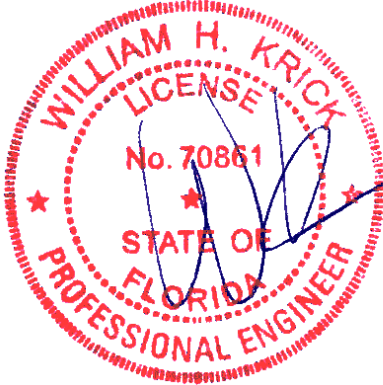




Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com



This item has been digitally signed by William H. Krick on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

COA #0 278

Florida Certificate of Product Approval #FL 1999

06/11/2024



Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 24-1260
Job Description: BARBER	
Address: FL	

Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res. HVHZ	IntelliVIEW Version: 23.02.04 JRef #: 1Y0M2150004
Wind Standard: ASCE 7-22 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 11 truss drawing(s) and 2 detail(s).

Item	Drawing Number	Truss
1	163.24.1349.59783	A01
3	163.24.1350.02187	A03
5	163.24.1350.05877	B02
7	163.24.1350.09330	B04
9	163.24.1350.13557	B06
11	163.24.1350.17287	PB02
13	PB160220723	

Item	Drawing Number	Truss
2	163.24.1350.01037	A02
4	163.24.1350.04273	B01
6	163.24.1350.07730	B03
8	163.24.1350.11090	B05
10	163.24.1350.15047	PB01
12	BRCLBSUB0119	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C-TW = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

General Notes (continued)

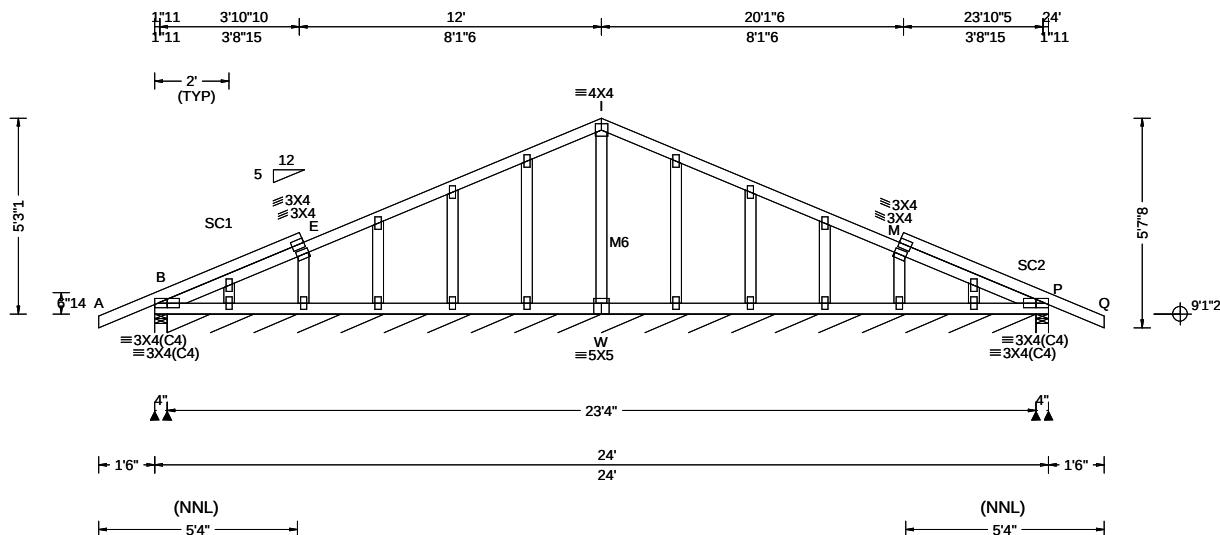
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com

SEQN: 767232 FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 24-1260 BARBER Truss Label: A01	Cust: R 215 JRef: 1Y0M2150004 T2 DrwNo: 163.24.1349.59783 SSB / FV 06/11/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 P 999 240 VERT(CL): 0.002 P 999 180 HORZ(LL): 0.002 M - - HORZ(TL): 0.002 M - - Creep Factor: 2.0 Max TC CSI: 0.239 Max BC CSI: 0.037 Max Web CSI: 0.730 VIEW Ver: 23.02.04.0123.14	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 280 /- /- /158 /60 /140 B* 69 /- /- /37 /12 /- P 280 /- /- /177 /60 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) B Brg Wid = 280 Min Req = - P Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B, B, & P are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; M6 2x4 SP #2;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/195.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.

The overall height of this truss excluding overhang is 53'-1.



