Roof

Force Engineering & Testing Inc.
19530 Ramblewood Drive
Humble,TX 77338

Product Evaluation Report

TRI COUNTY METALS

Min. 26 Ga. TCM-Lok Roof Panel over 15/32" Plywood

Florida Product Approval # 16522

Florida Building Code 2010
Per Rule 9N-3
Method: 1 –D

Category: Roofing
Subcategory: Metal Roofing
Compliance Method: 9N-3.005(1)(d)
NON HVHZ

Product Manufacturer:
Tri County Metals
301 SE 16th Street
Trenton, Florida 32693

Engineer Evaluator:
Terrence E. Wolfe, P.E. # 44923
Florida Evaluation ANE ID: 1920

Validator:
Locke Bowden, P.E., FL #49704
9450 Alysbury Place
Montgomery, AL 36117

Contents:
Evaluation Report Pages 1 – 4

TERRENCE E. WOLFE LICENSE No. 44923 STATE OF FLORIDA PROFESSIONAL ENGINEER

State of Florida C.O.A. #26778

August 13, 2013

Force Engineering & Testing Inc.
19530 Ramblewood Drive
Humble,TX 77338

Code Compliance: The product described herein has demonstrated compliance with The Florida Building Code 2010, Section 1504.3.2.

Evaluation Report Scope: The product evaluation is limited to compliance with the structural wind load requirements of the Florida Building Code 2010, as relates to Rule 9N-3.

Performance Standards: The product described herein has demonstrated compliance with:

- UL 580-06 - Test for Uplift Resistance of Roof Assemblies
- UL 1897-04 - Uplift Test for Roof Covering Systems

Reference Data:

1. UL 580-06 / 1897-04 Uplift Test
   Force Engineering & Testing, Inc. (FBC Organization # TST-5328)
   Report No. 136-0087T-13
2. Certificate of Independence
   By Terrence E. Wolfe, P.E. (No. 44923) @ Force Engineering & Testing, Inc.
   (FBC Organization # ANE ID: 1920)

Quality Assurance Entity: The manufacturer has established compliance of roof panel products in accordance with the Florida Building Code and Rule 9N-3.005 (3) for manufacturing under a quality assurance program audited by an approved quality assurance entity.

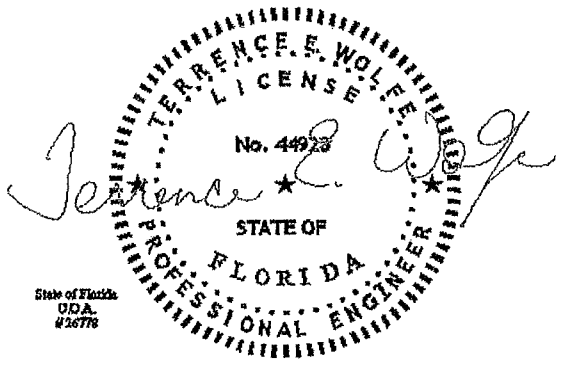
Minimum Slope Range: Minimum Slope shall comply with Florida Building Code 2010, including Section 1507.4.2 and in accordance with Manufacturers recommendations. For slopes less than 3:12, lap sealant must be used in the panel side laps.

Installation: Install per manufacturer's recommended details.

Underlayment: Per Manufacturer's installation guidelines per Florida Building Code 2010 Section 1507.4.5.

Roof Panel Fire Classification: Fire classification is not part of this acceptance.

Shear Diaphragm: Shear diaphragm values are outside the scope of this report.



FL# 16522

August 13, 2013

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Humble,TX 77338

Design Procedure:

Based on the dimensions of the structure, appropriate wind loads are determined using Chapter 16 of the Florida Building Code 2010 for roof cladding wind loads. These component wind loads for roof cladding are compared to the allowable pressure listed above. The design professional shall select the appropriate erection details to reference in his drawings for proper fastener attachment to his structure and analyze the panel fasteners for pullout and pullover. Support framing must be in compliance with Florida Building Code 2010 Chapter 22 for steel, Chapter 23 for wood and Chapter 16 for structural loading.

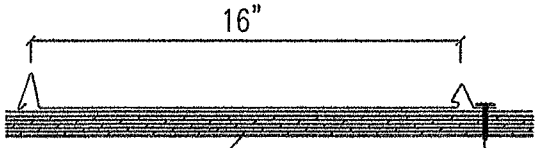
TERRENCE E. WOLFE
LICENSE
No. 44923
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

State of Florida
C.O.A.
#26778

FL# 16522

August 13, 2013

TCM-LOK 26 GA. ROOF PANEL



15/32" PLYWOOD
W/ ONE LAYER OF EXISTING
SHINGLES/FELT PAPER (OPTIONAL)

#10-12 x 1" TYPE A PANCAKE
AT 5 1/4" O.C.

State of Florida
C.O.A.
29774

TERRENCE E. WOLFE
LICENSE
No. 41923
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

August 13, 2013

JULIUS LEE PE.

RE 521322 - O'NEIL CONST - REEVES RES

1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435

Site Information:
Project Customer O'Neil Const Project Name 521322 Model Reeves Res
Lot/Block Subdivision
Address 417 SW Hilltop Terrace
City Columbia Cty State FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name Unknown at time of seals License # Unknown at time of seals
Address Unknown at time of seals
City Unknown at time of seals State Unknown at time of seals

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code FBC2010/TPI2007 Design Program. MiTek 20/20 7 3
Wind Code. ASCE 7-10 Wind Speed· 130 mph Floor Load· N/A psf
Roof Load. 50.0 psf

This package includes 6 individual, dated Truss Design Drawings and 0 Additional Drawings
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules
This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

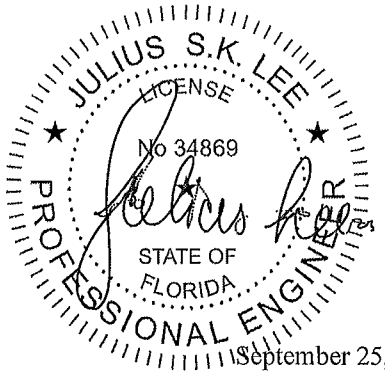
No	Seal#	Truss Name	Date
1	I7294655	T01	9/25/013
2	I7294656	T02	9/25/013
3	I7294657	T03	9/25/013
4	I7294658	T03G	9/25/013
5	I7294659	T04	9/25/013
6	I7294660	T05	9/25/013

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax)

Truss Design Engineer's Name Julius Lee

My license renewal date for the state of Florida is February 28, 2015

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2



September 25,2013

Julius Lee

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST REEVES RES	I7294655
521322	T01	ROOF TRUSS	13	1	Job Reference (optional)	

