

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1UWV487-Z0307142323



Truss Fabricator: **Anderson Truss Company**
Job Identification: **13-181--BRYAN ZECHER Hanger Building for Decke -- Lake City, FL**
Truss Count: **6**
Model Code: **Florida Building Code 2010**
Truss Criteria: **FBC2010Res/TPI-2007(STD)**
Engineering Software: **Alpine Software, Versions 12.03, 13.01.**
Structural Engineer of Record: **The identity of the structural EOR did not exist as of the seal date per section 61615-31.003(5a) of the FAC**
Address: **Roof - 37.0 PSF @ 1.25 Duration**
Minimum Design Loads: **Floor - N/A**
Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

William H. Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12030EC1-GBLLETIN-GABRST10-12015EC1-

#	Ref	Description	Drawing#	Date
1	65554--A	48' Common	13158009	06/07/13
2	65555--A1	48' Common	13158010	06/07/13
3	65556--ADG	48' Gable	13158011	06/07/13
4	65557-AGDG	48' Special	13158014	06/07/13
5	65558--B	40' Common	13158012	06/07/13
6	65559--BDG	40' Gable	13158013	06/07/13

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Page 1 of 1 Document ID:1UWV487-Z0307142323



Truss Fabricator: **Anderson Truss Company**
Job Identification: **13-181--BRYAN ZECHER Hanger Building for Decke -- Lake City, FL**
Truss Count: **1**
Model Code: **Florida Building Code 2010**
Truss Criteria: **FBC2010Res/TPI-2007(STD)**
Engineering Software: **Alpine Software, Versions 12.03, 13.01.**
Structural Engineer of Record:
Address:
Minimum Design Loads: **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

-Truss Design Engineer-
William H. Krick

1950 Marley Drive
Haines City, FL 33844

Revised Trusses

#	Ref	Description	Drawing#	Date
1	65557-AGDC	48' Special	13158014	06/07/13

ALPINE

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUCK AND

120 mph wind, 17.99 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCr1(+/-)=0.18

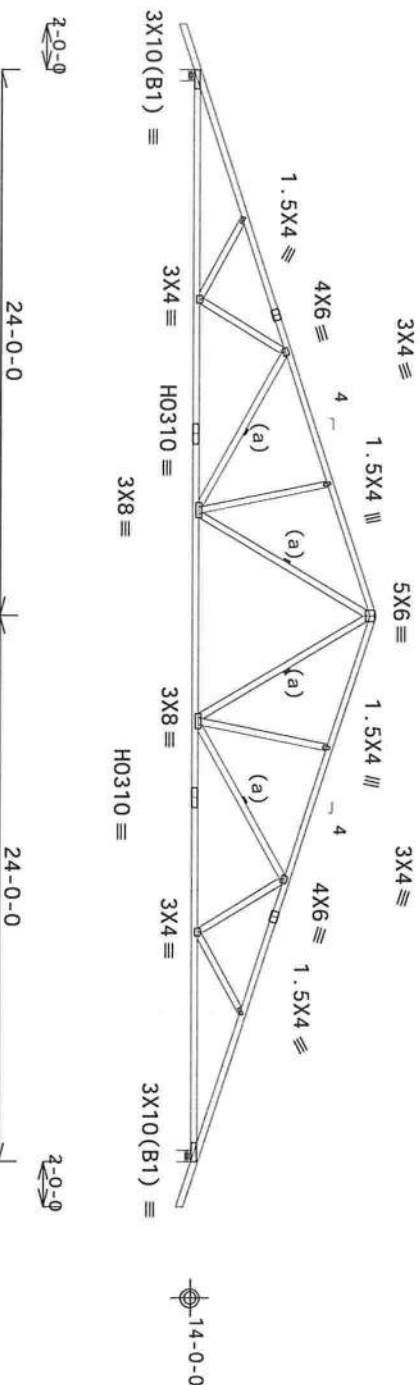
Wind loads and reactions based on MMFRS with additional C&C member design.

Calculated horizontal deflection is 0.15" due to live load and 0.17" due to dead load.

(a) Continuous lateral bracing equally spaced on member,

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Calculated vertical deflection is 0.48" due to live load and 0.55" due to dead load at $X = 28-7-10$.



RE-2004 U=118 W=6" (6" min.)

Design Crit: FBC2010Res/TP1-2007(STP)
FT/RT=10%(0%)/0(0)

No. 70861

FL-/-/3/-/-/R/-/

Scale = 125"/Ft

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

1

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278

ITW Building Components Group, Inc. (ITWBCG) was not responsible for any deviation from this drawing. ITWBCG was responsible for the design of the truss and for the handling, shipping, installation, bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Detail, unless noted otherwise. Refer to drawings T60A-2 for standard plate positions. A note on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer. For ANSI/SP-1 Sec. 2. For more information see: This job is a part of the ITWBCG Building Group. ITW Building Group, 1711 West 14th Street, Chicago, IL 60604-1000, U.S.A. Tel: 708/441-5000, Fax: 708/441-5001, E-mail: itwbcg@itw.com, www.itwbcg.com

A circular professional seal for William H. Kreck, a Professional Engineer in the State of Florida. The seal features the text "WILLIAM H. KRECK" around the top inner edge, "LICENSE" at the top, "STATE OF FLORIDA" on the left, and "PROFESSIONAL ENGINEER" around the bottom. In the center, it reads "No. 70861" and "12.03.04". There are two stars on the seal, one at the top and one at the bottom. The seal is stamped over a document with some text visible on the left and bottom edges.

TC LL	20.0 PSF	REF R487-- 65554
TC DL	7.0 PSF	DATE 06/07/13
BC DL	10.0 PSF	DRW HCUR487 13158009
BC LL	0.0 PSF	HC-ENG SSB/AP
TOT.LD.	37.0 PSF	SEQN- 308939
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UWV487_Z03

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

120 mph wind with 17.99 ft mean hgt., ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. $G_{CPI}(+/-)=0.18$

Wind loads and reactions based on MMFRS with additional C&C member design.