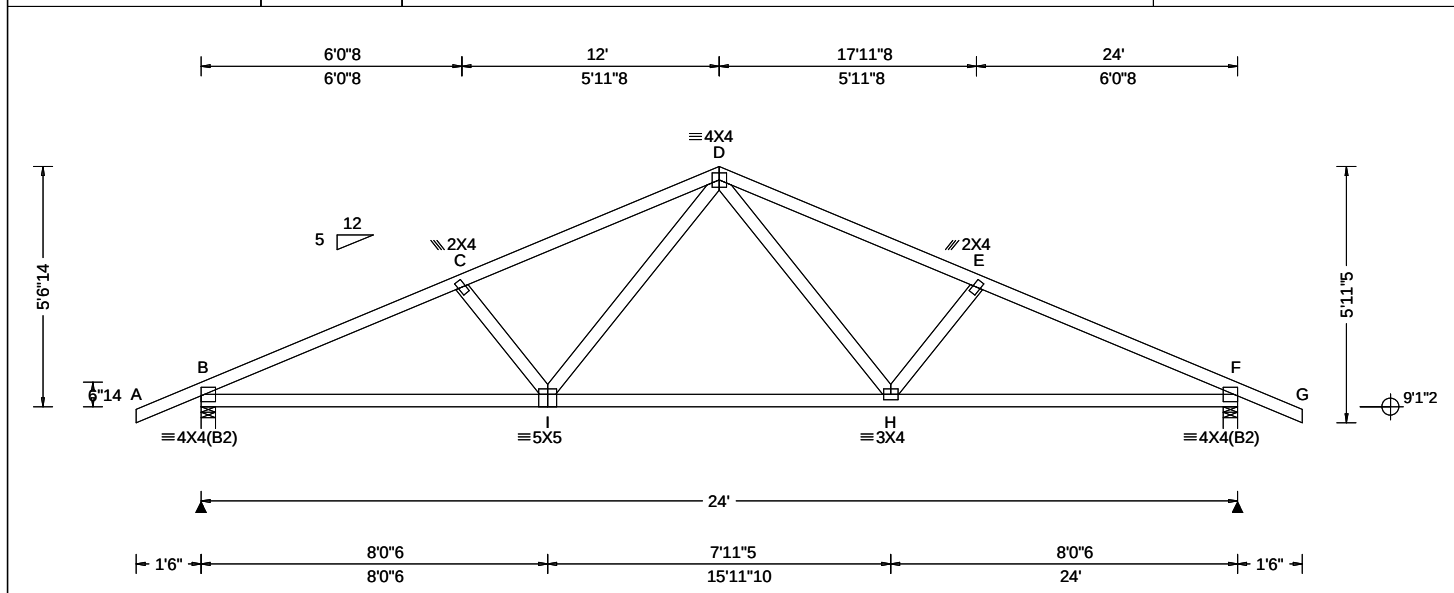
COA #0278

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 767235 FROM: CDM	COMN Ply: 1 Qty: 6	Job Number: 24-1260 BARBER Truss Label: A02	Cust: R 215 JRef: 1Y0M2150004 T1 DrwNo: 163.24.1350.01037 SSB / FV 06/11/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.103 H 999 240 VERT(CL): 0.206 H 999 180 HORZ(LL): 0.042 F - - HORZ(TL): 0.084 F - - Creep Factor: 2.0 Max TC CSI: 0.638 Max BC CSI: 0.743 Max Web CSI: 0.188 VIEW Ver: 23.02.04.0123.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1079 /- /- /619 /201 /142 F 1079 /- /- /619 /201 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 735 -1803 D - E 692 -1589 C - D 693 -1588 E - F 734 -1804

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 5-6-14.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - I	1587 -563	H - F	1588 -567
I - H	1111 -301		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
I - D	489 -167	D - H	492 -165



COA #0278

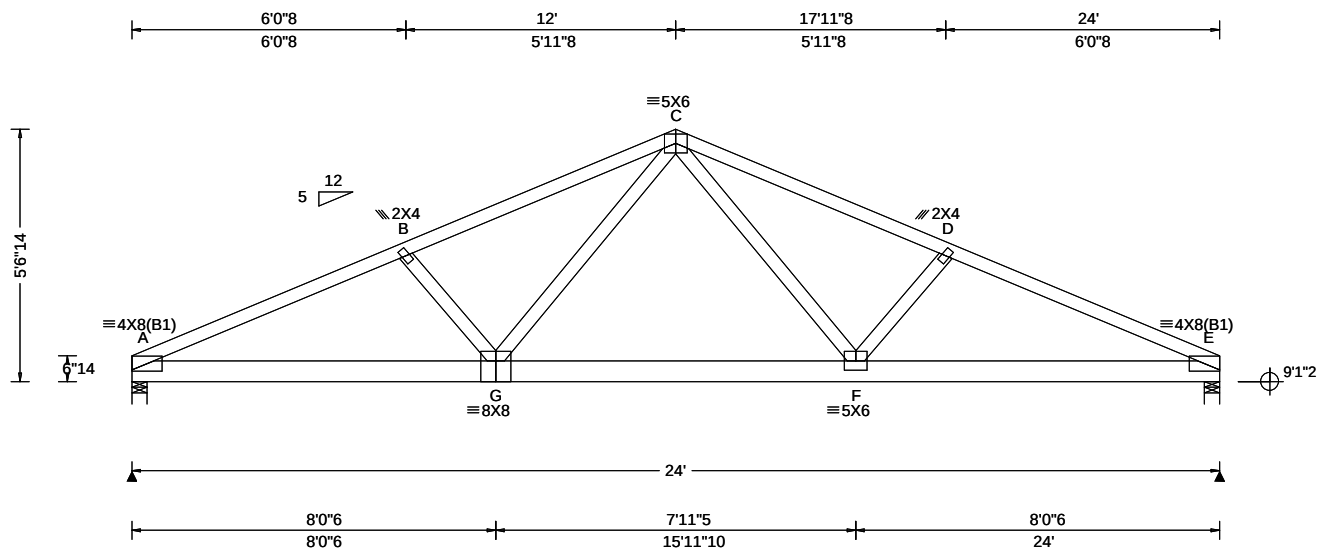
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 767238 FROM: CDM	COMN Ply: 1 Qty: 1	Job Number: 24-1260 BARBER Truss Label: A03	Cust: R 215 JRef: 1Y0M2150004 T10 DrwNo: 163.24.1350.02187 SSB / FV 06/11/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.175 F 999 240 VERT(CL): 0.336 F 846 180 HORZ(LL): 0.038 B - - HORZ(TL): 0.073 B - - Creep Factor: 2.0 Max TC CSI: 0.960 Max BC CSI: 0.505 Max Web CSI: 0.736 VIEW Ver: 23.02.04.0123.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 2568 -/- /- /- /541 -/ E 2550 -/- /- /- /538 -/ Wind reactions based on MWFRS A Brg Wid = 4.0 Min Req = 2.1 (Truss) E Brg Wid = 4.0 Min Req = 2.1 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 1029 -4774 C - D 943 -4566 B - C 943 -4563 D - E 1029 -4777

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Special Loads

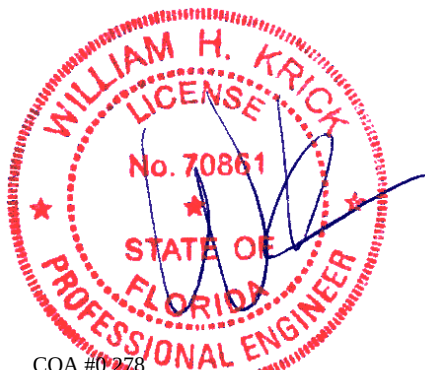
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 62 plf at 0.00 to 62 plf at 24.00
BC: From 10 plf at 0.00 to 10 plf at 24.00
BC: 309 lb Conc. Load at 1.94, 3.94, 5.94, 7.94
9.94, 11.94, 13.94, 15.94, 17.94, 19.94, 21.94

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 5-6-14.

