DATE 08/03/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024827

APPLICANT TRENT GIEBEIG PHONE 397-0545

ADDRESS 462 SW FAIRLINGTON COURT LAKE CITY FL 32055

OWNER PETE GIEBEIG PHONE 752-7968

ADDRESS 258 SW LUCILLE COURT LAKE CITY FL 32024

CONTRACTOR TRENT GIEBEIG PHONE 397-0545

LOCATION OF PROPERTY 247S, TR ON MAYFAIR LANE, TR ON LUCILLE COURT, END OF ROAD

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 80400.00

HEATED FLOOR AREA 1608.00 TOTAL AREA 2218.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 17

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 11-4S-16-02918-324 SUBDIVISION MAYFAIR

LOT 24 BLOCK PHASE 3 UNIT TOTAL ACRES

000001175		RR28281153		
Culvert Permit No.	Culvert Waiver	Contractor's License Number		Applicant/Owner/Contractor
CULVERT	06-0634-N	BK	JH	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 1890

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by | Foundation _____ date/app. by | Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by | Slab _____ date/app. by | Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by | Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by | Heat & Air Duct _____ date/app. by | Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by | C.O. Final _____ date/app. by | Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by | Pool _____ date/app. by

Reconnection _____ date/app. by | Pump pole _____ date/app. by | Utility Pole _____ date/app. by

M/H Pole _____ date/app. by | Travel Trailer _____ date/app. by | Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 405.00 CERTIFICATION FEE $ 11.09 SURCHARGE FEE $ 11.09

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 527.18

INSPECTORS OFFICE _____ CLERKS OFFICE _____

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Corrected Notice of Commencement

NOTICE OF COMMENCEMENT

Inst:2006018312 Date:08/02/2006 Time:13:14
GHH DC,P.DeWitt Cason,Columbia County B:1091 P:1448

STATE OF: Florida
COUNTY OF: Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statues, the following information is provided in this Notice of Commencement:

1. Description of Property: Lot #24 May Fair Unit III
258 SW Lucille Court
Lake City, Fl. 32024

2. General Description of Improvement: Construction of Single Family Residence

3. Owner Information:
   a. Name and Address: Peter W. Giebeig
   P.O. Box 1384 Lake City, FL 32056
   b. Interest in Property: Fee Simple
   c. Name and Address of Fee Simple titleholder (if other than Owner):

4. Contractor (Name and Address): Trent Giebeig Construction, Inc.
462 SW Fairlington Court Lake City, Fl. 32025

5. Surety:
   a. Name and Address: N/A
   b. Amount of Bond:

6. Lender (Name and Address): N/A

7. Persons within the State of Florida designated by Owner upon notices or other documents may be Served as provided by 713,13 (l)(a)(7), Florida Statues.
N/A

8. In addition to himself, the Owner designates the following person to recieve a copy of the Lienor's Notice as provided in 713.13 (l)(b), Florida Statues (Name and Address):
N/A

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of Recording unless a different date is specified):

Peter W. Giebeig
Type Owner Name: Peter W. Giebeig

Type Owner Name:

Elaine K. Tolar
Witness #1 ELAINE K. TOLAR

Marsha A. Dumas
Witness #2
MARSHA A. DUMAS

Sworn to and subscribed before me by the Owner (s) on this 1st day of Aug 2006

Elaine K. Tolar
Type Name: ELAINE K. TOLAR
Notary Public, State of Florida
COMMISSION EXPIRY / NUMBER:

Personally Known Peter W. Giebeig
Produced Identification
Did Take an Oath / Did Not Take an Oath

ELAINE K. TOLAR
MY COMMISSION # DD 436381
EXPIRES: October 2, 2008
Bonded Thru Notary Public Underwriters

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0607-77 Date Received 7/28/06 By G Permit # 1175/24827

Application Approved by - Zoning Official BLK Date 03.08.06 Plans Examiner OK JTH Date 8-2-06

Flood Zone X Plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res Low Den.

Comments SITE PLAN ON PAGE SP-1 OF PLANS

Applicants Name Trent Gibbeig Construction Inc Phone 397-0545

Address 462 SW Fairlington Ct Lake City FL

Owners Name ~~Trent Gibbeig Construction Inc~~ Pete Gibbeig Phone 752-7968

911 Address 258 SW Lucille Court Lake City FL 32025

Contractors Name Trent Gibbeig Phone 397-0545

Address 462 SW Fairlington Court Lake City FL

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address Freeman Design Group

Mortgage Lenders Name & Address

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 11-4S-16-02914-324 Estimated Cost of Construction 60,000

Subdivision Name Mayfair Lot 24 Block ____ Unit ____ Phase III

Driving Directions 247 South Right into Mayfair right SW Lucille Court @ end of road

Type of Construction frame Number of Existing Dwellings on Property ____

Total Acreage ____ Lot Size 1.52 Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 27 ✓ Side 16' ✓ Side 17' ✓ Rear 46'3" ✓

Total Building Height 17'6" Number of Stories 1 Heated Floor Area 1608 Roof Pitch 6/12

Porch 210 GARAGE 400 TOTAL 2,218

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Trent Gibbeig Construction Inc

Owner Builder or Agent (Including Contractor)

Trent Gibbeig

Contractor Signature

Contractors License Number RR0282811523

Competency Card Number 5754

STATE OF FLORIDA
COUNTY OF COLUMBIA

NOTARY STAMP/SEAL

ELAINE K. TOLAR
MY COMMISSION # DD 436381
EXPIRES: October 2, 2009
Bonded Thru Notary Public Underwriters

Sworn to (or affirmed) and subscribed before me this 28th day of July 2006.

Personally known X or Produced Identification ____

Elaine

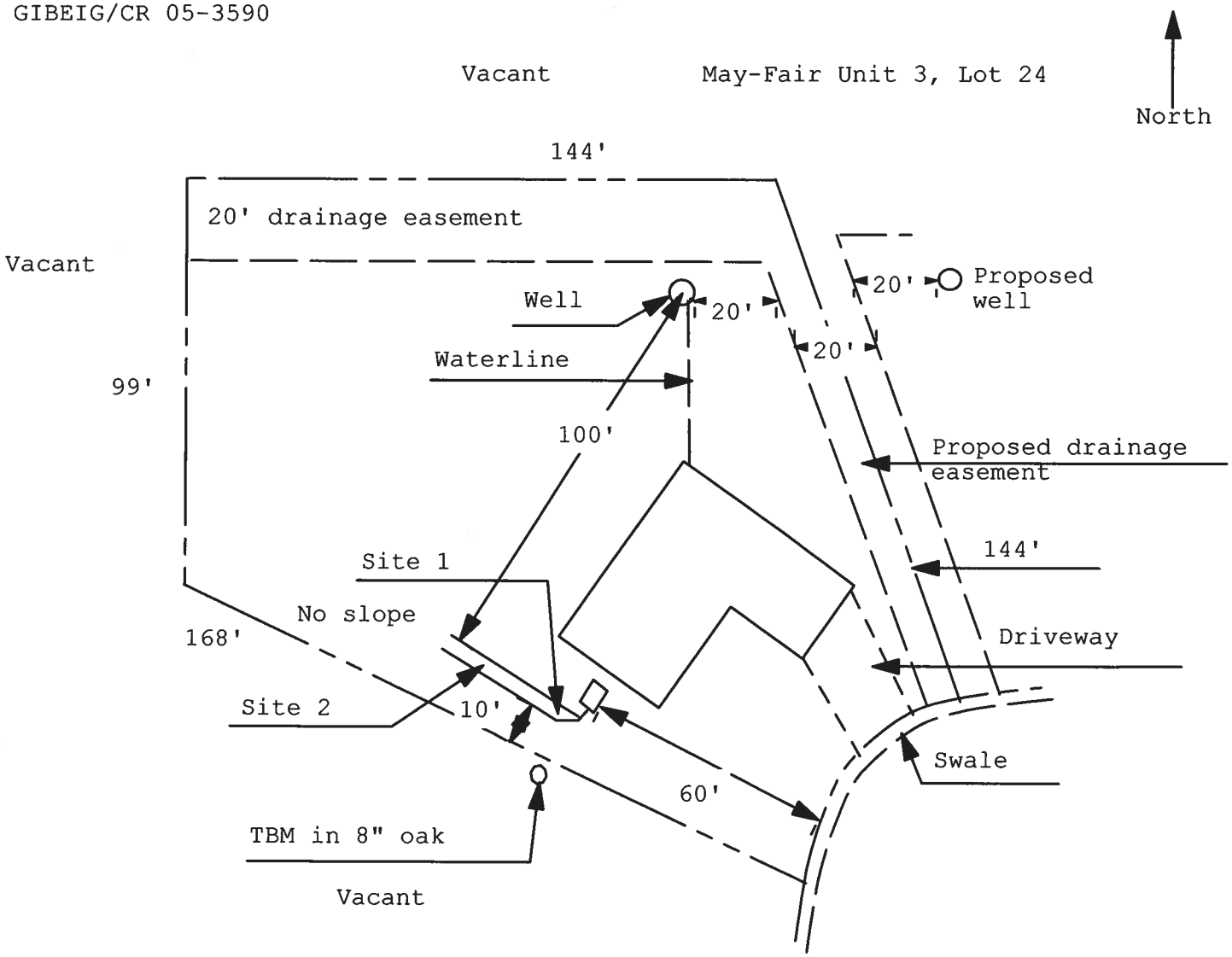
Notary Signature

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-0634N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

GIBEIG/CR 05-3590



1 inch = 40 feet

Site Plan Submitted By Paul Lloyd **Date** 4/23/06

Plan Approved ✓ **Not Approved** ____ **Date** ____

By [signature] Columbia **CPHU**

Notes: ____

The H16-2 series has a presloped seat of 5/12 for double trusses.

The H connector series provides wind and seismic ties for trusses and rafters.

The presloped 5/12 seat of the H16 provides for a tight fit and reduced deflection. The strap length provides for various truss heights up to a maximum of 13½" *(H16 series)*. Minimum heel height for H16 series is 4".

The HGA10 attaches to gable trusses and provides good lateral wind resistance. The HS24 attaches the bottom chord of a truss or rafter at pitches from 0:12 to 4:12 to double 2x4 top plates. Double skew nailing allows for higher lateral resistance.

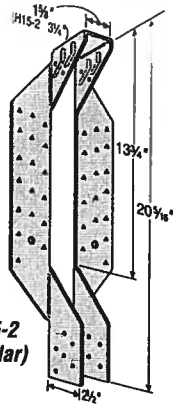
MATERIAL: See table

FINISH: Galvanized. See Corrosion-Resistance page 6-7.

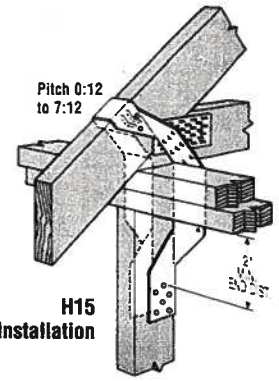
INSTALLATION: • Use all specified fasteners. See General Notes.

- The HGA10KT screws are provided.
- HS24 requires skew nailing only when bottom chord of truss or rafter has no slope
- Hurricane Ties do not replace solid blocking.

CODES: See page 12 for Code Listing Key Chart.



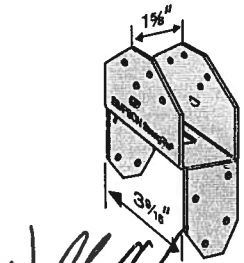
H15
(H15-2 similar)



H15 Installation

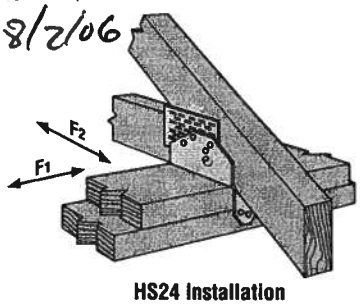
Model No.	Ga	Fasteners: To Rafters/Truss	Fasteners: To Plates	Fasteners: To Studs	DF/SP Allowable Loads[1]: Uplift (133)	DF/SP Allowable Loads[1]: Uplift (160)	DF/SP Allowable Loads[1]: Lateral (133/160) F_1	DF/SP Allowable Loads[1]: Lateral (133/160) F_2	SPF/HF Allowable Loads[1]: Uplift (133)	SPF/HF Allowable Loads[1]: Uplift (160)	SPF/HF Allowable Loads[1]: Lateral (133/160) F_1	SPF/HF Allowable Loads[1]: Lateral (133/160) F_2	Code Ref.
HGA10KT	14	4-SDS¼x1½	4-SDS¼x3	—	695	695	1165	940	595	595	870	815	125
HS24	18	8-8dx1½ & 2-8d slant	8-8d	—	605[3]	605[3]	645[3]	1025[3]	520	520	555	880	9, 62, 121
H15	16	4-10dx1½	4-10dx1½	12-10dx1½	1300	1300	480	—	1120	1120	410	—	6, 121
H15-2	16	4-10dx1½	4-10dx1½	12-10dx1½	1300	1300	480	—	1120	1120	410	—	
H16	18	2-10dx1½	10-10dx1½	—	1470	1470	—	—	1265	1265	—	—	125
H16S	18	2-10dx1½	10-10dx1½	—	1470	1470	—	—	1265	1265	—	—	
H16-2	18	2-10dx1½	10-10dx1½	—	1470	1470	—	—	1265	1265	—	—	
H16-2S	18	2-10dx1½	10-10dx1½	—	1470	1470	—	—	1265	1265	—	—	

1. Loads have been increased for earthquake or wind loading with no further increase allowed; reduce where other loads govern.
2. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
3. HS24 allowable loads without slant nailing are 625 lbs (uplift), 590 lbs (F1), 640 lbs (F2).
4. For H16-2S, S = short.
5. **NAILS: 10dx1½ = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8dx1½ = 0.131" dia. x 1½" long. See page 16-17 for other nail sizes and information.**

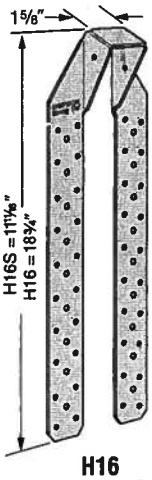


HS24
U.S. Patents 5,603,580

8/2/06

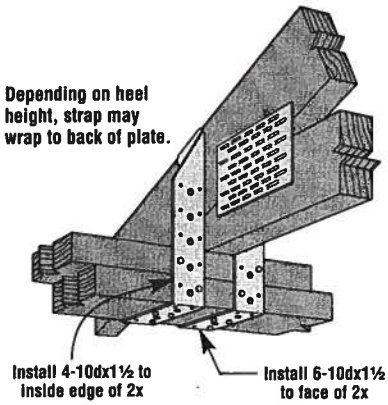


HS24 Installation

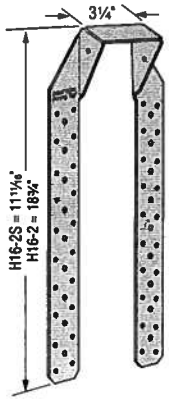


H16 and H16S

Presloped at 5:12. Pitch of 3:12 to 7:12 is acceptable

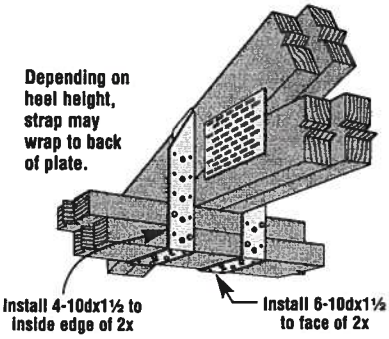


H16 Installation

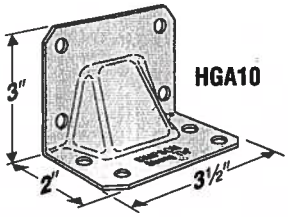


H16-2 and H16-2S

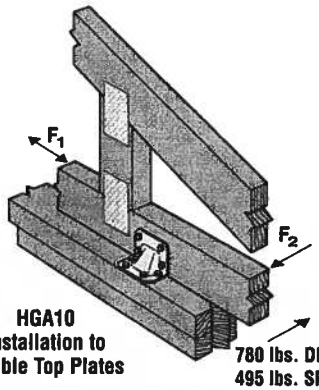
Presloped at 5:12. Pitch of 3:12 to 7:12 is acceptable



H16-2 Installation

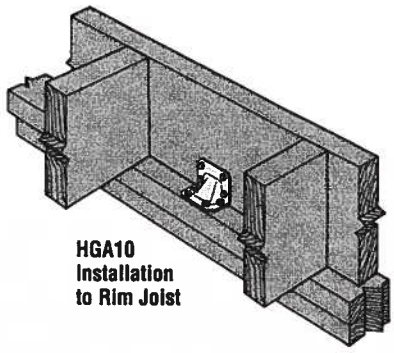


HGA10



HGA10 Installation to Double Top Plates

780 lbs. DF/SP
495 lbs. SPF/HF



HGA10 Installation to Rim Joist

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**May-Fair Lot 24**	Builder:	Trent Giebeig
Address:	**Lot: 24, Sub: May-Fair, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, Fl 32055-**	Permit Number:	24827
Owner:	**Giebeig, Pete**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		Yes	
6. Conditioned floor area (ft²)		1608 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	100.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 229.8(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1838.4 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1768.8 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 62.1 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 36.0 kBtu/hr	
	SEER: 13.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 36.0 kBtu/hr	
	HSPF: 8.00	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 50.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	MZ-C, PT, CF,	

Glass/Floor Area: 0.06

Total as-built points: 19602
Total base points: 26256

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Will A Free

DATE: 7/19/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 24, Sub: May-Fair, Plat: , Lake City, Fl, 32055-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1608.0	20.04	5800.4	Double, Clear	W	1.5	6.0	2.0	38.52	0.91	70.4	
				Double, Clear	W	1.5	6.0	20.0	38.52	0.91	703.7	
				Double, Clear	N	1.5	6.0	15.0	19.20	0.94	270.3	
				Double, Clear	N	1.5	5.0	8.0	19.20	0.92	140.6	
				Double, Clear	N	1.5	2.0	5.0	19.20	0.76	72.6	
				Double, Clear	E	1.5	7.0	30.0	42.06	0.94	1184.1	
				Double, Clear	E	1.5	6.0	20.0	42.06	0.91	767.9	
				As-Built Total:				**100.0**			**3209.6**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1838.4	1.50		2757.6	
Exterior	1838.4	1.70	3125.3									
Base Total:	**1838.4**		**3125.3**	**As-Built Total:**				**1838.4**			**2757.6**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Exterior Wood				30.0	6.10		183.0	
Exterior	30.0	6.10	183.0									
Base Total:	**30.0**		**183.0**	**As-Built Total:**				**30.0**			**183.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1608.0	1.73	2781.8	Under Attic			30.0	1768.8	1.73 X 1.00		3060.0	
Base Total:	**1608.0**		**2781.8**	**As-Built Total:**				**1768.8**			**3060.0**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	229.8(p)	-37.0	-8502.6	Slab-On-Grade Edge Insulation			0.0	229.8(p	-41.20		-9467.8	
Raised	0.0	0.00	0.0									
Base Total:			**-8502.6**	**As-Built Total:**				**229.8**			**-9467.8**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1608.0	10.21	16417.7					1608.0	10.21		16417.7	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 24, Sub: May-Fair, Plat: , Lake City, Fl, 32055-**	PERMIT #:

BASE			AS-BUILT					
Summer Base Points:		**19805.6**	**Summer As-Built Points:**					**16160.2**
Total Summer Points	System Multiplier	Cooling Points	Total Component	Cap Ratio	Duct Multiplier (DM x DSM x AHU)	System Multiplier	Credit Multiplier	Cooling Points
19805.6	**0.4266**	**8449.1**	16160.2 **16160.2**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.263 **0.263**	0.857 **0.857**	4138.5 **4138.5**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 24, Sub: May-Fair, Plat: , Lake City, Fl, 32055-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1608.0	12.74	3687.5	Double, Clear	W	1.5	6.0	2.0	20.73	1.02	42.4	
				Double, Clear	W	1.5	6.0	20.0	20.73	1.02	424.3	
				Double, Clear	N	1.5	6.0	15.0	24.58	1.00	369.5	
				Double, Clear	N	1.5	5.0	8.0	24.58	1.00	197.4	
				Double, Clear	N	1.5	2.0	5.0	24.58	1.01	124.7	
				Double, Clear	E	1.5	7.0	30.0	18.79	1.03	578.8	
				Double, Clear	E	1.5	6.0	20.0	18.79	1.04	389.2	
				As-Built Total:				**100.0**			**2126.3**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1838.4	3.40		6250.6	
Exterior	1838.4	3.70	6802.1									
Base Total:	**1838.4**		**6802.1**	**As-Built Total:**				**1838.4**			**6250.6**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	0.0	0.00	0.0	Exterior Wood				30.0	12.30		369.0	
Exterior	30.0	12.30	369.0									
Base Total:	**30.0**		**369.0**	**As-Built Total:**				**30.0**			**369.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1608.0	2.05	3296.4	Under Attic			30.0	1768.8	2.05 X	1.00	3626.0	
Base Total:	**1608.0**		**3296.4**	**As-Built Total:**				**1768.8**			**3626.0**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	229.8(p)	8.9	2045.2	Slab-On-Grade Edge Insulation			0.0	229.8(p	18.80		4320.2	
Raised	0.0	0.00	0.0									
Base Total:			**2045.2**	**As-Built Total:**				**229.8**			**4320.2**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1608.0	-0.59	-948.7					1608.0	-0.59		-948.7	

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 24, Sub: May-Fair, Plat: , Lake City, Fl, 32055-** PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**15251.4**	**Winter As-Built Points:**						**15743.4**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
15251.4	**0.6274**	**9568.8**	15743.4 **15743.4**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.426 **0.426**	0.950 **0.950**	7409.0 **7409.0**	

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 24, Sub: May-Fair, Plat: , Lake City, Fl, 32055-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2746.00	8238.0	50.0	0.90	3	1.00	2684.98	1.00	8054.9	
				As-Built Total:						**8054.9**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
8449	**9569**	**8238**	**26256**	**4138**	**7409**	**8055**	**19602**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 24, Sub: May-Fair, Plat: , Lake City, Fl, 32055-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 88.2

The higher the score, the more efficient the home.

Giebeig, Pete, Lot: 24, Sub: May-Fair, Plat: , Lake City, Fl, 32055-

1.	New construction or existing		New			
2.	Single family or multi-family		Single family			
3.	Number of units, if multi-family		1			
4.	Number of Bedrooms		3			
5.	Is this a worst case?		Yes			
6.	Conditioned floor area (ft²)		1608 ft²			
7.	Glass area & type	Single Pane	Double Pane			
a.	Clear - single pane	0.0 ft²	100.0 ft²			
b.	Clear - double pane	0.0 ft²	0.0 ft²			
c.	Tint/other SHGC - single pane	0.0 ft²	0.0 ft²			
d.	Tint/other SHGC - double pane					
8.	Floor types					
a.	Slab-On-Grade Edge Insulation		R=0.0, 229.8(p) ft			
b.	N/A					
c.	N/A					
9.	Wall types					
a.	Frame, Wood, Exterior		R=13.0, 1838.4 ft²			
b.	N/A					
c.	N/A					
d.	N/A					
e.	N/A					
10.	Ceiling types					
a.	Under Attic		R=30.0, 1768.8 ft²			
b.	N/A					
c.	N/A					
11.	Ducts					
a.	Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 62.1 ft			
b.	N/A					
12.	Cooling systems					
a.	Central Unit				Cap: 36.0 kBtu/hr	
					SEER: 13.00	
b.	N/A					
c.	N/A					
13.	Heating systems					
a.	Electric Heat Pump				Cap: 36.0 kBtu/hr	
					HSPF: 8.00	
b.	N/A					
c.	N/A					
14.	Hot water systems					
a.	Electric Resistance				Cap: 50.0 gallons	
					EF: 0.90	
b.	N/A					
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)					
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)				MZ-C, PT, CF,	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/488-7824.*

(EnergyGauge®/Version: FLRCPB v3.30)

Residential System Sizing Calculation

Summary

Giebeig, Pete

Lake City, Fl 32055-

Project Title:
May-Fair Lot 24

Code Only
Professional Version
Climate: North

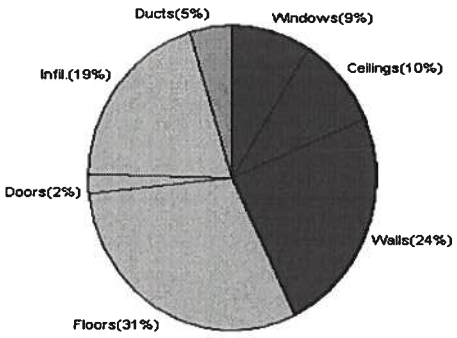
7/14/2006

Location for weather data: Gainesville - User customized: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (78F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	98	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	23	F
Total heating load calculation	**23684**	**Btuh**	**Total cooling load calculation**	**22332**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	152.0	36000	Sensible (SHR = 0.5)	101.7	18000
Heat Pump + Auxiliary(0.0kW)	152.0	36000	Latent	388.0	18000
			Total (Electric Heat Pump)	161.2	36000

WINTER CALCULATIONS

Winter Heating Load (for 1608 sqft)

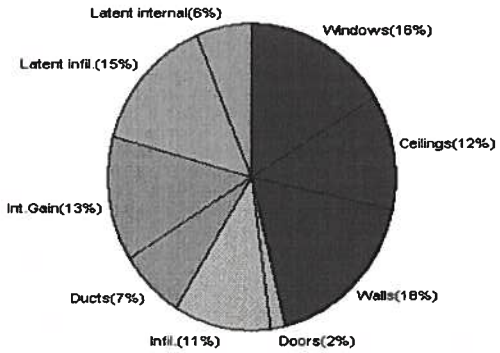
Load component			Load	
Window total	100	sqft	2150	Btuh
Wall total	1838	sqft	5699	Btuh
Door total	30	sqft	538	Btuh
Ceiling total	1769	sqft	2299	Btuh
Floor total	230	ft	7262	Btuh
Infiltration	107	cfm	4608	Btuh
Subtotal			**22556**	**Btuh**
Duct loss			1128	Btuh
TOTAL HEAT LOSS			**23684**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1608 sqft)

Load component			Load	
Window total	100	sqft	3644	Btuh
Wall total	1838	sqft	3934	Btuh
Door total	30	sqft	368	Btuh
Ceiling total	1769	sqft	2759	Btuh
Floor total			0	Btuh
Infiltration	94	cfm	2378	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			**16084**	**Btuh**
Duct gain			1608	Btuh
Total sensible gain			**17693**	**Btuh**
Latent gain(infiltration)			3259	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**4639**	**Btuh**
TOTAL HEAT GAIN			**22332**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY: Will H Free
DATE: 7/19/06

System Sizing Calculations - Winter

Residential Load - Component Details

Giebeig, Pete

Lake City, Fl 32055-

Project Title:
May-Fair Lot 24

Code Only
Professional Version
Climate: North

Reference City: Gainesville (User customized) Winter Temperature Difference: 39.0 F

7/14/2006

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Wood, DEF	N	2.0	21.5	43 Btuh
2	2, Clear, Wood, DEF	N	20.0	21.5	430 Btuh
3	2, Clear, Wood, DEF	E	15.0	21.5	322 Btuh
4	2, Clear, Wood, DEF	E	8.0	21.5	172 Btuh
5	2, Clear, Wood, DEF	E	5.0	21.5	108 Btuh
6	2, Clear, Wood, DEF	S	30.0	21.5	645 Btuh
7	2, Clear, Wood, DEF	S	20.0	21.5	430 Btuh
	Window Total		100		2150 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1838	3.1	5699 Btuh
	Wall Total		1838		5699 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		30	17.9	538 Btuh
	Door Total		30		538Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1769	1.3	2299 Btuh
	Ceiling Total		1769		2299Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	229.8 ft(p)	31.6	7262 Btuh
	Floor Total		230		7262 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	16080(sqft)	107	4608 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			107	4608 Btuh

Totals for Heating	**Subtotal**	**22556 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1128 Btuh**
	Total Btuh Loss	**23684 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Giebeig, Pete

Lake City, Fl 32055-

Project Title:
May-Fair Lot 24

Code Only
Professional Version
Climate: North

Reference City: Gainesville (User customized) Summer Temperature Difference: 23.0 F 7/14/2006

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load	
1	2, Clear, DEF, N, N	N	1.5	6	2.0	0.0	2.0	24	24	48	Btuh
2	2, Clear, DEF, N, N	N	1.5	6	20.0	0.0	20.0	24	24	480	Btuh
3	2, Clear, DEF, N, N	E	1.5	6	15.0	0.0	15.0	24	74	1110	Btuh
4	2, Clear, DEF, N, N	E	1.5	5	8.0	0.0	8.0	24	74	592	Btuh
5	2, Clear, DEF, N, N	E	1.5	2	5.0	3.1	1.9	24	74	214	Btuh
6	2, Clear, DEF, N, N	S	1.5	7	30.0	30.0	0.0	24	39	720	Btuh
7	2, Clear, DEF, N, N	S	1.5	6	20.0	20.0	0.0	24	39	480	Btuh
	Window Total				100					3644	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Frame - Exterior	13.0	1838.4	2.1	3934	Btuh
	Wall Total		1838.4		3934	Btuh

Doors	Type	Area	HTM	Load	
1	Wood - Exter	30.0	12.3	368	Btuh
	Door Total	30.0		368	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	1768.8	1.6	2759	Btuh
	Ceiling Total		1768.8		2759	Btuh

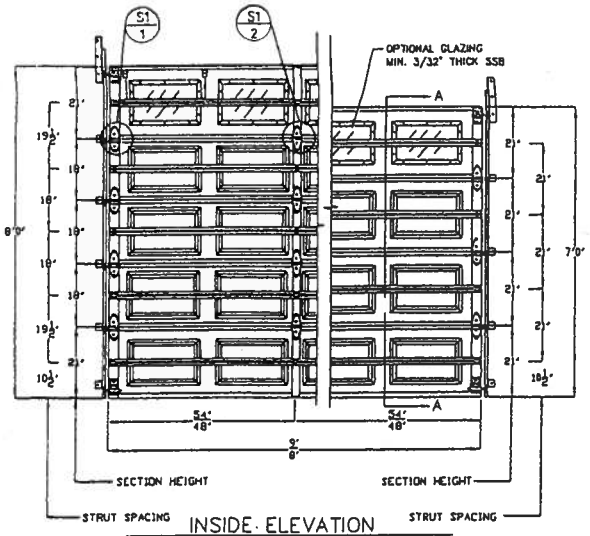
Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	229.8 ft(p)	0.0	0	Btuh
	Floor Total		229.8		0	Btuh

Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.35	16080	94.0	2378	Btuh
	Mechanical			0	0	Btuh
	Infiltration Total			94	2378	Btuh

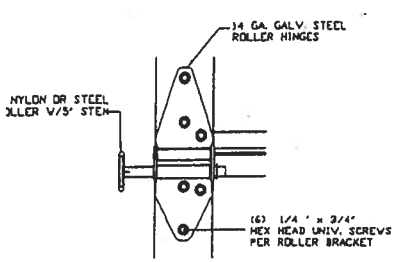
Internal gain		Occupants		Btuh/occupant		Appliance	Load	
		6	X	300	+	1200	3000	Btuh

Totals for Cooling	**Subtotal**	**16084**	**Btuh**
	Duct gain(using duct multiplier of 0.10)	**1608**	**Btuh**
	Total sensible gain	**17693**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**3259**	**Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**22332**	**Btuh**

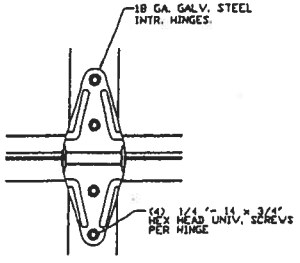
Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)



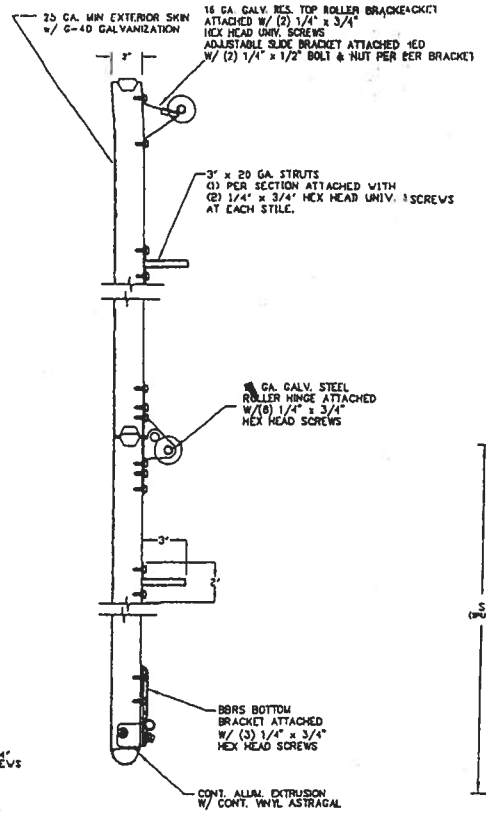
INSIDE ELEVATION



TYP. ROLLER BRACKET S1/1
N.T.S.