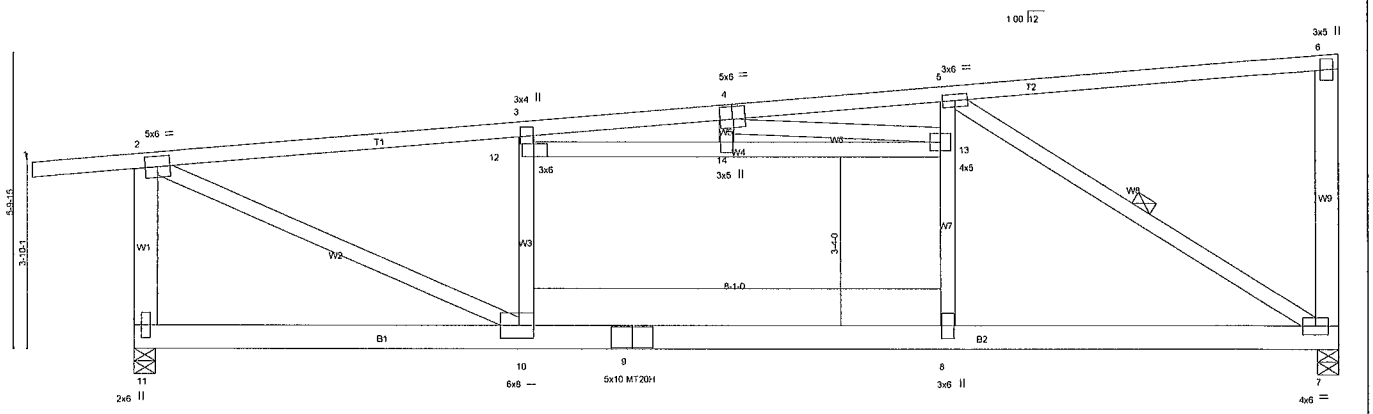
Builders FirstSource Lake City FL 32055

7.350 s Sep 27 2012 MiTek Industries Inc. Wed Sep 25 10:19:58 2013 Page 1

ID BdOFrdQ122jR1Mb5pLe1CxzwohK YtiRgsOuyFq8kbDpoVlNHOScOcs2wKjP7qmZz3ya7?V

-2-0-8 2-0-8 | 7 11-8 7 11-8 | 11-9-8 3-10-0 | 16-0-8 4-3-0 | 23-11-0 7 10-8

Scale 1 43.0



7 11-8 7 11-8 | 16-0-8 8-1-0 | 23-11-0 7 10-8

Plate Offsets (X,Y). [3.0-2-5,0-0-5], [4.0-3-0,0-3-0], [10 0-3-7,0-3-0], [12.0-2-12,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30 0	Plates Increase	1 25	TC	0 85	Vert(LL)	-0 23	8-10	>999	240	MT20	244/190
TCDL	10 0	Lumber Increase	1.25	BC	0 88	Vert(TL)	-0 47	8-10	>602	180	MT20H	187/143
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 98	Horz(TL)	0 03	7	n/a	n/a		
BCDL	10 0	Code FBC2010/TPI2007		(Matrix-M)		Attic	-0 14	8-10	717	360	Weight. 169 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2 *Except*
B2 2x6 SP SS
WEBS 2x4 SP No.3 *Except*
W9 W1 2x6 SP No.2 W7 2x4 SP No 2

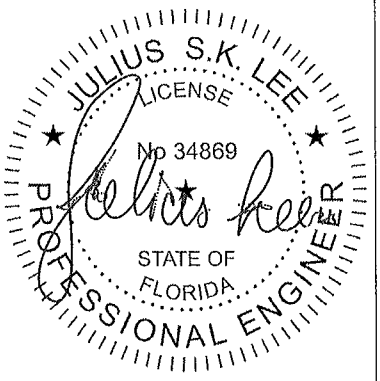
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1 12 oc purlins except end verticals
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=1406/0-5-0 (min 0-1 14) 11=1565/0-5-0 (min 0-2-0)
Max Horz 11=206(LC 9)
Max Uplift7=-125(LC 12) 11=-235(LC 8)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-2242/349 3-4=-2600/461 4-5=-1862/335 6-7=-262/147 2-11=-1616/456
BOT CHORD 10-11=-361/317 9-10=-488/2254 8-9=-488/2254 7-8=-464/2178
WEBS 10-12=-611/304 3-12=-548/306 8-13=0/791 5-13=0/895 5-7=-2488/452 2 10=-362/2353
12-14=-157/445, 13-14=-163/443 4-13=-833/286

NOTES (11 13)
1) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf· BCDL=3 0psf· h=21ft Cat. II Exp C Encl GCpi=0 18, MWFRS (envelope) and C-C Exterior(2) zone cantilever left and right exposed end vertical left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1 60
2) All plates are MT20 plates unless otherwise indicated
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
5) Ceiling dead load (5.0 psf) on member(s) 12-14 13-14 Wall dead load (5 0psf) on member(s) 10-12 8-13
6) Bottom chord live load (40 0 psf) and additional bottom chord dead load (10 0 psf) applied only to room 8-10
7) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 125 lb uplift at joint 7 and 235 lb uplift at joint 11
9) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
10) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
12) Note· Visually graded lumber designation SPp represents new lumber design values as per SPIB
13) Truss Design Engineer· Julius Lee, PE. Florida P E License No 34869· Address. 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard



September 25,2013

⚠ *WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.*
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design paramenters and proper incorporation of component is responsibility of building designer not truss designer Bracing shown is for lateral support of individual web members only Additional temporary bracing to insure stability during construction is the responsibility of the erector Additional permanent bracing of the overall structure is the responsibility of the building designer For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute 583 D'Onofrio Drive Madison, WI 53719

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST REEVES RES.	I7294656
521322	T02	Monopitch Truss	13	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 25 10:20:00 2013 Page 1