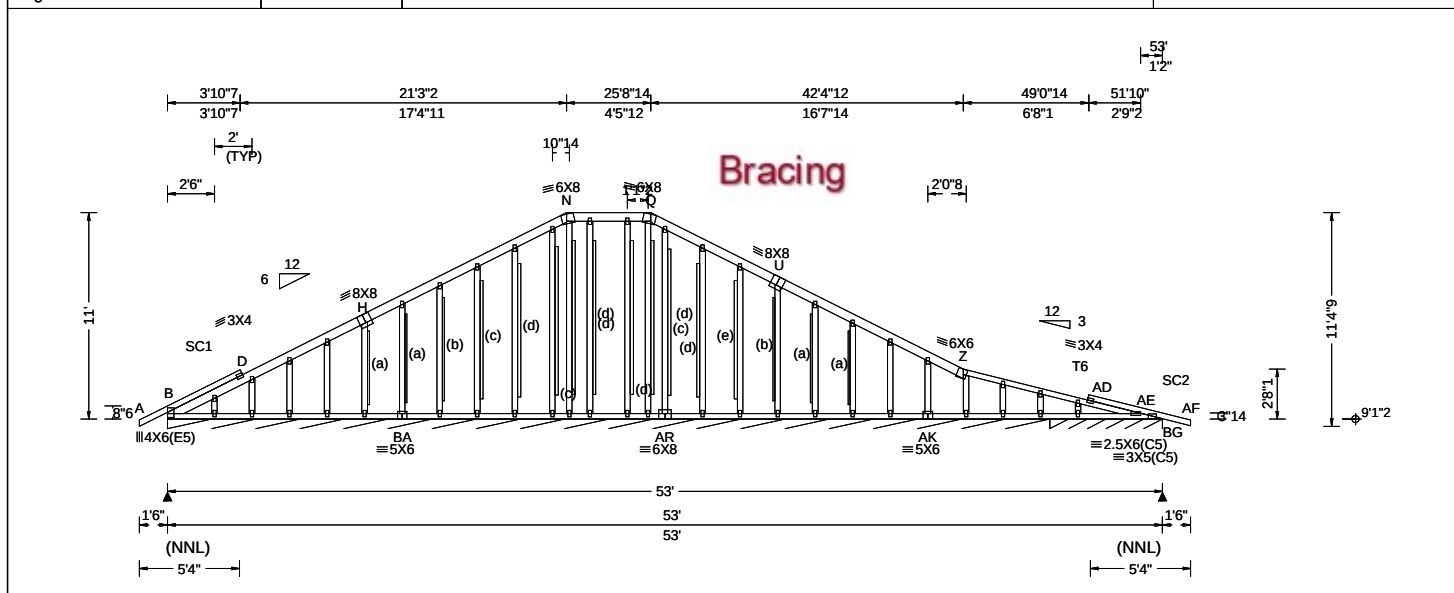


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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.13 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.30 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.004 AE 999 240 VERT(CL): 0.010 AE 999 180 HORZ(LL): 0.009 V - - HORZ(TL): 0.010 V - - Creep Factor: 2.0 Max TC CSI: 0.257 Max BC CSI: 0.094 Max Web CSI: 0.997 VIEW Ver: 23.02.04.0123.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B* 85 - / - / 45 / 12 / 7 BG*93 - / - / 51 / 15 / - Wind reactions based on MWFRS B Brg Wid = 564 Min Req = - BG Brg Wid = 72.0 Min Req = - Bearings B & AH are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. H - N 486 -132 Q - U 484 -54 N - Q 456 -50

Lumber

Top chord: 2x6 SP #2; T6 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/176.



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SEQN: 767221	GABL	Ply: 1	Job Number: 24-1260	Cust: R 215 JRef: 1Y0M2150004 T4
FROM: CDM		Qty: 1	BARBER	DrwNo: 163.24.1350.04273
Page 2 of 2			Truss Label: B01	SSB / FV 06/11/2024

Gable Reinforcement

- (a) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (b) 1x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (c) 2x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (d) 2x6 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (e) 2x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 11-0-0.



