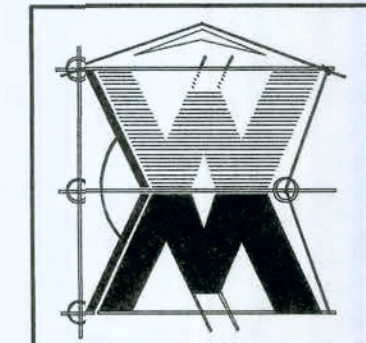


REVISIONS	DATE	BY	APP

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

A NEW SPEC FOR
LOT 14, ROLLING MEADOWS
ADAM'S FRAMING & CONSTRUCTION
LAKE CITY, FLORIDA

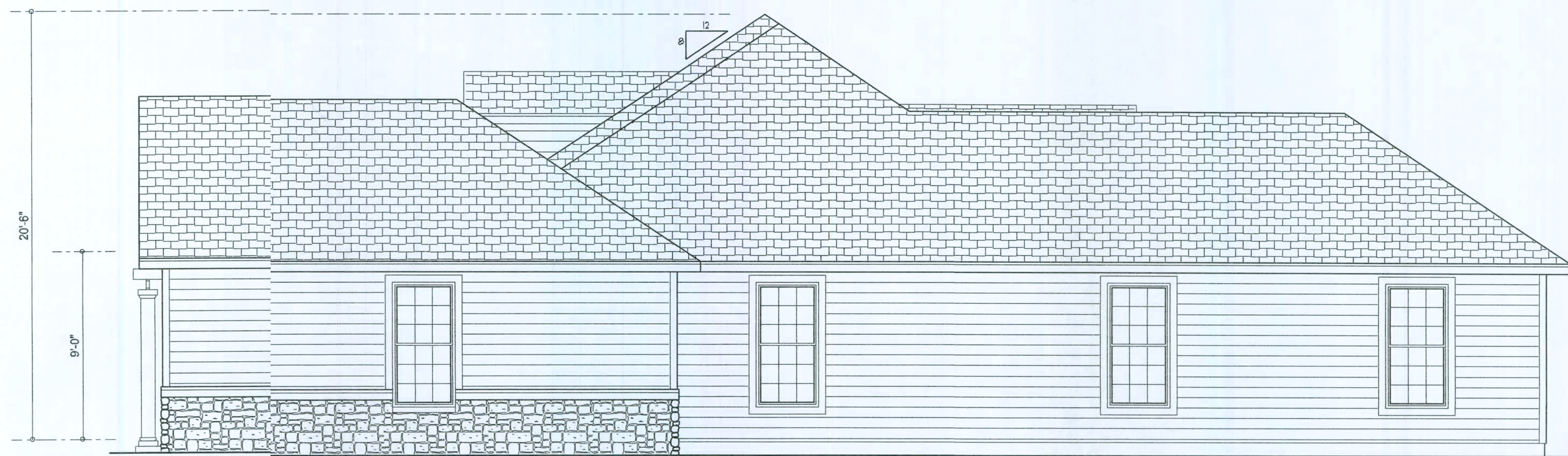
©WILLIAM MYERS
DESIGN
P.O. BOX 1513
LAKE CITY, FL 32055
(386) 758-8406
will@willmyers.net



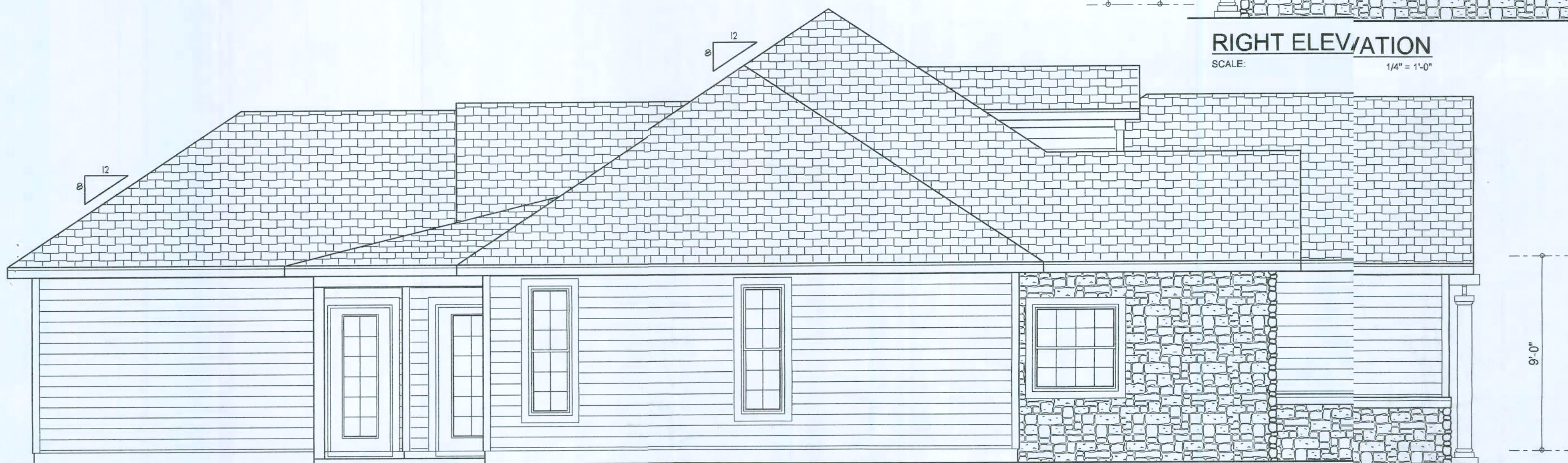
JOB NUMBER
080112

SHEET NUMBER

OF 3 SHEETS



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"

