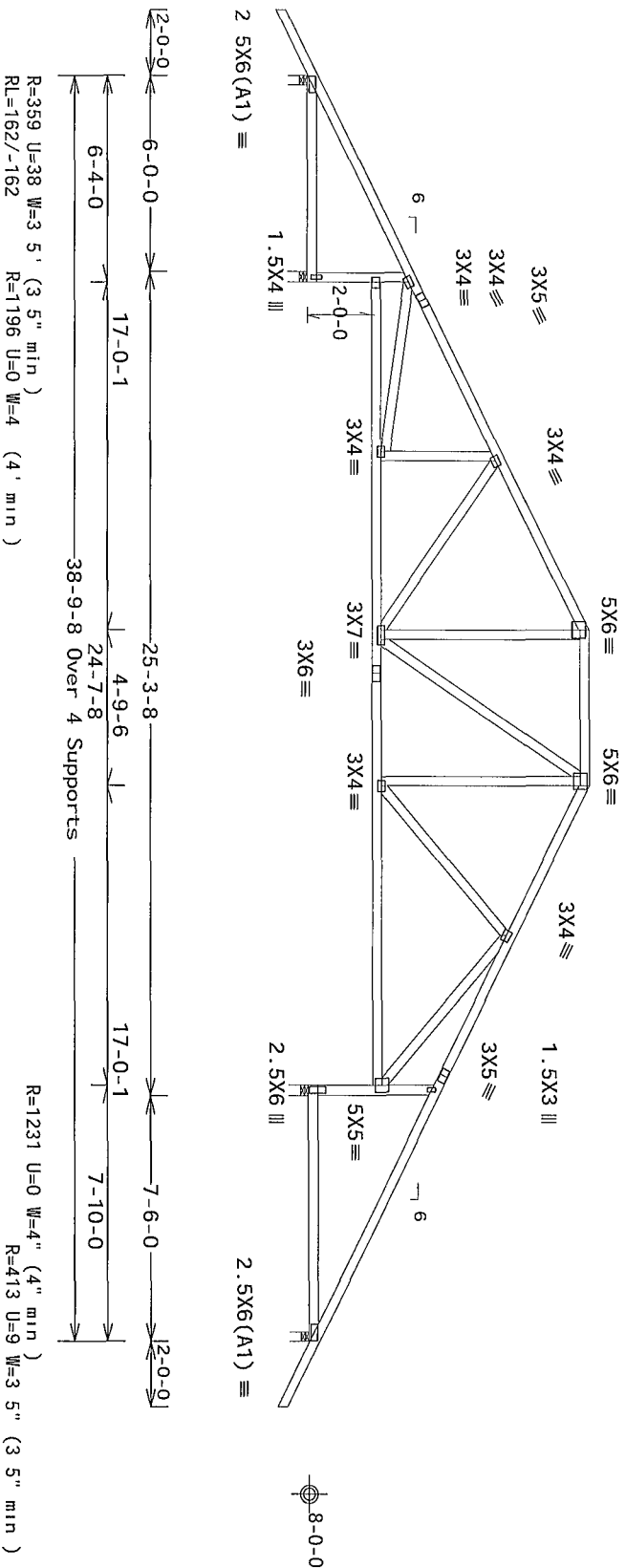
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE /-10', CLOSED bldg, not located
within 9 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf,
wind BC DL=5 0 psf GCP1 (+/-)=0 18

Wind loads and reactions based on MWFRS with additional CXC member design

Deflection meets 1/240 live and 1/180 total load

Factor for dead load is 1.30



Scale = .1875"/Ft.

TC LL	20 0 PSF	REF	R9114- 28444
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HGUSR9114 13326001
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN-	330910
DUR.FAC.	1.25		
SPACING	24 0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

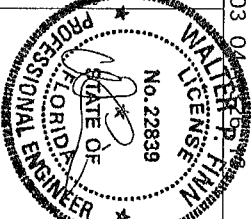
Bottom chord checked for 10 00 psf non-concurrent live load

factor for dead load is 1.50



Scale = 1875"/Ft.

ITW Building Components Group Inc.

[illegible]

1C LL	20 0 PSF	REF	R9114- 28445
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HCUSR9114 13326002
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN-	330912
DUR FAC	1 25		
SPACING	24.0"	JREF-	1V1J487_Z01

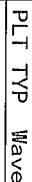
JREF- 1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Factor	Creep increase
Deflection meets L/240 live and L/180 total load	1.50
Factor for dead load is 1.50	


$$\text{FT/RT} = 10\%(0\%) / 0(0)$$

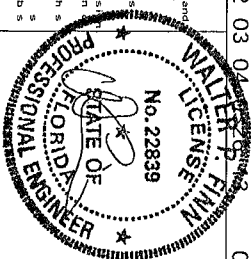
QTY 2 FL/-/5/-/-/R/-

Scale = 3125"/Ft

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

***** IMPORTANT *****
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING THE SUBS
Trustees, need to examine care in their contracting and not to follow the latest edition of BS51 (Build up Components Safety Information on by TPI and WTCO) for safety practice sets per or to performing these functions. Installers shall provide temporary bracing per BS51. Unbraced overhead shall have properly attached structural lateral ring and bottom chord shall have bracing provided per BS51. Bracing shall be of B10 or B10 as apply cable
17th Buld up Components Group Inc. (LITBROG) shall not be responsible for any design or from this design any failure to build the trusses in conformance with the ANSI/TR 11.1.5. Safety shall be the responsibility of the contractor. Apply plates to each face of truss and position as shown above and on the Joist. Details unbraced overhead shall Refer to drawings 100A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing indicates acceptance of professional engineering review. The responsibility to verify the Buld up design per ANSI/TR 11.1.5 Sec 2 for more information on this subject. The sub's general notes page 17th BCG www.litbrog.com TPI www.tpi.net.org WTCO www.sbc-industry.org
CDC www.cdc.org



TC LL	20 0 PSF	REF R9114- 28446
TC DL	7.0 PSF	DATE 11/22/13
BC DL	10 0 PSF	DRW H0USR9114 13326003
BC LL	0 0 PSF	HC-ENG AP/AP
TOT LD	37 0 PSF	SEQN- 339741
DUR_FAC	1 25	
SPACING	24.0"	JREF- 1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City FL - B1 19 7'8 Common)

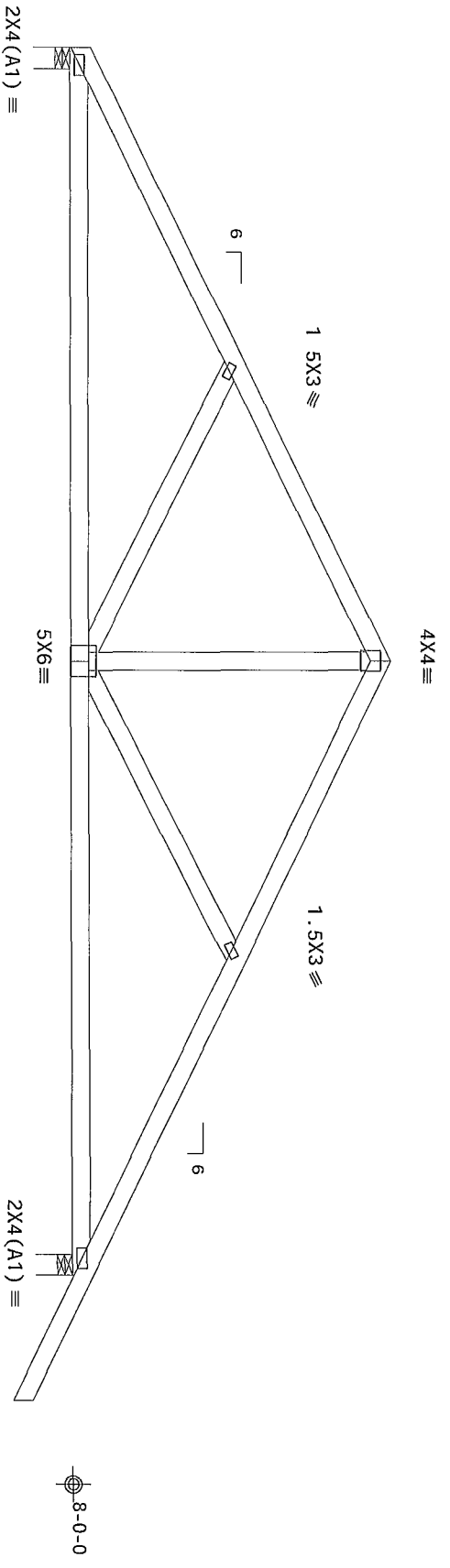
Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

MMFRS loads based on trusses located at least 7 50 ft from roof edge

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18
Wind loads and reactions based on MMFRS with additional C&C member design
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



9-9-12 19-7-8 Over 2 Supports 9-9-12 2-0-0
R=735 U=57 W=4 (4 min)
RL=106/-97
R=870 U=84 W=4" (4" min)

PLT TYP Wave Design Crit FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0) 12.03 04 2013 QTY 8 FL/-/5/-/-/R/- Scale = 375"/Ft

ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278

WALTER P. FINN
No. 22839
STATE OF FLORIDA
PROFESSIONAL ENGINEER
11/22/2013

TC LL	20 0 PSF	REF	R9114- 28447
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HCSR9114 13326004
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT. LD	37 0 PSF	SEQN-	339608
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Webs 2x4 SP #3-13B W2 2x4 SP #1-13B

Special loads	Dur Fac = 1.25 /	Plate	Dur Fac = 1.25)
-----Lumber			
TC- From	56 pif at -2.00 to	56 pif at 5.79	
TC- From	56 pif at 5.79 to	56 pif at 13.58	
BC- From	4 pif at -2.00 to	4 pif at 0.00	
BC- From	20 pif at 0.00 to	20 pif at 7.06	
DC- From	10 pif at 7.06 to	10 pif at 11.58	

	Dur	Fac = 1.25	/	Plate	Dur	Fac = 1.25
-----Lunmer						
TC-From	56	pif at -2.00	to	56	pif at 5.79	
TC-From	56	pif at 5.79	to	56	pif at 13.58	
BC-From	4	pif at -2.00	to	4	pif at 0.00	
BC-From	20	pif at 0.00	to	20	pif at 7.06	
BC-From	10	pif at 7.06	to	10	pif at 11.58	
BC-From	4	pif at 11.58	to	4	pif at 13.58	
BC-2873 16	1b	Conc Load at 7.06				
BC-1462 97	1b	Conc Load at 9.06				
BC-1456 92	1b	Conc Load at 11.06				

Bottom chord checked for 10 00 psf non-concurrent live load

load



R=4845 U=249 W=4 (4' min)

Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

QTY 1

FL/-/5/-/-/R/-

Scale = .5"/Ft

ALLEN P. F.

TC LL	20 0 F
-------	--------

REF R9114- 28448

W. J. CENSE

TC DL	7.0 F
-------	-------

DATE 11/22/13

No. 22839

BC DL	10.0 F
-------	--------

DRW HCUSR9114 1332603

BC II 00 F

HC-FNG AP/AP

STATE OF

TOT ID	37 0
--------	------

SEEN- 324379

OFFICE OF THE
FLORIDA
COMMISSIONER OF
REVENUE

101 ED	01:01
DIP EAC	1 25

0704070

PERSONAL EN

DOCK: 1 AC.	1:20
CDAC INC	24 00"

INDF 4114 1407 704

11/2

SPACING 24.0

REF - IV1J481_201

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Webs 2x4 SP #3-13B

1

130 mph wind 15 00 ft mean hgt., ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, Exp B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCPI(+/-)=0.18

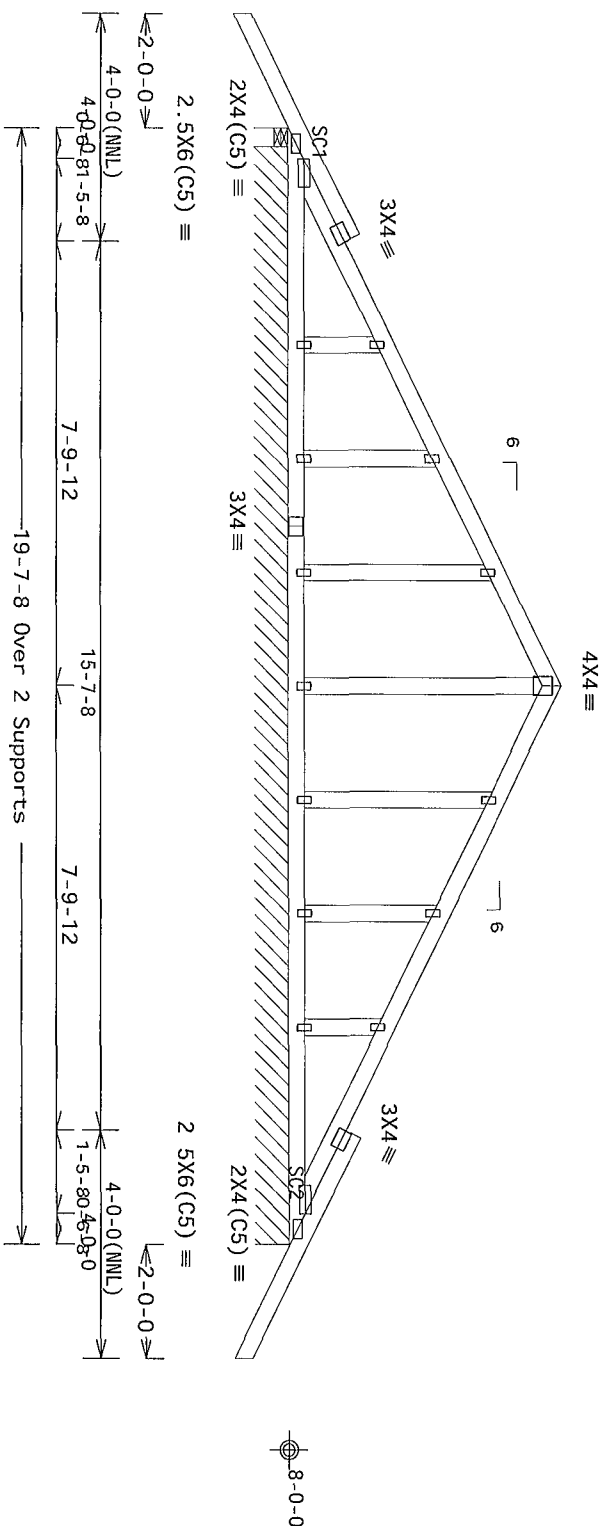
Wind loads and reactions based on MMFRS with additional C&C member design

Truss spaced at 24 0" OC designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase 1/4" @ 1.50



FILE 20621-2B6F W=19-3-8

Design Crit.: FBC2010Res/TP1-2007(STD)

$$FT/RT = 10\%(0\%)/0(0)$$

12 03.04.2013

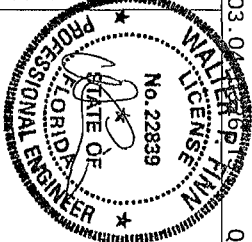
QTY:1 FL/-/5/-/-/R/-

Scale = .3125"/Ft.

****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

11/22/2013

TC LL	20 0 PSF	REF	R9114- 28449
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326022
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT.LD.	37.0 PSF	SEQN-	339601
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

(13-281--BRYAN ZECHEER /Columbia County Builders -- Lake City FL - C/J 1 Jack)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

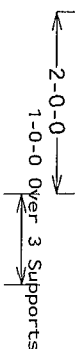
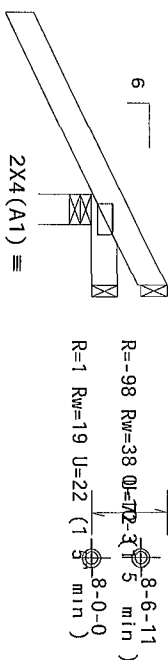
Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf 6Cp1(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member
design

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



R=326 U=75 W=4 (4" min)
RL=29/-24

PLT TYP Wave

Design Crit: FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.00

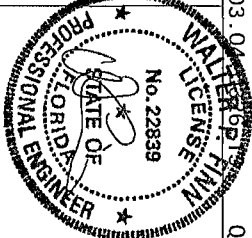
QTY: 8 FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

Ohlando FL 32837
FL COA #0278

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and
follow the latest edition of BCSI (Building Components Safety) Information by TPI and WFOA for safety
practices prior to performing these functions. Installers shall provide temporary bracing per BCSI
unless noted otherwise. The top chord shall have properly attached structural sheathing and bottom chord
shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs
shall have bracing installed per BCSI sections 83, 87 or 810 as applicable.
TPI Building Components Group Inc. (TPI/BCG) shall not be responsible for any deviation from this design
due to any failure to build the trusses in conformance with ANSI/TP1 1 or for handling, shipping, installation &
bracing of the trusses. The user of this design assumes all responsibility for the safety of the structure.
Details of connections shall be in accordance with the latest edition of the BCSI Standard Detail
drawing or cover page listing this drawing. The user shall verify and use of this design for any structure is
the responsibility of the building designer per ANSI/TP1 1 Sec 2. For more information see this job's
general notes page TPI-BCSI www.bcsi.com tpi www.printing.org WFOA www.structure.com
100 www.structure.org



11/22/2013

TC LL	20.0 PSF	REF	R9114- 28450
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326023
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT.LD	37.0 PSF	SEQN-	339603
DUR.FAC.	1 25		
SPACING	24.0"	JREF-	1V1J487_Z01

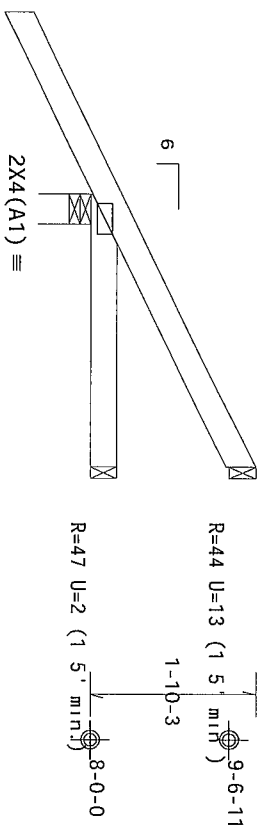
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf 6cpi(+/-)=0 18

anywhere in roof, risk at
DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



R=288 U=35 W=4' (4' min)
RL=48/-28

PLT TYP. Wave

Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

12.03.04 0326 13

QTY:4 FL/-/5/-/-/R/-

Scale = .5"/Ft.

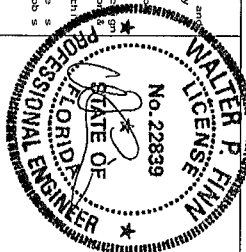
ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Tensile require care in fabricating, handling, and splicing. Insulating and bonding shall be done in accordance with the manufacturer's instructions. Refer to the latest edition of BCSI (Building Component Safety) Information on TPI and WIDA for safety practice used prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise as top chord shall have properly attached structural sheath and bottom chord shall have a properly attached radial ceiling. Locate one stimo for permanent lateral restraint or wheel shall have one installed per BCSI sections 83, 87 or 810 as appli cable

[illegible]

11/22/2013

TC LL	20 0 PSF	REF	R9114- 28451
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326024
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT. LD.	37 0 PSF	SEQN-	339703
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City, FL - CJS 5 Jack)

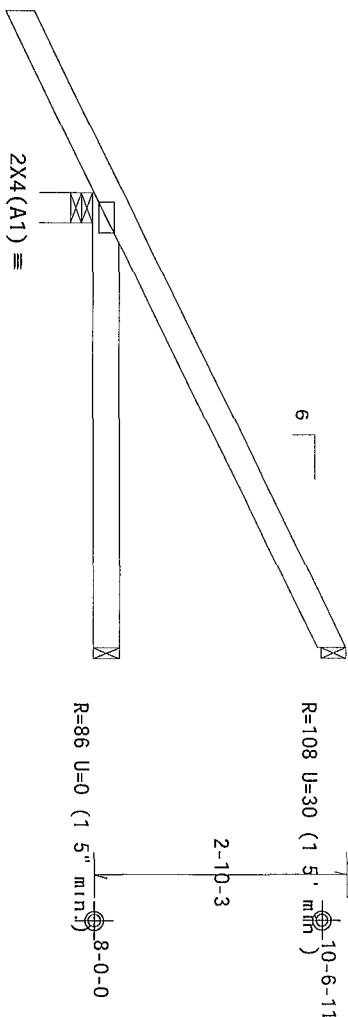
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with '13B" use design values approved
1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf,
wind BC DL=5 0 psf GCPI (+/-)=0 18
Wind loads and reactions based on MMFRS with additional C&C member
design
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



2-0-0
5-0-0 Over 3 Supports
R=343 U=32 W=4 (4" min)
RL=67/-32

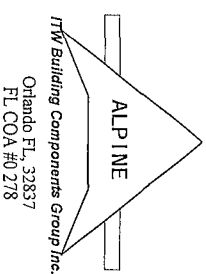
PLT TYP Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

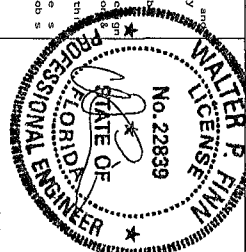
12.03.04 02:26:13

QTY:4 FL/-/5/-/-/R/-

Scale =.5"/Ft.