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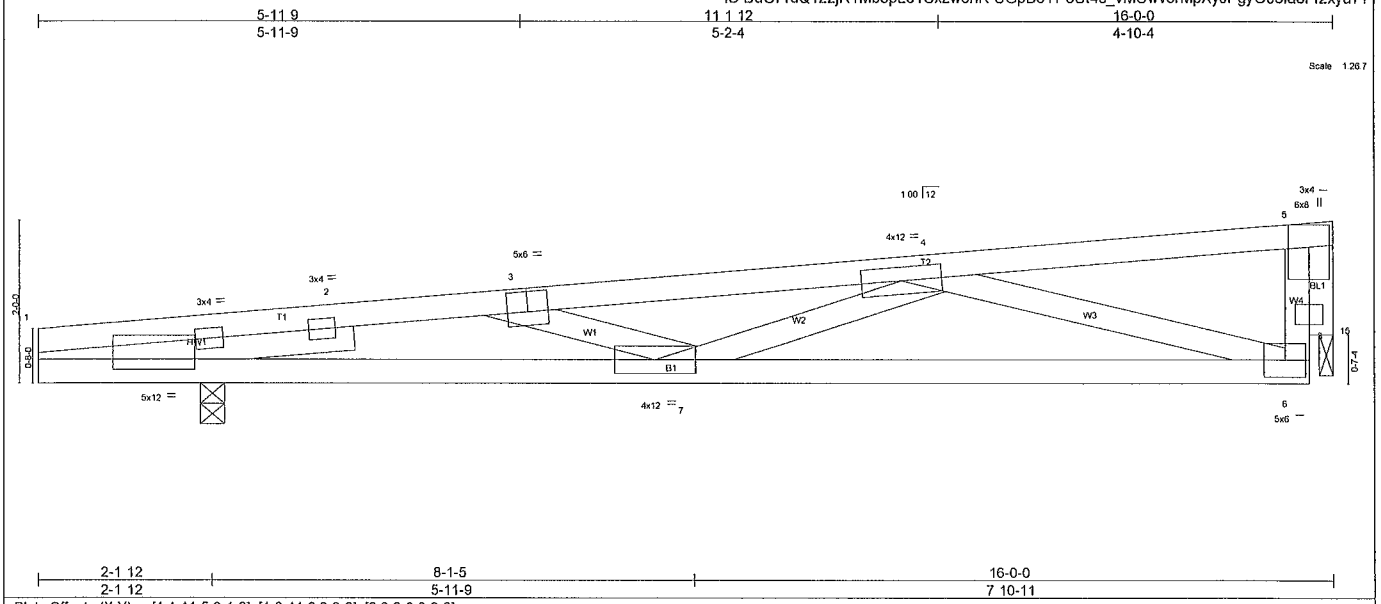


Plate Offsets (X,Y) [1 1 11-5,0-1 8], [1 0-11-2,0-2-8], [3 0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30 0	Plates Increase	1.25	TC	0 89	Vert(LL)	0.42	6-7	>453	240	MT20	244/190
TCDL	10 0	Lumber Increase	1.25	BC	0 34	Vert(TL)	-0 35	6-7	>542	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0 02	15	n/a	n/a		
BCDL	10 0	Code FBC2010/TPI2007		(Matrix-M)							Weight 73 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP M 31
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3
SLIDER Left 2x4 SP No 3 3-11-0

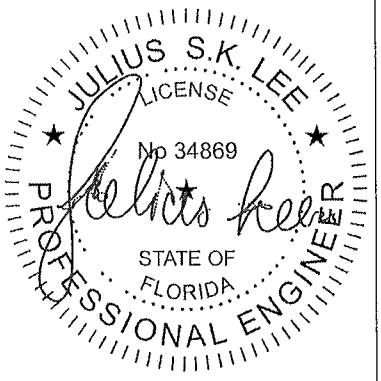
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-14 oc bracing

REACTIONS (lb/size) 1=773/0-3-8 (min. 0-1-8) 15=550/0-2 0 (min 0-1-8)
Max Horz 1=45(LC 8)
Max Uplift 1=-402(LC 8) 15=-291(LC 8)
Max Grav 1=910(LC 2) 15=646(LC 2)

FORCES (lb) Max Comp./Max. Ten All forces 250 (lb) or less except when shown
TOP CHORD 1 1=-261/385 1 2=-274/399 2-3=-1968/1960 3-4=-1891/2172 4-5=-242/262 6-8=-618/523
5-8=-618/523
BOT CHORD 1 9=-1981/1938 1 7=-1981/1938 6-7=-1606/1682
WEBS 4 7=-646/431 4-6=-1520/1394

NOTES (9-11)
1) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=21ft; Cat. II Exp C Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone, cantilever left exposed porch left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface
6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 15
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 402 lb uplift at joint 1 and 291 lb uplift at joint 15
8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
10) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB
11) Truss Design Engineer Julius Lee PE. Florida P E License No. 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



September 25,2013

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Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST REEVES RES	
521322	T03	Monopitch Truss	8	1		I7294657
Builders FirstSource	Lake City FL 32055				Job Reference (optional)	

7 350 s Sep 27 2012 MiTek Industries Inc. Wed Sep 25 10:20:00 2013 Page 1
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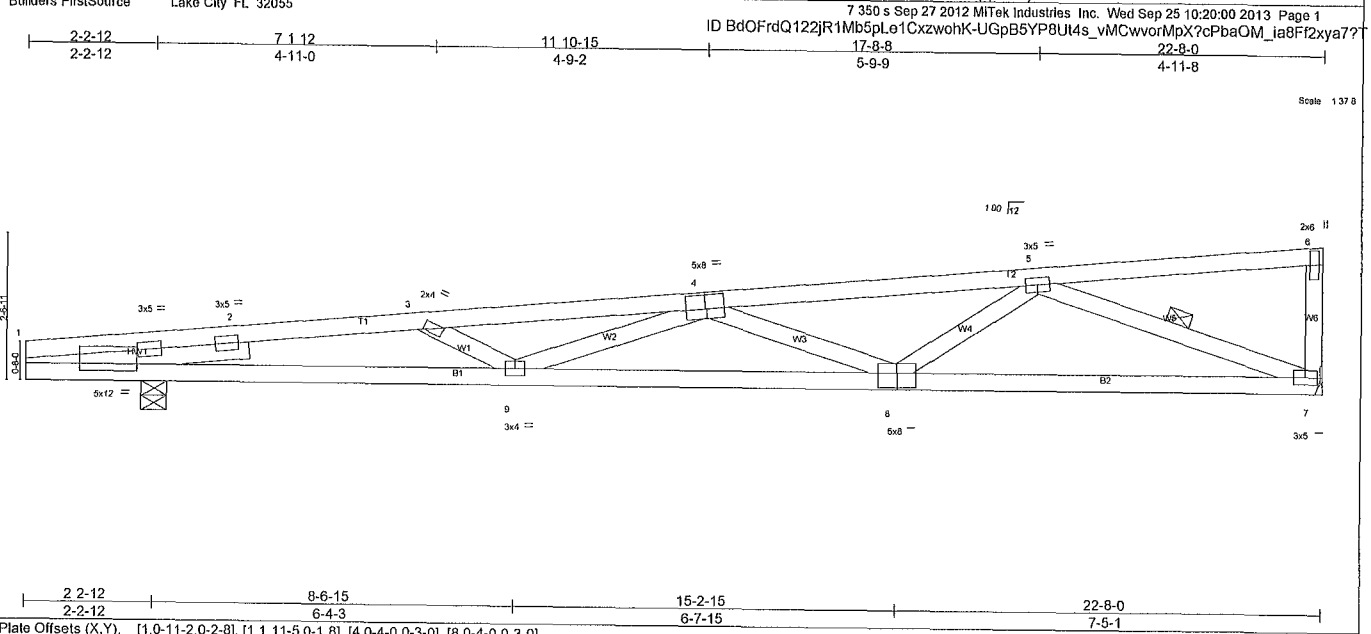


