

Julius Lee Engineering

RE: 315491 - Disosway Custom Residence

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: DAVID DISOSWAY Project Name: 315491 Model: CUSTOM
Lot/Block: 11 Subdivision: ROSE CREEK
Address:
City: COLUMIBA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: MARK D. DISOSWAY License #: 53915
Address: P.O. BOX 868
City: LAKE CITY, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
Wind Code: N/A Wind Speed: N/A mph Floor Load: 55.0 psf
Roof Load: N/A psf

This package includes 14 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date
1	I4119394	F1	10/2/09
2	I4119395	F10	10/2/09
3	I4119396	F11	10/2/09
4	I4119397	F12	10/2/09
5	I4119398	F2	10/2/09
6	I4119399	F3	10/2/09
7	I4119400	F4	10/2/09
8	I4119401	F5	10/2/09
9	I4119402	F6	10/2/09
10	I4119403	F6A	10/2/09
11	I4119404	F7	10/2/09
12	I4119405	F7A	10/2/09
13	I4119406	F8	10/2/09
14	I4119407	F9	10/2/09

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

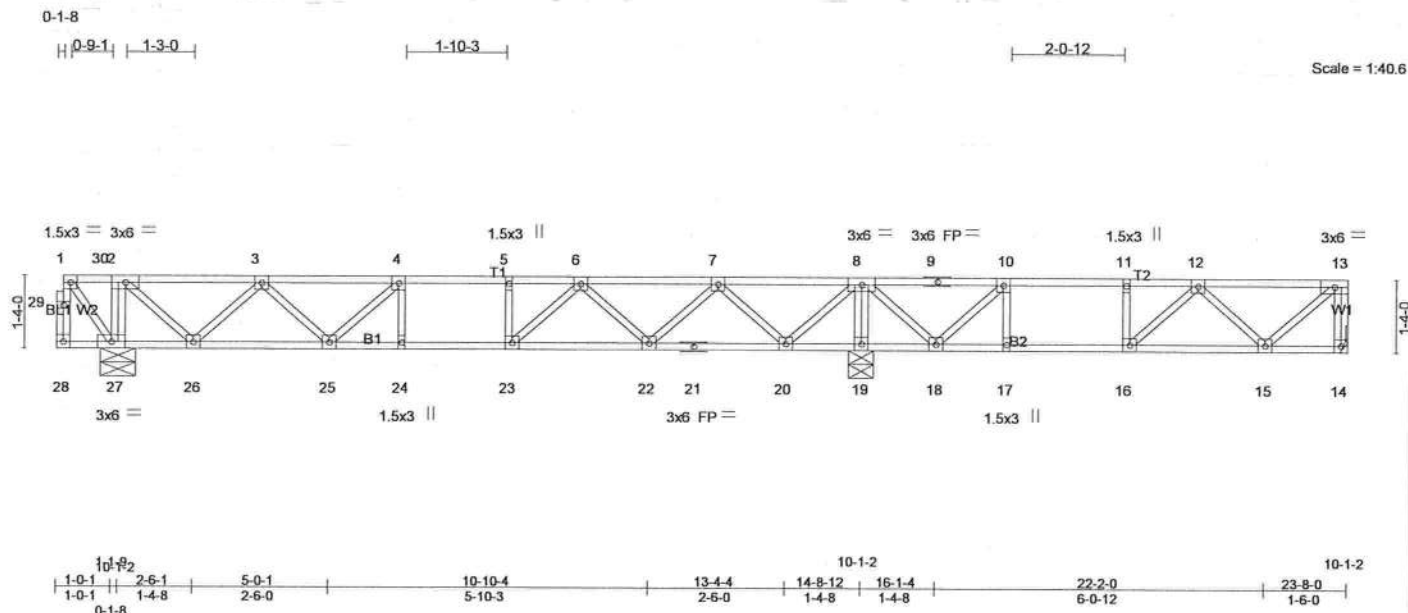
NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Job 315491	Truss F1	Truss Type FLOOR	Qty 1	Ply 1	Disosway Custom Residence Job Reference (optional)	I4119394
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Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase 1.00	TC 0.39	Vert(LL)	-0.06 22-23	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase 1.00	BC 0.40	Vert(TL)	-0.09 22-23	>999	240		
BCLL 0.0	Rep Stress Incr YES	WB 0.27	Horz(TL)	0.02 14	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)						
							Weight: 126 lb	

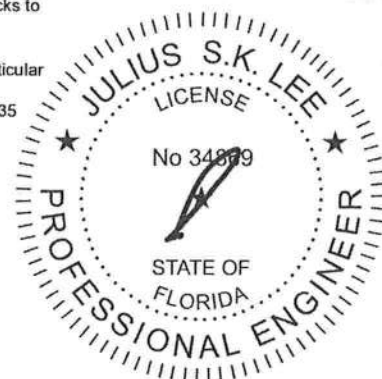
LUMBER	BRACING
TOP CHORD 4 X 2 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 4 X 2 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 4 X 2 SYP No.3	

REACTIONS (lb/size) 14=271/Mechanical, 27=537/0-7-12, 19=905/0-5-8
Max Grav 14=309(LC 4), 27=537(LC 2), 19=907(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 13-14=-311/0, 2-3=-459/0, 3-4=-1049/0, 4-5=-1256/0, 5-6=-1256/0, 6-7=-947/0, 7-8=-256/0, 10-11=-503/65, 11-12=-503/65, 12-13=-274/0
BOT CHORD 25-26=0/845, 24-25=0/1256, 23-24=0/1256, 22-23=0/1168, 21-22=0/702, 20-21=0/702, 19-20=-461/0, 18-19=-461/0, 17-18=-65/503, 16-17=-65/503, 15-16=0/490
WEBS 2-27=-525/0, 8-19=-870/0, 8-20=0/672, 2-26=0/585, 7-20=-631/0, 3-26=-538/0, 7-22=0/374, 3-25=0/284, 6-22=-350/0, 4-25=-304/0, 6-23=0/264, 13-15=0/364, 8-18=0/430, 12-15=-301/15, 10-18=-517/0

- NOTES** (8-10)
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x3 MT20 unless otherwise indicated.
 - 3) All bearings are assumed to be SYP No.2.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 7) CAUTION, Do not erect truss backwards.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - 10) Use Simpson THA422 to attach Truss to Carrying member

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315491	Truss F10	Truss Type FLOOR	Qty 1	Ply 1	Disosway Custom Residence	I4119395
					Job Reference (optional)	
Builders FrstSource, Lake City, FL 32055					7.140 s Aug 17 2009 MTEK Industries, Inc. Fri Oct 02 11:59:58 2009 Page 1	

0-1-8

1-3-0

1-10-0

0-1-8

Scale: 1/2"=1'

Plate Offsets (X,Y): [7-0-1-8,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.43	Vert(LL)	-0.13 10-11	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.73	Vert(TL)	-0.18 10-11	>883	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.37	Horz(TL)	0.03 8	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)						
								Weight: 72 lb	

LUMBER TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.2 WEBS 4 X 2 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS (lb/size) 15=741/0-5-2, 8=741/1-1-1

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 15-16=-738/0, 1-16=-737/0, 8-17=-734/0, 7-17=-733/0, 1-2=-725/0, 2-3=-1691/0, 3-4=-2057/0, 4-5=-2057/0, 5-6=-1696/0, 6-7=-724/0

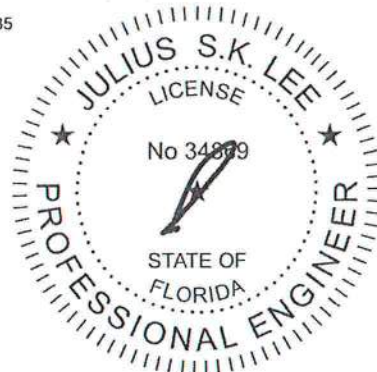
BOT CHORD 13-14=0/1353, 12-13=0/2057, 11-12=0/2057, 10-11=0/1987, 9-10=0/1361

WEBS 7-9=0/932, 1-14=0/934, 6-9=-887/0, 2-14=-873/0, 6-10=0/465, 2-13=0/480, 5-10=-405/0, 3-13=-582/0, 5-11=-109/355

NOTES (6-7)

- Unbalanced floor live loads have been considered for this design.
- All plates are 3x3 MT20 unless otherwise indicated.
- All bearings are assumed to be SYP No.2.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2,2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 315491	Truss F11	Truss Type FLOOR	Qty 1	Ply 1	Disosway Custom Residence Job Reference (optional) 7.140 s Aug 17 2009 Mitek Industries, Inc. Fri Oct 02 11:59:59 2009 Page 1	I4119396
Builders FrstSource, Lake City, FL 32055						

0-1-8

1-3-0 1-5-10

10-1-2

1-6-0 6-11-10 9-5-10 10-11-10

1-6-0 5-5-10 2-6-0 1-6-0

0-1-8
Scale = 1:18.8

15

10-1-2

Plate Offsets (X,Y): [6:0-1-8,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.40	Vert(LL)	-0.06 9-10	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.55	Vert(TL)	-0.09 9-10	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.28	Horz(TL)	0.01 7	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)						
								Weight: 59 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2

BOT CHORD 4 X 2 SYP No.2

WEBS 4 X 2 SYP No.3

REACTIONS (lb/size) 13=583/0-6-1, 7=583/1-1-1

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 13-14=-572/0, 1-14=-571/0, 7-15=-577/0, 6-15=-576/0, 1-2=-532/0, 2-3=-1275/0, 3-4=-1275/0, 4-5=-1181/0, 5-6=-549/0

BOT CHORD 11-12=0/1013, 10-11=0/1275, 9-10=0/1275, 8-9=0/1028

WEBS 6-8=0/706, 1-12=0/684, 5-8=-665/0, 2-12=-669/0, 5-9=0/252, 2-11=0/470

NOTES (5-6)

- Unbalanced floor live loads have been considered for this design.
- All bearings are assumed to be SYP No.2.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

October 2, 2009

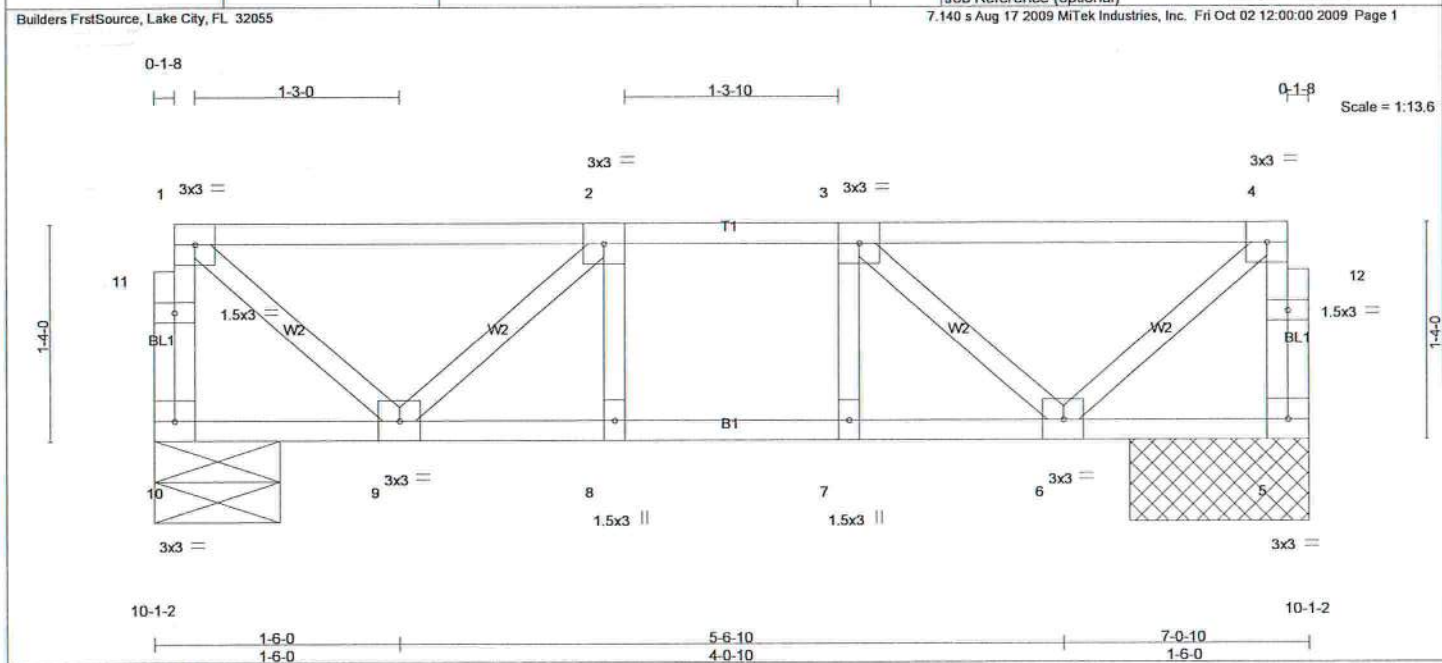


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315491	Truss F12	Truss Type FLOOR	Qty 1	Ply 1	Disosway Custom Residence	14119397
Builders FrstSource, Lake City, FL 32055					Job Reference (optional) 7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 12:00:00 2009 Page 1	



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 40.0	2-0-0	TC 0.33	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.00	BC 0.27	Vert(LL) -0.02 6-7 >999 360		
BCLL 0.0	Lumber Increase 1.00	WB 0.15	Vert(TL) -0.02 6-7 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 5 n/a n/a		
	Code FBC2007/TPI2002			Weight: 40 lb	

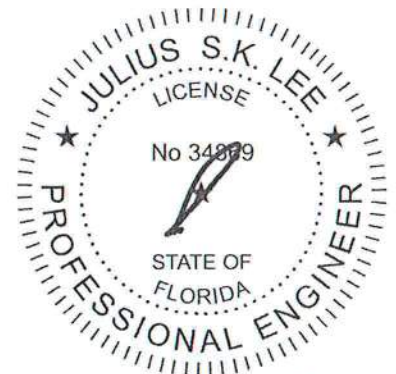
LUMBER	BRACING
TOP CHORD 4 X 2 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 4 X 2 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 4 X 2 SYP No.3	

REACTIONS (lb/size) 10=368/0-9-5, 5=368/1-1-1

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 10-11=-362/0, 1-11=-362/0, 5-12=-362/0, 4-12=-362/0, 1-2=-291/0, 2-3=-546/0, 3-4=-291/0
BOT CHORD 8-9=0/546, 7-8=0/546, 6-7=0/546
WEBS 4-6=0/370, 1-9=0/370, 3-6=347/0, 2-9=347/0

- NOTES** (5-6)
- Unbalanced floor live loads have been considered for this design.
 - All bearings are assumed to be SYP No.2.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard

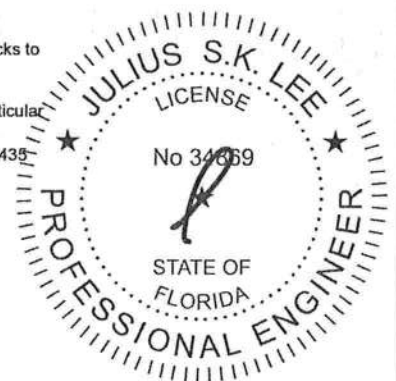
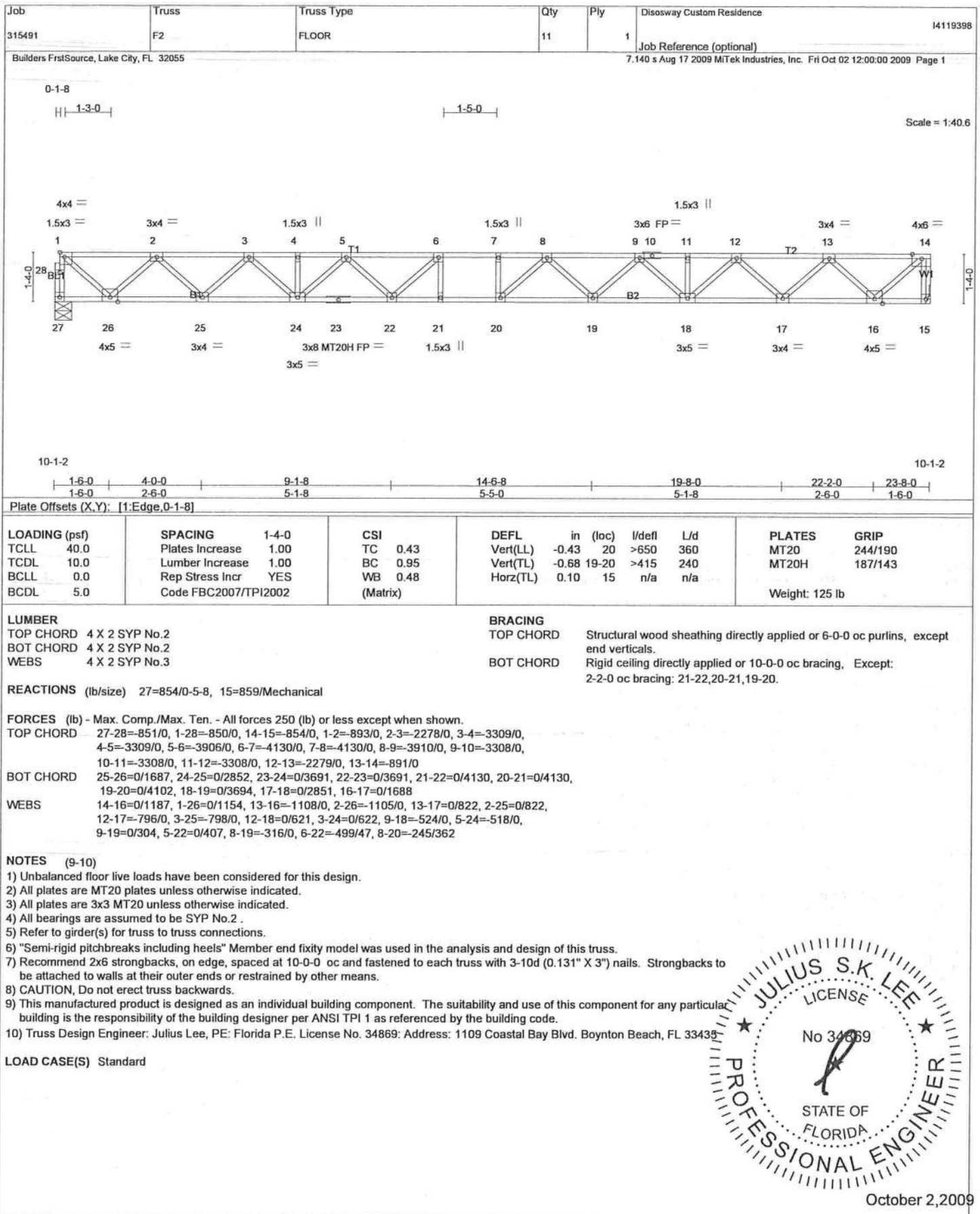


October 2, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435



October 2,2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Orazio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 315491	Truss F3	Truss Type FLOOR	Qty 6	Ply 1	Disosway Custom Residence	I4119399
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Builders FirstSource, Lake City, FL 32055

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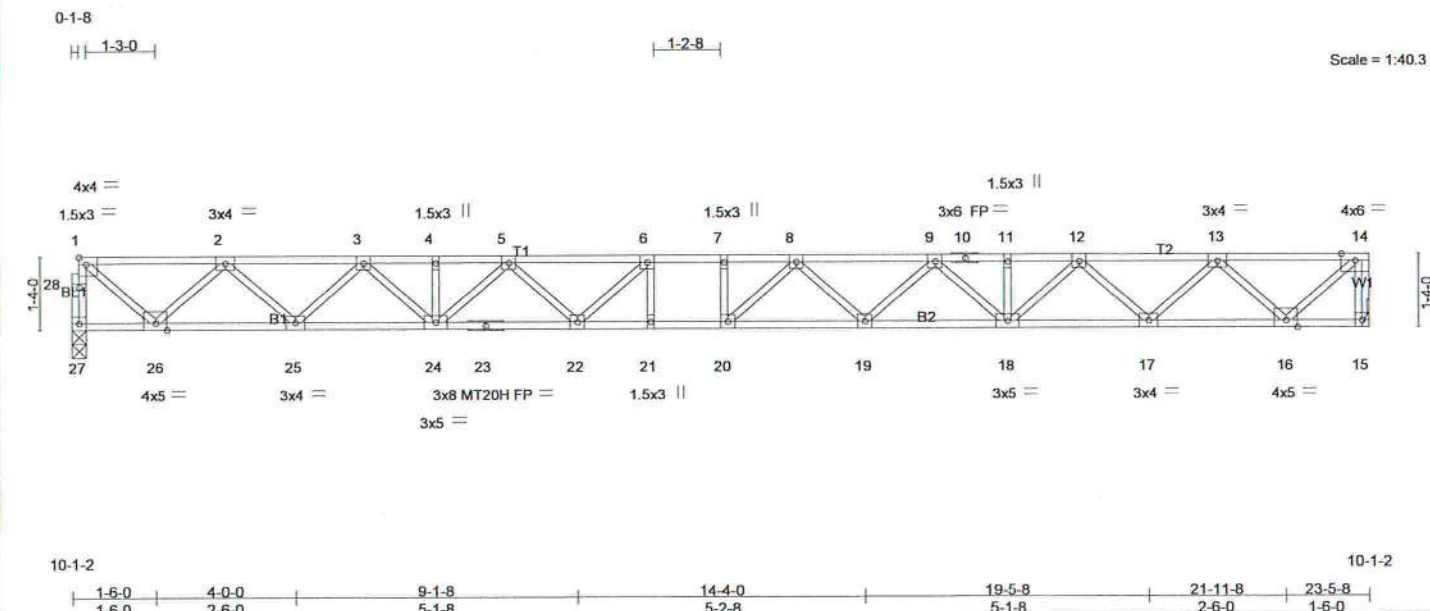


Plate Offsets (X,Y): [1:Edge,0-1-8]

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.40	Vert(LL)	-0.42	20	>670	360	MT20 244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.92	Vert(TL)	-0.65	19-20	>428	240	MT20H 187/143
BCLL 0.0	Rep Stress Incr	YES	WB 0.47	Horz(TL)	0.10	15	n/a	n/a	
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)						Weight: 124 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 20-21,19-20.

REACTIONS (lb/size) 27=847/0-3-0, 15=851/Mechanical

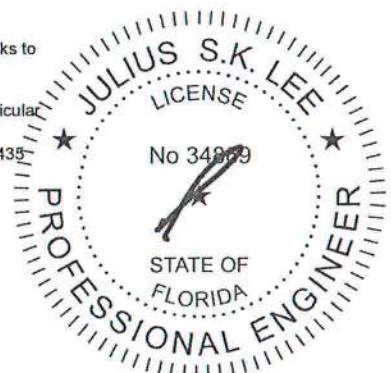
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 27-28=843/0, 1-28=842/0, 14-15=846/0, 1-2=884/0, 2-3=2254/0, 3-4=3268/0, 4-5=3268/0, 5-6=3849/0, 6-7=4059/0, 7-8=4059/0, 8-9=3852/0, 9-10=3267/0, 10-11=3267/0, 11-12=3267/0, 12-13=2255/0, 13-14=883/0
BOT CHORD 25-26=0/1670, 24-25=0/2820, 23-24=0/3643, 22-23=0/3643, 21-22=0/4059, 20-21=0/4059, 19-20=0/4038, 18-19=0/3645, 17-18=0/2819, 16-17=0/1672
WEBS 14-16=0/1175, 1-26=0/1142, 13-16=1097/0, 2-26=1094/0, 13-17=0/811, 2-25=0/811, 12-17=785/0, 3-25=787/0, 12-18=0/610, 3-24=0/610, 9-18=512/0, 5-24=509/0, 9-19=0/297, 5-22=0/391, 8-19=304/0, 6-22=470/51, 8-20=244/339

NOTES (9-10)

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x3 MT20 unless otherwise indicated.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 8) CAUTION, Do not erect truss backwards.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

Job 315491	Truss F4	Truss Type FLOOR	Qty 2	Ply 1	Disosway Custom Residence Job Reference (optional)	I4119400
Builders FrstSource, Lake City, FL 32055			7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 12:00:02 2009 Page 1			

0-1-8

Scale = 1:40.3

LOADING (psf)	SPACING 1-4-0	CSI	DEFL in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase 1.00	TC 0.40	Vert(LL) -0.42 20	>670	360	MT20	244/190
TCDL 10.0	Lumber Increase 1.00	BC 0.92	Vert(TL) -0.65 19-20	>428	240	MT20H	187/143
BCLL 0.0	Rep Stress Incr YES	WB 0.47	Horz(TL) 0.10 15	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)					
						Weight: 124 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2

BOT CHORD 4 X 2 SYP No.2

WEBS 4 X 2 SYP No.3

REACTIONS (lb/size) 27=847/0-3-0, 15=851/Mechanical

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 27-28=843/0, 1-28=842/0, 14-15=846/0, 1-2=884/0, 2-3=2254/0, 3-4=3268/0, 4-5=3268/0, 5-6=3849/0, 6-7=4059/0, 7-8=4059/0, 8-9=3852/0, 9-10=3267/0, 10-11=3267/0, 11-12=3267/0, 12-13=2255/0, 13-14=883/0

BOT CHORD 25-26=0/1670, 24-25=0/2820, 23-24=0/3643, 22-23=0/3643, 21-22=0/4059, 20-21=0/4059, 19-20=0/4038, 18-19=0/3645, 17-18=0/2819, 16-17=0/1672

WEBS 14-16=0/1175, 1-26=0/1142, 13-16=1097/0, 2-26=1094/0, 13-17=0/811, 2-25=0/811, 12-17=785/0, 3-25=787/0, 12-18=0/610, 3-24=0/610, 9-18=512/0, 5-24=509/0, 9-19=0/297, 5-22=0/391, 8-19=304/0, 6-22=470/51, 8-20=244/339

NOTES (9-10)

- Unbalanced floor live loads have been considered for this design.
- All plates are MT20 plates unless otherwise indicated.
- All plates are 3x3 MT20 unless otherwise indicated.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- CAUTION, Do not erect truss backwards.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 20-21, 19-20.

October 2, 2009

A circular professional engineer seal for Julius S.K. Lee. The outer ring contains the text "JULIUS S.K. LEE" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by two stars. Inside this ring, the word "LICENSE" is at the top and "STATE OF FLORIDA" is at the bottom, also separated by two stars. In the center, the license number "No 34269" is displayed. A stylized signature is written over the license number.