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety) Information by TPI and WTC for safety practices or to perform these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI section 83, 87 or 810 as applicable.
RTW Building Components Group Inc. (RTWBCG) shall not be responsible for any device or from this design any failure to build the truss in conformance with ANSI/TP1-1 or for handling, shipping, installing, bracing, or any other action taken by the contractor. RTWBCG shall not be responsible for any failure to build the truss in conformance with ANSI/TP1-1 or for handling, shipping, installing, bracing, or any other action taken by the contractor. Refer to drawing 1604-2 for standard practice. A seal on this drawing or cover page listing this drawing and the acceptance of professional engineering and the responsibility of the Building Designer per ANSI/TP1-1 Section 2. For more information see the general notes page TPI-2007 www.tpiinc.org WTC www.sbcindustry.com This Job is 100% www.tpiinc.org



TC LL	20.0 PSF	REF	R9114- 28452
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	H05R9114 13326025
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT.LD.	37.0 PSF	SEQN-	339691
DUR.FAC.	1 25		
SPACING	24.0"	JREF	1V1J487_Z01

11/22/2013

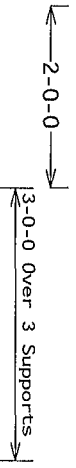
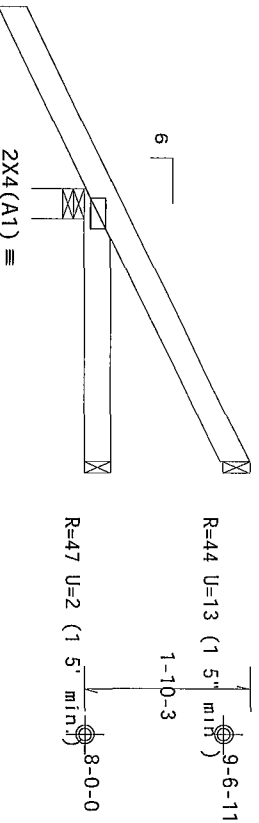
(13-281--BRYAN ZECHEER /Columbia County Builders -- Lake City FL - E3 3 End Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf 6Cpi(+/-)=0 18
Wind loads and reactions based on MWFRS with additional C&C member
design
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



R=288 U=35 W=4" (4' min)
RL=48/-28

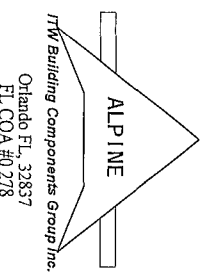
PLT TYP Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

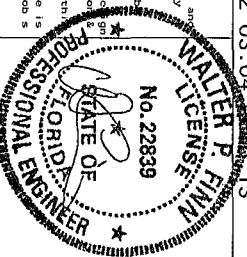
12 03 04 0000 13

QTY: 4 FL/-/5/-/-/R/-

Scale = .5"/Ft.



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and
follow the latest edition of BCSI (Building Component Safety) Information by TPI and WTC for safety
practices prior to performing these functions. Installers shall provide temporary bracing per BCSI
unless noted otherwise. See top chord shall have properly attached structural sheathing and bottom chord
shall have a properly attached rigid ceiling. Load as shown for permanent lateral restraint of web.
This building components group inc. (ITBEGS) shall not be responsible for any deviation from this design
applied to a truss. A complete set of drawings for this truss shall be provided to the installer. The
bracing of trusses shall be in accordance with the latest edition of BCSI (Building Component Safety)
Information by TPI and WTC. Refer to drawings 1604-Z for standard plate positions. A seal on the
drawing or cover page listing the design shown. The suitability and use of this design for any structure is
the responsibility of the building designer per ASCE/TP1 Sec 2. For more information see
this job's specifications. TPI www.tpi.com WTC www.wtcindustry.com
ICC www.iccsafe.org



TC LL	20 0 PSF	REF	R9114- 28453
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	H05R9114 13326026
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN-	339703
DUR. FAC.	1 25		
SPACING	24 0"	JREF-	1V1J487_Z01

11/22/2013

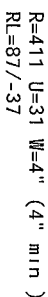
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

MMF-RS loads based on trusses located at least 7 50 ft from roof edge

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50



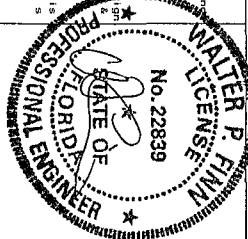
Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

12.03 04 0326 13

QTY: 8 FL/-/5/-/-/R/-

Scale = .5"/Ft.

ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0278

[illegible]

TC LL	20 0 PSF	REF	R9114- 28454
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326027
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD.	37 0 PSF	SEQN-	339602
DUR FAC.	1 25		
SPACING	24.0"	JREF -	1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City FL - EJ7A 7' End Jack)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpl(+/-)=0 18

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

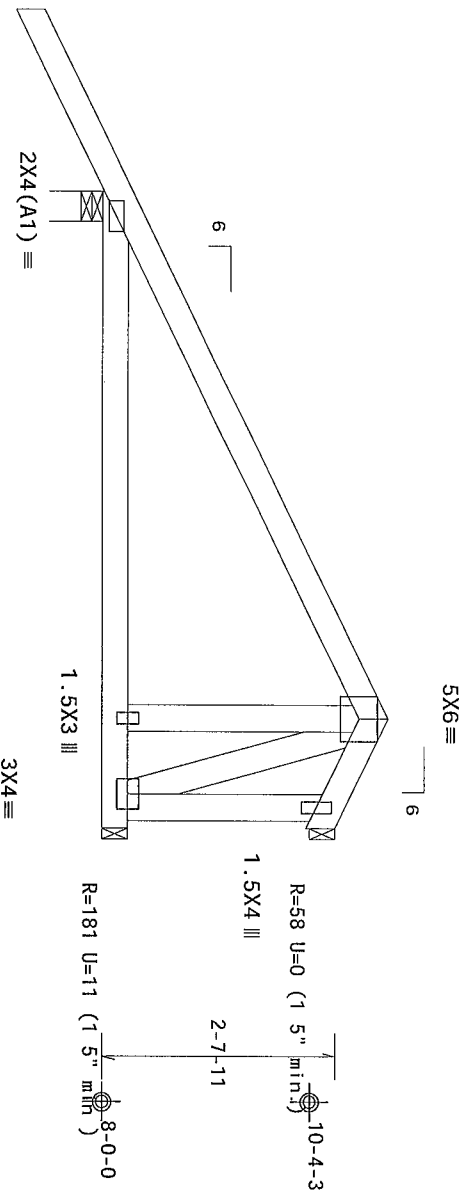
Wind loads and reactions based on MMFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase

MMFRS loads based on trusses located at least 7 50 ft from roof edge

factor for dead load is 1 50

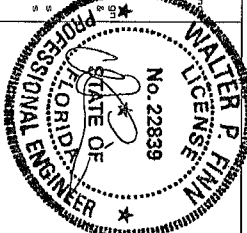


2-0-0
5-9-8
7-0-0 Over 3 Supports
1-2-8
R=411 U=23 W=4" (4 min)
RL=55/-35

PLT TYP WAVE Design Crit FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0) QTY: 1 FL/-/5/-/-/R/- Scale = 5"/Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, and installing. Refer to and follow the latest edition of the BCSI Building Component Safety Information by TP1 and WTC for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI section B3, B7 or B10 as applicable. Any failure to build the truss in conformance with ANSI/TP1 1, or for handling, shipping, installation, or erection, shall be the responsibility of the installer. The manufacturer shall not be responsible for any damage to the truss or any other component of the building system. Details of connections shall be as shown in the drawings. Refer to drawings 1604-2 for standard splice details. Drawing or cover page listing this drawing and dates acceptance of professional engineering and the responsibility of the Building Designer per ANSI/TP1 1 Sec 2. For more information see this job's special notes page TP1-BCSI www.bcsinfo.com TP1 www.tp1net.org WTC www.sbcindustry.com



ALPINE
ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278

TC LL	20 0 PSF	REF	R9114- 28455
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	H05R9114 13326005
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN	339857
DUR FAC.	1.25		
SPACING	24.0"	JREF	1V1J487_Z01

11/22/2013

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCp1(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

factor for dead load is 1.50



REF R9114- 28

Orlando FL, 32837
FL COA #0278

~~11/22/2013~~

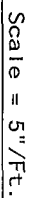
TC LL	20.0 PSF	REF	R9114- 28456
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HCUSR9114 13326006
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37.0 PSF	SEQN-	339694
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

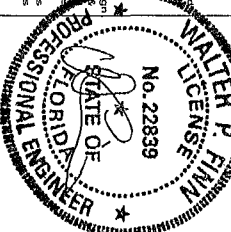
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, located
anywhere in roof, RISK CAT II EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCp1(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50



ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

[illegible]

TC LL	20.0 PSF	REF	R9114- 28457
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326007
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37.0 PSF	SEQN-	330594
DUR FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

(13-281--BRYAN ZECHEER /Columbia County Builders -- Lake City, FL - EUTC 7 End Jack)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved
1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

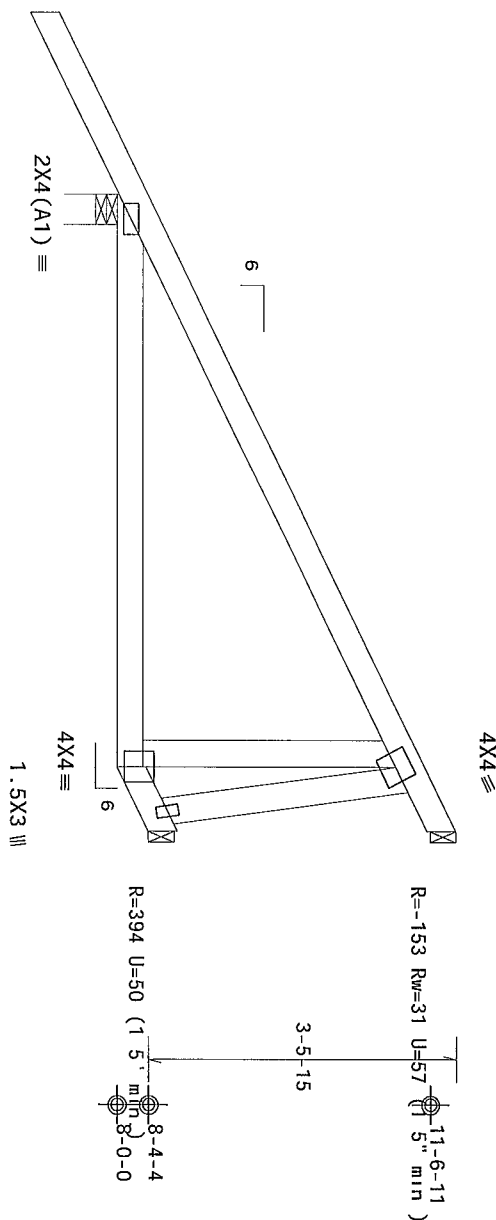
Shim all supports to solid bearing

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member
design

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50

MMFRS loads based on trusses located at least 7 50 ft from roof edge



6-3-8
7-0-0 Over 3 Supports
0-8-8
R=411 U=14 W=4 (4" min)
RL=75/-33

PLT TYP Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.04

QTY:6 FL/-5/-/-/R/-

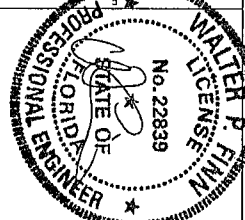
Scale = .5"/Ft

ALPINE

FLW Building Components Group Inc.

Orlando FL 32837
FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****WARNING**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabrication and erection. Refer to and follow the latest edition of BCSI (Building Components Safety Institute) practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3 B7 or B10 as applicable.
FLW Building Components Group Inc. (FLWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TP1-1 or for handling, shipping, installation, or erection of the truss. The user of this design shall be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the applicable building code. Refer to drawing 1604-2 for standard details. This drawing is not to be used for any other purpose without the written consent of FLWBCG. The responsibility of the building designer per ANSI/TP1-1 Sec 2. For more information see the general notes page FLW-BDS www.flwbcg.com TP1 www.tp1inc.org WTCA www.sdcindustry.com IBC www.ircsource.org



TC LL	20 0 PSF	REF	R9114- 28458
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HOURS9114 13326008
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37.0 PSF	SEQN-	330432
DUR FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

11/22/2013

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

W2 W3, W4, W11, W12 2x4 SP #2-13B
Lt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 500'

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

These support conditions used at bearings indicated
(H1) = HUS26 w/ (1)2x6 SP M-26 supporting member
(14) 0 148 x3" nails into supporting member,
(4) 0 148 x3 nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load
Calculated vertical deflection is 0 46" due to live load and 0 60' due to dead load at X = 18-2-8

120 mph wind 15 00 ft mean hgt., ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf wind BC DL=5 0 psf 6cpl(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Calculated horizontal deflection is 0 28" due to live load and 0 37" due to dead load

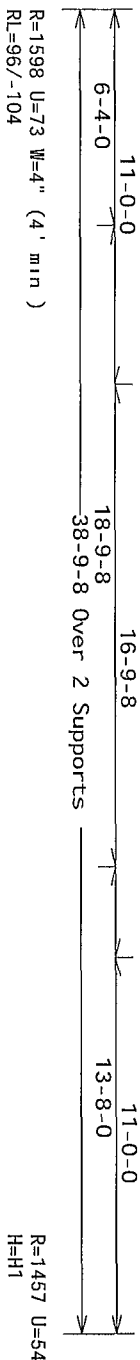
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MMFRS loads based on trusses located at least 7 50 ft from roof edge



PLT TYP	Design Crit	FBC2010Res/TP1-2007(STD)
20 Gauge HS, 18 Gauge HS, Wave		FT/RT=10%(0%)/0(0)

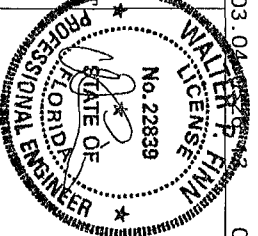
QTY.1 FL/-/5/-/-/R/-

Scale = .1875"/Ft.

ALPINE

Orlando FL, 32837
FL COA #0278

IMPORTANT
 WARNING: READ AND FOLLOW ALL INSTRUCTIONS ON THIS SHEET!
 FOR THIS DESIGN TO BE USED BY ALL CONTRACTORS, THE FOLLOWING INSTRUCTIONS
 MUST BE FOLLOWED:
 1. These require the extreme care in fabricating and handling an spigot and brace ng by TPI and WJMA. Refer to and
 follow the latest edition of BS61 (Building Component Safety) Information by TPI and WJMA. For safety
 reasons, no one performing the installation project must be allowed to proceed until industry training per BS61
 has been completed. No one should attempt to install the product without the proper training. The product should
 never be installed without a properly attended and rigid drilling. Loose items should be removed for permanent lateral restriction of weather
 until having bracing installed per BS61 section 8.3, 8.7 or 8.10 as apply cable.
 2. TPI Building Components Group Inc. (TIBCOG) shall not be responsible for any deviation from the design
 any fabricating or handling of the product. TIBCOG shall not be responsible for any damage to the product or any
 bracing ng of the product. Apply plates to each face of the steel, and position as shown above on the 10 mm
 Doka is, unless noted otherwise. Refer to drawings 180A-2 for standard plate position. As a result on the
 drawing ng or cover plate at the ng the drawing ng and careful acceptance of perfect and any need ng
 responsibility for solely for the design shown. The sub/typical and use of this design for any structure
 the responsibility of the Building Design Group per AS/NZS/1171 Sec 2. For more information on see This job
 general notes page TIBCOG www.tibcog.com TPI www.tpi.net.org WJMA www.theindustry.com
 TIBCOG www.tibcog.com TPI www.tpi.net.org WJMA www.theindustry.com



11/22/2013

TC LL	20.0 PSF	REF	R9114- 28459
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326009
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN-	330718
DUR.FAC	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load

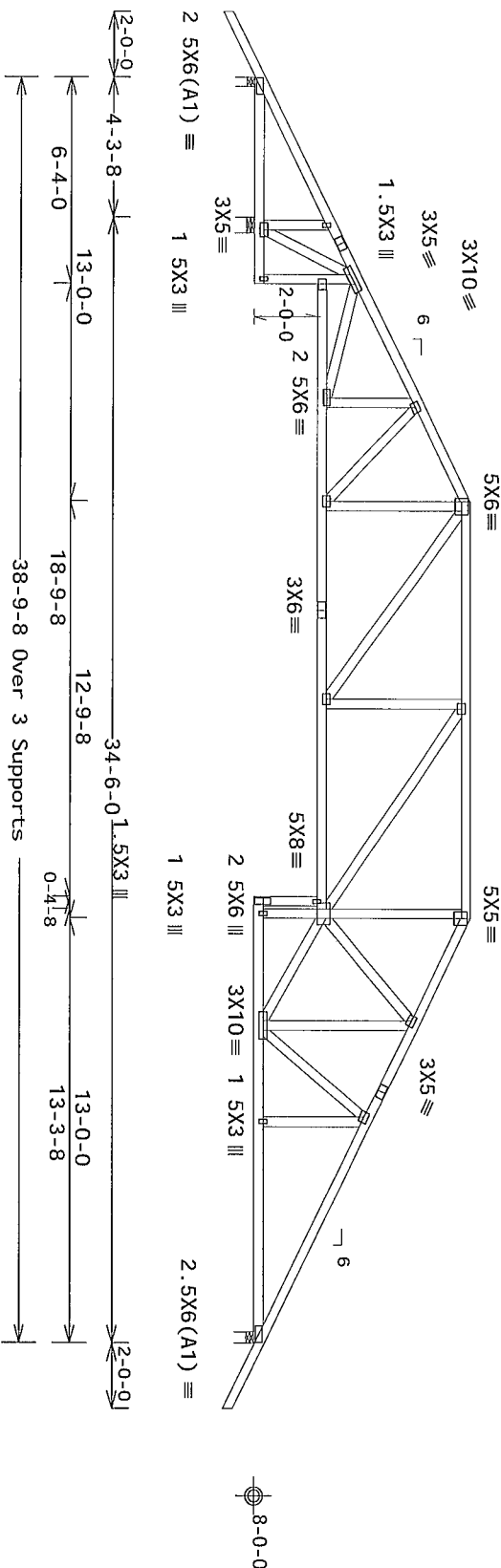
MMFRS loads based on trusses located at least 15 00 ft from roof edge

Negative reaction(s) of -303# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf 6Cp1 (+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50



R=128/-304 U=133 W=3 5" (3 5" min)
RL=131/-131 R=1921 U=0 W=5 657" (5

R=1362 U=0 W=4" (4" min)

Note All Plates Are 3X4 Except As Shown

PLT Typ	Wave	Design Crit	FBC2010Res/TP1-2007(Std) FT/RT=10%(0%)/0(0)
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12 03.04.2026 13:00

QTY:1 FL/-/5/-/-/R/-

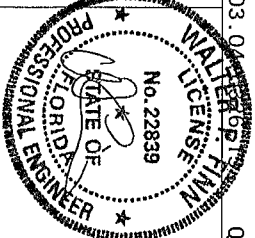
Scale = .1875"/Ft

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****IMPORTANT****
WARNING: READ AND FOLLOW ALL NOTES ON THIS SHEET
BEFORE THIS DESK SET OR TO ALL CONTRACTORS INCLUDING INSTALLERS
 This desk set is designed for hanging, stripping, installing and bracing
 to follow the latest edition of BCS (Building Code of Standards) and
 practices prior to performing the final installation. Installers shall provide temporary bracing per BCS
 unless noted otherwise. Two chord shall have properly attached structural shearing and bottom chord
 shall have a properly attached right side leg. Locations shown for permanent lateral resistance of webs
 shall have bracing installed per BCS section 8.5, B7 or B10 as applicable.
 The Building Components Group Inc. (TBGCO) shall not be responsible for any deviation from this design
 or any other code requirements. The user shall be responsible for the proper hanging, stripping and bracing
 of trusses. Apply plates to each piece of truss and post on as shown above for the joint.
 Details unless noted otherwise. Refer to draw ngs TBGCO-2 for standard BCS post connections. A seal on the
 drawing of cover plates listing the drawing number and dates acceptance of project and any other
 information shall be provided to the user. The user shall be responsible for the proper hanging, stripping and
 the responsibility of the Building Components Group Inc. (TBGCO) shall not be responsible for any deviation from this design
 general notes page 1TBGCO-2 www.tbccog.com TP1 www.tp.net.org WTCA www.wtcaindustry.com



11/22/2013

TC LL	20 0 PSF	REF	R9114 - 28461
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HOUSE114 13326011
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LB	37 0 PSF	SEQN-	330663
DUR FAC	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City, FL - H13A 26'4" Steepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x6 SP #1-13B
T3 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

Calculated horizontal deflection is 0.11' due to live load and 0.15
due to dead load

Bottom chord checked for 10.00 psf non-concurrent live load

(1) 2x6x10-1-15 SP #1-26 scab at left end Attach scab to face of chord
with 0.131"x3, min nails @ 8" OC, plus additional nail clusters at
BRG (4), heel (5), 1st panel point (2)

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf,
wind BC DL=5.0 psf GCP(+/-)=0.18