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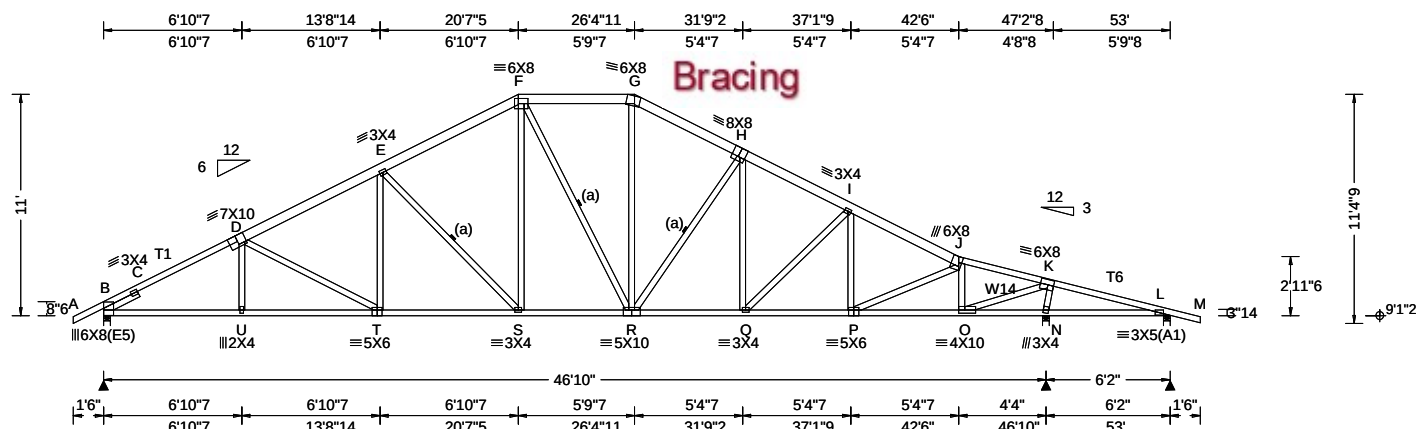
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SEQN: 767201 FROM: CDM	COMN Ply: 1 Qty: 6	Job Number: 24-1260 BARBER Truss Label: B02	Cust: R 215 JRef: 1Y0M2150004 T16 DrwNo: 163.24.1350.05877 SSB / FV 06/11/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.29 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.30 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.222 S 999 240 VERT(CL): 0.419 S 999 180 HORZ(LL): 0.101 O - - HORZ(TL): 0.191 O - - Creep Factor: 2.0 Max TC CSI: 0.850 Max BC CSI: 0.917 Max Web CSI: 0.901 VIEW Ver: 23.02.04.0123.14	Gravity Loc R+ / R- / Rh / Rw / U / RL B 2164 -/- /- /1221 /254 /321 N 2775 -/- /- /1460 /322 -/- L 133 -/230 -/- /39 /115 -/- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 2.6 (Truss) N Brg Wid = 4.0 Min Req = 2.9 (Truss) L Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B, N, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x6 SP #2; T1,T6 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W14 2x4 SP #2;
Lt Slider: 2x4 SP #3; block length = 1.914'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

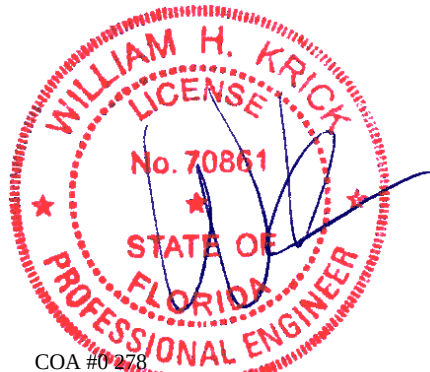
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
Negative reaction(s) of -230# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
The overall height of this truss excluding overhang is 11-0-0.

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	1040 -3801	G - H	813 -2606
C - D	1031 -3750	H - I	901 -3016
D - E	937 -3348	I - J	939 -3192
E - F	810 -2701	J - K	739 -2301
F - G	790 -2277	K - L	1401 -357

Chords	Tens.Comp.	Chords	Tens. Comp.
B - U	3281 -813	Q - P	2812 -594
U - T	3283 -806	P - O	2351 -629
T - S	2909 -556	O - N	468 -1544
S - R	2323 -234	N - L	392 -1337
R - Q	2619 -419		

Webs	Tens.Comp.	Webs	Tens. Comp.
D - T	280 -409	R - H	413 -621
T - E	442 -60	P - J	527 -92
E - S	468 -850	J - O	451 -1340
F - S	871 -274	O - K	3991 -1115
R - G	786 -302	N - K	927 -2600



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Glenview, IL 60025

[illegible]

Lumber	Maximum Top Chord Forces Per Ply (lbs)					
Top chord: 2x6 SP #2; T1,T7 2x4 SP #2;	Chords	Tens.Comp.	Chords	Tens.	Comp.	
Bot chord: 2x4 SP #2;	B - C	628 -438	I - J	247	-1442	
Webs: 2x4 SP #3;	C - D	594 -72	J - K	349	-1779	
Lt Slider: 2x4 SP #3; block length = 1.718'	D - E	1027 -6	K - L	303	-1011	

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 11-0-0.

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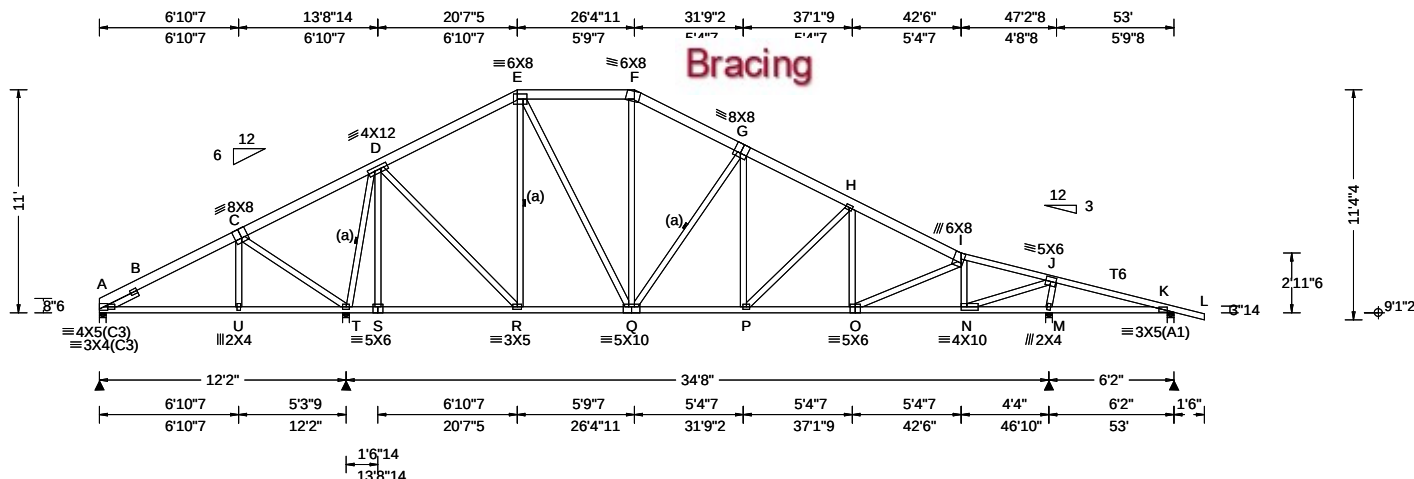
For more information see these web sites: Alpine: alpineitw.com; TPI: tointst.org; SBCCI: sbccacomponents.com; ICC: iccsafe.org; AWC: awc.org