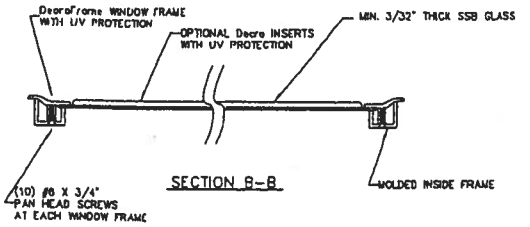
TYP. HINGE CONNECTION S1/2
N.T.S.



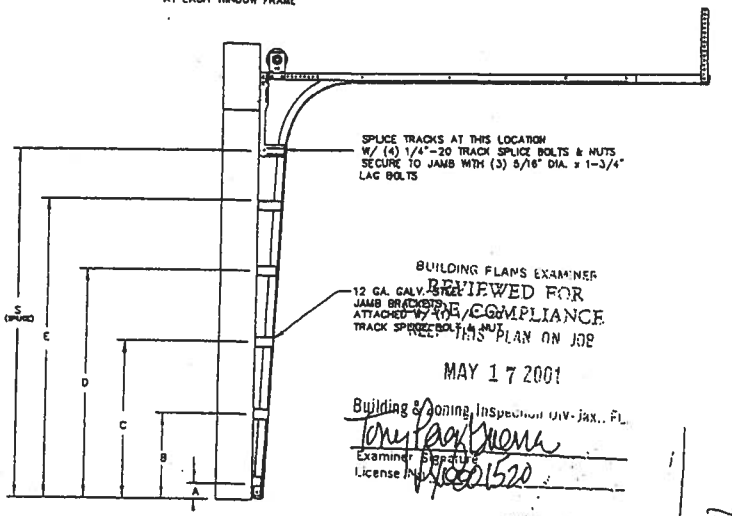
SECTION A-A (SIDE VIEW)

GLAZING OPTION CROSS SECTION

TEST No. SBC-580-011 ON OCTOBER 12, 1995 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +48.4 PSF AND -54.7 PSF. BY COMPARISION, FOUR (4) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 8' X 7' AND 8' X 8' MODEL 600 AND 850 DOORS.



SECTION B-B



BUILDING PLANS EXAMINER
REVIEWED FOR
CODE COMPLIANCE
KEEP THIS PLAN ON JOB

MAY 17 2001

Building & Zoning Inspection Div. Jax., FL

Examiner Signature

License No. 0000021520

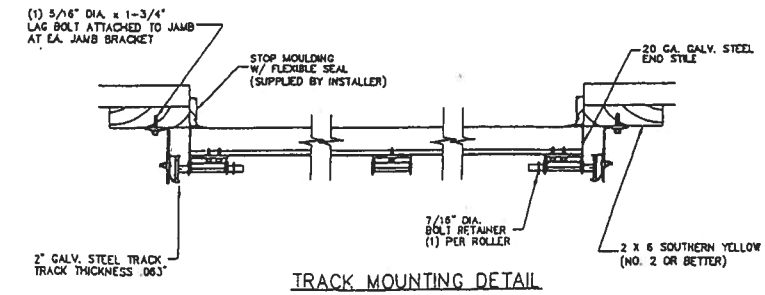
TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 25 GA. (.018") MIN. ROLL FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION.
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   - A. BASIC WIND SPEED OF 110 MPH
   - B. DOOR CAN BE INSTALLED WITH 6 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   - C. 15° MEAN ROOF HEIGHT AT ANY SLOPE
   - D. USE FACTOR OF 1.0
   - E. EXPOSURE RATING OF C



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-SO BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 6" FROM ENDS. USE PAIRS OF FASTENERS (3" APART) AT 24" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

MAX SIZE 8' x 8'

DESIGN LOADS +28.7 PSF -31.3 PSF

TEST LOADS +43.0 PSF -46.8 PSF

BY	EXCEPTION & REVISIONS	DATE	BY

Amarr GARAGE DOORS
9999 GRANDY CREEK BLVD. WINSTON-SALEM, N.C. 27105

MODEL #600 STRATFORD
MODEL #850 HERITAGE III

SIZE	DRAWN BY	DATE	DRAWING NUMBER
B	CHECKED BY	DATE	SBC-580-007-J
SCALE: NOT TO SCALE			SHEET 1 OF 1

MIAMI-DADE
BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358

outswing

SCOPE:
This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series "Regent" & "Omega" 18 ga. 3^0-7^0 Outswing Commercial Steel Door

APPROVAL DOCUMENT: Drawing No. **RD0087**, titled "3-0 x 7-0 Series", sheets 1 through 7 of 7, dated 5/30/97 with revision C dated 2/24/00,prepared by the manufacturer, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

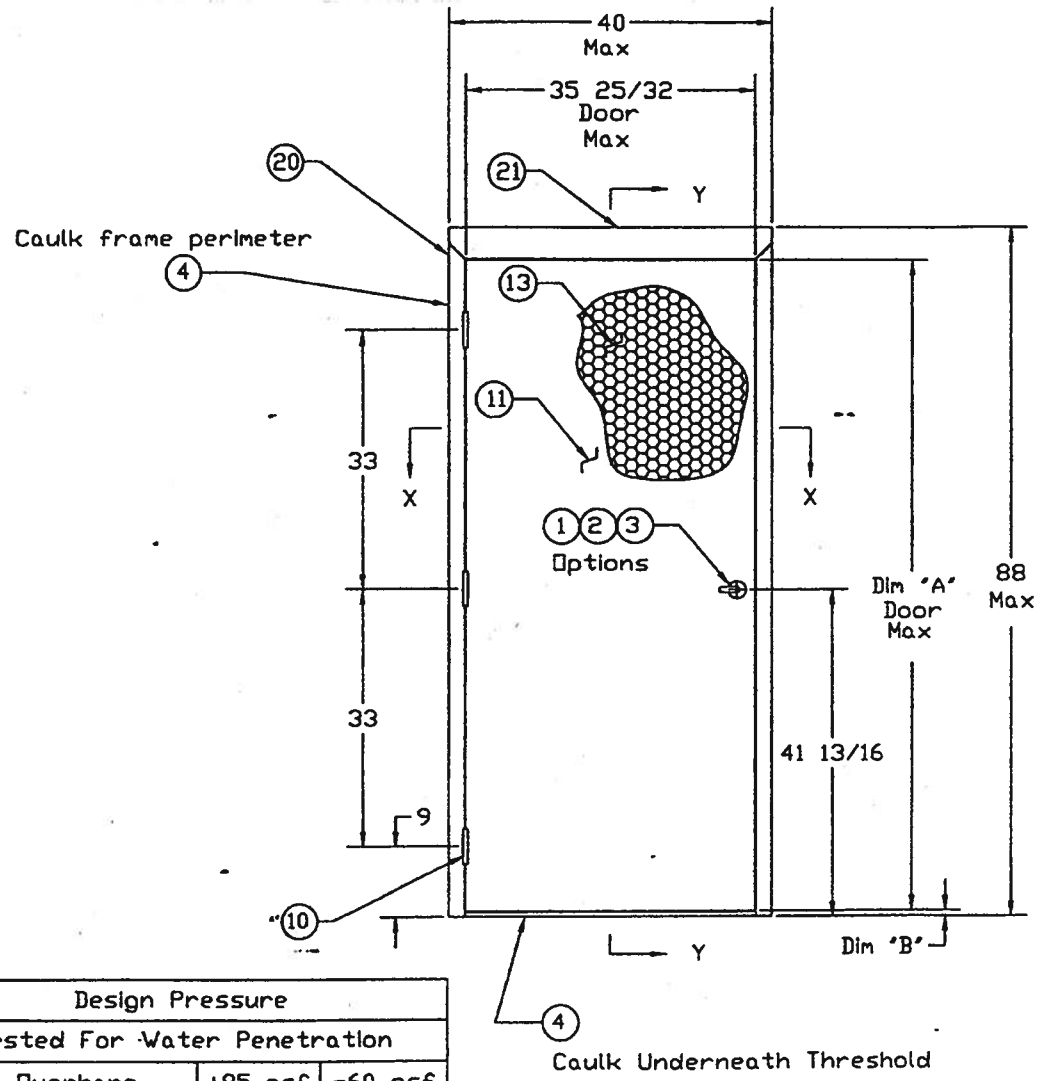
RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA **renews NOA # 00-0315.03** and consists of this page 1 as well as approval document mentioned above.
The submitted documentation was reviewed by **Manuel Perez, P.E.**

PRODUCT CONTROL ACCEPTED



Sheet 2	Frame Anchor Installation
Sheet 3	Threshold Installation
Sheet 3	Weatherstrip Installation
Sheet 4	Door Latch Reinforcement
Sheet 5-6	Cross Section View
Sheet 7	Bill Of Material

Design Pressure		
Tested For Water Penetration		
With Overhang	+85 psf	-60 psf
Without Overhang	+60 psf	-60 psf

	Dim "A"	Dim "B"
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08, 2009
BY [signature]
PRODUCT CONTROL DIV.S OF BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug. 14, 2008
By: [signature]
Miami-Dade Product Control Division

MATERIAL SPECIFICATIONS:

Finish: Rust Inhibitive Primer

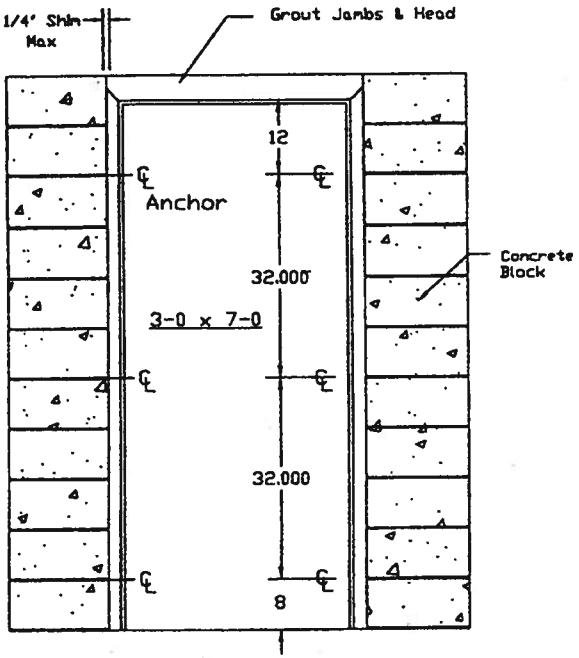
3-0 x 7-0 Series Elevation Drawing

CECO DOOR PRODUCTS
Milan, Tennessee 38358

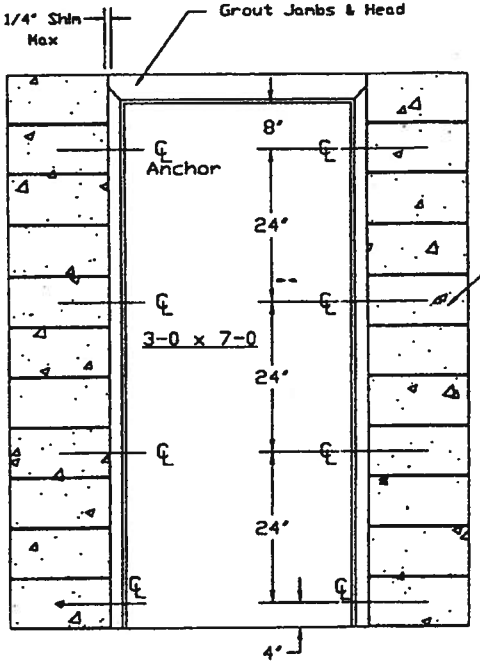
ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered information from NOA
B 7/22/97 GWS	Deleted CD Drawings Revised Sheet Numbers

DRAWN BY: GWS
DATE: 5/30/97

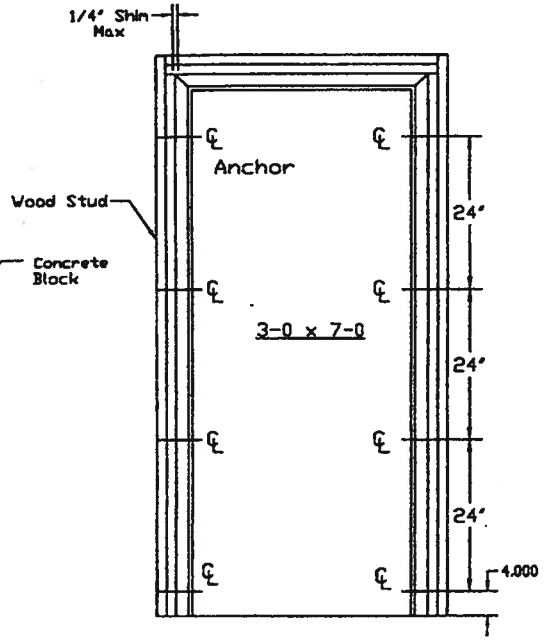
DRAWING NUMBER: RD0087
Sheet 1 Of 7



Masonry "T" Anchor

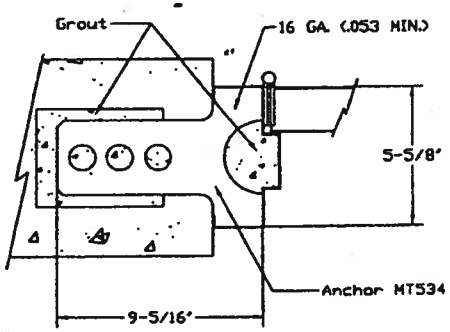


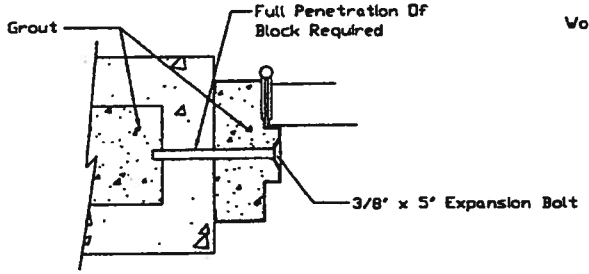
Existing Opening Anchor Into Block

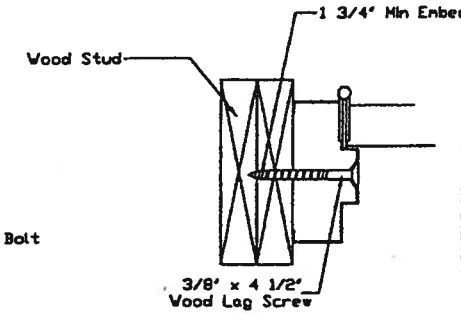


Existing Opening Anchor Into Wood Stud

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 14, 2008
By Manuel Perez
Miami-Dade Product Control Division







APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08, 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

NOTES:
1. SEE SHEET 7 FOR BILL OF MATERIALS

MATERIAL SPECIFICATIONS:

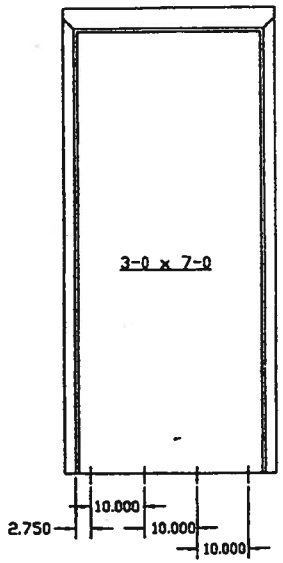
Frame Anchor Installation Details

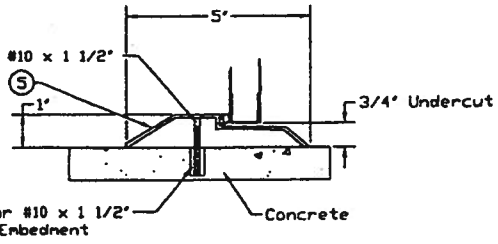
CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

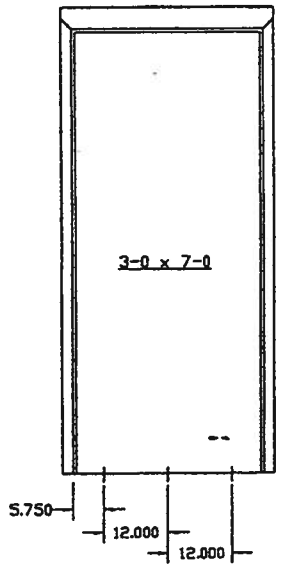
DRAWN BY:	DATE:
GWS	5/30/97

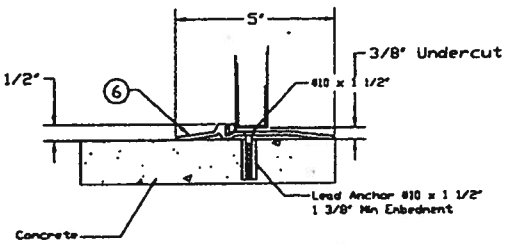
DRAWING NUMBER:
RD0087
Sheet 2 of 7



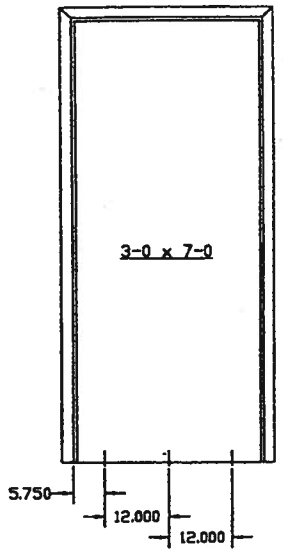


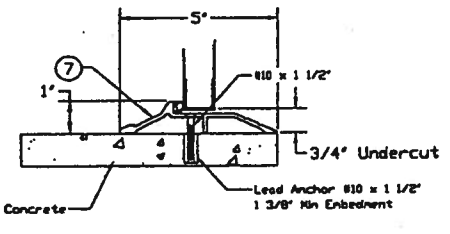
Threshold: National Guard 803S





Threshold: Penko 2005AV

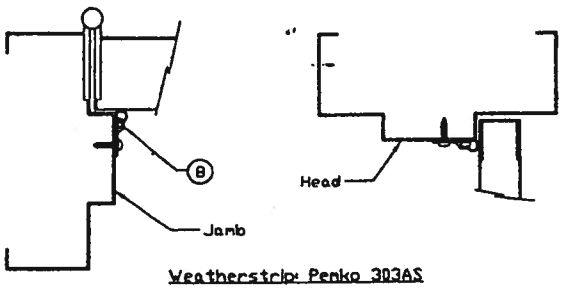




Threshold: Penko 181AV

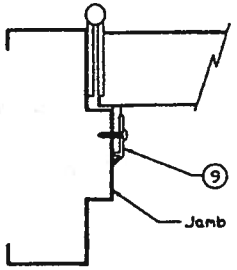
PRODUCT RENEWED
as complying with the Florida
Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 14, 2008
By Manuel Perez
Miami-Dade Product Control
Division

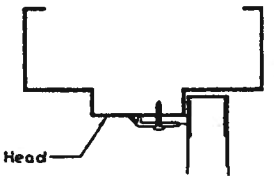
NOTE: 1. All thresholds shown are made from extruded aluminum with slide-in vinyl weatherstrip insert.



Weatherstrip: Penko 303AS

NOTE:
2. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.





Weatherstrip: National Guard 130NA

NOTE:
3. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.

NOTE: 4. See Sheet 7 For Bill of Material

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

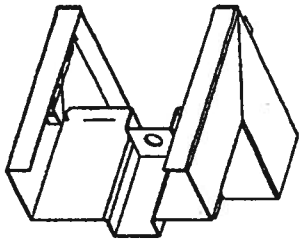
C 2/24/00 JAR	Revised Format, Transfered Information from NOA	
B 7/22/97 GWS	Revised Sheet Number	
ISSUE	REVISIONS	
DRAWN BY: GWS	DATE: 5/30/97	

MATERIAL SPECIFICATIONS:

Threshold & Weatherstrip Installation details

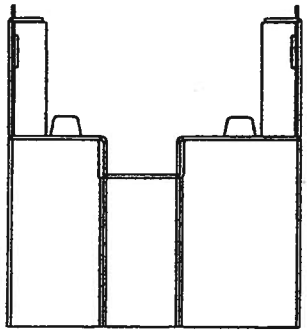
CECO DOOR PRODUCTS
Milan, Tennessee 38358

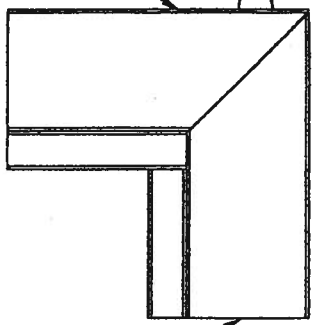
RD0087



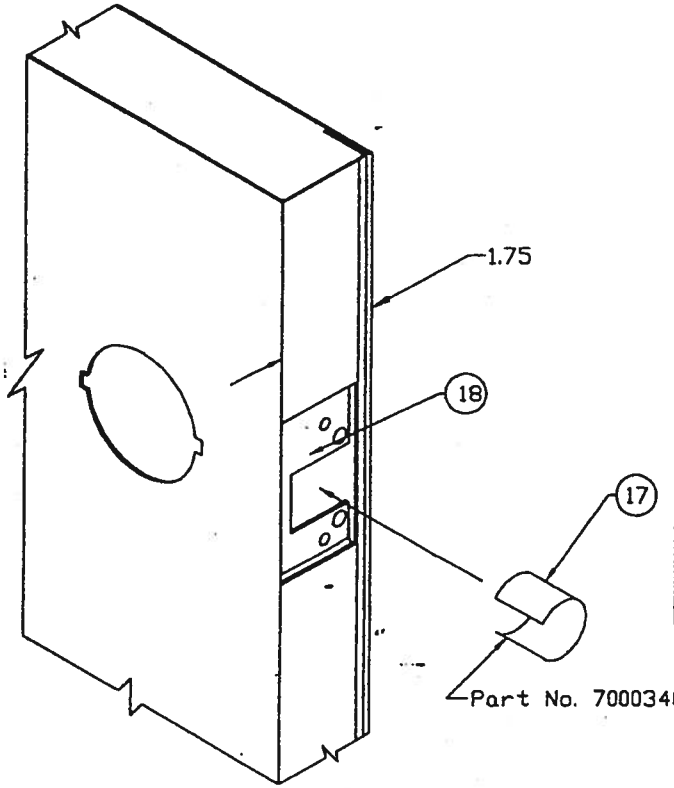
Interlocking Fold Over Tab

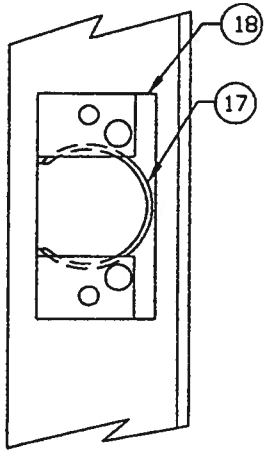
Frame Head





Frame Jamb





PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 14, 2008
By
Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

Note: 1. For Cylindrical Lock Only
2. See Sheet 7 For Bill Of Material

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

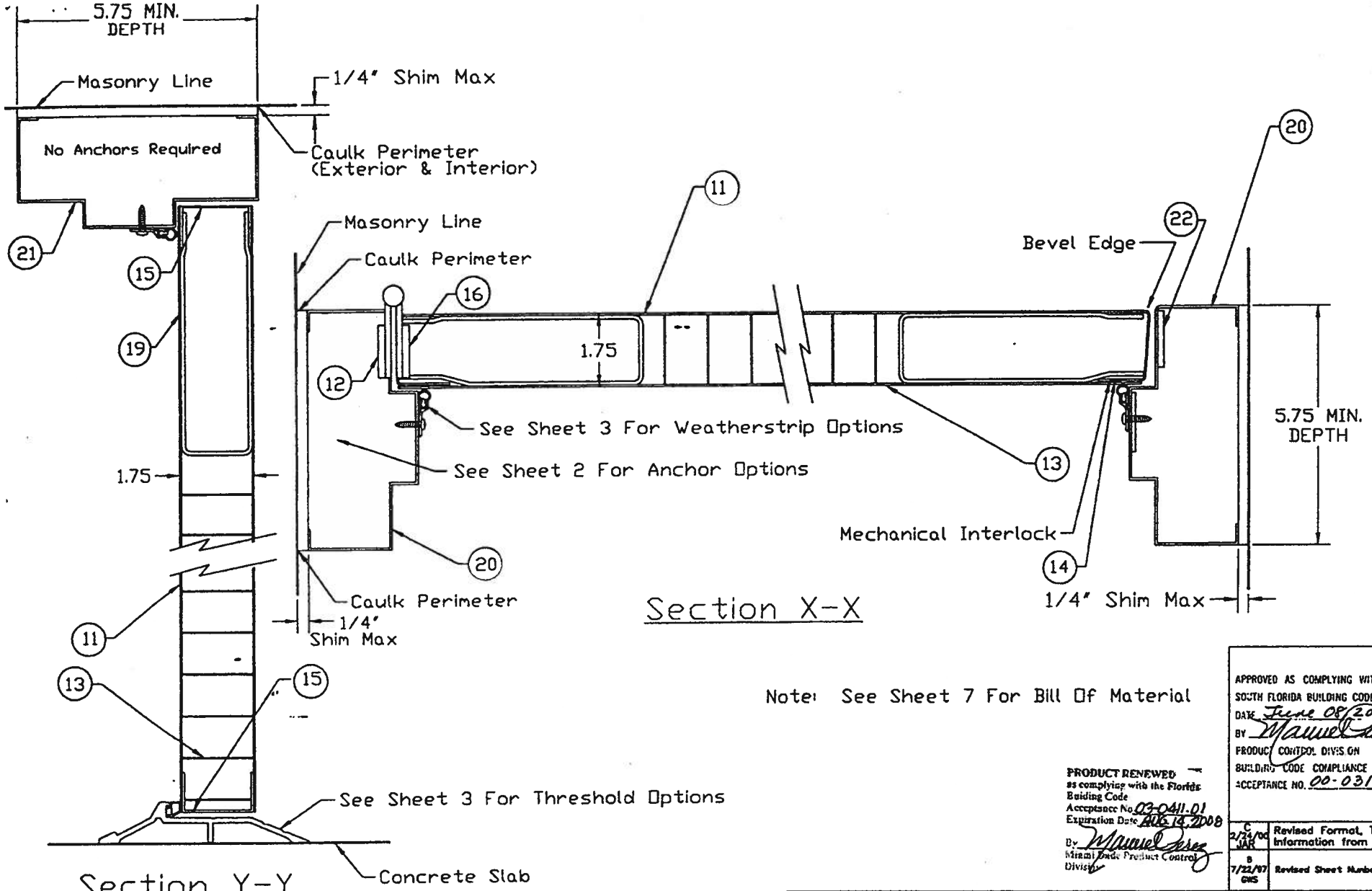
MATERIAL SPECIFICATIONS:

Cylindrical Lock Reinforcement and "SF" Seriers Frame Corner Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 6/06/97

DRAWING NUMBER:
RD0087
Sheet 4 of 7



Note: See Sheet 7 For Bill Of Material

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG 14 2008
By Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

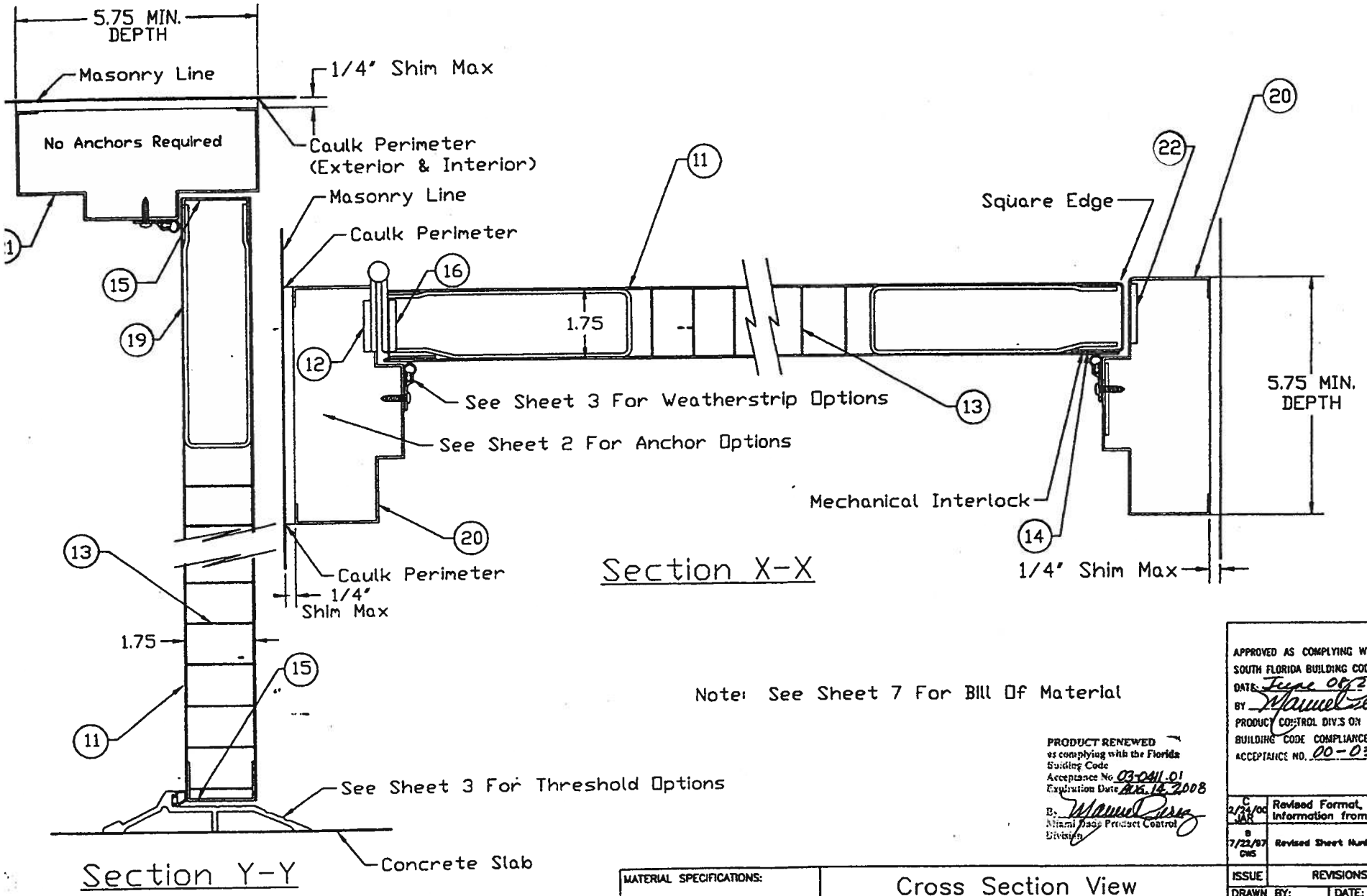
Cross Section View
Regent Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 5/30/97

DRAWING NUMBER: RD0087

Sheet 5 of 7



Note: See Sheet 7 For Bill Of Material

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Nov. 14, 2008
By: Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE: June 06, 2000
BY: Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

Cross Section View
Omega Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS DATE: 5/30/97

DRAWING NUMBER: RD0087

Sheet 6 of 7

ITEM	QTY	DESCRIPTION	MATERIAL	SIZE
1	1	SCHLAGE SERIES AL53PD GRADE 2, LATCH LOCK, SINGLE LEVER OR KNOB OPERATED		
2	1	MARKS SERIES 170AB GRADE 2, LATCH LOCK, INSIDE/OUTSIDE LEVER OPERATED		
3	1	YALE SERIES AU53070 GRADE 2 LATCH LOCK, SINGLE LEVER OR KNOB OPERATED.		
4	1	CAULK FOR INSTALLATION AND WEATHERSTRIP ADAPTER SCREWS FRAME PERIMETER (INSIDE & OUT) AND FRAME SILL CORNERS	GE SILICONE HOUSEHOLD SEALANT	
5	1	NATIONAL GUARD #803S		
6	1	PEMKO #2005AV		
7	1	PEMKO #181AV		
8	1 ROW	PEMKO #303AS HIGH SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPTER WITH A SILICON (TM) BULB INSERT		
9	1 ROW	NATIONAL GUARD #130NA 1-1/4" WIDE X 0.188" SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPT. WITH A FOAM INSERT		
10	3	HAGAR BB1279, 4-1/2" X 4-1/2" X .0134" THICK STEEL HINGE EACH ATTACHED WITH EIGHT #12-24 X 1/2" FH MS		
11	1	FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A568	COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=36,000 psi)	18 GAUGE (.042" MIN. THICK)
12	3	HINGE REINFORCING PLATE, PLATE SPOT WELDED TO FRAME JAMB AT EACH HINGE LOCATION	STEEL	1-1/4" X 9" X 7 GA.
13	1	CORE: FULL HONEYCOMB CORE PERMANENTLY BONDED TO THE INSIDE OF EACH FACE SKIN WITH NON-FLAMMABLE ADHESIVE	PHENOLIC RESIN-IMPREGNATED KRAFT PAPER	1-1/8" CELL
14	1	DENFLEX 3500 STRUCTURAL ADHESIVE EPOXY		
15	1	ROLL FORMED STEEL CHANNEL ON THE TOP AND BOTTOM OF THE DOOR SPOT WELDED TO EXTERIOR AND GLUED TO INTERIOR SKIN		1" X 1-3/4" X 1" X 16 GA. (.053" MIN)
16	3	DOOR HINGE REINFORCEMENT		1-1/4" X 9" X 7 GA.
17	1	DOOR LATCH REINFORCEMENT, STEEL "C" RING	28 GA. GALV.	.015" THICK X 1.313 INSIDE DIAMETER
18	1	DOOR LOCK REINFORCEMENT	STEEL	16 GA.
19	1	DOOR CLOSER REINFORCEMENT, ROLLED FORM CHANNELS TACK WELDED TO DOOR END CHANNELS	STEEL	12 GA. (.093")
20	2	SERIES "SF", FRAME JAMB, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. (.053" MIN.) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psi)	2" FACE, 5-3/4" DEPTH MIN.
21	1	SERIES "SF", FRAME HEAD, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. (.053" MIN.) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psi)	2" FACE, 5-3/4" DEPTH MIN.
22	1	JAMB LOCK STRIKE REINFORCING PLATE	STEEL	1-1/8" X 2-1/2" X 12 GA.