Plate Offsets (X,Y). [1,0-11-2,0-2-8], [1,1 11-5,0-1 8], [4 0-4-0,0-3-0], [8,0-4-0,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 30 0	Plates Increase 1 25	TC 0 68	Vert(LL)	-0.40	8-9	>670	240	MT20	244/190
TCDL 10 0	Lumber Increase 1 25	BC 0 69	Vert(TL)	-0.81	8-9	>333	180		
BCLL 0 0 *	Rep Stress Incr YES	WB 0.49	Horz(TL)	0.09	7	n/a	n/a		
BCDL 10 0	Code FBC2010/TPI2007	(Matrix-M)						Weight 101 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except*
T1 2x4 SP M 31
BOT CHORD 2x4 SP M 31
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No 3 3-11-0

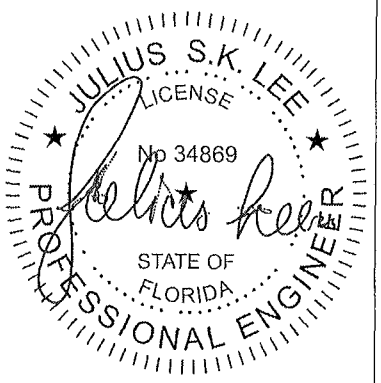
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11 1 oc purlins except end verticals
BOT CHORD Rigid ceiling directly applied or 6-3-8 oc bracing
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=864/Mechanical 1=1050/0-5-8 (min 0-1-8)
Max Horz 1=66(LC 8)
Max Uplift7=-213(LC 8) 1 250(LC 8)
Max Grav 7=1016(LC 2) 1=1236(LC 2)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 1 1=-220/407 1 2=-220/429 2-3=-3429/1280 3-4=-3588/1265 4-5=-2903/1007
BOT CHORD 1 10=-1339/3380 1-9=-1339/3380 8-9=-1505/3859 7-8=-824/2089
WEBS 3-9=0/325 4-9=-310/201 4-8=-1065/508 5-8=-262/980 5 7=-2186/871

NOTES (7 10)
1) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=21ft, Cat. II Exp C, Encl GCpi=0 18, MWFRS (envelope) and C-C Exterior(2) zone cantilever left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1.60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
4) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 7 and 250 lb uplift at joint 1
6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
8) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB
9) Truss Design Engineer Julius Lee PE. Florida P E. License No. 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435
10) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



September 25,2013

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Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST REEVES RES.
521322	T03G	GABLE	1	1	I7294658
					Job Reference (optional)

Builders FirstSource Lake City FL 32055 — 7.350 s Sep 27 2012 MiTek Industries Inc. Wed Sep 25 10:20:02 2013 Page 1

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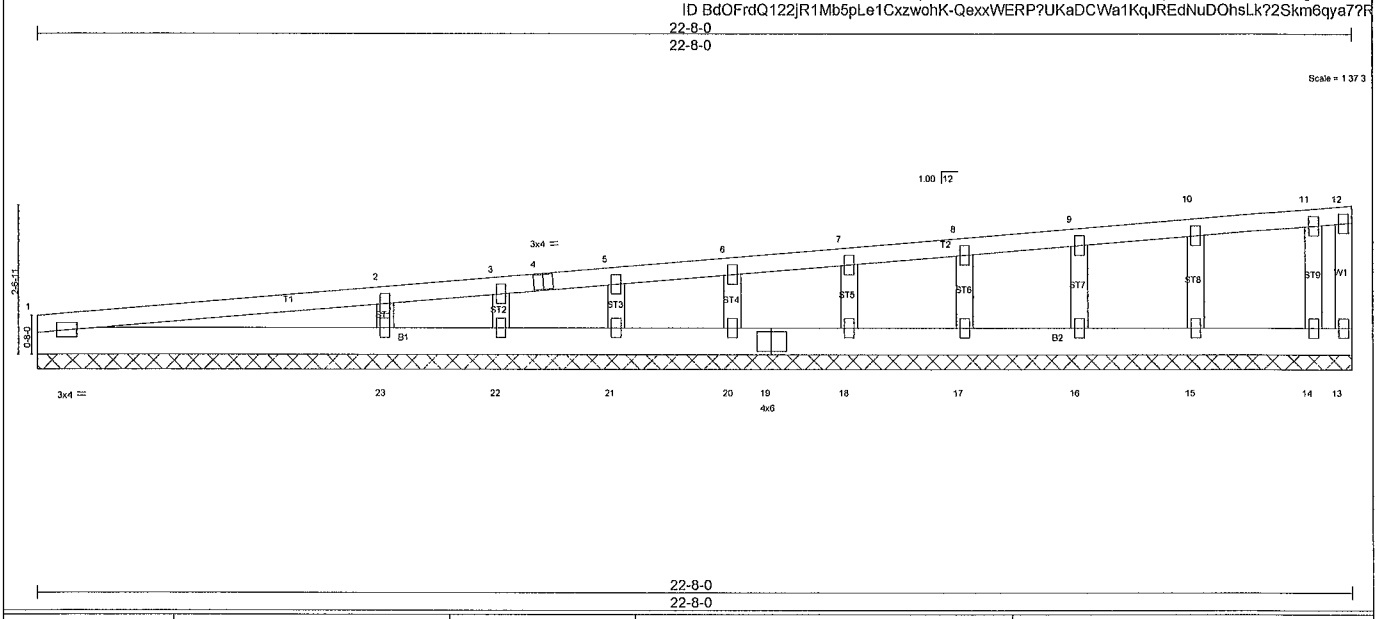


22-8-0
22-8-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30 0	Plates Increase	1.25	TC	0 50	Vert(LL)	n/a		n/a	999	MT20	244/190
TCDL	10 0	Lumber Increase	1 25	BC	0 13	Vert(TL)	n/a		n/a	999		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 09	Horz(TL)	-0 00	13	n/a	n/a		
BCDL	10 0	Code FBC2010/TPI2007		(Matrix)							Weight 104 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No 3
OTHERS 2x4 SP No 3

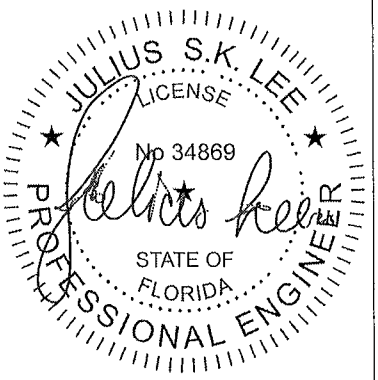
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS All bearings 22-8-0
(lb) Max Horz 1=76(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 13, 1 22 21 20 18 17 16, 15 14 except 23=-142(LC 8)
Max Grav All reactions 250 lb or less at joint(s) 1 22 20 18 17 16 15 14 except 23=663(LC 2) 21=267(LC 2)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown.
WEBS 2-23=-480/264