Maximum Bot Chord Forces Per Ply (lbs)					
Chords	Tens.Comp.		Chords	Tens. Comp.	
B - AC	149	-495	W - V	1629	-33
AC-AB	146	-497	V - T	1750	-151
AB-AA	192	-943	S - R	1521	-188
Z - Y	660	0	R - Q	122	-611
Y - X	1314	0	Q - O	148	-716
X - W	1305	0			

Maximum Web Forces Per Ply (lbs)					
Webs	Tens.Comp.		Webs	Tens. Comp.	
D -AB	188	- 624	W - I	491	- 272
E -AB	227	-1827	W - J	294	- 450
E -AA	1387	- 50	T - S	1553	- 189
AA- F	89	-1298	S - L	100	- 391
F - Z	950	0	M - R	214	- 567
G - Z	0	- 855	R - N	2102	- 357
G - Y	1061	- 34	N - O	370	- 1600



SEQN: 767207 FROM: CDM	COMN Ply: 1 Qty: 1	Job Number: 24-1260 BARBER Truss Label: B04	Cust: R 215 JRef: 1Y0M2150004 T9 DrwNo: 163.24.1350.09330 SSB / FV 06/11/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.29 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.30 ft Loc. from endwall: not in 13.00 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.074 P 999 240 VERT(CL): 0.152 P 999 180 HORZ(LL): 0.023 N - - HORZ(TL): 0.046 N - - Creep Factor: 2.0 Max TC CSI: 0.604 Max BC CSI: 0.455 Max Web CSI: 0.886 VIEW Ver: 23.02.04.0123.14	Gravity Loc R+ / R- / Rh / Rw / U / RL A 344 - / - / - /150 /17 /295 T 2220 - / - / - /1348 - / - M 1804 - / - / - /1101 /3 - K 261 - /19 - / - /98 /87 - Non-Gravity Wind reactions based on MWFRS A Brg Wid = 4.0 Min Req = 1.5 (Truss) T Brg Wid = 4.0 Min Req = 2.2 (Truss) M Brg Wid = 4.0 Min Req = 1.8 (Truss) K Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings A, T, M, & K are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x6 SP #2; T6 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.819'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

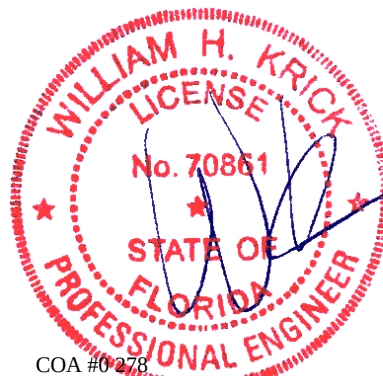
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

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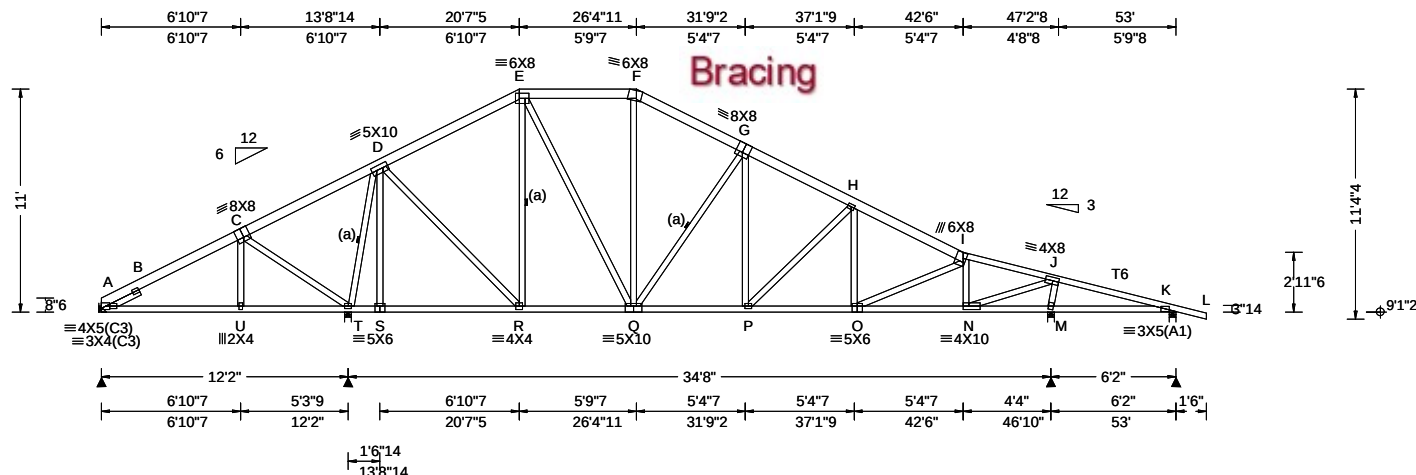