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG 14, 2008
By: Manuel Perez
Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08, 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

3-0 x 7-0 Series Bill Of Materials

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS | DATE: 6/02/97

DRAWING NUMBER: RD0087

Sheet 7 of 7

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMK s Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. esting was conducted using four nails per shingle. These shingles also comply with Florida Buil ıg Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-46

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA, AL 35403-2149 205-752-3555 FAX 205-349-2049

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 480/680/880 Drop-in
PRODUCT TYPE: Aluminum Horizontal Sliding Window (XO-Fin)

	Results	
Title	**Test Specimen #1**	**Test Specimen #2**
Rating	HS-C30 71 x 71	HS-C40 71 x 59
Operating Force	11 lbf max.	14 lbf max.
Air Infiltration	0.11 cfm/ft^2	0.09 cfm/ft^2
Water Resistance Test Pressure	5.3 psf	6.0 psf
Uniform Load Deflection Test Pressure	± 30.0 psf	+ 45.0 psf -47.2 psf
Uniform Structural Load Test Pressure	± 45.0 psf	+ 67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10	Grade 10

Reference should be made to ATI Report Identification No. 01-47320.03 for complete test specimen description and data

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

ATI Report Identification No.: 01-47320.03
Test Dates: 10/07/03
Through: 10/08/03
And: 12/01/03
And: 12/15/03
And: 03/17/04
Report Date: 04/16/04
Expiration Date: 10/07/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on two Series/Model 480/680/880 Drop-in, aluminum horizontal sliding windows at MI Home Products, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: HS-C30 71 x 71; Test Specimen #2: HS-C40 71 x 59. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 480/680/880 Drop-in

Product Type: Aluminum Horizontal Sliding Window (XO Fin)

Test Specimen #1: HS-C30 71 x 71

Overall Size: 5' 11-7/16" wide by 5' 11" high

Active Sash Size: 2' 11-5/8" wide by 5' 8-3/8" high

Fixed Daylight Opening Size: 2' 8-3/16" wide by 5' 5-5/8" high

Screen Size: 2' 10" wide by 5' 6-1/2" high

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.250" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails and fixed meeting rail interlock
0.250" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile

Test Specimen #2: HS-C40 71 x 59

Overall Size: 5' 11-3/8" wide by 4' 11-1/8" high

Active Sash Size: 2' 11-5/8" wide by 4' 8-1/4" high

Fixed Daylight Opening Size: 2' 8-1/4" wide by 4' 5-7/8" high

Screen Size: 2' 10-1/4" wide by 4' 7-1/8" high

Weatherstripping:

Description	Quantity	Location
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails
0.250" high by 0.187" backed polypile with center fin	1 Rows	Fixed meeting rail interlock
0.310" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile
0.550" high by 1" by 1" backed polypile pad	1 Pad	Corner of bottom rail and locking stile

Test Specimen Description: (Continued)

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: The window utilized 5/8" thick sealed insulating glass constructed from two sheets of 1/8" thick clear annealed glass and a Swiggle spacer system. The lites were interior glazed onto double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Frame Construction: The frame was constructed of thermally broken extruded aluminum. The corners were secured utilizing three #8 x 1" screws per corner through the jambs into the head and sill screw bosses. End caps were utilized on the ends of the fixed meeting rails and secured with two #8 x 3/4" screws per cap. The meeting rails were then secured to the frame with two #8 x 3/4" screws.

Sash Construction: The sash was constructed of thermally broken extruded aluminum. The corners were secured utilizing one #8 x 1" screw per corner through the head and sill into the jambs screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Cam lock	1	One midspan of active panel with integral lock keeper on fixed meeting stile
Roller assembly	2	One each end of bottom rail
Screen constant force spring	2	5" from rails on screen stiles
Screen lift handles	2	5" from rails on screen stiles

Drainage:

Description	Quantity	Location
1-1/4" long by 1/4" wide weepslot with cover	2	3-1/2" from jambs on sill face
1/2" long by 1/8" wide weepslot	2	2" from jambs on sill track

Reinforcement: No reinforcement was utilized.

Installation: The window was installed into a #2 Spruce-Pine-Fir wood buck. The window was secured utilizing #8 x 1-5/8" drywall screws located in corners and 12" on center around nail-fin perimeter. Silicone was utilized around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: HS-C30 71 x 71			
2.2.2.5.1	Operating Force	11 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.11 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	30.0 psf (positive)	0.75"	See Note #2
	30.0 psf (negative)	0.71"	See Note #2

Note #2: *The Uniform Load Deflection test is not requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	45.0 psf (positive)	0.13"	0.26" max.
	45.0 psf (negative)	<0.01"	0.26" max.
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.19"/38%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.09"/19%	0.50"/100%
	Bottom rail	0.06"/13%	0.50"/100%

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: HS-C30 71 x 71 (Continued)			
2.1.8	Forced Entry Resistance per ASTM F 588		
Type: A	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547-00 (with and without screen) 5.3 psf	No leakage	No leakage
Test Specimen #2: HS-C40 71 x 59			
2.2.2.5.1	Operating Force	14 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.09 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	30.0 psf (positive)	0.62"	See Note #2
	30.0 psf (negative)	0.51"	See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	45.0 psf (positive)	0.03"	0.21" max.
	45.0 psf (negative)	0.04"	0.21" max.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: HS-C40 71 x 59 (Continued)			
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.13"/25%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.03"/6%	0.50"/100%
	Bottom rail	0.03"/6%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547-00 (with and without screen)		
	6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	45.0 psf (positive)	0.62"	See Note #2
	47.2 psf (negative)	0.54"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	67.5 psf (positive)	0.04"	0.21" max.
	70.8 psf (negative)	0.08"	0.21" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC

Digitally Signed by: Eric Westphal

Eric Westphal
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P. E.
Senior Project Engineer

EW:dme
01-47320.03

APRIL 20, 2004

MIAMI-DADE
BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358

IN Swing

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: The Ceco Series Single Flush / Embossed Inswing Commercial Steel Doors –Impact

APPROVAL DOCUMENT: Drawing No RD0728, titled "3-0 x 7-0 , Series Regent, Omega, Imperial, Versa door", prepared by manufacturer, sheets 1 through 9 of 9 dated 05/22/02 and latest revised on 10-10-02, bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

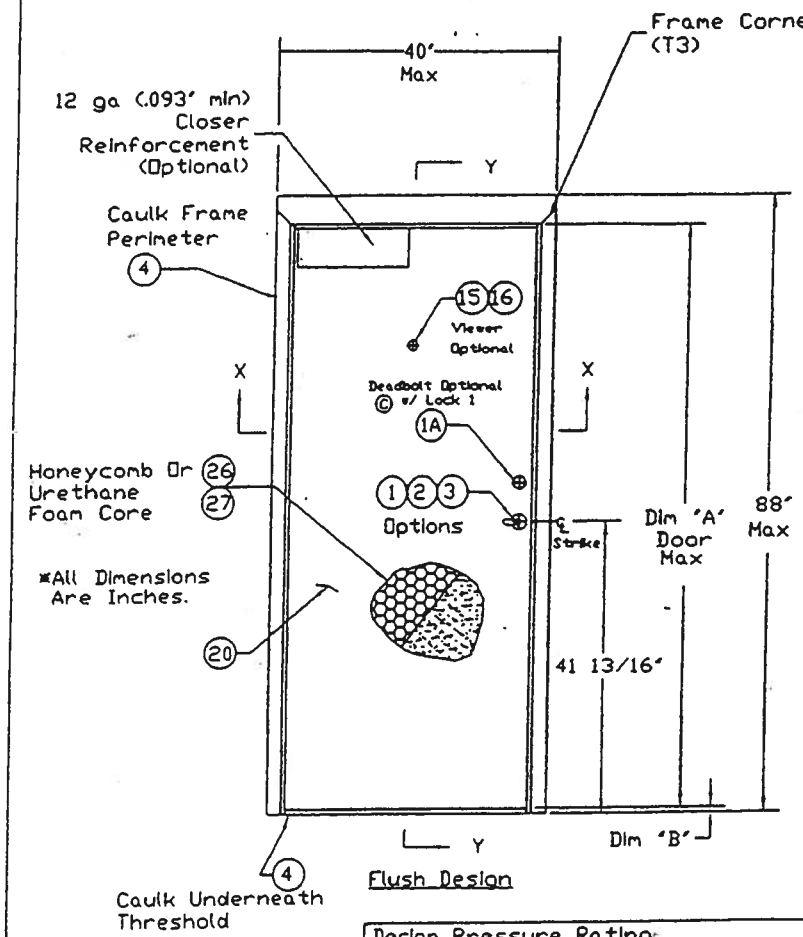
TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

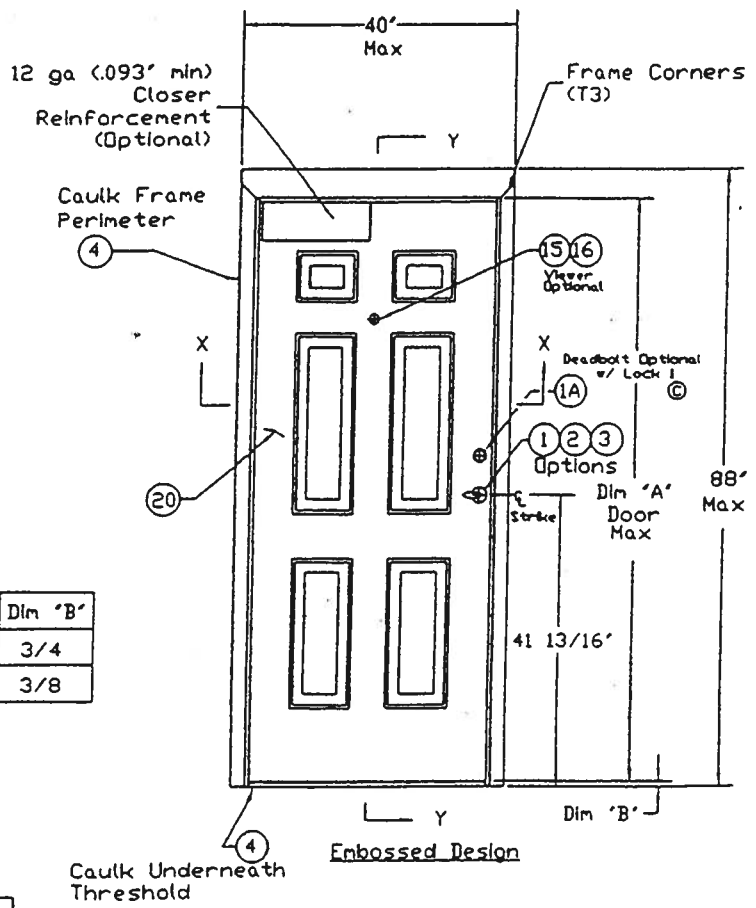
ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA consists of this page 1 as well as approval document mentioned above.

The submitted documentation was reviewed by **Ishaq I. Chanda, P.E.**





In-Swing Door
(Exterior View)

	Dim "A"	Dim "B"
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

Approved as complying with the Florida Building Code
Date OCTOBER 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

Design Pressure Rating		
	Where Water Infiltration Requirement Is Needed	Where Water Infiltration Requirement Is Not Needed
Positive	Not Approved	+70 PSF
Negative	Not Approved	-70 PSF

Notes:
1) In-swing Not Approved For Water Infiltration
2) This Door Does Not Need A Hurricane Protection Systen
3) Hinge Spacing Is 33" O.C., 13" From Top Of Frame & 9" From The Botton.

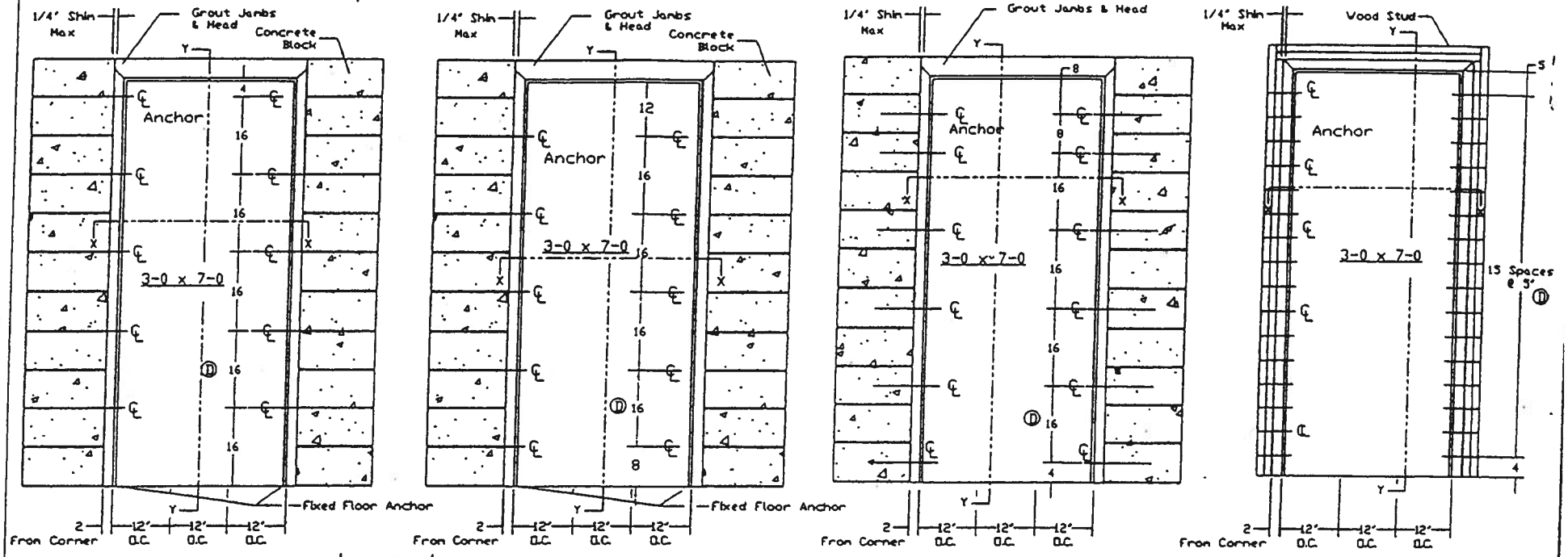
MATERIAL SPECIFICATIONS:
Finish: Rust Inhibitive Primer

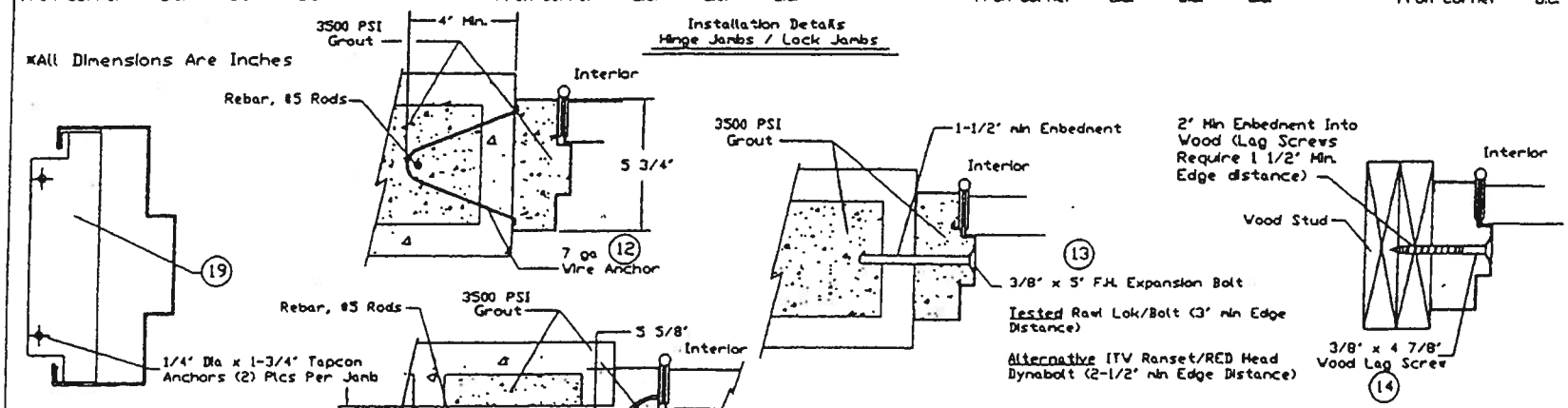
3-0 x 7-0 Series
Regent, Omega, Imperial, & Versadoor
In-Swing Elevation Drawing

CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
B 10/10/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
A 8/28/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
DRAWN BY: LT	DATE: 5/22/02

DRAWING NUMBER:
RD0728
Sheet 1 of 9





Approved as complying with the Florida Building Code
Date OCT 31 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

ISSUE	REVISIONS
A 5/22/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

MATERIAL SPECIFICATIONS:

Frame Anchor (Inswing Doors)
Regent, Omega, Imperial & Versadoor
Installation Details

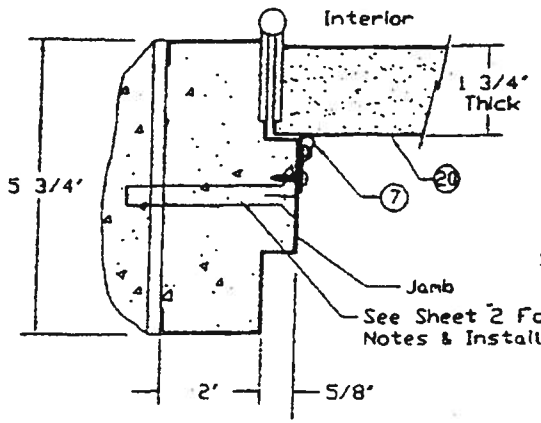
CECO DOOR PRODUCTS
Milan, Tennessee 38358

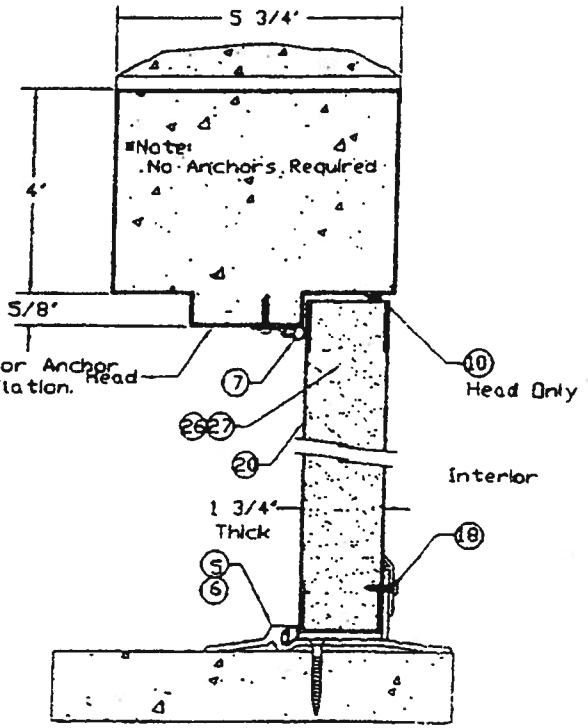
DRAWN BY: LT
DATE: 5/22/02

DRAWING NUMBER: RD0728

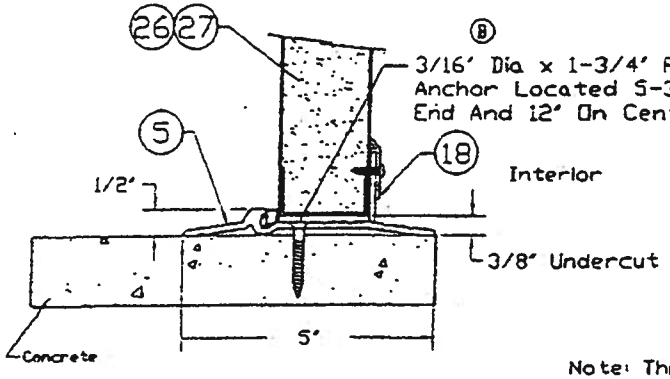
Sheet 2 of 9

*Note: Structural Member At Header Must Be Designed To carry 58.3#/ft load Imposed And Must Be Reviewed By Building Official.





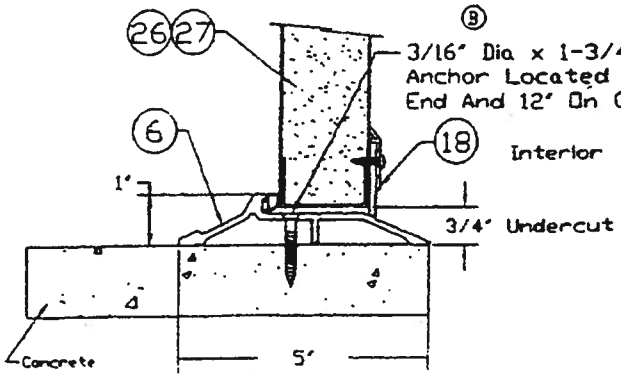
Section Y-Y



Threshold Pemko 200SAV

Inswing
(Not Approved For Water)

Note: Thresholds Not Approved For Water.



Threshold Pemko 181AV

Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

B	LT	Revised Per Marked-Up Drawings From Ishaq Chanda
C	LT	Revised Per Marked-Up Drawings From Ishaq Chanda

MATERIAL SPECIFICATIONS:

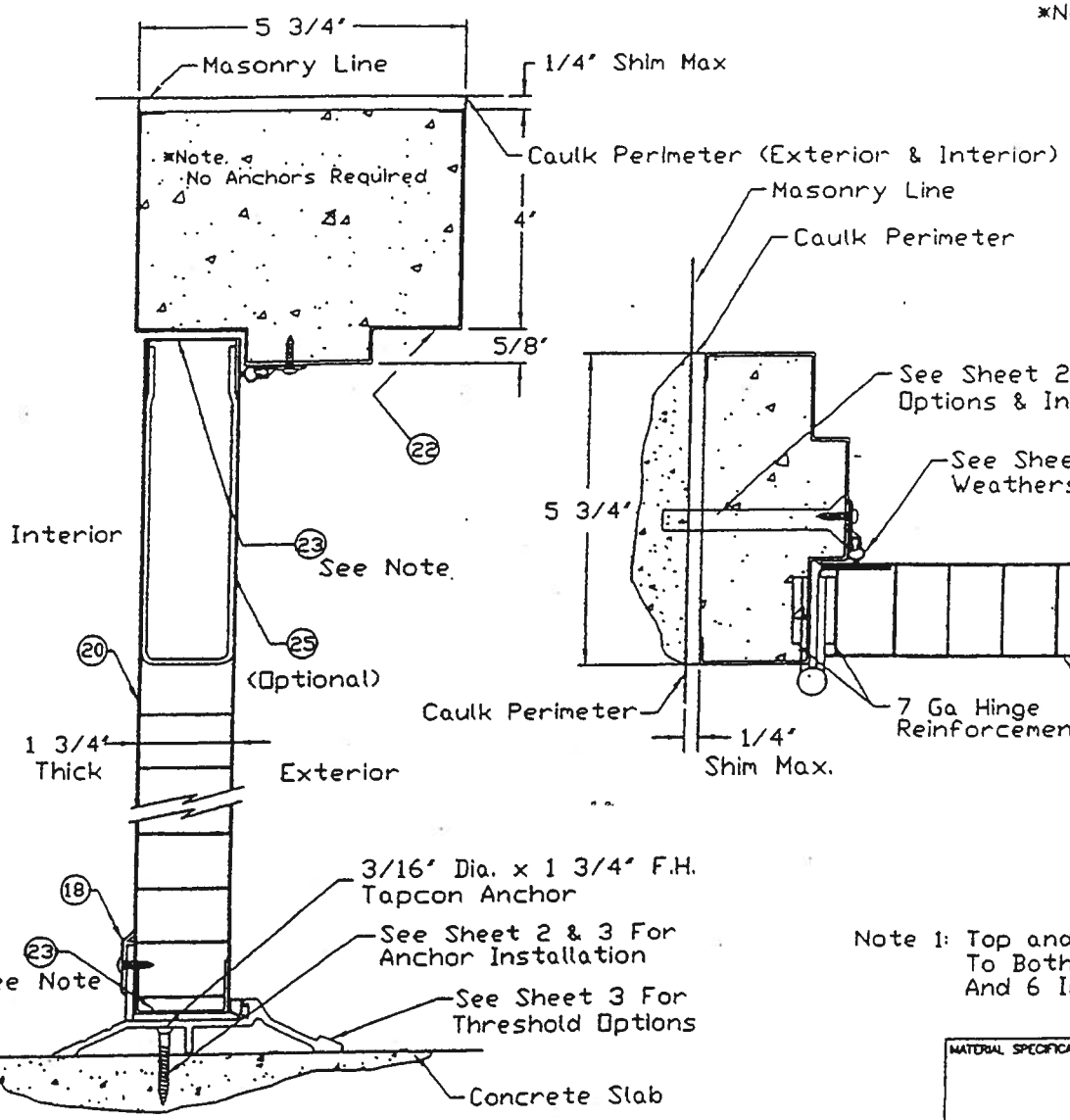
Threshold & Weatherstrip (Inswing Doors)
Regent, Omega, Imperial, Versadoor
Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

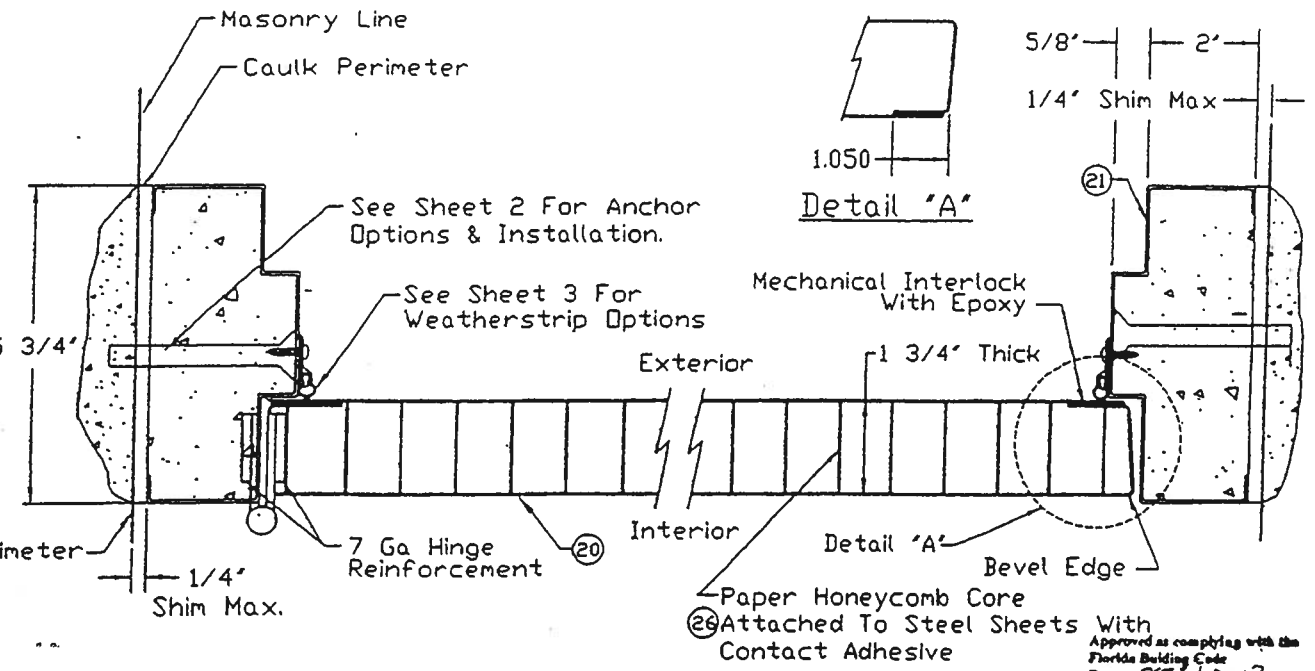
ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/22/02

RD0728
Sheet 3 Of 9

*Note: Structural Member At Head Must Be Designed To Carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section Y-Y



Section X-X

Note 1: Top and Bottom Channel Tack Welded To Both Skins 3 Inches From Lock Edge And 6 Inches On Centers

Approved as complying with the Florida Building Code
Date 07/31/2002
NOA# 02-0207.04
Miami Dade Product Control Division
By Ishaq Chanda

ISSUE	REVISIONS
C LT	Revised Per Marked-up Drawings From Ishaq Chanda.
B LT	Revised Per Marked-up Drawings From Ishaq Chanda.