NOTES (11 13)
1) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3 0psf h=21ft, Cat. II Exp C Encl GCpi=0 18, MWFRS (envelope) and C-C Exterior(2) zone cantilever left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
2) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face) see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1
3) All plates are 2x4 MT20 unless otherwise indicated
4) Gable requires continuous bottom chord bearing
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 1 22 21 20 18, 17 16 15 14 except (jt=lb) 23=142
10) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
12) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB.
13) Truss Design Engineer Julius Lee PE Florida P E License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



September 25,2013

WARNING ***Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.***
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer Bracing shown is for lateral support of individual web members only Additional temporary bracing to insure stability during construction is the responsibility of the erector Additional permanent bracing of the overall structure is the responsibility of the building designer For general guidance regarding fabrication, quality control, storage delivery erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST REEVES RES.
521322	T04	Monopitch Truss	12	1	I7294659

Builders FirstSource Lake City FL 32055

Job Reference (optional)

7.350 s Sep 27 2012 MiTek Industries Inc. Wed Sep 25 10:20:03 2013 Page 1

ID:BdOFrdQ122jR1Mb5pLe1CxzwohK-vrVJkaR1moSRrM5nb2LY_R9Wfdbzbie8G6UKeGya7?Q

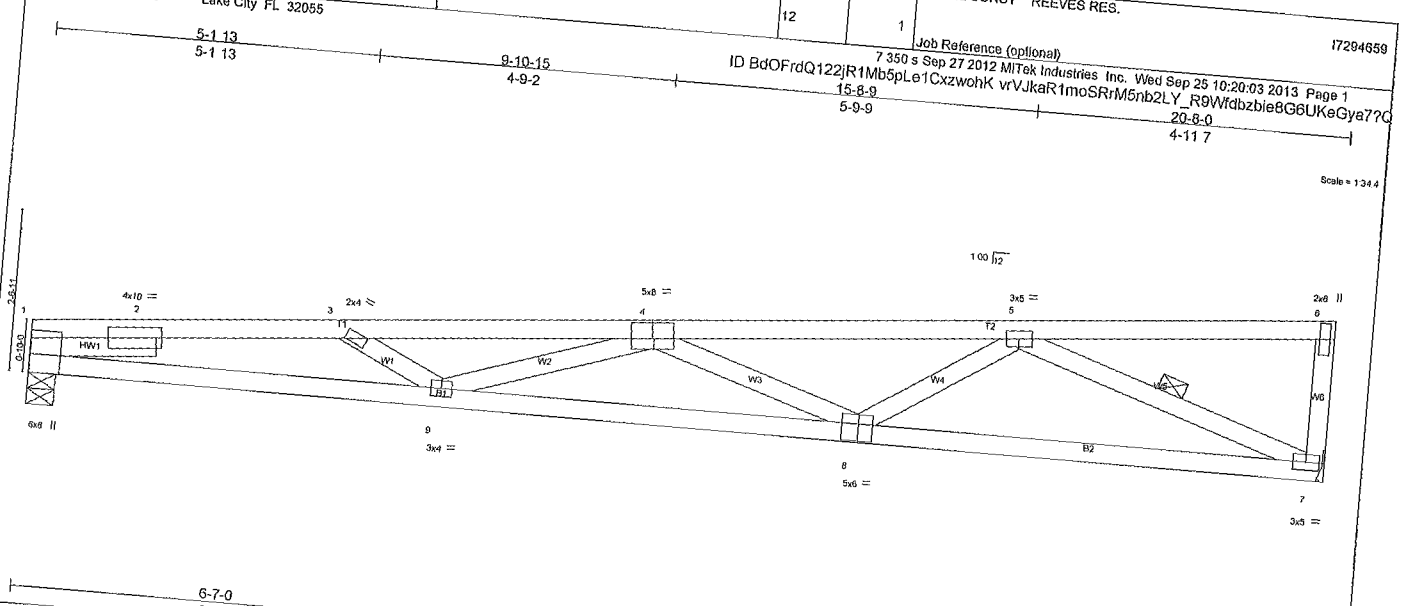


Plate Offsets (X,Y) [1:0-6-8,Edge], [4:0-4-0,0-3-0], [6:0-2-12,0-3-4]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.63	Vert(LL)	-0.39	8-9	>631	240	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.70	Vert(TL)	-0.79	8-9	>313	180		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.49	Horz(TL)	0.09	7	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix-M)						Weight: 92 lb	FT = 20%
	Code FBC2010/TPI2007								

LUMBER
TOP CHORD 2x4 SP No.2 *Except*
T1 2x4 SP M 31
BOT CHORD 2x4 SP M 31
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 1-11-15

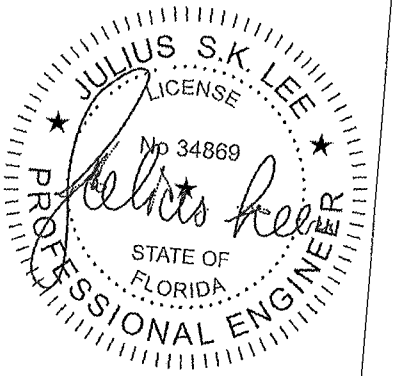
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-2-15 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=872/Mechanical, 1=872/0-5-8 (min. 0-1-8)
Max Horz 1=60(LC 8)
Max Uplift 7=-215(LC 8), 1=-207(LC 8)
Max Grav 7=1026(LC 2), 1=1026(LC 2)

FORCES (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-3637/1353, 3-4=-3743/1320, 4-5=-2942/1021
BOT CHORD 1-9=-1412/3590, 8-9=-1530/3931, 7-8=-833/2114
WEBS 3-9=0/266, 4-8=-1100/520, 5-8=-269/998, 5-7=-2212/881

NOTES (7-10)
1) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf; BCDL=3.0psf; h=21ft, Cat. II, Exp C, Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=215, 1=207.
6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI/TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE. Florida P.E. License No. 34869. Address. 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
10) Use Simpson HTU26 to attach Truss to Carrying member.

LOAD CASE(S) Standard



September 25,2013

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Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive; Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST REEVES RES.	I7294660
521322	T05	Special Truss	19	1	Job Reference (optional)	

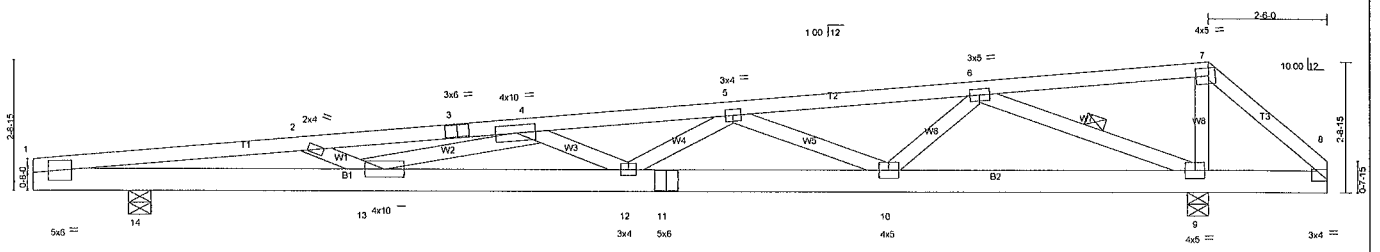
Builders FirstSource Lake City FL 32055

7 350 s Sep 27 2012 MiTek Industries Inc. Wed Sep 25 10:20:04 2013 Page 1

ID:BdOFrdQ122jR1Mb5pLe1CxzwohK-N13ixvSfX5aISWgz9lsnXfife0uAK9nIVmDtBjya7?P

5-11-9	10-6-11	14-9-14	20-0-11	24-11-0	27-5-0
5-11-9	4-7 1	4-3-4	5-2-13	4-10-5	2-6-0