October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BC311 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, Fl. 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119402
315491	F6	FLOOR	3	1	Job Reference (optional)	
Builders FrstSource, Lake City, FL 32055			7,140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 12:00:04 2009 Page 1			

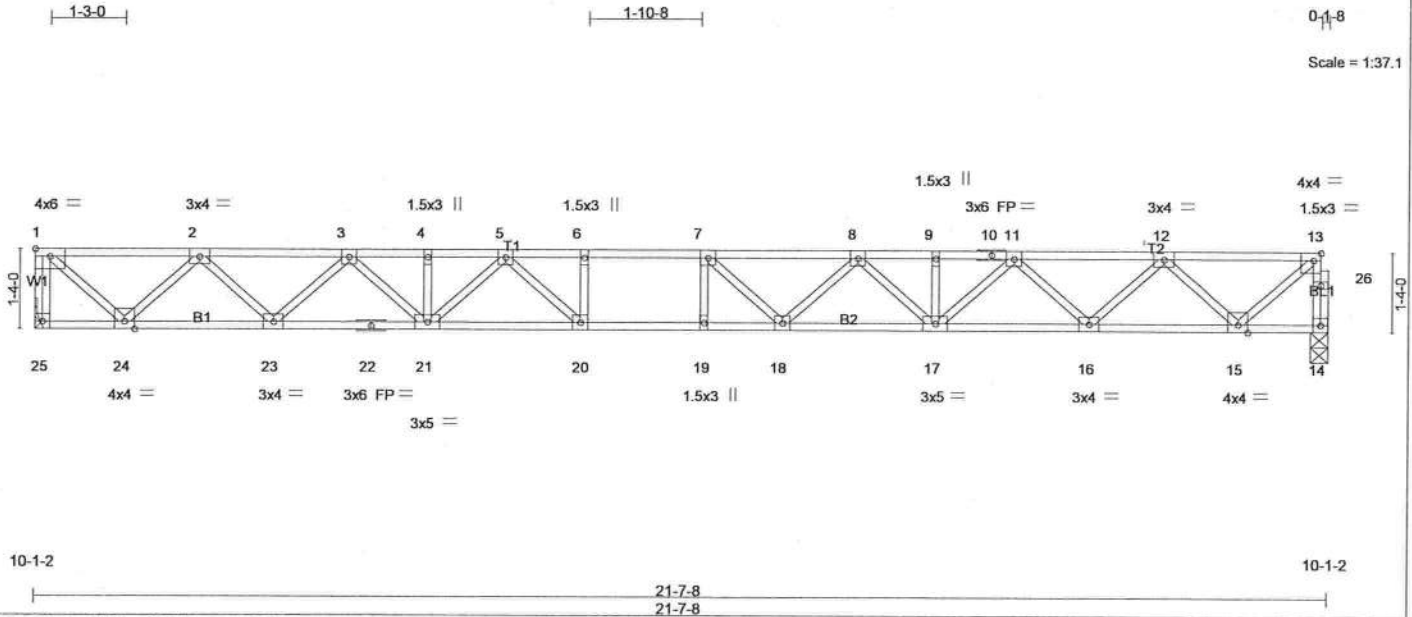


Plate Offsets (X,Y): [1:Edge,0-1-8], [7:0-0-0,0-0-0], [8:0-0-0,0-0-0], [9:0-0-0,0-0-0], [11:0-0-0,0-0-0], [12:0-0-0,0-0-0], [13:0-1-8,Edge], [14:0-0-0,0-0-0], [16:0-0-0,0-0-0], [17:0-0-0,0-0-0], [18:0-0-0,0-0-0], [19:0-0-0,0-0-0], [22:0-0-0,0-0-0], [26:0-0-0,0-0-0]

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.52	Vert(LL)	-0.32	19	>805	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.98	Vert(TL)	-0.50	19	>517	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.43	Horz(TL)	0.08	14	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)							
										Weight: 114 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 19-20,18-19.

REACTIONS (lb/size) 25=784/Mechanical, 14=780/0-3-8

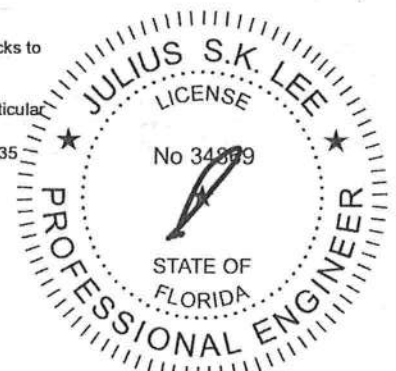
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-25=779/0, 14-26=776/0, 13-26=775/0, 1-2=806/0, 2-3=2041/0, 3-4=2912/0, 4-5=2912/0, 5-6=3432/0, 6-7=3432/0, 7-8=3348/0, 8-9=2908/0, 9-10=2908/0, 10-11=2908/0, 11-12=2040/0, 12-13=808/0
BOT CHORD 23-24=0/1527, 22-23=0/2532, 21-22=0/2532, 20-21=0/3199, 19-20=0/3432, 18-19=0/3432, 17-18=0/3224, 16-17=0/2535, 15-16=0/1525
WEBS 13-15=0/1043, 1-24=0/1073, 12-15=998/0, 2-24=1002/0, 12-16=0/716, 2-23=0/714, 11-16=690/0, 3-23=684/0, 11-17=0/506, 3-21=0/516, 8-17=429/0, 5-21=392/0, 8-18=1/310, 5-20=30/538, 7-18=368/148

NOTES (8-10)

- Unbalanced floor live loads have been considered for this design.
- All plates are 3x3 MT20 unless otherwise indicated.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- CAUTION, Do not erect truss backwards.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson THA422 to attach Truss to Carrying member

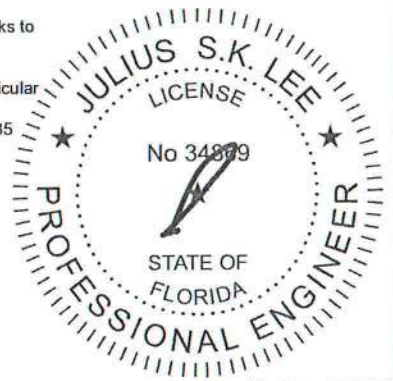
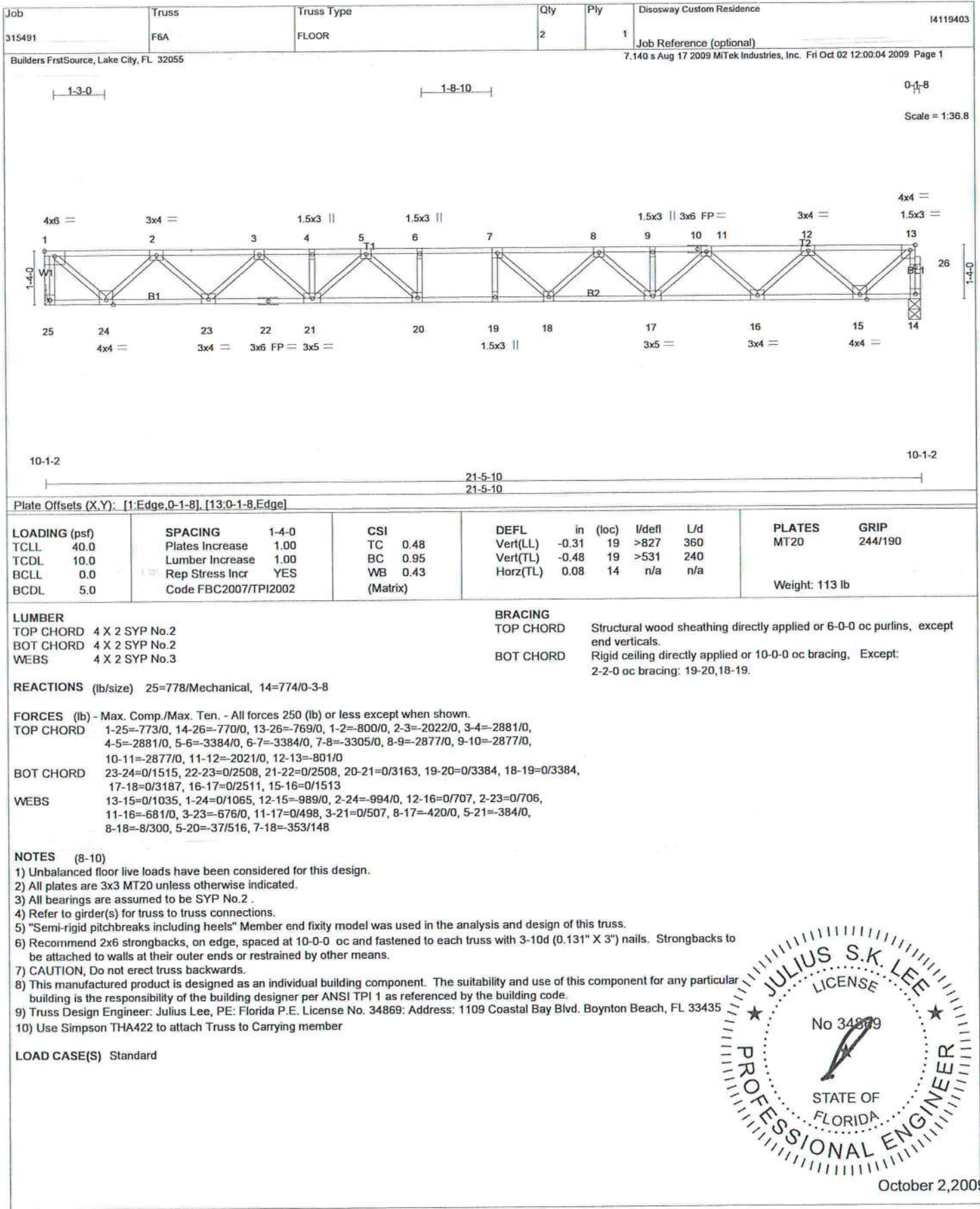
LOAD CASE(S) Standard



October 2, 2009

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435



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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

A circular professional engineer seal for Julius S.K. Lee. The outer ring contains the text "JULIUS S.K. LEE" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by two stars. Inside this ring, the word "LICENSE" is at the top and "STATE OF FLORIDA" is at the bottom, also separated by two stars. In the center, the license number "No 34669" is displayed above a stylized signature.

October 2, 2009

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315491	Truss F7A	Truss Type FLOOR	Qty 8	Ply 1	Disosway Custom Residence Job Reference (optional)	I4119405
Builders FrstSource, Lake City, FL 32055			7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 12:00:05 2009 Page 1			

Plate Offsets (X,Y): [1:Edge,0-1-8], [8:0-1-8,Edge]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 40.0	2-0-0	TC 0.43	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.00	BC 0.96	Vert(LL) -0.20 13-14 >999 360		
BCLL 0.0	Lumber Increase 1.00	WB 0.51	Vert(TL) -0.31 13-14 >671 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.06 9 n/a n/a		
	Code FBC2007/TPI2002			Weight: 91 lb	

LUMBER TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.2 WEBS 4 X 2 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
--	--

REACTIONS (lb/size) 18=949/Mechanical, 9=943/0-3-8

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-18=-943/0, 9-19=-938/0, 8-19=-937/0, 1-2=-952/0, 2-3=-2342/0, 3-4=-3121/0, 4-5=-3373/0, 5-6=-3121/0, 6-7=-2341/0, 7-8=-953/0

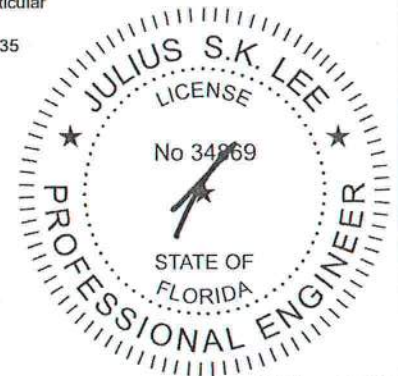
BOT CHORD 16-17=0/1796, 15-16=0/2861, 14-15=0/3373, 13-14=0/3373, 12-13=0/3373, 11-12=0/2861, 10-11=0/1794

WEBS 1-17=0/1267, 2-17=-1174/0, 2-16=0/759, 3-16=-722/0, 3-15=0/451, 4-15=-560/12, 8-10=0/1230, 7-10=-1169/0, 7-11=0/761, 6-11=-723/0, 6-12=0/451, 5-12=-560/12

NOTES (7-9)

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All bearings are assumed to be SYP No.2 .
- 3) Refer to girder(s) for truss to truss connections.
- 4) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 9) Use Simpson THA422 to attach Truss to Carrying member

LOAD CASE(S) Standard



October 2,2009

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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119406
315491	F8	FLOOR	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 12:00:06 2009 Page 1

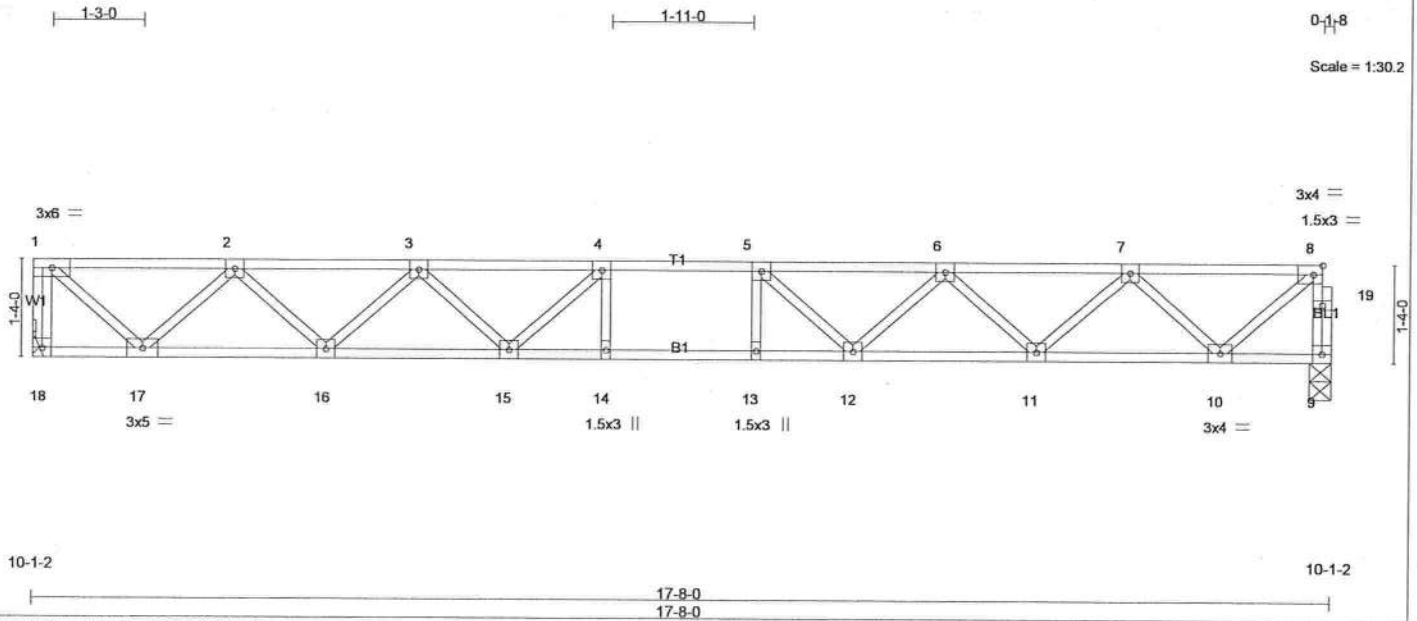


Plate Offsets (X,Y): [8-0-1-8,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.27	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.00	BC 0.66	Vert(LL) -0.14 13-14 >999 360		
BCLL 0.0	Lumber Increase 1.00	WB 0.34	Vert(TL) -0.21 13-14 >980 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.04 9 n/a n/a		
	Code FBC2007/TPI2002				
					Weight: 91 lb

LUMBER
 TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 18=639/Mechanical, 9=634/0-3-8

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-18=-634/0, 9-19=-631/0, 8-19=-630/0, 1-2=-641/0, 2-3=-1579/0, 3-4=-2111/0, 4-5=-2286/0, 5-6=-2111/0, 6-7=-1579/0, 7-8=-642/0

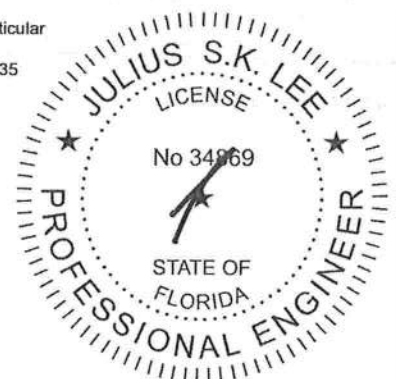
BOT CHORD 16-17=0/1210, 15-16=0/1931, 14-15=0/2286, 13-14=0/2286, 12-13=0/2286, 11-12=0/1931, 10-11=0/1208

WEBS 1-17=0/853, 2-17=-791/0, 2-16=0/514, 3-16=-489/0, 3-15=0/311, 4-15=-389/3, 8-10=0/828, 7-10=-788/0, 7-11=0/515, 6-11=-490/0, 6-12=0/311, 5-12=-389/3

NOTES (8-9)

- Unbalanced floor live loads have been considered for this design.
- All plates are 3x3 MT20 unless otherwise indicated.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- CAUTION, Do not erect truss backwards.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 315491	Truss F9	Truss Type FLOOR	Qty 1	Ply 1	Disosway Custom Residence	I4119407
					Job Reference (optional)	
Builders FrstSource, Lake City, FL 32055					7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 12:00:08 2009 Page 1	

1-3-0 1-3-10

0-1-8
Scale = 1:26.9

Plate Offsets (X,Y): [1-Edge,0-1-8], [8-0-1-8,Edge]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 40.0	Plates Increase	1.00	TC 0.42	Vert(LL)	-0.15 12-13	>999	360
TCDL 10.0	Lumber Increase	1.00	BC 0.85	Vert(TL)	-0.23 12-13	>805	240
BCLL 0.0	Rep Stress Incr	YES	WB 0.45	Horz(TL)	0.05 9	n/a	n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)				
				PLATES		GRIP	
				MT20		244/190	
				Weight: 83 lb			

LUMBER TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.2 WEBS 4 X 2 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
--	---

REACTIONS (lb/size) 17=855/Mechanical, 9=849/0-3-8

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-17=-851/0, 9-18=-845/0, 8-18=-844/0, 1-2=-850/0, 2-3=-2028/0, 3-4=-2723/0, 4-5=-2723/0, 5-6=-2626/0, 6-7=-2042/0, 7-8=-847/0

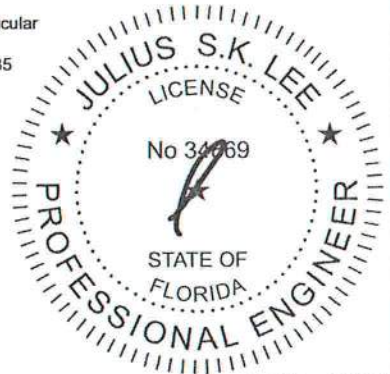
BOT CHORD 15-16=0/1596, 14-15=0/2452, 13-14=0/2723, 12-13=0/2723, 11-12=0/2473, 10-11=0/1589

WEBS 8-10=0/1092, 1-16=0/1132, 7-10=-1032/0, 2-16=-1037/0, 7-11=0/630, 2-15=0/601, 6-11=-599/0, 3-15=-589/0, 6-12=0/322, 3-14=0/559, 5-12=-348/107

NOTES (8-9)

- Unbalanced floor live loads have been considered for this design.
- All plates are 3x3 MT20 unless otherwise indicated.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- CAUTION, Do not erect truss backlogs.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



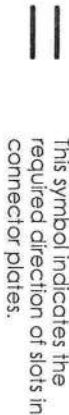
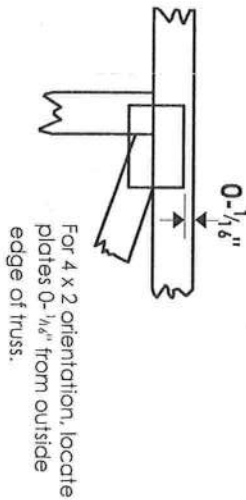
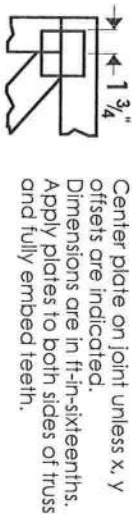
October 2,2009

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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



*Plate location details available in MTEK 20/20 software or upon request.

PLATE SIZE

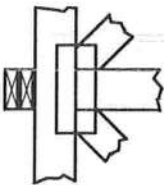
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



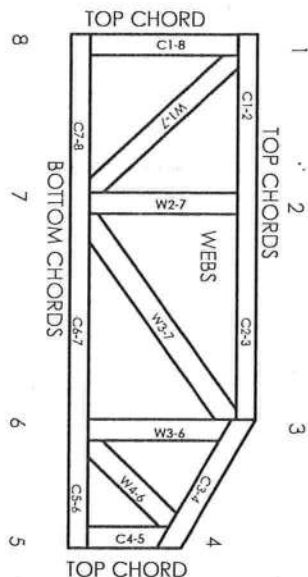
BEARING



Industry Standards:

ANSI/FP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear lightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/FP11.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/FP11.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/FP11 Quality Criteria.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

STEPPED DOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
BOT CHORD 2X4 SO. PINE #2 or Better
WEBS 2X4 SO. PINE #3 or Better

120 MPH MAX

Setback 7' or Less

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE

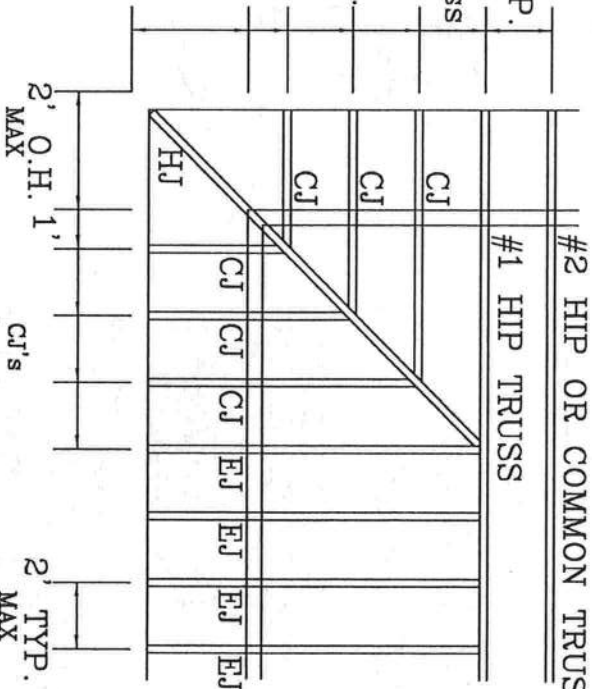
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

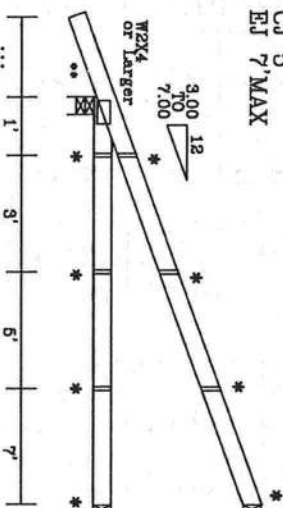
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

#2 HIP OR COMMON TRUSS

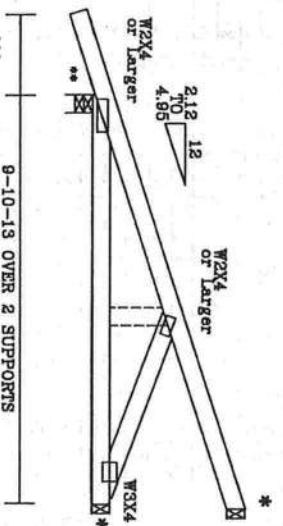
#1 HIP TRUSS



CJ 1' ALL HEELS TO BE STANDEAR WITH NO CANTILEVER
CJ 3'
CJ 5'
EJ 7" MAX



HJ ALL HEELS TO BE STANDEAR WITH NO CANTILEVER

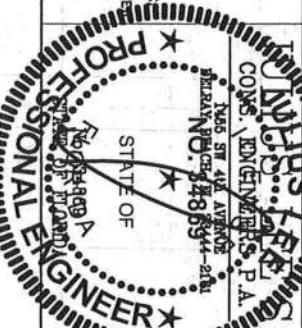


* (3) 16d TOENAILS
** SEE EOR FOR TIE DOWN

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET
SETBACK
7'0" MAX

REMARKS: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-63 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 600 HANCOCK ST., SUITE 200, WILMINGTON, DE 19801. THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO FOLLOW THIS DESIGN, OR ANY OTHER PROVISIONS OF THIS DESIGN. INSTALLATION SPEC BY APRAV AND TPE. ALPINE CONNECTOR PLATES ARE MADE OF 80/18/16GA. C/A/S/NO ASH 4653 GRADE 40/60 C/A/S/NO GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER ANEX AS OF TPI 1-2002 SET 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SEAL FOR THE TRUSS COMPONENT DESIGN SHOWN. THE DESIGN AND CONSTRUCTION OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANXI/TPI 1 SEC. 2.