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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.29 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.30 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.089 P 999 240 VERT(CL): 0.170 P 999 180 HORZ(LL): 0.026 N - - HORZ(TL): 0.050 N - - Creep Factor: 2.0 Max TC CSI: 0.625 Max BC CSI: 0.527 Max Web CSI: 0.959 VIEW Ver: 23.02.04.0123.14	Gravity Loc R+ / R- / Rh / Rw / U / RL A 309 - / - / - /136 /55 /295 T 2521 - / - / - /1367 /236 - /- M 1917 - / - / - /1098 /241 - /- K 244 - /35 - / - /99 /87 - /- Wind reactions based on MWFRS A Brg Wid = - Min Req = - T Brg Wid = 4.0 Min Req = 2.6 (Truss) M Brg Wid = 4.0 Min Req = 1.9 (Truss) K Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings T, M, & K are a rigid surface. Members not listed have forces less than 375#

Lumber
Top chord: 2x6 SP #2; T6 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.819'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
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Maximum Top Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
A - B	386 -339	G - H	583 -1767
C - D	678 -67	H - I	653 -2069
D - E	368 -983	I - J	583 -1680
E - F	487 -1092	J - K	673 -175
F - G	477 -1292		

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
R - Q	791 0	O - N	1686 -462
Q - P	1501 -128	N - M	274 -768
P - O	1797 -335	M - K	216 -632

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
C - T	332 -612	G - P	481 -150
T - D	482 -2169	P - H	290 -409
D - R	1201 -72	I - N	319 -812
E - R	116 -598	N - J	2518 -747
E - Q	660 -195	M - J	714 -1761
Q - G	442 -728		



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Glenview, IL 60025

SEQN: 767204	COMN	Ply: 1	Job Number: 24-1260	Cust: R 215 JRef:1Y0M2150004 T6
FROM: CDM		Qty: 11	BARBER	DrwNo: 163.24.1350.11090
Page 2 of 2			Truss Label: B05	SSB / FV 06/11/2024

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

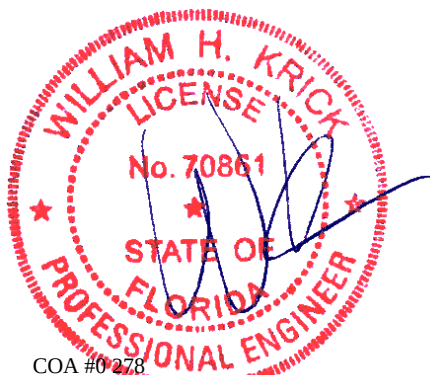
Bearing at location $x=0'$ uses the following support conditions: 0'

Bearing A (0', 9'1"2) LUS26

Supporting Member: (1)2x6 SP 2400f-2.0E

(4) 0.148"x3" nails into supporting member,

(3) 0.148"x3" nails into supported member.



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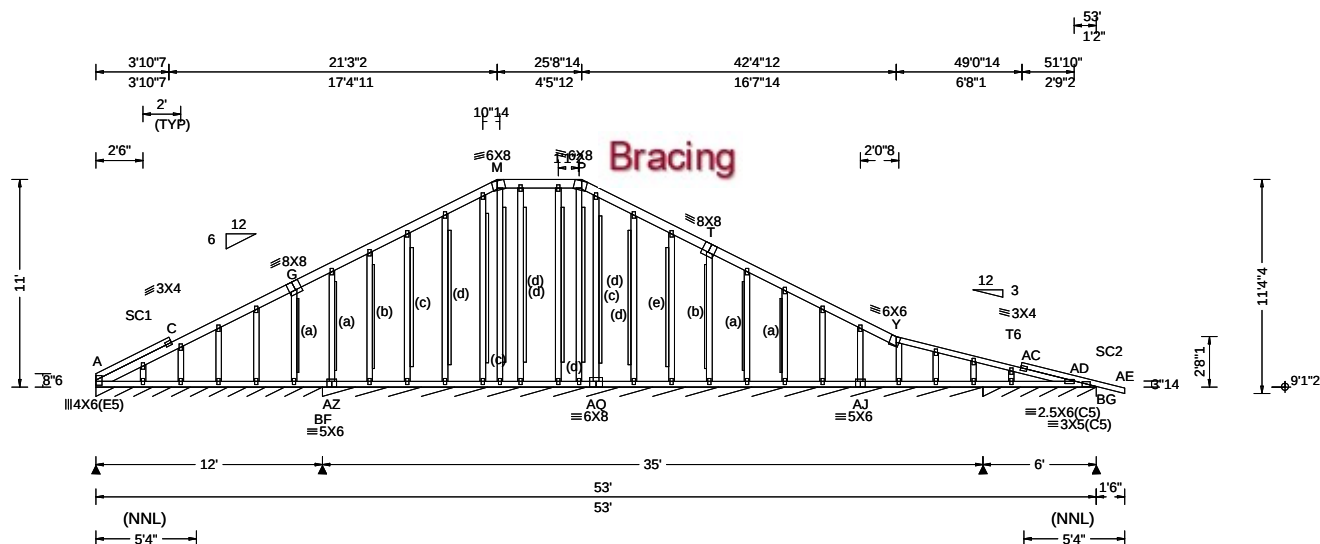
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.13 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.30 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 AD 999 240 VERT(CL): 0.010 AD 999 180 HORZ(LL): 0.008 Y - - HORZ(TL): 0.009 Y - - Creep Factor: 2.0 Max TC CSI: 0.326 Max BC CSI: 0.094 Max Web CSI: 0.996 VIEW Ver: 23.02.04.0123.14	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A* 78 /- /- /49 /17 /25 BF* 84 /- /- /46 /9 /- BG* 93 /- /- /53 /15 /- Wind reactions based on MWFRS A Brg Wid = 144 Min Req = - BF Brg Wid = 420 Min Req = - BG Brg Wid = 72.0 Min Req = - Bearings A, BF, & AG are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x6 SP #2; T6 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/176.