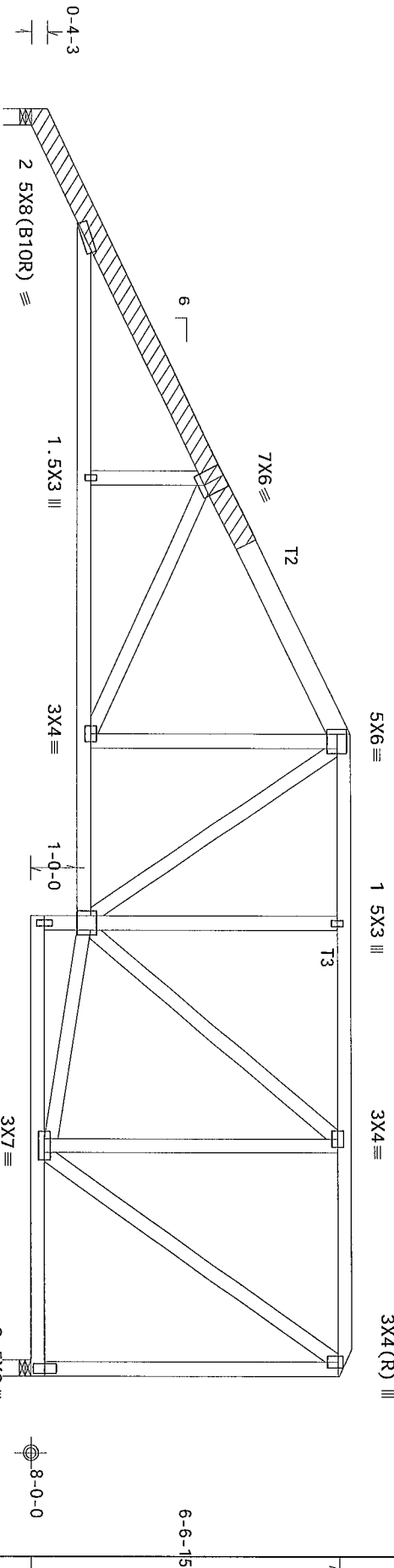
Wind loads and reactions based on MMFRS with additional C&C member
design

Right end vertical not exposed to wind pressure

In lieu of structural panels use purlins to brace all flat TC @ 24"
OC

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1.50

MMFRS loads based on trusses located at least 7.50 ft from roof edge



R=1009 U=19 W=4" (4 min)
RL=111/-28

R=983 U=46 W=4" (4 min)

PLT TYP. Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%/0(0))

12.03 04 08 26 13

QTY 1 FL/-/5/-/-/R/-

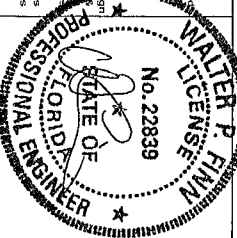
Scale = .3125"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, erecting and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI section 83.87 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TP1-1 or for handling, shipping, erecting, bracing or any other failure. The user of this design shall be responsible for any deviation from this design. Details unless noted otherwise. Refer to drawings 1604-2 for standard placement of a seal on the drawing or cover page. This drawing indicates acceptance of professional engineering and the responsibility of the building designer per ANSI/TP1-1 Section 2. For more information see this job's general notes page. ITW BCG www.bcgeng.com TP1 www.tp1net.org WTC www.sbcindustry.com This job's IBC www.icbcare.org



TC LL	20.0 PSF	REF	R9114-28462
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326012
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37.0 PSF	SEQN	330493
DUR FAC.	1.25		
SPACING	24.0"	JREF	1V1J487_Z01

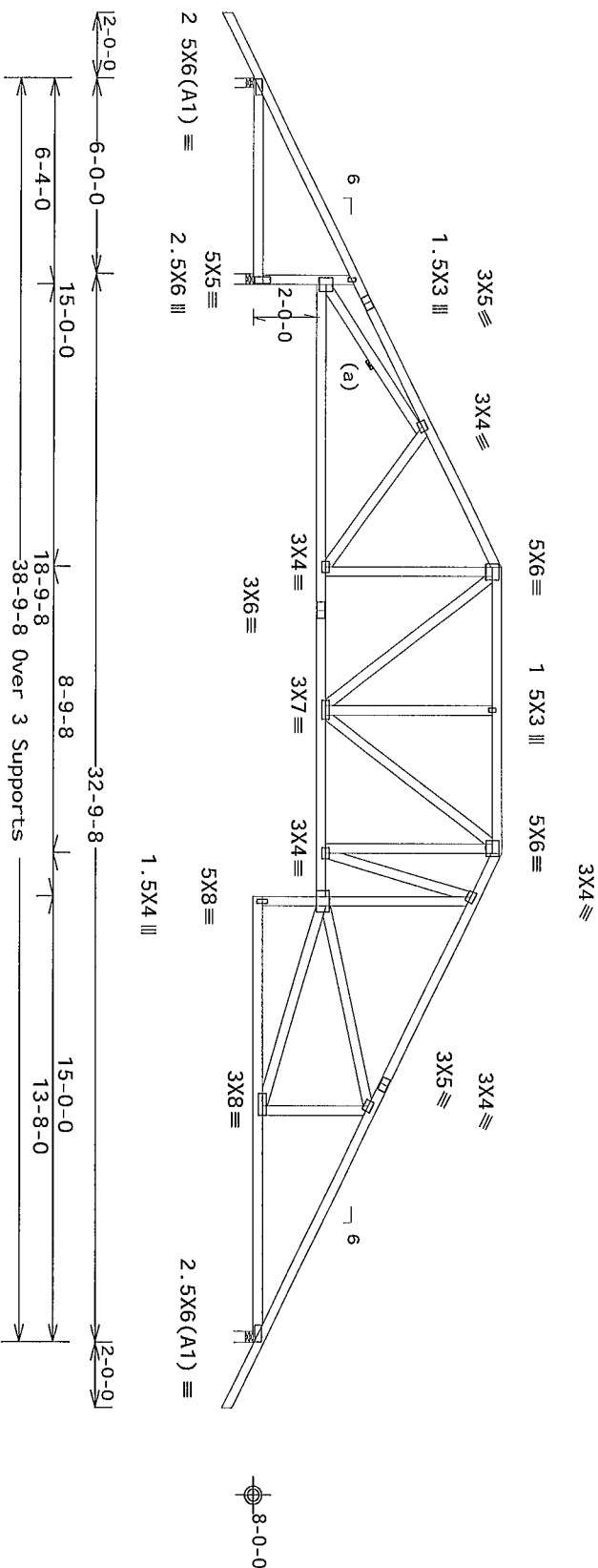
11/22/2013

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf wind BC DL=5 0 psf GCPI (+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member	In lieu of structural panels use purlins to brace all flat TC @ 24' OC
Bottom chord checked for 10 00 psf non-concurrent live load	Deflection meets L/240 live and L/180 total load
MFPRS loads based on trusses located at least 15 00 ft from roof edge	Creep increase factor for dead load is 1 50



R=364 U=32 W=3 5' (3 5' min)
RL=146/-146 R=1454 U=0 W=4' (4' min)

R=1361 U=0 W=4' (4" min)

PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

12 03 04 0226 13

QTY 1 FL/-/5/-/-/R/-

Scale = 1875"/Ft.

ALPINE

ITW Building Components Group Inc.

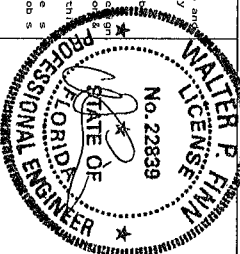
Orlando FL, 32837
FL COA #0278

*******IMPORTANT***** FINISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS**

Trustees require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Builing Component Safety Information) by TPI and WITCA) for safety practices prior to performing these functions. Insulators shall provide temporary bracing per BCSI shall have a properly attached rigid ceiling. Locations shown for structural bracing and bottom end of steel shall have bracing installed per BCSI section 83, B7 or B10, as applicable.

17W Builing Component Group Inc. (17WBOSI) shall not be responsible for any new or from a third party failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, metal fabrication of trusses. Apply plates to each face of truss and post on as shown above and on the joint details unless noted otherwise. Refer to draw ngs 180A-2 for standard gable post trons. A seal on the underside of the steel deck covering the deck details of profess and engineer. The responsibility of the building Designer per ANSI/TPI 1, Section 2. For more information on see general notes page 17W BCS www building.com TPI www tpi.net org WITCA www sbc industry.com

17W BCS www building.com TPI www tpi.net org WITCA www sbc industry.com



11/22/2013

TC LL	20 0 PSF	REF	R9114 - 28463
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326013
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD	37.0 PSF	SEQN-	330427
DUR FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City, FL - H15A 26 4 Steepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B T1 2x6 SP M-26
T2 2x6 SP #2-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved
1/30/2013 by ALSC

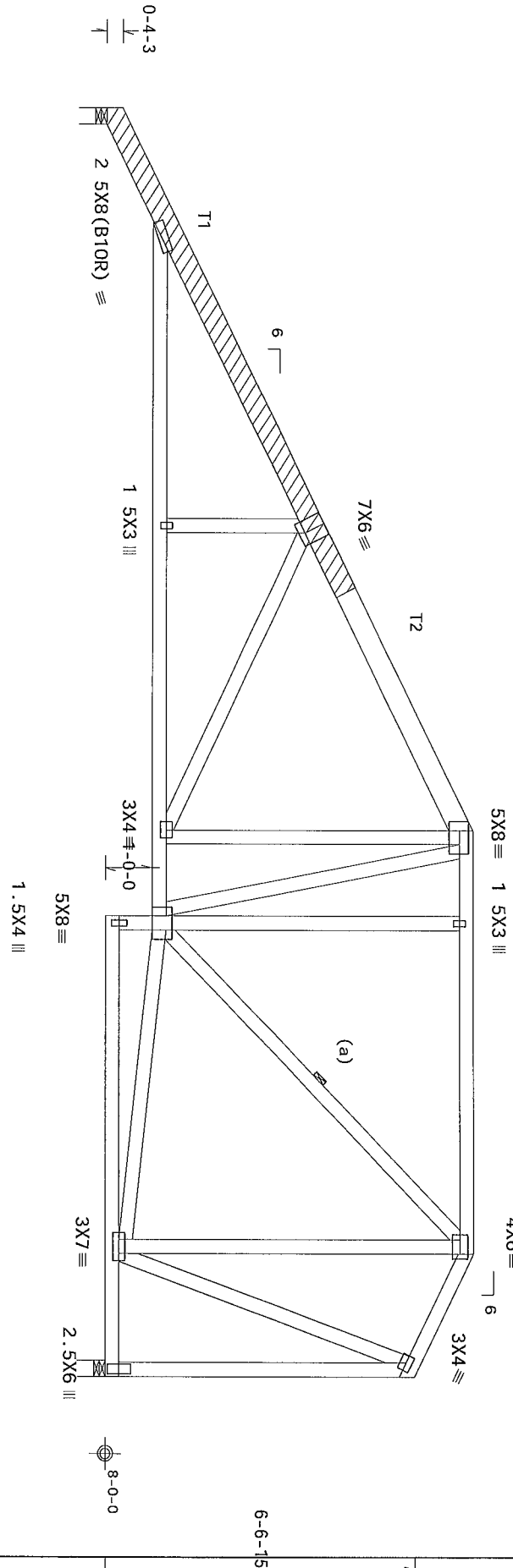
Calculated horizontal deflection is 0.12 due to live load and 0.16
due to dead load

In lieu of structural panels use purlins to brace all flat TC @ 24
OC

(1) 2x6x11-3-6 SP M-26 scab at left end Attach scab to face of chord
with 0.131 x3 min nails @ 8 OC Plus additional nail clusters at
BRG (4) heel (5) 1st panel point (2)

MMFRS loads based on trusses located at least 15.00 ft from roof
edge

120 mph wind 15.00 ft mean hgt ASCE 7-10, CLOSED bldg not located
within 9.00 ft from roof edge RISK CAT II EXP 8 wind TC DL=3.5 psf
wind BC DL=5.0 psf GCPI (+/-)=0.18
Wind loads and reactions based on MMFRS with additional C&C member
design
Right end vertical not exposed to wind pressure
(a) Continuous lateral restraint equally spaced on member
Bottom chord checked for 10.00 psf non-concurrent live load
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1.50



12-7-8 14-5-0 26-4-0 Over 2 Supports 8-9-8 9-6-8 2-6-8
R=1009 U=0 W=4' (4' min)
RL=91/-46
R=983 U=0 W=4' (4' min)

PLT TYP Wave Design Crit FBC2010Res/TPI-2007(STD) 12 03 04 QTY 1 FL/-/5/-/-/R/- Scale = 3125"/Ft

ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278

****IMPORTANT** READ AND FOLLOW ALL NOTES ON THIS SHEET!**
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and
follow the latest edition of BCSI (Building Component Safety Information by TPI and WTC) for safety
practices prior to performing those functions. Installers shall provide temporary bracing per BCSI
unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord
shall have a properly attached rafter gable end. Locate one shown for permanent lateral restraint of web.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design
any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installing or
bracing of the truss. Trusses are shown above and on the joint to be installed on the roof deck.
Details unless noted otherwise. Refer to drawings 160A-2, 160A-3, 160A-4, 160A-5, 160A-6, 160A-7, 160A-8, 160A-9, 160A-10, 160A-11, 160A-12, 160A-13, 160A-14, 160A-15, 160A-16, 160A-17, 160A-18, 160A-19, 160A-20, 160A-21, 160A-22, 160A-23, 160A-24, 160A-25, 160A-26, 160A-27, 160A-28, 160A-29, 160A-30, 160A-31, 160A-32, 160A-33, 160A-34, 160A-35, 160A-36, 160A-37, 160A-38, 160A-39, 160A-40, 160A-41, 160A-42, 160A-43, 160A-44, 160A-45, 160A-46, 160A-47, 160A-48, 160A-49, 160A-50, 160A-51, 160A-52, 160A-53, 160A-54, 160A-55, 160A-56, 160A-57, 160A-58, 160A-59, 160A-60, 160A-61, 160A-62, 160A-63, 160A-64, 160A-65, 160A-66, 160A-67, 160A-68, 160A-69, 160A-70, 160A-71, 160A-72, 160A-73, 160A-74, 160A-75, 160A-76, 160A-77, 160A-78, 160A-79, 160A-80, 160A-81, 160A-82, 160A-83, 160A-84, 160A-85, 160A-86, 160A-87, 160A-88, 160A-89, 160A-90, 160A-91, 160A-92, 160A-93, 160A-94, 160A-95, 160A-96, 160A-97, 160A-98, 160A-99, 160A-100, 160A-101, 160A-102, 160A-103, 160A-104, 160A-105, 160A-106, 160A-107, 160A-108, 160A-109, 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160

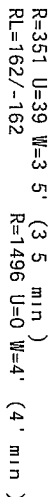
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCp1(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1 50



Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

QTY 1 FL/-/5/-/-/R/-

Scale = 1875"/Ft

ITW Building Components Group Inc.

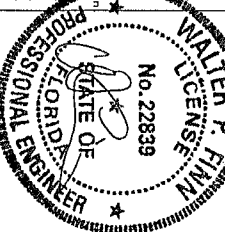
Orlando FL, 32837
FL COA #0278

WARNING!! **READ AND FOLLOW ALL NOTES ON THIS SHEET**
FORNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
 Trussmen require extreme care in fabricating, handling, shipping, installing, and bracing. Follow the latest edition of BCOS (Building Component Safety Information) from TPI and WTCO for safety practices and/or to perform these functions. Installers shall provide temporary bracing per BCOS. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCOS. Sections 83, 87, or 810 for permanent lateral restraint are of wood.

TPI Building Components Group, Inc. (TBCG) shall not be responsible for any deviation from this design. Any failure to build the truss in conformance with the ANSI/TPI 1 or for handling or shipping metal installation shall be the responsibility of the fabricator. Refer to drawings for details. The position as shown above and on the joint is not to be covered. The design shall not be altered without the written approval of the designer. The responsibility for the design shall be the responsibility of the Building Designer. The suitability and use of this design for any structure is the responsibility of the Building Designer.

general notes page TPI-BCG www.tbcg.com TPI www.tpi.net.org WTCO www.sbcindustry.com
 www.cstate.org

Refer to and



11/22/2013

TC LL	20.0 PSF	REF	R9114-28465
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326015
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37.0 PSF	SEQN-	330415
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City FL - H19 38 9 8 Steepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24"
OC

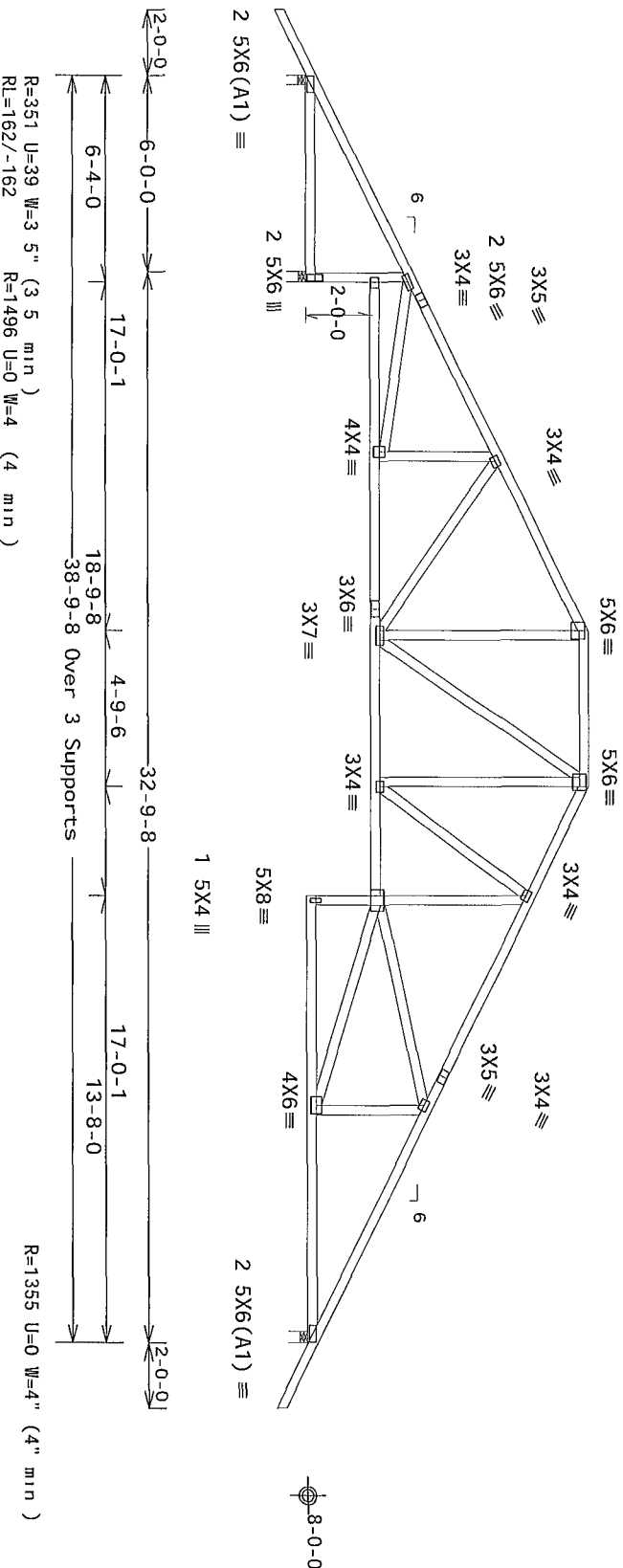
MMFRS loads based on trusses located at least 15 00 ft from roof
edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf,
wind BC DL=5 0 psf GCPI (+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member
design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



PLT TYP Wave Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.04 QTY: 1 FL/-/5/-/-/R/- Scale = .1875"/Ft.

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS SHEET

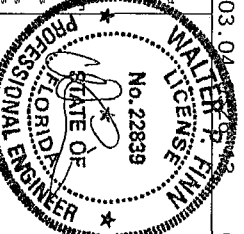
Trusses are the extreme care in fabricating, handling, shipping, installing and bracing. For the safety of the structure, the installer shall follow the instructions for the use of the trusses. The installer shall provide temporary bracing per BCS1 unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCS1 sections B3 B7 or B10 as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TP1 1 or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joist. It is the responsibility of the Building Designer to ensure the truss is installed in accordance with the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec 2. For more information on see This Job's general notes page. ITW-BCG www.itwbcg.com TP1 www.tp1.net.org WTCA www.sbc-industry.com

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278



11/22/2013

TC LL	20.0 PSF	REF	R9114- 28467
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326017
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT. LD.	37.0 PSF	SEQN-	330908
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

T3 2x4 SP #1-13B

Bot	chord	2x4	SP	#1-13B
	Webs	2x4	SP	#3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50

(2) 2x6x12-3-6 SP M-26 scabs at left end Attach one scab to each outer face of chord with 0 131" x3", min nails @ 8" OC, Plus additional nail clusters at BRG (3), heel (3), 1st panel point (0)

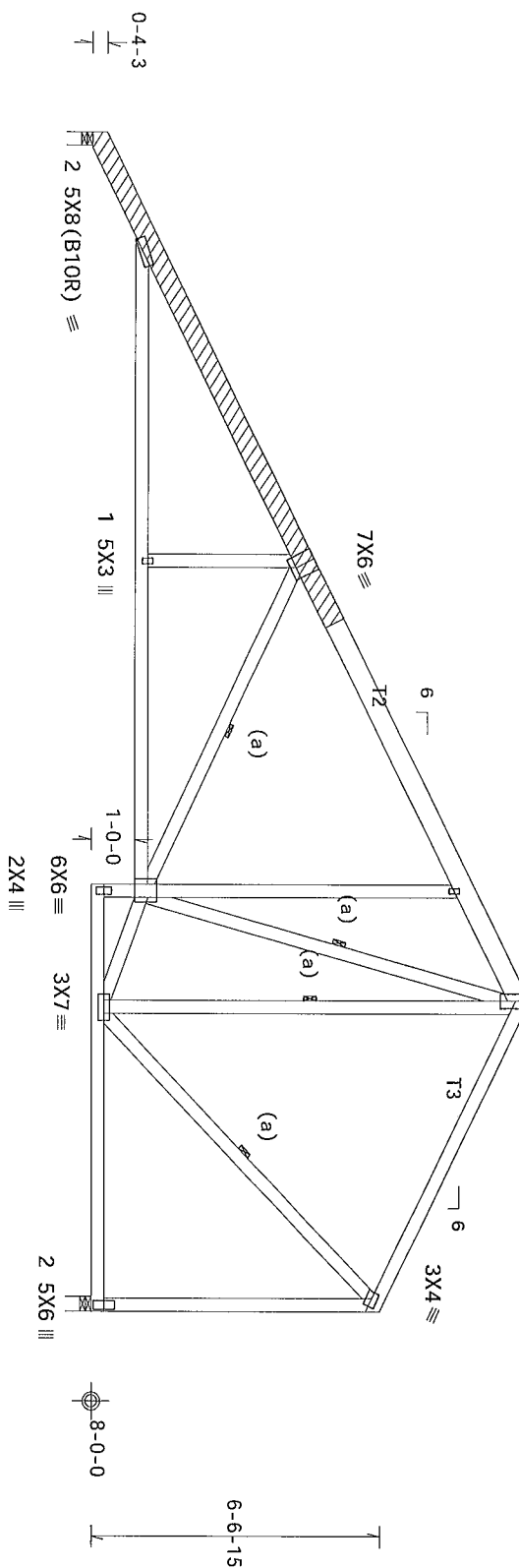
120 mph wind 15 00 ft mean hgt., ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf wind BC DL=5 0 psf 6Cp(1+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 psf non-concurrent live load

MMFRS loads based on trusses located at least 15 00 ft from roof edge



17-0-4

6-11-4

14-5-0

26-4-0 Over 2 Supports

R=1007 U=0 W=3 5" (3 5" min)

R=985 U=0 W=4" (4" min)

PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
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12.03.04 0326 13

QTY:1 FL/-/5/-/-/R/-

Scale = .25"/Ft.