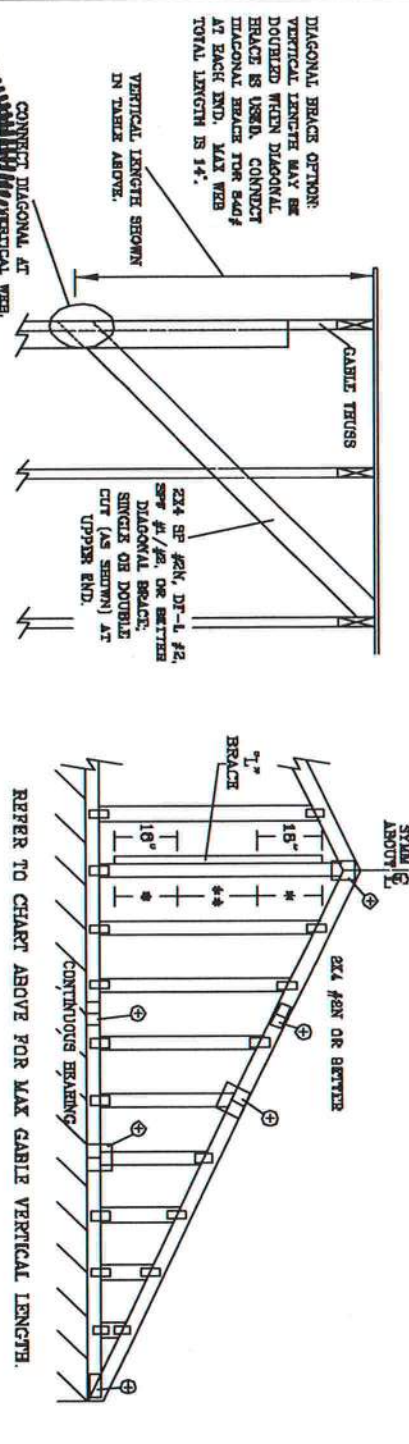


ITEM	QTY	UNIT	PRICE	TOTAL
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2	20	MAX PSF		
3	20	MAX PSF		
4	20	MAX PSF		
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97	20	MAX PSF		
98	20	MAX PSF		
99	20	MAX PSF		
100	20	MAX PSF		

REVIEWED
By Julius Lee at 10:52 am, Jun 27, 2008

ASCE 7-02: 130 MPH WIND SPEED, 16' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

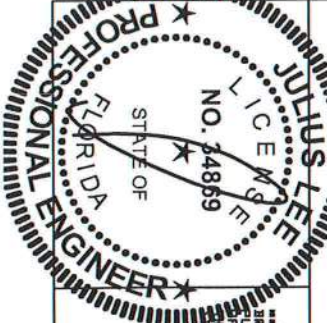
MAX GABLE VERTICAL LENGTH		BRACE		NO		(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X8 "L" BRACE *	
CABLE VERTICAL SPACING	SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"		
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"		
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"		
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 11"	8' 11"	11' 8"	11' 8"		
16" O.C.	SPF	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"		
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"		
		#3	3' 6"	5' 0"	6' 0"	6' 6"	6' 6"	8' 3"	8' 3"	10' 4"	10' 4"	12' 11"	13' 7"		
		STUD	3' 6"	5' 0"	6' 0"	6' 6"	6' 6"	8' 3"	8' 3"	10' 3"	10' 3"	12' 11"	13' 7"		
24" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"		
		#3	3' 8"	6' 0"	6' 0"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"		
		STUD	3' 8"	6' 0"	6' 0"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"		
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	9' 10"	9' 10"	11' 8"	11' 8"		



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS L/240.	
PROVIDE VERTICAL CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TO DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.	
ATTACH EACH "L" BRACE WITH 104 NAILS.	
* FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.	
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO. SPICES
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT	2X4
LESS THAN 11' 0"	2X4
GREATER THAN 11' 0"	2X6

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



REVIEWED

By Julius Lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S

CONS. ENGINEERS P.A.

1455 SW 4th AVENUE

MIAMI BEACH, FL 33444-8161

No. 34869

STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE 7-02-CAB130M15

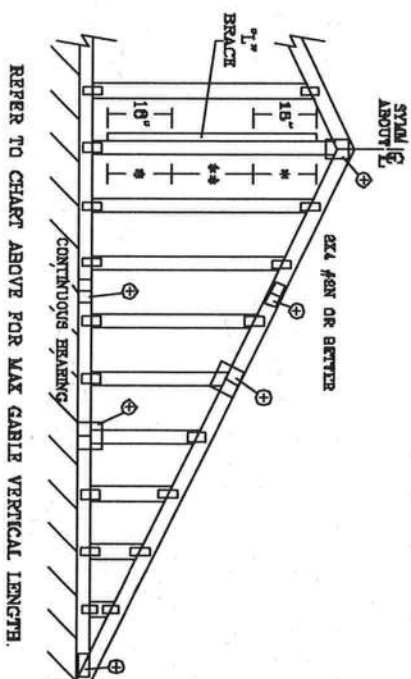
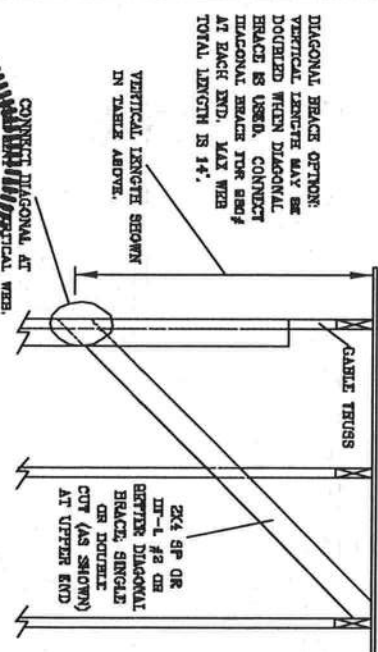
DATE 11/26/03

DRWG MTKX STD CABLE 19 E IT

-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH																		
CABLE VERTICAL SPACING	2x4 SPECIES	BRACE GRADE	NO BRACES	(1) 1x4 T" BRACE *		(1) 2x4 T" BRACE *		(2) 2x4 T" BRACE **		(1) 2x6 T" BRACE *		(2) 2x6 T" BRACE *		(2) 2x8 T" BRACE **				
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B					
12" O.C.	SPF	#1 / #2	#1	3' 2"	5' 6"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"				
			#3	3' 1"	4' 5"	4' 5"	6' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"				
		HF	STANDARD	3' 1"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"			
			#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"				
			#2	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"				
	SP	#3	STUD	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"				
			STANDARD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"				
		DFL	#1 / #2	STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"			
			#3	STUD	3' 8"	3' 8"	6' 4"	6' 6"	7' 6"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"				
			#8	STUD	3' 7"	3' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"				
16" O.C.	SPF	#1 / #2	#1	3' 7"	5' 6"	5' 6"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	14' 0"	14' 0"					
			#3	3' 7"	5' 6"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	14' 0"	14' 0"					
		HF	STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	13' 11"	12' 11"				
			#1	4' 0"	6' 4"	6' 10"	7' 6"	6' 1"	8' 11"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"				
			#2	STUD	3' 11"	8' 4"	8' 10"	7' 6"	6' 1"	8' 11"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"			
	SP	#3	STUD	3' 8"	5' 6"	6' 7"	7' 3"	7' 3"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"				
			STANDARD	3' 8"	5' 6"	6' 7"	7' 3"	7' 3"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"				
		DFL	STANDARD	3' 8"	4' 0"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"				
			#1 / #2	STUD	4' 0"	6' 11"	7' 2"	6' 3"	6' 3"	8' 10"	10' 1"	12' 11"	12' 11"	14' 0"	14' 0"			
			#3	STUD	3' 11"	8' 3"	8' 3"	6' 3"	6' 3"	8' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"			
24" O.C.	SPF	#1 / #2	#1	3' 11"	6' 8"	6' 8"	8' 1"	8' 1"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"					
			#3	3' 11"	6' 8"	6' 8"	8' 1"	8' 1"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"					
		HF	STANDARD	3' 11"	6' 8"	6' 8"	8' 1"	8' 1"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"					
			#1	3' 11"	6' 8"	6' 8"	8' 1"	8' 1"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"					
			#2	STUD	3' 11"	6' 8"	6' 8"	8' 1"	8' 1"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"				
	SP	#3	STUD	3' 11"	6' 8"	6' 8"	8' 1"	8' 1"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"					
			STANDARD	3' 11"	6' 8"	6' 8"	8' 1"	8' 1"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"					
		DFL	STANDARD	3' 11"	6' 8"	6' 8"	8' 1"	8' 1"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"					
			#1 / #2	STUD	3' 11"	6' 8"	6' 8"	8' 1"	8' 1"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"				
			#3	STUD	3' 11"	6' 8"	6' 8"	8' 1"	8' 1"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"				

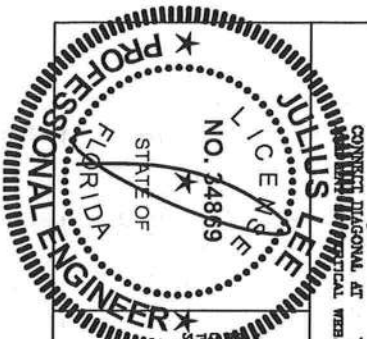


BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPF	#1 / #2	SPF	#1 / #2
STUD	#3	STUD	#3
STUD	STANDARD	STUD	STANDARD
STUD	STANDARD	STUD	STANDARD

CABLE TRUSS DETAIL NOTES:

- LIVE LOAD DEFLECTION CRITERIA IS $L/240$.
- PROVIDE UPLIFT CONNECTIONS FOR 150 PSF OVER CONTINUOUS BEARING (6 PSF TO DEAD LOAD).
- CABLE END SUPPORTS LOAD FROM 4' 0" OUTDOORS WITH 2' 0" OVERHANG, OR 12" PLWOOD OVERHANG.
- ATTACH EACH T" BRACE WITH 104 NAILS.
- * FOR (1) T" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
- ** FOR (2) T" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
- T" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES			
VERTICAL LENGTH	NO. SPICES	LESS THAN 4' 0"	1x4 OR 2x3
LESS THAN 4' 0"	NO. SPICES	LESS THAN 4' 0"	1x4 OR 2x3
LESS THAN 4' 0"	NO. SPICES	LESS THAN 4' 0"	1x4 OR 2x3
LESS THAN 4' 0"	NO. SPICES	LESS THAN 4' 0"	1x4 OR 2x3



PROFESSIONAL ENGINEER
JULIUS LEE
STATE OF FLORIDA
LICENSE NO. 34869

REVIEWED
By Julius Lee at 12:00 pm, Jun 11, 2008

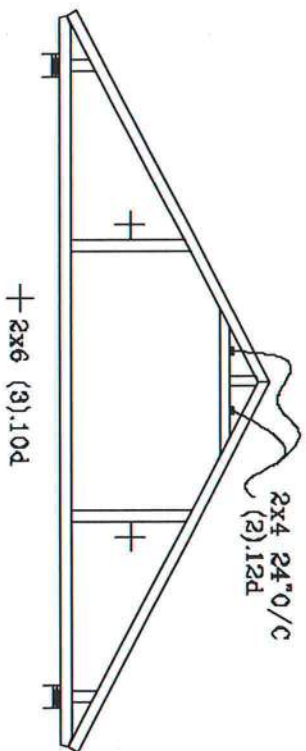
JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DELRAY BEACH, FL 33444-4161

No. 34869
STATE OF FLORIDA

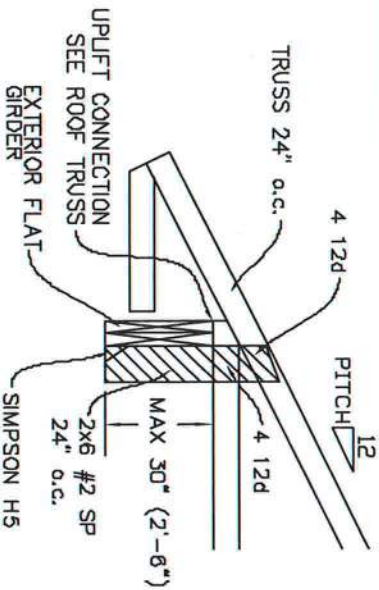
MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REF ASCE 7-02-CAB13030
DATE 11/26/03
DWG. WRTX STD GABLE 30' x 17'
-ENG

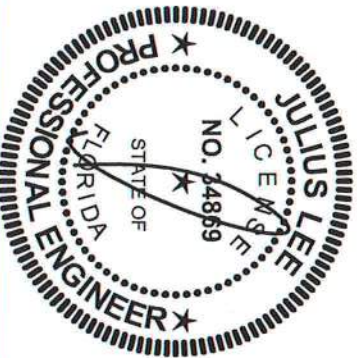
TYPICAL ATTIC TRUSS BRACING



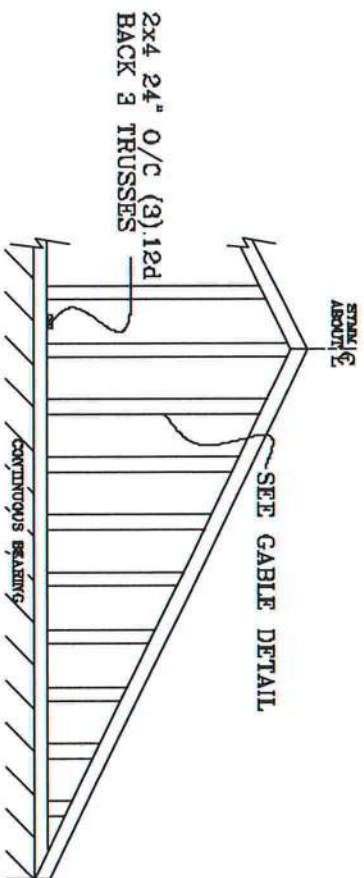
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

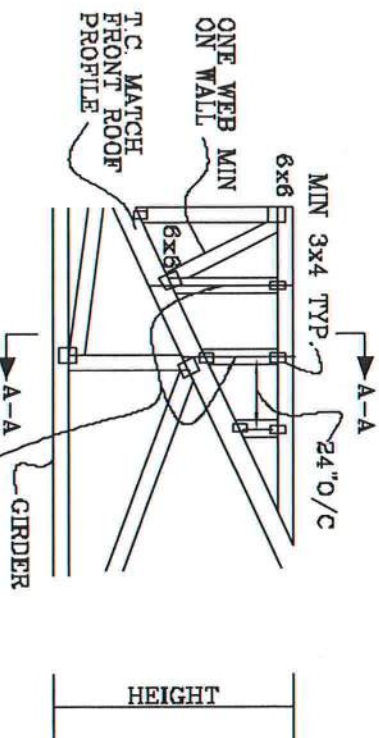


GABLE END TRUSS DETAIL



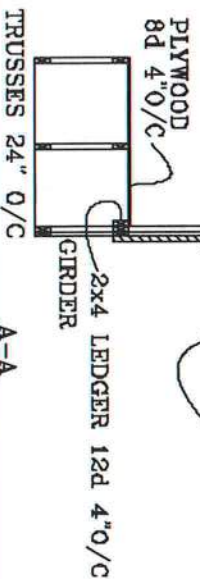
MINIMUM BC BRACING ON GABLE TRUSS OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT
ROOF 24" O/C

SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



A-A

JULIUS LEE'S
CONS. ENGINEERS P.A.
1454 SW 4th AVENUE
DIKELEY BEACH, FL 33444-2661

No: 34869
STATE OF FLORIDA

TOP CHORD 2X4 1/2 OR BETTER
BOT CHORD 2X4 1/2 OR BETTER
WEBS 2X4 #3 OR BETTER

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG,
LOCATED ANYWHERE IN BOAZ 1ST ELEV. COLISEUM

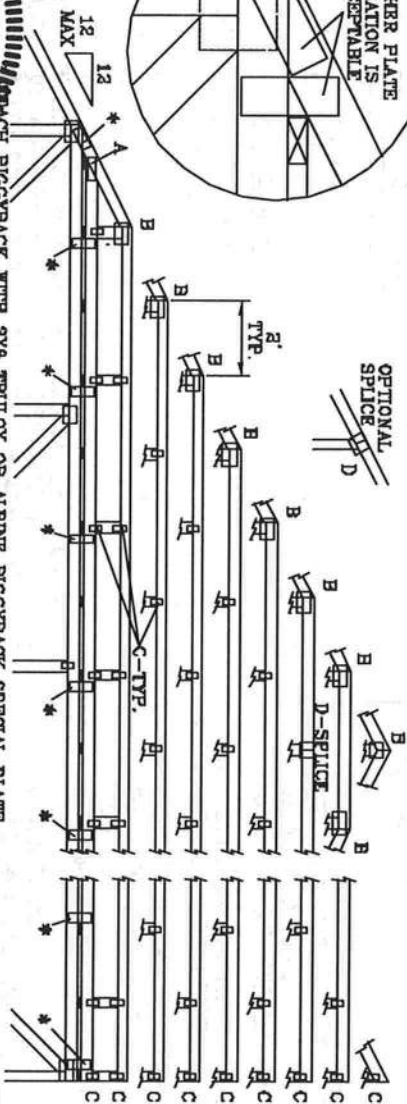
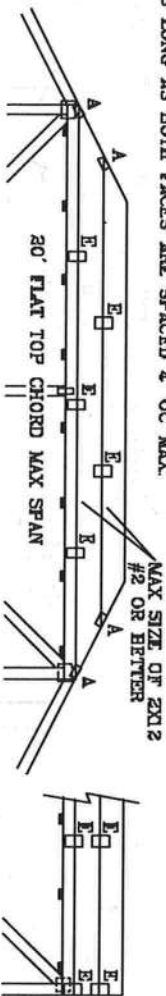
CAT 1, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 LFB BLDG. 30 MEAN HGT; EBC
ENCLOSED BLDG. LOCATED ANYWHERE IN

WIND TC DL-5 PSF, WIND BC DL-5 PSF

FRONT FACE (E_1^*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4" OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED
BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C,
WIND TC DL=6 PSF, WIND BC DL=6 PSF



JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.6X4	2.6X4	3X6
B	4X6	6X6	6X8	5X8
C	1.5X8	1.6X4	1.6X4	1.5X4
D	5X4	6X6	6X5	5X6
E	4X8 OR 3X8 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TADLOX PLATES WITH (6) 0.120" X 1.575" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TADLOX INFORMATION.

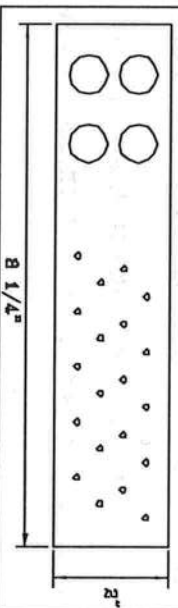
WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "L" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4" OC.
10' TO 14'	2x4 "L" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

OPTIONAL SPLICER

D

*** PIGGYBACK SPECIAL PLATE**

ATTACH LEETS TO THE PIGBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

FOR VARIATIONS. ITEMS BECAME EXTENSIVE CARE, FABRICATING, HANDLING, INSTALLING AND REPAIRS. REFER TO NEXT-100 QUALITY COMPONENT SAFETY INCORPORATED, P.O. BOX 100, 10000 W. 24TH AVENUE, 2400 DENVER PARK, SUITE 200, WADSWORTH, WY 82201 AND VITA EXCEL TRUSS COMPANY, ENTERPRISE LA WADSWORTH, WY 82201 FOR SAFETY PRACTICES PRIOR TO PROCEEDING FOR FUNCTIONS. THESE OTHERWISE DEDICATED, TOP CROWN SHALL HAVE BEING ATTACHED TO STRUCTURAL PLANTS AND BOTTOM CHORD SHALL HAVE A CROWN ATTACHED BEING ATTACHED.

**JULIUS LEE'S
CONS. ENGINEERS P.A.**

1400 SW 4th AVENUE
DUNBAR PLACE, FL 33411-2161

MAX LOADING
55 PSF AT
1.33 DUR. FAC.

50 PSF AT

1.25 DUR. FAC.

47 PST AT

1.15 DUR. FAC.

SPACING 24.0'

JULIUS LEE

NO. 74869

STATE OF

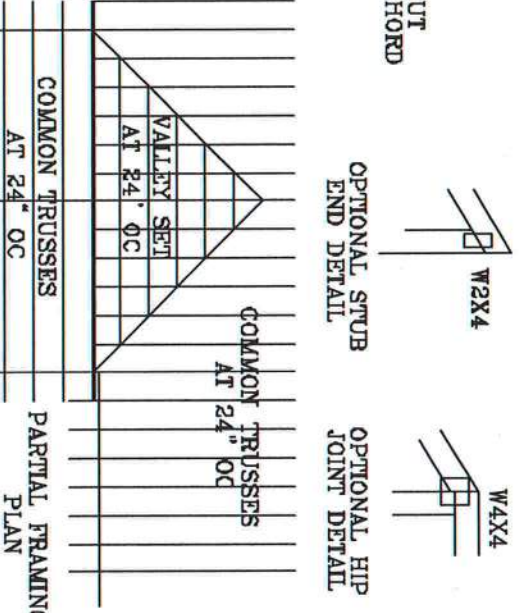
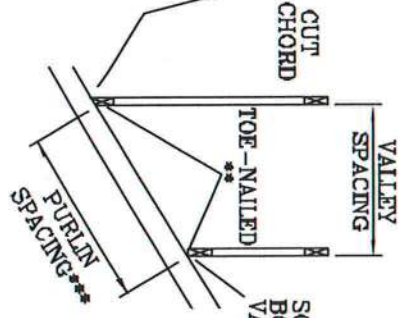
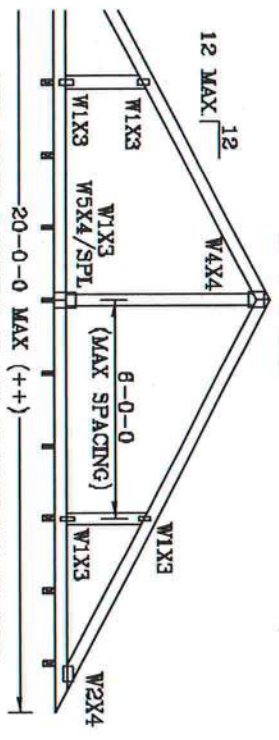
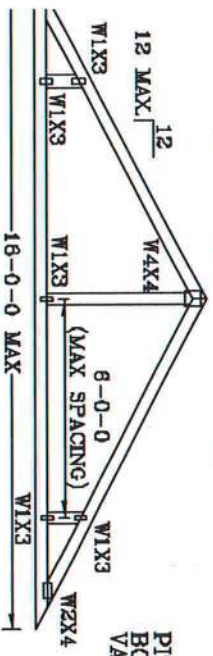
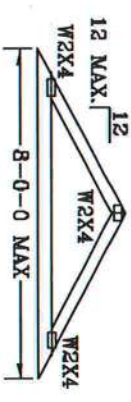
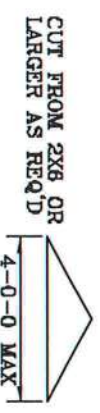
REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
 BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
 WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
 ** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
 (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
 FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
 ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
 BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF.

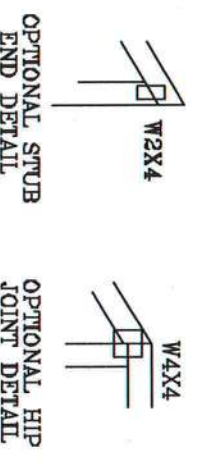


UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

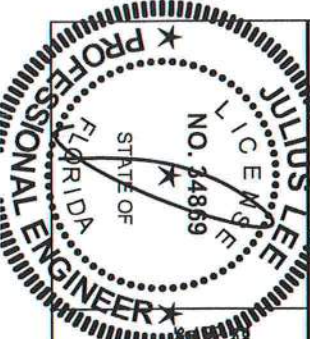
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
 PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
 INSTALLATION
 OR
 PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
 OR
 BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON
 ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS
 BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
 ++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES
 NOT EXCEED 12'0".
 BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



THIS DRAWING REPLACES DRAWING A105

WARNING: TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND
 MAINTAINING. REFER TO THE 1-800-848-8888 BUILDING DEPARTMENT INFORMATION. PUBLISHED BY THE TRUSS
 AND JOINT INSTITUTE, 560 CONCORD DR., SUITE 200, MADISON, VA 53719 AND VITCO CYCLO TRUSS CYCLO
 AMERICA, 6500 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING
 THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED
 STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



REVIEWED
 By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
 CONSULTING ENGINEERS P.A.
 1455 SW 4th Avenue
 DEPT. BLDG. 7L 5044-0101
 MIAMI, FL 33135

No. 34868
 STATE OF FLORIDA

TC LL	20	20	PSF	REF	VALLEY DETAIL
BC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		
DURFAC	1.25	1.25			
SPACING	24"				

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AP&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

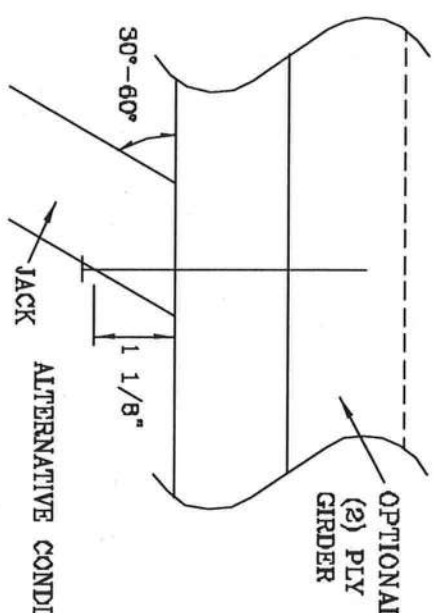
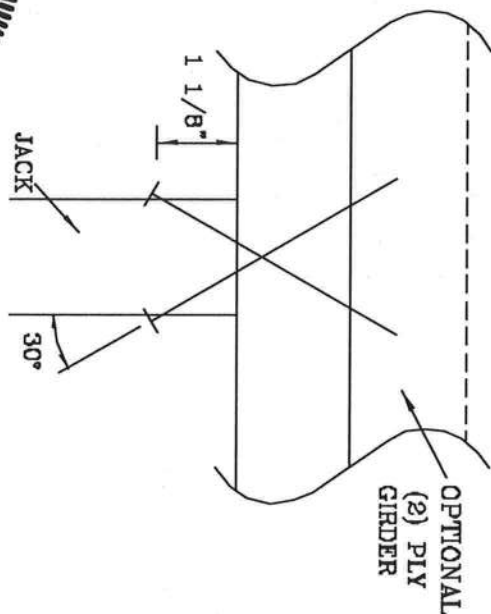
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

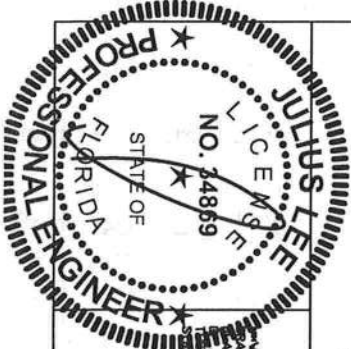
NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	187#	256#	181#	234#	156#	203#	154#	189#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, STORING, INSTALLING AND ERECTING. REFER TO BCST 1-43 CALLING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 218 POUNDER DR., SUITE 200, NATION, VA 20719 AND VTOA (WOOD TRUSS COUNCIL), 1000 17TH AVENUE, SUITE 100, DENVER, CO 80202. UNLESS OTHERWISE INDICATED, THE GIRDER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BATTEN GIRDERS SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 8TH AVE. SUITE 100
DENVER, CO 80202