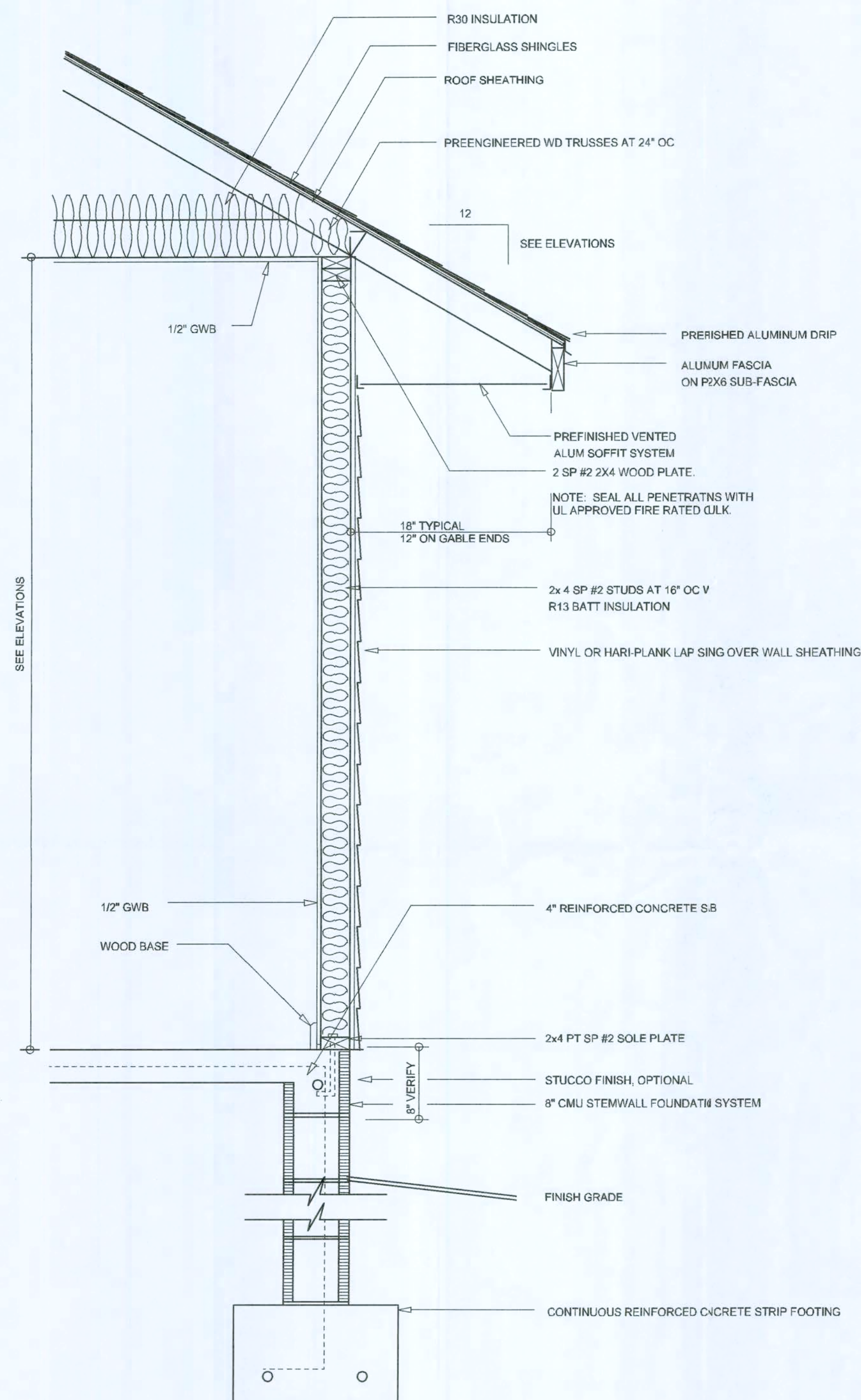


REAR ELEVATION
SCALE: 1/4" = 1'-0"



8/12 FRONT
SCALE: 1/4" = 1'-0"

W.C.M.