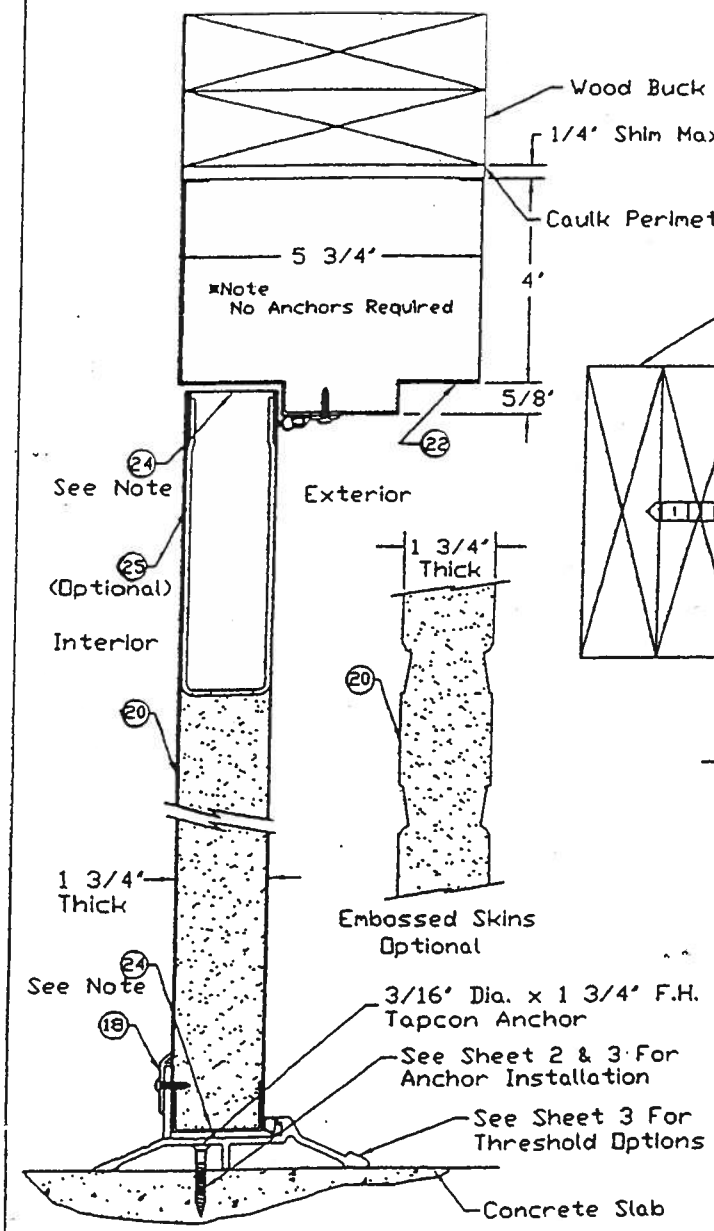
MATERIAL SPECIFICATIONS:

Cross Section View
(Inswing Doors)
Regent Handed Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

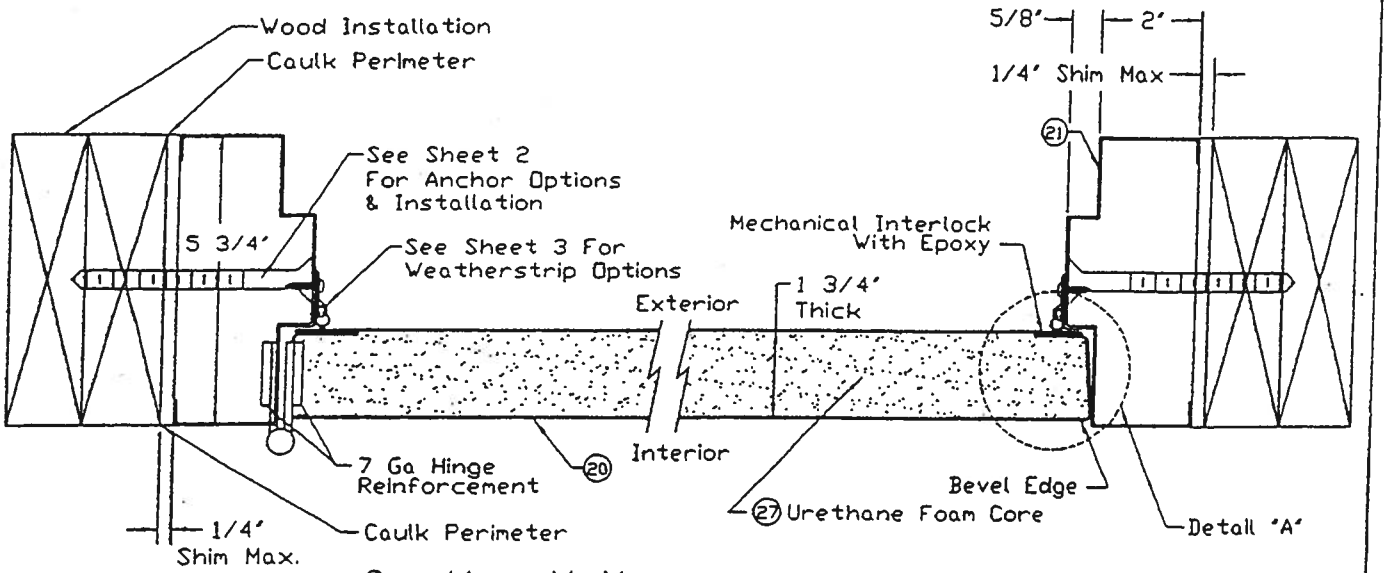
DRAWN BY: LT
DATE: 5/22/02

DRAWING NUMBER: RD0728
Sheet 5 of 9



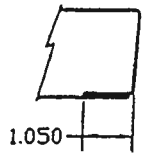
Section Y-Y

Note: Structural member at Head Must Be Designed To carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section X-X

Note: Top & Bottom Channels Assembled Std Method To Skins With Spot Welds & Tape. Channels Are Then Tack Welded To Both Skins 3' From Lock Edge And 6 Inches On Center.



Detail 'A'

Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02-0307.04
Miami-Dade Product Control
Division
By Ishaq J. Chanda

B	Revised Per Marked-Up Drawings From Ishaq Chanda.
LT	

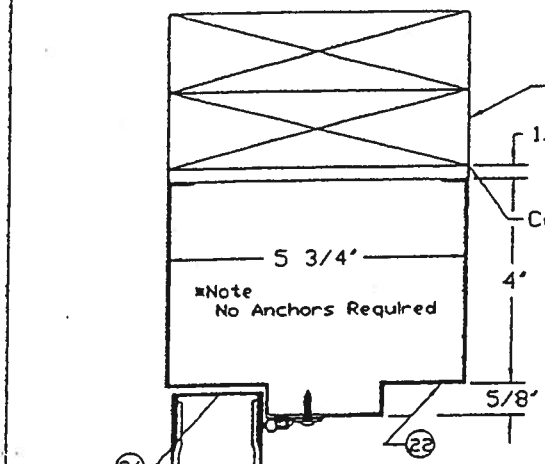
ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/23/02

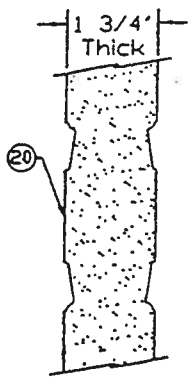
DRAWING NUMBER: RD0728
Sheet 7 of 9

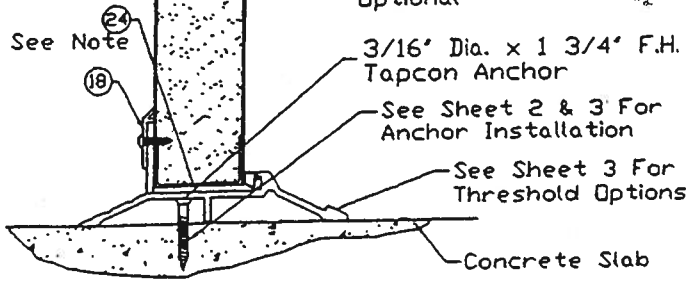
MATERIAL SPECIFICATIONS:

Cross Section View
(In-Swing Doors)
Imperial Handed Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

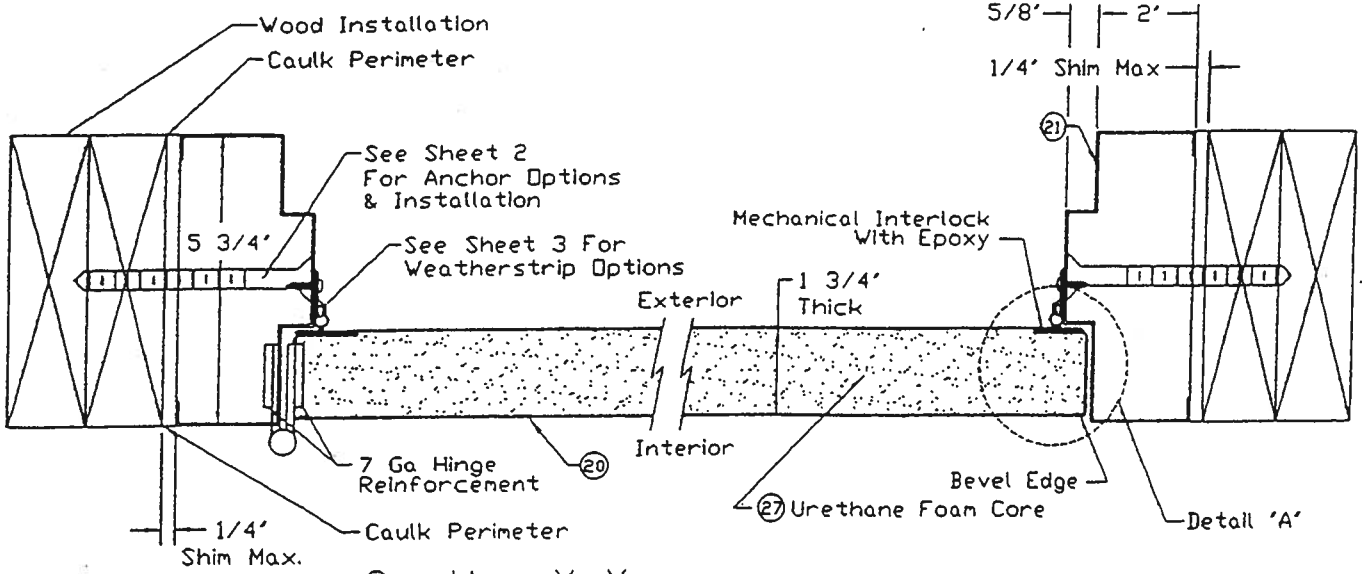






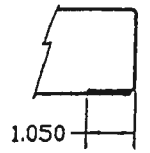
Section Y-Y

Note: Structural member at Head Must Be Designed To carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section X-X

Note: Top & Bottom Channels Assembled Std Method To Skins With Spot Welds & Tape. Channels Are Then Tack Welded To Both Skins 3" From Lock Edge And 6 Inches On Center.



Detail 'A'

Approved as complying with the Florida Building Code
Date: OCT 21, 2002
NOA# 02-0507.04
Miami Dade Product Control
Division
By: Isaac Chanda

B 9/4/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
ISSUE	REVISIONS

DRAWN BY: LT
DATE: 5/23/02

MATERIAL SPECIFICATIONS:

Cross Section View
(In-Swing Doors)
Versadoor Handed Door

CECO DOOR PRODUCTS
Mkan, Tennessee 38358

DRAWING NUMBER:
RD0728
Sheet 8 of 9

1	Cylindrical Lock & Lock Reinforcement (RD0528)	Schlage	AL53PD
1A	Deadbolt (Optional) Ⓓ	Schlage	B100
2	Or Cylindrical Lock & Lock Reinforcement	Saflok	Premier SL2500
3	Or Mortise Lock	Saflok	MT
4	Caulk	Dow Corning	899 Silicone Glazing Sealant
5	Threshold	Pemko	2005AV36
6	Or	Pemko	181AV36
7	Weatherstrip	Pemko	303AV3684
8	Hinge (Ball Bearing)	Hager or Equal (Attached w/ (8) #12-24 x 1/2 MS Per Hinge)	4-1/2 x 4-1/2 x .134 (Std Weight)
9	Or (Spring)	Hager or Equal (Attached w/ (8) #12-24 x 1/2 MS Per Hinge)	4-1/2 x 4-1/2 x .134 (Std Weight)
10	Weatherstrip	Pemko	S88
11	Frame Anchor	Masonary Tee (RD0057)	16 ga (.053' min) Galv Steel Fymin = 30ksi
12	Or	Wire, Relaxed Dimension 9' x 8'	#7 (.167 ' min) Galv Steel Wire (70,000 - 90,000 psi Tensile Strength)
13	Or	Expansion Bolt	3/8' x 5' F.H. Rawl Lok/Bolt Or 3/8' x 5' F.H. Ramset/RED Head
14	Or	Wood Lag Screw	3/8' x 4-5/8'
15	Viewer	Hager	1755
16	Or	MAG Security	8724-C
17	Drip Cap, Top	Pemko	346
18	Sweep	Pemko	315 N
19	Floor Anchor	Fixed Floor Anchor	16 ga (.053' min) galvanized Steel
20	Face Sheet A60 Galv Conforming To ASTM A653	Connercial Steel Type B (Minimum Yield Strength 30,000psi)	16 Ga (.053' min)
21	Series SF, Frame Jamb, Double Rabbet Profile, A60 Galv Conforming To ASTM A653	16 Ga (.053' min) Connercial Steel Type B (Minimum Yield Strength 30,000psi)	2' Face, 5-3/4' Depth Min. (RD0033)
22	Series SF, Frame Head, Double Rabbet, Profile A60 Galv Conforming To ASTM A653	16 Ga (.053' min) Connercial Steel Type B (Minimum Yield Strength 30,000psi)	4' Face, 5-3/4' Depth Min. (RD0033)
23	Door Channels, Spot Welded To Bottom Skin Glued To Top Skin, Tack Welded To Both	16 Ga (.053' min) A60 Galv Conforming To ASTM A653 Connercial Steel Type B (Minimum Yield Strength 30,000psi)	16 ga (.053' min) x 1' x 1-3/4' x 1'
24	Door Channels, Spot Welded To Bottom Skin Taped To Top Skin, Tack Welded To Both	16 Ga (.053' min) A60 Galv Conforming To ASTM A653 Connercial Steel Type B (Minimum Yield Strength 30,000psi)	16 ga (.053' min x 1' x 1-3/4' x 1'
25	Closer Reinforcement (Optional)	12 Ga (.093' min) CS Type B	12 ga (.093' min) x 5-3/8' x 16'
26	Honeycomb Core	Non-Impregnated Kraft Paper Ⓔ	1.2' Nominal Cell Size
27	Urethane Core	Foam Enterprises	2 lb/ft³ Density

Approved as complying with the Florida Building Code
Date: Oct 31, 2002
NOA#: 02-0507.04
Miami-Dade Product Control Division
By: Ishaq I. Chanda

B	10/10/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
A	9/4/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

MATERIAL SPECIFICATIONS:

3-0 x 7-0 Series In-Swing Bill Of Materials

CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/28/02
DRAWING NUMBER: RD0728	
Sheet 9 Of 9	

H Seismic & Hurricane Ties

SIMPSON Strong-Tie

The H16-2 series has a presloped seat of 5/12 for double trusses.

The H connector series provides wind and seismic ties for trusses and rafters.

The presloped 5/12 seat of the H16 provides for a tight fit and reduced deflection. The strap length provides for various truss heights up to a maximum of 13½" *(H16 series)*. Minimum heel height for H16 series is 4".

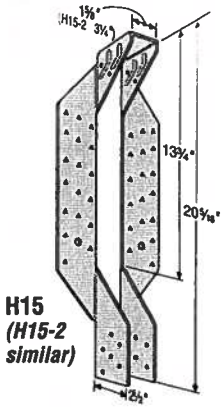
The HGA10 attaches to gable trusses and provides good lateral wind resistance. The HS24 attaches the bottom chord of a truss or rafter at pitches from 0:12 to 4:12 to double 2x4 top plates. Double shear nailing allows for higher lateral resistance.

MATERIAL: See table

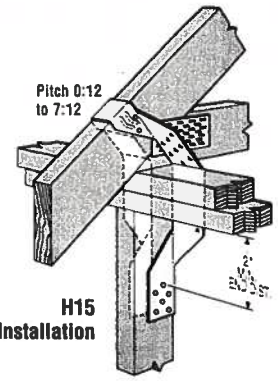
FINISH: Galvanized. See Corrosion-Resistance page 6-7.

INSTALLATION:
- Use all specified fasteners. See General Notes.
- The HGA10KT screws are provided.
- HS24 requires slant nailing only when bottom chord of truss or rafter has no slope.
- Hurricane Ties do not replace solid blocking.

CODES: See page 12 for Code Listing Key Chart.



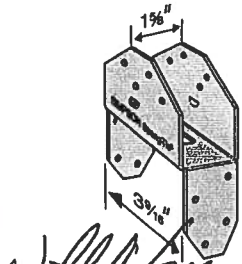
H15
(H15-2 similar)



H15 Installation

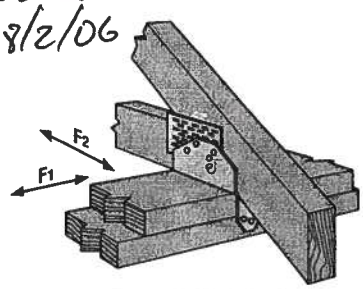
Model No.	Ga	Fasteners: To Rafters/Truss	Fasteners: To Plates	Fasteners: To Studs	DF/SP Allowable Loads[1]: Uplift (133)	DF/SP Allowable Loads[1]: Uplift (160)	DF/SP Allowable Loads[1]: Lateral (133/160) F_1	DF/SP Allowable Loads[1]: Lateral (133/160) F_2	SPF/HF Allowable Loads[1]: Uplift (133)	SPF/HF Allowable Loads[1]: Uplift (160)	SPF/HF Allowable Loads[1]: Lateral (133/160) F_1	SPF/HF Allowable Loads[1]: Lateral (133/160) F_2	Code Ref.
HGA10KT	14	4-SDS¼x1½	4-SDS¼x3	—	695	695	1165	940	595	595	870	815	125
HS24	18	8-8dx1½ & 2-8d slant	8-8d	—	605[3]	605[3]	645[3]	1025[3]	520	520	555	880	9, 62, 121
H15	16	4-10dx1½	4-10dx1½	12-10dx1½	1300	1300	480	—	1120	1120	410	—	6, 121
H15-2	16	4-10dx1½	4-10dx1½	12-10dx1½	1300	1300	480	—	1120	1120	410	—	
H16	18	2-10dx1½	10-10dx1½	—	1470	1470	—	—	1265	1265	—	—	125
H16S	18	2-10dx1½	10-10dx1½	—	1470	1470	—	—	1265	1265	—	—	
H16-2	18	2-10dx1½	10-10dx1½	—	1470	1470	—	—	1265	1265	—	—	
H16-2S	18	2-10dx1½	10-10dx1½	—	1470	1470	—	—	1265	1265	—	—	

1. Loads have been increased for earthquake or wind loading with no further increase allowed; reduce where other loads govern.
2. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
3. HS24 allowable loads without slant nailing are 625 lbs (uplift), 590 lbs (F1), 640 lbs (F2).
4. For H16-2S, S = short.
5. **NAILS: 10dx1½ = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8dx1½ = 0.131" dia. x 1½" long. See page 16-17 for other nail sizes and information.**

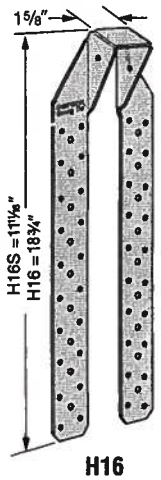


HS24
U.S. Patents 5,603,580

8/2/06

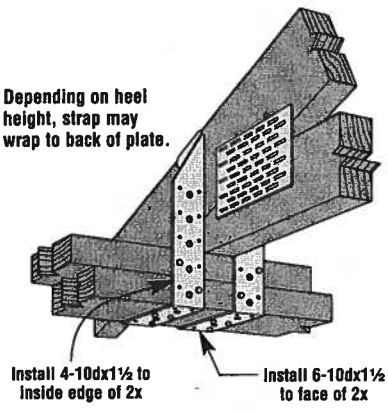


HS24 Installation

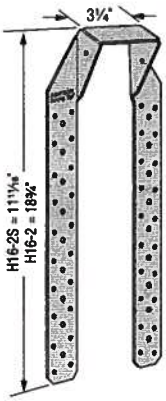


H16 and H16S

Presloped at 5:12. Pitch of 3:12 to 7:12 is acceptable

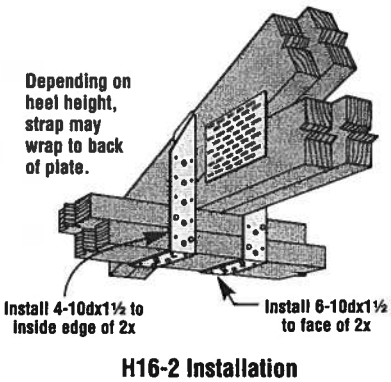


H16 Installation

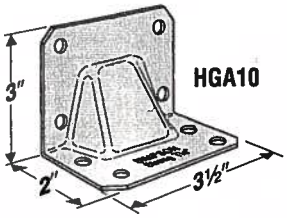


H16-2 and H16-2S

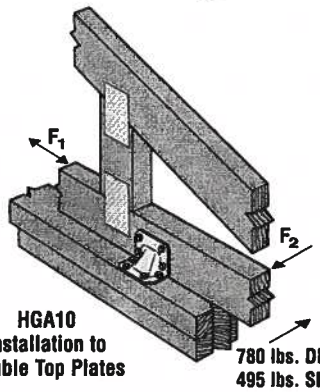
Presloped at 5:12. Pitch of 3:12 to 7:12 is acceptable



H16-2 Installation

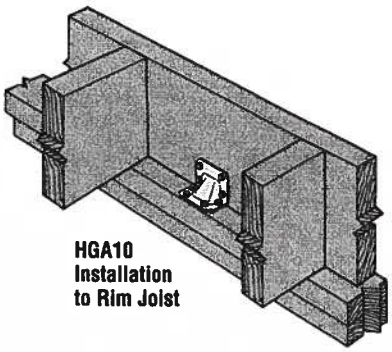


HGA10



HGA10 Installation to Double Top Plates

780 lbs. DF/SP
495 lbs. SPF/HF



HGA10 Installation to Rim Joist

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 11-4S-16-02914-324 **Building permit No.** 000024827

Use Classification SFD,UTILITY **Fire:** 61.38

Permit Holder TRENT GIEBEIG **Waste:** 184.25

Owner of Building PETE GIEBEIG **Total:** 245.63

Location: 258 SW LUCILLE CT(MAYFAIR, LOT 24, UNIT 3)

Date: 11/14/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

#24827

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JB109476 Company Phone No. 386-755-3611
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: Trent Bebry Trust Company Phone No.

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 264 SW Yucilla Ct Lake City FL

Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside 12 Inside 12 Type of Fill Mix

Section 4: Treatment Information

Date(s) of Treatment(s) 4-16-06
Brand Name of Product(s) Used Termidor
EPA Registration No. 7969-210
Approximate Final Mix Solution % 0.06 %
Approximate Size of Treatment Area: Sq. ft. 2314 Linear ft. 204 Linear ft. of Masonry Voids 246
Approximate Total Gallons of Solution Applied 434
Was treatment completed on exterior? ☐ Yes ☒ No
Service Agreement Available? ☒ Yes ☐ No
Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments

Name of Applicator(s) Steve Brannan Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Date 4-16-06

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001175

DATE 08/03/2006 PARCEL ID # 11-4S-16-02914-324

APPLICANT TRENT GIEBEIG PHONE 397-0545

ADDRESS 462 SW FAIRLINGTON COURT LAKE CITY FL 32055

OWNER PETE GIEBEIG PHONE 752-7968

ADDRESS 258 SW LUCILLE COURT LAKE CITY FL 32025

CONTRACTOR TRENT GIEBEIG PHONE 397-0545

LOCATION OF PROPERTY 247S, TR ON MAYFAIR LANE, TR ON LUCILLE COURT, END OF ROAD

SUBDIVISION/LOT/BLOCK/PHASE/UNIT MAYFAIR 24 3

SIGNATURE

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

Project Information for:	L200435		
Builder:	GIEBEG HOMES	Date:	7/5/2006
Lot:	LOT 24 MAYFAIR	Start Number:	**1217**
Subdivision:	N/A	SEI Ref:	L200435
County or City:	COLUMBIA COUNTY		
Truss Page Count:	33		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORRIS, JOHN DAVID RG 0066597
Address: 351 NW CORWIN GLN
LAKE CITY, FL. 32025 Designer: **107**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0705061217	7/5/2006				
2	CJ3	0705061218	7/5/2006				
3	CJ5	0705061219	7/5/2006				
4	EJ3	0705061220	7/5/2006				
5	EJ5	0705061221	7/5/2006				
6	EJ7	0705061222	7/5/2006				
7	HJ4	0705061223	7/5/2006				
8	HJ7	0705061224	7/5/2006				
9	HJ9	0705061225	7/5/2006				
10	T01	0705061226	7/5/2006				
11	T02	0705061227	7/5/2006				
12	T03	0705061228	7/5/2006				
13	T04	0705061229	7/5/2006				
14	T05	0705061230	7/5/2006				
15	T06	0705061231	7/5/2006				
16	T07	0705061232	7/5/2006				
17	T08	0705061233	7/5/2006				
18	T09	0705061234	7/5/2006				
19	T10	0705061235	7/5/2006				
20	T11	0705061236	7/5/2006				
21	T12	0705061237	7/5/2006				
22	T13	0705061238	7/5/2006				
23	T14	0705061239	7/5/2006				
24	T15	0705061240	7/5/2006				
25	T16	0705061241	7/5/2006				
26	T17	0705061242	7/5/2006				
27	T18	0705061243	7/5/2006				
28	T19	0705061244	7/5/2006				
29	T20	0705061245	7/5/2006				
30	T21	0705061246	7/5/2006				
31	T22	0705061247	7/5/2006				
32	T23	0705061248	7/5/2006				
33	T24	0705061249	7/5/2006				

JUL 05 2006

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Online Help

07:05:33 AM

Licensee Details

Licensee Information

Name:	**NORRIS, JOHN DAVID (Primary Name)**
	INDIVIDUAL (Alternate Name)
Main Address:	**351 NW CORWIN GLN** **LAKE CITY, Florida 32055**
Lic. Location:	**WOODGLEN DRIVE** **LAKE CITY, FL 32055** **Columbia**

License Information

License Type:	**Registered General Contractor**
Rank:	**Reg General**
License Number:	**RG0066597**
Status:	**Current, Active**
Licensure Date:	**06/20/1996**
Expires:	**08/31/2005**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
No Qualified Business License Required	02/20/2004

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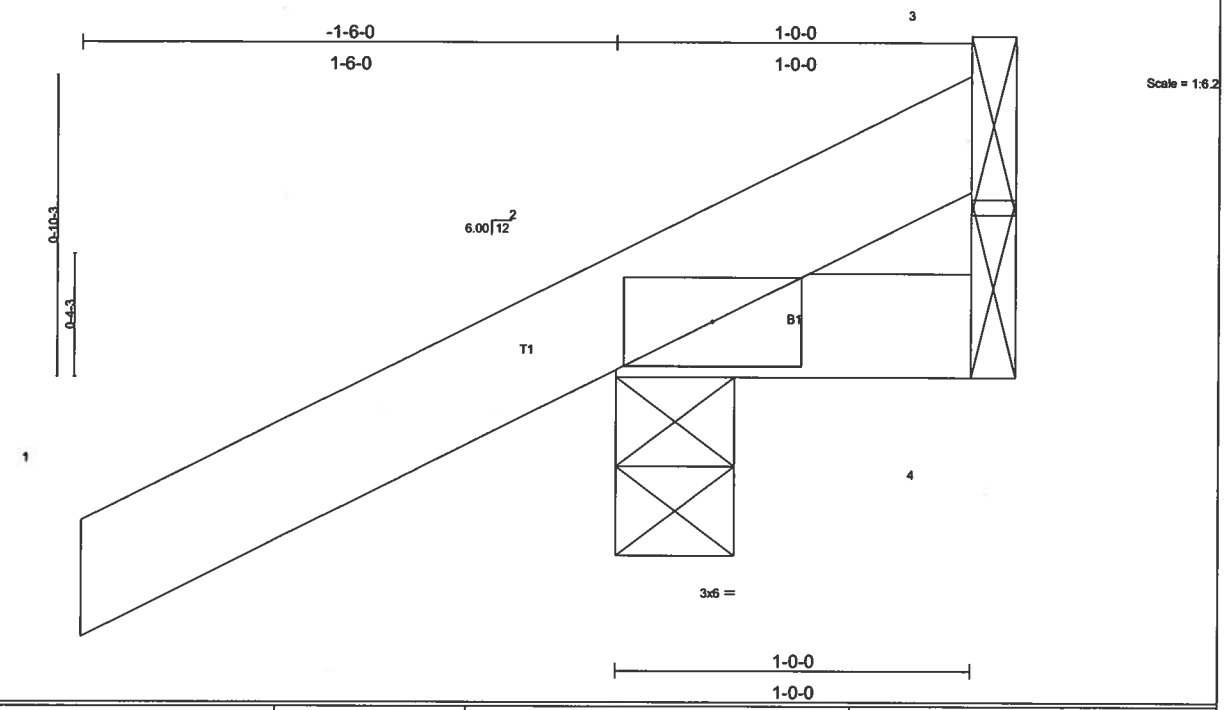
JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061217

Job	Truss	Truss Type	Qty	Ply	ST JOHNS MODEL III
L200435	CJ1	JACK	18	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:41:07 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-4-0, 4=14/Mechanical, 3=-41/Mechanical
Max Horz 2=70(load case 5)
Max Uplift2=-193(load case 5), 4=-9(load case 3), 3=-41(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=62(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/35
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 2, 9 lb uplift at joint 4 and 41 lb uplift at joint 3.

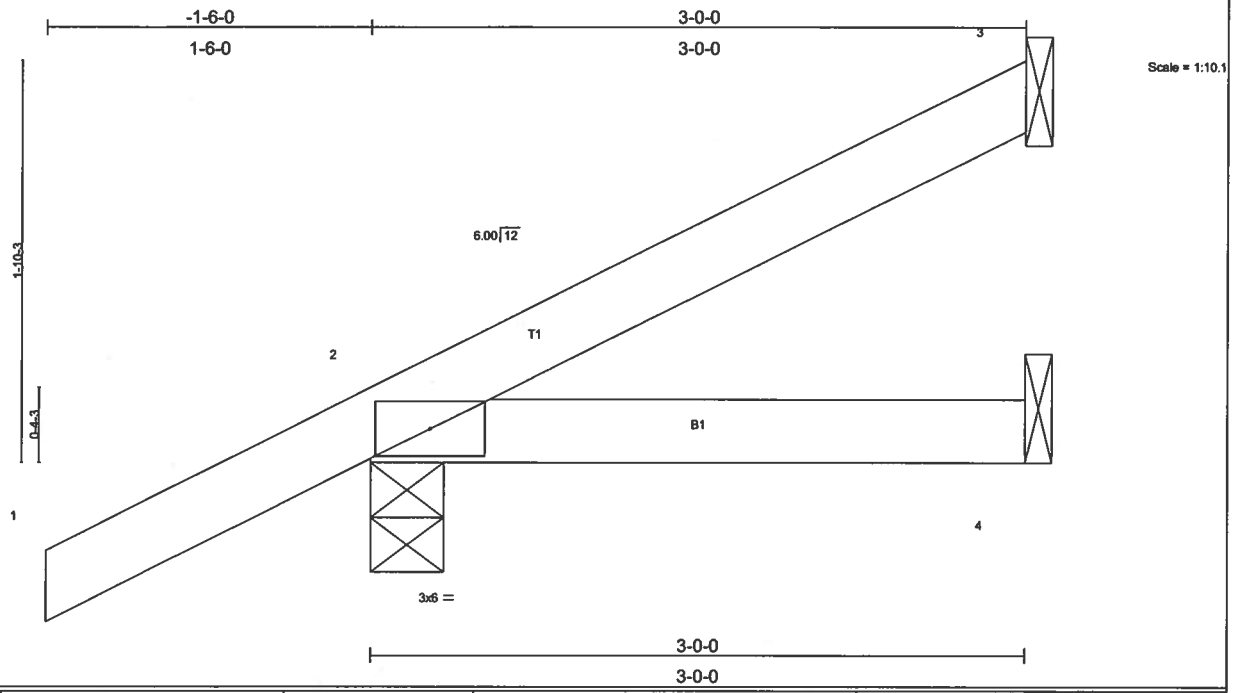
LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061218

Job	Truss	Truss Type	Qty	Ply	ST JOHNS MODEL III
L200435	CJ3	JACK	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:41:34 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=233/0-4-0, 4=42/Mechanical
Max Horz 2=115(load case 5)
Max Uplift3=-37(load case 5), 2=-187(load case 5), 4=-26(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-49/16
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3, 187 lb uplift at joint 2 and 26 lb uplift at joint 4.