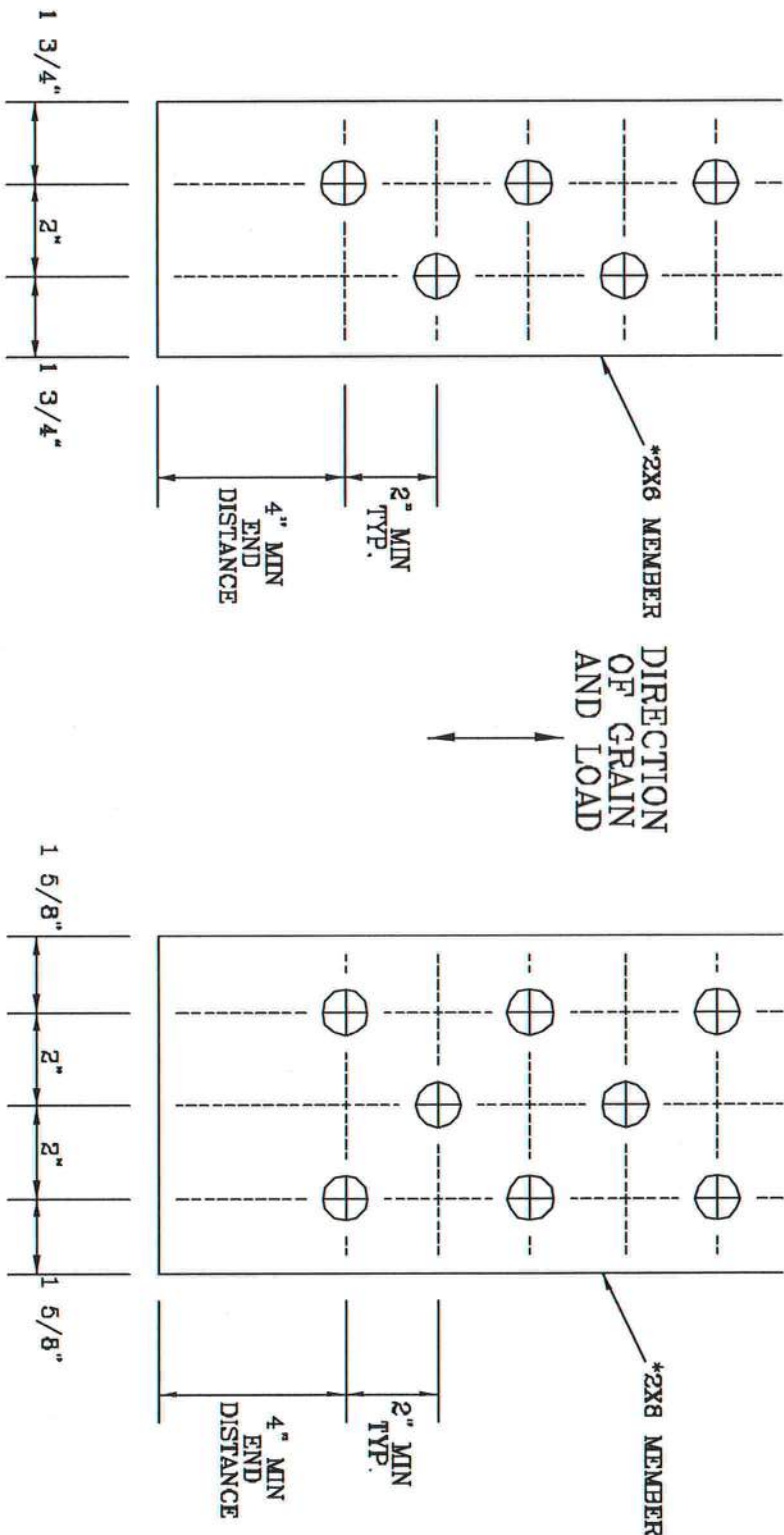
No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT

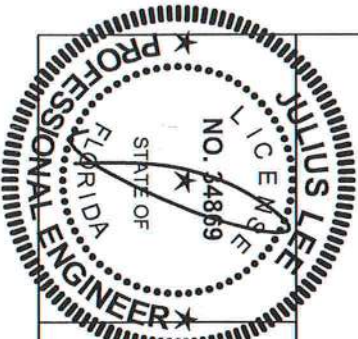


2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTION. REFER TO BEST PRACTICES FOR BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS AND JOIST MANUFACTURERS ASSOCIATION (TJMA) AND THE NATIONAL WOOD JOINT COMMISSION (NWJC). THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, TOP CREW SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND EDITION CHECK SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1450 57th AVE
DELRAY BEACH, FL 33444-2191

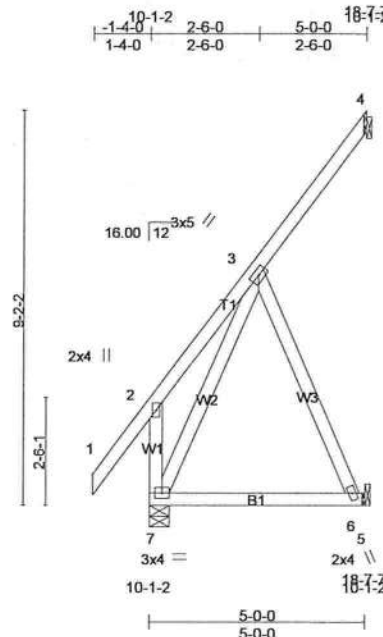
No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI1103
BC LL	PSF	ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

Job 315479	Truss CJ5A	Truss Type JACK	Qty 1	Ply 1	Disosway Custom Residence	I4119347
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Builders FrstSource, Lake City, FL 32055

7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:28 2009 Page 1



Scale = 1:51.6

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.51	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.15	Vert(LL) -0.02 6-7 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.78	Vert(TL) -0.04 6-7 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.03 4 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.00 6 **** 240		
				Weight: 44 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3 *Except* W1: 2 X 4 SYP No.2	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

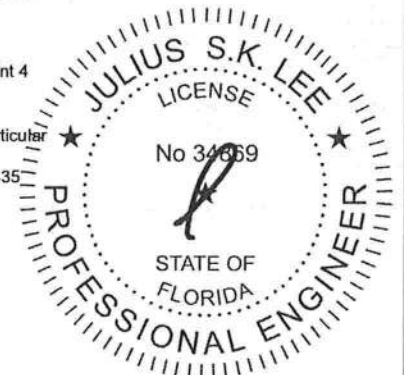
REACTIONS (lb/size) 7=243/0-5-8, 4=56/Mechanical, 6=89/Mechanical
 Max Horz 7=642(LC 6)
 Max Uplift 7=-43(LC 4), 4=-174(LC 6), 6=-460(LC 6)
 Max Grav 7=289(LC 6), 4=56(LC 1), 6=114(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-7=159/829, 2-3=-63/733
 BOT CHORD 6-7=-257/41
 WEBS 3-6=-107/668, 3-7=-1300/42

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 7, 174 lb uplift at joint 4 and 460 lb uplift at joint 6.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

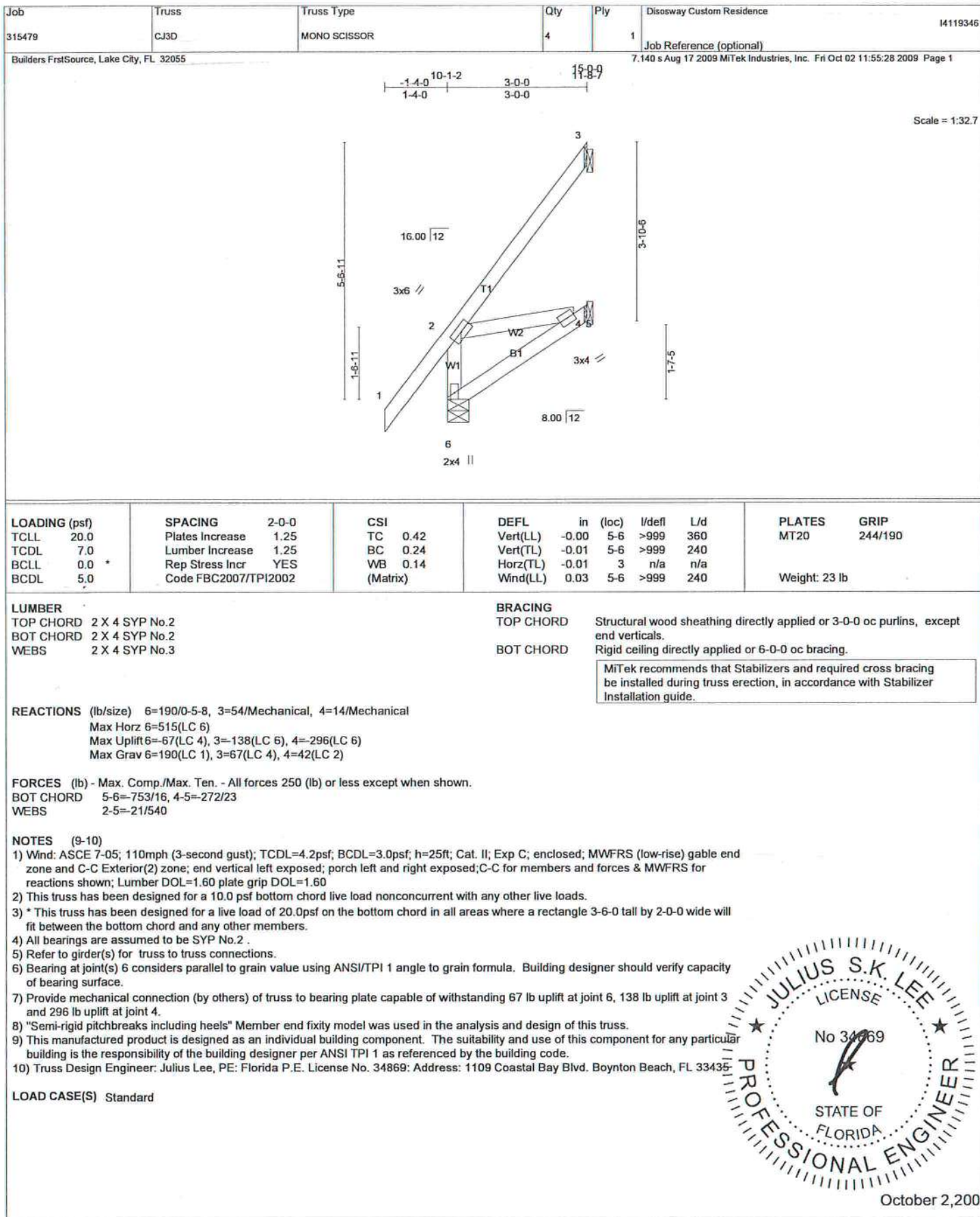
LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

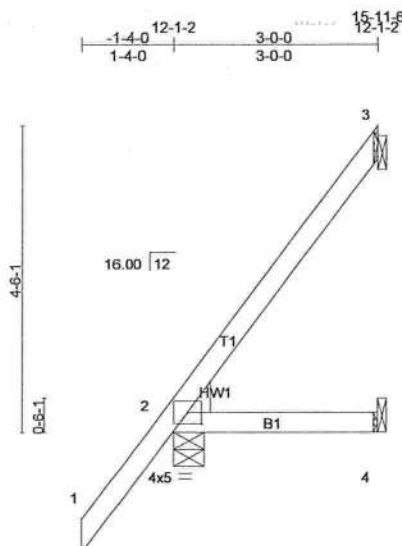


Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence
315479	CJ3C	JACK	2	1	

I4119345

Builders FirstSource, Lake City, FL 32055

7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:28 2009 Page 1



Scale = 1:32.7

Plate Offsets (X,Y): [2-0-2-9,0-2-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.35	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.11	Vert(TL)	-0.00	2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.01	2-4	>999	240		
									Weight: 17 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 6 SYP No.1D

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=49/Mechanical, 2=195/0-5-8, 4=14/Mechanical

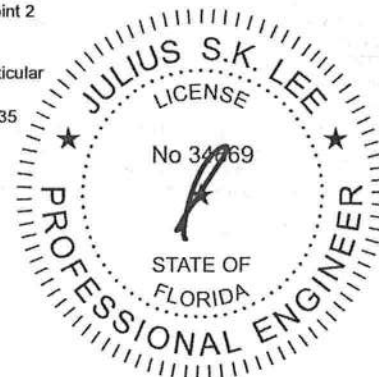
Max Horz 2=396(LC 6)

Max Uplift 3=180(LC 6), 2=126(LC 6), 4=46(LC 4)

Max Grav 3=69(LC 4), 2=195(LC 1), 4=41(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.**NOTES** (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 3, 126 lb uplift at joint 2 and 46 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence
315479	CJ3B	JACK	1	1	

I4119344

Builders FirstSource, Lake City, FL 32055

7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:27 2009 Page 1

10-1-2
3-0-0
3-0-0
15-11-5
10-1-2

Scale = 1:36.5

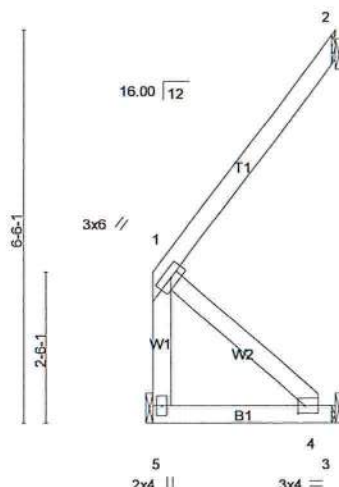


Plate Offsets (X,Y): [4:0-1-6,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	-0.00	4-5	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.08	Vert(TL)	-0.01	4-5	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.10	Horz(TL)	-0.01	2	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.01	4-5	>999	240		
									Weight: 20 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=89/Mechanical, 2=75/Mechanical, 3=14/Mechanical
Max Horz 5=232(LC 6)
Max Uplift 5=20(LC 4), 2=229(LC 6), 3=183(LC 6)
Max Grav 5=279(LC 6), 2=75(LC 1), 3=42(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

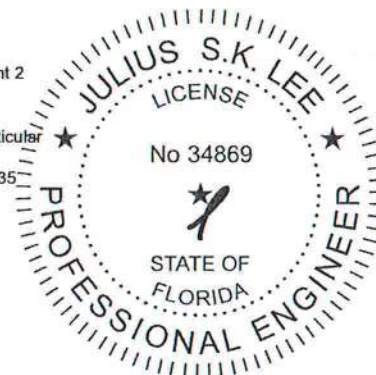
TOP CHORD 1-5=361/31, 1-2=-251/48

BOT CHORD 4-5=293/36

WEBS 1-4=49/394

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 5, 229 lb uplift at joint 2 and 183 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

October 2, 2009

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

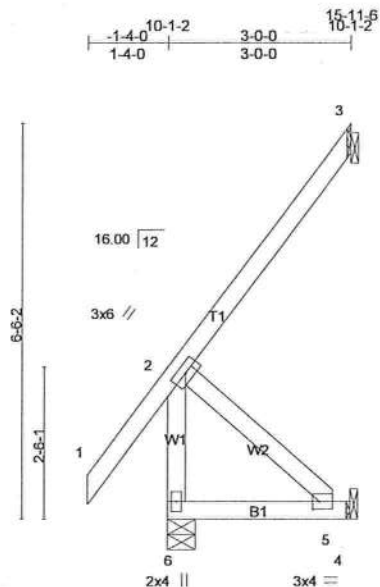
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	14119343
315479	CJ3A	JACK	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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Scale = 1:36.5

Plate Offsets (X,Y): [5:0-1-6,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL)	-0.00	5-6	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.14	Vert(TL)	-0.01	5-6	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.17	Horz(TL)	-0.01	3	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.01	5-6	>999	240		
									Weight: 24 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2 *Except*
W2: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-8-11 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=190/0-5-8, 3=54/Mechanical, 4=14/Mechanical
Max Horz 6=404(LC 6)
Max Uplift 6=-23(LC 4), 3=-138(LC 6), 4=-326(LC 6)
Max Grav 6=214(LC 6), 3=67(LC 4), 4=42(LC 2)

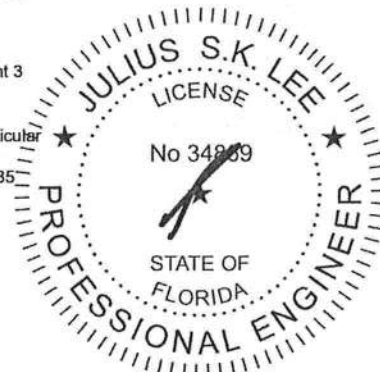
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-6=-299/31
BOT CHORD 5-6=-489/0
WEBS 2-5=0/659

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 6, 138 lb uplift at joint 3 and 326 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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October 2, 2009

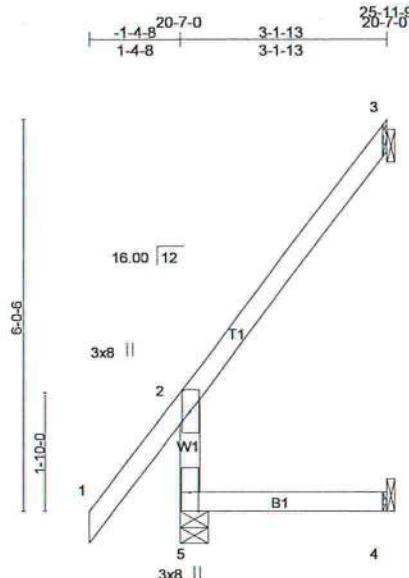
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119342
315479	CJ2	JACK	3	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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Scale = 1:34.0

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.89	Vert(LL)	-0.00	4-5	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.50	Vert(TL)	-0.00	4-5	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.15	3	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.04	4-5	>940	240		
									Weight: 19 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-1-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

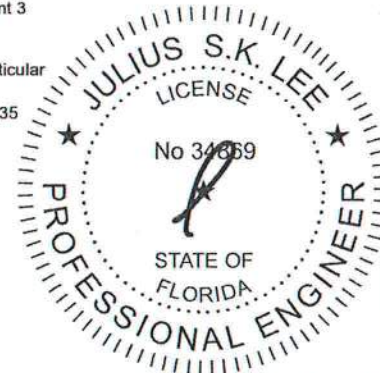
REACTIONS (lb/size) 5=132/0-5-0, 3=40/Mechanical, 4=9/Mechanical
Max Horz 5=315(LC 6)
Max Uplift 5=-25(LC 4), 3=-177(LC 6), 4=-82(LC 6)
Max Grav 5=132(LC 1), 3=42(LC 4), 4=28(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 5, 177 lb uplift at joint 3 and 82 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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October 2, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

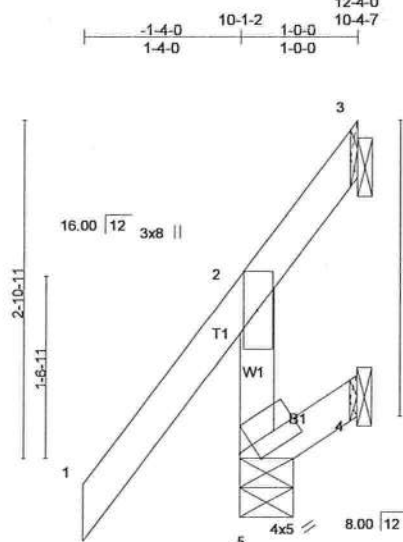
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence
315479	CJ1D	MONO SCISSOR	4	1	I4119341

Builders FrstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Aug 17 2009 MITek Industries, Inc. Fri Oct 02 11:55:26 2009 Page 1



Scale = 1:19.0

Plate Offsets (X,Y): [5:0-1-9,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.72	Vert(LL)	0.00	5	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.26	Vert(TL)	0.00	5	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.03	3	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.00	4-5	>999	240		
									Weight: 10 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=177/0-5-8, 4=9/Mechanical, 3=34/Mechanical
Max Horz 5=261(LC 6)
Max Uplift 5=32(LC 4), 4=197(LC 6), 3=54(LC 7)
Max Grav 5=177(LC 1), 4=9(LC 2), 3=38(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (9-10)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 5, 197 lb uplift at joint 4 and 54 lb uplift at joint 3.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss CJ1C	Truss Type JACK	Qty 2	Ply 1	Disosway Custom Residence Job Reference (optional) 7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:26 2009 Page 1
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Builders FrstSource, Lake City, FL 32055
Scale = 1:19.0

Plate Offsets (X,Y): [2:0-2-9,0-2-0]									
LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	I/defl	L/d	PLATES	GRIP		
TCLL 20.0	Plates Increase 1.25	TC 0.33	Vert(LL) -0.00	2 >999	360	MT20	244/190		
TCDL 7.0	Lumber Increase 1.25	BC 0.01	Vert(TL) -0.00	2 >999	240				
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00	3 n/a	n/a				
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.00	2 >999	240			Weight: 9 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 6 SYP No.1D

REACTIONS (lb/size) 2=159/0-5-8, 4=5/Mechanical, 3=-29/Mechanical

Max Horz 2=234(LC 6)

Max Uplift 2=-286(LC 6), 4=-16(LC 4), 3=-29(LC 1)

Max Grav 2=159(LC 1), 4=14(LC 2), 3=113(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 16 lb uplift at joint 4 and 29 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

October 2, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

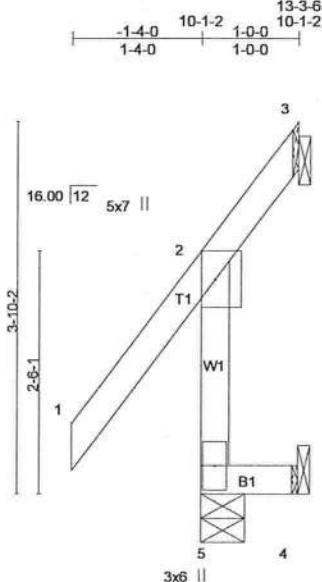
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss CJ1A	Truss Type JACK	Qty 2	Ply 1	Disosway Custom Residence I4119339
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:25 2009 Page 1



Scale = 1:22.9

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.58	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.46	Vert(LL) 0.00 5 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) 0.00 5 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.09 3 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.00 4-5 >999 240		
				Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

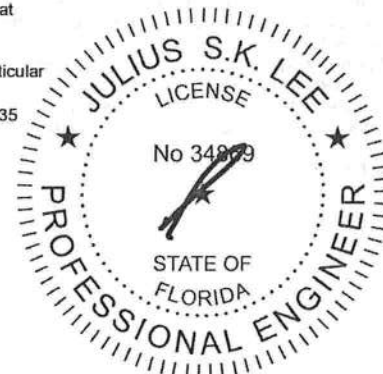
REACTIONS (lb/size) 5=177/0-5-8, 4=7/Mechanical, 3=36/Mechanical
Max Horz 5=242(LC 6)
Max Uplift 4=306(LC 6), 3=121(LC 6)
Max Grav 5=270(LC 6), 4=9(LC 2), 3=29(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 306 lb uplift at joint 4 and 121 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss CJ1	Truss Type JACK	Qty 3	Ply 1	Disosway Custom Residence Job Reference (optional) 7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:24 2009 Page 1
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Builders FrstSource, Lake City, FL 32055

I4119338

Scale = 1:25.0

LOADING (psf) TCCL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 1-4-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2007/TPI2002	CSI TC 0.65 BC 0.33 WB 0.00 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.00 5 >999 360 Vert(TL) -0.00 5 >999 240 Horz(TL) -0.06 3 n/a n/a Wind(LL) 0.01 4-5 >999 240	PLATES GRIP MT20 244/190 Weight: 13 lb
---	---	---	---	---

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-9-13 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=115/0-5-0, 4=0/Mechanical, 3=10/Mechanical
 Max Horz 5=244(LC 6)
 Max Uplift 5=33(LC 4), 4=-99(LC 6), 3=-104(LC 6)
 Max Grav 5=115(LC 1), 4=15(LC 2), 3=34(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 5, 99 lb uplift at joint 4 and 104 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

October 2, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-87 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

RE: 315479 - Disosway Custom Residence

Site Information:

Project Customer: DAVID DISOSWAY Project Name: 315479 Model: CUSTOM

Lot/Block: 11 Subdivision: ROSE CREEK

Address:

City: COLUMBIA CTY

State: FL

No.	Seal#	Truss Name	Date
35	I4119372	T03	10/2/09
36	I4119373	T04	10/2/09
37	I4119374	T05	10/2/09
38	I4119375	T05A	10/2/09
39	I4119376	T05G	10/2/09
40	I4119377	T06	10/2/09
41	I4119378	T07	10/2/09
42	I4119379	T08	10/2/09
43	I4119380	T09	10/2/09
44	I4119381	T09A	10/2/09
45	I4119382	T10	10/2/09
46	I4119383	T11	10/2/09
47	I4119384	T12	10/2/09
48	I4119385	T13	10/2/09
49	I4119386	T14	10/2/09
50	I4119387	T15	10/2/09
51	I4119388	T16	10/2/09
52	I4119389	T17	10/2/09
53	I4119390	T18	10/2/09
54	I4119391	T19	10/2/09
55	I4119392	T20	10/2/09
56	I4119393	T21	10/2/09

Julius Lee Engineering

RE: 315479 - Disosway Custom Residence

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: DAVID DISOSWAY Project Name: 315479 Model: CUSTOM
Lot/Block: 11 Subdivision: ROSE CREEK
Address:
City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: MARK D. DISOSWAY License #: 53915
Address: P.O. BOX 868
City: LAKE CITY, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 56 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

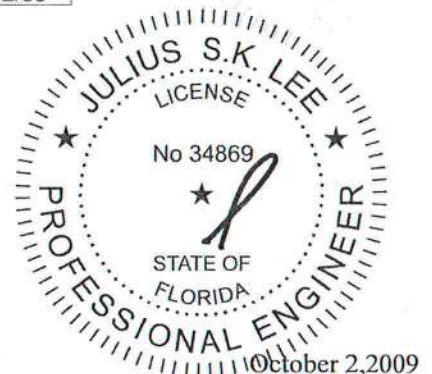
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I4119338	CJ1	10/2/09	18	I4119355	FG1	10/2/09
2	I4119339	CJ1A	10/2/09	19	I4119356	HJ1	10/2/09
3	I4119340	CJ1C	10/2/09	20	I4119357	HJ6	10/2/09
4	I4119341	CJ1D	10/2/09	21	I4119358	HJ6A	10/2/09
5	I4119342	CJ2	10/2/09	22	I4119359	HJ6B	10/2/09
6	I4119343	CJ3A	10/2/09	23	I4119360	HJ9	10/2/09
7	I4119344	CJ3B	10/2/09	24	I4119361	HJ9A	10/2/09
8	I4119345	CJ3C	10/2/09	25	I4119362	HJ9D	10/2/09
9	I4119346	CJ3D	10/2/09	26	I4119363	MR1	10/2/09
10	I4119347	CJ5A	10/2/09	27	I4119364	MR2	10/2/09
11	I4119348	CJ5B	10/2/09	28	I4119365	MR3	10/2/09
12	I4119349	CJ5C	10/2/09	29	I4119366	MR4	10/2/09
13	I4119350	CJ5D	10/2/09	30	I4119367	PB1	10/2/09
14	I4119351	EJ1	10/2/09	31	I4119368	PB2	10/2/09
15	I4119352	EJ7A	10/2/09	32	I4119369	T01	10/2/09
16	I4119353	EJ7B	10/2/09	33	I4119370	T01G	10/2/09
17	I4119354	EJ7D	10/2/09	34	I4119371	T02	10/2/09

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 ⁽¹⁾	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
3 3/4" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽²⁾	1,775 ⁽²⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽²⁾	2,665 ⁽²⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽²⁾	3,550 ⁽²⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

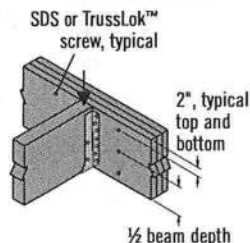
(2) 6" long screws required.