Scale = 1 45 8



2-2-12	7-0-8	12-7-5	18-1-9	24-8-4	24-11-0	27-5-0
2-2 12	4-9-12	5-6-13	5-6-4	6-6-11	0-2-12	2-6-0

Plate Offsets (X,Y): [1,0-2-4,0-2-8], [7:0-3-4,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30 0	Plates Increase	1.25	TC	0 74	Vert(LL)	-0.48	12-13	>568	240	MT20	244/190
TCDL	10 0	Lumber Increase	1 25	BC	0 90	Vert(TL)	-0 94	12-13	>289	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0.50	Horz(TL)	0 08	9	n/a	n/a		
BCDL	10 0	Code FBC2010/TPI2007		(Matrix-M)							Weight. 141 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except*
T1 2x4 SP M 31
BOT CHORD 2x6 SP SS *Except*
B2. 2x6 SP No.2
WEBS 2x4 SP No 3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-7 oc purlins
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
WEBS 1 Row at midpt 6-9

REACTIONS (lb/size) 9=1187/0-5-8 (min 0-1 10) 14=1144/0-5-8 (min 0-1 10)
Max Horz 14=49(LC 11)
Max Uplift9=-258(LC 8) 14=-333(LC 8)
Max Grav 9=1396(LC 2) 14=1355(LC 27)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 1 2=-3266/1075 2-3=-3089/925, 3-4=-3070/927 4-5=-4581/1563 5-6=-2763/903
BOT CHORD 1 14=-1049/3222 13-14=-1055/3222 12-13=-1592/4644 11 12=-1356/3995 10-11=-1356/3995
9-10=-658/2100
WEBS 4-13=-1658/735, 5-12=-184/680 5-10=-1394/579 6-10=-245/910 6-9=-2260/868, 7-9=-333/218

NOTES (8-10)

1) Unbalanced roof live loads have been considered for this design.
2) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf· BCDL=3 0psf· h=21ft, Cat. II Exp C Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone· cantilever left and right exposed ·C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1 60
3) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=258 14=333.
7) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
9) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
10) Truss Design Engineer· Julius Lee PE Florida P E License No. 34869· Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard

September 25,2013

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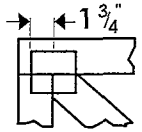
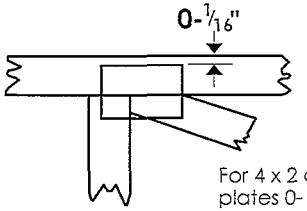
Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated
Dimensions are in ft-in-sixteenths
Apply plates to both sides of truss and fully embed teeth



For 4 x 2 orientation locate plates 0-1/16" from outside edge of truss

This symbol indicates the required direction of slots in connector plates

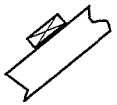
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

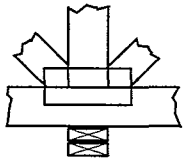
The first dimension is the plate width measured perpendicular to slots Second dimension is the length parallel to slots

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output Use T I or Eliminator bracing if indicated

BEARING

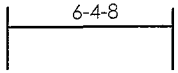


Indicates location where bearings (supports) occur Icons vary but reaction section indicates joint number where bearings occur

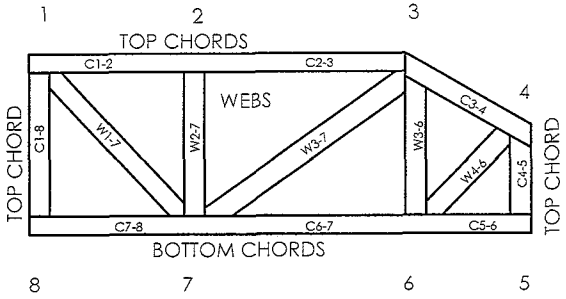
Industry Standards:

ANSI/TPI1 National Design Specification for Metal Plate Connected Wood Truss Construction
DSB-89 Design Standard for Bracing
BCSI1 Building Component Safety Information Guide to Good Practice for Handling Installing & Bracing of Metal Plate Connected Wood Trusses

Numbering System



dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS

PRODUCT CODE APPROVALS

ICC-ES Reports

ESR-1311 ESR-1352, ER-5243 9604B
9730 95-43 96-31 9667A
NER-487 NER-561
95110 84-32, 96-67 ER-3907 9432A

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Julius Lee PE
1109 Coastal Bay ,
Boynton Beach ,FL 33435

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1 Additional stability bracing for truss system e g diagonal or X-bracing is always required See BCSI1

2 Truss bracing must be designed by an engineer For wide truss spacing individual lateral braces themselves may require bracing or alternative T I, or Eliminator bracing should be considered

3 Never exceed the design loading shown and never stack materials on inadequately braced trusses.

4 Provide copies of this truss design to the building designer erection supervisor property owner and all other interested parties

5 Cut members to bear tightly against each other

6 Place plates on each face of truss at each joint and embed fully Knots and wane at joint locations are regulated by ANSI/TPI 1

7 Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1

8 Unless otherwise noted moisture content of lumber shall not exceed 19% at time of fabrication

9 Unless expressly noted this design is not applicable for use with fire retardant preservative treated, or green lumber

10 Camber is a non-structural consideration and is the responsibility of truss fabricator General practice is to camber for dead load deflection

11 Plate type size orientation and location dimensions indicated are minimum plating requirements

12 Lumber used shall be of the species and size and in all respects, equal to or better than that specified

13 Top chords must be sheathed or purlins provided at spacing indicated on design

14 Bottom chords require lateral bracing at 10 ft spacing or less if no ceiling is installed unless otherwise noted

15 Connections not shown are the responsibility of others.

16 Do not cut or alter truss member or plate without prior approval of an engineer

17 Install and load vertically unless indicated otherwise

18 Use of green or treated lumber may pose unacceptable environmental, health or performance risks Consult with project engineer before use

19 Review all portions of this design (front back, words and pictures) before use Reviewing pictures alone is not sufficient

20 Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria

August 10, 2010	T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY	ST - T-BRACE 2