TYPICAL WALL SECTION

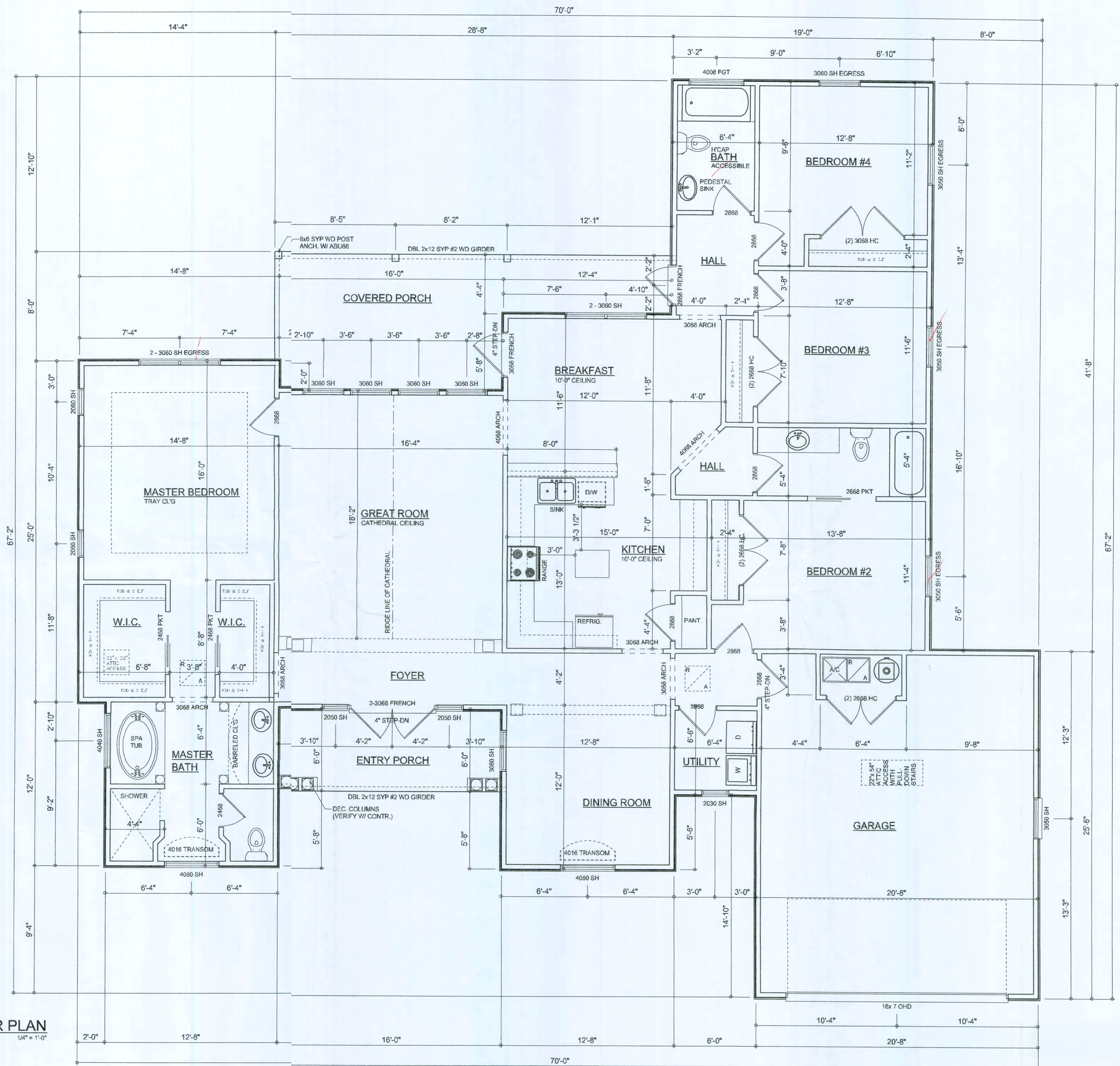
SCALE: 1/4" = 1'-0"

AREA SUMMARY

LIVING AREA	2262	S.F.
GARAGE AREA	519	S.F.
ENTRY PORCH AREA	96	S.F.
COVERED PORCH AREA	216	S.F.
TOTAL AREA	3093	S.F.

FLOOR PLAN

SCALE: 1/4" = 1'-0"



Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.

2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.

3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.

4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

Will C. Myers

REVISIONS
January 14, 2008

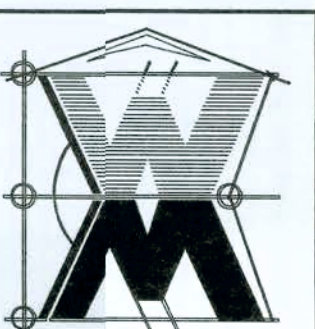
SCOTPLAN
ARCHITECTURAL DESIGN SOFTWARE

TYPICAL WALL SECTION
SCALE: 1/4" = 1'-0"

FLOOR PLAN
SCALE: 1/4" = 1'-0"

A NEW SPEC FOR
LOT 14, ROLLING MEADOWS
ADAM'S FRAMING & CONSTRUCTION
LAKE CITY, FLORIDA

©WILLIAM MYERS
DISIGN
P.O. BOX 1513
LAKE CITY, FL 32056
(386) 758-8406
will@willmyers.net