ALPINE

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

****IMPORTANT****
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI's (Building Component Safety) Information by TPI and WTCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI practices noted otherwise. Truss chord shall have monogrammed attachment anchorage and bearing plates.

shall have bracing installed per BCS sections B3, B7 or B10 as applicable

ITW Building Components Group Inc (ITBGC) shall not be responsible for any deviation from the design or installation instructions provided by ITBGC. The user assumes all liability for any failure to build the truss in conformance with ANSI/PPI 1 or for handling, shipping, installation, or use of the truss. Apply charges to each frame of truss as shown above.

Drawings of a proposed project should be submitted to the Department of Planning and Development, 1000 Pennsylvania Avenue, N.E., Washington, D.C. 20002. The drawings should be submitted in triplicate, and should include a cover page, a title page, a drawing of the project, and a drawing of the project showing the location of the project on the map. The drawings should be submitted in triplicate, and should include a cover page, a title page, a drawing of the project, and a drawing of the project showing the location of the project on the map. The drawings should be submitted in triplicate, and should include a cover page, a title page, a drawing of the project, and a drawing of the project showing the location of the project on the map.

[illegible]

ILU www.iccsate.org

A circular professional engineer seal for the State of Florida. The outer ring contains the text "PROFESSIONAL ENGINEER" at the top and "STATE OF FLORIDA" at the bottom, separated by two stars. The inner circle contains the name "WALTER P. FINN" at the top, "LICENSEE" at the bottom, and the license number "No. 22839" in the center. A signature is written across the seal.

TC LL	20.0 PSF	REF	R9114- 28468
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326018
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37.0 PSF	SEQN-	330516
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487 Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City, FL - H3 11'7 Steppdown Hip Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP 2850F-2 3E

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf,
wind BC DL=5.0 psf GCPI(+/-)=0.18

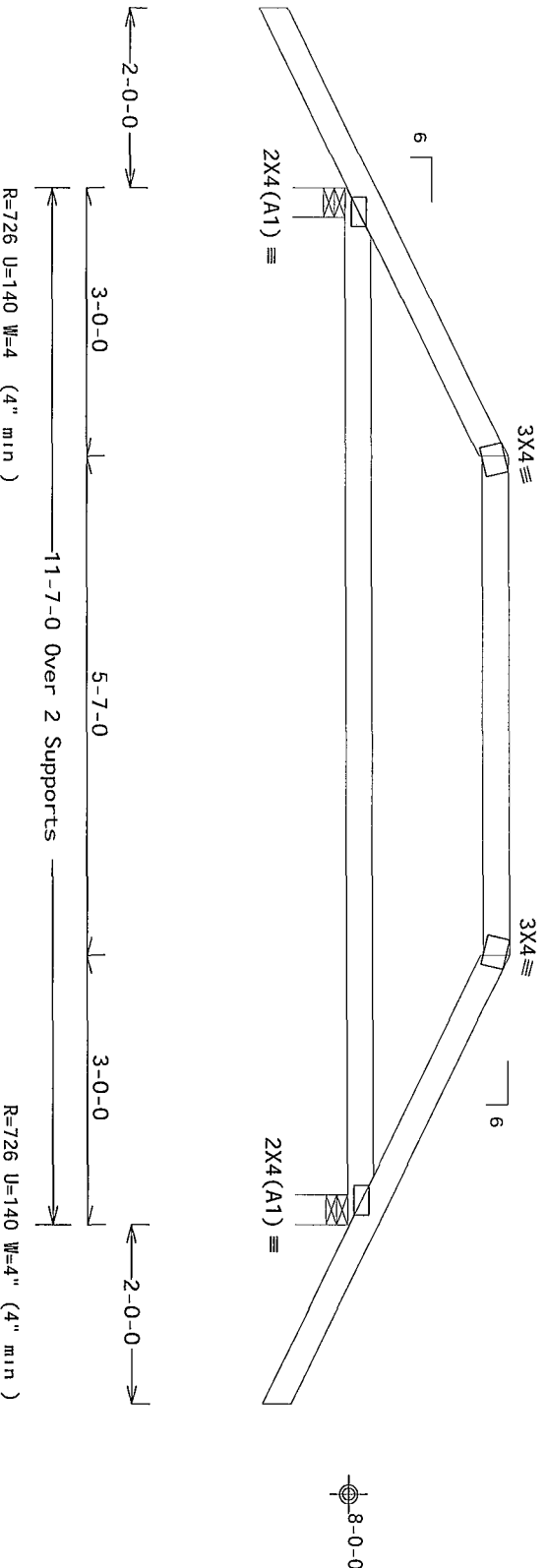
Wind loads and reactions based on MMFRS

In lieu of structural panels use purlins to brace all flat TC @ 24"

Bottom chord checked for 10.00 psf non-concurrent live load

Special loads			
-----Lumber			
TC-From	Dur Fac =1.25 /	Plate Dur Fac =1.25)	
TC-From	56 pif at -2.00 to	56 pif at 3.00	
TC-From	28 pif at 3.00 to	28 pif at 8.58	
TC-From	56 pif at 8.58 to	56 pif at 13.58	
BC-From	4 pif at -2.00 to	4 pif at 0.00	
BC-From	20 pif at 0.00 to	20 pif at 3.03	
BC-From	10 pif at 3.03 to	10 pif at 8.55	
BC-From	20 pif at 8.55 to	20 pif at 11.58	
BC-From	4 pif at 11.58 to	4 pif at 13.58	
TC-70.58 lb Conc	Load at 3.03,	8.55	
TC-43.90 lb Conc	Load at 5.06,	6.52	
BC-111.48 lb Conc	Load at 3.03,	8.55	
BC-46.51 lb Conc	Load at 5.06,	6.52	

Deflection meets L/240 live and L/180 total load Creep increase
Factor for dead load is 1.50



PLT TYP Wave
Design Crit. FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

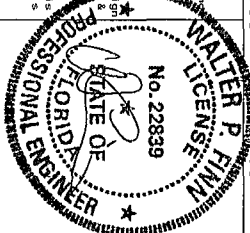
12.03.04.0326.13 QTY: 1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to manufacturer's instructions for details. Temporary bracing per BCSI practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI section 83 B7 or B10 as applicable.
1TW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. It is the responsibility of the fabricator to ensure that the truss is built in accordance with the design. Refer to drawings 1604-2 for standard plate positions. A detail on the back of the truss is provided for the fabricator's reference. The suitability and use of this design for any structure is the responsibility of the fabricator. The suitability and use of this design for any structure is the responsibility of the fabricator. The suitability and use of this design for any structure is the responsibility of the fabricator.
ICC www.iccsafe.org www.twbcg.com TP1 www.tp1inc.org WTC www.abclindustry.com This job is



TC LL	20.0 PSF	REF R9114- 28469
TC DL	7.0 PSF	DATE 11/22/13
BC DL	10.0 PSF	DRW HCUSR9114 13326019
BC LL	0.0 PSF	HC-ENG AP/AP
TOT LD	37.0 PSF	SEQN- 339854
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1V1J487_Z01

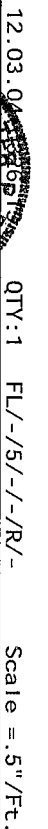
11/22/2013

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCp1(+/-)=0 18

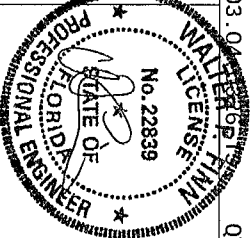
Wind loads and reactions based on MMFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets $L/240$ live and $L/180$ total load Creep increases
factor for dead load is 1 50



ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0 278

[illegible]

FL/-/5/-/-/R/-		Scale = .5"/Ft.
TC LL	20.0 PSF	REF R9114- 28470
TC DL	7.0 PSF	DATE 11/22/13
BC DL	10.0 PSF	DRW HCUSR9114 13326020
BC LL	0.0 PSF	HC-ENG AP/AP
TOT. LD.	37 0 PSF	SEQN- 339734
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1V1J487_Z01

Top chord 2x4 SP 2850F-2 3E T2, T3 2x6 SP M-26
Bot chord 2x4 SP 2850F-2 3E B2, B3 2x6 SP M-26
Webs 2x4 SP #3-13B W3, W4, W14 2x4 SP 2850F-2 3E
W5, W12, W13 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf 60psi(+/-)=0 18

(J) Hanger Support Required, by others

In lieu of structural panels use purlins to brace all flat TC @ 24
OC

Calculated vertical deflection is 1 08' due to live load and 1 43' due
to dead load at X = 17-7-11

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131"x3", min nails
Top Chord 1 Row @12 00" o c
Bot Chord 1 Row @12 00" o c
Webs 1 Row @ 4' o c
Use equal spacing between rows and stagger nails
in each row to avoid splitting

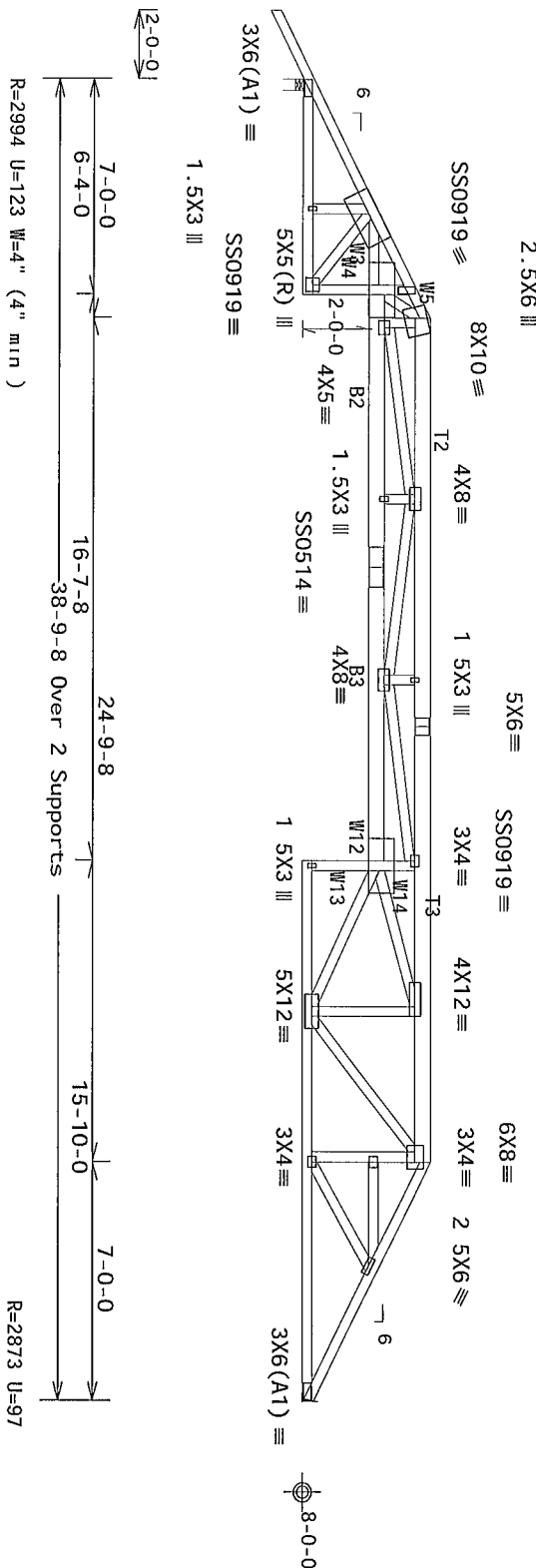
Wind loads and reactions based on MMFRS

Calculated horizontal deflection is 0 46" due to live load and 0 61"
due to dead load

#1 hip supports 7-0-0 jacks with no webs

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50

WARNING Furnish a copy of this DWG to the installation contractor
Special care must be taken during handling, shipping and installation
of trusses See "WARNING" note below



PLT TYP. 18 Gauge HS, Wave

Design Crit. FBC2010Res/TP1-2007 (STD)
FT/RT=10%(0%/0(0))

12 03 01

QTY 1 FL/-/5/-/-/R/-

Scale = .1875"/ft

ALPINE

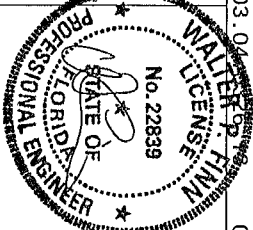
ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0278

IMPORTANT READ AND FOLLOW ALL NOTES ON THIS SHEET!

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information) by TPI and WFOA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or for any damage to the truss or its components. The user of this design shall be responsible for obtaining all necessary permits and for obtaining all necessary approvals from the local building department. The user of this design shall be responsible for obtaining all necessary approvals from the local building department. The user of this design shall be responsible for obtaining all necessary approvals from the local building department.



11/22/2013

TC LL	20.0 PSF	REF	R9114- 28471
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326021
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37.0 PSF	SEQN-	324376
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

These support conditions used at bearings indicated

Bottom chord checked for 10 00 psf non-concurrent live load
Calculated vertical deflection is 0 69" due to live load and 0 91" due to dead load at X = 18-0-8

120 mph wind 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf $G C P_i(+/-)=0.18$

Wind loads and reactions based on MMFRS with additional C&C member design

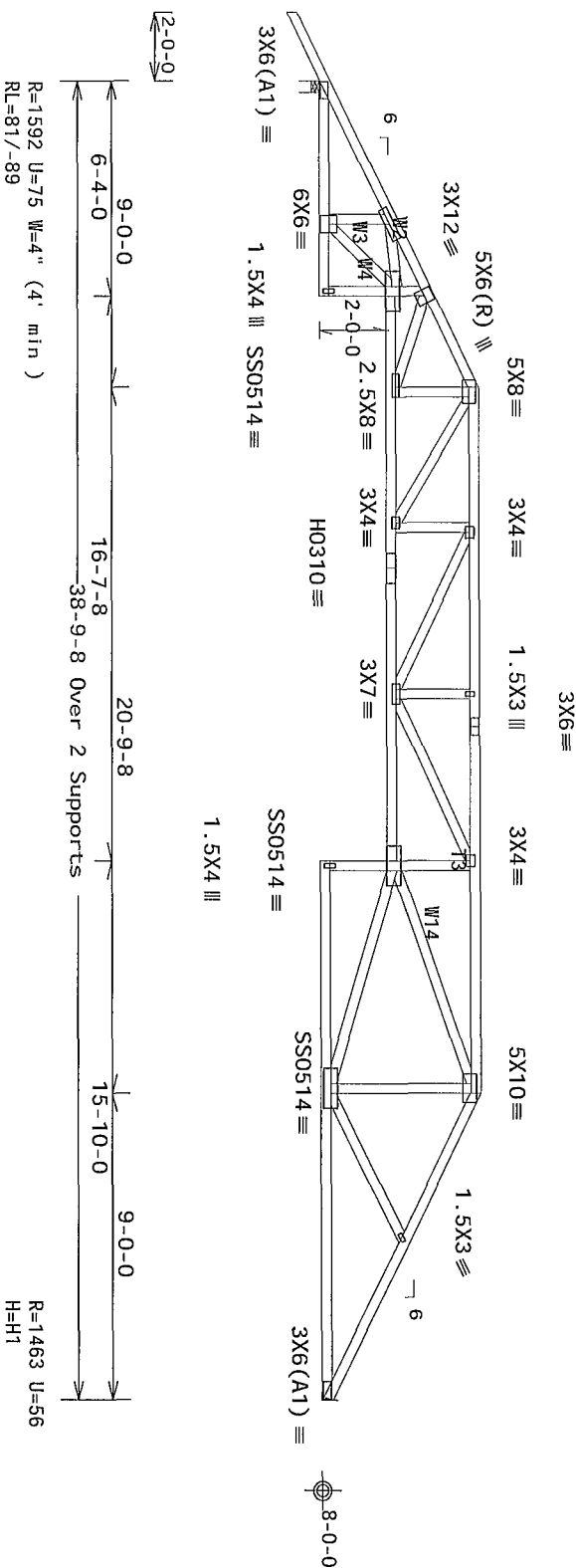
Calculated horizontal deflection is 0.34' due to live load and 0.45' due to dead load

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

MMFRS loads based on trusses located at least 7.50 ft from roof edge



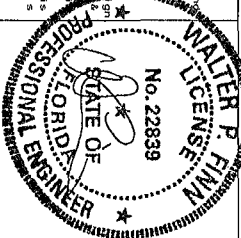
PLT_TYP	20 Gauge HS, 18 Gauge HS, Wave	Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
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12.03.04 0326 13 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

TC LL	20 0 PSF	REF	R9114- 28473
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSUR9114 13326028
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37 0 PSF	SEON-	330752
DUR FAC	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bid, not located within 9.00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCP1 (+/-)=0.18

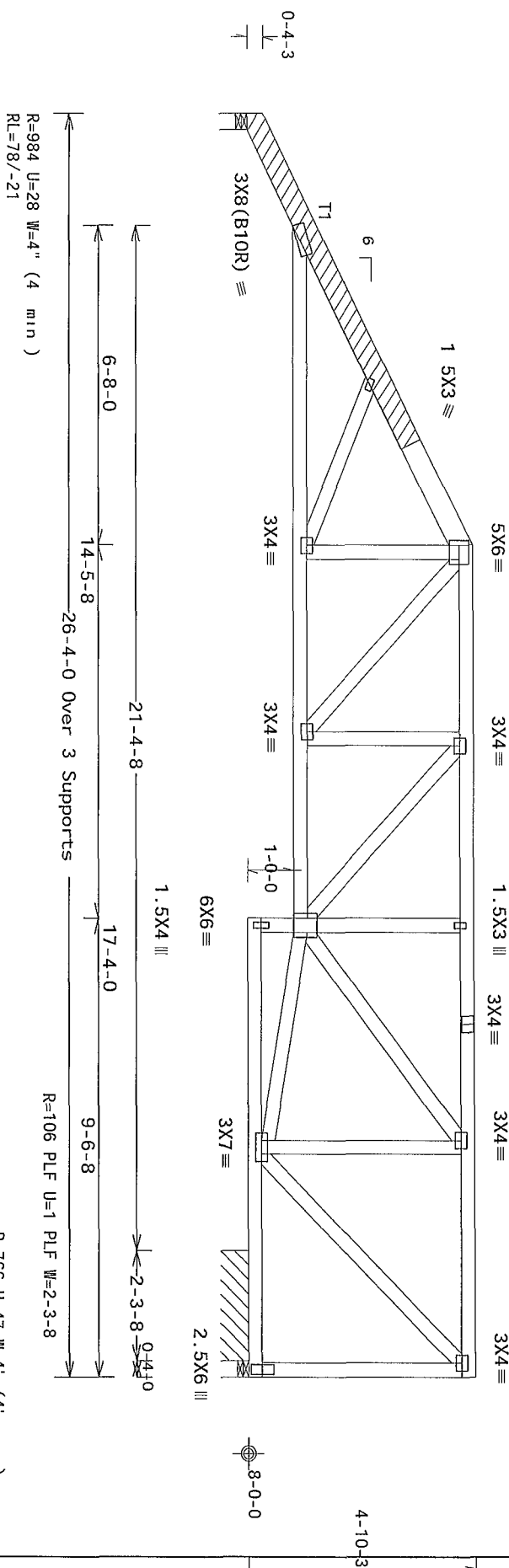
Wind loads and reactions based on MMFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 pst non-concurrent live load

MMFRS loads based on trusses located at least 7 50 ft from roof edge

(1) 2x6x/-8'-14 SP M-26 scab at left end Attach scab to face of chorio with 0 131"x3 , min nails @ 8" OC, plus additional nail clusters at BRG (4), heel (6), 1st panel point (3)