COA #0278

06/11/2024
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 767225	GABL	Ply: 1	Job Number: 24-1260	Cust: R 215 JRef: 1Y0M2150004 T5
FROM: CDM		Qty: 1	BARBER	DrwNo: 163.24.1350.13557
Page 2 of 2			Truss Label: B06	SSB / FV 06/11/2024

Gable Reinforcement

- (a) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (b) 1x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (c) 2x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (d) 2x6 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
- (e) 2x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

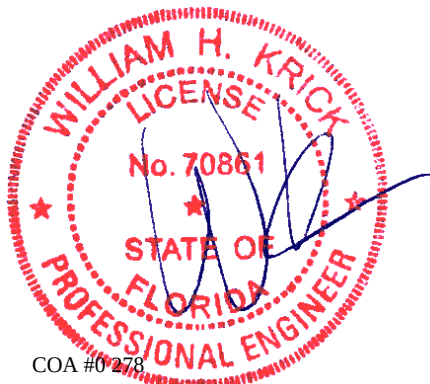
Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 11-0-0.



COA #0278

06/11/2024
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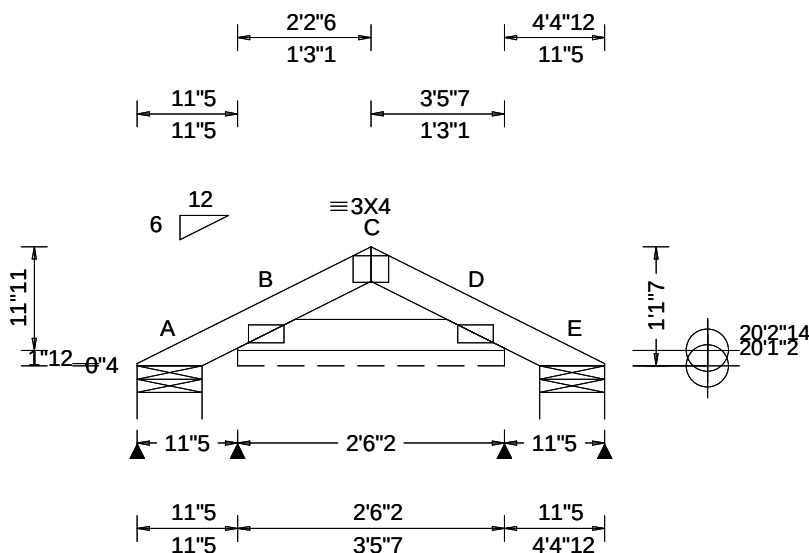
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 767227 FROM: CDM	GABL Ply: 1 Qty: 2	Job Number: 24-1260 BARBER Truss Label: PB01	Cust: R 215 JRef: 1Y0M2150004 T3 DrwNo: 163.24.1350.15047 SSB / FV 06/11/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.13 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): -0.000 D - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.012 Max BC CSI: 0.027 Max Web CSI: 0.000 VIEW Ver: 23.02.04.0123.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 26 /- /- /20 /7 /27 B* 105 /- /- /61 /16 /- E 26 /- /- /19 /7 /- Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 (Truss) B Brg Wid = 30.1 Min Req = - E Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Plating Notes

All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160220723 for piggyback details.

The overall height of this truss excluding overhang is 1-1-7.



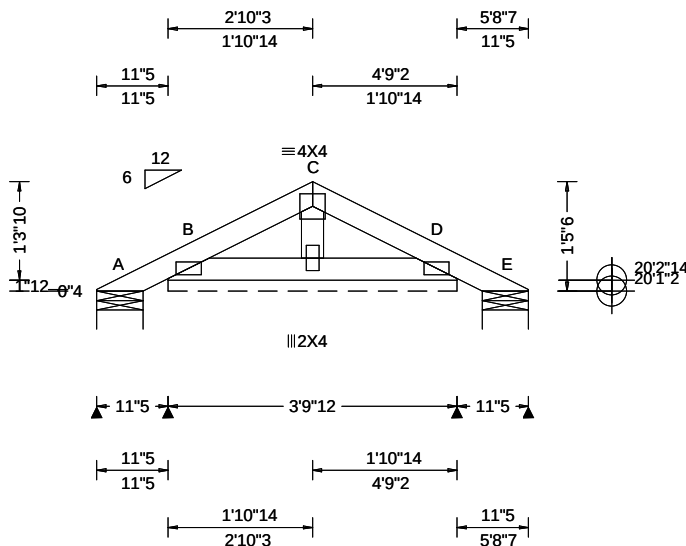
COA #0278

06/11/2024
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Glenview, IL 60025

SEQN: 767229 FROM: CDM	COMN Ply: 1 Qty: 32	Job Number: 24-1260 BARBER Truss Label: PB02	Cust: R 215 JRef:1Y0M2150004 T15 DrwNo: 163.24.1350.17287 SSB / FV 06/11/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.29 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.029 Max BC CSI: 0.024 Max Web CSI: 0.012 VIEW Ver: 23.02.04.0123.14	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 19 /- /- /25 /11 /36 B* 101 /- /- /58 /18 /- E 19 /- /- /14 /3 /- Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 (Truss) B Brg Wid = 45.8 Min Req = - E Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

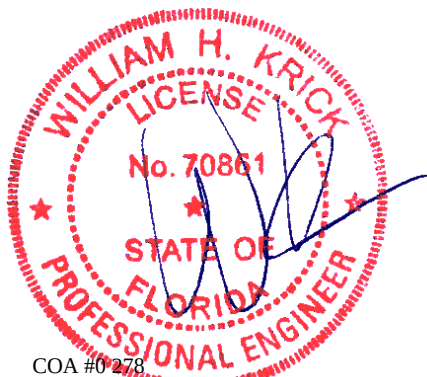
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160220723 for piggyback details.

The overall height of this truss excluding overhang is 1-5-6.