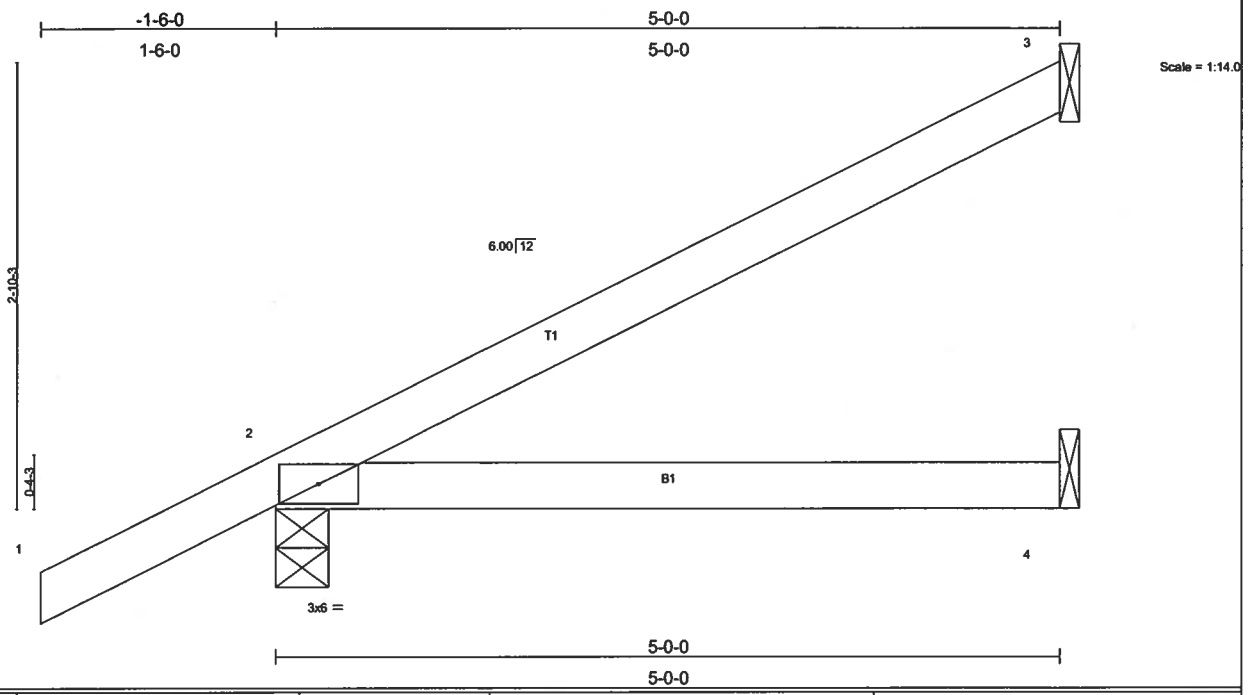
LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061219

Job	Truss	Truss Type	Qty	Ply	ST JOHNS MODEL III
L200435	CJ5	JACK	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:41:58 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.09	2-4	>671	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>784	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=113/Mechanical, 2=306/0-4-0, 4=72/Mechanical
Max Horz 2=162(load case 5)
Max Uplift3=-101(load case 5), 2=-219(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-96/41
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3, 219 lb uplift at joint 2 and 46 lb uplift at joint 4.

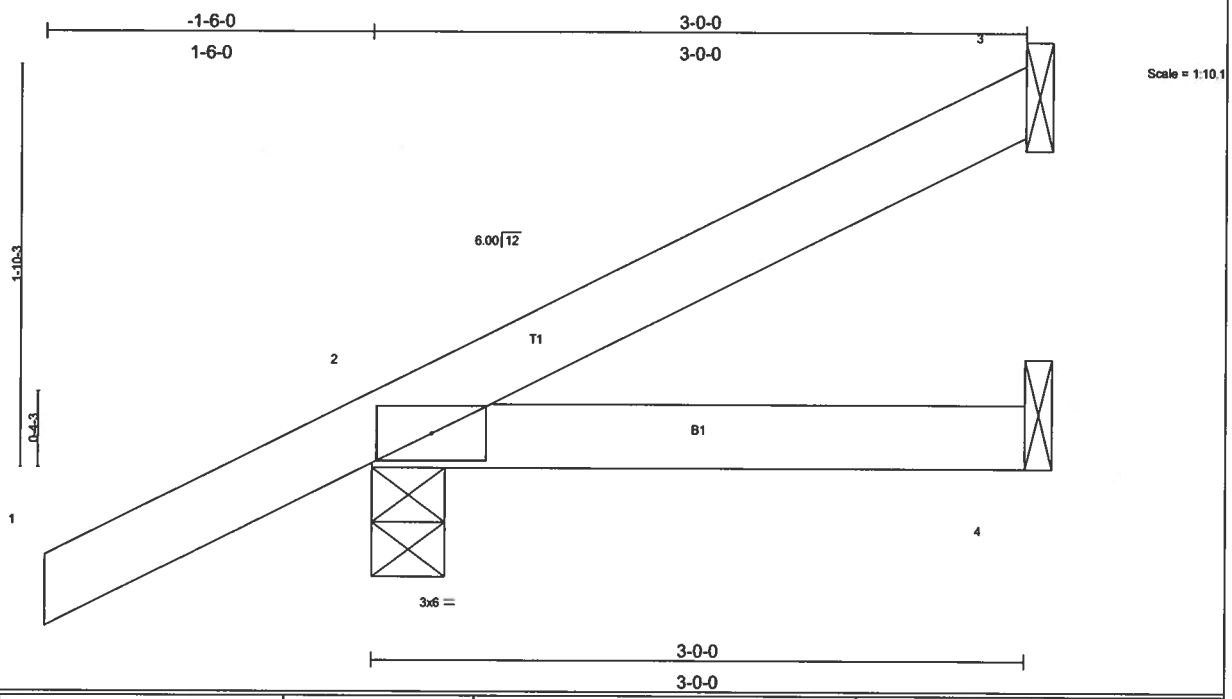
LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061220

Job	Truss	Truss Type	Qty	Ply	ST JOHNS MODEL III
L200435	EJ3	JACK	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:42:26 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=233/0-4-0, 4=42/Mechanical
Max Horz 2=115(load case 5)
Max Uplift3=-37(load case 5), 2=-187(load case 5), 4=-26(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-49/16
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3, 187 lb uplift at joint 2 and 26 lb uplift at joint 4.

LOAD CASE(S) Standard

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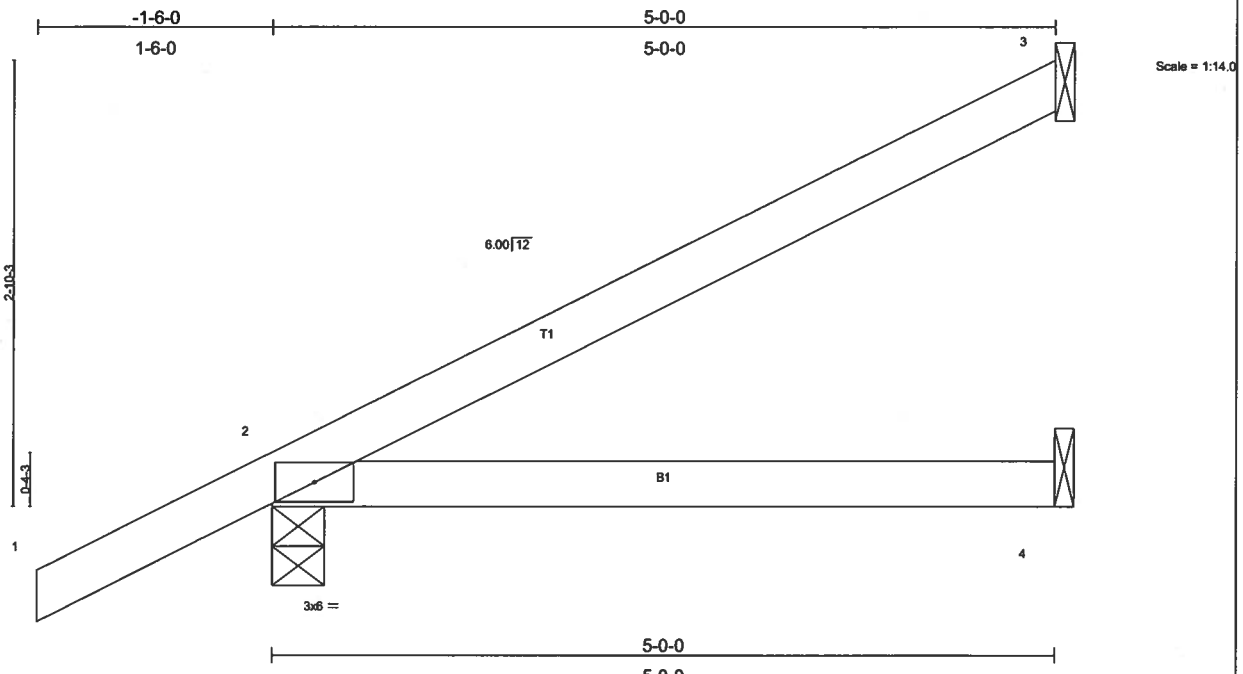
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061221

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	EJ5	JACK	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:36:44 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.09	2-4	>672	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>784	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=113/Mechanical, 2=306/0-4-0, 4=72/Mechanical
Max Horz 2=162(load case 5)
Max Uplift3=-101(load case 5), 2=-219(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-96/41
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3, 219 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061222

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	EJ7	MONO TRUSS	28	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:36:46 2006 Page 1

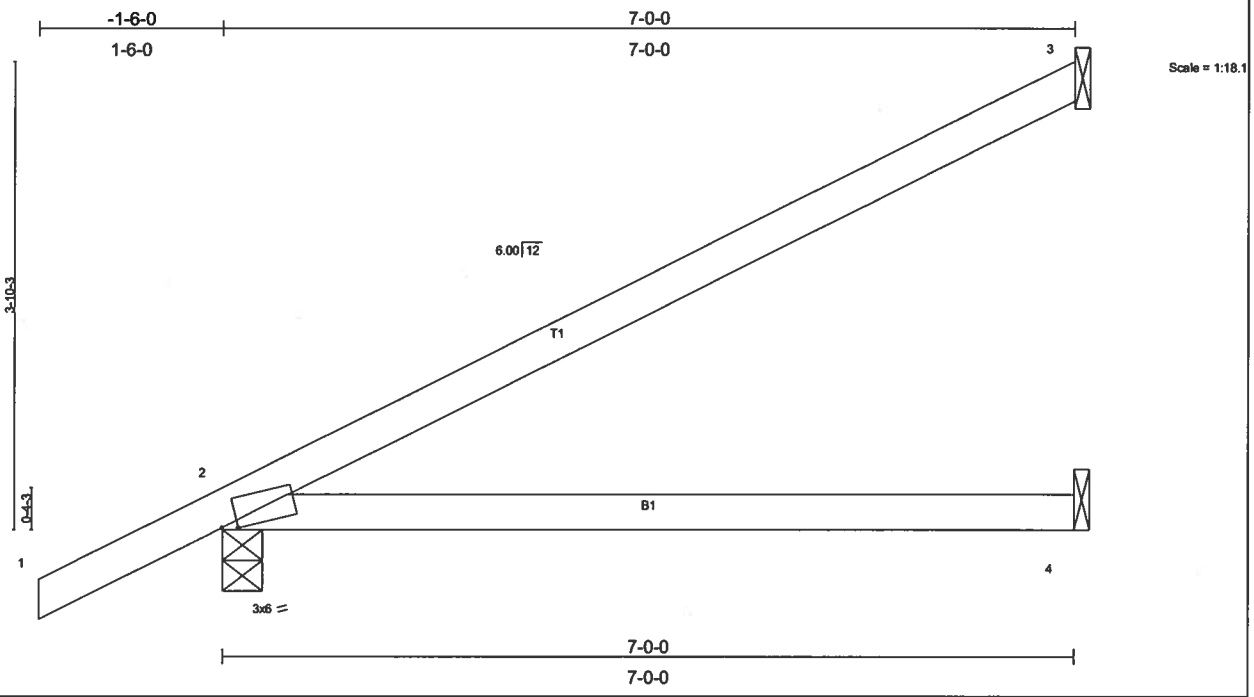


Plate Offsets (X,Y): [2:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.13	2-4	>614	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>370	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=165/Mechanical, 2=385/0-4-0, 4=108/Mechanical
Max Horz 2=208(load case 5)
Max Uplift3=-138(load case 5), 2=-173(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-122/59
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.91

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 3 and 173 lb uplift at joint 2.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061223

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	HJ4	JACK	2	1	Job Reference (optional)

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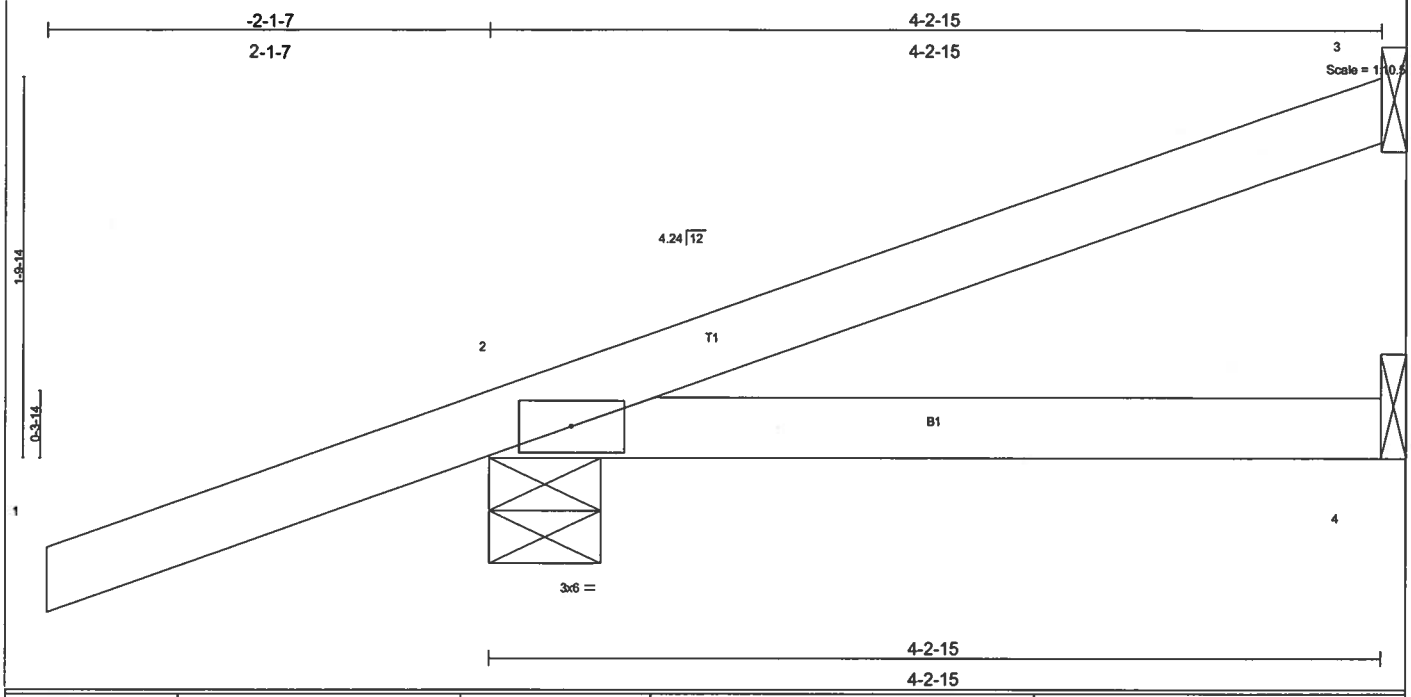

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=38/Mechanical, 2=231/0-6-7, 4=42/Mechanical
Max Horz 2=82(load case 2)
Max Uplift3=-15(load case 2), 2=-238(load case 2), 4=-41(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-29/6
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.09

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 3, 238 lb uplift at joint 2 and 41 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-3=-57(F=-2, B=-2), 2=0(F=15, B=15)-to-4=-32(F=-1, B=-1)

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061224

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	HJ7	JACK	2	1	Job Reference (optional)

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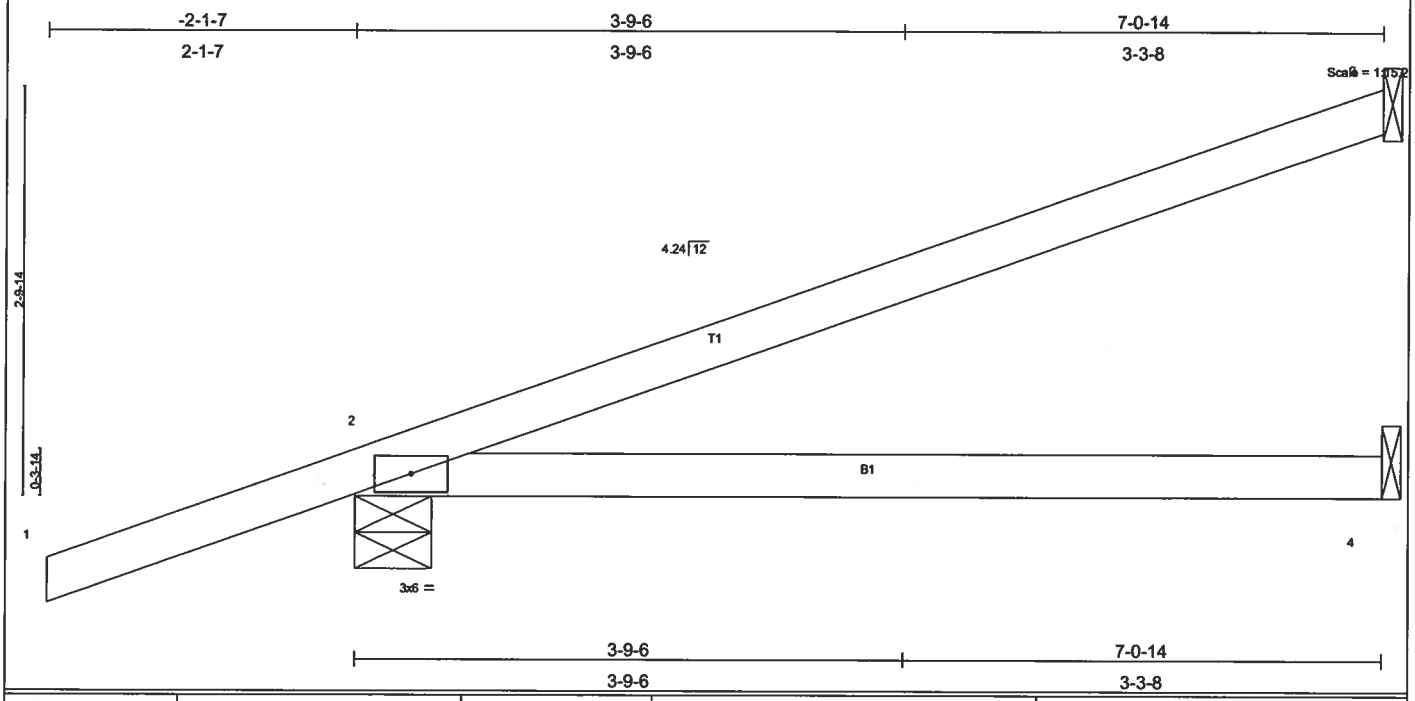

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	0.12	2-4	>670	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.16	2-4	>507	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=197/Mechanical, 2=327/0-6-7, 4=118/Mechanical
Max Horz 2=152(load case 2)
Max Uplift3=-162(load case 2), 2=-280(load case 2), 4=-57(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-74/47
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 3, 280 lb uplift at joint 2 and 57 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-3=-95(F=-21, B=-21), 2=0(F=15, B=15)-to-4=-53(F=-12, B=-12)

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061225

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	HJ9	MONO TRUSS	5	1	Job Reference (optional)

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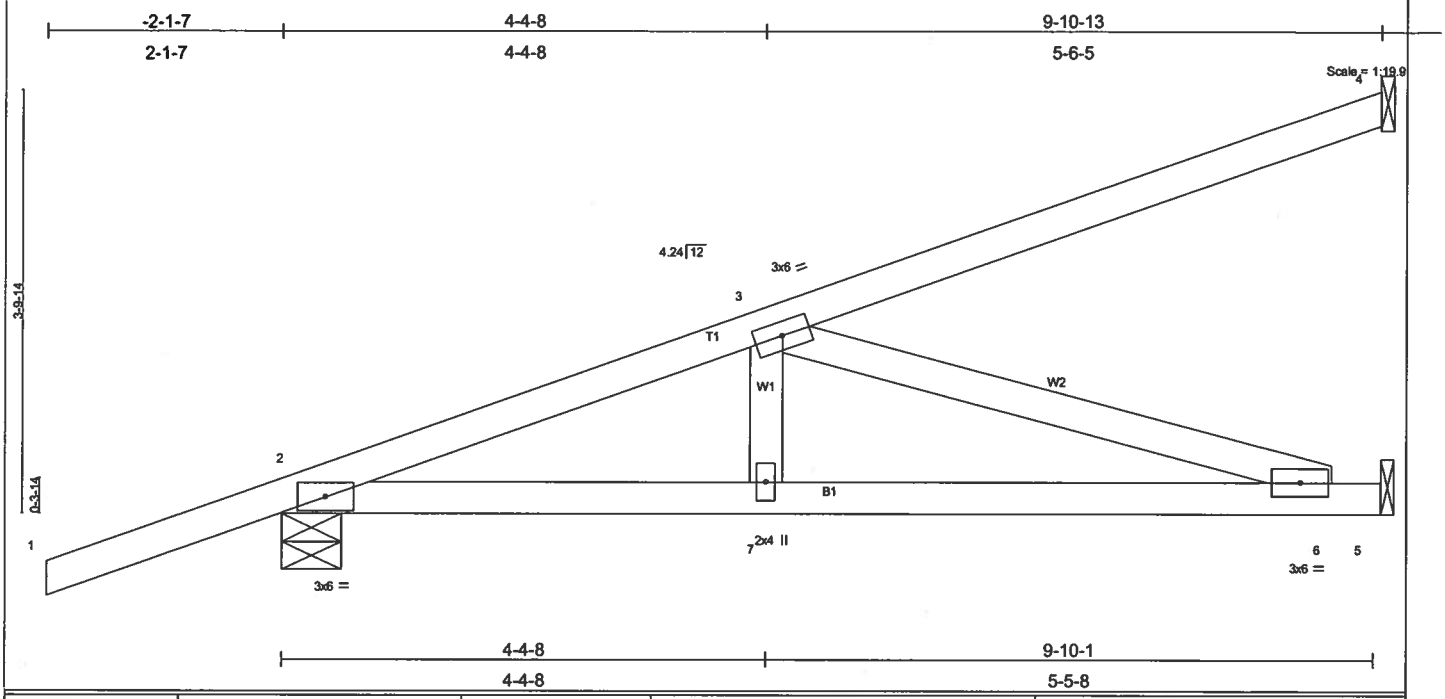

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.10	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.17	6-7	>665	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.47	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-3 oc bracing.

REACTIONS (lb/size) 4=264/Mechanical, 2=488/0-6-6, 5=389/Mechanical
Max Horz 2=253(load case 2)
Max Uplift4=-226(load case 2), 2=-231(load case 2), 5=-80(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-911/172, 3-4=-103/64
BOT CHORD 2-7=-360/846, 6-7=-360/846, 5-6=0/0
WEBS 3-7=0/208, 3-6=-885/377

JOINT STRESS INDEX
2 = 0.62, 3 = 0.23, 6 = 0.25 and 7 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 4, 231 lb uplift at joint 2 and 80 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061226

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T01	HIP	1	1	Job Reference (optional)

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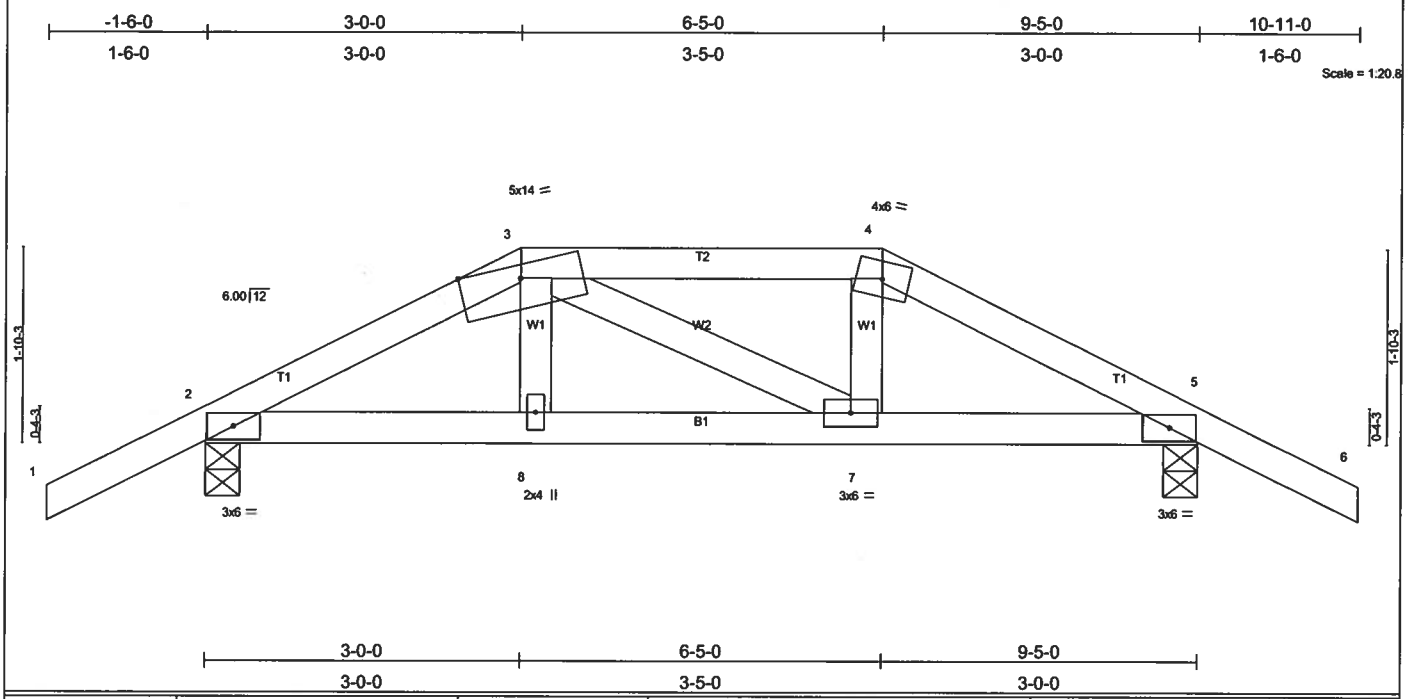


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.02	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.03	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=558/0-4-0, 5=558/0-4-0
Max Horz 2=-50(load case 5)
Max Uplift2=-372(load case 4), 5=-372(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-744/402, 3-4=-636/380, 4-5=-745/402, 5-6=0/35
BOT CHORD 2-8=-329/621, 7-8=-337/635, 5-7=-314/622
WEBS 3-8=-93/162, 3-7=-34/34, 4-7=-94/175

JOINT STRESS INDEX
2 = 0.40, 3 = 0.21, 4 = 0.21, 5 = 0.40, 7 = 0.11 and 8 = 0.12

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 2 and 372 lb uplift at joint 5.
5) Girder carries hip end with 3-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 32 lb up at 6-5-0, and 63 lb down and 32 lb up at 3-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

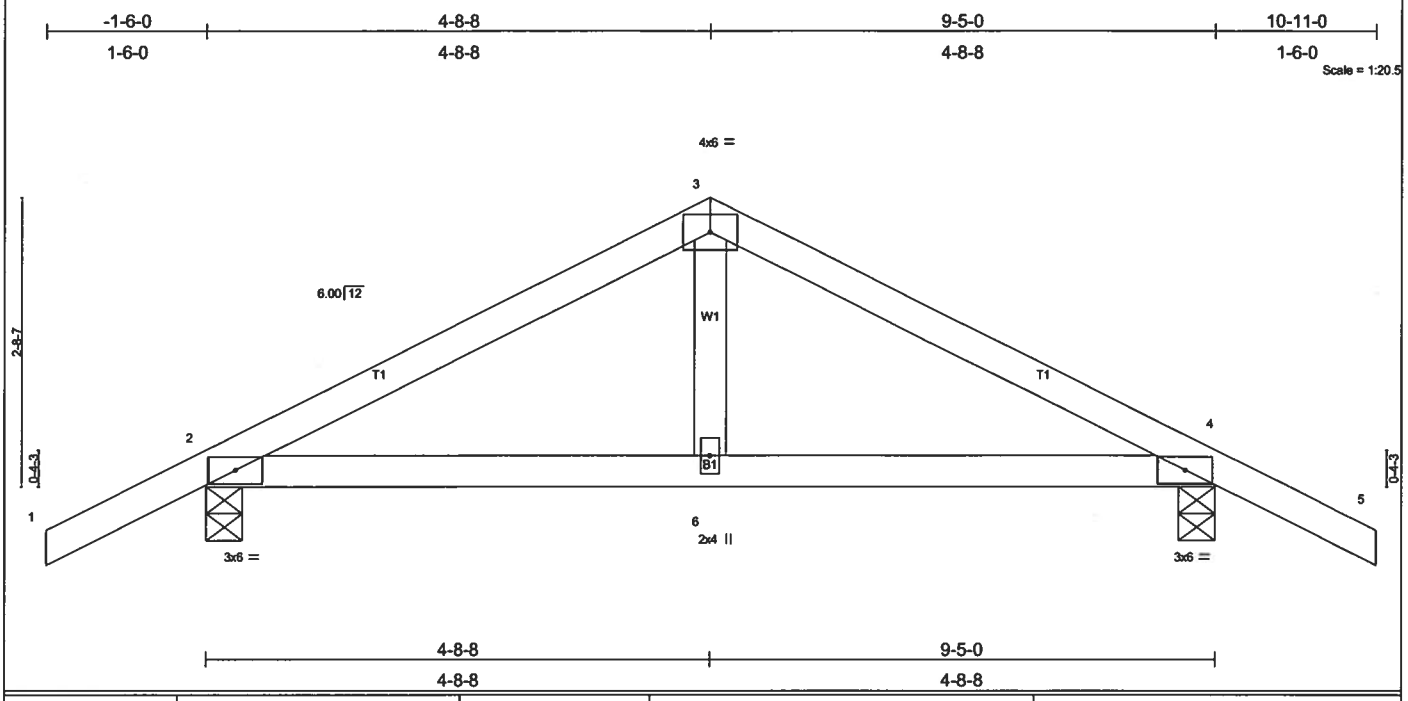
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-63(F=-9), 4-6=-54, 2-8=-30, 7-8=-35(F=-5), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-63(F) 7=-63(F)

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061227

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T02	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:36:59 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.03	4-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	0.03	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 38 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=472/0-4-0, 4=472/0-4-0
Max Horz 2=61(load case 5)
Max Uplift2=-338(load case 5), 4=-338(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-504/582, 3-4=-504/582, 4-5=0/35
BOT CHORD 2-6=-382/399, 4-6=-382/399
WEBS 3-6=-276/156

JOINT STRESS INDEX
2 = 0.36, 3 = 0.55, 4 = 0.36 and 6 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 338 lb uplift at joint 2 and 338 lb uplift at joint 4.