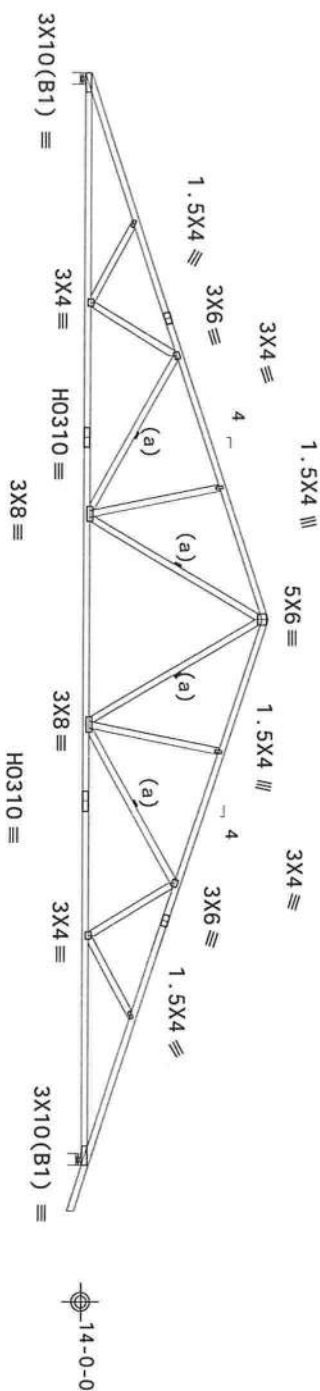
Calculated horizontal deflection is 0.15" due to live load and 0.17" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Calculated vertical deflection is 0.48" due to live load and 0.56" due to dead load at $X = 28-7-10$.

MMFRS loads based on trusses located at least 9.00 ft. from roof edge.



R=1883 U=97 W=6" (6" min.)
RL=137/-131

24-0-0 48-0-0 Over 2 Supports 24-0-0

WILLIAM H. KIRBY
JULY 20 1919
LIBRARY
U.S. DEPT. OF COMMERCE

PLT TYP. 20 Gauge HS, Wave

Design Crit: FBC2010Res/TP1-2007 (Std)
FT/RT=10%(0%)/0(0)

12.03.04:0826:14 QTY

FL/-/3/-/-/R/-

Scale = .125"/Ft.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278

****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Tension requires extreme care in fabricating, handling, shipping, installing and bracing. For example, the design of the bracing system must take into account the effects of thermal expansion and contraction as well as the lateral extension of BGS's Building Component Safety Information, by TPI and RITA) practices prior to performing those functions. Installers shall provide temporary bracing procedures and methods to ensure that the building components are not damaged or distorted during installation. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BGS' sections B3, D7 or D10, as applicable.

any Building Components Group Inc. (TBCCO) shall not be responsible for any deviation from this standard. The responsibility for the design of the building shall remain with the design engineer. The responsibility for the Building Designer per ANSI/TPI 1 Sec 2, for more information see: This job's general notes sheet: TPI-DOC: www.tbccco.org, TBICA: www.tbindustry.com, www.tbccco.org

12.03.04:08:26.14

TC LL	20.0 PSF	REF R487-- 65555
TC DL	7.0 PSF	DATE 06/07/13
BC DL	10.0 PSF	DRW HCUR487 13156010
BC LL	0.0 PSF	HC-ENG SSB/AP
TOT. LD.	37.0 PSF	SEQN- 308943
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UWV487_Z03

(13-181--BRYAN ZECHER Hanger Building for Decke -- Lake City, FL - ADG 48' Gable)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

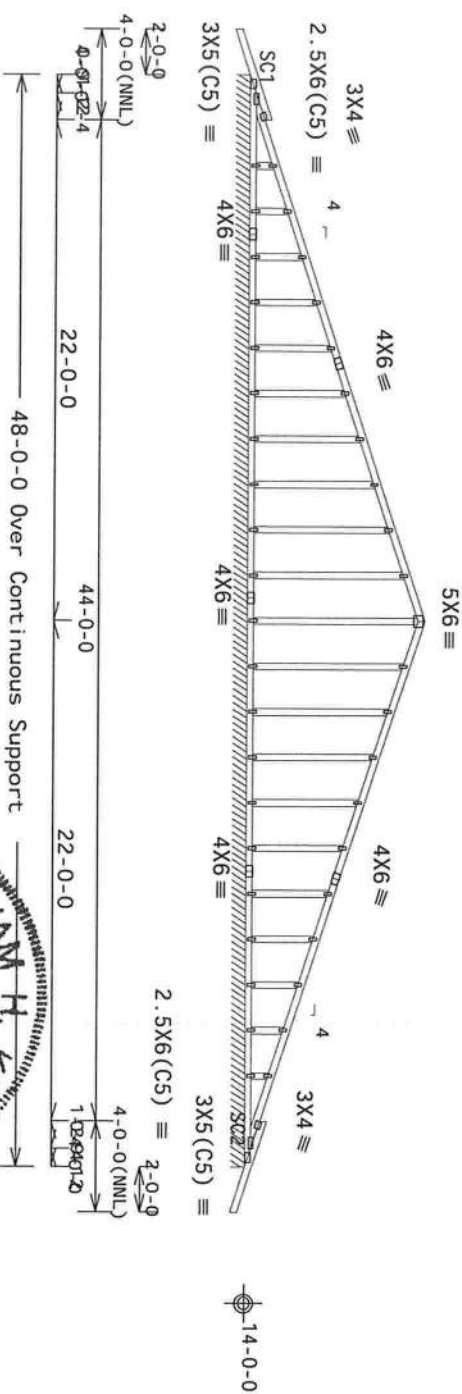
Top chord 2x4 SP #1_12A
Bot chord 2x4 SP #1_12A
Webs 2x4 SP #3_12A
Stack Chord SC1 2x4 SP #1_12A::Stack Chord SC2 2x4 SP #1_12A:
Lumber grades designated with "12A" use design values approved
1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013
and shall only be used on projects designed and permitted prior to
this date unless specifically approved in writing by the building
authority having jurisdiction, the building designer and the project
owner.