COA #0278

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North Building, 4th Floor
Glenview, IL 60025

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

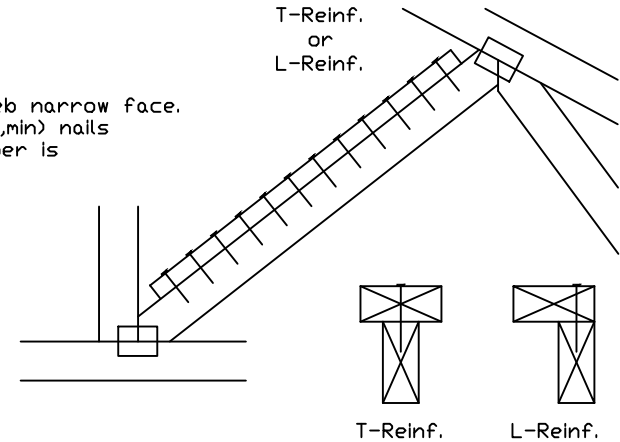
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x6(✕)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(✕)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(✕) Center scab on wide face of web. Apply (1) scab to each face of web.

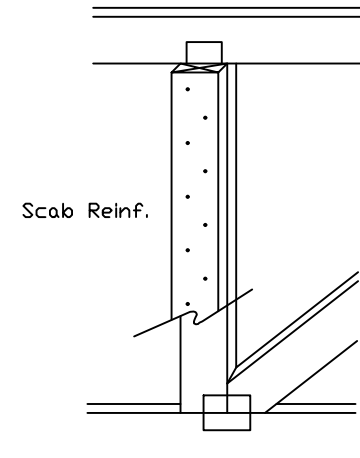
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



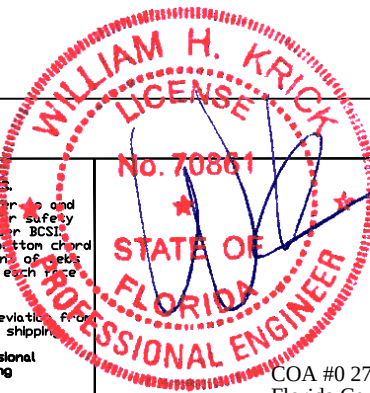
Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
06/11/2024 DUR. FAC. SPACING		

COA #0 278
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Piggyback Detail - ASCE 7-22: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

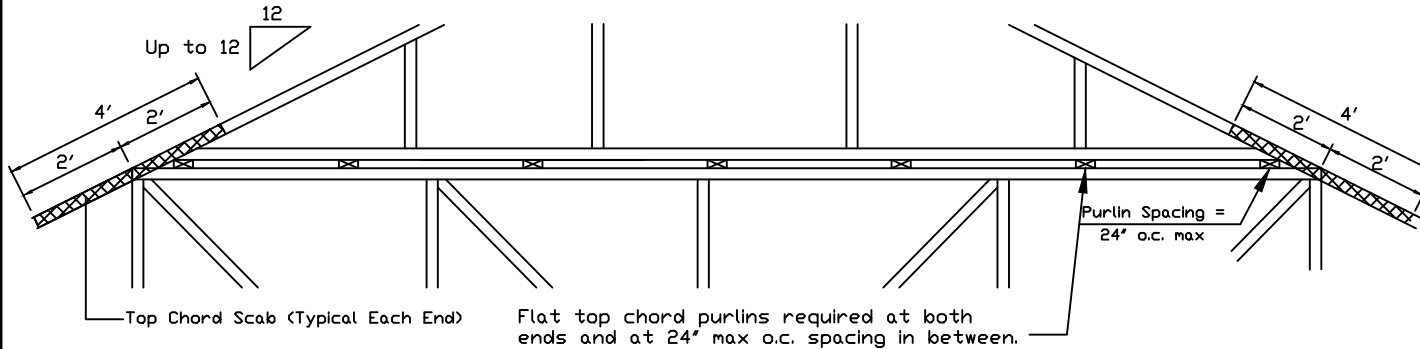
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building designer shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. Detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

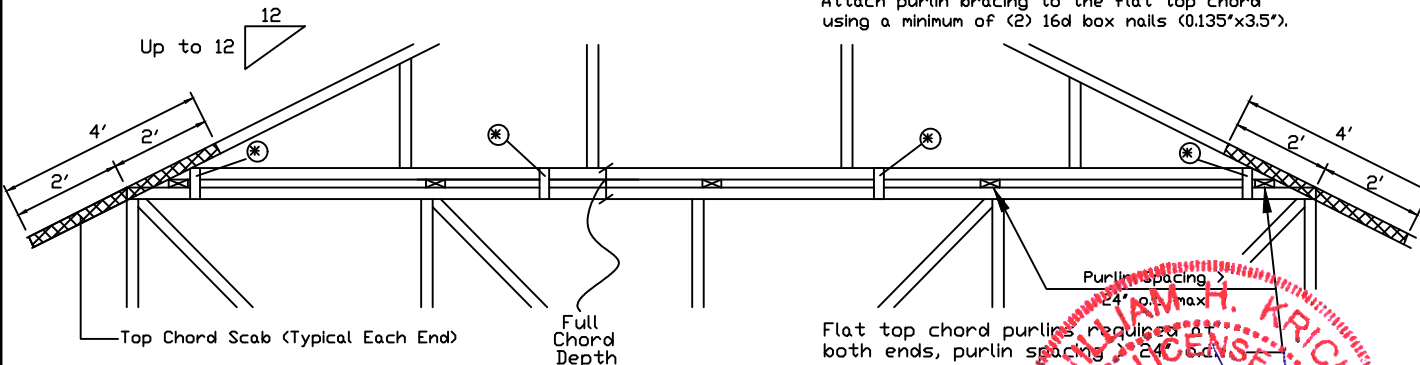


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

* In addition, provide connection with one of the following methods:

Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.
2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.
28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this Job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



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No. 70861



COA #0 278
Florida Certificate of Product Approval #FL

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