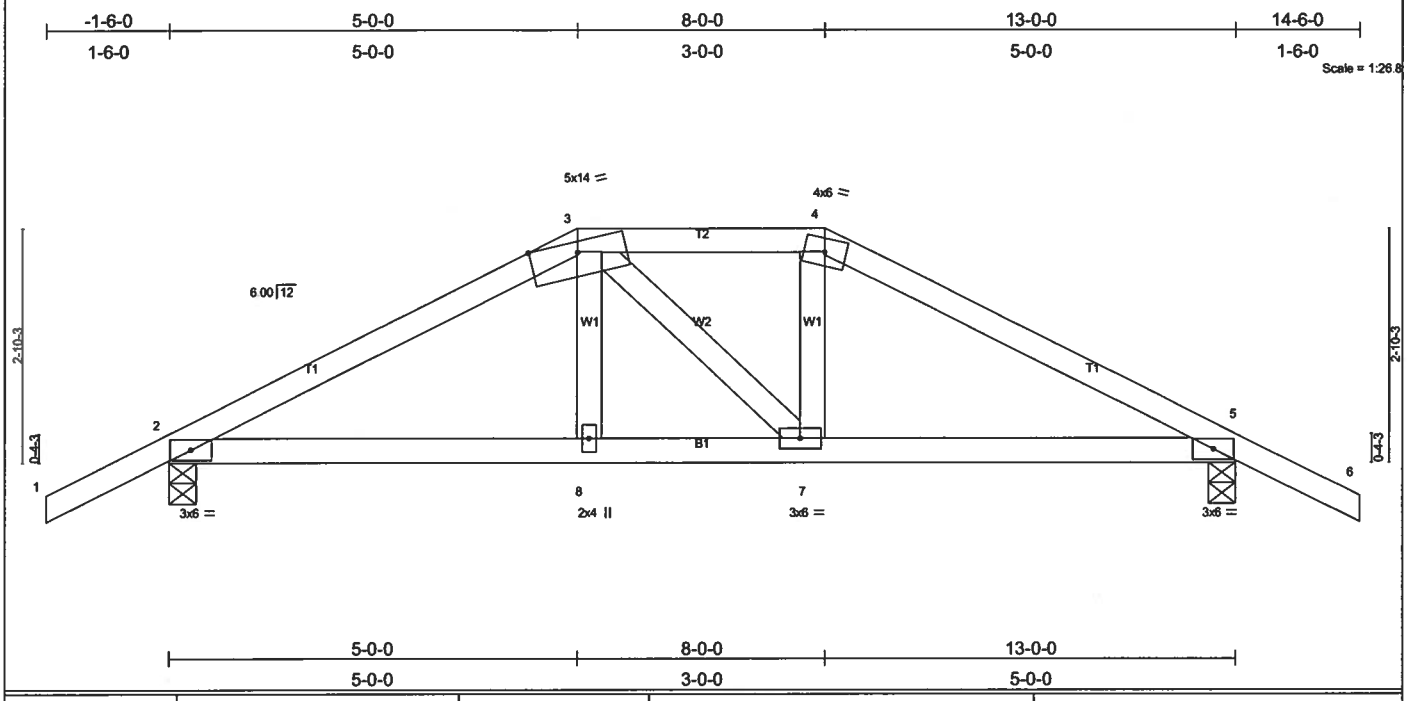
LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061228

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T03	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:02 2006 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.23	Vert(LL)	0.05	5-7	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.33	Vert(TL)	-0.07	2-8	>999	180		
BCLL 10.0	Rep Stress Incr NO	WB 0.14	Horz(TL)	0.03	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						Weight: 57 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-9-15 oc bracing.

REACTIONS (lb/size) 2=948/0-4-0, 5=948/0-4-0
Max Horz 2=-64(load case 5)
Max Uplift2=-591(load case 4), 5=-591(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1482/780, 3-4=-1284/751, 4-5=-1484/780, 5-6=0/35
BOT CHORD 2-8=-640/1259, 7-8=-651/1282, 5-7=-621/1261
WEBS 3-8=-201/394, 3-7=-79/84, 4-7=-220/441

JOINT STRESS INDEX
2 = 0.65, 3 = 0.46, 4 = 0.46, 5 = 0.66, 7 = 0.29 and 8 = 0.29

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 591 lb uplift at joint 2 and 591 lb uplift at joint 5.
5) Girder carries hip end with 5-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 126 lb up at 8-0-0, and 245 lb down and 126 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-90(F=-36), 4-6=-54, 2-8=-30, 7-8=-50(F=-20), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-245(F) 7=-245(F)

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061229

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T04	COMMON	3	1	Job Reference (optional)

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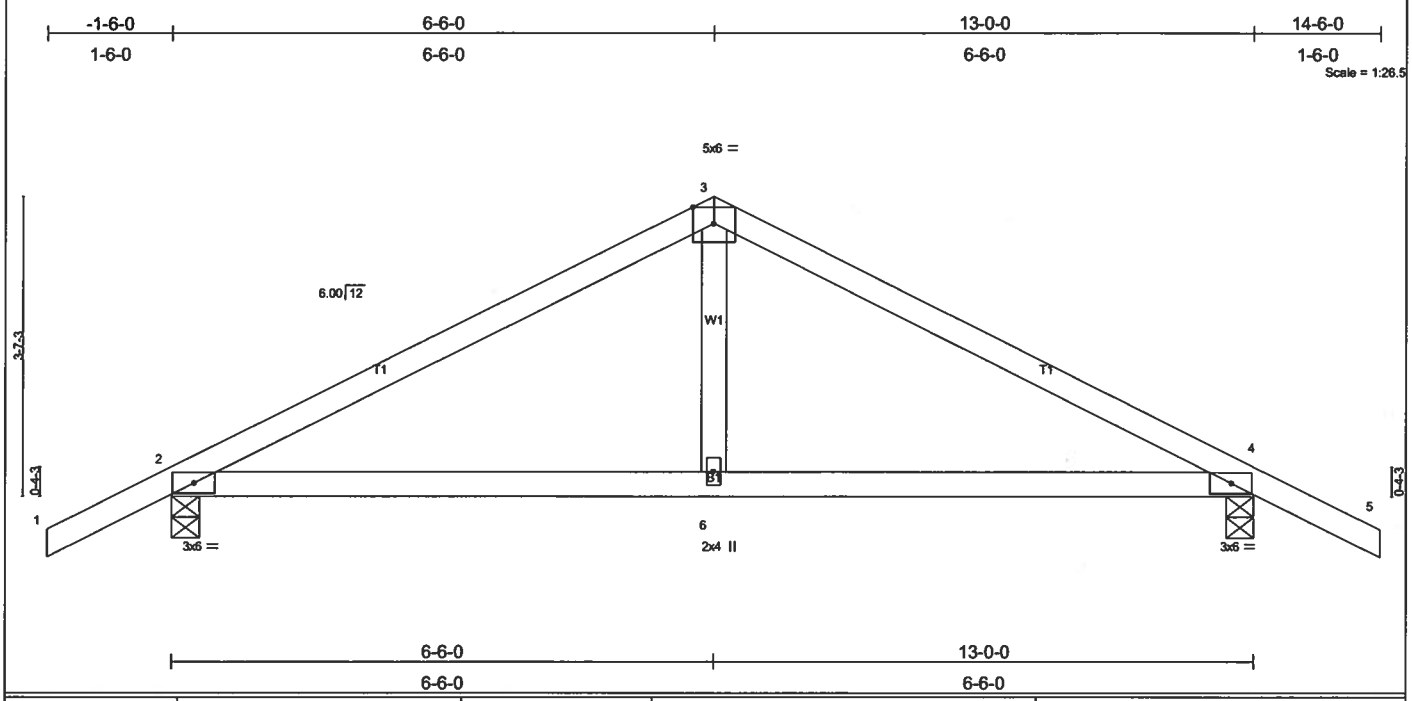

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.12	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	0.10	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 51 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-14 oc bracing.

REACTIONS (lb/size) 2=622/0-4-0, 4=622/0-4-0
Max Horz 2=-74(load case 6)
Max Uplift2=-430(load case 5), 4=-430(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-746/824, 3-4=-746/824, 4-5=0/35
BOT CHORD 2-6=-583/601, 4-6=-583/601
WEBS 3-6=-404/238

JOINT STRESS INDEX
2 = 0.42, 3 = 0.75, 4 = 0.42 and 6 = 0.17

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint 2 and 430 lb uplift at joint 4.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061230

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T05	HIP	1	1	Job Reference (optional)

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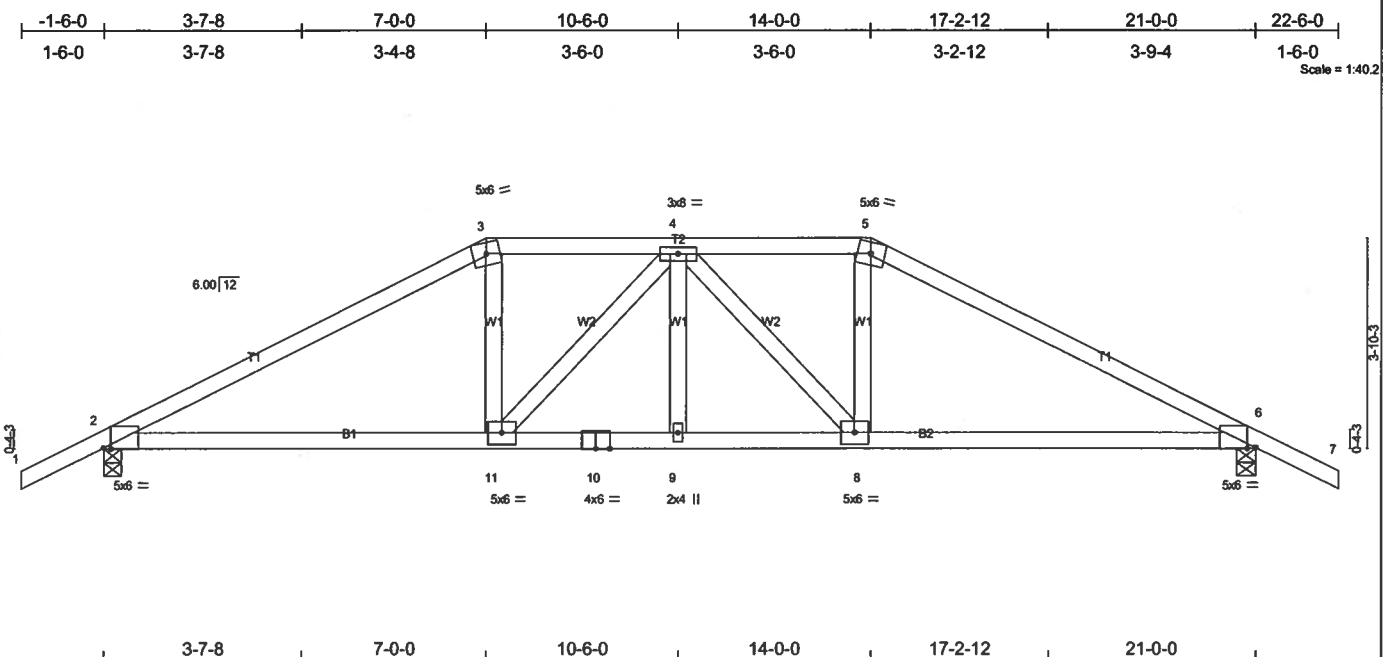

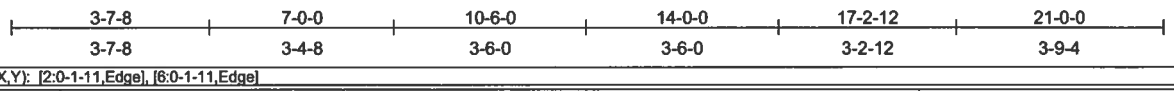

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.16	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.26	6-8	>963	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 98 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-7-6 oc bracing.

REACTIONS (lb/size) 2=1835/0-4-0, 6=1835/0-4-0
Max Horz 2=-78(load case 5)
Max Uplift2=-810(load case 4), 6=-810(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3318/1352, 3-4=-2920/1283, 4-5=-2920/1283, 5-6=-3318/1352, 6-7=0/35
BOT CHORD 2-11=-1138/2877, 10-11=-1264/3130, 9-10=-1264/3130, 8-9=-1264/3130, 6-8=-1091/2877
WEBS 3-11=-408/1105, 4-11=-436/257, 4-9=0/157, 4-8=-436/257, 5-8=-408/1105

JOINT STRESS INDEX
2 = 0.83, 3 = 0.78, 4 = 0.57, 5 = 0.78, 6 = 0.83, 8 = 0.39, 9 = 0.34, 10 = 0.93 and 11 = 0.39

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 810 lb uplift at joint 2 and 810 lb uplift at joint 6.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 14-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 5-7=-54, 2-11=-30, 8-11=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 11=-539(F) 8=-539(F)

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061231

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T06	HIP	1	1	Job Reference (optional)

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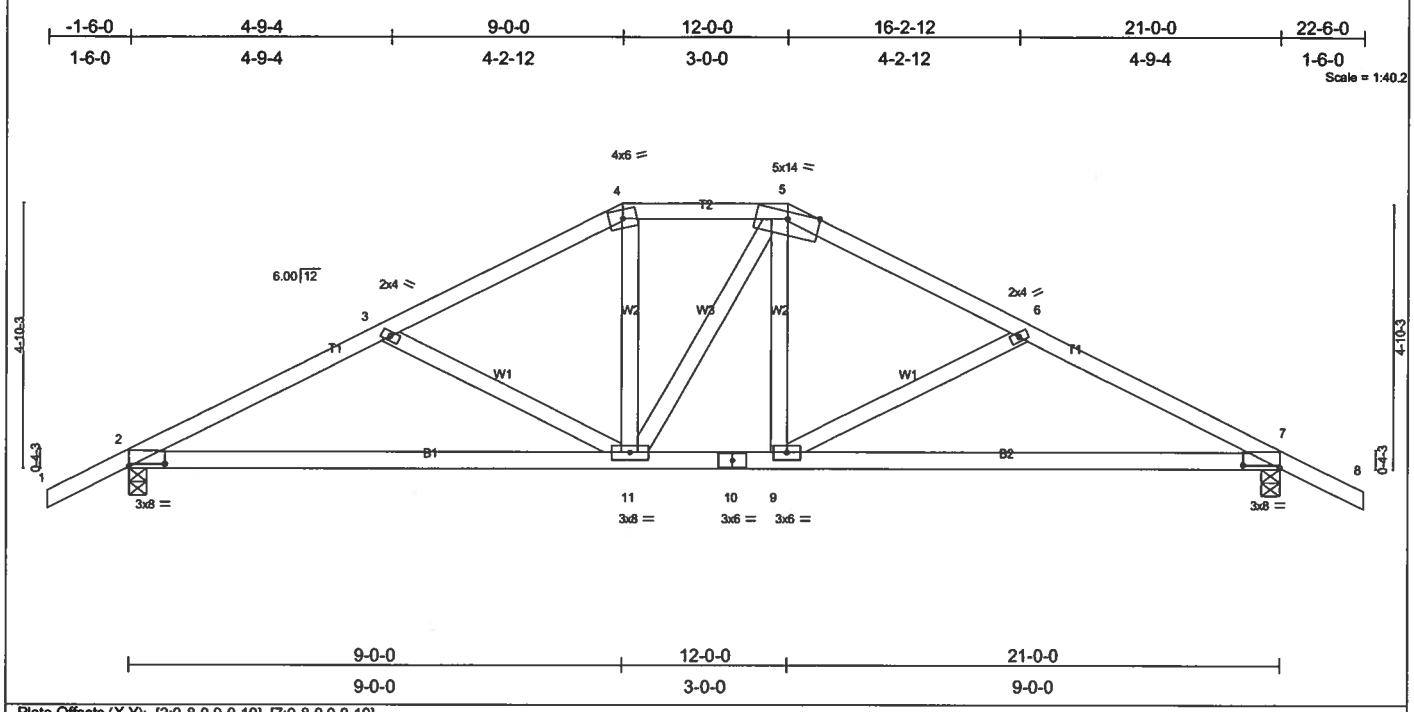

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [7:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.17	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.30	7-9	>839	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 105 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-7-6 oc bracing.

REACTIONS (lb/size) 2=958/0-4-0, 7=958/0-4-0
Max Horz 2=92(load case 5)
Max Uplift2=-370(load case 5), 7=-370(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1431/632, 3-4=-1164/507, 4-5=-998/503, 5-6=-1163/507, 6-7=-1431/632, 7-8=0/35
BOT CHORD 2-11=-428/1243, 10-11=-214/996, 9-10=-214/996, 7-9=-428/1243
WEBS 3-11=-289/243, 4-11=-74/308, 5-11=-101/106, 5-9=-75/309, 6-9=-291/243

JOINT STRESS INDEX
2 = 0.72, 3 = 0.34, 4 = 0.35, 5 = 0.29, 6 = 0.34, 7 = 0.73, 9 = 0.35, 10 = 0.62 and 11 = 0.60

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 2 and 370 lb uplift at joint 7.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061232

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T07	COMMON	6	1	Job Reference (optional)

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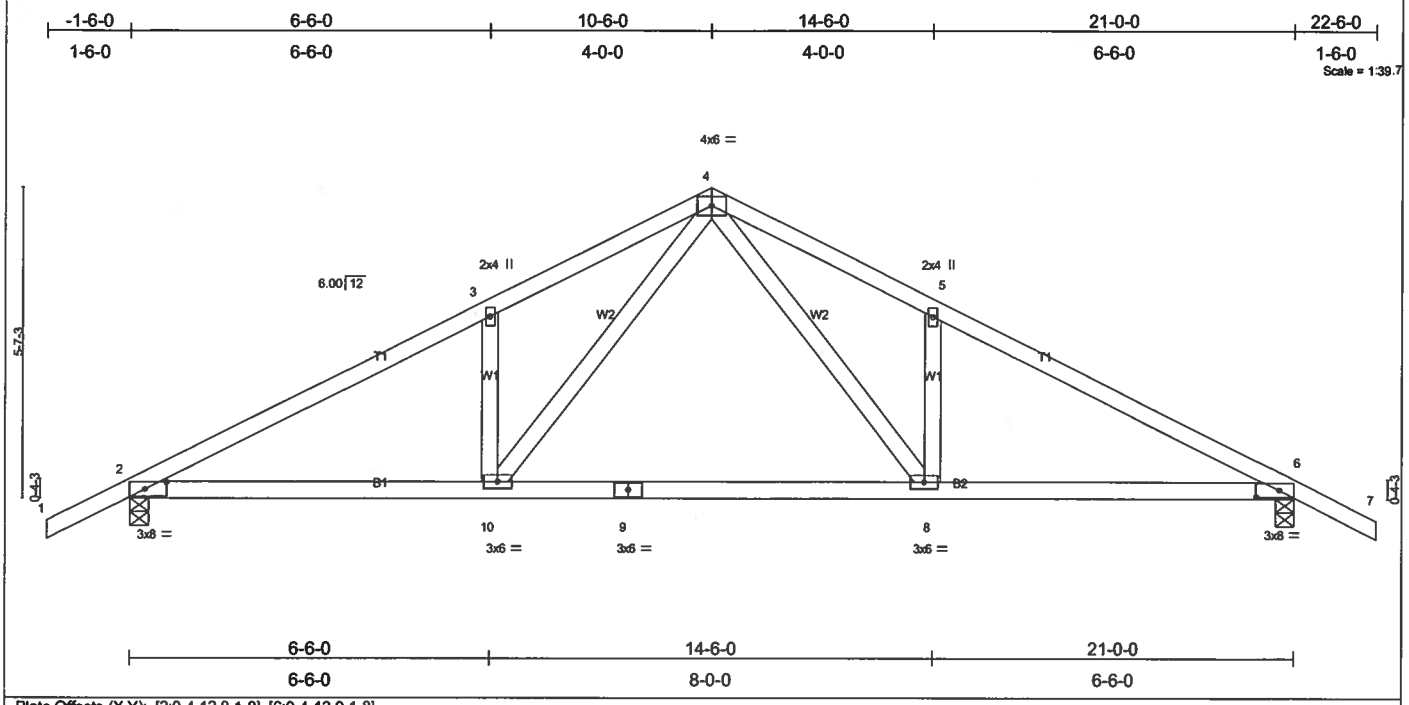

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [6:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.31	8-10	>793	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.51	8-10	>487	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.33	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-7 oc bracing.

REACTIONS (lb/size) 2=1158/0-4-0, 6=1158/0-4-0
Max Horz 2=-102(load case 6)
Max Uplift2=-454(load case 5), 6=-454(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1951/795, 3-4=-1911/939, 4-5=-1911/939, 5-6=-1951/795, 6-7=0/35
BOT CHORD 2-10=-549/1660, 9-10=-286/1101, 8-9=-286/1101, 6-8=-549/1660
WEBS 3-10=-269/284, 4-10=-437/940, 4-8=-437/940, 5-8=-269/284

JOINT STRESS INDEX
2 = 0.73, 3 = 0.34, 4 = 0.75, 5 = 0.34, 6 = 0.73, 8 = 0.67, 9 = 0.85 and 10 = 0.67

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 454 lb uplift at joint 2 and 454 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

JULY 05, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061233

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T08	HIP	1	1	Job Reference (optional)

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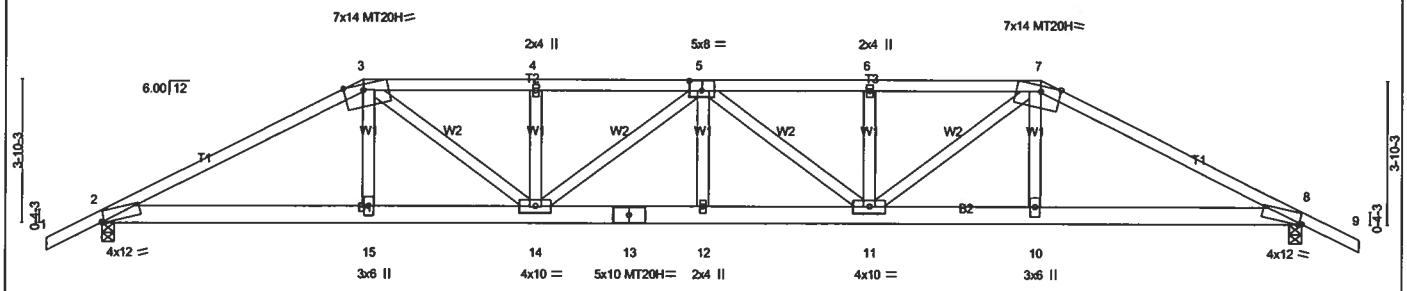
-1-6-0	3-9-4	7-0-0	11-7-2	16-0-8	20-5-14	25-1-0	28-3-12	32-1-0	33-7-0
1-6-0	3-9-4	3-2-12	4-7-2	4-5-6	4-5-6	4-7-2	3-2-12	3-9-4	1-6-0

Scale = 1:58.8



3-9-4	7-0-0	11-7-2	16-0-8	20-5-14	25-1-0	28-3-12	32-1-0
3-9-4	3-2-12	4-7-2	4-5-6	4-5-6	4-7-2	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-0-13,Edge], [3:0-6-3,Edge], [5:0-4-0,0-3-0], [7:0-6-3,Edge], [8:0-0-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.85	Vert(LL)	-0.47	12	>810	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.75	12	>506	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.66	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-10-2 oc bracing.

REACTIONS (lb/size) 2=2843/0-4-0, 8=2843/0-4-0
Max Horz 2=-80(load case 5)
Max Uplift2=-1194(load case 4), 8=-1194(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-5592/2357, 3-4=-6506/2860, 4-5=-6505/2860, 5-6=-6507/2861, 6-7=-6507/2861, 7-8=-5591/2357, 8-9=0/39
BOT CHORD 2-15=-2070/4917, 14-15=-2080/4952, 13-14=-3016/7061, 12-13=-3016/7061, 11-12=-3015/7065, 10-11=-2038/4951, 8-10=-2028/4916
WEBS 3-15=-241/834, 3-14=-963/2048, 4-14=-528/444, 5-14=-738/331, 5-12=0/336, 5-11=-737/327, 6-11=-536/450, 7-11=-965/2051, 7-10=-242/834

JOINT STRESS INDEX
2 = 0.83, 3 = 0.98, 4 = 0.34, 5 = 0.98, 6 = 0.34, 7 = 0.98, 8 = 0.83, 10 = 0.27, 11 = 0.99, 12 = 0.34, 13 = 0.96, 14 = 0.99 and 15 = 0.27

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1194 lb uplift at joint 2 and 1194 lb uplift at joint 8.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 25-1-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-117(F=-63), 7-9=-54, 2-15=-30, 10-15=-65(F=-35), 8-10=-30
Concentrated Loads (lb)
Vert: 15=-539(F) 10=-539(F)

JULY 05, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061234

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T09	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:17 2006 Page 1

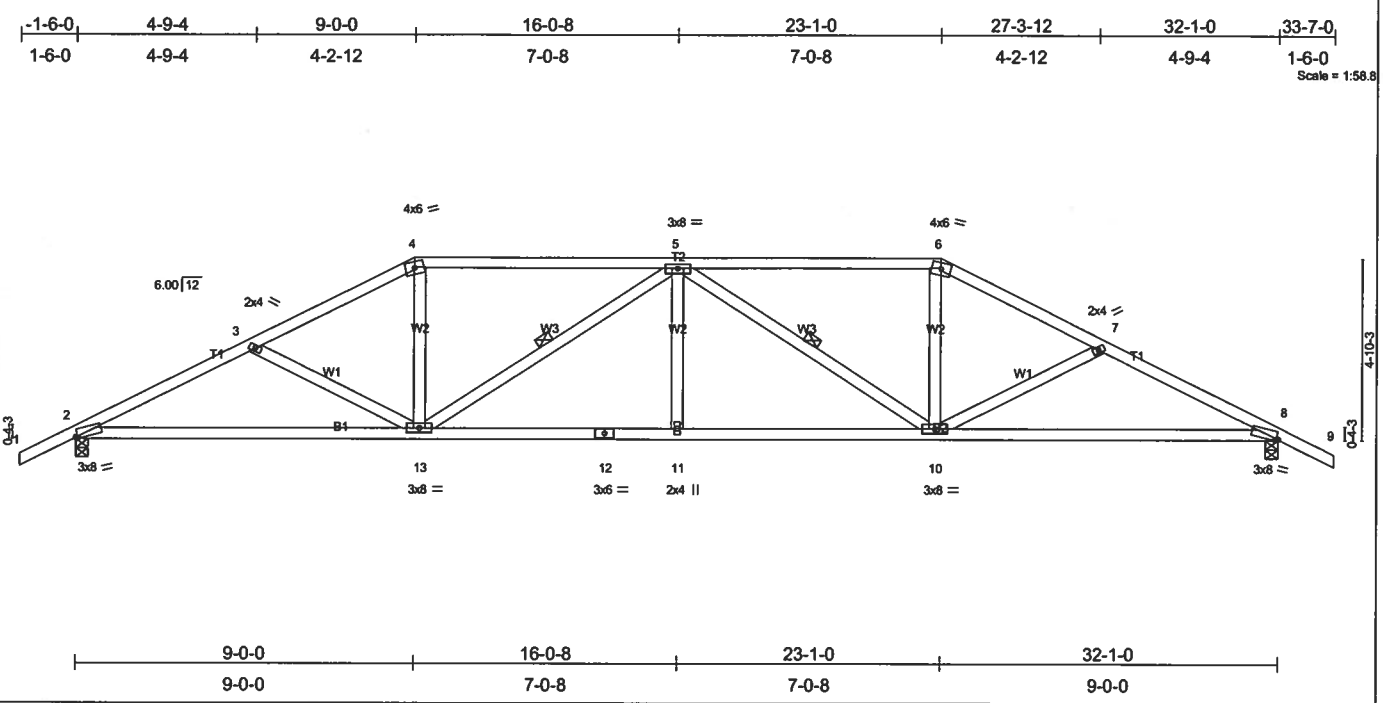


Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.22	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.36	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 161 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-0 oc bracing.
WEBS 1 Row at midpt 5-13, 5-10

REACTIONS (lb/size) 2=1423/0-4-0, 8=1423/0-4-0
Max Horz 2=92(load case 5)
Max Uplift2=-479(load case 6), 8=-479(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2416/1011, 3-4=-2195/910, 4-5=-1943/874, 5-6=-1943/874, 6-7=-2195/910, 7-8=-2416/1011, 8-9=0/35
BOT CHORD 2-13=-761/2108, 12-13=-772/2375, 11-12=-772/2375, 10-11=-772/2375, 8-10=-761/2108
WEBS 3-13=-214/204, 4-13=-163/644, 5-13=-613/270, 5-11=0/174, 5-10=-613/270, 6-10=-163/644, 7-10=-214/204

JOINT STRESS INDEX
2 = 0.81, 3 = 0.34, 4 = 0.77, 5 = 0.57, 6 = 0.77, 7 = 0.34, 8 = 0.81, 10 = 0.57, 11 = 0.34, 12 = 0.87 and 13 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 479 lb uplift at joint 2 and 479 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061235

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T10	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:21 2006 Page 1

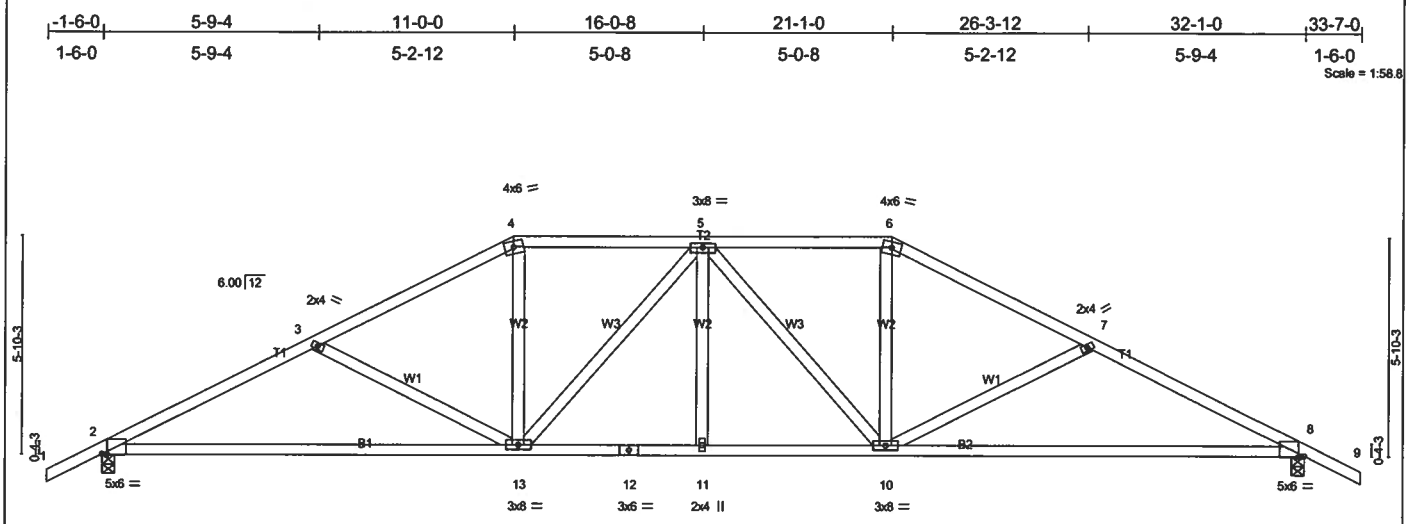


Plate Offsets (X,Y): [2:0-1-11,Edge], [8:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.38	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.65	8-10	>590	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-6 oc bracing.