Stacked top chord must NOT be notched or cut in area (NML). Dropped
top chord braced at 24" o.c. intervals. Attach stacked top chord (SC)
to dropped top chord in notched area using 3x4 tie-plates 24" o.c.
Center plate on stacked/dropped chord interface, plate length
perpendicular to chord length. Splice top chord in notched area
using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor.
Special care must be taken during handling, shipping and installation
of trusses. See "WARNING" note below.

120 mph wind, 17.84 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC
DL=5.0 psf. GCPI(+/-)=0.18
Wind loads and reactions based on MMFRS with additional C&C member
design.
Truss spaced at 24.0" OC designed to support 2-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.
See DWGS A12030ENC100212, GBLLET1N0212, & GABRST100212 for more
requirements.
In lieu of structural panels use purlins to brace TC @ 24" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.



R=172 PLF U=18 PLF W=48-0-0
RL=6/-6 PLF
Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: FBC2010Res/TP1-2007 (STD)
PLT TYP. Wave

FT/RT=10%(0%/0/0)

No. 70861

06/07/2013

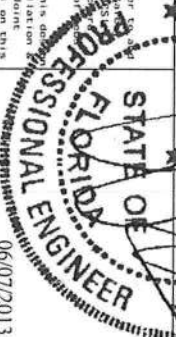
FL/-/3/-/-/R/-

Scale = .125"/Ft.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278



****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS, INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the
latest edition of BCSI (Building Component Safety Information) by TPI and WTA for the most current
practices prior to performing these functions. Installers shall provide temporary bracing per BCSI
unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom
chord shall have properly attached structural sheathing. Locations shown for permanent lateral restraint of
shall have bracing installed per BCSI sections B1, B2 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design
any fabricator, installer, or contractor. ITWBCG shall not be responsible for any deviation from this design
bracing of trusses. Apply plates to each free end of top chord and bottom chord. On installation, refer to
details, unless noted otherwise. Refer to drawings 1604-2 for standard plate positions. A note on this
drawing or cover page listing this drawing. The suitability and use of this design for any structure is
the responsibility of the Building Designer per ANSI/TP1 1 Sec.2. For more information see: This Job's
IDC: www.alcscite.org
178-BDG: www.trading.com; TPI: www.tpinet.org; WTA: www.abnindustry.com

TC LL	20.0 PSF	REF R487--	65556
TC DL	7.0 PSF	DATE	06/07/13
BC DL	10.0 PSF	DRW	HCSR487 13158011
BC LL	0.0 PSF	HC-ENG	SSB/AP
TOT. LD.	37.0 PSF	SEQN-	309003
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1UWV487_Z03

2 COMPLETE TRUSSES REQUIRED

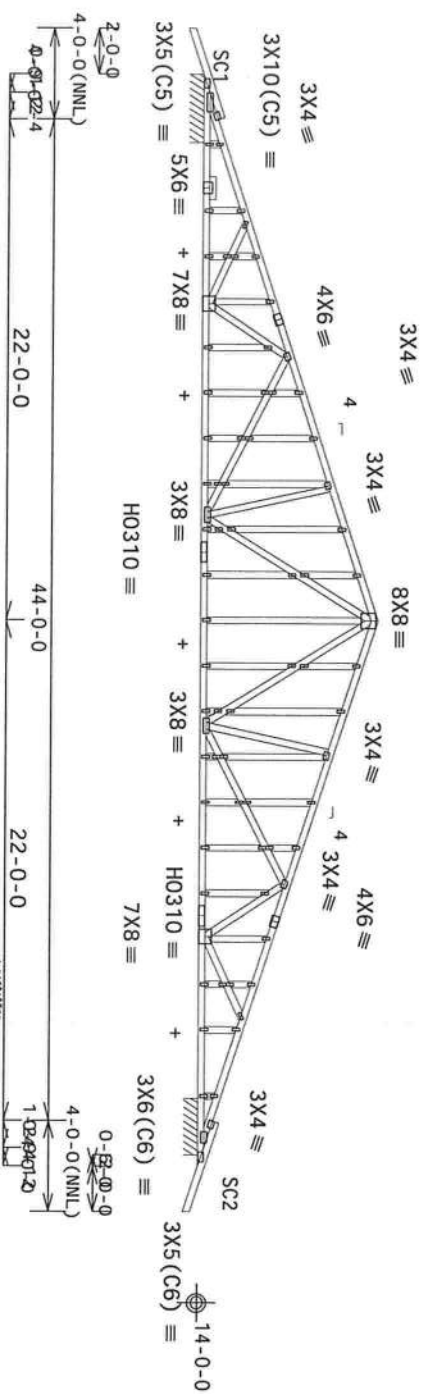
Top chord 2x4 SP #1_12A
Bot chord 2x4 SP 2850F-2.3E
Webs 2x4 SP #3_12A
Stack Chord SC1 2x4 SP #1_12A; Stack Chord SC2 2x4 SP #1_12A;
Lt Splice Block 2x4 SP #2_12A;
Lumber grades designated with "12A" use design values approved
1/5/2012 by ALSC.

120 mph wind, 17.84 ft mean hgt, ASCE 7-10, CLOSED bldg, located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC
DL=5.0 psf, GCPI(+/-)=0.18

Truss spaced at 24.0" OC designed to support 2-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.

Stacked top chord must NOT be notched or cut in area (NML). Dropped
top chord braced at 24" o.c. intervals. Attach stacked top chord (SC)
to dropped top chord in notchable area using 3x4 tie-plates 24" o.c.
Center plate on stacked/dropped chord interface, plate length
perpendicular to chord length. Splice top chord in notchable area
using 3x6.

+ Member to be laterally braced for horizontal wind loads.
Bracing system to be designed and furnished by others.



Nail Schedule: 0.131"x3" min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

Wind loads and reactions based on MMFRS with additional C&C member
design.