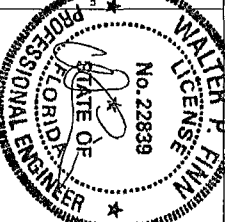
12 03.04.2013 QTY: 1 FL/-/5/-/-/R/- Scale = 3125"/Ft

••IMPORTANT••
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

11/22/2012

TC LL	20 0 PSF	REF	R9114- 28474
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HCSR9114 13326029
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT. LD	37 0 PSF	SEQN-	330480
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

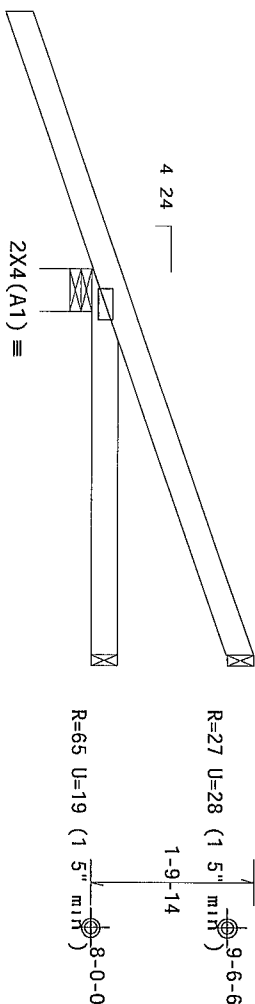
Special loads
----- (Lumber Dur Fac = 1.25 / Plate Dur Fac = 1.25)

TC- BC- From	2 pif at 0 pif at 2 pif at	0 00 to -2 83 to 0 00 to	2 pif at 4 pif at 2 pif at	4 24 0 00 4 24
TC- BC- From	2 pif at 0 pif at 2 pif at	0 00 to -2 83 to 0 00 to	2 pif at 4 pif at 2 pif at	4 24 0 00 4 24

TC-	63 45 1b	Conc	Load at	1 48
BC-	1 08 1b	Conc	Load at	1 48

Wind loads and reactions based on MMFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



2-9-15

←4-2-15 Over 3 Supports →

PLT TYP. Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
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12 03 04 05 06 07 08 09 10 11 12 13

QTY:2 FL/-/5/-/-/R/-

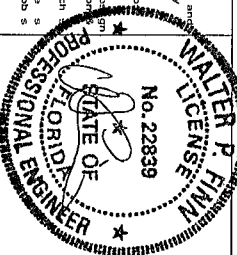
Scale = .5"/Ft

**** IMPORTANT ****
**** WARNING **** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS (INCLUDING INSTALLERS)

ADDITIONAL INFORMATION

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278



11/22/2013

TC LL	20 0 PSF	REF	R9114- 28475
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326030
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT.LD.	37.0 PSF	SEQN-	339851
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

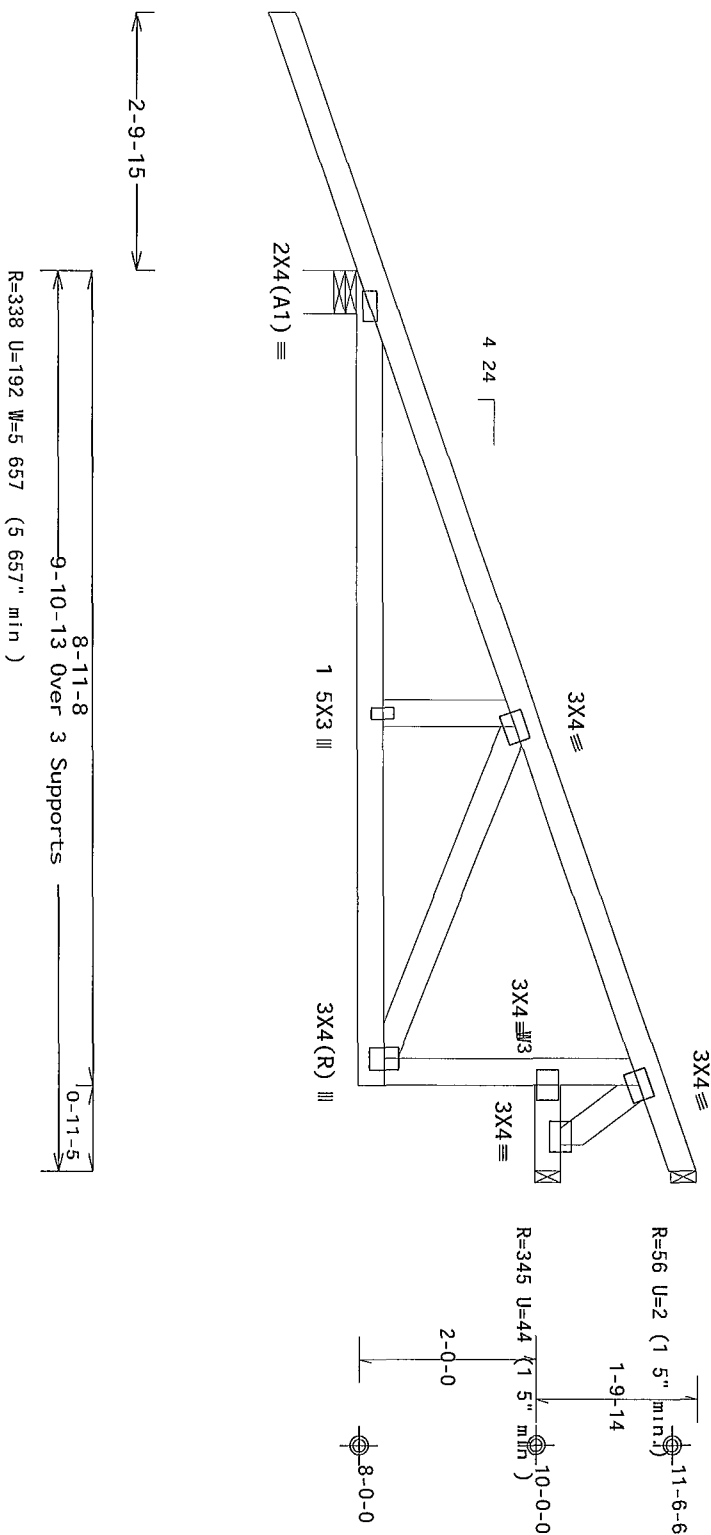
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MMFRS

Bottom chord checked for 10 00 psf non-concurrent live load

Special loads	Dur	Fac = 1 25 /	Plate	Dur	Fac = 1 25)
-----Lumber					
TC- From	0 pif	at -2 83	to	55 pif	at 0 00
TC- From	2 pif	at -0 00	to	2 pif	at 9 90
BC- From	0 pif	at -2 83	to	4 pif	at 0 00
BC- From	2 pif	at 0 00	to	2 pif	at 8 96
BC- From	2 pif	at 8 96	to	2 pif	at 9 90
TC- 63 45 1b Conc	Load	at	4 31		
TC- 87 80 1b Conc	Load	at	4 31		
TC- 215 63 1b Conc	Load	at	7 13		
BC- 1 08 1b Conc	Load	at	1 48		
BC- 93 02 1b Conc	Load	at	4 31		
BC- 172 85 1b Conc	Load	at	7 13		

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1 50



PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

12.03 04 0326 13

QTY:1

FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

****IMPORTANT****

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

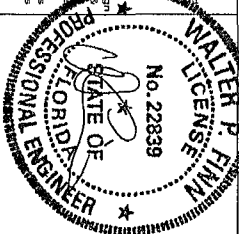
PURSUANT TO THIS DESIGN FOR THE CONSTRUCTION OF THE INSTALLATIONS

Trustees require you to execute care in fabricating handling and shipping installing and bracing Refer to and follow the latest edition of BCSI Building Component Survey Information on by TPI and WTCA for safety practice prior to performing these functions Installers shall provide temporary bracing per BCSI shall have a property attached if it is falling Location shown for permanent lateral restraint or modification shall have been installed per BCSI section BS 87 or B10 as apply cable

TPI Building Components Group Inc. (TBCG) shall not be responsible for any deviation from the design drawings of this product. The responsibility for compliance with all applicable codes and standards for the application of this product shall remain solely with the user. It is the responsibility of the user to ensure that the product is used in accordance with the manufacturer's instructions. The user shall be responsible for obtaining all necessary permits and approvals for the installation of this product. The user shall be responsible for ensuring that the installation is completed in accordance with the manufacturer's instructions. The user shall be responsible for ensuring that the installation is completed in accordance with the manufacturer's instructions.

Drawings are subject to change without notice. Refer to drawing BS 1604-2 for standard plate positions. A seal on this drawing or cover page indicating this drawing does not cater to acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per AS/NZS 1171 Part 1 Sec 2. For more information on see This is job general notes page. TPI BCS www.tbcg.com www.tpi.org www.wtca www.abc-industry.com

ICC NEW Castle.org



11/22/2013

TC LL	20.0 PSF	REF	R9114- 28476
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326031
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37.0 PSF	SEQN-	330597
DUR. FAC	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

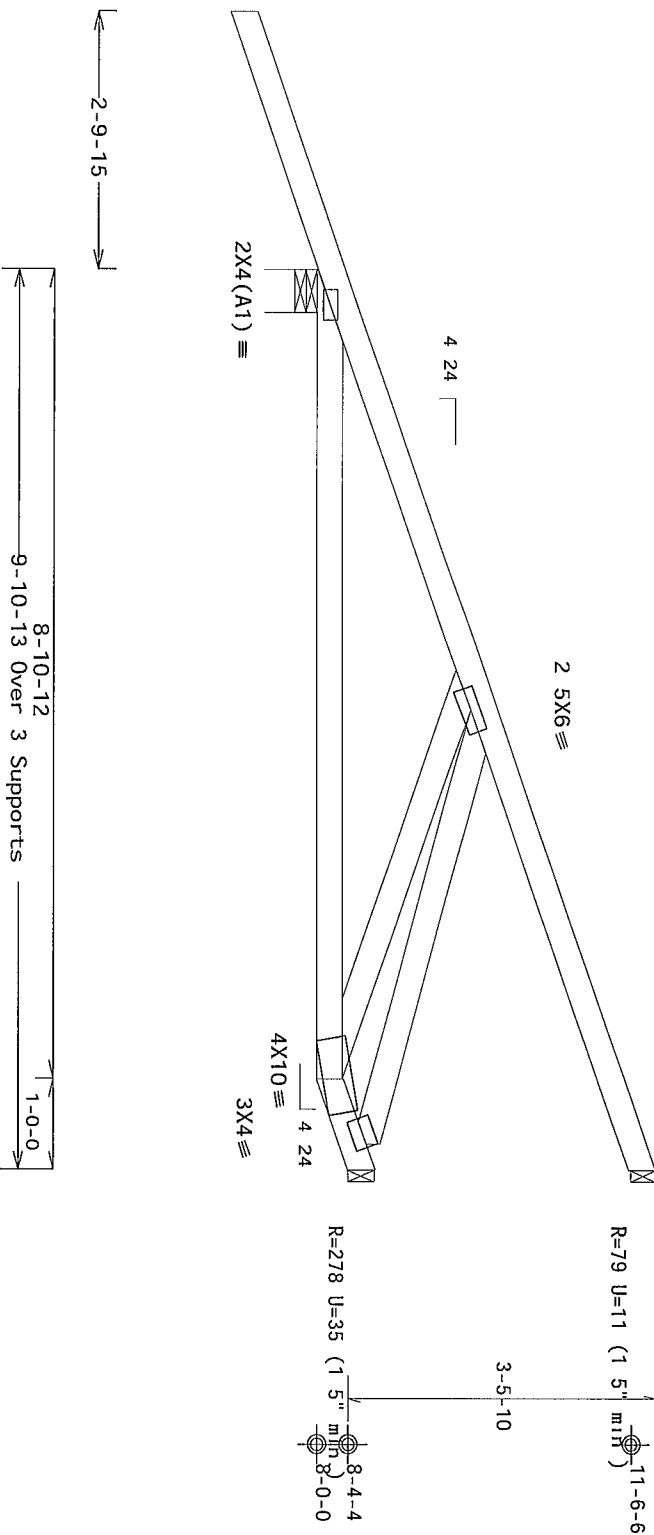
130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS

Bottom chord checked for 10 00 psf non-concurrent live load

Shim all supports to solid bearing

Special loads					
-----Lumber Dur Fac = 1.25 / Plate Dur Fac = 1.25)					
TC- From	0 pif at -2.83 to	55 pif at	0.00		
TC- From	2 pif at 0.00 to	2 pif at	9.90		
BC- From	0 pif at -2.83 to	4 pif at	0.00		
BC- From	2 pif at 0.00 to	2 pif at	8.90		
BC- From	2 pif at 8.90 to	2 pif at	9.90		
TC- -63 45 lb Conc	Load at	1 48			
TC- 87 80 lb Conc	Load at	4 31			
TC- 215 63 lb Conc	Load at	7 13			
BC- 1 08 lb Conc	Load at	1 48			
BC- 93 02 lb Conc	Load at	4 31			
BC- 172 85 lb Conc	Load at	7 13			
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50					



R=338 U=197 W=5 657' (5 657" min)

PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

12 03.04 2025

QTY: 1

FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc

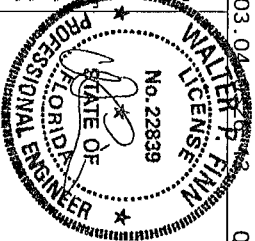
Orlando FL, 32837
FL COA #0278

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCOS (Building Component Safety) information from TP1 and WTCO for survey practices prior to performing these functions. Installers shall provide temporary bracing per BCOS. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint or webbs shall have bracing detailed per BCOS sections 83, B7 or B10 as applicable.

TP1 Building Components Group Inc. (TBCOG) shall not be responsible for any delay due to from this design. Any failure to build the truss in conformance with ANSI/TP1-1 or for handling, shipping or installation, brace ng of trusses. Apply plates to each face of truss and post it on as shown above and on the Joist Details unless noted otherwise. Refer to drawing 160A-2 for standard plate positions. A seal on this drawing may appear as if it were a drawing and carries acceptance of project only and no other responsibility is assumed by TBCOG for its use. For more information see ANSI/TPI-1 Section 2. This job is the responsibility of the building designer per ANSI/TPI-1 Section 2. For more information see general notes page 17B-BGG www.tbcog.com TP1 www.tpinc.org WTCO www.abcdindustry.com CCC www.cccsteel.com



11/22/2013

TC LL	20 0 PSF	REF	R9114 - 28477
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	H00SR9114 13326032
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37.0 PSF	SEQN-	339827
DUR FAC	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

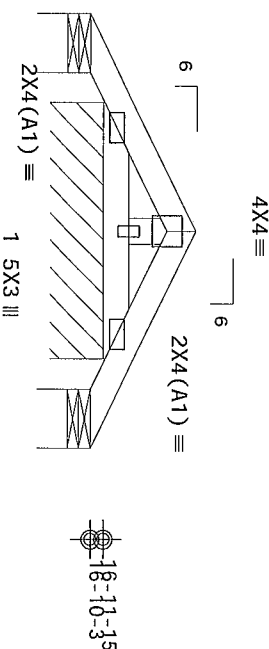
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MMFRS with additional C&C member design

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

MMWFRS loads based on trusses located at least 17 45 ft from roof edge

Refer to drawing PB160100212 for piggyback detail. Top chord of supporting truss under piggyback to be braced @ 24" O C, unless otherwise specified.



4-9-6 Over 3 Supports

2-9-12 1-4-14 1-4-14

0-11-13

R=19 U=5 W=7 826" (7 826 R=18.81 lb) 3 W=7 826 (7 826' min)

R=7/4 PLF U=3 PLF W=2-9-12

PLT TYP Wave

Design Crit.	FBC2010Res/TP1-2007(STD)
	FT/RT=10%(0%)/0(0)

12.03.04

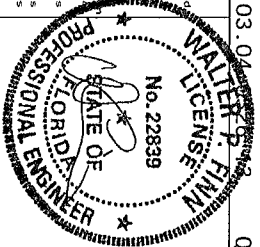
QTY:4 FL/-/5/-/-/R/-

Scale = .5"/Ft

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Orlando FL, 32837
FL COA #0278

[illegible]

11/22/2013

Special loads

-----Lumber	Dur	Fac = 1 25 /	Plate	Dur	Fac = 1 25)
TC-From	56 p1f at	0 00 to	56 p1f at	2 39	
TC-From	56 p1f at	2 39 to	56 p1f at	4 78	
BC-From	4 p1f at	0 00 to	4 p1f at	4 78	

120 mph wind, 17 45 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCp1(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

TC LL	20.0 PSF	REF	R9114- 28478
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326033
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN-	330906
DUR FAC.	1 25		
SPACING	24.0"	JREF-	1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City, FL - PBAT 4 9'6 Steepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MMFRS with additional C&C member design

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

MMFRS loads based on trusses located at least 17 35 ft from roof edge

Refer to drawing PB160100212 for piggyback detail Top chord of supporting truss under piggyback to be braced @ 24" OC, unless otherwise specified

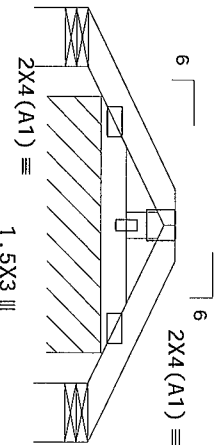
Special Loads

-----Lumber Dur Fac =1.25 / Plate Dur Fac =1.25
TC-From 56 pif at 0.00 to 56 pif at 2.39
TC-From 56 pif at 2.39 to 56 pif at 4.78
BC-From 4 pif at 0.00 to 4 pif at 4.78

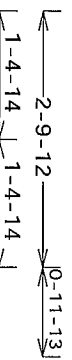
120 mph wind, 17 35 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf GCpl (+/-)=0.18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

4X4 ≡



18-10-35



4-9-6 Over 3 Supports

R=19 U=4 W=7 826" (7 826 R=7 826 W=7 826" min)
R=74 PLF U=2 PLF W=2-9-12

PLT TYP Wave Design Crit FBC2010Res/TP1-2007(STD)
RT/RT=10%(0%)/0(0)

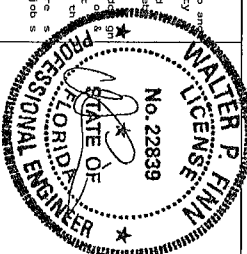
IMPORTANT READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses from re extreme care in fabricating and bracing. Refer to and follow the latest edition of BCS1 (Building Component Safety Information on by TP1 and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCS1. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r gide 1 ng. Locations shown for permanent lateral restraint of web.

1TW Building Components Group Inc. (1TWBCG) shall not be responsible for any deviation from this design drawing or cover page listing the design shown. The building designer shall not be responsible for the responsibility of the Building Designer per ASCE/TP1 1 Sec 2. For more information see this job's general notes page 1TW-BGC www twbcg com TP1 www tp net org WTCA www stc industry com

ALPINE

1TW Building Components Group Inc.
Orlando FL, 32837
FL COA #0278



TC LL	20 0 PSF	REF	R9114- 28479
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HCSR9114 13326034
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN-	330914
DUR FAC	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

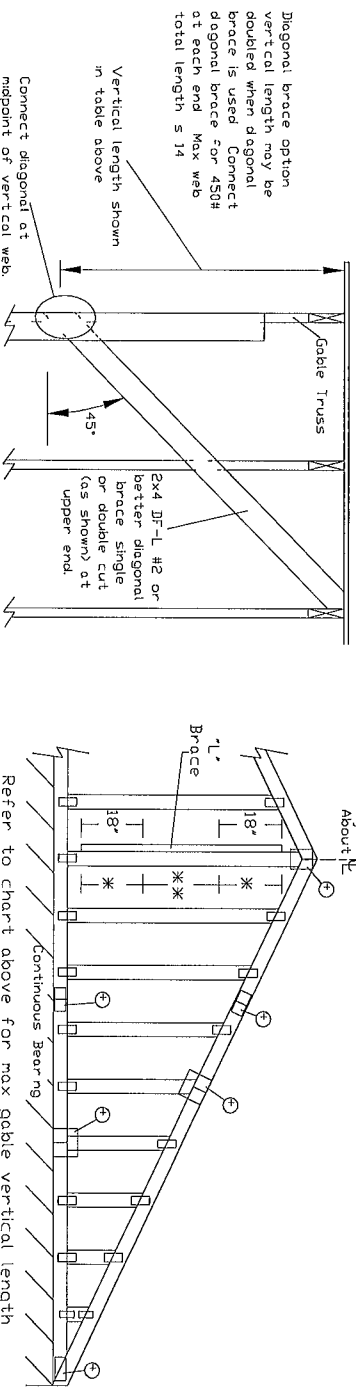
11/22/2013

ASCE 7-10 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 100

Dr 120 mph Wind Speed 15 Mean Height Partially Enclosed, Exposure C Kzt = 100
 Dr 120 mph Wind Speed 15 Mean Height Enclosed, Exposure D Kzt = 100
 Dr 100 mph Wind Speed 15 Mean Height Partially Enclosed, Exposure D Kzt = 100

Gable Stud Reinforcement Detail

Gable Vertical Species	2x4 Braces	Max Gable Vertical Length											
		Spacing	Grade	No	Braces	(1) 1x4 1'L' Brace	Group A	Group B	Group A	Group B	Group A	Group B	Group A
SPF	#1 / #2	12" o.c.	DFL	4 3'	7 3'	7 3'	7 7'	8 7'	8 11'	10 3'	10 8'	13 6'	14 0'
				4 1'	6 7'	7 1'	7 1'	8 6'	8 10'	10 1'	10 6'	13 4'	14 0'
				4 1'	7 2'	7 2'	7 5'	8 6'	8 10'	10 1'	10 6'	13 4'	14 0'
				4 1'	6 11'	7 5'	7 5'	8 6'	8 10'	10 1'	10 6'	13 4'	14 0'
SP	#1	16" o.c.	DFL	4 4'	7 4'	7 4'	7 7'	8 8'	9 0'	10 3'	10 8'	13 7'	14 0'
				4 3'	7 3'	7 3'	7 7'	8 7'	8 11'	10 3'	10 8'	13 6'	14 0'
				4 1'	5 11'	6 4'	7 11'	8 5'	10 1'	10 6'	12 5'	13 3'	14 0'
				4 1'	5 11'	6 4'	7 11'	8 5'	10 1'	10 6'	12 5'	13 3'	14 0'
SPF	#1 / #2	24" o.c.	DFL	4 11'	5 2'	5 5'	6 10'	7 4'	9 3'	9 11'	10 9'	11 6'	14 0'
				4 8'	8 1'	8 8'	9 10'	10 3'	11 8'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'
SP	#1	12" o.c.	DFL	4 11'	8 4'	8 8'	9 10'	10 3'	11 8'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'
SPF	#1 / #2	16" o.c.	DFL	4 11'	8 4'	8 8'	9 10'	10 3'	11 8'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'
SP	#1	24" o.c.	DFL	4 11'	8 4'	8 8'	9 10'	10 3'	11 8'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'
				4 8'	8 2'	8 6'	9 8'	10 1'	11 7'	12 1'	14 0'	14 0'	14 0'



Refer to chart above for max gable vertical length

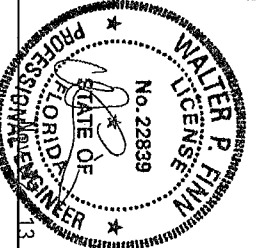


Building Components Group Inc.