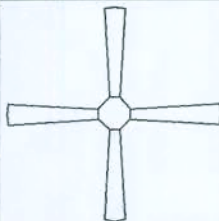


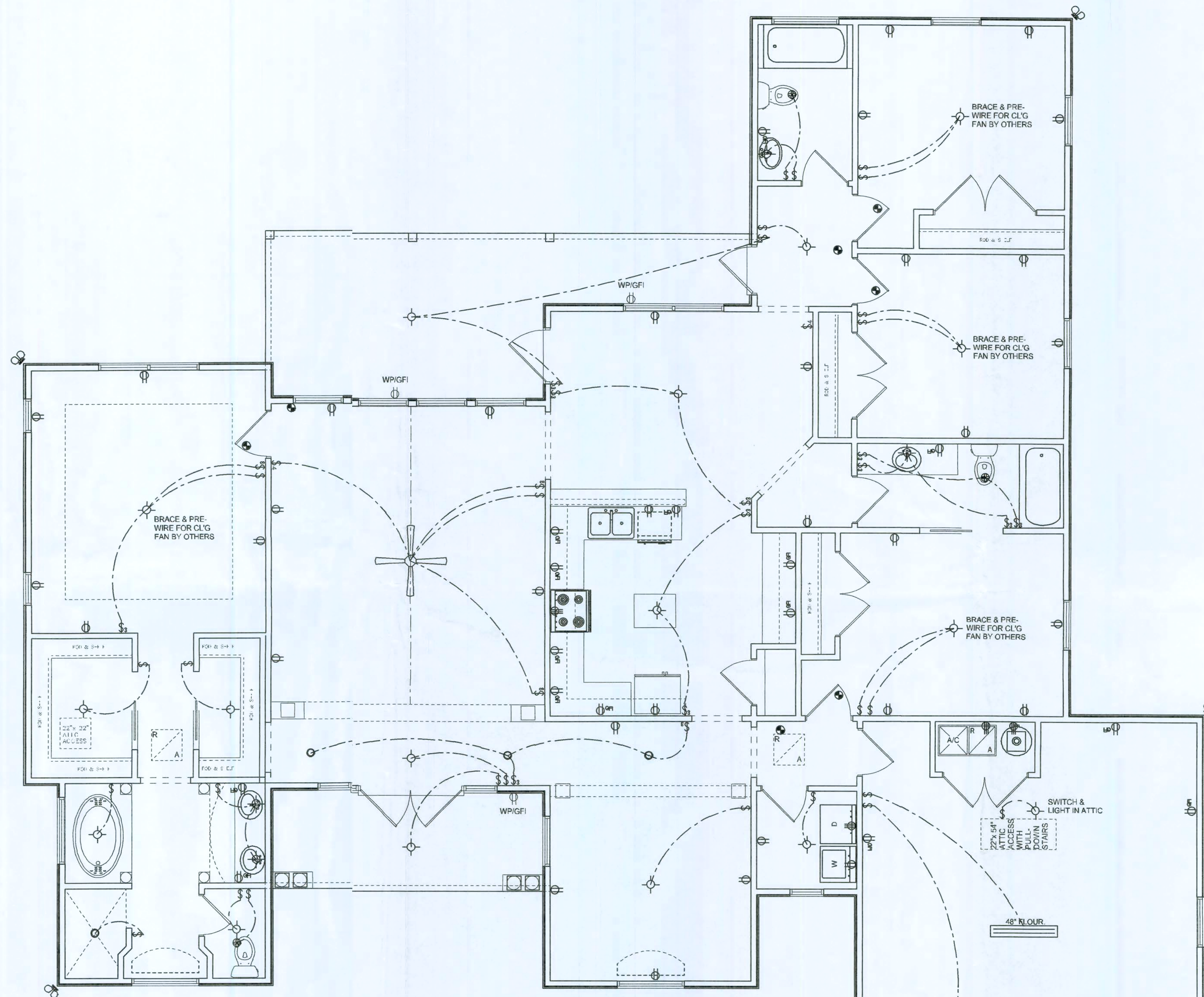
JOB NUMBER
080112

SHEET NUMBER

A.2

OF 3 SHEETS

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLUORESCENT FIXTURE



ELECTRICAL PLAN

SCALE: 1/4" = 1'-0"

NOTE:
ALL BEDROOM RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT)

ALL SMOKE DETECTORS SHALL HAVE BATTERY BACKUP POWER
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY
ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR
SHALL BE USED AS AN EQUIPMENT GROUND.

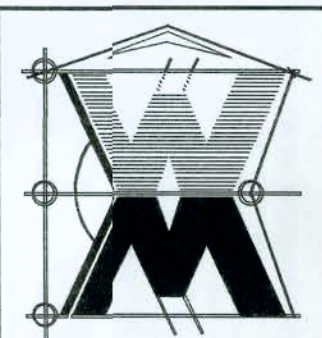
REVISIONS
January 14, 2008

SOTPLAN
ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

A NEW SPEC FOR
LOT 14, ROLLING MEADOWS
ADAM'S FRAMING & CONSTRUCTION
LAKE CITY, FLORIDA

©VILLIAM MYERS
DESIGN
P.O. BOX 1513
LAKE CITY, FL 32055
(386) 756-8406
williammyers.net



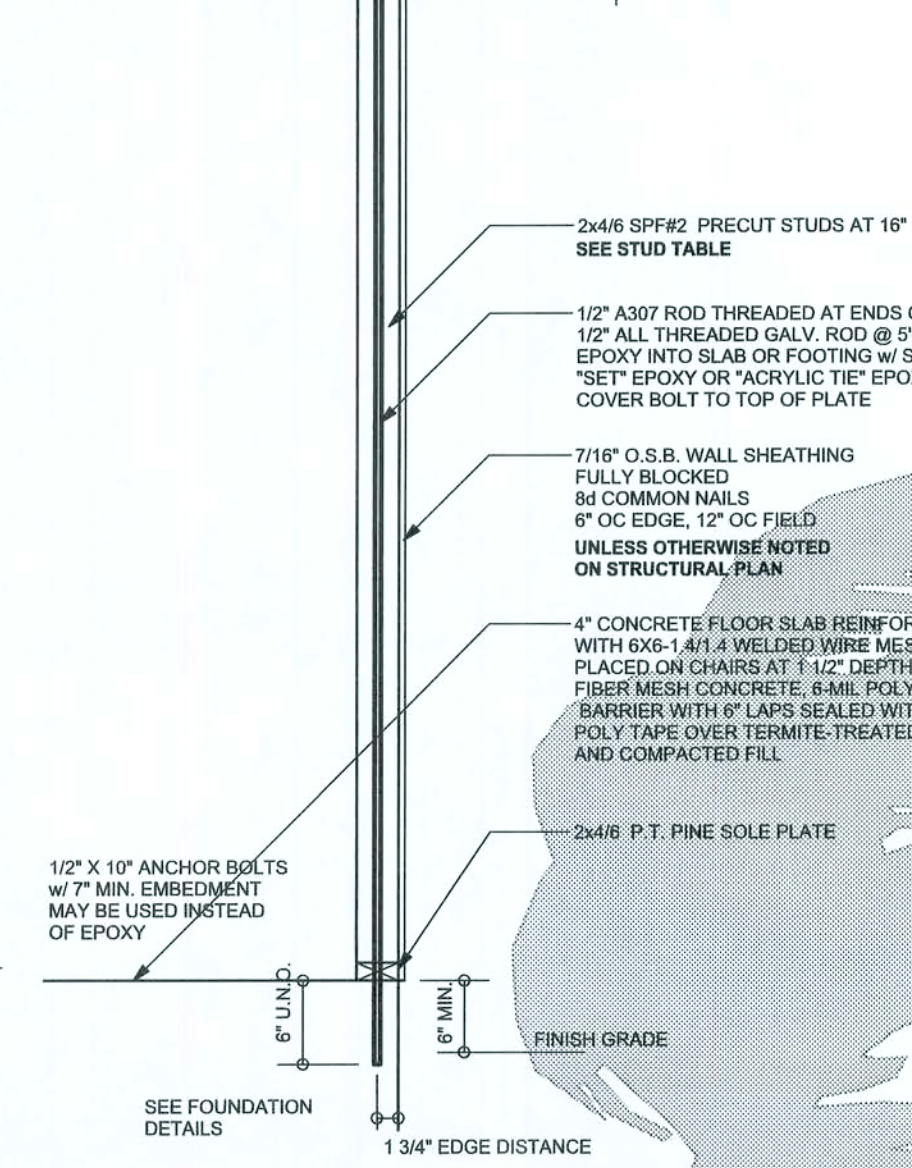
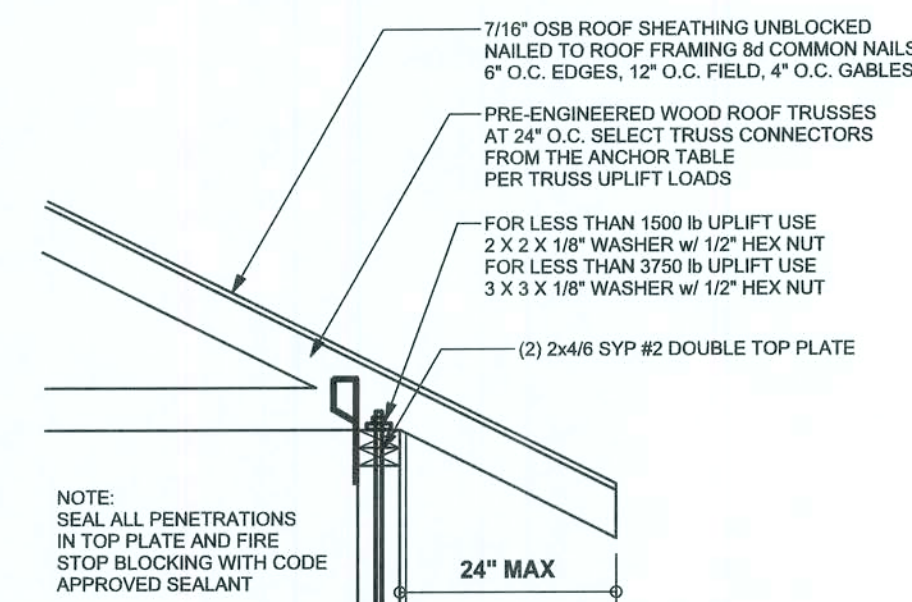
JOE NUMBER
080112