Right cantilever is exposed to wind

See DWGS A12030ENC100212, GBLETT100212, & GABRST100212 for more
requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor.
Special care must be taken during handling, shipping and installation
of trusses. See "WARNING" note below.

Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: FBC2010Res/TP1-2007 (STD)
FT/RT=10% (0%)/0(0)

PLT TYP. 20 Gauge HS, Wave

R=1379 PLF U=145 PLF W=3-0-0
RL=96/-96 PLF

48-0-0 Over 2 Supports

22-0-0

44-0-0

22-0-0

13-01.02.05/6.20

06/07/2013

TC LL	20.0 PSF	REF R487-- 65557
TC DL	7.0 PSF	DATE 06/07/13
BC DL	10.0 PSF	DRW HCUR487 13158014
BC LL	0.0 PSF	HC-ENG SSB/AP
TOT.LD.	37.0 PSF	SEQN- 3223 REV
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UWV487_Z03

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0 278

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WTA) for the most current practices prior to performing these functions. Installers shall provide temporary bracing per BCSI for all trusses. Top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. The user of this design shall be responsible for the design, fabrication, handling, shipping, installing and bracing of trusses. Apply plates to each face of truss and position details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page indicating this drawing, indicates acceptance of professional engineering the responsibility of the Building Designer per ANSI/TP1 1 Sec.2. For more information see: This Job's ITC: www.itcarch.org



Top chord 2x4 SP #1_12A
Bot chord 2x4 SP #1_12A
Webs 2x4 SP #3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

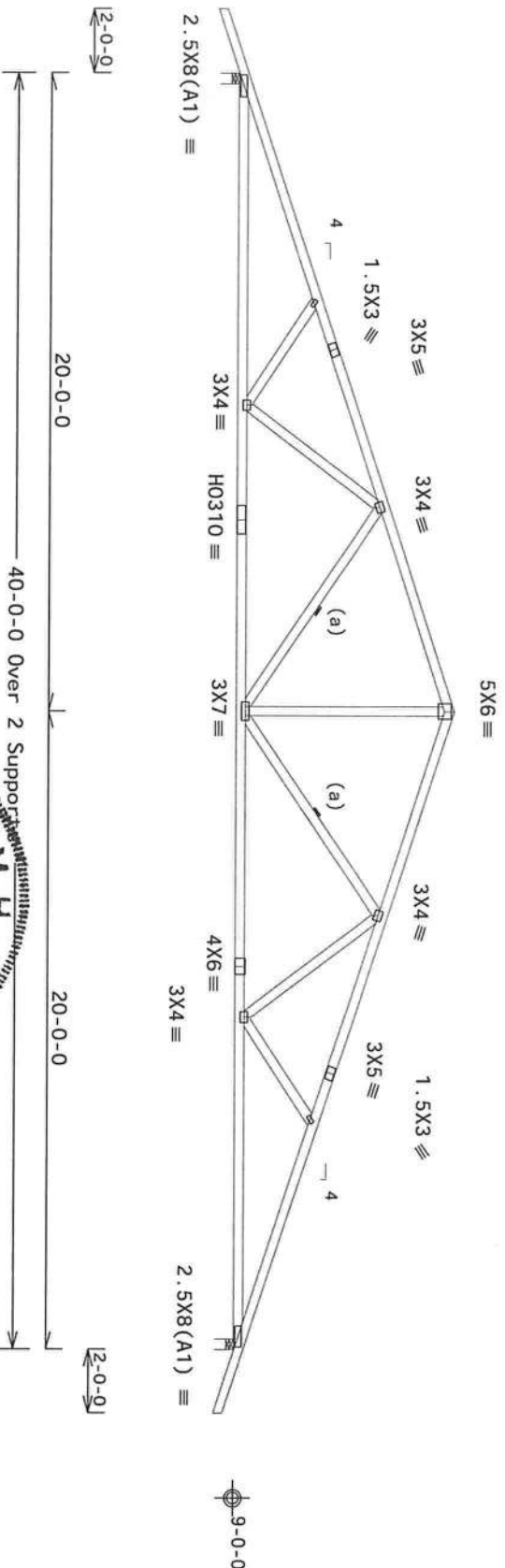
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1613 U=78 W=4" (4" min.)
RL=117/-117

R=1613 U=78 W=4" (4" min.)

PLT TYP. 20 Gauge HS, Wave

Design Crit: FBC2010Res/TP1-2007(SD)
FT/RT=10%(0%)/0(0)

QTY: 10 FL/-/3/-/1/R/-

Scale = .1875"/Ft.

IMPORTANT READ AND FOLLOW ALL NOTES ON THIS SHEET!

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCS1 (Building Component Safety Information, by TPI and WTC) and practices noted otherwise. Top chord shall have properly attached structural sheathing and bracing. Bottom chord shall have bracing installed per BCS1 sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. Any failure or damage to the truss or its components, including but not limited to, handling, shipping, installing, or bracing of trusses. Apply plates to each face of truss and position plates on the top and bottom flanges of the truss. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec. 2. For more information see: This Job's drawings. ITWBCG: www.itwbcg.com; TPI: www.truss.org; WTC: www.woodindustry.com; ICC: www.icc.org