Earth City, MO 63045

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and from the manufacturer's literature for detailed instructions. Trusses are not to be used for any other purpose unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs, shall have bracing installed per BCS sections B3, B7 or B10 as applicable. Apply plates to Refer to drawings 150A-2 for standard plate positions.

11W Building Components Group Inc. shall not be responsible for any deviation from this drawing any other drawings or specifications. Trusses shall conform with AISI/PSI 1 or for handling, shipping, installation & bracing of trusses. Trusses shall be braced in accordance with the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer. Refer to AISI/PSI 1 Sec.2 for more information see this job's general notes page and these web sites: 11W/BCG www.bcg.com, 11W www.11w.com, 11W www.11w.com, 11W www.11w.com, 11W www.11w.com



MAX TOT LD 60 PSF
 MAX SPACING 24 0"

Refer to the Building Designer for conditions not addressed by this detail

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4 0'	1x4 or 2x3
Greater than 4 0' but less than 11 6'	25x4
Greater than 11 6'	3x4

+ Refer to common truss design for peak splice and heel plates.

Attach 1" L' braces with 10d (0.128x3.0") nails
 * For (1) "L' brace space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.
 * For (2) "L' braces space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.
 "L' bracing must be a minimum of 80% of web member length.

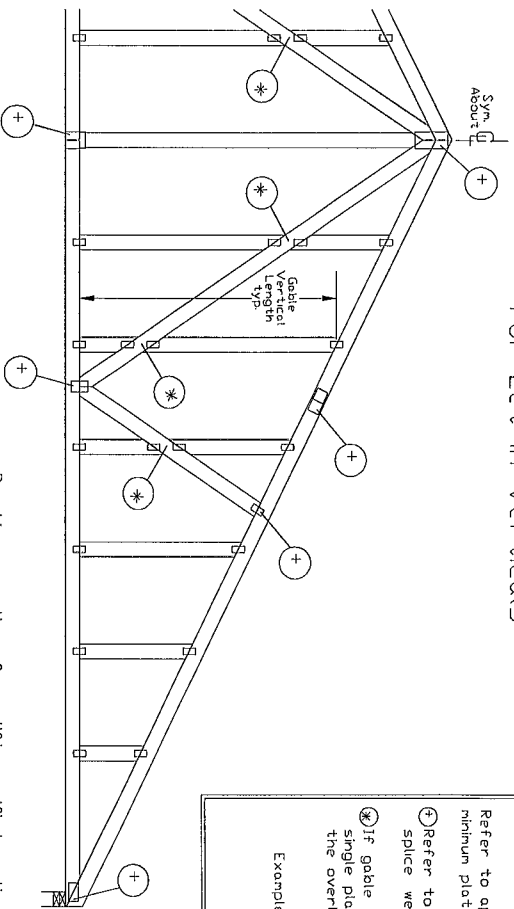
Gable Truss Detail Notes
 Wind Load deflection criterion is L/240
 Provide uplift connections for 55 psf over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4 0" outcroppers with 2 0" overhang, or 12" plywood overhang.
 So Pine lumber design values based on the ALSC January 2012 ruling

1x4 Braces shall be SR3 (Stress-Rated Board) ***For 1x4 So Pine use only Industrial S5 or Industrial S5 Stress-Rated Boards. Group B values may be used with these grades.

Group A		Group B	
Service-Pine-Fir	Heim-Fir	Service-Pine-Fir	Heim-Fir
#1 / #2 Standard	#2 Stud	#1 / #2 Standard	#2 Stud
#3 Stud		#3 Stud	

Group A		Group B	
Douglas Fir-Larch	Southern Pine***	Douglas Fir-Larch	Southern Pine***
#1 Standard	#2 Stud	#1 Standard	#2 Stud
#3 Stud		#3 Stud	

Gable Detail For Let-in Verticals



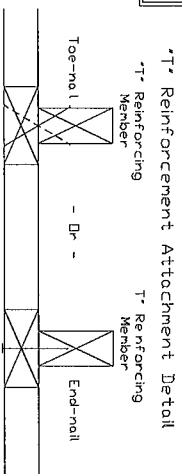
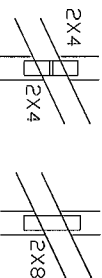
Gable Truss Plate Sizes

Refer to appropriate ITV gable detail for minimum plate sizes for vertical studs.

➊ Refer to Engineered truss design for peak splice web and heel plates

➋ If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web

Example



To convert from 'L' to 'T' reinforcing members multiply 'T' increase by length (based on appropriate ITV gable detail)

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

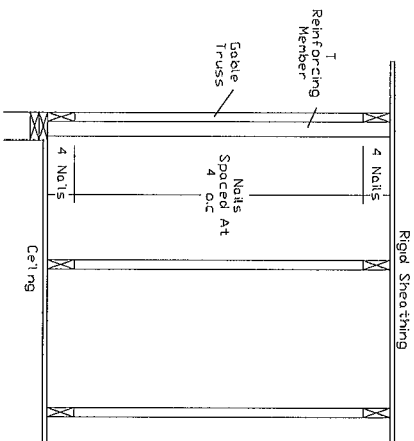
'T' reinforcing member material must match size specie and grade of the 'L' reinforcing member

Web Length Increase w/ 'T' Brace

'T' Reinf Mbr Size	'T' Increase
2x4	30 %
2x6	20 %

Example
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft Kzt = 1.00
Gable Vertical = 24'0" SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (from Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length 130 x 8' 7" = 11' 2"

See appropriate ITV gable detail for maximum unreinforced gable vertical length.



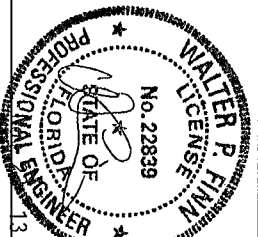
IMPORTANT READ AND FOLLOW ALL NOTES ON THIS DRAWING. ***WARNING*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.



Building Components Group Inc.

Earth City, MO 63045

ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure of the structure, or any damage to the structure or its contents. The user of this drawing assumes all liability for the design, construction, and use of the structure. A seal on this drawing or cover page listing the name and title of the professional engineer is required for the design. The liability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/FP 1 Sec-2. ITV BCS: www.itvbcg.com | P1: www.p1.com | B1: www.b1.com | L1: www.l1.com



MAX TOT LD 60 PSF
DUR FAC ANY
MAX SPACING 24'0"

REF LET-IN VERT
DATE 2/16/12
DRWG GBLETTIN0212

11/22/2013

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement
Alternative reinforcement specified in chart below may be conservative for minimum alternative reinforcement, re-run design with appropriate reinforcement type

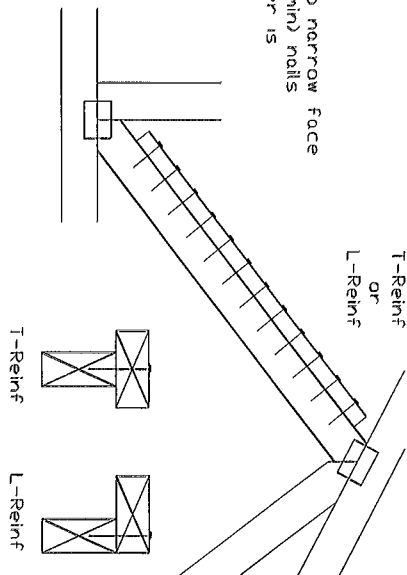
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row 2 rows	2x4 2x6	1-2x4 2-2x4
2x6	1 row 2 rows	2x4 2x6	1-2x6 2-2x4(*)
2x8	1 row 2 rows	2x6 2x8	1-2x8 2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web Apply (1) scab to each face of web

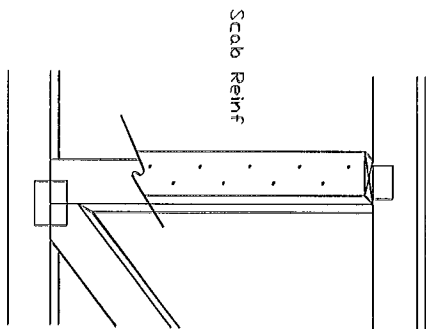
T-Reinforcement or L-Reinforcement

Apply to either side of web narrow face
Attach with 10d (0128"x30"min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length



Scab Reinforcement

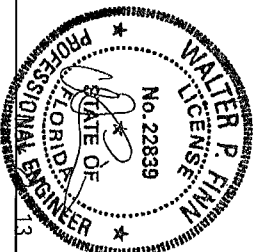
Apply scab(s) to wide face of web.
No more than (1) scab per face
Attach with 10d (0128"x30"min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length.



Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
TRUSSES REQUIRE EXTREME CARE IN REPAIRING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO AND FOLLOW THE LATEST EDITION OF BC31 (Building Component Safety Information, by ISI and SBCA) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. INSTALLERS SHALL PROVIDE TEMPORARY BRACING PER BC31. ALL TRUSSES SHALL BE PROPERLY ATTACHED TO CEILING LOCATIONS SHOWN FOR PERMANENT LATERAL RESTRAINT OF WEB. SHELL HAVE A PROPERLY ATTACHED PER BC31 SECTIONS B3, B7 OR B10, AS APPLICABLE. APPLY PLATES TO EACH FACE OF TRUSS AND POSITION AS SHOWN ABOVE AND ON THE JOINT DETAILS, UNLESS NOTED OTHERWISE.
REFER TO DRAWINGS FOR STANDARD PLATE POSITIONS.
ALL BRACING SHALL BE INSTALLED IN ACCORDANCE WITH AISC/ISI 1, OR FOR HANDLING, SHIPPING, INSTALLATION & BRACING OF TRUSSES.
A SEAL ON THIS DRAWING OR COVER PAGE LISTING THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING BY THE SIGNING ENGINEER. THE SIGNING ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN AND CONSTRUCTION OF ANY STRUCTURE IS THE RESPONSIBILITY OF THE SIGNING ENGINEER. SEE THE GENERAL NOTES PAGE AND THESE WEB SITES: www.bcsca.com www.isi.com www.aisc.org



TC LL	PSF	REF	CLR Subst
TC DL	PSF	DATE	8/15/13
BC DL	PSF	DRWG	BRCLBSUB0813
BC LL	PSF		
TOT LB	PSF		
DUR FAC			
SPACING			

11/22/2013

Wind Speed	Wind Dir	Mean Hgt	ASCE 7-10	Enclosed Bldg	located anywhere in roof	Exp C	Wind DL = 5.0 psf (min)	Kzt=1.0	
150 mph	Wind	3000 ft	Mean Hgt	ASCE 7-10	Enclosed Bldg	located anywhere in roof	Exp C	Wind DL = 5.0 psf (min)	Kzt=1.0
140 mph	Wind	3000 ft	Mean Hgt	ASCE 7-10	Enclosed Bldg	located anywhere in roof	Exp D	Wind DL = 5.0 psf (min)	Kzt=1.0

Note Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Diagram illustrating the placement of top chord purlins on a roof truss. The purlins are required at both ends and at 24" maximum on-center (o.c.) spacing in between. The diagram shows the purlin spacing as 24" o.c. max. The purlin is shown with dimensions 2, 4, and 2. The roof slope is indicated as 12°.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following (1) 3x8 Tru-lux plate attached with (8) 0120x1375 nails (4) into cap TC & (4) into base truss TC or (2) 28PB wave 0120x1375 plate attached to the P-gy/back truss TC and attached to the base truss TC with (4) 0120x1375 nails Note Nothing thru holes of wave plate is acceptable

Up to 12

12

Top Chord Scab (Typical Each End)

Full Chord Depth

Purlin Spacing > 24" o.c. max

Flat top chord purlins required at both ends purlin spacing > 24" o.c.

2 4 2

2 4 2

P gyo backstap top truss slant nailed to sl top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5")

* In addition provide connection with one of the following methods
<u>Trufox</u>
Use Trufox plates for 2x4 chord member and 3x10 Trufox plates for 2x6 and longer chord members. Attach to each face @ 8 o.c with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trufox plates may be staggered 4 o.c front to back faces
<u>APA Rated Gussset</u>
APA's x8"x7/16" (min) APA rated sheathing gusssets (8"x8" face) Attach @ 8 o.c with (3) 5d common (0.115"x2") nails per gussset (4) in cap bottom chord and (4) in base truss top chord. Gusssets may be staggered 4 o.c front to back faces
<u>2x4 Vertical Scabs</u>
2x4 SPF #2 full chord depth scabs (each face) Attach @ 8 o.c with (5) 10d box nails (0.128"x3") per scab (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4 o.c front to back faces
<u>2x6B Wave Piggyback Plate</u>
One 2x6B wave piggyback plate to each face of each o.c. Attach front to piggyback at time of field o.c. Attach back to piggyback at time of field o.c. 0.120"x1.375" nails per plate plus (4) o.c front to back faces

truss purlins must be installed at 24" oc max

Fiat top chord purlins required at both ends purlin spacing > 24' occur the fiat top of the base ax and use Detail A

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
 IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

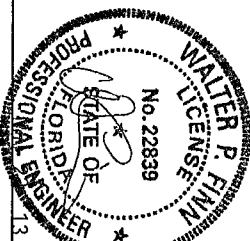


Building Components Group Inc.

Building Components Group Inc.

Earth City MO 63045

11 V Building Components Group Inc. shall not be responsible for construction from this drawing or failure to build the truss in conformance with ANSI/TPI-1 or for handling, shipping, installation or erection of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of the drawings for any structure is the responsibility of the building designer per ANSI/TPI-1 Sec 2.6. Drawing was developed by: www.trussco.com, info@trussco.com, www.trussco.net, vita.walsh@trussco.com, ltdc.walshtrussco.com



SPACING

240

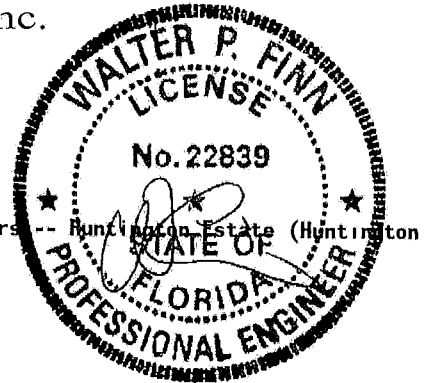
REF PIGGYBACK

DATE 2/14/12

DRWG PB160100212

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V31487-Z0115131106



01/15/2014

Walter P. Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Truss Fabricator **Anderson Truss Company**
Job Identification **13-281--Columbia County Bldrs Ass /Columbia County Builders**
Truss Count **1**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61015-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 130 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: 160TL-

#	Ref	Description	Drawing#	Date
1	76811-H3	repair 11'7"	14015003	01/15/14

ALPINE

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP 2850F-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with 13B" use design values approved
1/30/2013 by ALSC

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf,
wind BC DL=5.0 psf GCPI(+/-)=0.18

Wind loads and reactions based on MMFRS

In lieu of structural panels use purlins to brace all flat TC @ 24"

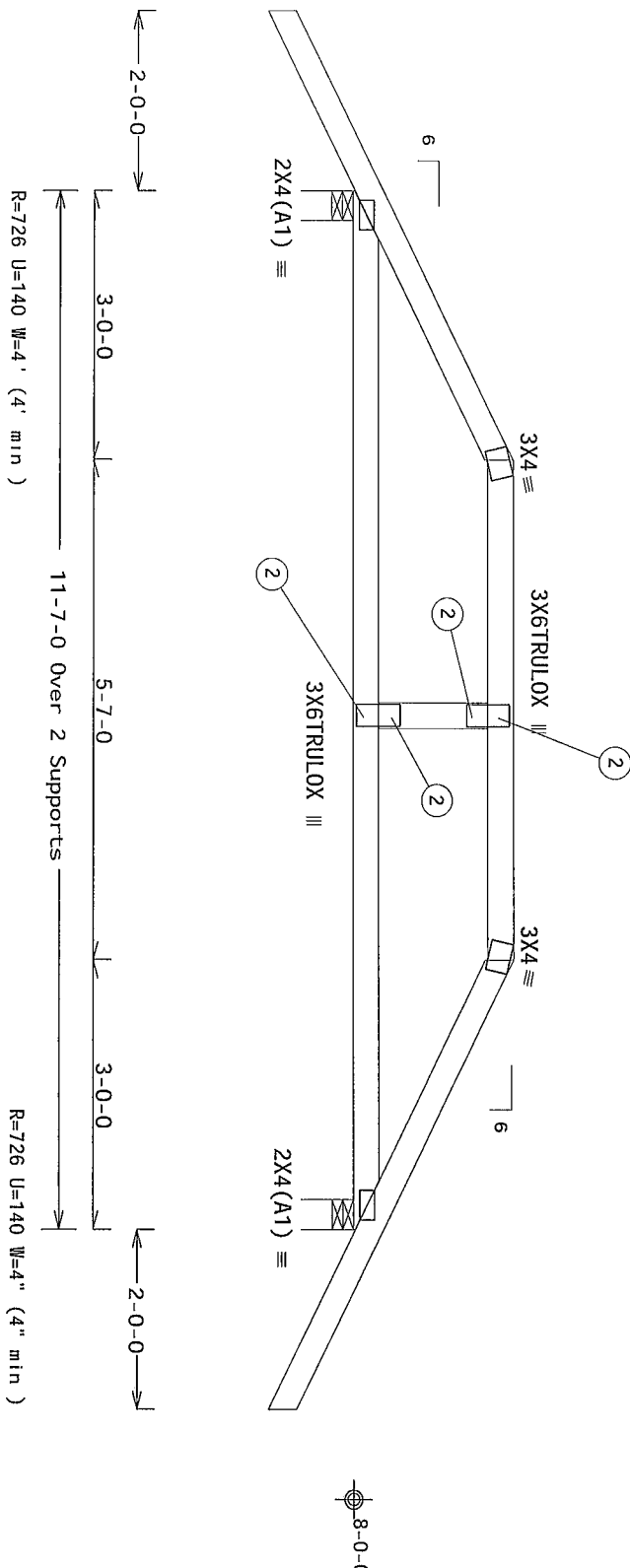
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1.50

Special loads

-----Lumber	Dur	Fac = 1.25 /	Plate	Dur	Fac = 1.25
TC-From	56 pif	at -2.00 to	56 pif	at 3.00	
TC-From	28 pif	at 3.00 to	28 pif	at 8.58	
TC-From	56 pif	at 8.58 to	56 pif	at 13.58	
BC-From	4 pif	at -2.00 to	4 pif	at 0.00	
BC-From	20 pif	at 0.00 to	20 pif	at 3.03	
BC-From	10 pif	at 3.03 to	10 pif	at 8.55	
BC-From	20 pif	at 8.55 to	20 pif	at 11.58	
BC-From	4 pif	at 11.58 to	4 pif	at 13.58	
TC-70.58 lb Conc	Load at	3.03, 8.55			
TC-43.90 lb Conc	Load at	5.06, 6.52			
BC-111.48 lb Conc	Load at	3.03, 8.55			
BC-46.51 lb Conc	Load at	5.06, 6.52			

Bottom chord checked for 10.00 psf non-concurrent live load

11 GAUGE (0.120")X1.375" nails required for trulox plate attachment
Nails specified in circles must be applied to each face of each truss
ply See DWG 1601L for nailing and trulox plate requirements



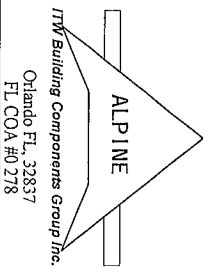
PLT TYP Wave, Trulox

Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

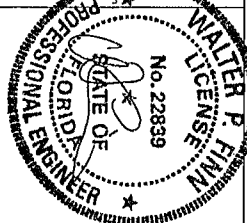
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QTY: 1 FL/-/5/-/-/R/-

Scale = .5"/Ft.