SHEET NUMBER

A.3

OF 3 SHEETS

W.M.C. Myers

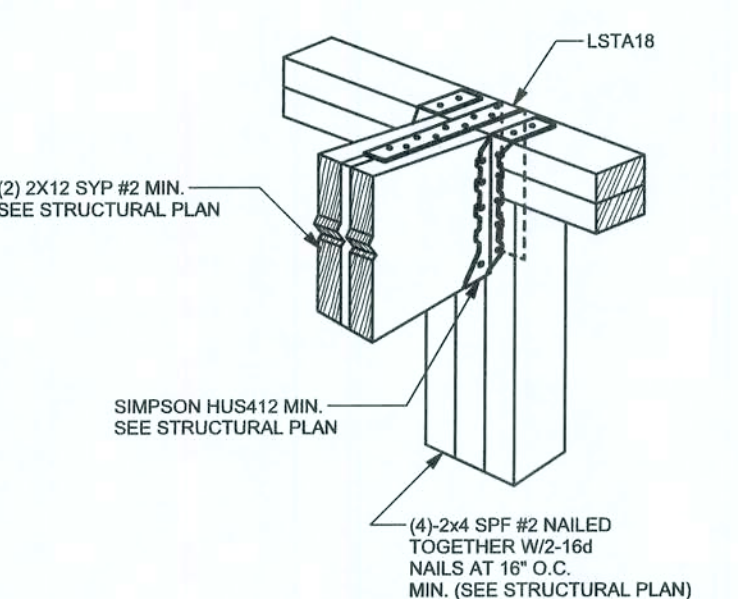


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

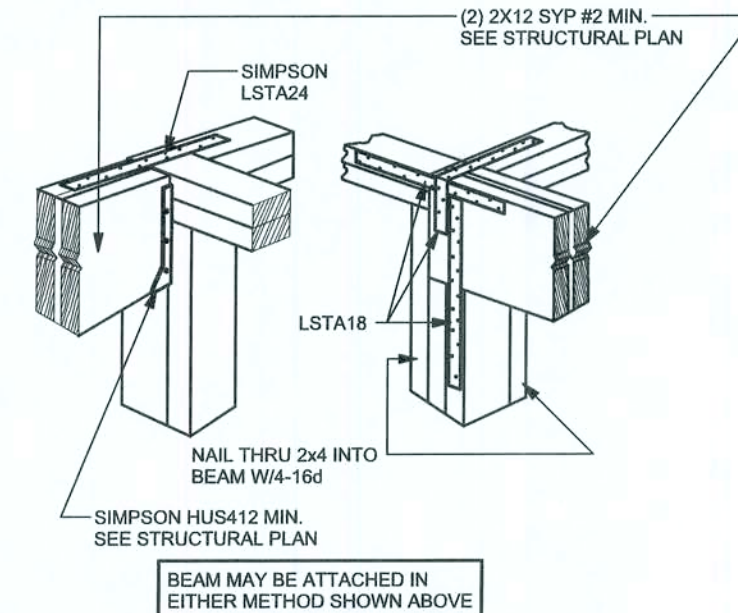
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B,
EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS
RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B
STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING
LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.
EXAMPLE 16" O.C. x 0.85 = 13" O.C.