MiTek Industries, Inc

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

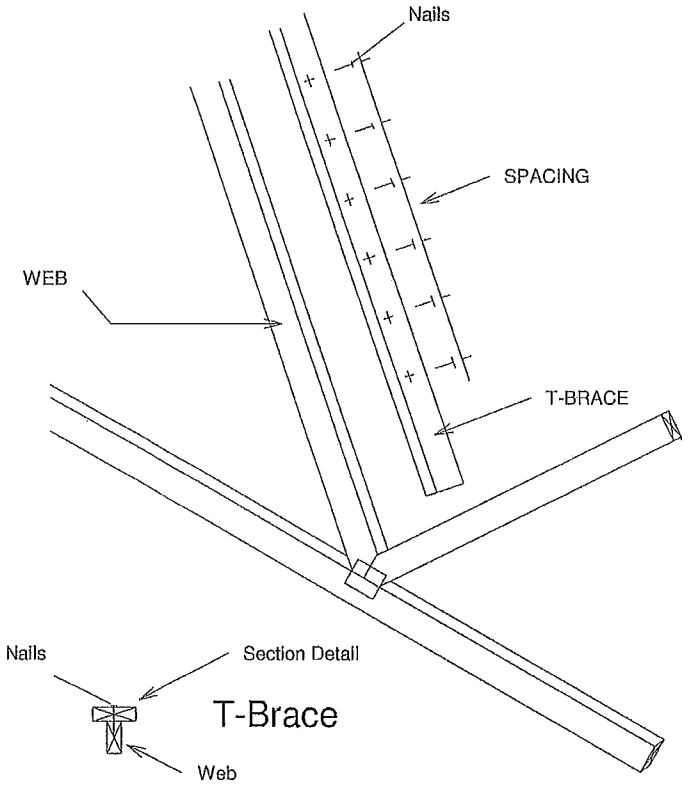
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note. Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

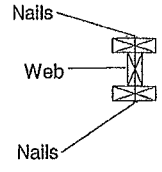
	Brace Size for One-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

	Brace Size for Two-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I Brace must be same species and grade (or better) as web member.



T-Brace



I-Brace

1109 COASTAL BAY
BOYNTON BC, FL 33435

JANUARY 1, 2009	LATERAL TOE-NAIL DETAIL	ST-TOENAIL_SP

MiTek Industries, Inc.

MiTek Industries, Chesterfield, MO

NOTES
1 TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
2 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD
3 ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)					
	DIAM	SYP	DF	HF	SPF	SPF-S
3.5" LONG	131	88 0	80 6	69 9	68 4	59 7
	135	93 5	85.6	74.2	72 6	63 4
	162	108 8	99.6	86.4	84.5	73.8
3.25" LONG	128	74 2	67.9	58.9	57 6	50 3
	.131	75 9	69.5	60 3	59 0	51 1
	148	81 4	74 5	64 6	63.2	52.5

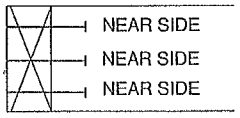
THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

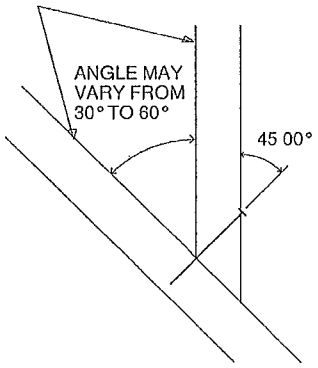
SIDE VIEW

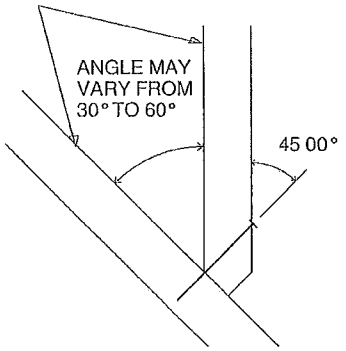
3 NAILS

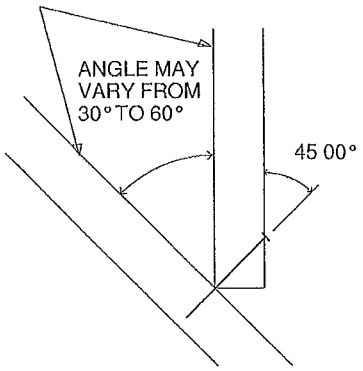


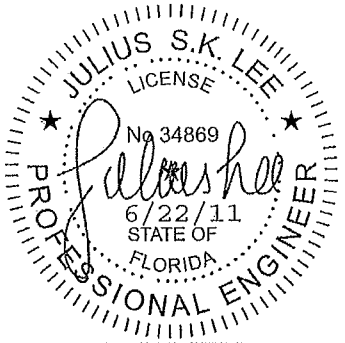
EXAMPLE
(3) 16d NAILS (162' diam x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1 15
3 (nails) X 84 5 (lb/nail) X 1 15 (DOL) = 291 5 lb Maximum Capacity









1109 COASTAL BAY
BOYNTON BC, FL 33435

FEBRUARY 14, 2012	STANDARD PIGGYBACK TRUSS CONNECTION DETAIL	ST-PIGGY-7-10

MiTek Industries, Chesterfield, MO

MiTek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7 10 DURATION OF LOAD INCREASE 1 60
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES) ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED

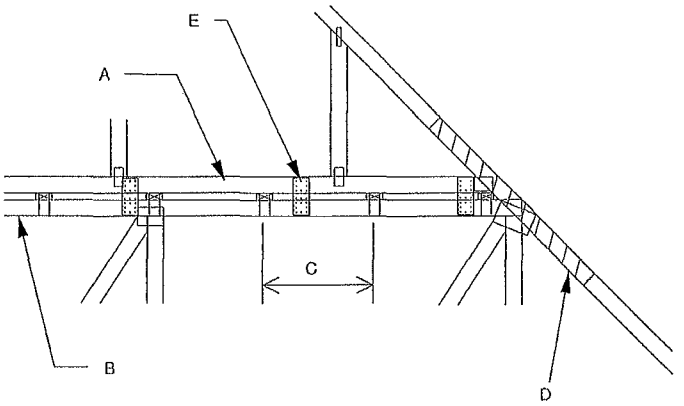
A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5' TOE NAILED

B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0 131 X 3.5" NAILS EACH.