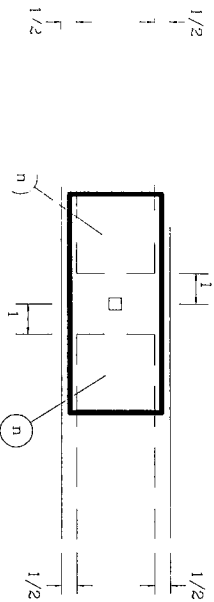
IMPORTANT READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to any follow the latest edition of BCSI Building Component Safety Information by TPI and WTC for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed rigid to ceiling. Locate one shown for permanent lateral restraint of web.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installing or bracing the truss. ITWBCG shall not be responsible for any damage to the truss or any other structure or property caused by the use of this design. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information on this job's structural needs page ITW BCS www.bcsinc.com TPI www.tpinet.org WTC www.stcindustry.com ITCA www.stcindustry.com



TC LL	20.0 PSF	REF	R9114- 76811
TC DL	7.0 PSF	DATE	01/15/14
BC DL	10.0 PSF	DRW	HCSR9114 14015003
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT. LD.	37.0 PSF	SEON-	24462
DUR. FAC.	1.25		
SPACING	24.0"	JREF	1V31487_Z01

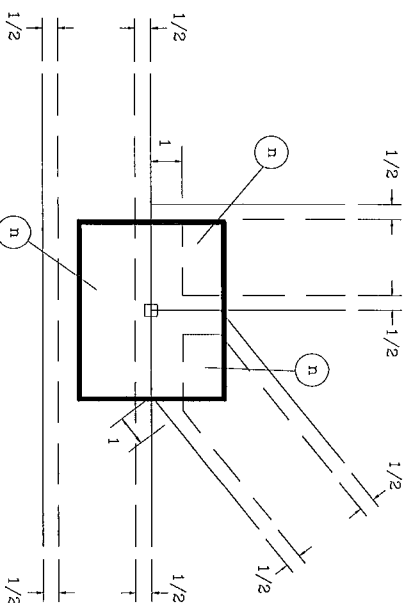
TRULOX INFORMATION DETAIL

TYPICAL OFF PANEL SPLICE

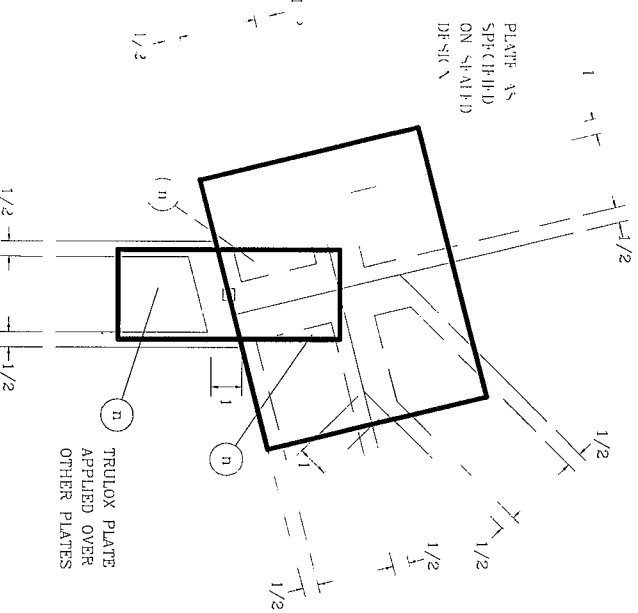


DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE AS SHOWN BY DASHED LINES
NAILS MUST NOT SPLIT LUMBER

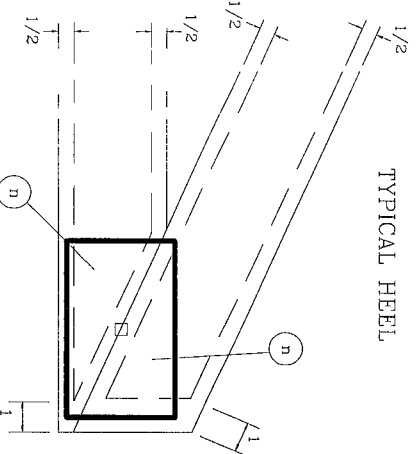
TYPICAL PANEL POINT WITHOUT SPLICE



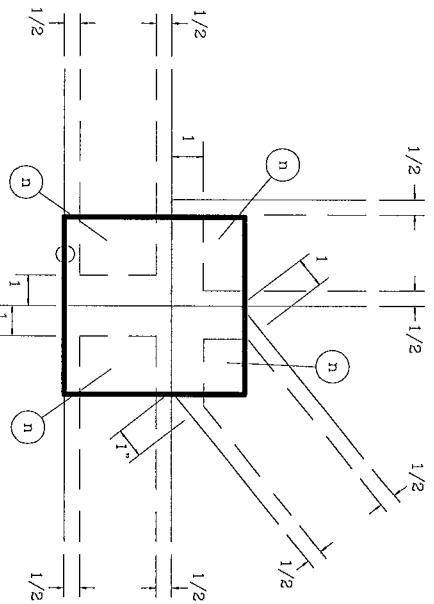
TYPICAL FILLER



TYPICAL HEEL



TYPICAL PANEL POINT SPLICE



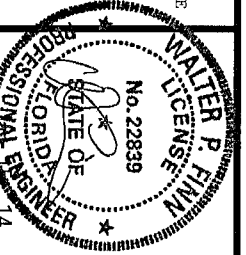
TRULOX PLATING



Building Components Group Inc.

Earth City, MO 63045

NOTES
(n) IS THE REQUIRED NUMBER OF 0.120 X 1.375 NAILS OR EQUAL, PER FACE PER PLATE AS SPECIFIED ON THE SEALED DESIGN REFERENCE THIS DETAIL.
○ LOCATES PLATE CORNER OR FLUSH EDGE
□ LOCATES PLATE CENTER



160

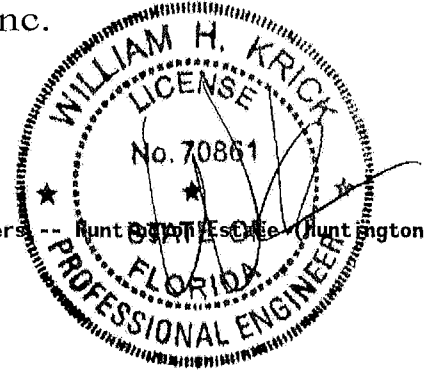
TT

PAGE 1 OF 1
DATE 4/1/09

01/15/2014

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID IV31487-Z0415133529



Truss Fabricator **Anderson Truss Company**
Job Identification **13-281--Columbia County Bldrs Ass /Columbia County Builders** -- Hunt & Hunt
Truss Count **1**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61015-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

01/15/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: -

#	Ref	Description	Drawing#	Date
1	81579-EJ7Cr 7 ¹	End Jac	14015027	01/15/14



Top	chord	2x4	SP	#1-13B
Bot	chord	2x4	SP	#1-13B
	Wbs	2x4	SP	#3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

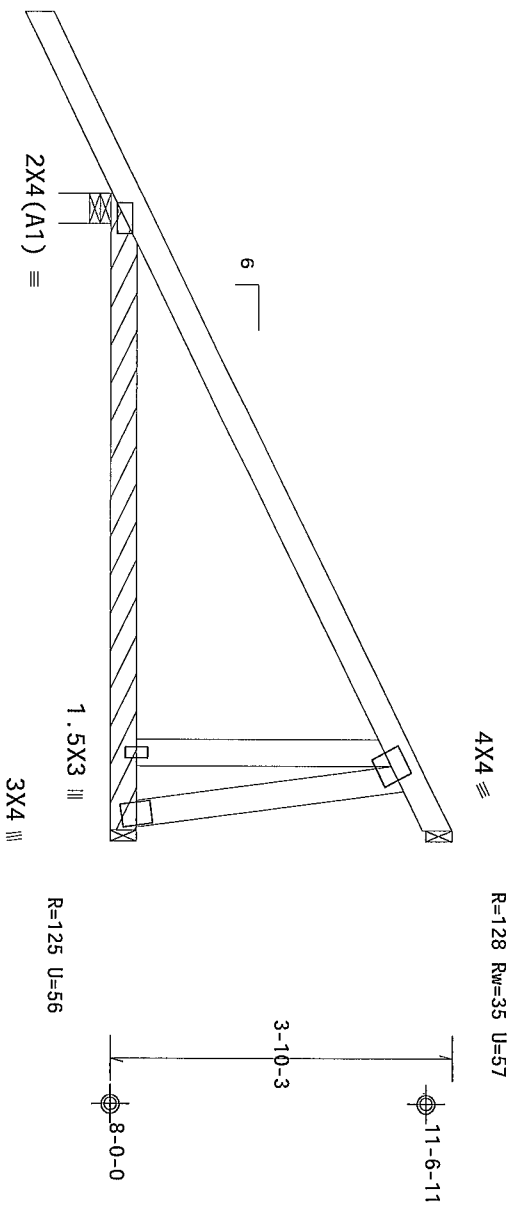
(1) 2x4X7'-0-0 SP #1-13B Bottom chord scab centered 3'-6-0 from left end Attach to one face of chord with (2) rows of 0 131"x3", min nails @ 6 0 C, staggered 3"

Provide (2) 16d common nails(0 162"x3 5"), toe nailed at Top chord
Provide (2) 16d common nails(0 162"x3 5"), toe nailed at Bot chord

120 mph wind, 15.00 ft mean hgt ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf $G C p 1 (+/-)=0.18$

Deflection meets $L/240$ live and $L/180$ total load Creep increases factor for dead load is 1.50

MMFRS loads based on trusses located at least 7 50 ft from roof edge



7-0-0 Over 3 Supports (4' min)

PLT TYP	Wave	Design Cnt	1203040326.13	QTY:1	FL/-/5/-/-/R/-	Scale = 5"/ft
	FT/RT=10% (0%)/0 (0)	1203040326.13				

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLER

ALPINE

ITV Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

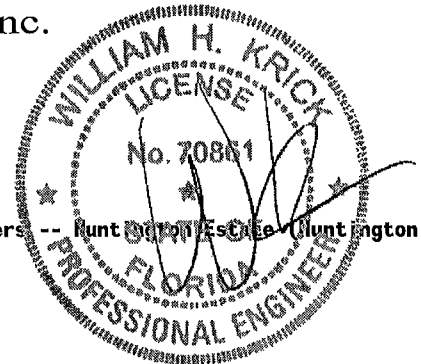
01/15/2014

TC LL	20.0 PSF	REF	R9114- 81579
TC DL	7.0 PSF	DATE	01/15/14
BC DL	10.0 PSF	DRW	HGUSR9114 14015027
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	24814
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V31487_Z04

Permit # 31631

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V31487-Z0415133529



Truss Fabricator **Anderson Truss Company**
Job Identification. **13-281--Columbia County Bldrs Ass /Columbia County Builders -- Hunt Don Escabe /Huntington**
Truss Count **1**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record. **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61G15-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

01/15/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: -

#	Ref	Description	Drawing#	Date
1	81579-EJ7Cr	7' End Jac	14015027	01/15/14

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10.00 psf non-concurrent live load.

(1) 2x4X7-0-0 SP #1-13B Bottom chord scab centered 3-6-0" from left end. Attach to one face of chord with (2) rows of 0.131"x3", min. nails @ 6" O.C., staggered 3".

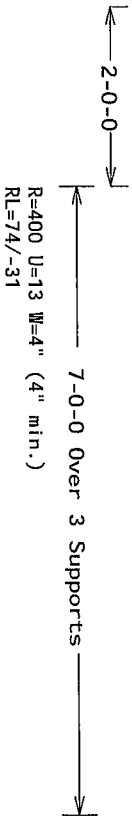
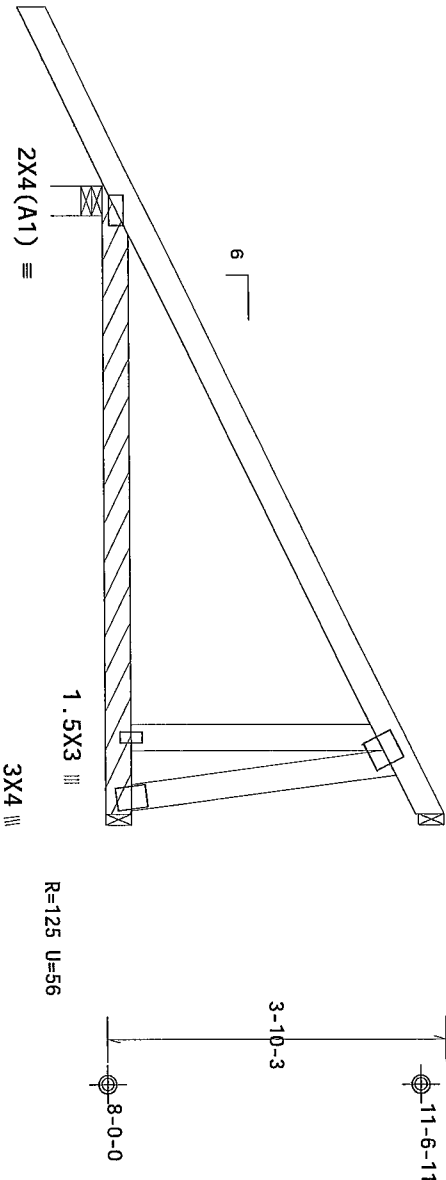
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord. Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.



R=400 U=13 W=4" (4" min.)
RL=74/-31

PLT TYP. Wave

Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

12 03 04 0326.13

QTY:1

FL/-/5/-/-/R/-

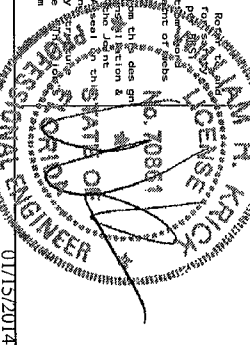
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ALPINE

Alpine Building Components Group Inc.

Orlando FL 32837
FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating handling shipping installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information by TPI and WTC) for practices prior to performing these functions. Installers shall provide temporary bracing unless noted otherwise. Top chord shall have properly attached structural sheathing and bracing. All other members shall have properly attached structural sheathing and bracing. All members shall have proper bracing installed per BCSI sections 83, 87 or 810 as applicable.
TPI Building Components Group Inc. (TPI/BCG) shall not be responsible for any delay or damage to the project caused by the contractor's failure to follow the design or specifications. The contractor shall be responsible for obtaining all necessary permits and for obtaining all necessary approvals from the local building department. The contractor shall be responsible for obtaining all necessary approvals from the local building department. The contractor shall be responsible for obtaining all necessary approvals from the local building department.
TPI www.tpi.net WTC www.wtcindustry.com
ICC www.iccsafe.org

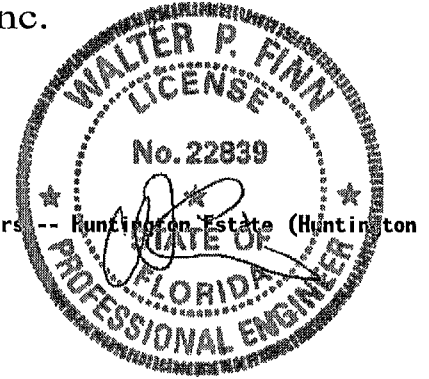


TC LL	20.0 PSF	REF	R9114- 81579
TC DL	7.0 PSF	DATE	01/15/14
BC DL	10.0 PSF	DRW	HCUSR9114 14015027
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	24814
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V31487_Z04

Permit # 31631

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V31487-Z0115131106



Truss Fabricator **Anderson Truss Company**
Job Identification. **13-281--Columbia County Bldrs Ass /Columbia County Builders -- Huntington Estate (Huntington**
Truss Count. **1**
Model Code **Florida Building Code 2010**
Truss Criteria. **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61G15-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 130 MPH ASCE 7-10 -Closed

01/15/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: 160TL-

#	Ref	Description	Drawing#	Date
1	76811-H3	repair 11'7"	14015003	01/15/14

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP 2850F-2.3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 4.50 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3.5 psf,
wind BC DL=5.0 psf. GCPI (+/-)=0.18

Wind loads and reactions based on MMFRS.

In lieu of structural panels use purlins to brace all flat TC @ 24"

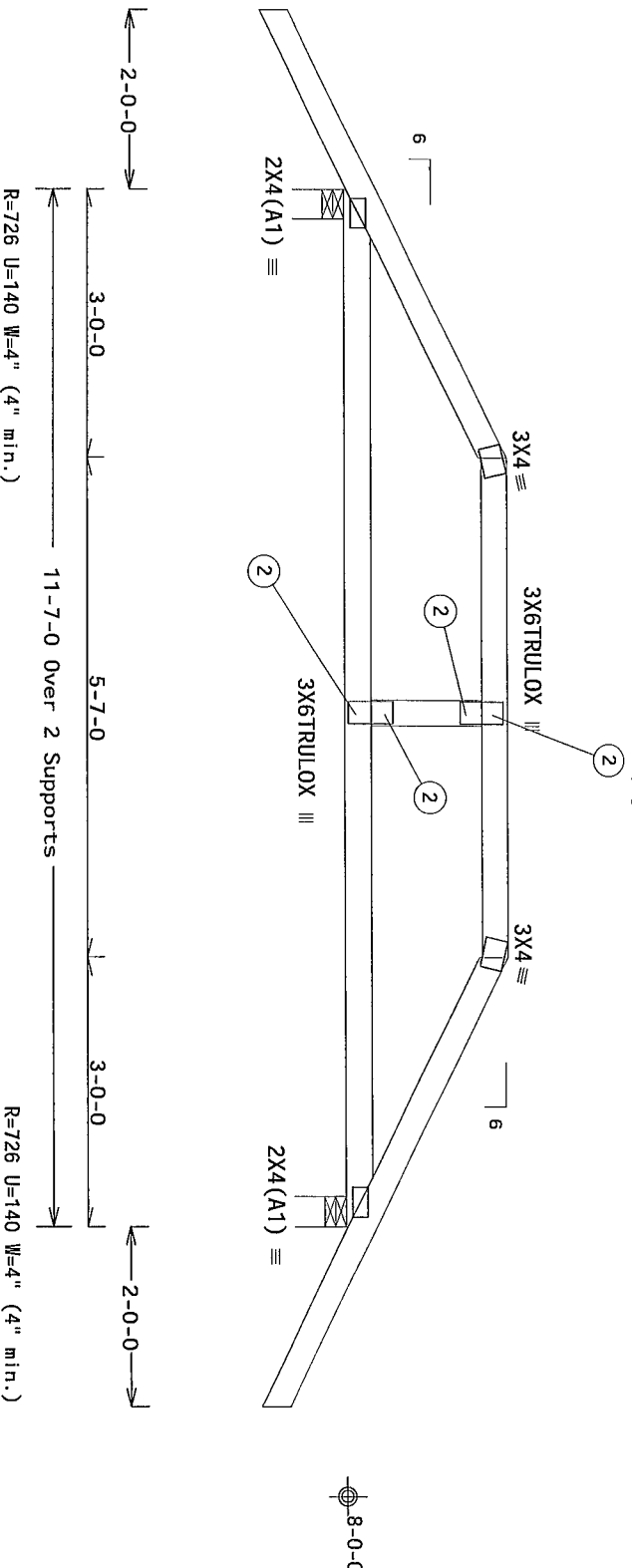
Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50

Special loads

TC-From	Dur.Fac.=1.25 / Plate Dur.Fac.=1.25	56 pif at -2.00 to 3.00
TC-From	56 pif at -2.00 to 3.00	28 pif at 8.58
TC-From	56 pif at 8.58 to 13.58	4 pif at 0.00
BC-From	20 pif at -2.00 to 3.03	10 pif at 8.55
BC-From	20 pif at 3.03 to 8.55	4 pif at 13.58
BC-From	4 pif at 11.58 to 13.58	4 pif at 13.58
TC-70.58 lb Conc.	Load at 3.03, 8.55	
TC-43.90 lb Conc.	Load at 5.06, 6.52	
BC-111.48 lb Conc.	Load at 3.03, 8.55	
BC-46.51 lb Conc.	Load at 5.06, 6.52	

Bottom chord checked for 10.00 psf non-concurrent live load.

11 GAUGE (0.120")X1.375" nails required for trulox plate attachment.
Nails specified in circles must be applied to each face of each truss
ply See DWG 160TL for nailing and trulox plate requirements.



PLT TYP. Wave, Trulox

Design Crit: FBC2010Res/TP1-2007(STD)
FT/RT=10%(%) / 0(0)

12.03.04.0326.13

QTY:1

FL/-/5/-/R/-

Scale = .5"/Ft.