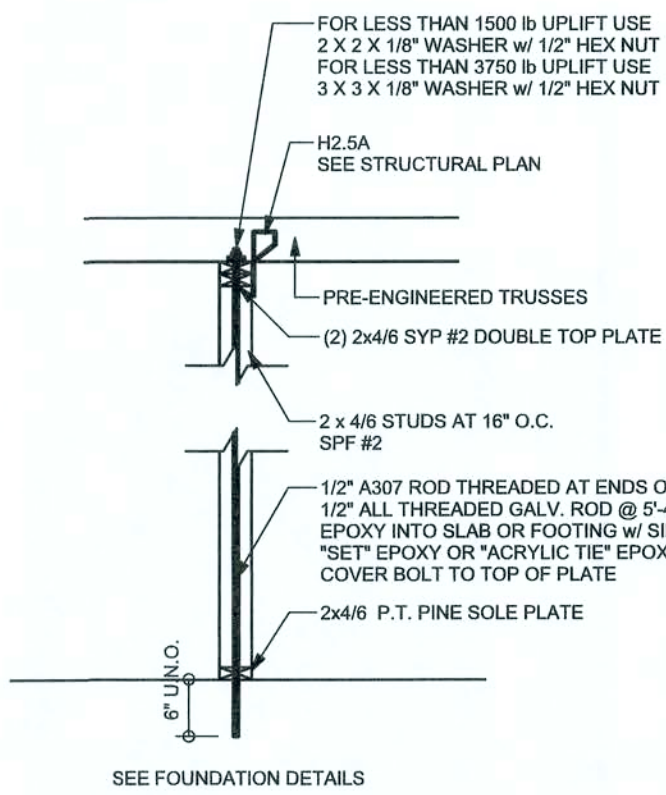


BEAM MID-WALL CONNECTION DETAIL
SCALE: N.T.S.

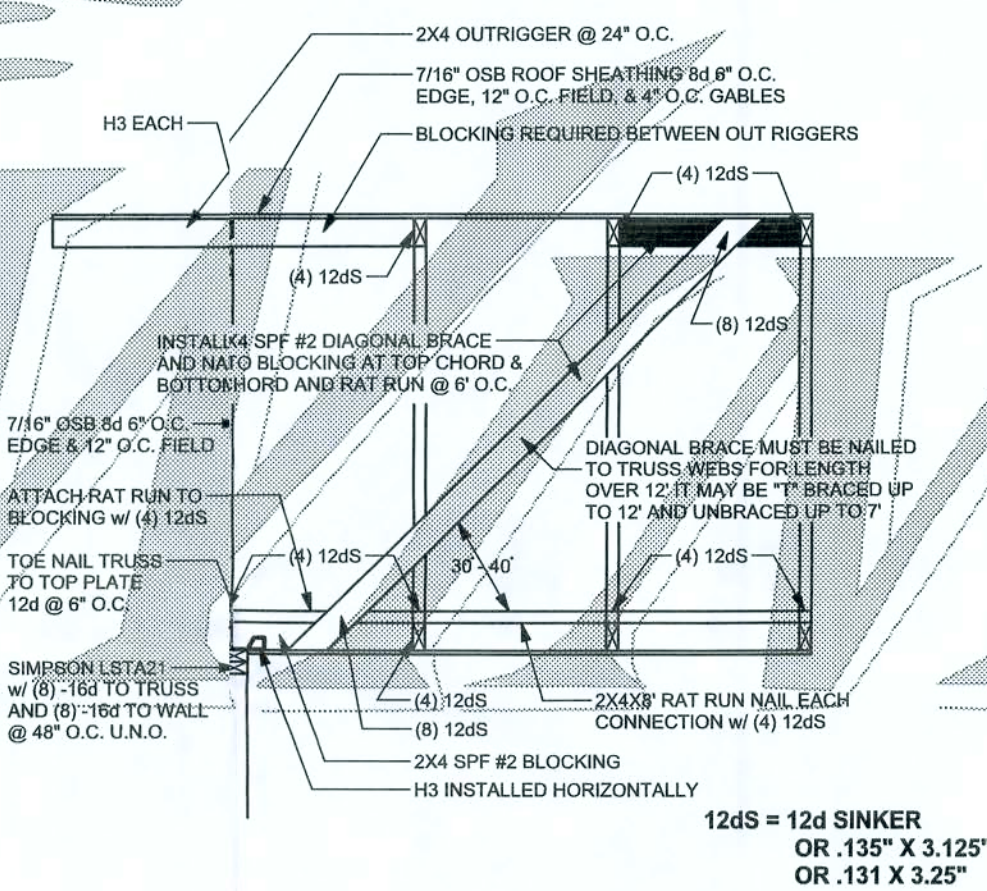


BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.