REACTIONS (lb/size) 2=1423/0-4-0, 8=1423/0-4-0
Max Horz 2=106(load case 5)
Max Uplift2=-496(load case 5), 8=-496(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2366/1026, 3-4=-2043/870, 4-5=-1779/842, 5-6=-1779/842, 6-7=-2043/870, 7-8=-2366/1026, 8-9=0/35
BOT CHORD 2-13=-766/2070, 12-13=-574/1900, 11-12=-574/1900, 10-11=-574/1900, 8-10=-766/2070
WEBS 3-13=-350/302, 4-13=-160/608, 5-13=-299/178, 5-11=0/14, 5-10=-299/178, 6-10=-160/608, 7-10=-350/302

JOINT STRESS INDEX
2 = 0.72, 3 = 0.34, 4 = 0.65, 5 = 0.57, 6 = 0.65, 7 = 0.34, 8 = 0.72, 10 = 0.57, 11 = 0.34, 12 = 0.64 and 13 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 496 lb uplift at joint 2 and 496 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061236

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:23 2006 Page 1

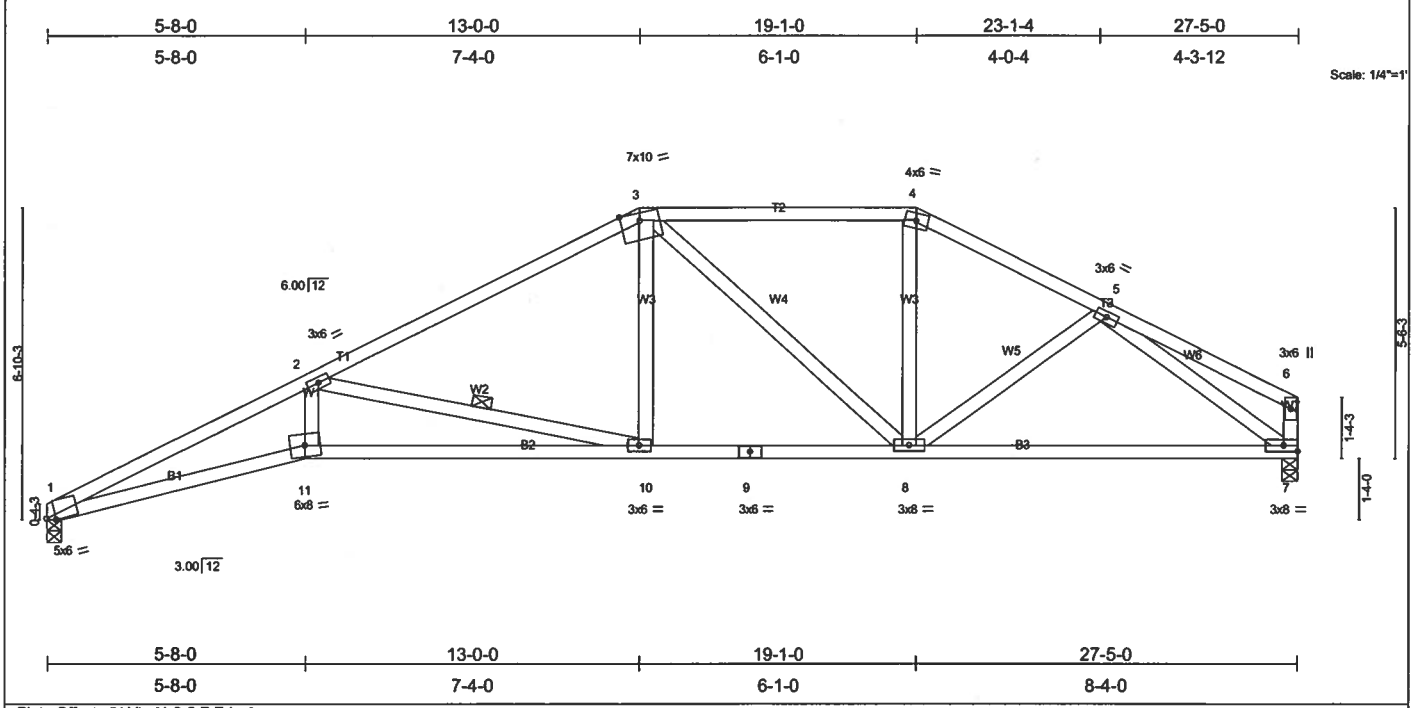


Plate Offsets (X,Y): [1:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.33	10-11	>989	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.53	10-11	>609	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.21	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-9 oc bracing.
WEBS 1 Row at midpt 2-10

REACTIONS (lb/size) 1=1138/0-4-0, 7=1138/0-4-0
Max Horz 1=172(load case 5)
Max Uplift1=-362(load case 5), 7=-331(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3848/1673, 2-3=-1794/797, 3-4=-1300/668, 4-5=-1486/691, 5-6=-363/115, 6-7=-264/134
BOT CHORD 1-11=-1581/3486, 10-11=-1499/3266, 9-10=-589/1549, 8-9=-589/1549, 7-8=-531/1229
WEBS 2-11=-339/1040, 2-10=-1775/936, 3-10=-171/593, 3-8=-412/188, 4-8=-120/375, 5-8=-49/191, 5-7=-1208/615

JOINT STRESS INDEX
1 = 0.99, 2 = 0.77, 3 = 0.77, 4 = 0.62, 5 = 0.41, 6 = 0.38, 7 = 0.65, 8 = 0.57, 9 = 0.51, 10 = 0.50 and 11 = 0.90

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 362 lb uplift at joint 1 and 331 lb uplift at joint 7.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061237

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:27 2006 Page 1

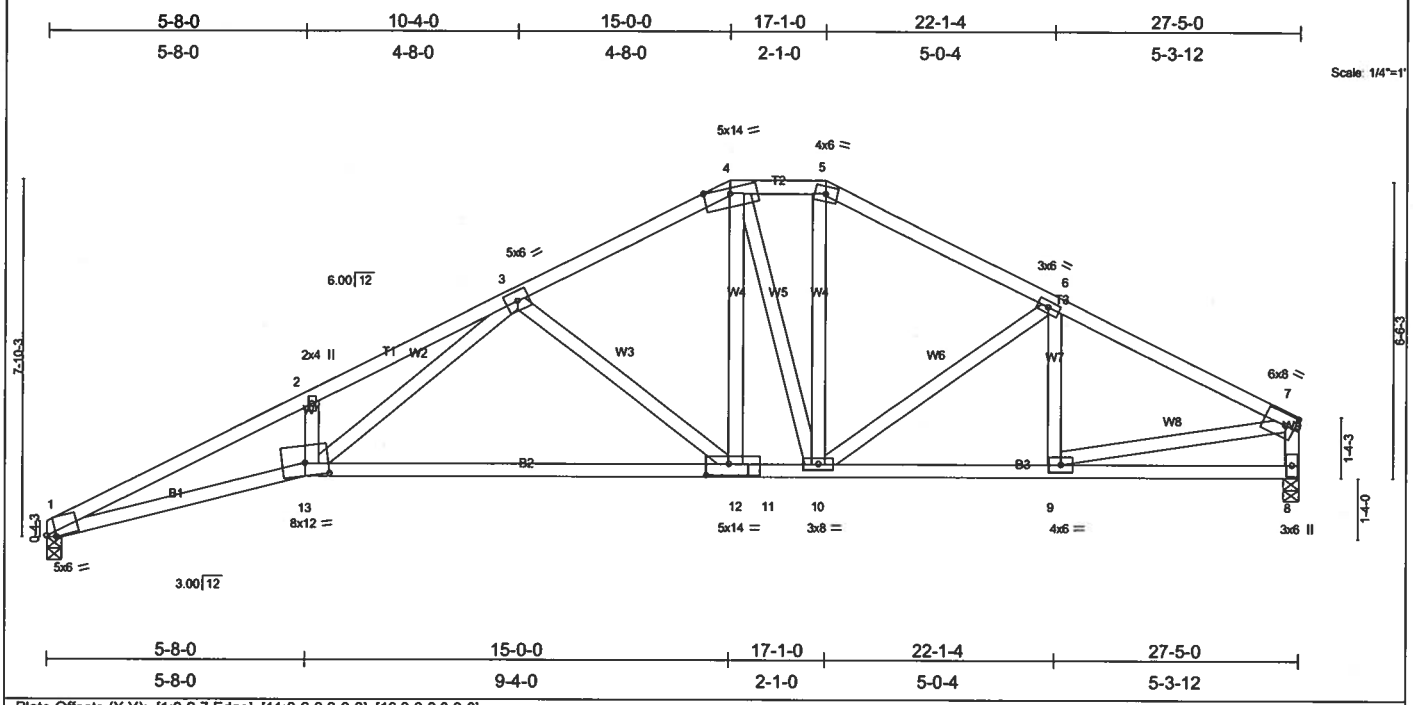


Plate Offsets (X,Y): [1:0-2-7,Edge], [11:0-6-0,0-3-0], [13:0-6-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.41	12-13	>799	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.67	12-13	>484	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.19	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 155 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-10 oc bracing.

REACTIONS (lb/size) 1=1138/0-4-0, 8=1138/0-4-0
Max Horz 1=186(load case 5)
Max Uplift1=-372(load case 5), 8=-346(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3852/1639, 2-3=-3796/1791, 3-4=-1472/718, 4-5=-1190/676, 5-6=-1384/695, 6-7=-1553/688, 7-8=-1054/503
BOT CHORD 1-13=-1541/3471, 12-13=-812/1856, 11-12=-436/1278, 10-11=-436/1278, 9-10=-542/1332, 8-9=-107/195
WEBS 2-13=-173/252, 3-13=-911/1946, 3-12=-765/490, 4-12=-255/763, 4-10=-406/107, 5-10=-160/403, 6-10=-237/168, 6-9=-93/139, 7-9=-445/1163

JOINT STRESS INDEX
1 = 0.99, 2 = 0.34, 3 = 0.79, 4 = 0.31, 5 = 0.44, 6 = 0.41, 7 = 0.77, 8 = 0.33, 9 = 0.51, 10 = 0.68, 11 = 0.94, 12 = 0.00 and 13 = 0.90

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 1 and 346 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061238

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T13	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:30 2006 Page 1

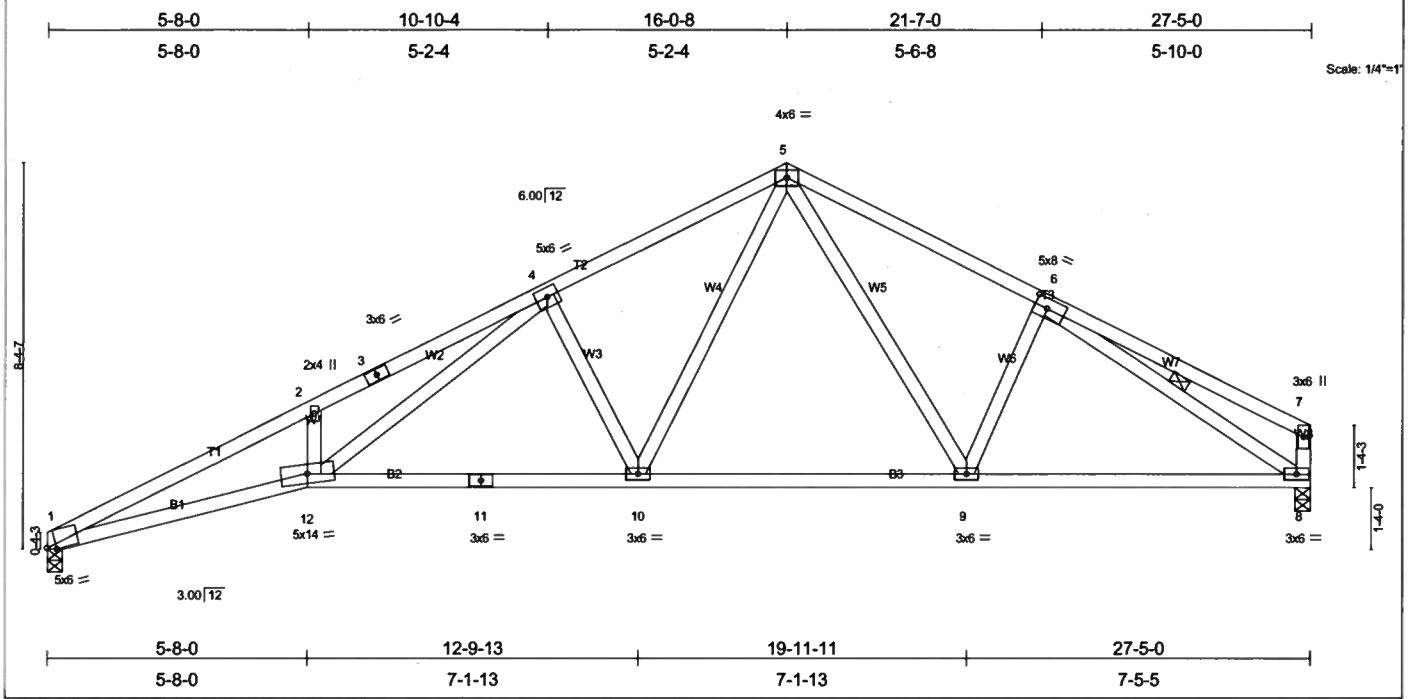


Plate Offsets (X,Y): [1:0-2-7,Edge], [6:0-3-6,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.32	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.51	10-12	>636	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.19	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 144 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-3 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 1=1138/0-4-0, 8=1138/0-4-0
Max Horz 1=193(load case 5)
Max Uplift1=-377(load case 5), 8=-353(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3796/1662, 2-3=-3756/1808, 3-4=-3702/1825, 4-5=-1765/898, 5-6=-1507/783, 6-7=-390/219, 7-8=-302/211
BOT CHORD 1-12=-1563/3418, 11-12=-772/1806, 10-11=-772/1806, 9-10=-396/1125, 8-9=-555/1319
WEBS 2-12=-199/268, 4-12=-976/1924, 4-10=-658/455, 5-10=-417/922, 5-9=-166/388, 6-9=-138/209, 6-8=-1261/536

JOINT STRESS INDEX
1 = 0.97, 2 = 0.34, 3 = 0.65, 4 = 0.79, 5 = 0.76, 6 = 0.27, 7 = 0.52, 8 = 0.76, 9 = 0.48, 10 = 0.78, 11 = 0.80 and 12 = 0.97

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 377 lb uplift at joint 1 and 353 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

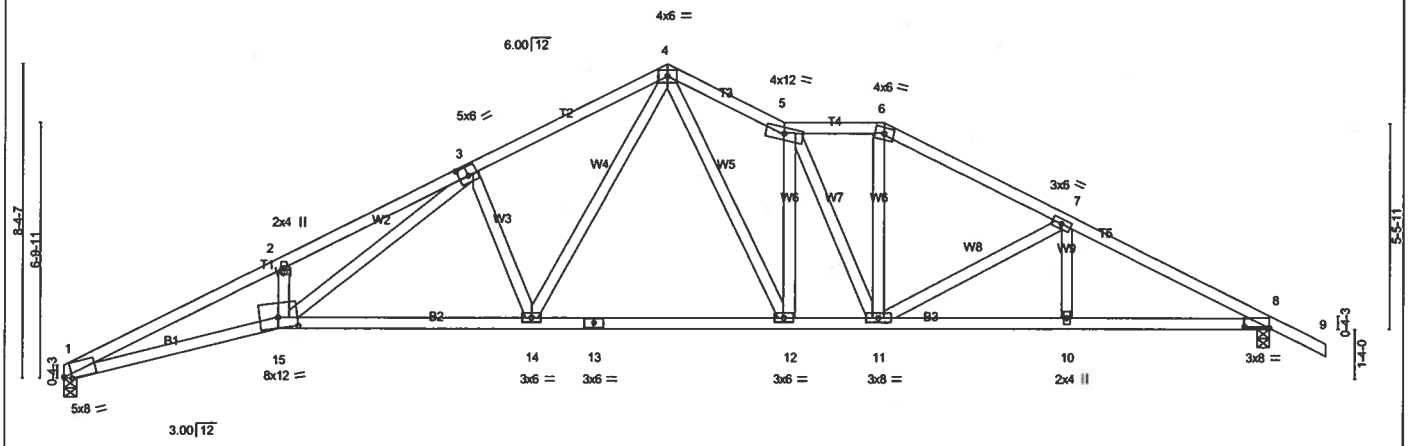
Dwg.#0705061239

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T14	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:33 2006 Page 1

5-8-0	10-10-4	16-0-8	19-2-0	21-10-0	26-8-4	32-1-0	33-7-0
5-8-0	5-2-4	5-2-4	3-1-8	2-8-0	4-10-4	5-4-12	1-6-0

Scale = 1:58.5



5-8-0	12-5-0	19-2-0	21-10-0	26-8-4	32-1-0
5-8-0	6-9-0	6-9-0	2-8-0	4-10-4	5-4-12

Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0], [8:0-8-0,0-0-6], [15:0-6-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.40	14-15	>942	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.65	14-15	>585	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.27	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 1=1331/0-4-0, 8=1426/0-4-0
Max Horz 1=139(load case 5)
Max Uplift1=-432(load case 5), 8=-529(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4542/1883, 2-3=-4488/2041, 3-4=-2370/1136, 4-5=-2226/1105, 5-6=-1799/891, 6-7=-2052/932, 7-8=-2505/1033, 8-9=0/35
BOT CHORD 1-15=-1646/4092, 14-15=-838/2314, 13-14=-454/1575, 12-13=-454/1575, 11-12=-628/1966, 10-11=-777/2164, 8-10=-777/2164
WEBS 2-15=-184/259, 3-15=-993/2114, 3-14=-710/469, 4-14=-459/1027, 4-12=-424/939, 5-12=-702/417, 5-11=-381/132, 6-11=-225/641, 7-11=-449/243, 7-10=0/171

JOINT STRESS INDEX
1 = 0.87, 2 = 0.34, 3 = 0.59, 4 = 0.84, 5 = 0.58, 6 = 0.53, 7 = 0.41, 8 = 0.78, 10 = 0.34, 11 = 0.64, 12 = 0.80, 13 = 0.52, 14 = 0.83 and 15 = 0.88

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 432 lb uplift at joint 1 and 529 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061240

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T15	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:36 2006 Page 1

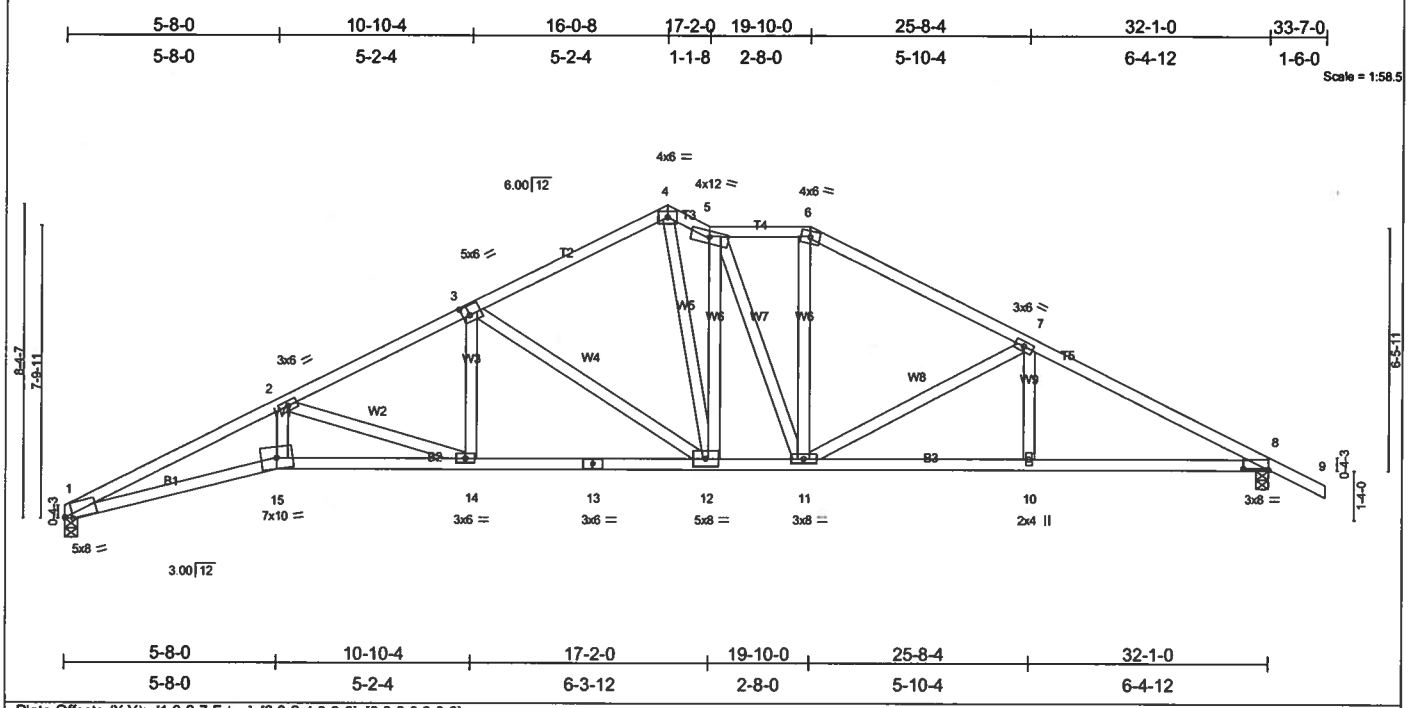


Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-2-4,0-3-0], [8:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.34	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.55	14-15	>690	180		
BCLL	10.0	Rep Stress Incr	YES	WB	1.00	Horz(TL)	0.27	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 180 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 1=1331/0-4-0, 8=1426/0-4-0
Max Horz 1=139(load case 5)
Max Uplift1=-432(load case 5), 8=-529(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4537/1899, 2-3=-2666/1166, 3-4=-1742/840, 4-5=-1779/890, 5-6=-1643/854, 6-7=-1898/882, 7-8=-2470/1025, 8-9=0/35
BOT CHORD 1-15=-1662/4102, 14-15=-1573/3861, 13-14=-838/2353, 12-13=-838/2353, 11-12=-488/1690, 10-11=-759/2128, 8-10=-759/2128
WEBS 2-15=-376/1136, 2-14=-1603/777, 3-14=-205/685, 3-12=-1007/507, 4-12=-476/1155, 5-12=-536/237, 5-11=-235/74, 6-11=-144/495, 7-11=-575/319, 7-10=0/212

JOINT STRESS INDEX
1 = 0.87, 2 = 0.84, 3 = 0.66, 4 = 0.60, 5 = 0.91, 6 = 0.69, 7 = 0.41, 8 = 0.77, 10 = 0.34, 11 = 0.66, 12 = 0.61, 13 = 0.91, 14 = 0.45 and 15 = 0.89

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 432 lb uplift at joint 1 and 529 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061241

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T16	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:39 2006 Page 1

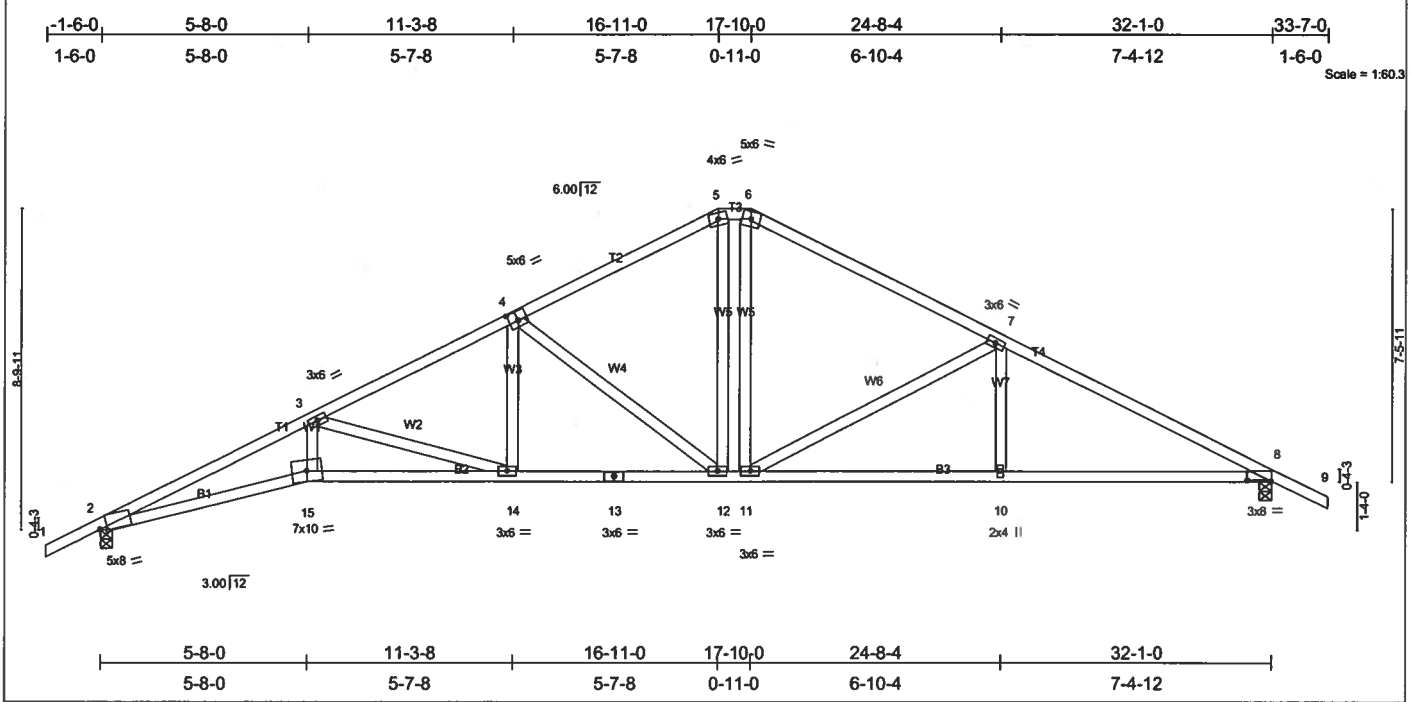


Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-0,0-3-0], [8:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.35	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.56	14-15	>678	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.27	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 170 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-11-10 oc bracing.

REACTIONS (lb/size) 2=1423/0-4-0, 8=1423/0-4-0
Max Horz 2=189(load case 5)
Max Uplift2=-533(load case 5), 8=-513(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4509/1828, 3-4=-2578/1123, 4-5=-1717/838, 5-6=-1484/814, 6-7=-1738/827, 7-8=-2420/1011, 8-9=0/35
BOT CHORD 2-15=-1590/4060, 14-15=-1510/3823, 13-14=-784/2240, 12-13=-784/2240, 11-12=-391/1484, 10-11=-735/2079, 8-10=-735/2079
WEBS 3-15=-338/1127, 3-14=-1637/753, 4-14=-187/658, 4-12=-971/505, 5-12=-303/629, 6-11=-148/515, 7-11=-713/394, 7-10=0/257

JOINT STRESS INDEX
2 = 0.87, 3 = 0.84, 4 = 0.55, 5 = 0.62, 6 = 0.59, 7 = 0.41, 8 = 0.76, 10 = 0.34, 11 = 0.35, 12 = 0.41, 13 = 0.83, 14 = 0.46 and 15 = 0.88

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 533 lb uplift at joint 2 and 513 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061242

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T17	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:41 2006 Page 1

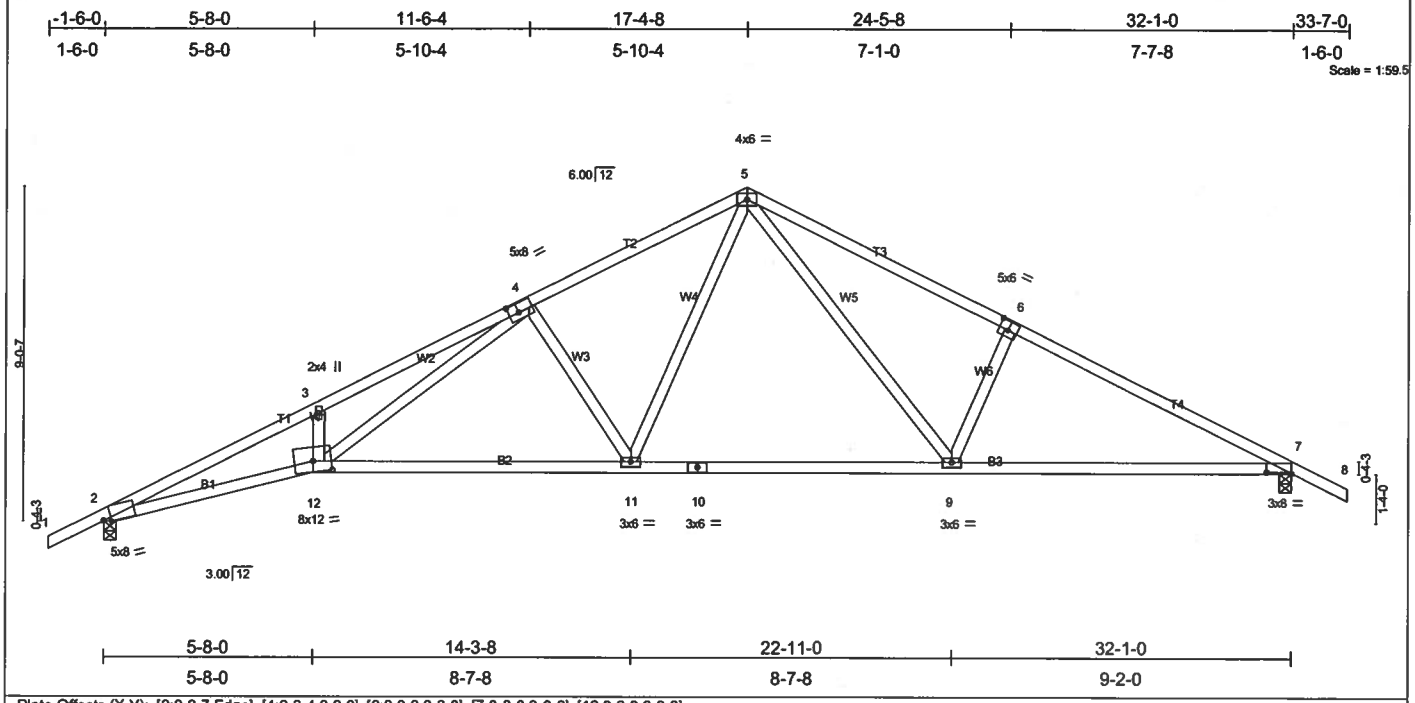


Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-4,0-3-0], [6:0-3-0,0-3-0], [7:0-8-0,0-0-6], [12:0-6-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.48	11-12	>789	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.79	11-12	>485	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.90	Horz(TL)	0.27	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-11-6 oc bracing.