(3) 5" long screws required.

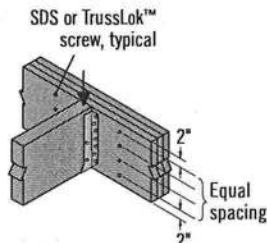
(4) 3 1/2" and 3 3/4" long screws must be installed on both sides.

Connections

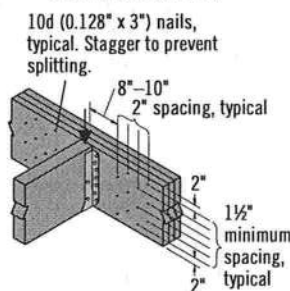
4 or 6 Screw Connection



8 Screw Connection

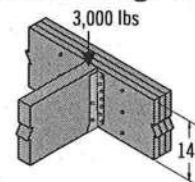


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/4" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/4" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

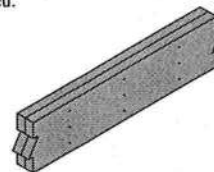
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 1/16" maximum.

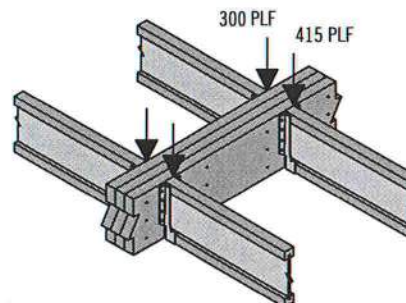
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example

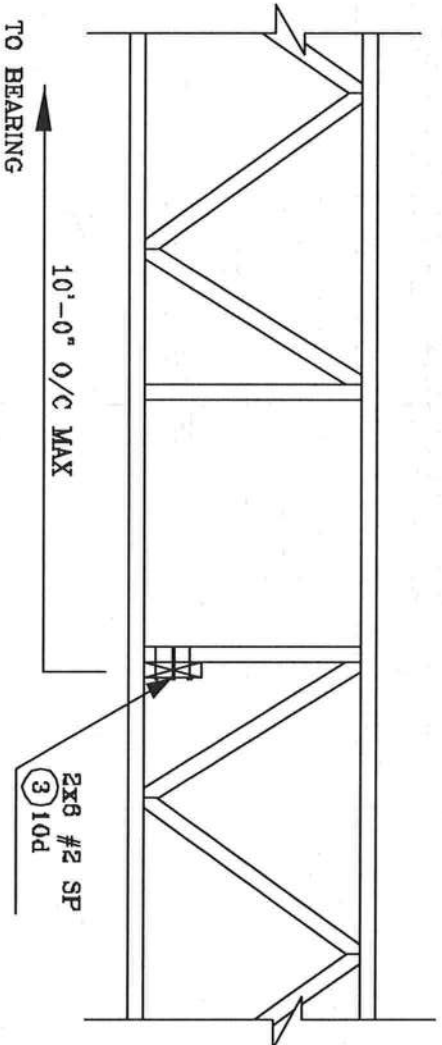


First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

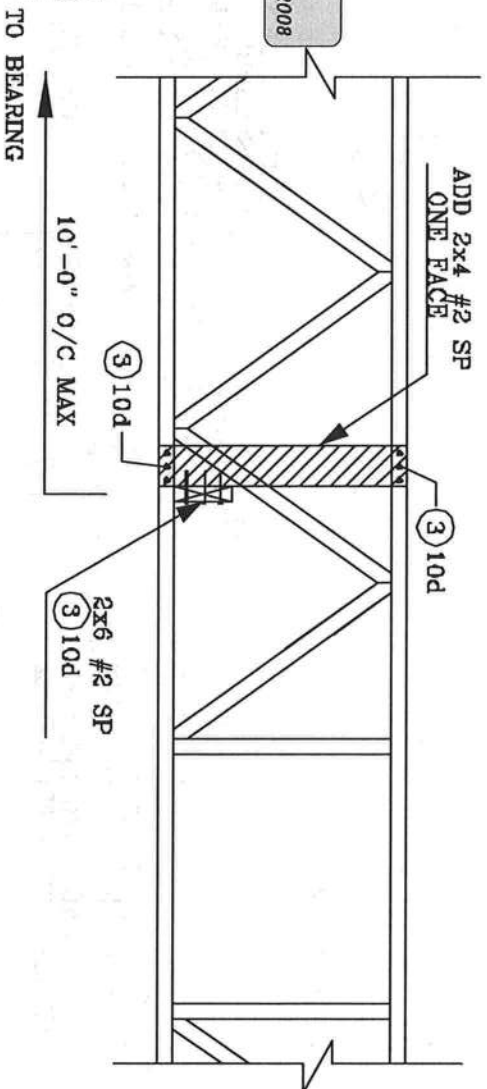
Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS

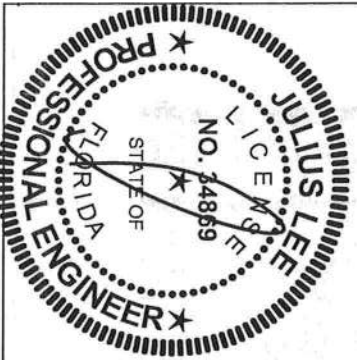


ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED

By Julius Lee at 11:58 am, Jun 11, 2008



JULIUS LEE'S
CONS. ENGINEERS P.A.

1426 SW 4th AVENUE
DIKEWAY BEACH, FL 33444-2691

No. 34869
STATE OF FLORIDA

TRULOX CONNECTION DETAIL

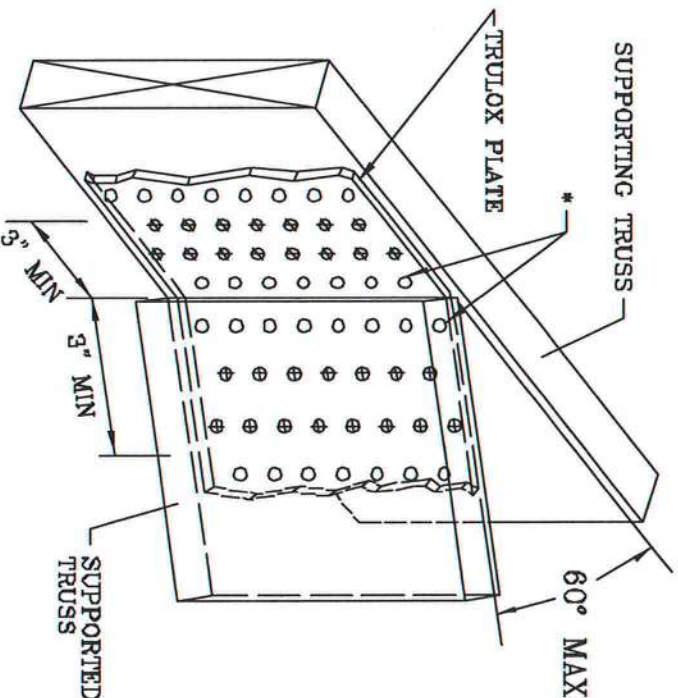
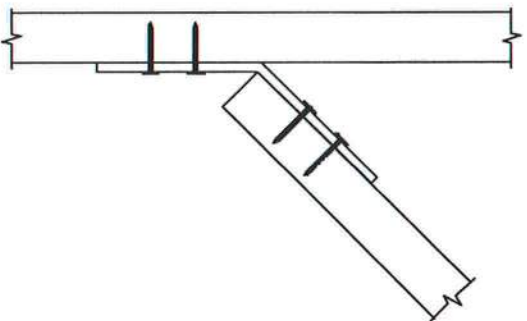
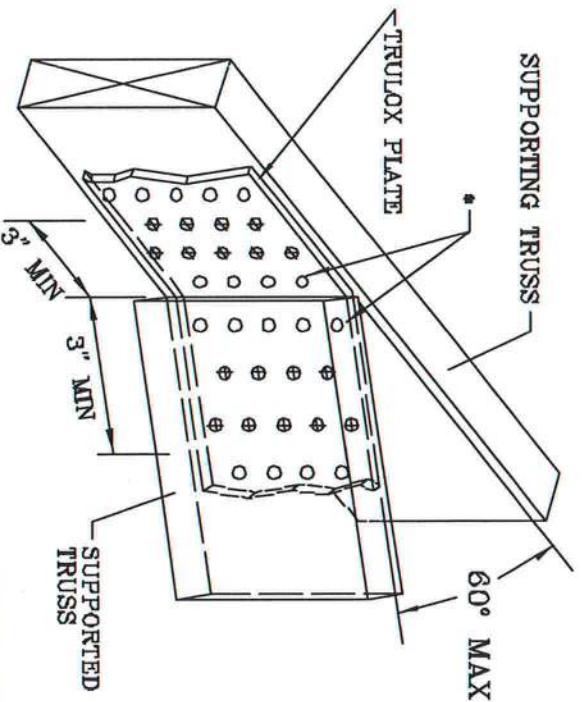
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

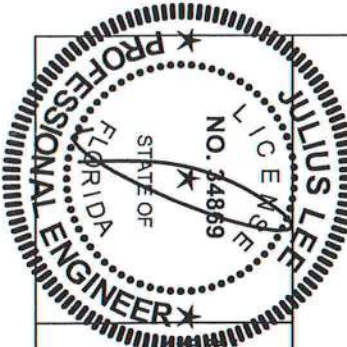


MINIMUM 3X6 TRULOX PLATE

MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

THIS DRAWING REPLACES DRAWINGS 1,158,869 1,158,989/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 2031-1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS ASSOCIATION, 583 DOWNSIDE DR., SUITE 200, WATSON, VA 22795 AND VITA CYCLO TRUSS COUNCIL, 1000 N. 10TH ST., SUITE 100, ARLINGTON, VA 22201 FOR SAFETY PRACTICES PRIOR TO PERFORMING TRUSS CONSTRUCTION. THESE PRACTICES SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DECATUR, GA 30034-2100

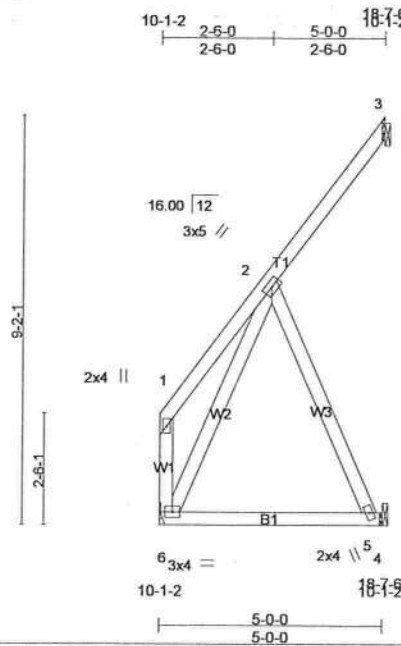
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

Job 315479	Truss CJ5B	Truss Type JACK	Qty 1	Ply 1	Disosway Custom Residence	I4119348
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Builders FirstSource, Lake City, FL 32055

7.140 s Aug 17 2009 Mitek Industries, Inc. Fri Oct 02 11:55:29 2009 Page 1



Scale = 1:49.8

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.24	Vert(LL) -0.02 5-6 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.15	Vert(TL) -0.04 5-6 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.40	Horz(TL) -0.02 3 n/a n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.00 5 **** 240		
				Weight: 40 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

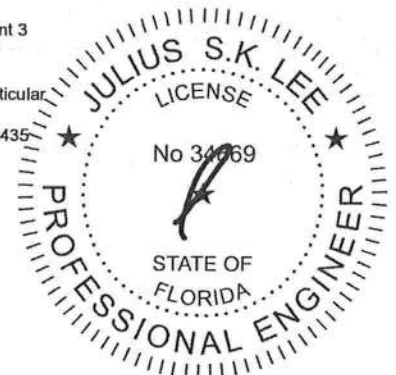
REACTIONS (lb/size) 6=150/Mechanical, 3=50/Mechanical, 5=107/Mechanical
Max Horz 6=397(LC 6)
Max Uplift 6=17(LC 4), 3=151(LC 6), 5=435(LC 6)
Max Grav 6=357(LC 6), 3=50(LC 1), 5=107(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-6=666/67, 2-5=92/620

NOTES (9-10)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 6, 151 lb uplift at joint 3 and 435 lb uplift at joint 5.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

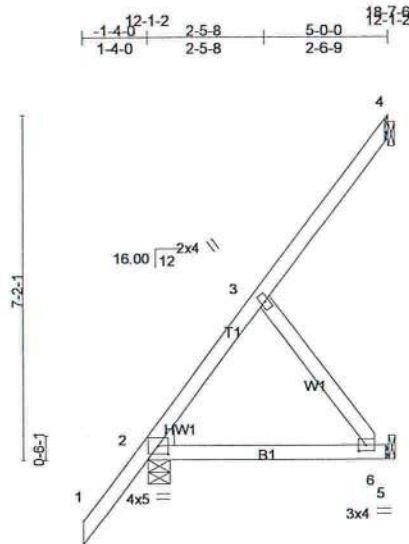
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence
315479	CJSC	JACK	2	1	14119349

Builders FirstSource, Lake City, FL 32055

7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:29 2009 Page 1



Scale = 1:46.0

Plate Offsets (X,Y): [2:0-2-9,0-2-0], [6:0-1-13,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.35	Vert(LL)	-0.03	2-6	>999	360	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.37	Vert(TL)	-0.05	2-6	>999	240	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.08	Horz(TL)	-0.01	4	n/a	n/a	
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.13	2-6	>447	240	Weight: 30 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 6 SYP No.1D

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=62/Mechanical, 2=249/0-5-8, 5=75/Mechanical
Max Horz 2=562(LC 6)
Max Uplift 4=186(LC 6), 2=109(LC 4), 5=257(LC 6)
Max Grav 4=62(LC 1), 2=249(LC 1), 5=82(LC 2)

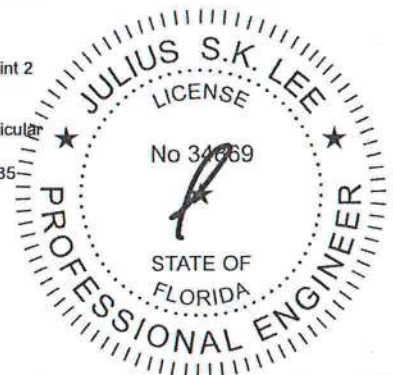
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=327/36
WEBS 3-6=87/324

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 186 lb uplift at joint 4, 109 lb uplift at joint 2 and 257 lb uplift at joint 5.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.

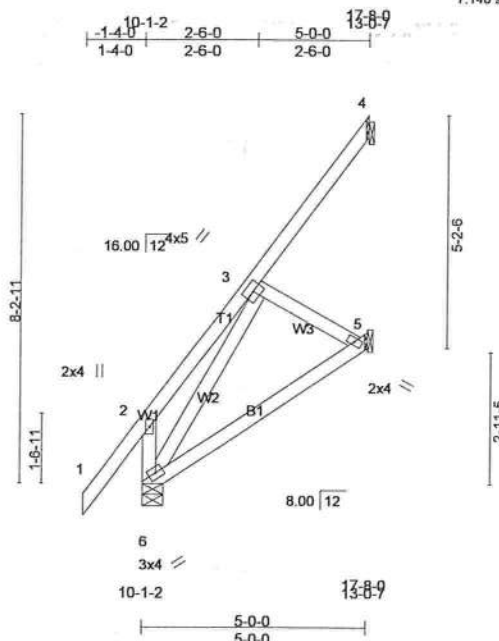
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss CJSD	Truss Type MONO SCISSOR	Qty 4	Ply 1	Disosway Custom Residence	14119350
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Builders FrstSource, Lake City, FL 32055

7.140 s Aug 17 2009 MITek Industries, Inc. Fri Oct 02 11:55:30 2009 Page 1



Scale = 1:49.5

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.50	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.63	Vert(LL) -0.03 5-6 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.35	Vert(TL) -0.05 5-6 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.02 4 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.22 5-6 >254 240		
				Weight: 38 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-8-6 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=244/0-5-8, 4=60/Mechanical, 5=82/Mechanical
Max Horz 6=724(LC 6)
Max Uplift 6=79(LC 4), 4=184(LC 6), 5=455(LC 6)
Max Grav 6=244(LC 1), 4=60(LC 1), 5=86(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-6=156/785, 2-3=61/683
BOT CHORD 5-6=645/68
WEBS 3-6=842/0, 3-5=75/429

NOTES (9-10)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 6, 184 lb uplift at joint 4 and 455 lb uplift at joint 5.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



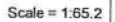
October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.
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Weight: 56 lb

October 2, 2009

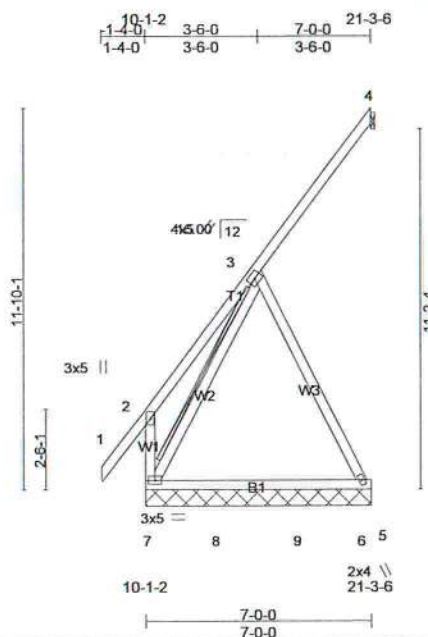
Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disoway Custom Residence
315479	EJTB	MONO TRUSS	1	1	

I4119353

Builders FirstSource, Lake City, FL 32055

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Scale = 1:68.6

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.44	Vert(LL)	-0.04	6-7	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.28	Vert(TL)	-0.07	6-7	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.37	Horz(TL)	-0.04	4	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.01	6-7	>999	240		
									Weight: 56 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3 *Except*
 W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 3-7
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 7-0-0.
 (lb) - Max Horz 7=603(LC 6)
 Max Uplift All uplift 100 lb or less at joint(s) 7 except 4=160(LC 6), 5=680(LC 1), 6=295(LC 6)
 Max Grav All reactions 250 lb or less at joint(s) 4, 4 except 7=330(LC 1), 5=679(LC 1), 6=891(LC 1)

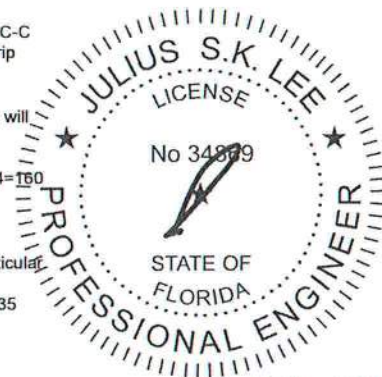
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-7=185/696, 2-3=94/591, 3-4=252/47
 BOT CHORD 7-8=348/54, 8-9=348/54, 6-9=348/54
 WEBS 3-6=121/776, 3-7=1210/38

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 4) All bearings are assumed to be SYP No.2.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 4=160, 5=680, 6=295.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-T473 BEFORE USE.**

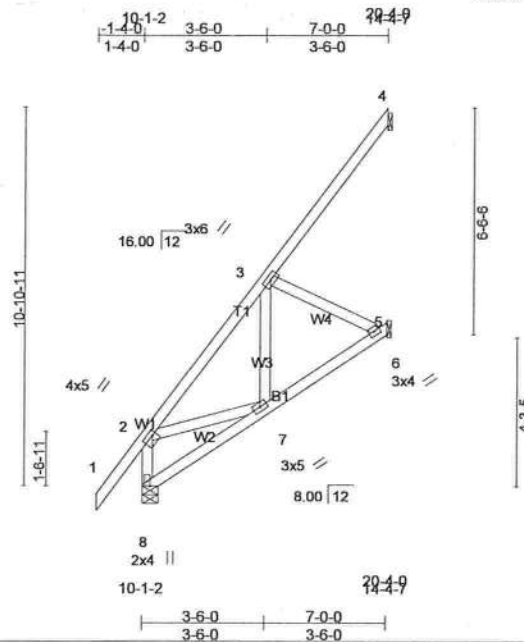
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-B9 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 315479	Truss EJ7D	Truss Type MONO SCISSOR	Qty 4	Ply 1	Disoway Custom Residence	I4119354
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Builders FrstSource, Lake City, FL 32055

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Scale: 3/16\"=1'

Plate Offsets (X,Y): [2-0-1-4,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.41	Vert(LL)	-0.01	6-7	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.31	Vert(TL)	-0.01	6-7	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.22	Horz(TL)	-0.02	5	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.06	6-7	>999	240		Weight: 51 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-8 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=306/0-5-8, 4=74/Mechanical, 5=134/Mechanical
Max Horz 8=728(LC 6)
Max Uplift 8=77(LC 4), 4=155(LC 6), 5=467(LC 6)

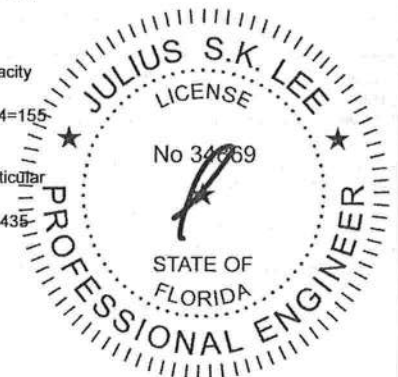
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-8=290/607, 2-3=254/447
BOT CHORD 7-8=1480/39, 6-7=1019/134, 5-6=570/75
WEBS 3-6=123/863, 3-7=655/93

NOTES (9-10)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (j=lb) 4=155, 5=467.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

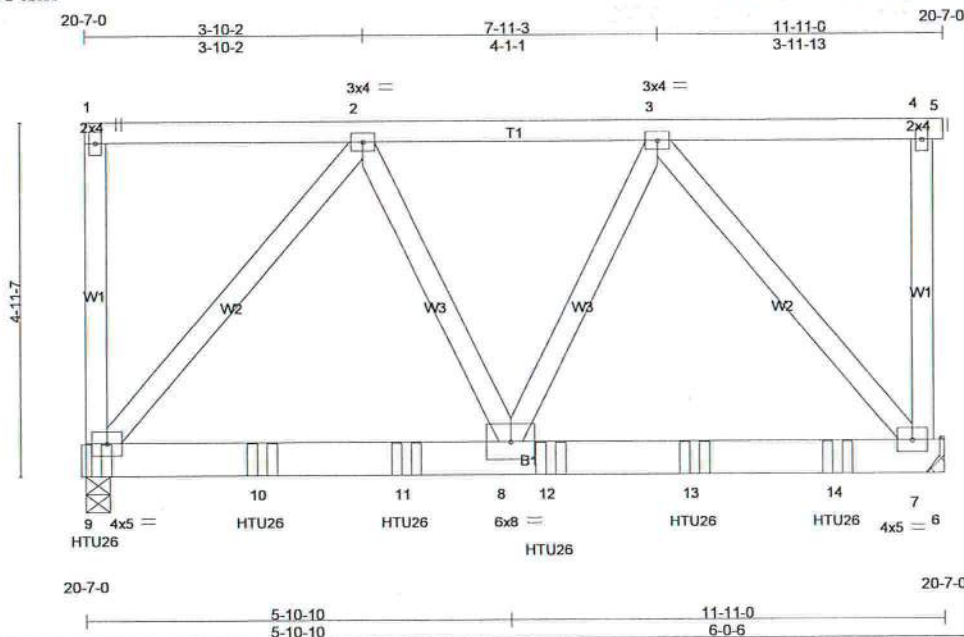
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disoway Custom Residence	I4119355
315479	FG1	SPECIAL	1	2	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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Scale = 1:30.9

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.24	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.39	Vert(LL) -0.04 7-8 >999 360		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.26	Vert(TL) -0.07 7-8 >999 240		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.01 7 n/a n/a		
			Wind(LL) 0.04 7-8 >999 240		
				Weight: 177 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 6 SYP No.1D	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

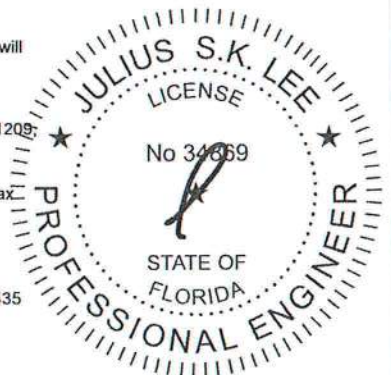
REACTIONS (lb/size) 9=2568/0-4-0, 7=2229/Mechanical
Max Uplift 9=1209(LC 3), 7=1054(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=1690/787
BOT CHORD 9-10=552/1161, 10-11=552/1161, 8-11=552/1161, 8-12=553/1162, 12-13=553/1162, 13-14=553/1162, 7-14=553/1162
WEBS 2-9=1806/860, 2-8=575/1298, 3-8=573/1293, 3-7=1788/851