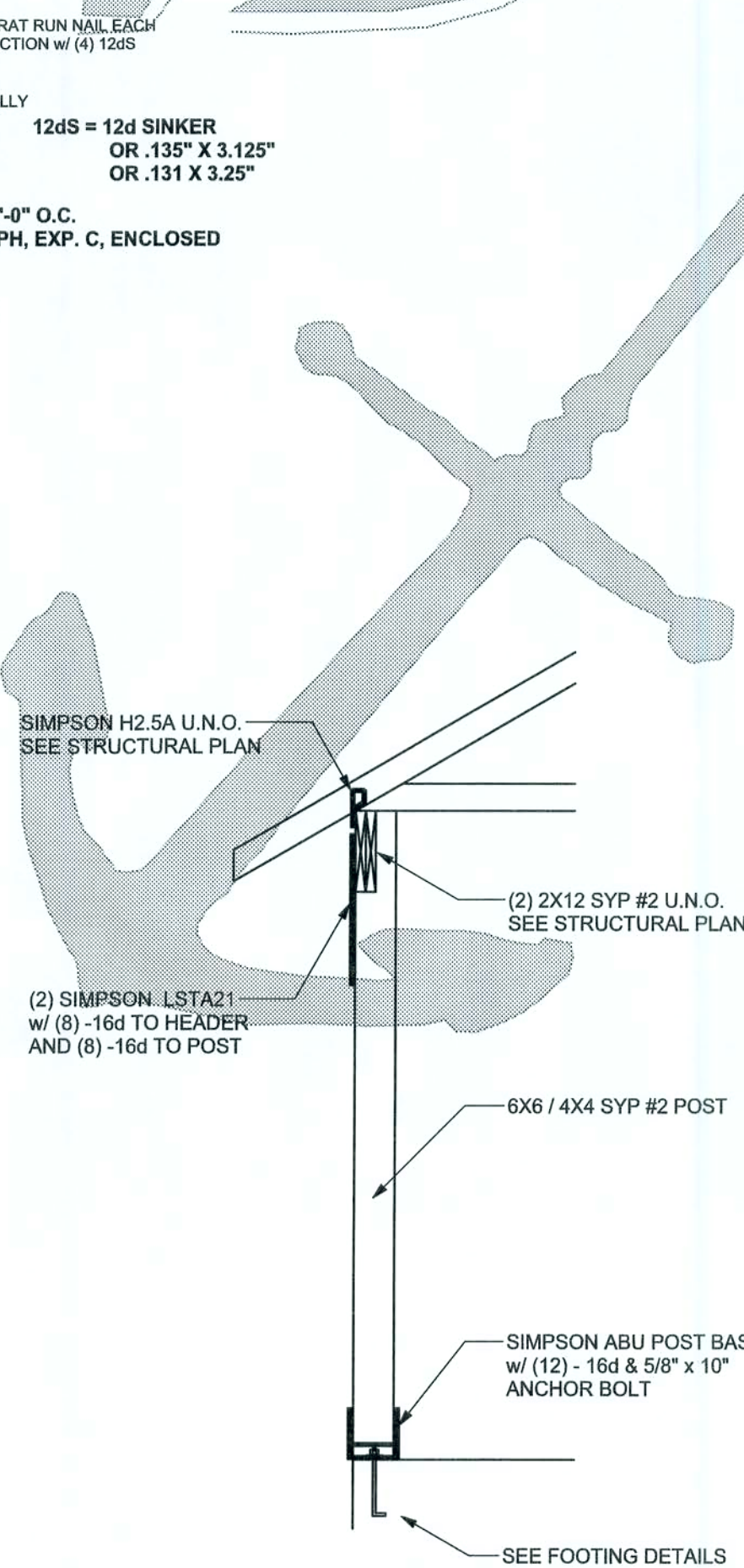
SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.



INTERIOR BEARING WALL
S.C.E. 1/2" = 1'-0"



ABLE BRACING DETAIL
SCALE: 1/2" = 1'-0"



4X4 / 6X6 PORCH POST DETAIL
SCALE: 1/2" = 1'-0"

OPTIONAL ALUMINUM PORCH POST & HEADER ANCHORS
SCALE: N.T.S.

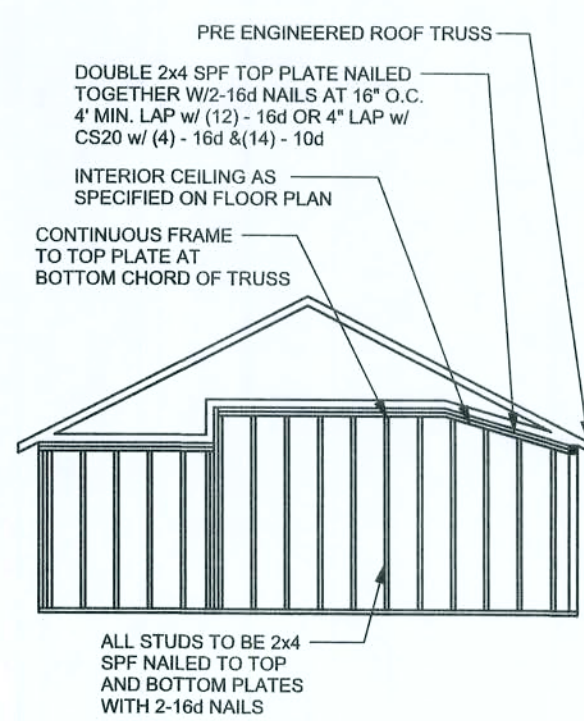
ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS
MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 890	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2400	2 - HTS24			
< 2050	< 1785	LGT2	14 - 16d	14 - 16d	
HEAVY ORDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MGT		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 8250	< 8250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	16-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	H32A	2-5/8" BOLTS		5/8" AB
< 4175	< 3685	HJT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	PAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

GRADE & SPECIES TABLE

	Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200 1.6
2x10	SYP #2	1050 1.6
2x12	SYP #2	975 1.6
GLB	24F-V3 SP	2400 1.8
LSL	TIMBERSTRAND	1700 1.7
LVL	MICROLAM	2900 2.0
PSL	PARALAM	2900 2.0



CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE
FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND
PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR
ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER
AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDERS
RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO
SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR
INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR
REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT
CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET
GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS
VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 80ksi, WELDED WIRE REINFORCEMENT FABRIC
(W1.4) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED
MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.
FIBER LENGTH 1 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD
PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER
TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN
ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH /
WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12". DO NOT
CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO
OWNER, OBSERVATION OF BUILDING STRUCTURE. CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS
BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI, ALL LAP SPICES 40" DB (25' FOR #5 BARS);
U.N.O. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi, U.N.O. SUPPLIER MAY SUPPLY AN
ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.
ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED,
APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES
STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6" O.C. PANEL EDGES, 12" O.C. INTERMEDIATE
MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" O.C. U.N.O.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS,
AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE
SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE
TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS INSTALLATION
INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO
LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH
3/4" BOLTS TO BE 2" x 2" x 9/64" WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", U.N.O.