REACTIONS (lb/size) 2=1423/0-4-0, 7=1423/0-4-0
Max Horz 2=192(load case 5)
Max Uplift2=-535(load case 5), 7=-516(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4538/1817, 3-4=-4473/1982, 4-5=-2083/1004, 5-6=-2213/1072, 6-7=-2347/1013, 7-8=0/35
BOT CHORD 2-12=-1579/4089, 11-12=-780/2181, 10-11=-383/1430, 9-10=-383/1430, 7-9=-734/2033
WEBS 3-12=-181/267, 4-12=-974/2239, 4-11=-721/473, 5-11=-385/963, 5-9=-361/800, 6-9=-379/363

JOINT STRESS INDEX
2 = 0.88, 3 = 0.34, 4 = 0.61, 5 = 0.82, 6 = 0.84, 7 = 0.74, 9 = 0.57, 10 = 0.53, 11 = 0.85 and 12 = 0.93

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 535 lb uplift at joint 2 and 516 lb uplift at joint 7.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061243

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T18	MONO HIP	1	1	Job Reference (optional)

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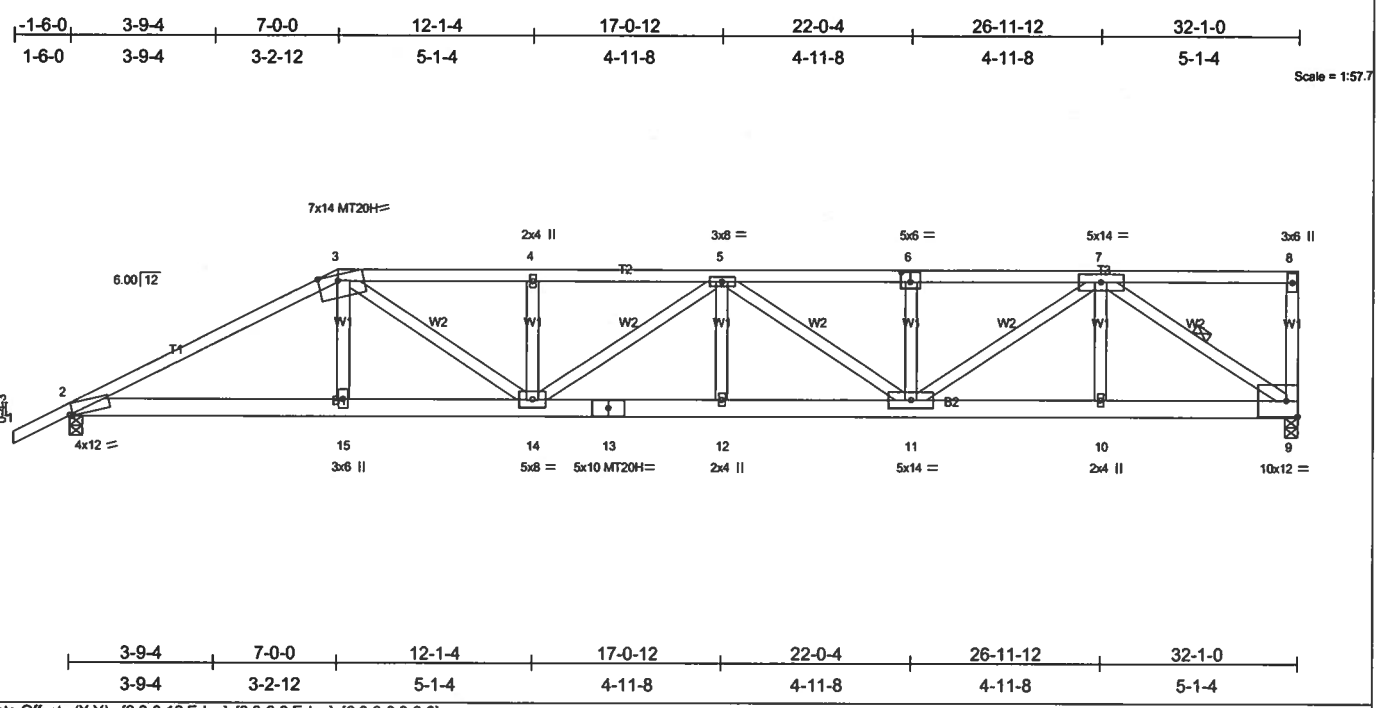


Plate Offsets (X,Y): [2:0-0-13,Edge], [3:0-6-3,Edge], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	-0.46	12-14	>837	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.73	12-14	>523	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.94	Horz(TL)	0.15	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-10-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-7 oc bracing.
WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 9=2934/0-4-0, 2=2802/0-4-0
Max Horz 2=212(load case 4)
Max Uplift9=-1321(load case 3), 2=-1166(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-5511/2359, 3-4=-6500/2916, 4-5=-6499/2916, 5-6=-5855/2633, 6-7=-5852/2630, 7-8=-99/57, 8-9=-279/220
BOT CHORD 2-15=-2147/4846, 14-15=-2157/4882, 13-14=-3079/6856, 12-13=-3079/6856, 11-12=-3079/6856, 10-11=-1635/3636, 9-10=-1635/3636
WEBS 3-15=-236/848, 3-14=-973/1955, 4-14=-569/499, 5-14=-435/215, 5-12=0/361, 5-11=-1221/545, 6-11=-564/462, 7-11=-1215/2705, 7-10=0/323, 7-9=-4318/1926

JOINT STRESS INDEX
2 = 0.82, 3 = 0.96, 4 = 0.34, 5 = 0.59, 6 = 0.77, 7 = 0.75, 8 = 0.56, 9 = 0.75, 10 = 0.34, 11 = 0.79, 12 = 0.34, 13 = 0.95, 14 = 0.90 and 15 = 0.28

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1321 lb uplift at joint 9 and 1166 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-8=-117(F=-63), 2-15=-30, 9-15=-65(F=-35)
Concentrated Loads (lb)
Vert: 15=-539(F)

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061244

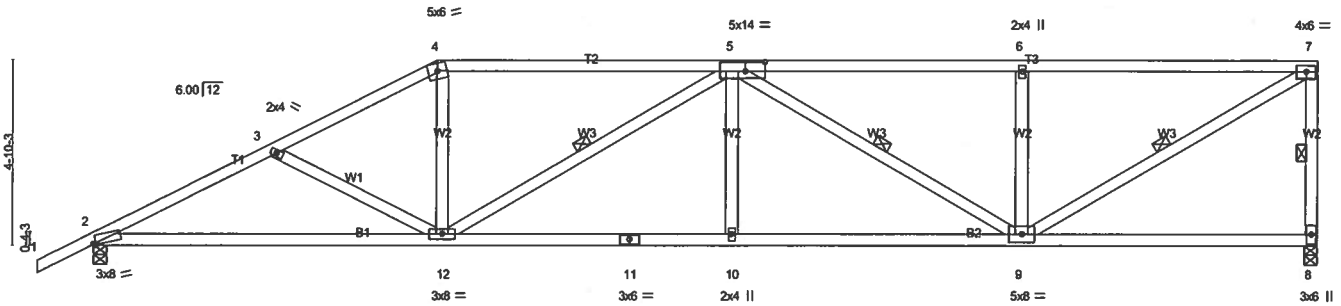
Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T19	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:47 2006 Page 1

-1-6-0 | 4-9-4 | 9-0-0 | 16-8-15 | 24-4-1 | 32-1-0

1-6-0 | 4-9-4 | 4-2-12 | 7-8-15 | 7-7-3 | 7-8-15

Scale = 1:57.7



9-0-0 | 16-8-15 | 24-4-1 | 32-1-0

9-0-0 | 7-8-15 | 7-7-3 | 7-8-15

Plate Offsets (X,Y): [2:0-0-10,Edge], [5:0-6-4,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.22	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.36	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 170 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-9 oc bracing, Except: 10-0-0 oc bracing: 8-9.
WEBS 1 Row at midpt 7-8, 5-12, 5-9, 7-9

REACTIONS (lb/size) 8=1332/0-4-0, 2=1427/0-4-0
Max Horz 2=256(load case 5)
Max Uplift8=-490(load case 4), 2=-469(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2420/971, 3-4=-2208/875, 4-5=-1959/846, 5-6=-1777/724, 6-7=-1777/724, 7-8=-1219/553
BOT CHORD 2-12=-1014/2110, 11-12=-1001/2399, 10-11=-1001/2399, 9-10=-1001/2399, 8-9=-28/71
WEBS 3-12=-193/195, 4-12=-127/620, 5-12=-513/287, 5-10=0/216, 5-9=-724/323, 6-9=-419/307, 7-9=-811/1989

JOINT STRESS INDEX
2 = 0.81, 3 = 0.34, 4 = 0.67, 5 = 0.52, 6 = 0.34, 7 = 0.81, 8 = 0.44, 9 = 0.92, 10 = 0.34, 11 = 0.90 and 12 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 490 lb uplift at joint 8 and 469 lb uplift at joint 2.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061245

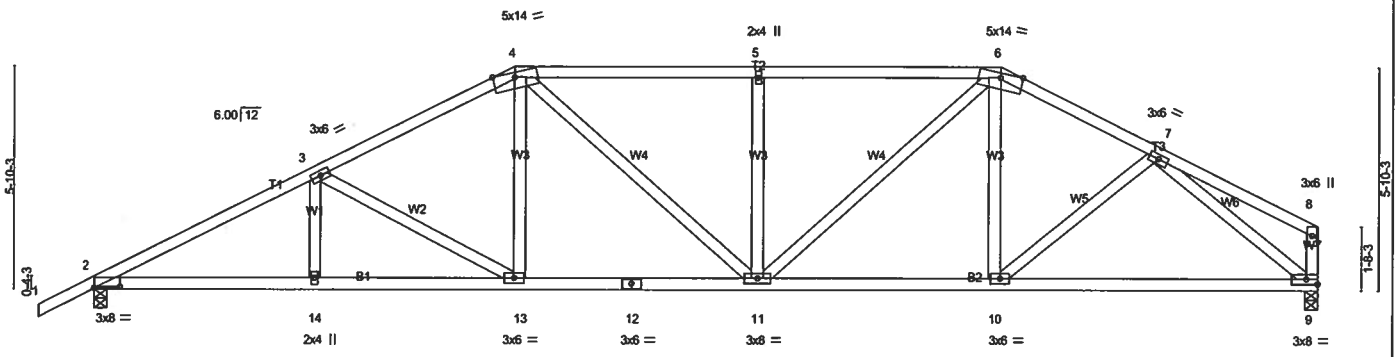
Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T20	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:50 2006 Page 1

-1-6-0 | 5-9-4 | 11-0-0 | 17-4-8 | 23-9-0 | 27-9-4 | 32-1-0

1-6-0 | 5-9-4 | 5-2-12 | 6-4-8 | 6-4-8 | 4-0-4 | 4-3-12

Scale = 1:57.7



5-9-4 | 11-0-0 | 17-4-8 | 23-9-0 | 32-1-0

5-9-4 | 5-2-12 | 6-4-8 | 6-4-8 | 8-4-0

Plate Offsets (X,Y): [2:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.15	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.25	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.09	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-9 oc bracing.

REACTIONS (lb/size) 2=1427/0-4-0, 9=1332/0-4-0
Max Horz 2=170(load case 5)
Max Uplift2=-496(load case 5), 9=-375(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2488/996, 3-4=-2011/880, 4-5=-1937/916, 5-6=-1937/916, 6-7=-1727/772, 7-8=-288/104, 8-9=-233/130
BOT CHORD 2-14=-871/2147, 13-14=-871/2147, 12-13=-639/1754, 11-12=-639/1754, 10-11=-531/1512, 9-10=-554/1335
WEBS 3-14=0/171, 3-13=-462/267, 4-13=-95/400, 4-11=-179/365, 5-11=-363/259, 6-11=-248/635, 6-10=-3/105, 7-10=-94/321, 7-9=-1488/684

JOINT STRESS INDEX
2 = 0.78, 3 = 0.41, 4 = 0.73, 5 = 0.34, 6 = 0.70, 7 = 0.45, 8 = 0.38, 9 = 0.61, 10 = 0.35, 11 = 0.62, 12 = 0.71, 13 = 0.35 and 14 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 496 lb uplift at joint 2 and 375 lb uplift at joint 9.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061246

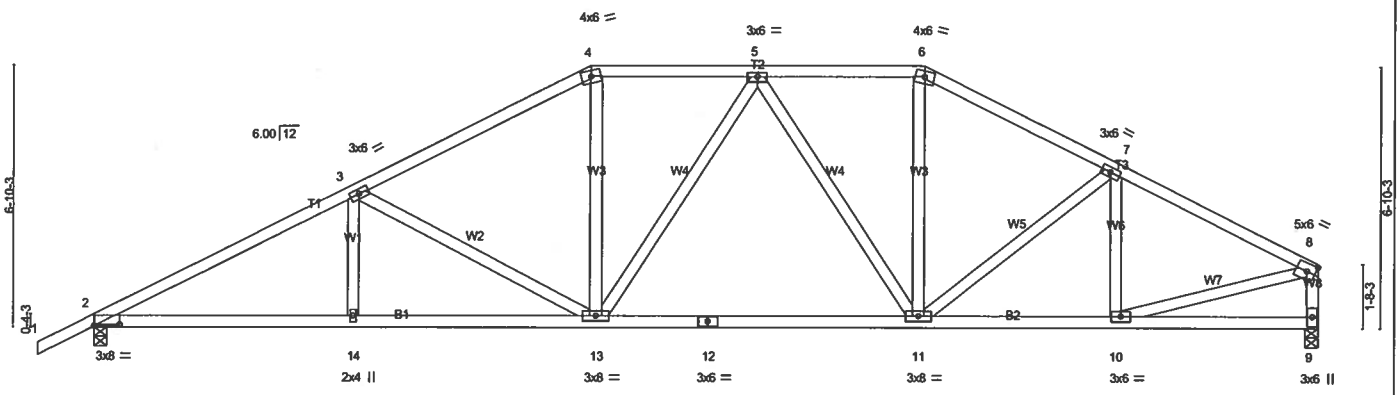
Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T21	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:52 2006 Page 1

-1-6-0 | 6-9-4 | 13-0-0 | 17-4-8 | 21-9-0 | 26-9-4 | 32-1-0

1-6-0 | 6-9-4 | 6-2-12 | 4-4-8 | 4-4-8 | 5-0-4 | 5-3-12

Scale = 1:57.7



6-9-4 | 13-0-0 | 21-9-0 | 26-9-4 | 32-1-0

6-9-4 | 6-2-12 | 8-9-0 | 5-0-4 | 5-3-12

Plate Offsets (X,Y): [2:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.20	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.34	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 182 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-13 oc bracing.

REACTIONS (lb/size) 2=1427/0-4-0, 9=1332/0-4-0
Max Horz 2=184(load case 5)
Max Uplift2=-511(load case 5), 9=-392(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2449/1001, 3-4=-1856/837, 4-5=-1599/816, 5-6=-1454/754, 6-7=-1676/780, 7-8=-1699/731, 8-9=-1240/569
BOT CHORD 2-14=-865/2109, 13-14=-865/2109, 12-13=-566/1591, 11-12=-566/1591, 10-11=-581/1462, 9-10=-79/156
WEBS 3-14=0/203, 3-13=-596/354, 4-13=-137/498, 5-13=-133/131, 5-11=-346/171, 6-11=-142/454, 7-11=-62/140, 7-10=-249/177, 8-10=-521/1356

JOINT STRESS INDEX
2 = 0.77, 3 = 0.41, 4 = 0.71, 5 = 0.43, 6 = 0.54, 7 = 0.41, 8 = 0.74, 9 = 0.36, 10 = 0.76, 11 = 0.59, 12 = 0.73, 13 = 0.59 and 14 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 511 lb uplift at joint 2 and 392 lb uplift at joint 9.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061247

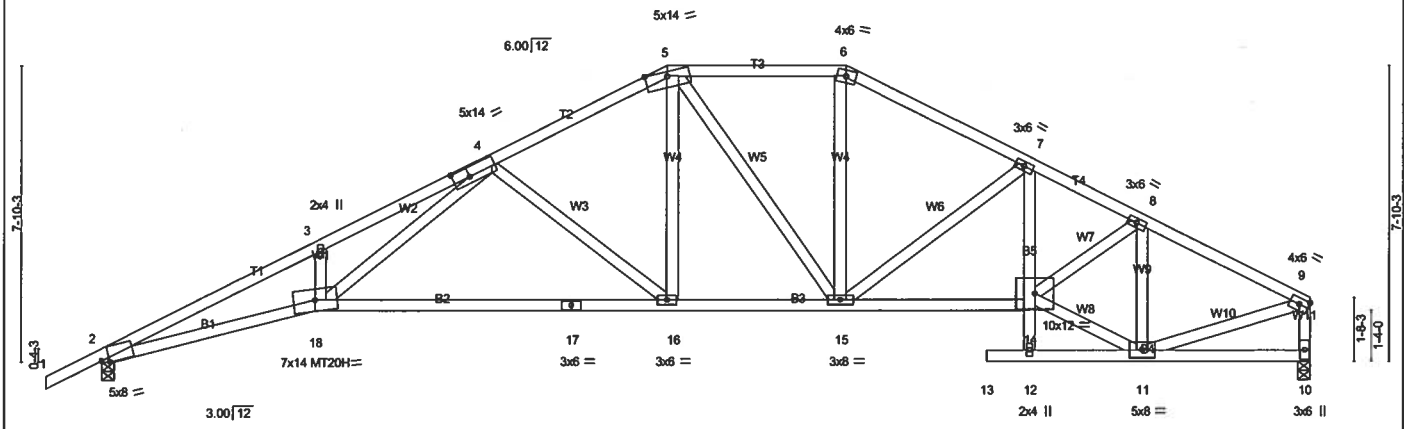
Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T22	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:55 2006 Page 1

-1-6-0 | 5-8-0 | 10-4-0 | 15-0-0 | 19-9-0 | 23-5-8 | 27-7-8 | 32-1-0

1-6-0 | 5-8-0 | 4-8-0 | 4-8-0 | 4-9-0 | 3-8-8 | 4-2-0 | 4-5-8

Scale = 1:58.5



5-8-0 | 15-0-0 | 19-9-0 | 23-5-8 | 24-5-8 | 27-7-8 | 32-1-0

5-8-0 | 9-4-0 | 4-9-0 | 3-8-8 | 1-0-0 | 3-2-0 | 4-5-8

Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-5-4,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.52	16-18	>736	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.85	16-18	>448	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.31	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 191 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-15 oc bracing.

REACTIONS (lb/size) 2=1435/0-4-0, 10=1358/0-4-0
Max Horz 2=197(load case 5)
Max Uplift2=-522(load case 5), 10=-402(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4610/1830, 3-4=-4523/1970, 4-5=-2011/917, 5-6=-1663/847, 6-7=-1901/882, 7-8=-2460/1054, 8-9=-1650/695, 9-10=-1277/567
BOT CHORD 2-18=-1654/4154, 17-18=-938/2375, 16-17=-938/2375, 15-16=-565/1762, 14-15=-805/2199, 12-14=0/94, 7-14=-118/466, 12-13=0/0, 11-12=-45/45, 10-11=-69/128
WEBS 3-18=-138/229, 4-18=-891/2132, 4-16=-810/486, 5-16=-263/767, 5-15=-297/101, 6-15=-191/553, 7-15=-681/341, 11-14=-575/1550, 8-14=-281/898, 8-11=-1062/472, 9-11=-515/1367

JOINT STRESS INDEX
2 = 0.89, 3 = 0.34, 4 = 0.55, 5 = 0.48, 6 = 0.58, 7 = 0.70, 8 = 0.61, 9 = 0.70, 10 = 0.31, 11 = 0.71, 12 = 0.42, 14 = 0.68, 15 = 0.58, 16 = 0.50, 17 = 0.99 and 18 = 0.65

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 522 lb uplift at joint 2 and 402 lb uplift at joint 10.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061248

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T23	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:37:59 2006 Page 1

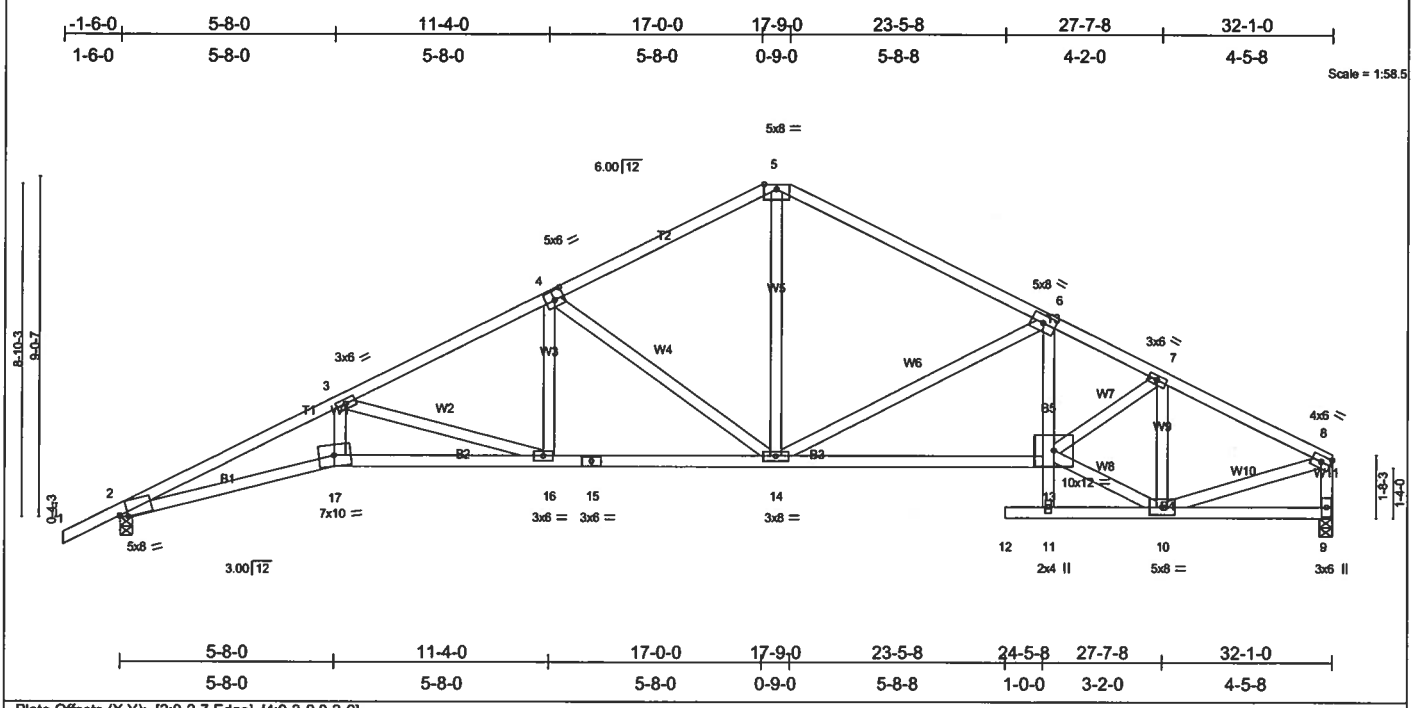


Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.36	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.57	16-17	>663	180		
BCLL	10.0	Rep Stress Incr	YES	WB	1.00	Horz(TL)	0.32	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1435/0-4-0, 9=1358/0-4-0
Max Horz 2=214(load case 5)
Max Uplift2=-534(load case 5), 9=-417(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4561/1897, 3-4=-2600/1144, 4-5=-1727/844, 5-6=-1747/833, 6-7=-2474/1077, 7-8=-1648/705, 8-9=-1277/576
BOT CHORD 2-17=-1718/4107, 16-17=-1632/3867, 15-16=-865/2258, 14-15=-865/2258, 13-14=-856/2250, 11-13=0/92, 6-13=-86/455, 11-12=0/0, 10-11=-63/29, 9-10=-73/132
WEBS 3-17=-367/1141, 3-16=-1663/795, 4-16=-198/653, 4-14=-961/510, 6-14=-858/451, 10-13=-564/1563, 7-13=-309/929, 7-10=-1065/470, 8-10=-517/1359, 5-14=-446/1112

JOINT STRESS INDEX
2 = 0.88, 3 = 0.85, 4 = 0.52, 5 = 0.84, 6 = 0.59, 7 = 0.63, 8 = 0.74, 9 = 0.31, 10 = 0.72, 11 = 0.37, 13 = 0.80, 14 = 0.57, 15 = 0.76, 16 = 0.47 and 17 = 0.89

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 534 lb uplift at joint 2 and 417 lb uplift at joint 9.

LOAD CASE(S) Standard

JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0705061249

Job	Truss	Truss Type	Qty	Ply	0 0
L200435	T24	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 05 10:38:02 2006 Page 1

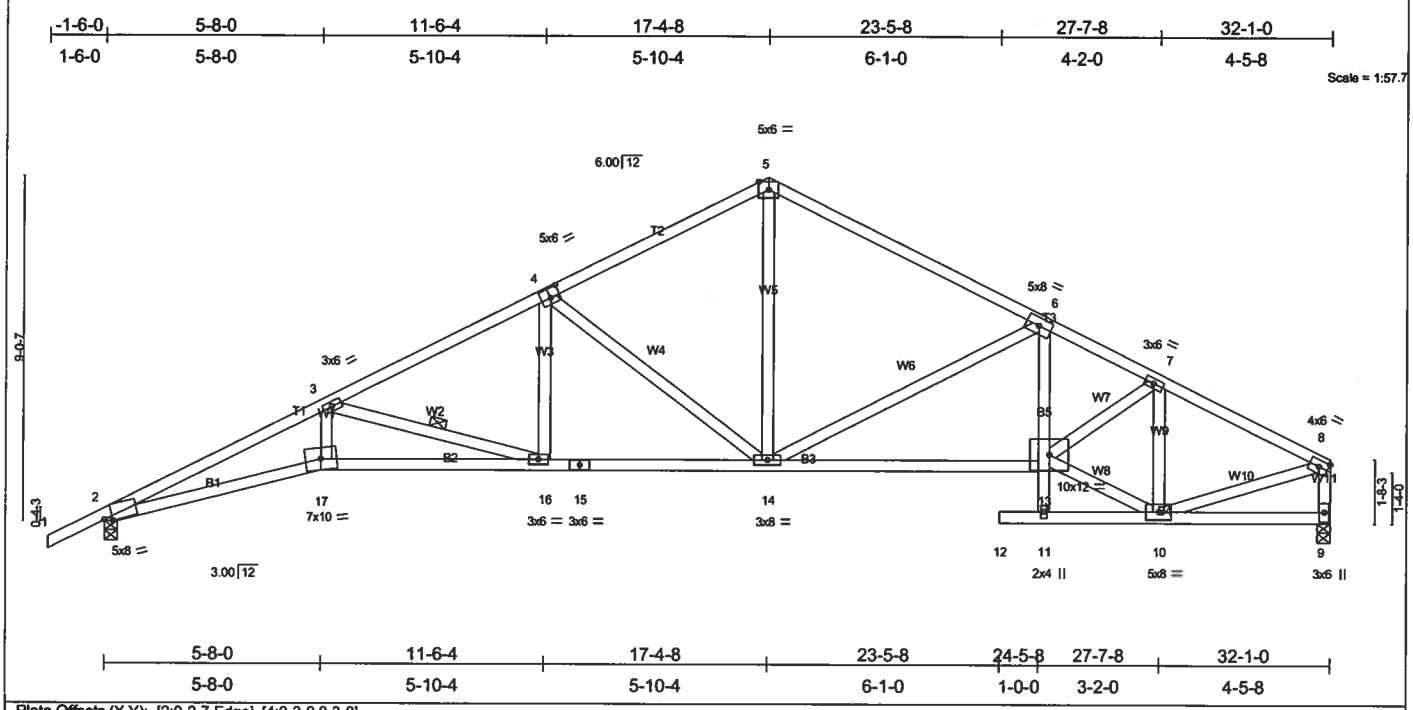


Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.36	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.59	16-17	>651	180		
BCLL	10.0	Rep Stress Incr	YES	WB	1.00	Horz(TL)	0.32	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-2 oc bracing.
WEBS 1 Row at midpt 3-16

REACTIONS (lb/size) 2=1435/0-4-0, 9=1358/0-4-0
Max Horz 2=214(load case 5)
Max Uplift2=-534(load case 5), 9=-417(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4568/1900, 3-4=-2569/1130, 4-5=-1723/845, 5-6=-1746/833, 6-7=-2475/1077, 7-8=-1648/705, 8-9=-1277/576
BOT CHORD 2-17=-1722/4114, 16-17=-1636/3873, 15-16=-849/2228, 14-15=-849/2228, 13-14=-857/2251, 11-13=0/92, 6-13=-86/455, 11-12=0/0, 10-11=-63/29, 9-10=-73/132
WEBS 3-17=-366/1147, 3-16=-1697/813, 4-16=-197/647, 4-14=-944/501, 5-14=-452/1118, 6-14=-860/453, 10-13=-563/1564, 7-13=-310/930, 7-10=-1065/469, 8-10=-517/1359

JOINT STRESS INDEX
2 = 0.88, 3 = 0.85, 4 = 0.52, 5 = 0.62, 6 = 0.59, 7 = 0.63, 8 = 0.74, 9 = 0.31, 10 = 0.72, 11 = 0.37, 13 = 0.81, 14 = 0.57, 15 = 0.78, 16 = 0.48 and 17 = 0.89

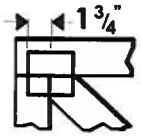
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 534 lb uplift at joint 2 and 417 lb uplift at joint 9.

LOAD CASE(S) Standard

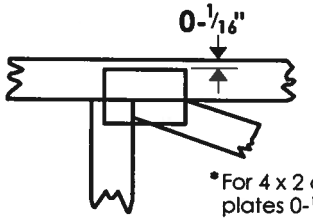
JULY 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

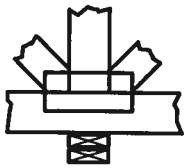
PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

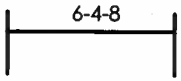


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

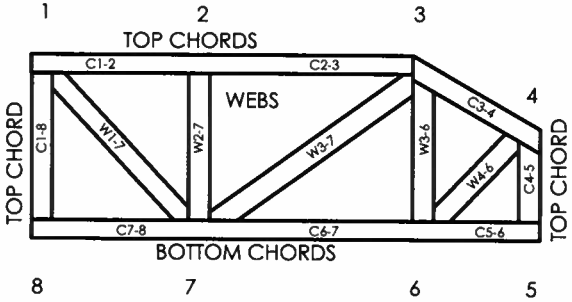
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

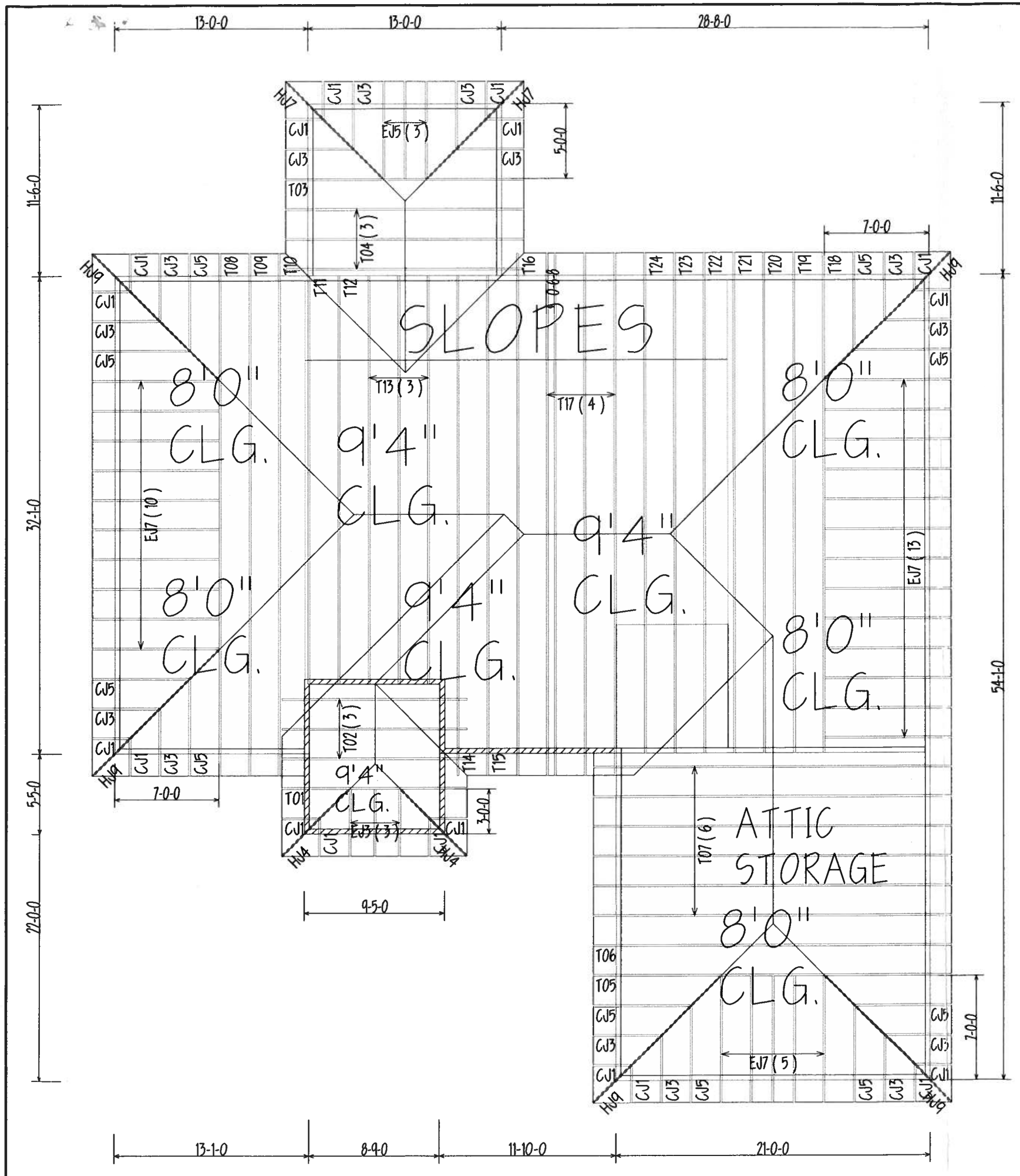
TEE-LOK®

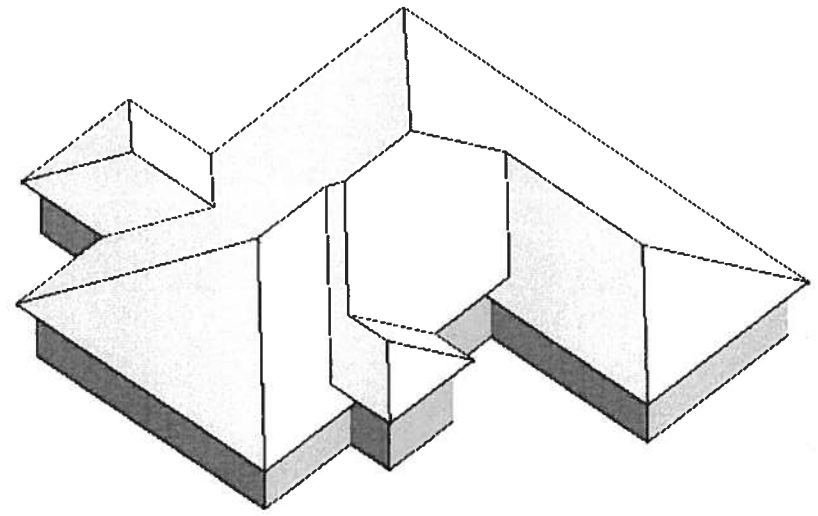
MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.





BEARING HEIGHT SCHEDULE

8'0"

9'4"

6/12 PITCH

18" O.H.

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved By: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 386-755-6894 FAX: 386-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: GIEBEIG HOMES			
LEGAL ADDRESS: LOT 24 MAYFAIR			
MODEL: ST JOHNS III		REVISION:	
		SCALE: NTS	
DATE: 7/5/06	DRAWN BY: JOE	JOB #: L200435	