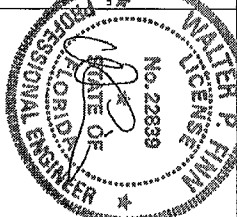
IMPORTANT READ AND FOLLOW ALL NOTES ON THIS SHEET

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTC) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Trusses shall be braced in accordance with the bracing locations shown for permanent lateral restraint of web. Trusses shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any delay or damage to the building or any other structure caused by the use of this design. The user of this design shall be responsible for the building's safety and use of this design shall be the responsibility of the Building Designer per ANSI/TP1 1 Sec 2. For more information see the general notes page ITW-BCG www.tbcbg.com www.sp.net.org WTC www.structure.com
ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278

ALPINE

Orlando FL 32837
FL COA #0278

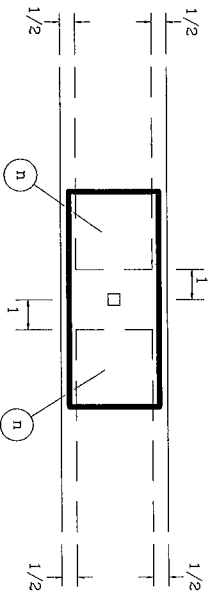


01/15/2014

TC LL	20.0 PSF	REF R9114- 76811
TC DL	7.0 PSF	DATE 01/15/14
BC DL	10.0 PSF	DRW HCUSR9114 14015003
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEQN- 24462
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1V31487_Z01

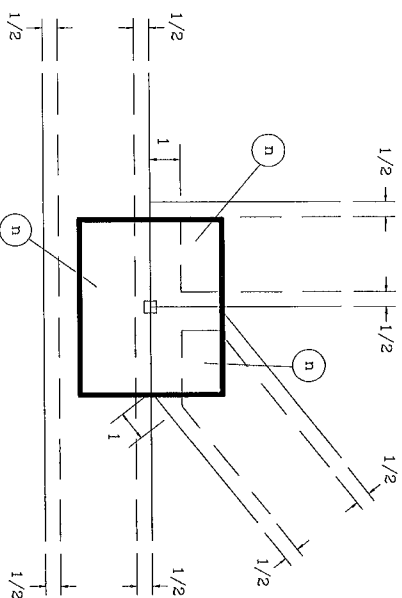
TRULOX INFORMATION DETAIL

TYPICAL OFF PANEL SPLICE

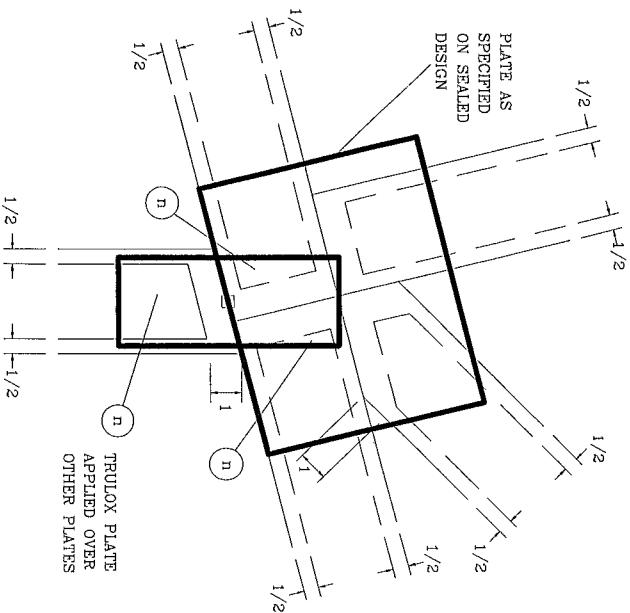


DO NOT APPLY NAILS WITHIN 1/2 OF LUMBER EDGES OR 1 OF LUMBER ENDS ON EACH FACE AS SHOWN BY DASHED LINES
NAILS MUST NOT SPLIT LUMBER

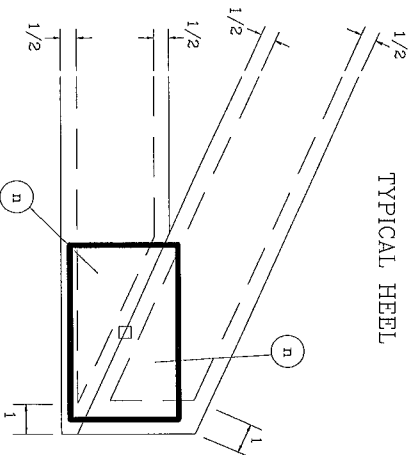
TYPICAL PANEL POINT WITHOUT SPLICE



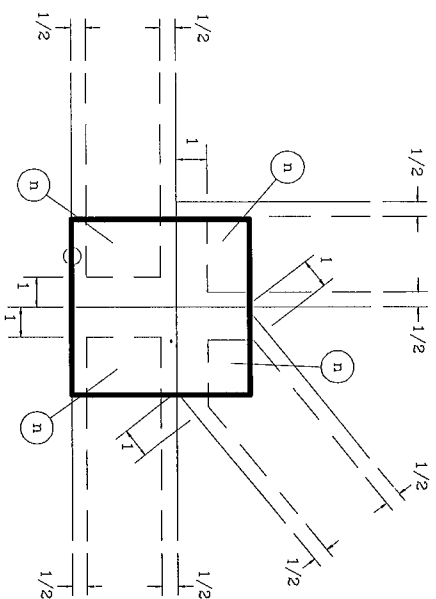
TYPICAL FILLER



TYPICAL HEEL



TYPICAL PANEL POINT SPLICE



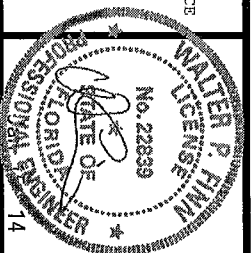
TRULOX PLATE
APPLIED OVER
OTHER PLATES

NOTES

- (n) IS THE REQUIRED NUMBER OF 0.120 X 1.375 NAILS OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL
- LOCATES PLATE CORNER OR FLUSH EDGE
- LOCATES PLATE CENTER



Earth City MO 63045



01/15/2014

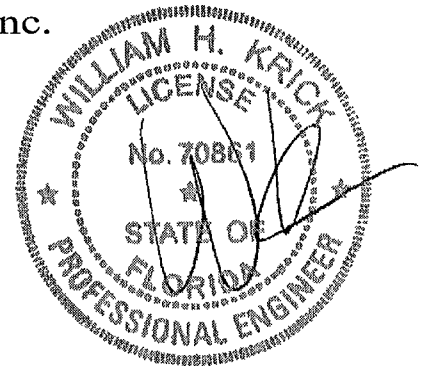
TRULOX PLATING

160
TL

PAGE 1 OF 1
DATE 4/1/09

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
 Florida Engineering Certificate of Authorization Number: 0 278
 Florida Certificate of Product Approval # FL1999
 Page 1 of 1 Document ID 1V329114Z0215110638



01/15/2014

Truss Fabricator: Anderson Truss Company
 Job Identification: REPAIR / 13-281 -H9 -BRYAN ZE (REPAIR / 13 281-)
 Truss Count: 1
 Model Code: Florida Building Code 2010
 Truss Criteria: FBC2010Res/TPI 2007(STD)
 Engineering Software: Alpine Software, Version 13.02.
 Structural Engineer of Record: The identity of the structural EOR did not exist as of
 Address: the seal date per section 61615-31.003(5a) of the FAC
 Minimum Design Loads: Roof - 37.0 PSF @ 1.25 Duration
 Floor - N/A
 Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: -

William H Krick
 -Truss Design Engineer-

1950 Marley Drive
 Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	82924--H9	Jack Scab	14015002	01/15/14

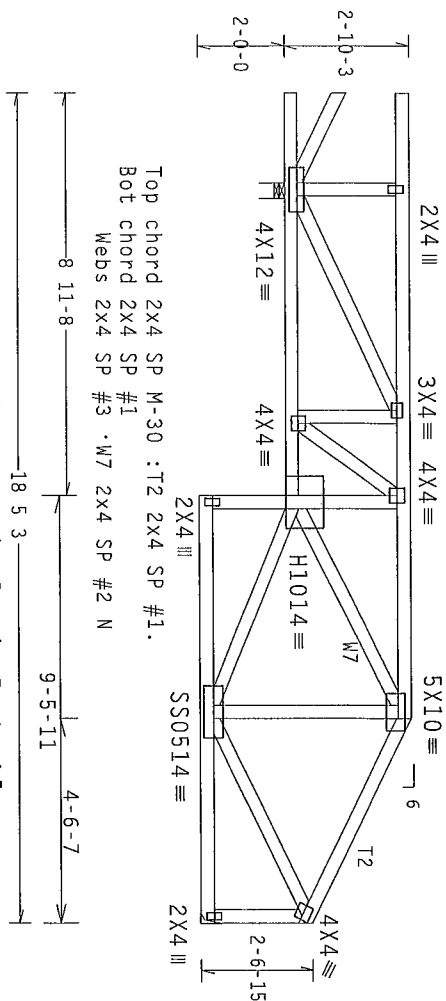
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

FIELD REPAIR MUST COMPLY WITH ALPINE DESIGNS AND SPECIFICATIONS
REFER TO DRAWING HQUSP9114 13326028 FOR LUMBER, PLATES AND OTHER
DATA NOT GIVEN HERE

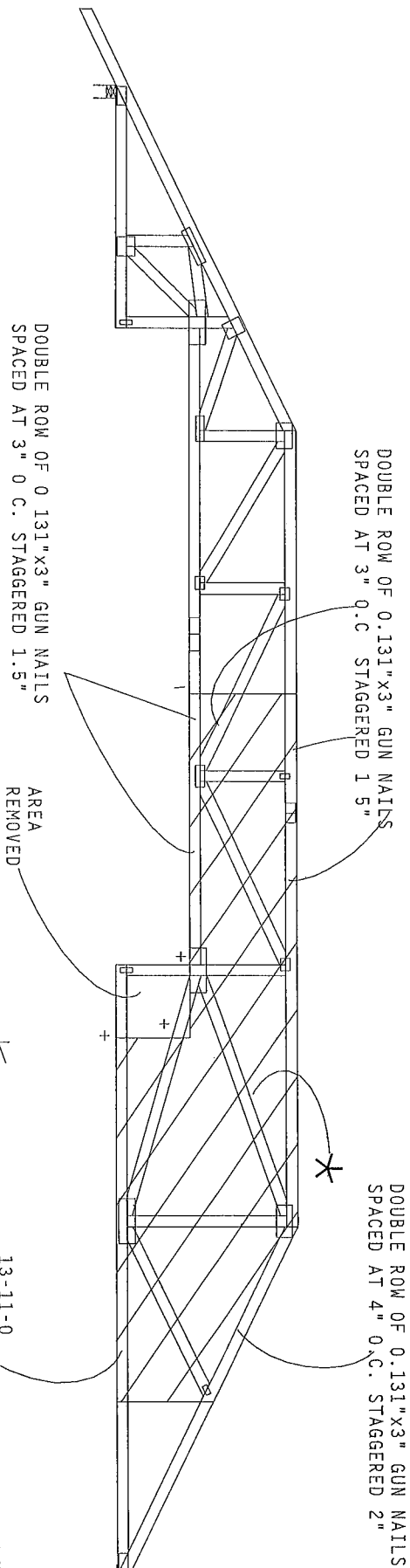
NOTE PRIOR TO AND DURING THE REPAIR OPERATION, THIS TRUSS AND ANY SUPPORTED SPANS MUST BE TEMPORARILY BRACED AND SHORED THE DESIGN AND POSITIONING OF THIS BRACING AND SHORING TO BE DESIGNED BY OTHERS.

+ USE A SHARP METAL CUTTING SAW BLADE TO CAREFULLY REMOVE UNWANTED MATERIAL FROM THE TRUSS AS SHOWN DASHED PORTIONS OF TRUSS MUST BE FREE FROM DAMAGE.

* ONE (1) JACK SCAB, CHORUS AND WEBS AS SHOWN IN DETAIL ONE (1) JACK SCAB REQUIRED ATTACH JACK SCAB TO ONE FACE OF TUBS WITH A DOUBLE ROW OF 0 131"x3" GIN NAILS SPACED AT 24" O C STAGGERED 12" THROUGHOUT MATCHING MEMBERS, UNLESS OTHERWISE NOTED REFER TO DRAWING CNNALLSP0109 FOR ADDITIONAL MAIL SPACING INFORMATION.



* 18-5-3 Jack Scab Detail



PLT TYP. 20 Gauge HS, 18 Gauge HS, Design Crit FBC2010Res/TPI-2007(STD)

Wave

$$F_T/RT=10\%(0\%)/0(0)$$

13 02 00 0926 18

QTY:0

FL/-/3/-/-/R/-

Scale = .25" / Ft.

TRUSS REPAIR

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

DAMAGED TROUSERS MUST BE CAREFULLY EVALUATED TO DETERMINE THE EXTENT OF DAMAGE AND THE FEASIBILITY OF REPAIR. IN SOME CASES, THE PRUDENT SOLUTION IS TO SCRAP THE DAMAGED TROUSERS AND REBUILD. INTERNAL, WOOD FIBER, DAMAGE AND EXCESSIVE CONNECTED STRESS FROM REMOVAL OR SHOCK, CANNOT BE READILY DETECTED. THEREFORE, IT IS VITAL THAT THE TROUSER MANUFACTURER AND BUILDING CONTRACTOR, CONSIDER THE CAUSE OF THE DAMAGE IN THEIR DECISION WHETHER TO REPAIR OR REBUILD.

DOUBLE ROW OF 0.131"x3" GUN NAILS
SPACED AT 4" O.C. STAGGERED 2"

Scale = .25" / Ft.

TC LL	20.0 PSF	REF	R9114- 82924
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TC DL	7.0 PSF	DATE	01/15/14
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BC DL	10.0 PSF	DRW	HCUSR9114	14015002
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BC LL	0 0 PSF	HC-ENG WHK/WHK
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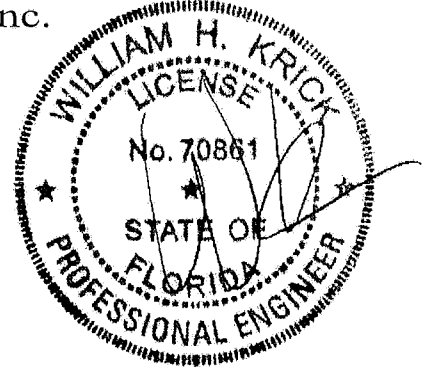
TOT LD	37 0 PSF	SEQN -	5139
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DUR.FAC 1.25

SPACING 24.0" JREF- 1V329114Z02

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V329114Z0215110638



Truss Fabricator **Anderson Truss Company**
Job Identification **REPAIR / 13-281 -H9 -BRYAN ZE (REPAIR / 13-281-)**
Truss Count **1**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 13.02.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61615-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

01/15/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: -

#	Ref	Description	Drawing#	Date
1	82924--H9	Jack Scab	14015002	01/15/14

ALPINE

(REPAIR / 13-281 -H9 -BRYAN ZE - H9 Jack Scab)

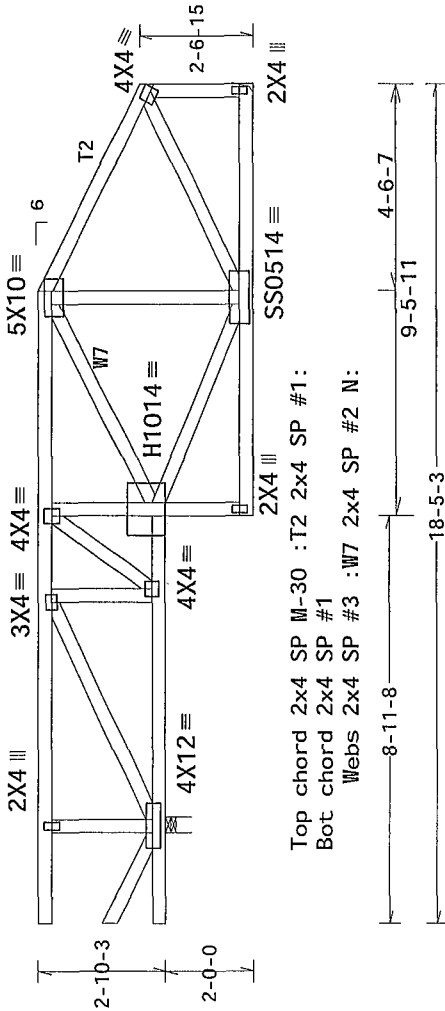
REPAIR TO CHANGE EXTERIOR GEOMETRY OF TRUSS SHOWN BELOW

FIELD REPAIR MUST COMPLY WITH ALPINE DESIGNS AND SPECIFICATIONS
REFER TO DRAWING HCUSR9114 13326028 FOR LUMBER, PLATES AND OTHER
DATA NOT GIVEN HERE

NOTE PRIOR TO AND DURING THE REPAIR OPERATION, THIS TRUSS AND
ANY SUPPORTED SPANS MUST BE TEMPORARILY BRACED AND SHORED
THE DESIGN AND POSITIONING OF THIS BRACING AND SHORING TO
BE DESIGNED BY OTHERS

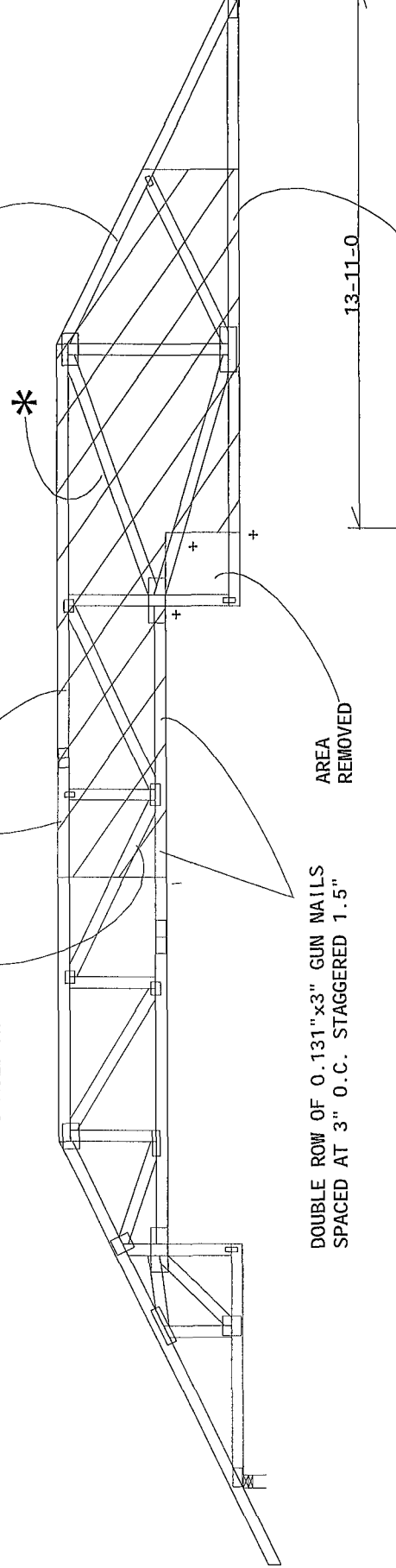
+ USE A SHARP METAL CUTTING SAW BLADE TO CAREFULLY REMOVE
UNWANTED MATERIAL FROM THE TRUSS AS SHOWN DASHED REMAINING
PORTIONS OF TRUSS MUST BE FREE FROM DAMAGE

* ONE (1) JACK SCAB CHORDS AND WEBS AS SHOWN IN DETAIL ONE
(1) JACK SCAB REQUIRED ATTACH JACK SCAB TO ONE FACE OF
TRUSS WITH A DOUBLE ROW OF 0.131"x3" GUN NAILS SPACED AT
24" O.C. STAGGERED 12" THROUGHOUT MATCHING MEMBERS, UNLESS
OTHERWISE NOTED REFER TO DRAWING CNAILSP0109 FOR ADDITIONAL
NAIL SPACING INFORMATION



* 18-5-3 Jack Scab Detail

DOUBLE ROW OF 0.131"x3" GUN NAILS
SPACED AT 3" O.C. STAGGERED 1.5"



DOUBLE ROW OF 0.131"x3" GUN NAILS
SPACED AT 3" O.C. STAGGERED 1.5"

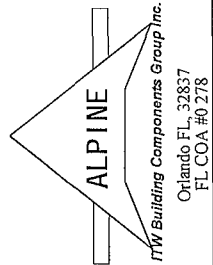
DOUBLE ROW OF 0.131"x3" GUN NAILS
SPACED AT 4" O.C. STAGGERED 2"

PLT TYP 20 Gauge HS, 18 Gauge HS, Design Crit: FBC2010Res/TPI-2007(STD)
Wave FT/RT=10%(0%)/0(0)

TRUSS REPAIR

DAMAGED TRUSSES MUST BE CAREFULLY EVALUATED TO DETERMINE THE EXTENT OF DAMAGE
AND THE FEASIBILITY OF REPAIR. IN SOME CASES THE PRUDENT SOLUTION IS TO SCRAP
THE DAMAGED TRUSSES AND REBUILD. INTERNAL WOOD FIBER DAMAGE AND EXCESSIVE
CONNECTOR STRESS FROM BENDING OR SHOCK CANNOT BE READILY DETECTED. THEREFORE
IT IS VITAL THAT THE TRUSS FABRICATOR AND BUILDING CONTRACTOR CONSIDER THE
CAUSE OF THE DAMAGE IN THEIR DECISION WHETHER TO REPAIR OR REBUILD.

REPAIR WORK SHOWN ON THIS DRAWING APPLIES ONLY TO THOSE SECTIONS OF THE TRUSS
INSPECTED BY THE TRUSS MANUFACTURER TO HAVE BEEN DAMAGED. A QUALIFIED THIRD PARTY
INSPECTOR SHALL CHECK TRUSSES TO DETERMINE THE EXTENT OF ANY FURTHER DAMAGE. IF ANY
AND VERIFY THAT REPAIRS HAVE BEEN PERFORMED AS INDICATED ON THIS DRAWING.



13.02.00.00.0926.18

QTY:0 FL/-/3/-/-/R/- Scale =.25"/Ft.

TC LL	20.0 PSF	REF	R9114- 82924
TC DL	7.0 PSF	DATE	01/15/14
BC DL	10.0 PSF	DRW	HCUSR9114 14015002
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	5139
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V329114Z02