NAILS: ALL NAILS ARE COMMON NAIL UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST
REPORTS AS HAVING EQUAL STRUCTURAL VALUES

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE
SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND
BACKSLOPE, WIND HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004
REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU
BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL
THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS
DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,
TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL
BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION
R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN
TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS
THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE
COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS
MANUFACTURER AND HAVE IT DESIGNED AND SEALED BY A DESIGN
PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED
LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO
REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF
SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL
BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF
DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT
RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE
TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES
RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED
TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;
MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT
ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%
SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1. BASIC WIND SPEED = 110 MPH

2. WIND EXPOSURE = B

3. WIND IMPORTANCE FACTOR = 1.0

4. BUILDING CATEGORY = II

5. ROOF ANGLE = 10-45 DEGREES

6. MEAN ROOF HEIGHT = <30 FT

7. INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8. COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	
1	19.9 - 21.8	18.1 - 18.1
2	19.9 - 25.5	18.1 - 21.8
2 On g	-40.6	-40.6
3 On g	19.9 - 25.5	18.1 - 21.8
4	19.9 - 25.5	18.1 - 21.8
5	21.8 - 29.1	18.1 - 22.6
Doors & Windows	21.8	-29.1
Worst Case (Zone 5, 10 ft ²)		
8x7 Garage Door	19.5	-22.9
16x7 Garage Door	18.5	-21.0

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12)

12 PSF (12:12 AND GREATER)

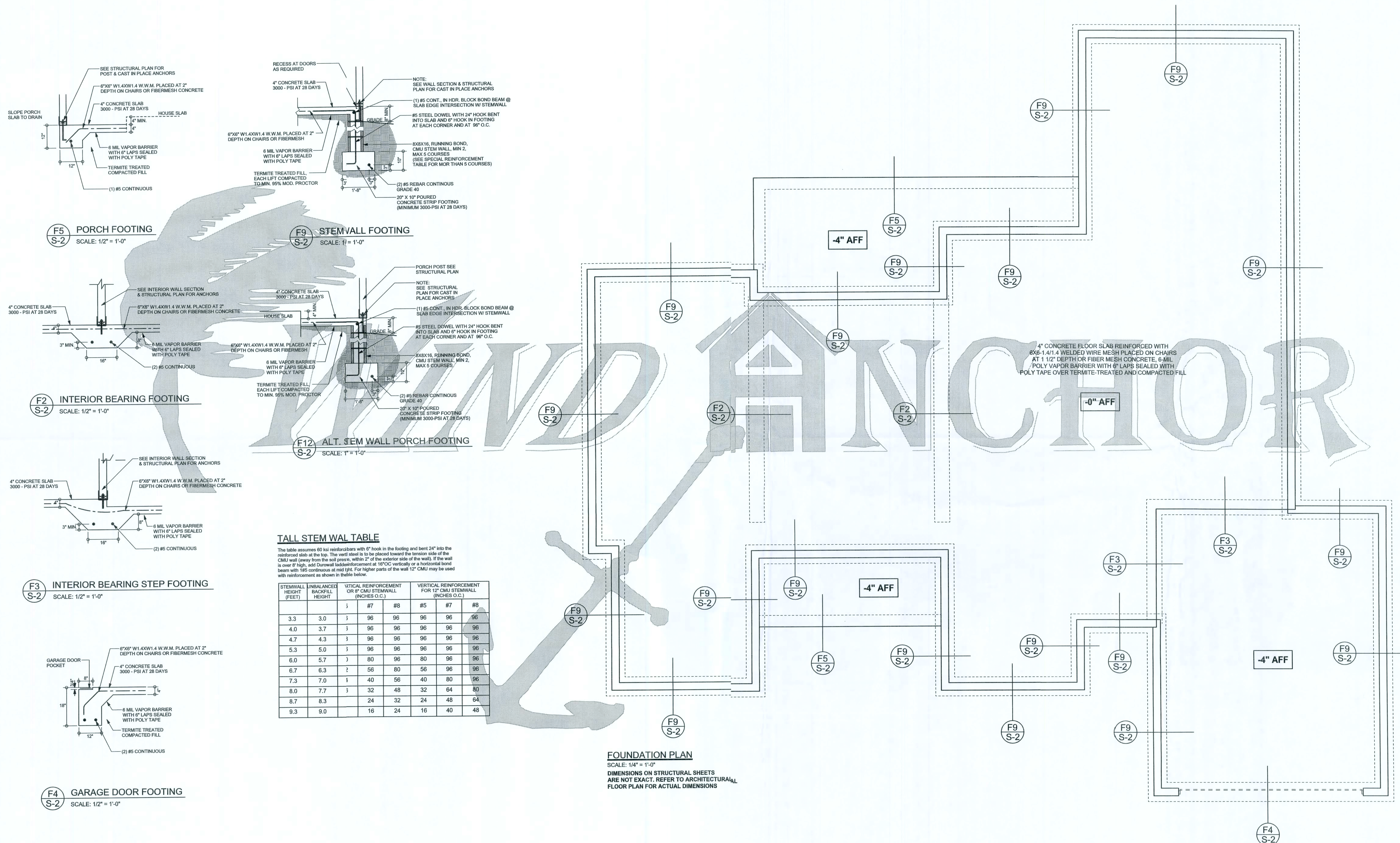
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

NO.	DESCRIPTION
1	
2	
3	
4	
5	
6	
7	
8	
9	</



WINDLOAD ENGINEER: Mark Disosway
P.E. No. 53915, P.B. 868, Lake City, FL
32056, 386-754-419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 1994, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MRK DISOSWAY
P.E. 53915

1/15/08

SEAL

Adan's Framing & Construction

Spec House
Lot 14
Rolling Meadows S/D

ADDRESS:
Lot 14 Rolling Meadows S/D
Columbia County, Florida

MarkDisosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
January 14, 2008

DRAWN BY: CHECKED BY:

FINALS DATE:
14 / Jan / 07

JOB NUMBER:
801117

DRAWING NUMBER

S-2

OF 3 SHEETS

OF 3 SHIFTS