- NOTES** (13-15)
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=l=lb) 9=1209; 7=1054.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Use Simpson Strong-Tie HTU26 (20-10dx1 1/2 Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-3-8 oc max starting at 0-1-12 from the left end to 10-5-4 to connect truss(es) T02 (1 ply 2 X 4 SYP) to back face of bottom chord.
 - Fill all nail holes where hanger is in contact with lumber.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - Use Simpson HGUS28-2 to attach Truss to Carrying member

LOAD CASE(S) Standard

Continued on page 2



October 2, 2009

Job 315479	Truss FG1	Truss Type SPECIAL	Qty 1	Ply 2	Disosway Custom Residence	I4119355
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Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

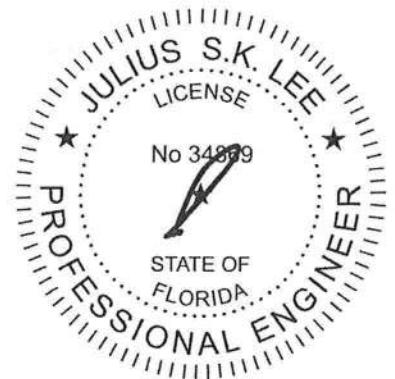
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-14, 6-9=-10

Concentrated Loads (lb)

Vert: 9=-675(B) 10=-675(B) 11=-675(B) 12=-677(B) 13=-677(B) 14=-677(B)



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

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 Boynton, FL 33435

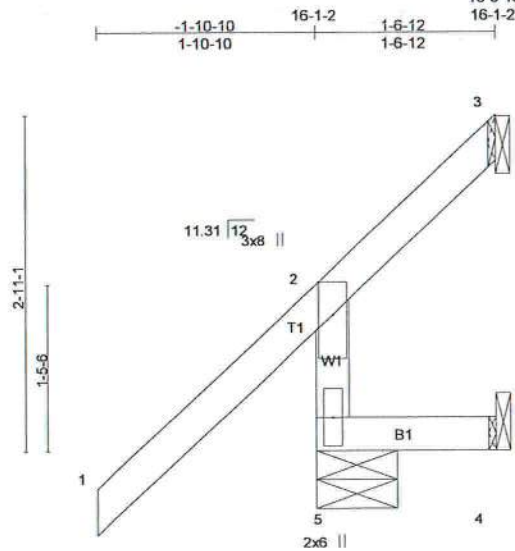
Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence
315479	HJ1	JACK	3	1	

I4119356

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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Scale = 1:19.2

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.53	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.14	Vert(LL) 0.00 5 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) 0.00 5 >999 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) -0.01 3 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.00 4-5 >999 240		
				Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-6-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

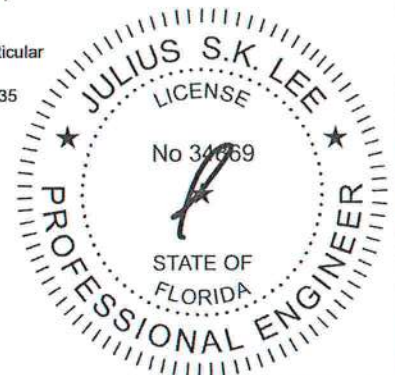
REACTIONS (lb/size) 5=234/0-8-8, 4=13/Mechanical, 3=21/Mechanical
Max Horz 5=249(LC 5)
Max Uplift 5=164(LC 5), 4=56(LC 5), 3=38(LC 6)
Max Grav 5=234(LC 1), 4=15(LC 2), 3=36(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 3 except (jt=lb) 5=164.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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14119357

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October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BC511 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119357
315479	HJ6	MONO TRUSS	1	1	Job Reference (optional)	

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7,140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:34 2009 Page 2

LOAD CASE(S) Standard

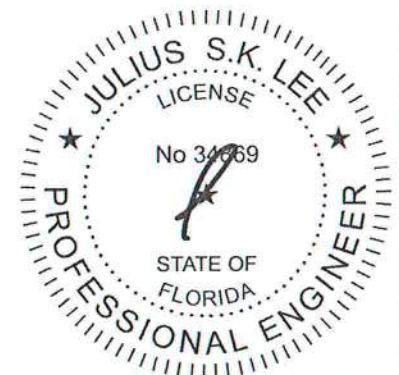
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-36, 2-4=-36, 5-7=-7

Concentrated Loads (lb)

Vert: 8=-153(B) 9=14(F) 10=-16(F) 11=2(B) 12=-202(F=4, B=-206) 13=-237(F=-4, B=-233)



October 2, 2009

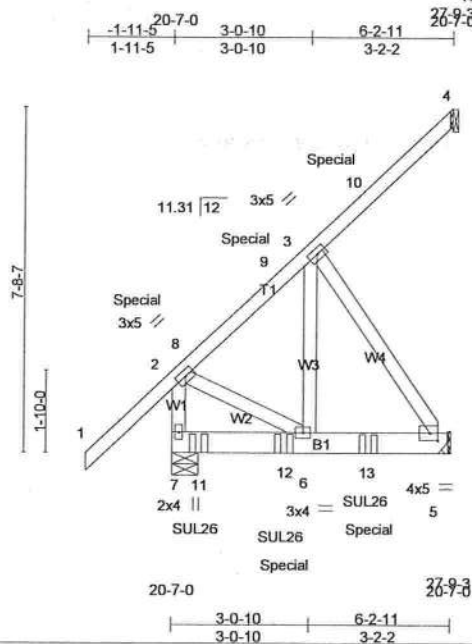
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
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 is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the
 erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component**
Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119358
315479	HJ6A	MONO TRUSS	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:34 2009 Page 1



Scale = 1:49.5

Plate Offsets (X,Y): [5:0-2-2,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.45	Vert(LL)	-0.00	6	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.14	Vert(TL)	-0.01	5-6	>999	240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.20	Horz(TL)	0.00	4	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL)	0.01	5-6	>999	240		Weight: 51 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3 *Except*
 W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=51/Mechanical, 7=534/0-7-1, 5=338/Mechanical
 Max Horz 7=382(LC 5)
 Max Uplift 4=147(LC 5), 7=744(LC 3), 5=787(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

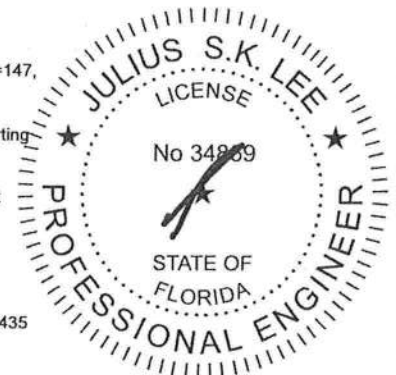
TOP CHORD 2-8=360/601, 8-9=196/412, 3-9=201/346, 2-7=512/715
 BOT CHORD 7-11=366/4, 11-12=366/4, 6-12=366/4, 6-13=426/166, 5-13=426/166
 WEBS 2-6=347/188, 3-6=630/275, 3-5=297/762

NOTES (13-15)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=147, 7=744, 5=787.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Use Simpson Strong-Tie SUL26 (6-10d Girder, 6-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 1-10-10 oc max. starting at 0-7-3 from the left end to 4-4-7 to connect truss(es) MR1 (1 ply 2 X 6 SYP) to front face of bottom chord.
- 10) Fill all nail holes where hanger is in contact with lumber.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 153 lb down and 276 lb up at 0-7-3, and 110 lb up at 2-6-13, and 16 lb down and 183 lb up at 4-5-7 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 15) Use Simpson THJA26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Continued on page 2



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
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 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119358
315479	HJ6A	MONO TRUSS	1	1	Job Reference (optional)	

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LOAD CASE(S) Standard

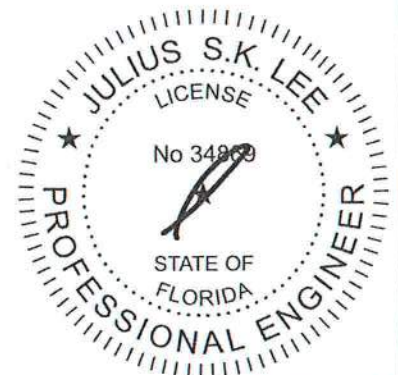
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-36, 2-4=-36, 5-7=-7

Concentrated Loads (lb)

Vert: 8=-153(F) 9=14(B) 10=-16(B) 11=2(F) 12=-202(F=-206, B=4) 13=-237(F=-233, B=4)



October 2, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

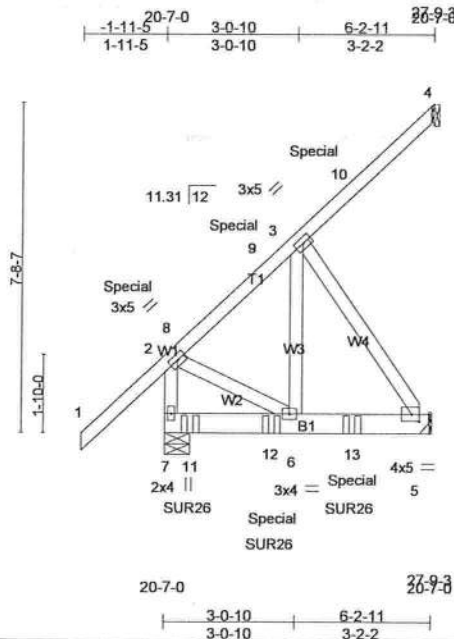
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Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119359
315479	HU6B	MONO TRUSS	1	1		

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:35 2009 Page 1



Scale = 1:51.8

Plate Offsets (X,Y): [5.0-2.2.0-2.0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.45	Vert(LL)	-0.00	6	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.14	Vert(TL)	-0.01	5-6	>999	240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.19	Horz(TL)	0.00	4	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL)	0.01	5-6	>999	240		Weight: 51 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=51/Mechanical, 7=534/0-7-1, 5=338/Mechanical
Max Horz 7=382(LC 5)
Max Uplift 4=149(LC 5), 7=741(LC 3), 5=795(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-8=360/598, 8-9=196/410, 3-9=201/346, 2-7=513/712
BOT CHORD 7-11=366/4, 11-12=366/4, 6-12=366/4, 6-13=431/166, 5-13=431/166
WEBS 2-6=346/188, 3-6=627/275, 3-5=297/772

NOTES (13-15)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 4=149, 7=741, 5=795.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Use Simpson Strong-Tie SUR26 (6-10d Girder, 6-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 1-10-10 oc max. starting at 0-7-3 from the left end to 4-4-7 to connect truss(es) MR1 (1 ply 2 X 6 SYP) to back face of bottom chord.
- 10) Fill all nail holes where hanger is in contact with lumber.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 153 lb down and 274 lb up at 0-7-3, and 108 lb up at 2-6-13, and 16 lb down and 182 lb up at 4-5-7 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 15) Use Simpson THJA26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Continued on page 2



October 2, 2009

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Julius Lee Engineering
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Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119359
315479	HJ6B	MONO TRUSS	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

7,140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:35 2009 Page 2

LOAD CASE(S) Standard

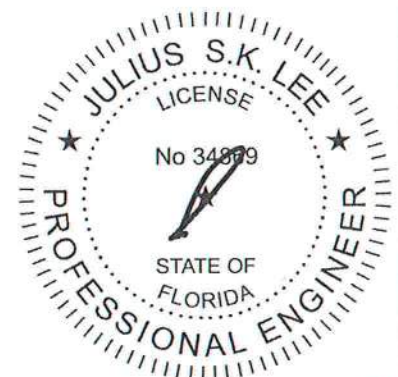
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-36, 2-4=-36, 5-7=-7

Concentrated Loads (lb)

Vert: 8=-153(B) 9=14(F) 10=-16(F) 11=2(B) 12=-202(F=4, B=-206) 13=-237(F=-4, B=-233)



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

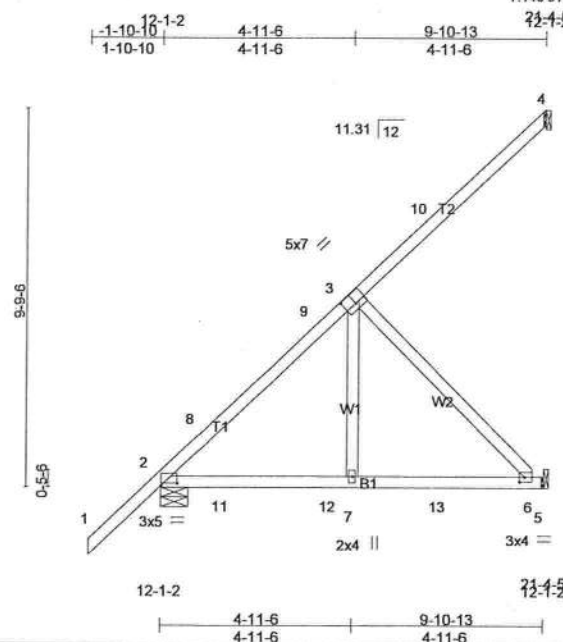
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Boynton, FL 33435

Job 315479	Truss HJ9	Truss Type MONO TRUSS	Qty 1	Ply 1	Disosway Custom Residence	14119360
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Builders FrstSource, Lake City, FL 32055

7.140 s Aug 17 2009 Mitek Industries, Inc. Fri Oct 02 11:55:36 2009 Page 1



Scale = 1:57.4

Plate Offsets (X,Y): [2-0-5-1,0-0-11], [3-0-3-8,0-3-4], [6-0-1-9,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.72	Vert(LL)	-0.04	6-7	>999	360	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.95	Vert(TL)	-0.09	6-7	>999	240	244/190
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.28	Horz(TL)	-0.01	5	n/a	n/a	
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.27	6-7	>421	240	
									Weight: 56 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-11-3 oc bracing.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=117/Mechanical, 2=430/0-8-8, 5=270/Mechanical
Max Horz 2=729(LC 5)
Max Uplift 4=-306(LC 5), 2=-642(LC 3), 5=-969(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-8=-368/866, 8-9=-251/852, 3-9=-246/590
BOT CHORD 2-11=-781/212, 11-12=-781/212, 7-12=-781/212, 7-13=-786/214, 6-13=-786/214
WEBS 3-7=-595/269, 3-6=-303/1115

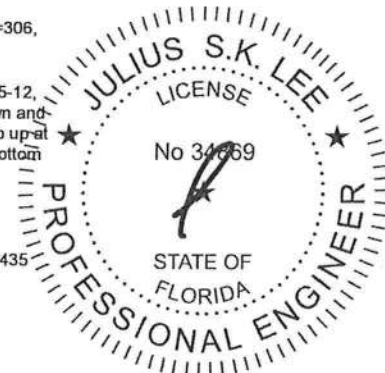
NOTES (10-11)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=306, 2=642, 5=969.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 52 lb down and 22 lb up at 1-5-12, 52 lb down and 22 lb up at 1-5-12, 194 lb up at 4-3-11, 194 lb up at 4-3-11, and 8 lb down and 199 lb up at 7-1-10, and 8 lb down and 199 lb up at 7-1-10 on top chord, and 18 lb down and 16 lb up at 1-5-12, 18 lb down and 16 lb up at 1-5-12, 11 lb down and 18 lb up at 4-3-11, 11 lb down and 18 lb up at 4-3-11, and 65 lb down and 236 lb up at 7-1-10, and 65 lb down and 236 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 2-5=-10

Continued on page 2



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss HJ9	Truss Type MONO TRUSS	Qty 1	Ply 1	Disosway Custom Residence Job Reference (optional)	I4119360
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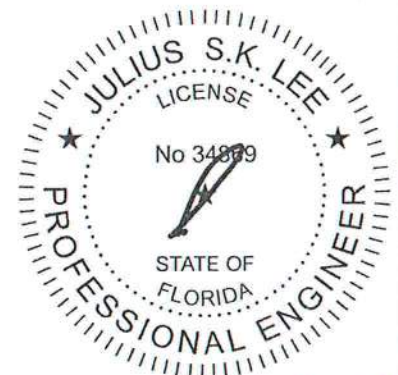
Builders FrstSource, Lake City, FL 32055

7,140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:36 2009 Page 2

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 8=43(F=22, B=22) 9=10(F=5, B=5) 10=-16(F=-8, B=-8) 11=11(F=5, B=5) 12=-7(F=-4, B=-4) 13=-130(F=-65, B=-65)



October 2, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

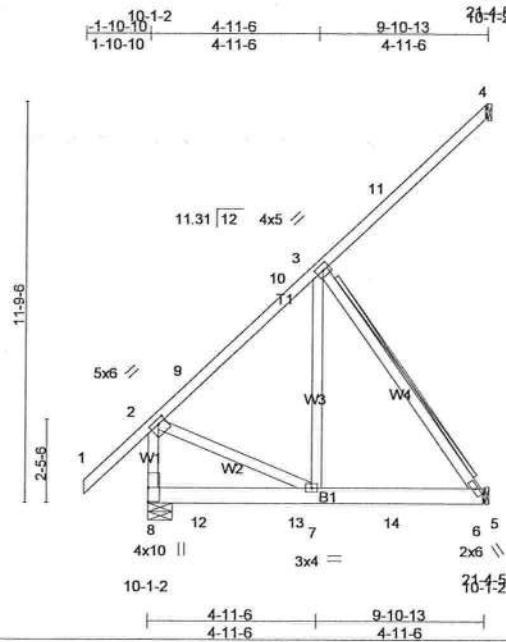
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss HJ9A	Truss Type MONO TRUSS	Qty 1	Ply 1	Disosway Custom Residence	14119361
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Builders FrstSource, Lake City, FL 32055

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Scale = 1:65.2

Plate Offsets (X,Y): [2-0-2-12,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.72	Vert(LL)	-0.01	6-7	>999	360	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.42	Vert(TL)	-0.02	6-7	>999	240	244/190
BCLL 0.0	Rep Stress Incr	NO	WB 0.90	Horz(TL)	0.01	4	n/a	n/a	
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.07	6-7	>999	240	
									Weight: 79 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-6
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=105/Mechanical, 8=439/0-8-9, 6=323/Mechanical
Max Horz 8=803(LC 5)
Max Uplift 4=274(LC 5), 8=1538(LC 3), 6=1567(LC 5)

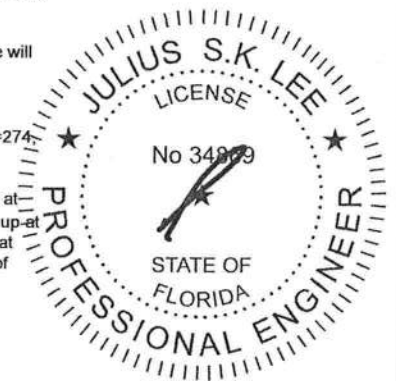
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-9=298/1093, 9-10=183/908, 3-10=155/693, 2-8=447/1146
BOT CHORD 8-12=780/0, 12-13=780/0, 7-13=780/0, 7-14=935/172, 6-14=935/172
WEBS 3-7=1193/213, 3-6=295/1599, 2-7=680/210

NOTES (11-12)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=274, 8=1538, 6=1567.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 135 lb up at 1-5-11, 135 lb up at 1-5-12, 21 lb down and 243 lb up at 4-3-10, 0 lb down and 99 lb up at 4-3-11, and 165 lb up at 7-1-10, and 2 lb down and 188 lb up at 7-1-10 on top chord, and 314 lb up at 1-5-11, 314 lb up at 1-5-12, 12 lb down and 189 lb up at 4-3-10, 12 lb down and 247 lb up at 4-3-11, and 95 lb down and 442 lb up at 7-1-10, and 79 lb down and 466 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Continued on page 2
LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119361
315479	HJ9A	MONO TRUSS	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

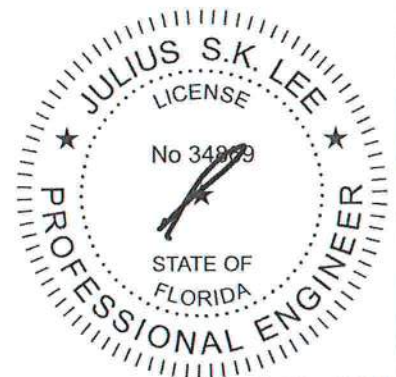
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54, 2-4=-54, 6-8=-10

Concentrated Loads (lb)

Vert: 9=47(F=23, B=23) 10=-22(F=-0, B=-21) 11=1(F=-2, B=4) 12=18(F=9, B=9) 13=8(F=-4, B=-4) 14=-173(F=-79, B=-95)



October 2, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

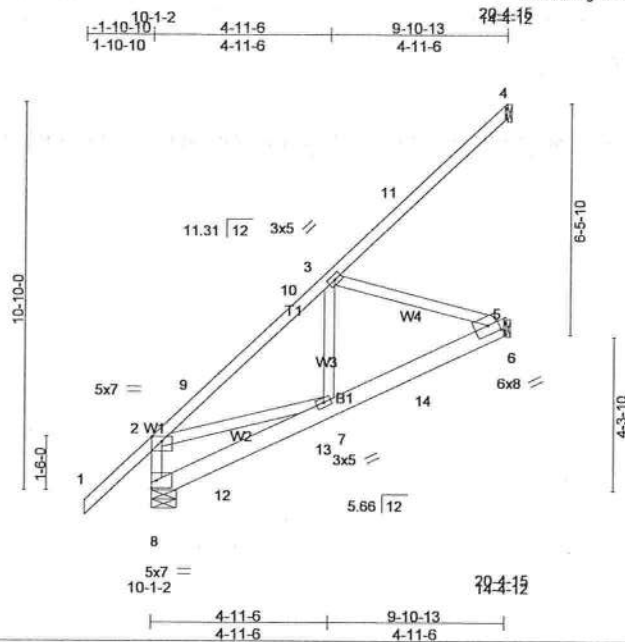
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disoway Custom Residence	I4119362
315479	HJ9D	SPECIAL	2	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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Scale = 1:62.1

Plate Offsets (X,Y): [6-0-3-8,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.93	Vert(LL)	-0.01	6-7	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.52	Vert(TL)	-0.03	6-7	>999	240		
BCLL 0.0	Rep Stress Incr	NO	WB 0.57	Horz(TL)	0.02	4	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.12	6-7	>999	240		Weight: 71 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-8-12 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=425/0-8-8, 4=111/Mechanical, 5=294/Mechanical
Max Horz 8=935(LC 5)
Max Uplift 8=1353(LC 3), 4=297(LC 5), 5=1649(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

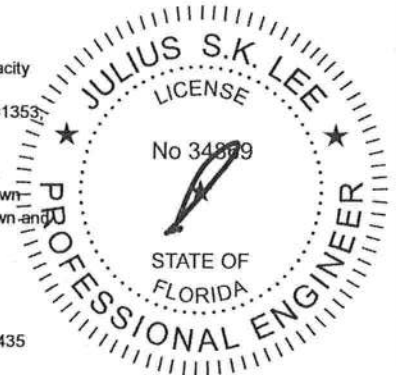
TOP CHORD 2-8=416/1331, 2-9=465/1944, 9-10=342/1850, 3-10=336/1700
BOT CHORD 8-12=835/0, 12-13=979/6, 7-13=1204/24, 7-14=1699/282, 6-14=2066/352,
5-6=704/125
WEBS 2-7=1330/334, 3-7=1231/202, 3-6=309/1828

NOTES (11-12)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=1353, 4=297, 5=1649.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 68 lb up at 1-5-12, 68 lb up at 1-5-12, 0 lb down and 152 lb up at 4-3-11, 0 lb down and 152 lb up at 4-3-11, and 6 lb down and 198 lb up at 7-1-10, and 6 lb down and 198 lb up at 7-1-10 on top chord, and 171 lb up at 1-5-12, 171 lb up at 1-5-12, 12 lb down and 272 lb up at 4-3-11, 12 lb down and 272 lb up at 4-3-11, and 72 lb down and 437 lb up at 7-1-10, and 72 lb down and 437 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119362
315479	HJ9D	SPECIAL	2	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

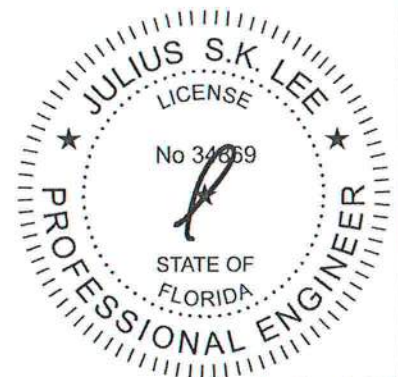
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54, 2-4=-54, 5-8=-10

Concentrated Loads (lb)

Vert: 9=46(F=23, B=23) 10=-0(F=-0, B=-0) 11=-12(F=-6, B=-6) 12=19(F=9, B=9) 13=-8(F=-4, B=-4) 14=-144(F=-72, B=-72)



October 2, 2009

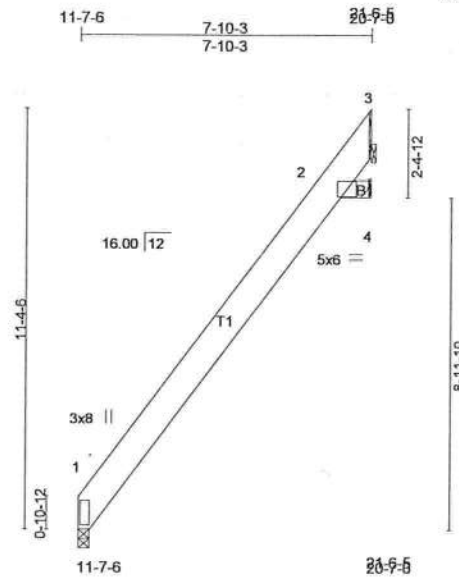
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119363
315479	MR1	SPECIAL	3	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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Scale = 1:59.9

Plate Offsets (X,Y): [2:0-5-1,0-0-4], [2:6-11-9,6-8-10]

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	-0.02	1-2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.00	Vert(TL)	-0.05	1-2	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.05	4	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.12	1-2	>794	240		Weight: 56 lb