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278



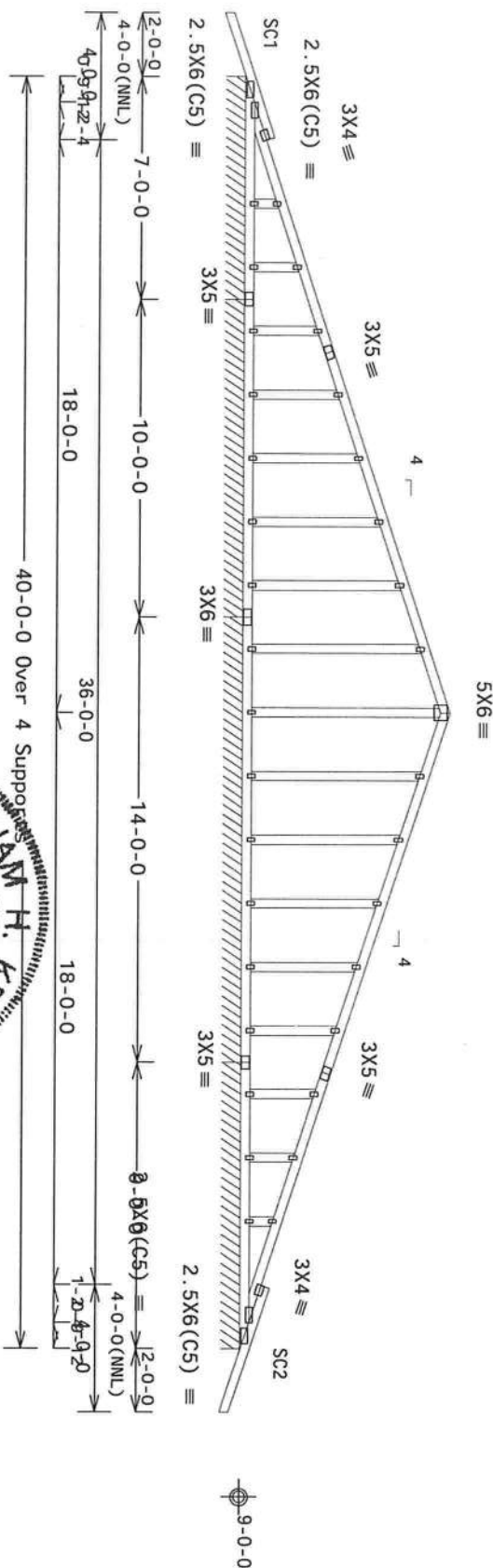
TC LL	20.0 PSF	REF	R487--	65558
TC DL	7.0 PSF	DATE	06/07/13	
BC DL	10.0 PSF	DRW	HCSR487	13158012
BC LL	0.0 PSF	HC-ENG	SSB/AP	
TOT. LD.	37.0 PSF	SEQN-	14030	
DUR. FAC.	1.25			
SPACING	24.0"	JREF-	1UWV487_Z03	

Top chord 2x4 SP #1 12A
Bot chord 2x4 SP #1 12A
Webs 2x4 SP #3 12A
Stack Chord SC1 2x4 SP #1 12A::Stack Chord SC2 2x4 SP #1 12A:
Lumber grades designated with "12A" use design values approved
1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013
and shall only be used on projects designed and permitted prior to
this date unless specifically approved in writing by the building
authority having jurisdiction, the building designer and the project
owner.

Stacked top chord must NOT be notched or cut in area (NML). Dropped
top chord braced at 24" o.c. intervals. Attach stacked top chord (SC)
to dropped top chord in notchable area using 3x4 tie-plates 24" o.c.
Center plate on stacked/dropped chord interface, plate length
perpendicular to chord length. Splice top chord in notchable area
using 3x6.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC
DL=5.0 psf, GCpl(+/-)=0.18
Wind loads and reactions based on MMFRS with additional C&C member
design.
Truss spaced at 24.0" OC designed to support 2-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.
See DWGS A12015ENC100212, GBLLET1N0212, & GABRS1T100212 for more
requirements.
In lieu of structural panels use purlins to brace TC @ 24" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.



Note: All Plates Are 1.5X3 Except As Shown.
Design Crit: FBC2010Res/TP1-2007(STB)
FT/RT=10%(0%)/0(0)
PLT TYP. Wave

WILLIAM H. KRECK
FLORIDA PROFESSIONAL ENGINEER
No. 70861
06/07/2013

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information) by TPI and WTC for practices noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure of the truss system. The user of this design shall be responsible for the design, detailing, bracing of trusses. Apply plates to each face of truss and per standard plate positions. A seal on this drawing or cover page indicating this drawing, indicating acceptance of professional engineering responsibility of the Building Designer per ANSI/TP1 1 Sec. 2. For more information see: This Job's sheet, www.itwbcg.com, TPI 1, www.trusscraft.org, WTC: www.bdcindustry.com, TDC: www.trusscraft.org

FL/-/3/-/-/R/-		Scale = .1875"/Ft.	
TC LL	20.0 PSF	REF	R487-- 65559
TC DL	7.0 PSF	DATE	06/07/13
BC DL	10.0 PSF	DRW	HGUSR487 13158013
BC LL	0.0 PSF	HC-ENG	SSB/AP
TOT. LD.	37.0 PSF	SEQN-	14038
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1UWV487_Z03

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

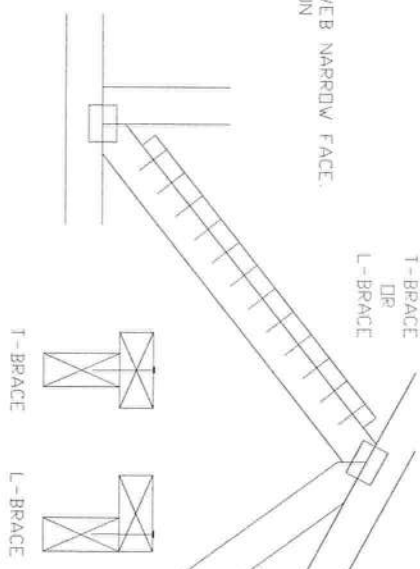
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SCALED DESIGN.

2) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

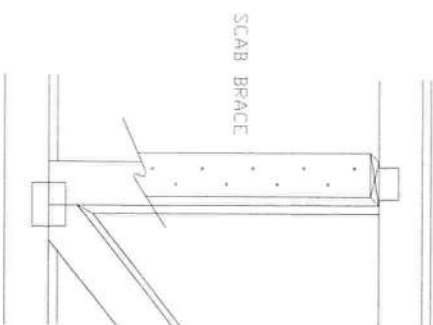
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE. ATTACH WITH 10d BDX OR GUN 0.128" x 3" MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH.