D 2 X__ X 4'-0' SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3' NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT IN BOTH DIRECTIONS AND·
1 WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

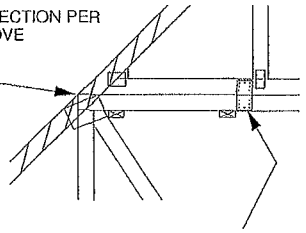
E FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72' O.C W/ (4) 0 131 X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5' EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ REGARDLESS OF SPAN)



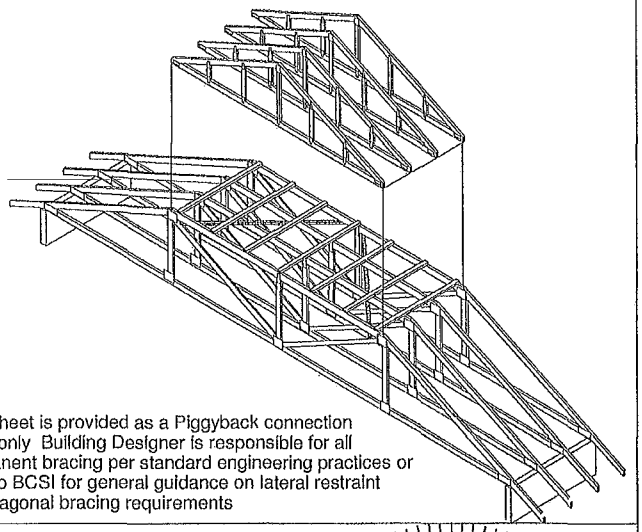
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING

SCAB CONNECTION PER NOTE D ABOVE

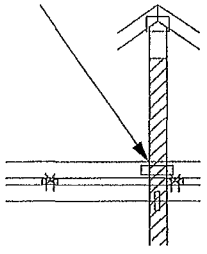


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48 O C. W/ (4) 0 131 X 1.5' PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5 EDGE DISTANCE



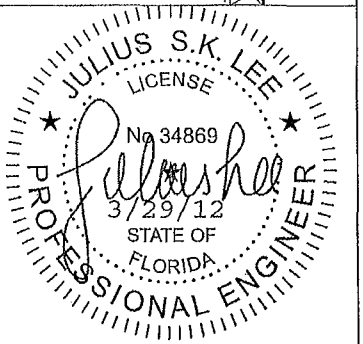
This sheet is provided as a Piggyback connection detail only Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE AND MUST LINE UP AS SHOWN IN DETAIL.
2) ATTACH 2 x ___ x 4 0' SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0 131 X 3") NAILS SPACED 4 O C. FROM EACH FACE (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1 15) REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN



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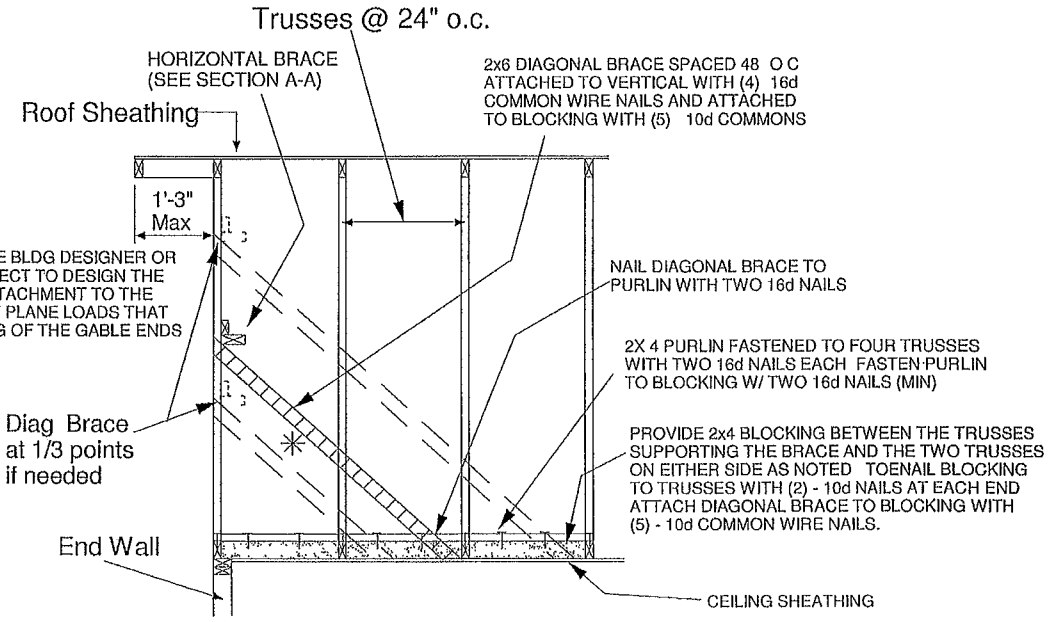
FEBRUARY 14, 2012	Standard Gable End Detail	SHEET 2

MiTek Industries, Inc

ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED

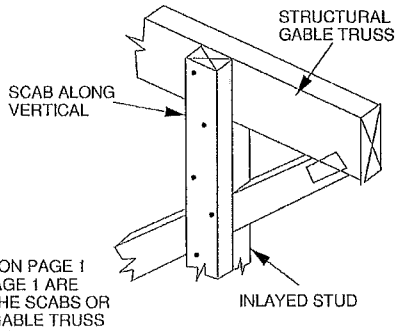
METHOD 1 ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE

METHOD 2 ATTACH 2X _ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

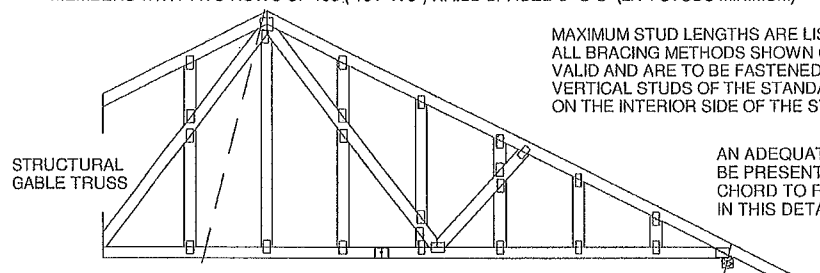
NAILING SCHEDULE

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7 10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (131" X 3") NAILS SPACED 6 O C

FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7 10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (131" X 3 ') NAILS SPACED 6 O C (2X 4 STUDS MINIMUM)

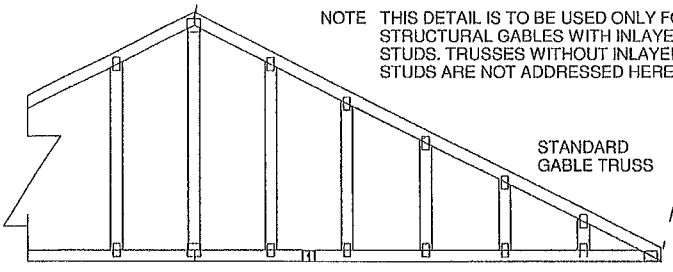


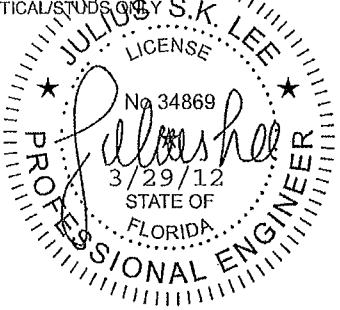
MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1 ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.



AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY

NOTE THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAYED STUDS. TRUSSES WITHOUT INLAYED STUDS ARE NOT ADDRESSED HERE





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