LUMBER

TOP CHORD 2 X 10 SYP No.2
BOT CHORD 2 X 6 SYP No.1D

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

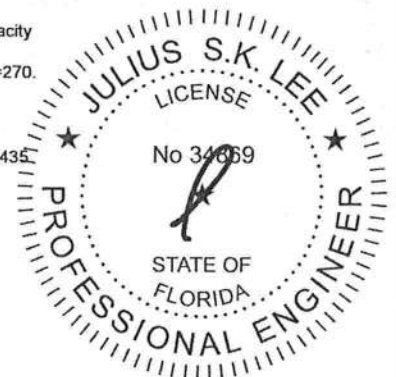
REACTIONS (lb/size) 1=183/0-3-8, 4=3/Mechanical, 3=177/Mechanical
Max Horz 1=301(LC 6)
Max Uplift 3=-270(LC 6)
Max Grav 1=183(LC 1), 4=8(LC 2), 3=177(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-493/88, 2-3=-465/153

NOTES (10-12)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=270.
- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435
- 12) Use Simpson SURIL26 to attach Truss to Carrying member

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

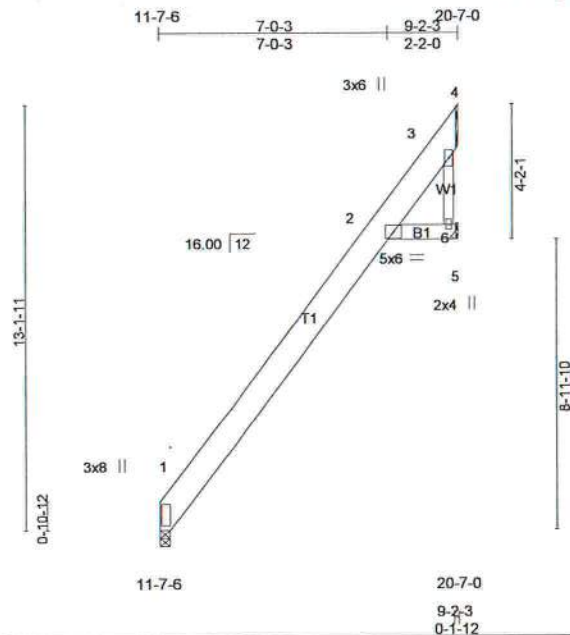
Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence
315478	MR2	SPECIAL	3	1	

I4119364

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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Scale = 1:68.3

Plate Offsets (X,Y): [3:8-10-11,8-7-2]

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.35	Vert(LL)	-0.04	1-2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.10	Vert(TL)	-0.08	1-2	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.15	6	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.18	1-2	>576	240		
									Weight: 71 lb	

LUMBER

TOP CHORD 2 X 10 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

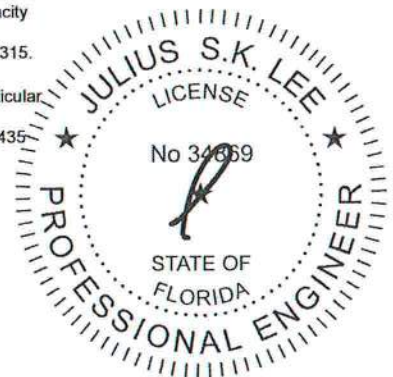
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=204/0-3-8, 6=210/Mechanical
 Max Horz 1=344(LC 6)
 Max Uplift 6=315(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=601/67, 2-3=442/113, 3-6=166/545

NOTES (9-11)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=315.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 11) Use Simpson SURIL26 to attach Truss to Carrying member

LOAD CASE(S) Standard

October 2, 2009

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.**

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component

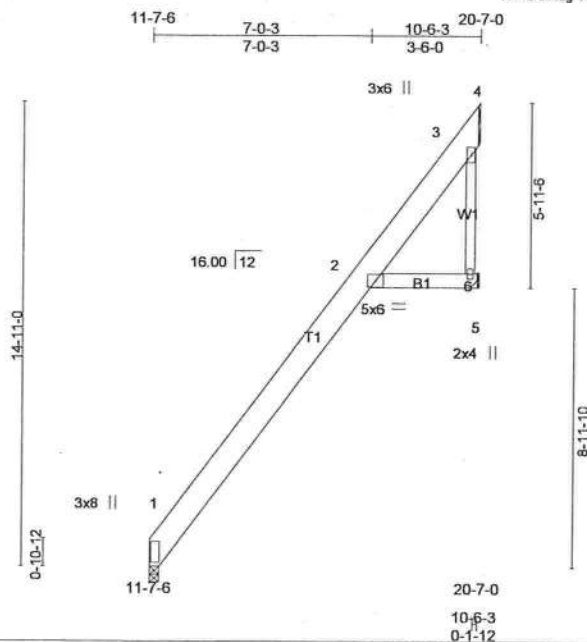
Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 315479	Truss MR3	Truss Type SPECIAL	Qty 3	Ply 1	Disosway Custom Residence	14119365
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Builders FrstSource, Lake City, FL 32055

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Scale = 1:71.2

Plate Offsets (X,Y): [3:10-2-11,9-11-2]

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.47	Vert(LL)	-0.06	1-2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.19	Vert(TL)	-0.13	1-2	>938	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.34	6	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.32	1-2	>382	240		
									Weight: 86 lb	

LUMBER

TOP CHORD 2 X 10 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

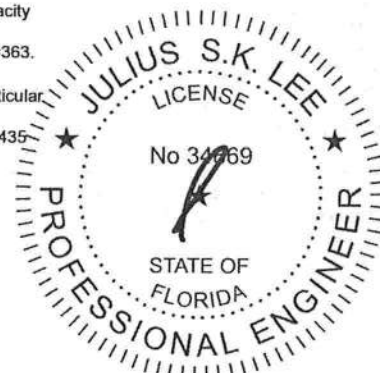
REACTIONS (lb/size) 1=234/0-3-8, 6=237/Mechanical
Max Horz 1=396(LC 6)
Max Uplift 6=363(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-686/63, 2-3=-525/129, 3-6=-191/643

NOTES (9-11)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=363.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 11) Use Simpson SURIL26 to attach Truss to Carrying member

LOAD CASE(S) Standard



October 2, 2009

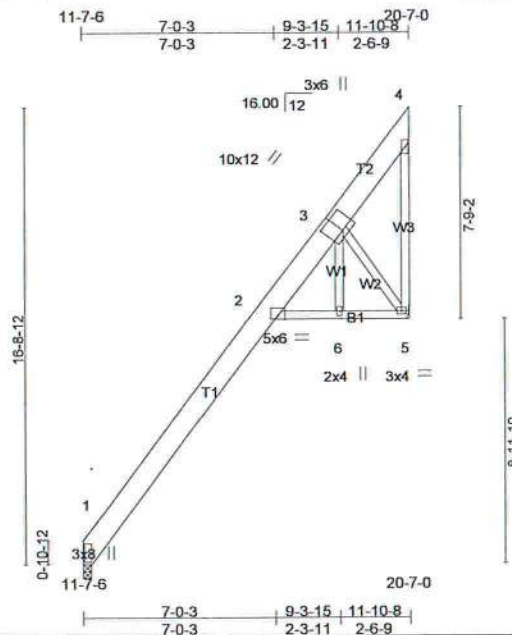
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disoway Custom Residence
315479	MR4	SPECIAL	21	1	14119366

Builders FirstSource, Lake City, FL 32055

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Scale = 1:80.9

Plate Offsets (X,Y): [3:0-6-0,0-7-8], [4:11-8-12,11-5-4]

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.61	Vert(LL)	-0.09	1-2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.15	Vert(TL)	-0.19	1-2	>735	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.24	Horz(TL)	-0.54	5	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.47	1-2	>298	240		
									Weight: 107 lb	

LUMBER

TOP CHORD 2 X 10 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-1-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-10-9 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=250/Mechanical, 1=267/0-3-8
Max Horz 1=440(LC 6)
Max Uplift 5=392(LC 6)

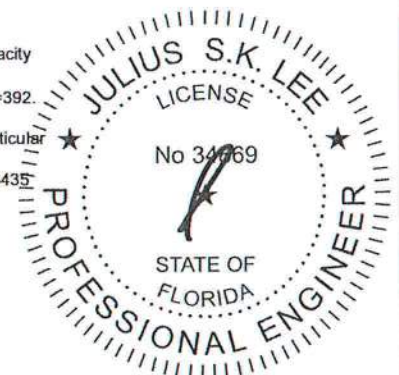
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-716/52, 2-3=-298/17
BOT CHORD 2-6=-598/198, 5-6=-600/200
WEBS 3-5=-326/976

NOTES (9-12)

- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=392.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member.
- Use with THJA26 when in junction with Diagonal Hip member.

LOAD CASE(S) Standard



October 2, 2009

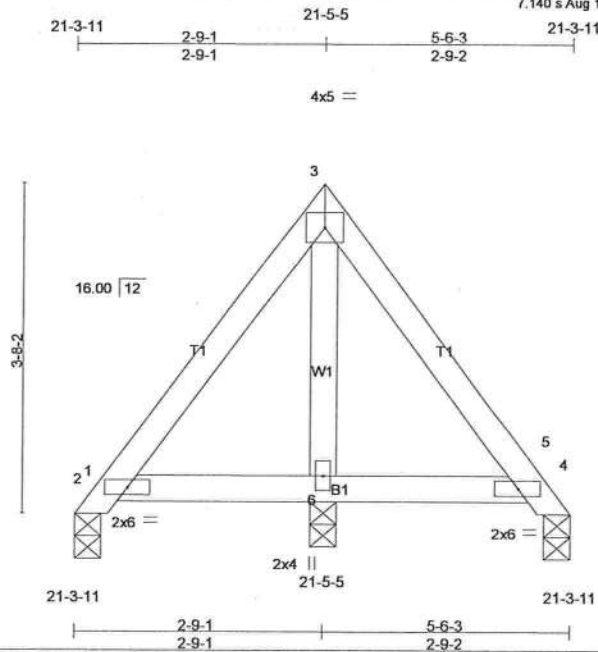
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119367
315479	PB1	PIGGYBACK	13	1		

Builders FrstSource, Lake City, FL 32055

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Scale = 1:24.7

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.08	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.11	Vert(LL) -0.00 2 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.09	Vert(TL) -0.00 2 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.01 5 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.00 2-6 >999 240		
				Weight: 25 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-6-3 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=37/0-3-8, 5=37/0-3-8, 6=263/0-3-8
 Max Horz 1=141(LC 5)
 Max Uplift 1=-43(LC 4), 5=-1(LC 4), 6=-179(LC 6)
 Max Grav 1=71(LC 5), 5=52(LC 11), 6=263(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-6=-205/347

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 6=179.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

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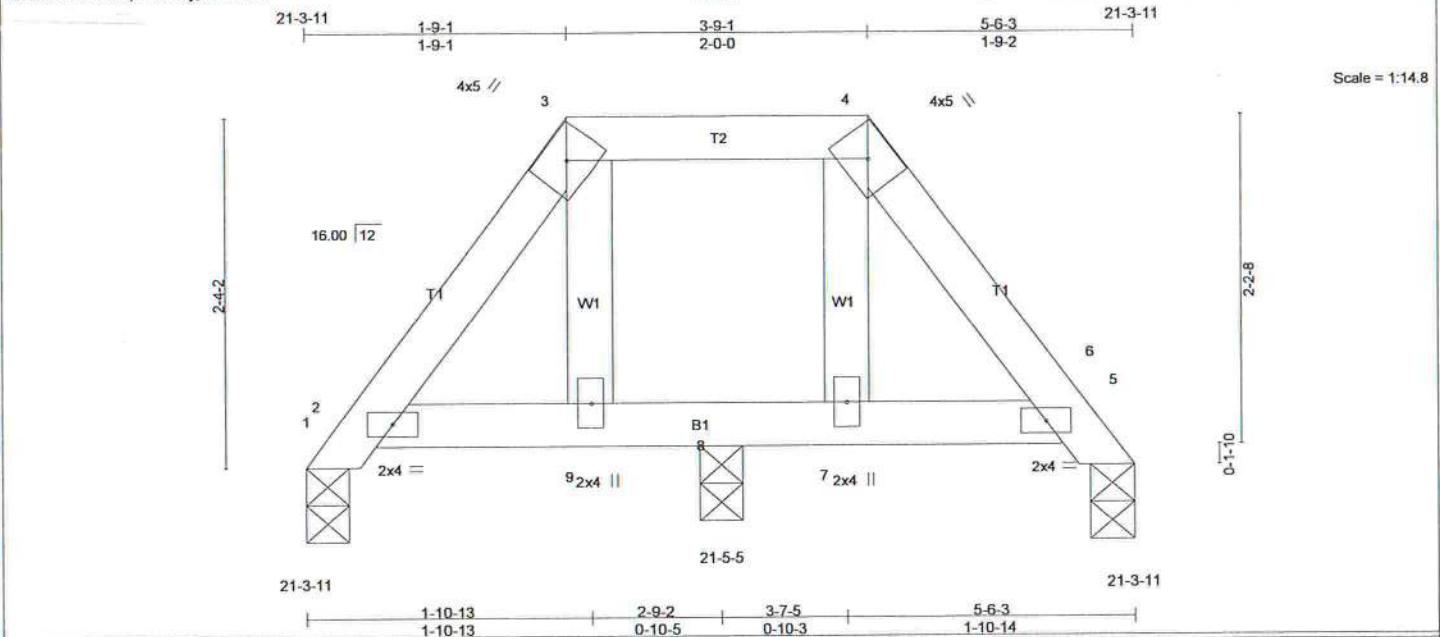
Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 315479	Truss PB2	Truss Type HIP CAP	Qty 1	Ply 1	Disosway Custom Residence	I4119368
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Builders FirstSource, Lake City, FL 32055

21-5-5

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.05	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.09	Vert(LL) -0.00 2 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.02	Vert(TL) -0.00 9 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 6 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.00 9 >999 240		
				Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5-6-3 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

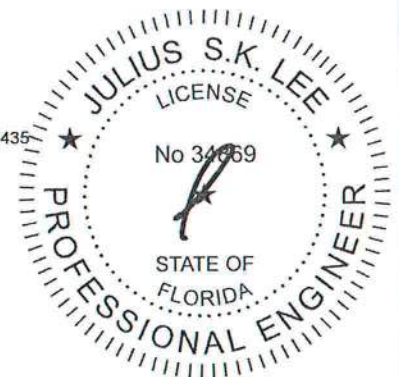
REACTIONS (lb/size) 1=99/0-3-8, 6=99/0-3-8, 8=139/0-3-8
Max Horz 1=89(LC 5)
Max Uplift 1=30(LC 4), 6=38(LC 4), 8=61(LC 5)
Max Grav 1=101(LC 10), 6=101(LC 11), 8=139(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (11-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6, 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.
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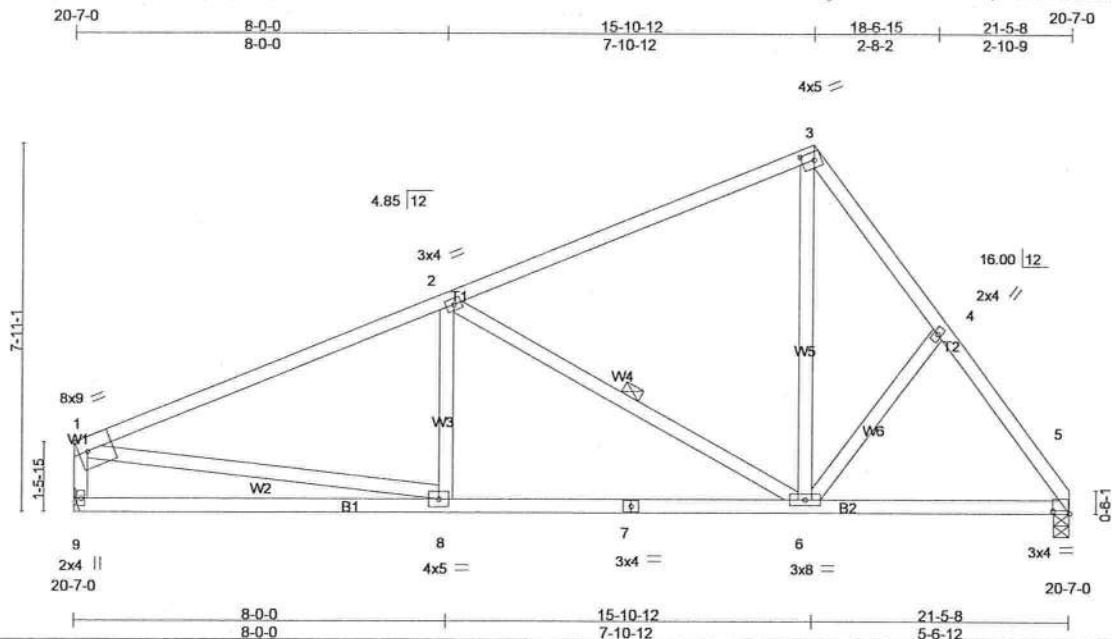
Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	14119369
315479	T01	DUAL PITCH	10	1		

Builders FrstSource, Lake City, FL 32055

Job Reference (optional)

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Scale: 1/4"=1'

Plate Offsets (X,Y): [1:Edge,0-3-8], [3:0-3-3,0-2-0], [5:0-4-4,0-0-10]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.58	Vert(LL)	-0.08	8-9	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.33	Vert(TL)	-0.15	8-9	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.55	Horz(TL)	0.02	5	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.06	6-8	>999	240	Weight: 121 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-9-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-11-14 oc bracing.
WEBS 1 Row at midpt 2-6

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=677/0-4-0, 9=677/Mechanical
Max Horz 9=282(LC 5)
Max Uplift 5=251(LC 6), 9=275(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=980/787, 2-3=504/490, 3-4=668/704, 4-5=759/611, 1-9=639/572
BOT CHORD 8-9=332/130, 7-8=619/845, 6-7=619/845, 5-6=241/404
WEBS 2-6=542/598, 3-6=459/390, 1-8=501/724

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=251, 9=275.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T01G	Truss Type GABLE	Qty 1	Ply 1	Disosway Custom Residence	14119370
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Aug 17 2009 Mitek Industries, Inc. Fri Oct 02 11:55:41 2009 Page 1

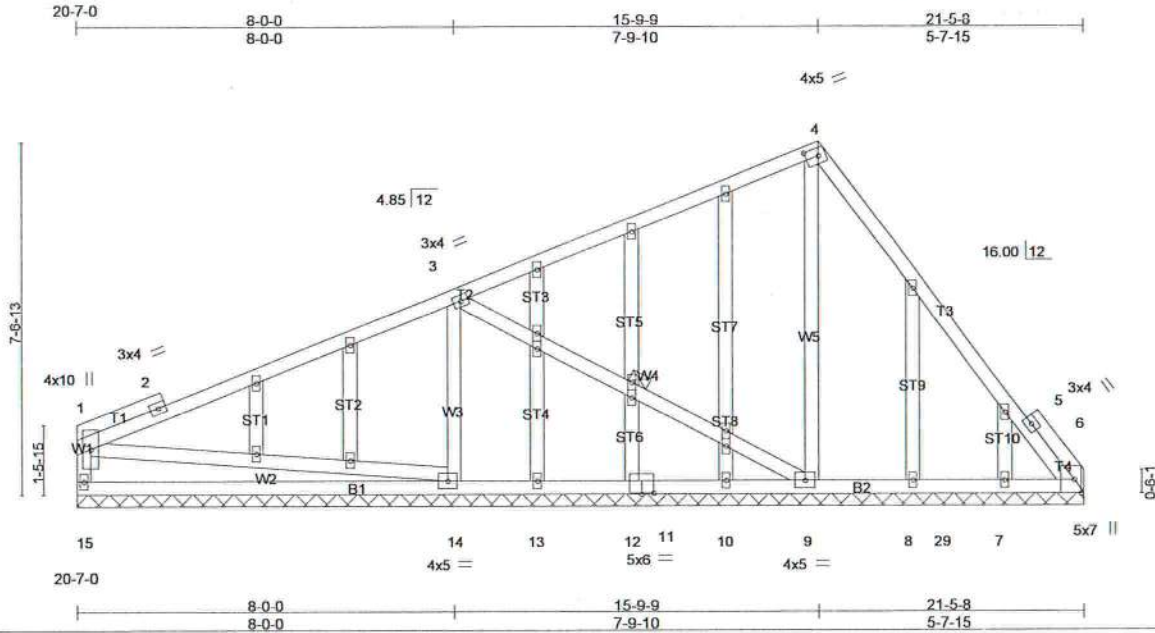


Plate Offsets (X,Y): [4:0-3:3,0-2,0], [6:0-3:8,Edge], [11:0-3:0,0-0,4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.47	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.54	Vert(TL)	n/a	-	n/a		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.45	Horz(TL)	0.01	6	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)						
								Weight: 156 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1: 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
9-1-3 oc bracing: 14-15.
1 Row at midpt 3-9

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings Mechanical.
(lb) - Max Horz 15=371(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 15, 13, 8 except 6=181(LC 5).
14=434(LC 5), 9=963(LC 7), 7=310(LC 7)
Max Grav All reactions 250 lb or less at joint(s) 15, 10, 12, 8, 7 except 6=569(LC 7), 14=621(LC 10), 9=578(LC 1)

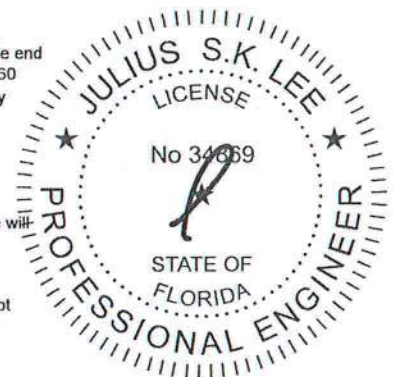
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=265/175, 4-5=591/295, 5-6=631/208
BOT CHORD 14-15=482/197, 13-14=175/374, 12-13=175/374, 11-12=175/374, 10-11=175/374,
9-10=175/374, 8-9=180/603, 8-29=180/603, 7-29=180/603, 6-7=180/603
WEBS 3-14=502/442, 3-9=75/281, 4-9=489/825, 1-14=272/501

NOTES (13-14)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 13, 8 except (jt=lb) 6=181, 14=434, 9=963, 7=310.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119370
315479	T01G	GABLE	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard



October 2, 2009

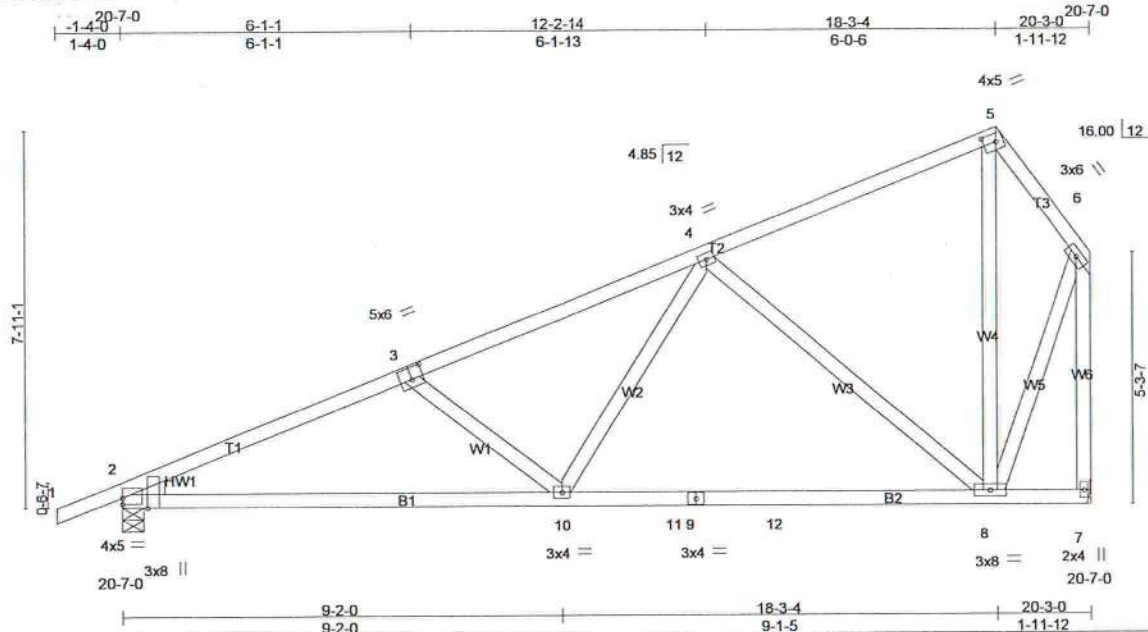
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 fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component**
Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

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 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119371
315479	T02	DUAL PITCH	3	1	Job Reference (optional)	

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Scale = 1:46.4

Plate Offsets (X,Y): [2:0-0-0,0-1-9], [2:0-2-11,Edge], [3:0-3-0,0-3-0], [5:0-3-3,0-2-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.54	Vert(LL)	-0.13	2-10	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.57	Vert(TL)	-0.25	2-10	>938	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.70	Horz(TL)	0.03	7	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.07	2-10	>999	240		
									Weight: 120 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W6: 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

REACTIONS (lb/size) 2=755/0-5-8, 7=685/Mechanical
Max Horz 2=310(LC 5)
Max Uplift 2=342(LC 6), 7=296(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1256/846, 3-4=-999/684, 4-5=-267/216, 5-6=-313/290, 6-7=-738/553
BOT CHORD 2-10=-1045/1087, 10-11=-609/666, 9-11=-609/666, 9-12=-609/666, 8-12=-609/666
WEBS 3-10=-286/410, 4-10=-243/400, 4-8=-641/653, 6-8=-315/509

NOTES (9-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=342, 7=296.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

BRACING

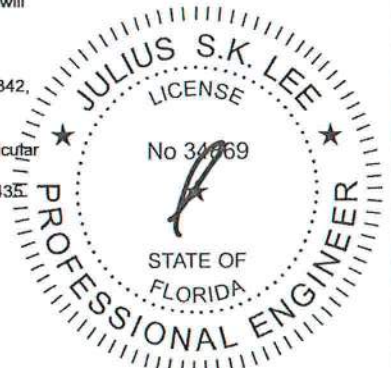
TOP CHORD

Structural wood sheathing directly applied or 5-4-15 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



October 2, 2009



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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119372
315479	T03	DUAL PITCH	3	1		