SCAB BRACING:

APPLY SCABS TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BDX OR GUN 0.128" x 3" MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH.



Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Before construction, the fabricator must follow BCSI Building Component Safety Information, by TPI and WTCI for safety practices and proper bracing. Truss fabricators shall provide temporary bracing per BCSI. Unless noted otherwise, all bracing shall be installed in accordance with the provisions of the BCSI. Truss fabricators shall provide a detailed bracing plan for each truss. See the job's general notes page for more information.

WARNING FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

1) The fabricator shall be responsible for any deviation from this design. Only failure to build the truss in accordance with TPI and WTCI design is the responsibility of the fabricator. The fabricator shall be responsible for any deviation from this design. Only failure to build the truss in accordance with TPI and WTCI design is the responsibility of the fabricator. The fabricator shall be responsible for any deviation from this design. Only failure to build the truss in accordance with TPI and WTCI design is the responsibility of the fabricator.



STATE OF FLORIDA
PROFESSIONAL ENGINEER
No. 70861

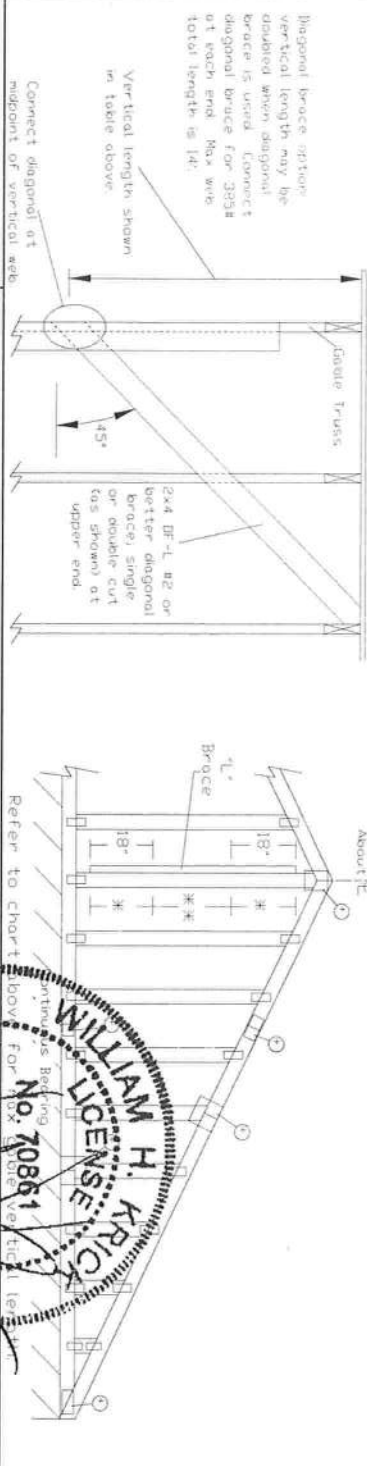
Jun 07 '13

06/07/2013

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	BRCLBSUB0109
BC LL	PSF		
TDI, L.D.	PSF		
DUR. FAC.			
SPACING			

ASCE 7-10: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00 Dr: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00 Di: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Length		Brace		(1) 1x4 L ¹ Brace *		(1) 2x4 L ¹ Brace *		(2) 2x4 L ¹ Brace *		(1) 2x6 L ¹ Brace *		(2) 2x6 L ¹ Brace *	
Spacing	Species	No	Braces	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
12" o.c.	SPF	#1 / #2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 8"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	4' 3"	5' 7"	5' 11"	7' 5"	7' 11"	10' 0"	10' 9"	12' 5"	14' 0"	14' 0"	14' 0"
	SPF	#3	4' 4"	6' 5"	6' 10"	8' 7"	9' 2"	10' 10"	11' 4"	13' 5"	14' 0"	14' 0"	14' 0"
	HF	Standard	4' 4"	6' 5"	6' 10"	8' 7"	9' 2"	10' 10"	11' 4"	13' 5"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 8"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	SPF	#1 / #2	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	5' 4"	9' 0"	9' 4"	10' 7"	11' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"



Building Components Group Inc.
Earth City, MO 63045



Bracing Group Species and Grades:	
Group A:	Hem-Fir
Group B:	Hem-Fir
Group C:	Hem-Fir
Group D:	Hem-Fir
Group E:	Hem-Fir
Group F:	Hem-Fir
Group G:	Hem-Fir
Group H:	Hem-Fir
Group I:	Hem-Fir
Group J:	Hem-Fir
Group K:	Hem-Fir
Group L:	Hem-Fir
Group M:	Hem-Fir
Group N:	Hem-Fir
Group O:	Hem-Fir
Group P:	Hem-Fir
Group Q:	Hem-Fir
Group R:	Hem-Fir
Group S:	Hem-Fir
Group T:	Hem-Fir
Group U:	Hem-Fir
Group V:	Hem-Fir
Group W:	Hem-Fir
Group X:	Hem-Fir
Group Y:	Hem-Fir
Group Z:	Hem-Fir

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 70 pif over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" out-toe-overs with 2' 0" overhang, or 12" plywood overhang.

5a. Five lumber design values based on the ALSC January, 2012 rule.

Attach 1" braces with 10d (0.128x3.0" min) nails.

For (1) 1" brace: space nails at 2' o.c. in 18" end zones and 4" o.c. between zones.

For (2) 1" brace: space nails at 3' o.c. in 18" end zones and 6" o.c. between zones.

1" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

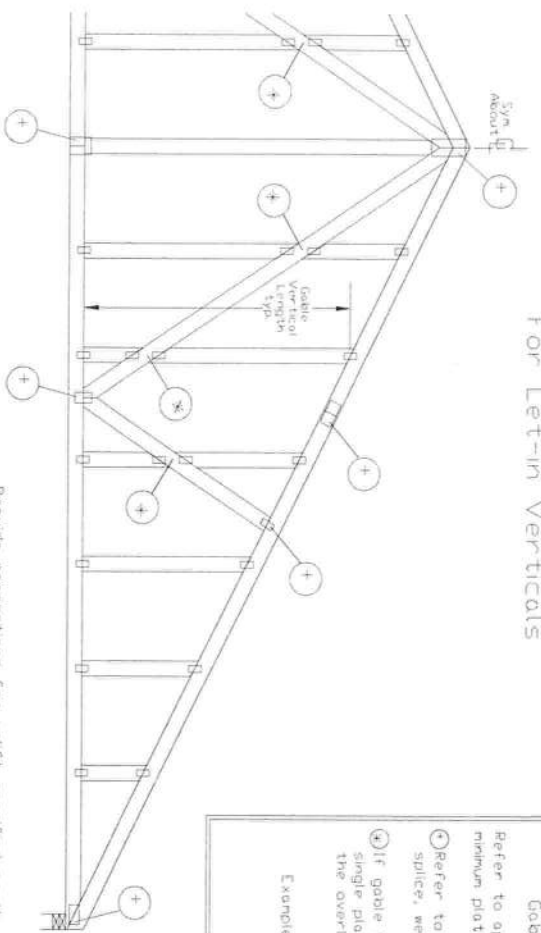
Vertical Length	No Splice
Less than 4' 0"	1x4 or 2x3
Greater than 4' 0", but less than 11' 6"	2x4
Greater than 11' 6"	2.5x4

* Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer For conditions not addressed by this detail.

REF	ASCE7-10-CAB12030
DATE	2/14/12
DRWG	A12030ENC100212
MAX. TDT. LD.	60 PSF
MAX. SPACING	24.0"

Gable Detail For Let-in Verticals



Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

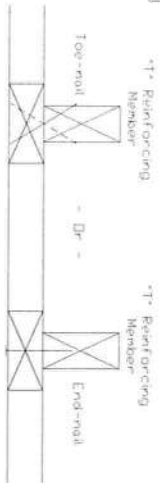
Refer to Engineered Truss design for peak, splice, web, and heel plates.

If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



1/2" Reinforcement Attachment Detail



To convert from 1/2" to 1/2" reinforcing members, multiply 1/2" increase by length (based on appropriate ITW gable detail).

Maximum allowable 1/2" reinforced gable vertical length is 14' from top to bottom chord.

1/2" reinforcing member material must match size, species, and grade of the 1/2" reinforcing member.

Web Length Increase w/ 1/2" Brace

1/2" Reinf.	1/2"
Hor. Size	Increase
2x4	30 %
2x6	20 %

Example: ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. Sp #3

1/2" Reinforcing Member Size = 2x4

1/2" Brace Increase (from above) = 30% = 1.30

(1) 2x4 1/2" Brace Length = 8' 7"

Maximum 1/2" Reinforced Gable Vertical Length 1.30 x 8' 7" = 11' 2"

This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings

A10015980109, A12015980109, A11015980109, A10015980109

ASCE 7-02 Gable Detail Drawings

A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-05 Gable Detail Drawings

A13030020109, A12030020109, A11030020109, A10030020109

ASCE 7-10 Gable Detail Drawings

A13030050109, A12030050109, A11030050109, A10030050109

ASCE 7-10 Gable Detail Drawings

A13030050109, A12030050109, A11030050109, A10030050109

ASCE 7-10 Gable Detail Drawings

A13030050109, A12030050109, A11030050109, A10030050109

ASCE 7-10 Gable Detail Drawings

A13030050109, A12030050109, A11030050109, A10030050109

ASCE 7-10 Gable Detail Drawings

A13030050109, A12030050109, A11030050109, A10030050109

ASCE 7-10 Gable Detail Drawings

A13030050109, A12030050109, A11030050109, A10030050109

ASCE 7-10 Gable Detail Drawings



Building Components Group Inc.

Earth City, MO 63045



Jun 07 '13

REF LET-IN VERT

DATE 2/16/12

DRWG GBLLETT0212

MAX. TOT. LD 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"

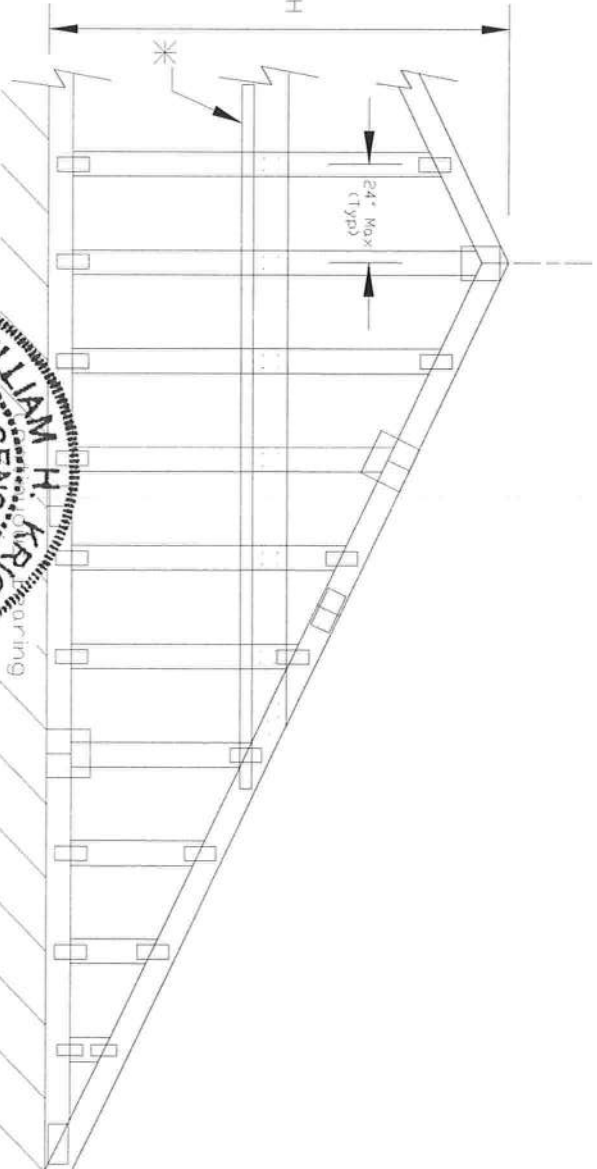
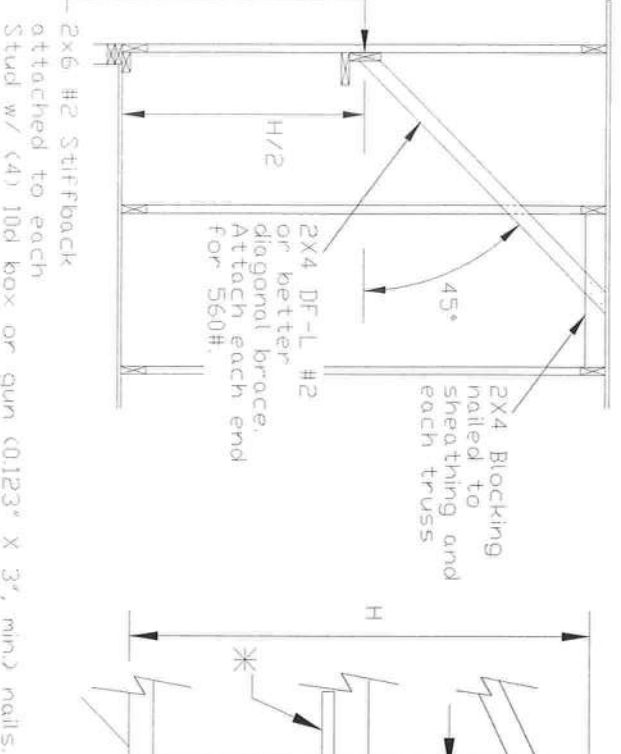
ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or 100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or 100 mph, 30ft. Mean Hgt, ASCE 7-10, Por't, Enclosed, Exp C, Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

Nails: 10d box or gun (0.128"x3" min) nails.



- H Less than 4'6" - no stud bracing required
- H Greater than 4'6" to 7'6" in length provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC10).
- H Greater than 7'6" to 12'0" max: provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC10).
- * Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min) nails @ 6" o.c.



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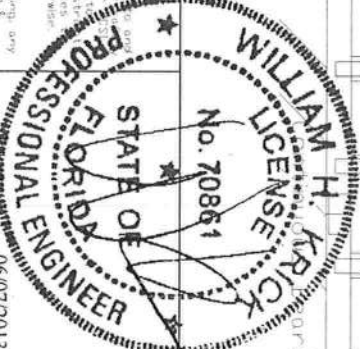
Earth City, MO 63045

Tw Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in accordance with AISI/TPI 1, or for installing, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design shall have to properly attached right ceiling. Locations shown for permanent lateral bracing of truss shall have to properly attached per ECSI sections B3, B7 or B10, as applicable. Apply plates each face of truss and position as shown above and on the sheet details, unless noted otherwise. Refer to drawings 10d/2 for standard plate positions.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
WARNING FURNISH THIS DRAWING TO ALL SUBCONTRACTORS INCLUDING THE INSTALLERS

Tw Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in accordance with AISI/TPI 1, or for installing, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design shall have to properly attached right ceiling. Locations shown for permanent lateral bracing of truss shall have to properly attached per ECSI sections B3, B7 or B10, as applicable. Apply plates each face of truss and position as shown above and on the sheet details, unless noted otherwise. Refer to drawings 10d/2 for standard plate positions.

For more information see this job's general notes page and the following website: <http://www.twbuilding.com>, <http://www.tpi.com>, <http://www.electrifying.org>, <http://www.iccsafe.org>



Jun 07 '13

REF	GE	WHALE
DATE	2/14/12	
DRWG	GABRST100212	
MAX. TOT. LD.	60 PSF	
MAX. SPACING		

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, $K_{zt} = 1.00$

Dr- 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Dr- 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Bracing Group Species and Grades

Group A: Hem-Fir

Spruce/Pine-Fir

#1 / #2	Standard
#3	Stud

Douglas Fir-Larch

#3	
Stud	
Standard	

Group B: Hem-Fir

#1 & Str	
#1	

Southern Pine***

#1	
#2	

Group C: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#3	
Stud	
Standard	

Group D: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group E: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group F: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group G: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group H: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group I: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group J: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group K: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group L: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group M: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group N: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group O: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group P: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group Q: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group R: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group S: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group T: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group U: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group V: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group W: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group X: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group Y: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group Z: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group AA: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group AB: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group AC: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group AD: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group AE: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group AF: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group AG: Hem-Fir

#2	Stud
#3	Standard

Southern Pine***

#1	
#2	

Group AH

Gable Truss Detail Notes:

Wind Load deflection criterion is $L/240$

Provide uplift connections for 35 pcf over

continuous bearing (5 psf TC Dead Load)

Gable end supports load from 4' 0" outboard

with a 1/2" overhang, or 1" plywood over

So: Pine lumber design values based on the AISC Handbook 2010 and 11th Edition.

the 25th January, due rain

Attach 1" braces with 10d (0.128"x30" min) nails

* For (1) L brace space holes at 2" o.c.
in 18" end zones and 4" o.c. between zones.

* * * * * (2) "L" braces: space runs at 3' oc.

in 18° end zones and 6° o.c. between zones.

"L" bracing must be a minimum of 80% of web

member length:

Cable Vertical Plate Sizes

Vertical Length	No Splice
-----------------	-----------

Less than 4' 0"	1X4 or 2X3
-----------------	------------

Greater than 4' 0", but 2x4

less than 11.6°	
Greater than 11.6°	29X4

	DATE	BY	CHECKED BY
* Refer to common truss design for			

peak, splice, and heel plates.

Refer to the Budgets section for more information.

not addressed by this detail.

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
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REF ASCE7-10-CAB12015

DATE 3/14/12

DATE 5/17/16

DRWG A12015ENC10021

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1760 PSE

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
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ING 24.0"

[illegible]