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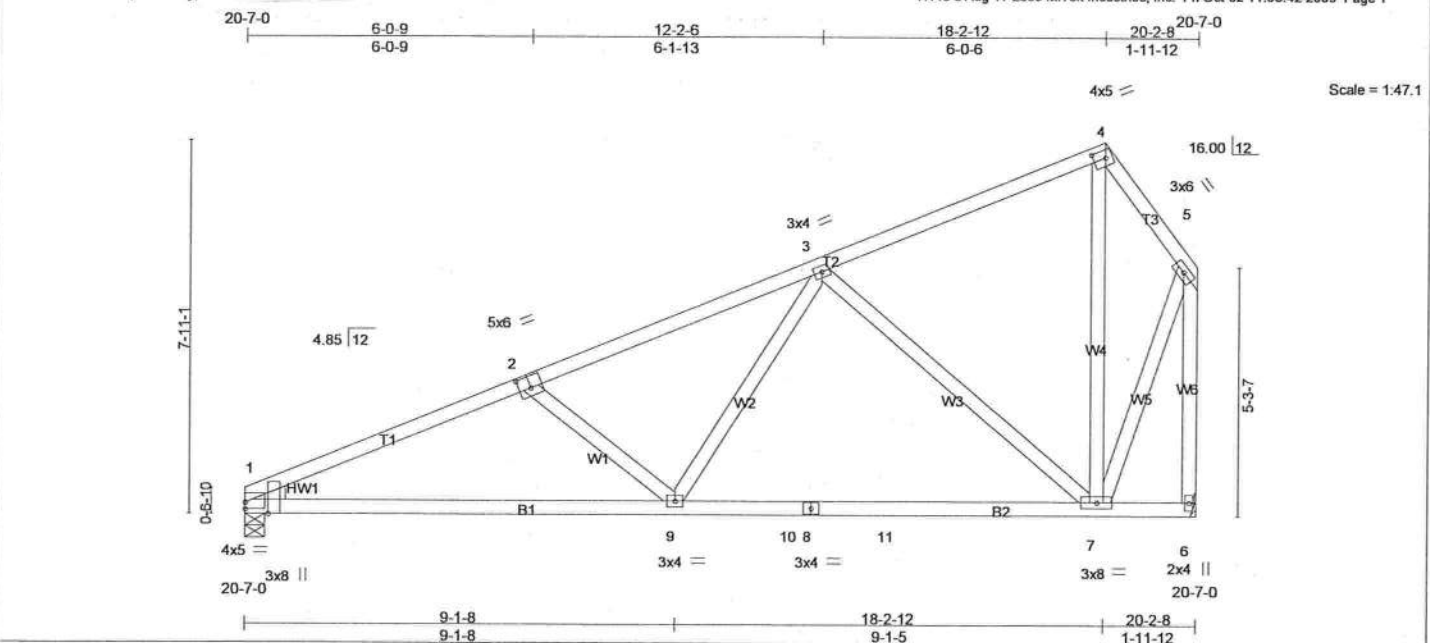


Plate Offsets (X,Y): [1:0-2-14,Edge], [1:0-0-0,0-1-11], [2:0-3-0,0-3-0], [4:0-3-3,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.46	Vert(LL)	-0.14	1-9	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.49	Vert(TL)	-0.27	1-9	>892	240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.70	Horz(TL)	0.03	6	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(LL)	0.08	1-9	>999	240		
	Code FBC2007/TPI2002							Weight: 118 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3 *Except*
 W6: 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-1 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

(lb/size) 6=687/Mechanical, 1=667/0-5-0
 Max Horz 1=302(LC 5)
 Max Uplift 6=300(LC 6), 1=245(LC 6)

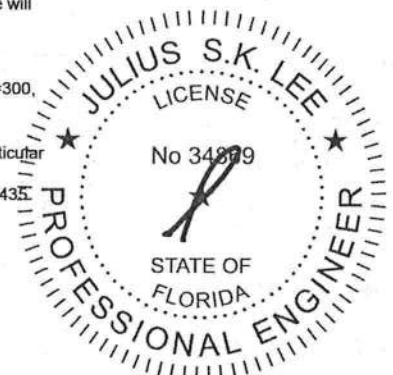
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=1258/881, 2-3=1010/709, 3-4=268/217, 4-5=314/292, 5-6=740/557
 BOT CHORD 1-9=1085/1105, 9-10=617/669, 8-10=617/669, 8-11=617/669, 7-11=617/669
 WEBS 2-9=297/436, 3-9=267/411, 3-7=645/662, 5-7=319/510

NOTES (9-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=300, 1=245.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



October 2, 2009

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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	14119373
315479	T04	HIP	1	2	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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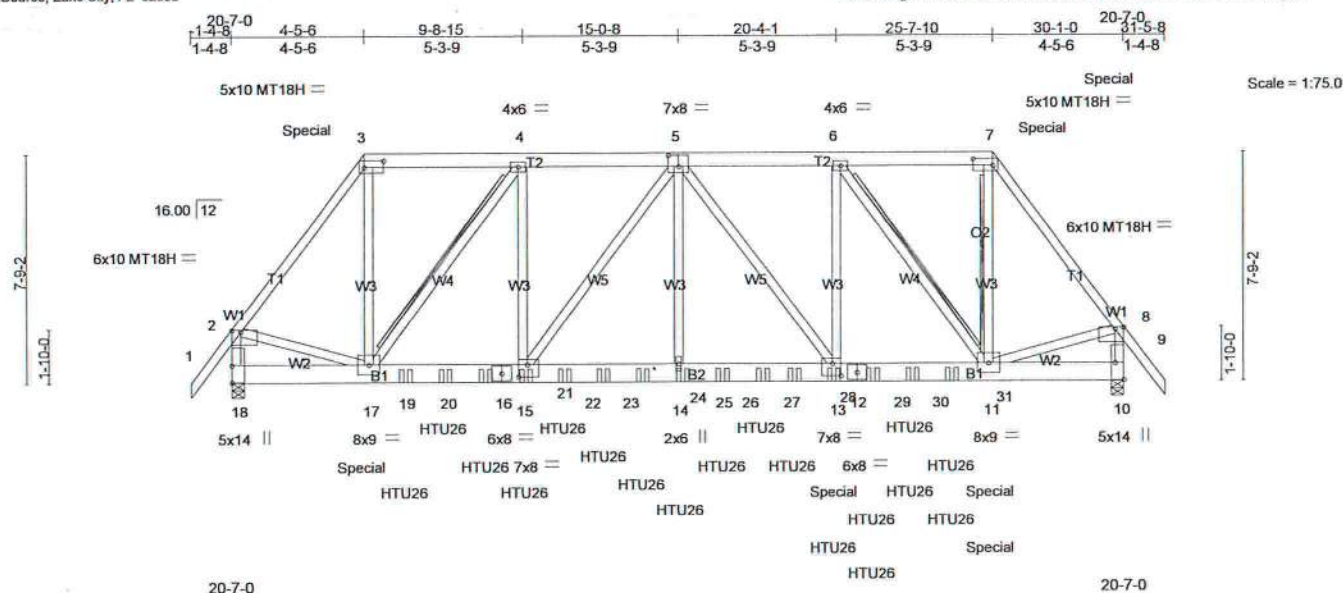


Plate Offsets (X,Y): [2:0-3-8,0-0-12], [3:0-8-0,0-2-12], [5:0-4-0,0-4-8], [7:0-8-0,0-2-12], [8:0-3-8,0-0-12], [10:Edge,0-3-8], [13:0-3-8,0-5-0], [15:0-3-8,0-5-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.66	Vert(LL)	-0.12 13-14	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.41	Vert(TL)	-0.22 13-14	>999	240	MT18H	244/190
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.95	Horz(TL)	-0.05 10	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.26 13-14	>999	240	Weight: 575 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*
T2: 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-5-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-13 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-17, 6-11, 7-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 18=4549/0-5-0, 10=6107/0-5-0
Max Horz 18=315(LC 4)
Max Uplift 18=5566(LC 4), 10=7520(LC 3)
Max Grav 18=4550(LC 9), 10=6107(LC 1)

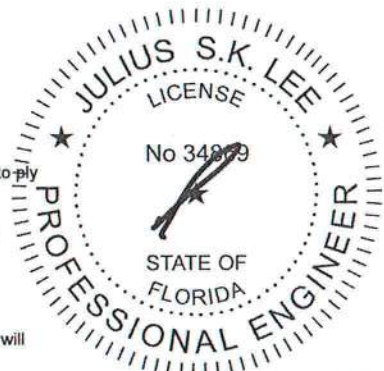
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=4470/5629, 3-4=2688/3548, 4-5=4901/6047, 5-6=6309/7290, 6-7=3677/4783,
7-8=6075/7641, 2-18=4556/5607, 8-10=6158/7614

BOT CHORD 17-18=391/316, 17-19=6063/4901, 19-20=6063/4901, 20-21=6063/4901,
16-21=6063/4901, 15-16=6063/4901, 15-22=7273/6120, 22-23=7273/6120,
23-24=7273/6120, 14-24=7273/6120, 14-25=7273/6120, 25-26=7273/6120,
26-27=7273/6120, 27-28=7273/6120, 13-28=7273/6120, 12-13=7269/6309,
12-29=7269/6309, 29-30=7269/6309, 30-31=7269/6309, 11-31=7269/6309
WEBS 3-17=4241/3279, 4-17=3831/4433, 4-15=3363/2806, 5-15=2075/2150,
5-14=1448/1142, 5-13=122/331, 6-13=3403/3418, 6-11=4537/4445, 7-11=5677/4522,
2-17=3500/2698, 8-11=4813/3725

NOTES (17-18)

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2.

Continued on page 2



October 2, 2009

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119373
315479	T04	HIP	1	2	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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NOTES (17-18)

- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 18=5566, 10=7520.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 12) Use Simpson Strong-Tie HTU26 (20-10dx1 1/2 Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 1-4-0 oc max. starting at 21-6-14 from the left end to 24-2-14 to connect truss(es) MR4 (1 ply 2 X 4 SYP) to front face of bottom chord.
- 13) Use Simpson Strong-Tie HTU26 (20-10dx1 1/2 Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 1-4-0 oc max. starting at 5-10-2 from the left end to 24-2-14 to connect truss(es) MR4 (1 ply 2 X 4 SYP) to back face of bottom chord.
- 14) Fill all nail holes where hanger is in contact with lumber.
- 15) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 130 lb up at 4-5-6, and 37 lb down and 163 lb up at 25-7-10, and 37 lb down and 165 lb up at 25-7-10 on top chord, and 567 lb down and 1195 lb up at 4-5-6, and 567 lb down and 1203 lb up at 25-6-14, and 567 lb down and 1195 lb up at 25-6-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 16) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 17) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 18) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54, 2-3=-54, 3-7=-54, 7-8=-54, 8-9=-54, 18-20=-10, 15-20=-50, 15-23=-10, 23-26=-50, 13-26=-10, 13-30=-50, 10-30=-10

Concentrated Loads (lb)

Vert: 3=3(B) 7=6(F=3, B=3) 17=-567(B) 15=-240(B) 14=-240(B) 13=-2419(F=-2179, B=-240) 11=-1134(F=-567, B=-567) 19=-240(B) 20=-240(B) 21=-240(B) 22=-240(B) 23=-240(B) 24=-240(B) 25=-240(B) 27=-240(B) 28=-240(B) 29=-479(F=-240, B=-240) 30=-479(F=-240, B=-240) 31=-479(F=-240, B=-240)



October 2, 2009

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 1109 Coastal Bay Blvd.
 Boynton, FL 33435

The seal is circular with a double-lined border. The outer ring contains the text "JULIUS S.K. LEE" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by two stars. The inner circle contains the word "LICENSE" at the top, the license number "No 34879" in the center, and "STATE OF FLORIDA" at the bottom. A signature is written across the center of the seal.

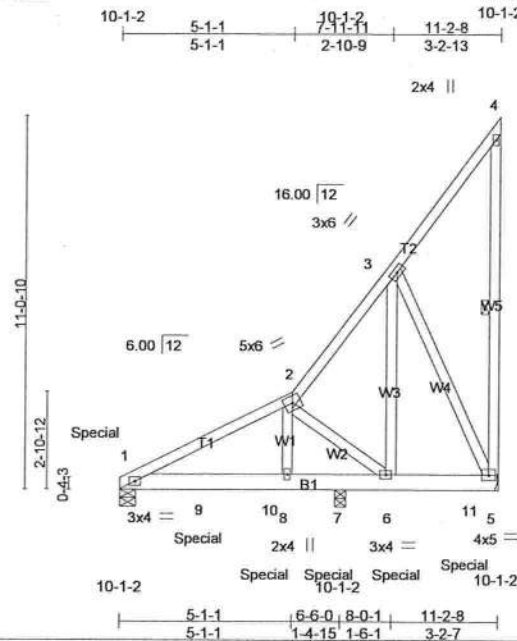
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-T473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BC311 Building Component Safety Information**, available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence
315479	T05A	PORCH TRUSS	1	1	I4119375

Builders FrstSource, Lake City, FL 32055

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Scale = 1:65.7

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.18	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.46	Vert(LL) -0.03 1-8 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.37	Vert(TL) -0.05 1-8 >999 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) -0.01 5 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.05 1-8 >999 240		
				Weight: 93 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9'-0-15 oc bracing.
WEBS 1 Row at midpt 4-5

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=612/0-5-8, 5=974/Mechanical, 7=1229/0-4-0
Max Horz 1=450(LC 5)
Max Uplift 1=404(LC 3), 5=708(LC 5), 7=948(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=661/623, 2-3=387/134
BOT CHORD 1-9=705/537, 9-10=705/537, 8-10=705/537, 7-8=671/532, 6-7=671/532
WEBS 2-8=572/148, 2-6=441/648, 3-6=211/474, 3-5=437/396

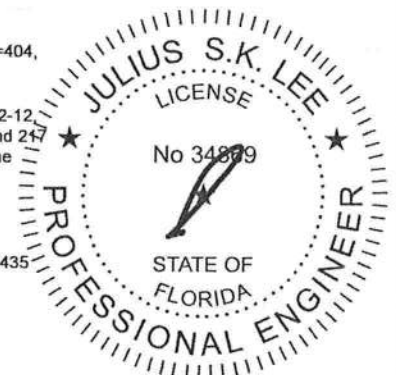
NOTES (10-12)

- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6-0 tall by 2'-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 1=404, 5=708, 7=948.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 79 lb down and 26 lb up at 0-2-12, 140 lb down and 23 lb up at 2-5-1, 377 lb down and 863 lb up at 4-4-12, 359 lb down and 333 lb up at 6-3-4, and 583 lb down and 241 lb up at 8-3-4, and 583 lb down and 217 lb up at 10-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T05A	Truss Type PORCH TRUSS	Qty 1	Ply 1	Disosway Custom Residence Job Reference (optional)	I4119375
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Builders FirstSource, Lake City, FL 32055

7,140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:46 2009 Page 2

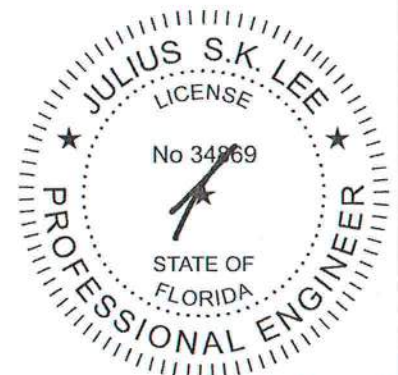
LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-2=-54, 2-4=-54, 1-5=-10

Concentrated Loads (lb)

Vert: 1=-79(F) 6=-583(F) 7=-359(F) 9=-140(F) 10=-377(F) 11=-583(F)



October 2, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

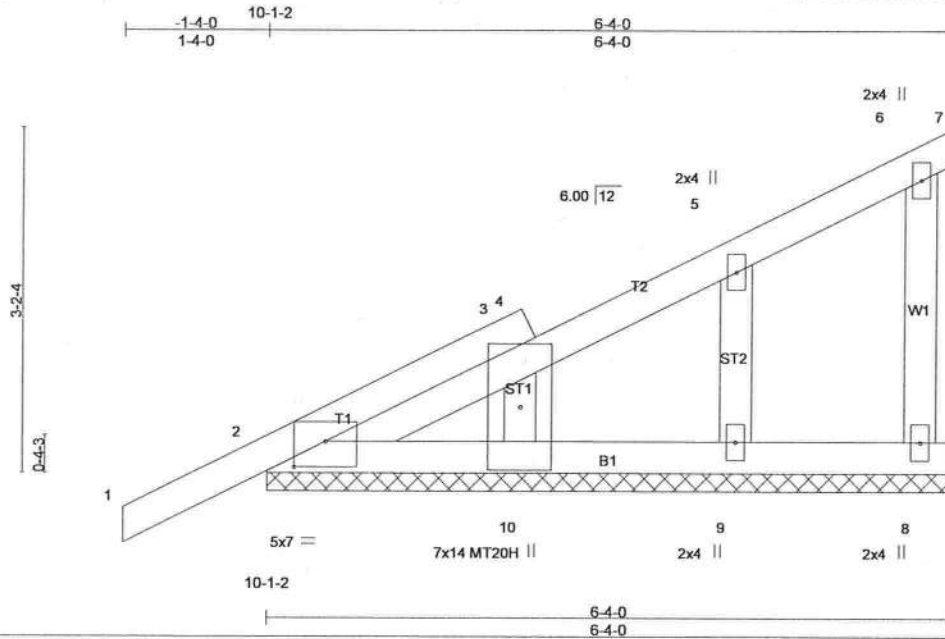
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119376
315479	T05G	GABLE	1	1	Job Reference (optional)	

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Scale = 1:20.5

Plate Offsets (X,Y): [2-0-3-8,0-2-13]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.16	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.03	Vert(TL)	-0.00	1	n/r	90	MT20H	187/143
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.05	Horz(TL)	-0.00	7	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)							
									Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

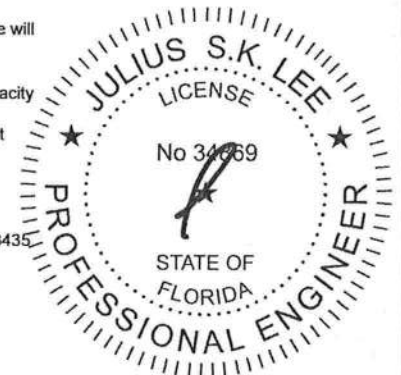
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 6-4-0.
(lb) - Max Horz 2=234(LC 6)
Max Uplift All uplift 100 lb or less at joint(s) 7, 8, 10 except 2=130(LC 6), 9=123(LC 6)
Max Grav All reactions 250 lb or less at joint(s) 2, 7, 8, 9, 10

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (12-13)
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - 3) All plates are MT20 plates unless otherwise indicated.
 - 4) Gable requires continuous bottom chord bearing.
 - 5) Gable studs spaced at 2-0-0 oc.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) All bearings are assumed to be SYP No.2.
 - 9) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 10 except (jt=lb) 2=130, 9=123.
 - 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T06	Truss Type SPECIAL	Qty 1	Ply 1	Disoway Custom Residence	14119377
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Builders FirstSource, Lake City, FL 32055

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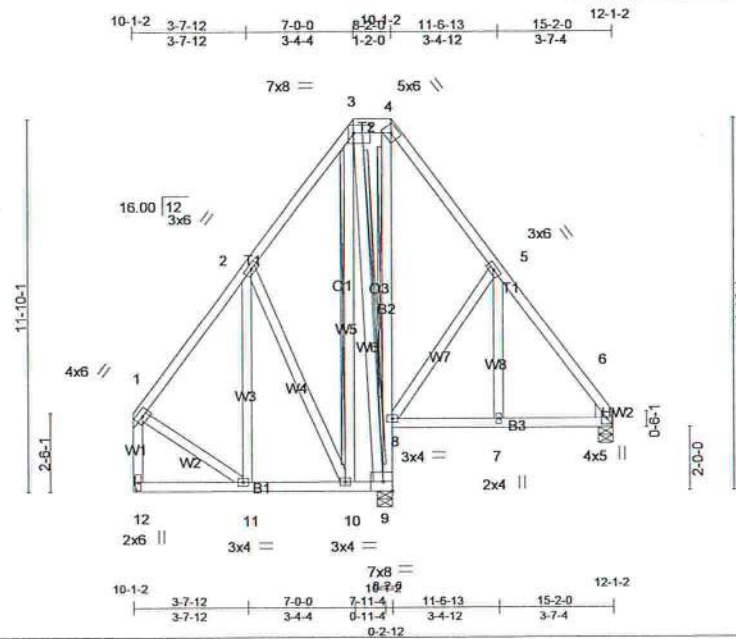


Plate Offsets (X,Y): [3-0-6,4-0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.26	Vert(LL)	-0.02 10-11	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.75	Vert(TL)	-0.03 10-11	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.75	Horz(TL)	-0.06 6	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.08 10-11	>999	240		Weight: 157 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 "Except"
T2: 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 "Except"
B2: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 "Except"
W1: 2 X 4 SYP No.2
WEDGE
Right: 2 X 6 SYP No.1D

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 4-8
T-Brace: 2 X 4 SYP No.3 - 3-9
2 X 6 SYP No.1D - 3-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

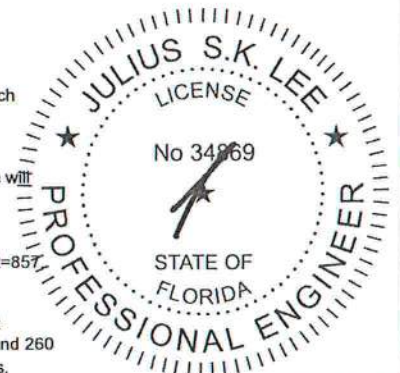
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 12=380/Mechanical, 9=977/0-5-8, 6=276/0-5-8
Max Horz 12=382(LC 3)
Max Uplift 12=857(LC 3), 9=3597(LC 4), 6=588(LC 6)
Max Grav 12=387(LC 9), 9=977(LC 1), 6=276(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=318/757, 2-3=246/939, 3-4=42/478, 4-5=152/658, 5-6=296/636, 1-12=376/879
BOT CHORD 11-12=383/386, 10-11=454/151, 9-10=319/91, 8-9=305/417, 4-8=170/300,
7-8=261/132, 6-7=261/132
WEBS 2-10=229/272, 3-10=1736/491, 3-9=557/1808, 5-8=153/331, 1-11=550/168

NOTES (13-15)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=857, 9=3597, 6=588.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 114 lb down and 426 lb up at 7-0-0, and 167 lb down and 442 lb up at 8-2-0 on top chord, and 490 lb down and 1962 lb up at 7-0-0, and 223 lb up at 8-0-4, and 260 lb down and 947 lb up at 8-0-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- Additional bracing CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T06	Truss Type SPECIAL	Qty 1	Ply 1	Disosway Custom Residence	14119377
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Builders FirstSource, Lake City, FL 32055

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13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

15) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 4-6=-54, 9-12=-10, 6-8=-10

Concentrated Loads (lb)

Vert: 3=-74(F) 4=-87(F) 10=-490(F) 9=-36(F=-260)



October 2, 2009

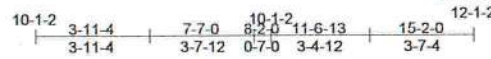
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Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence
315479	T07	SPECIAL	1	1	14119378

Builders FrstSource, Lake City, FL 32055

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3x5 ||

Scale = 1:77.1

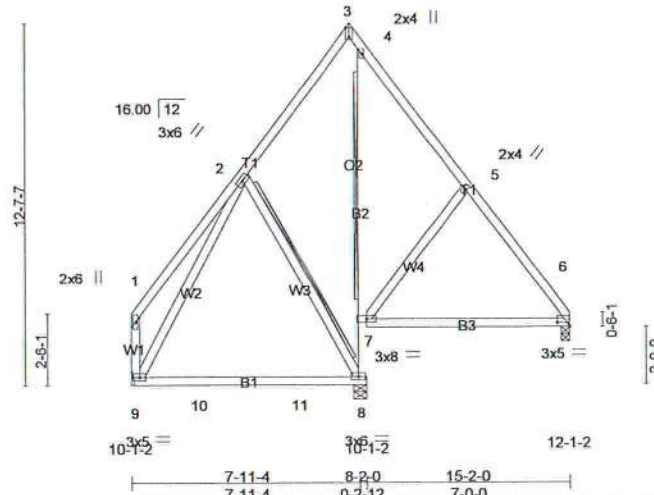


Plate Offsets (X,Y): [3:Edge,0-1-8], [6:0-5-4,0-1-6]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.49	Vert(LL)	-0.16	8-9	>604	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.81	Vert(TL)	-0.24	8-9	>390	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.53	Horz(TL)	0.14	6	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.28	6-7	>303	240		
									Weight: 113 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 4-7
T-Brace: 2 X 4 SYP No.3 - 2-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=271/0-3-8, 8=455/0-5-8, 9=364/Mechanical
Max Horz 9=413(LC 4)
Max Uplift 6=314(LC 7), 8=581(LC 5), 9=327(LC 4)
Max Grav 6=271(LC 1), 8=455(LC 1), 9=369(LC 10)

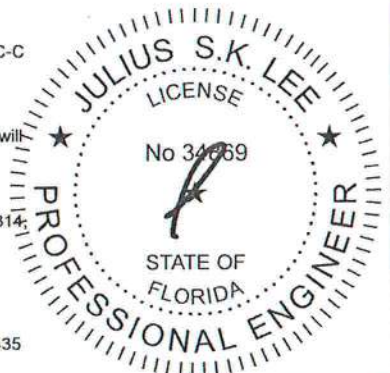
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=165/305, 2-3=200/484, 4-5=154/522, 5-6=250/358, 1-9=164/324
BOT CHORD 7-8=238/285, 4-7=595/219
WEBS 2-8=180/423, 5-7=137/489, 2-9=471/398

NOTES (10-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=314, 8=581, 9=327.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

October 2, 2009



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Boynton, FL 33435

7/14/2009 12:17:58 PM



Weight: 124 lb

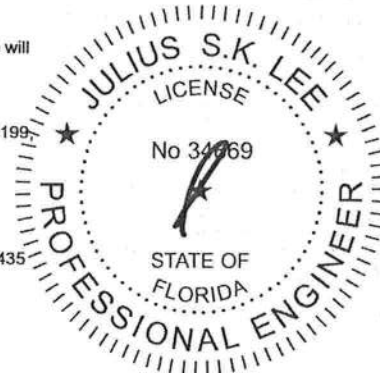
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

(lb/size) 8=555/Mechanical, 6=555/0-3-0
Max Horz 8=400(LC 5)
Max Uplift 8=199(LC 7), 6=205(LC 6)

WEBS 2-7=120/401, 3-7=544/232, 4-7=126/374, 2-8=436/292, 4-6=457/336

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDF=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 5) All bearings are assumed to be SYP No.2 .
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=199; 6=205.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	Disoway Custom Residence	14119380
315479	T09	HIP	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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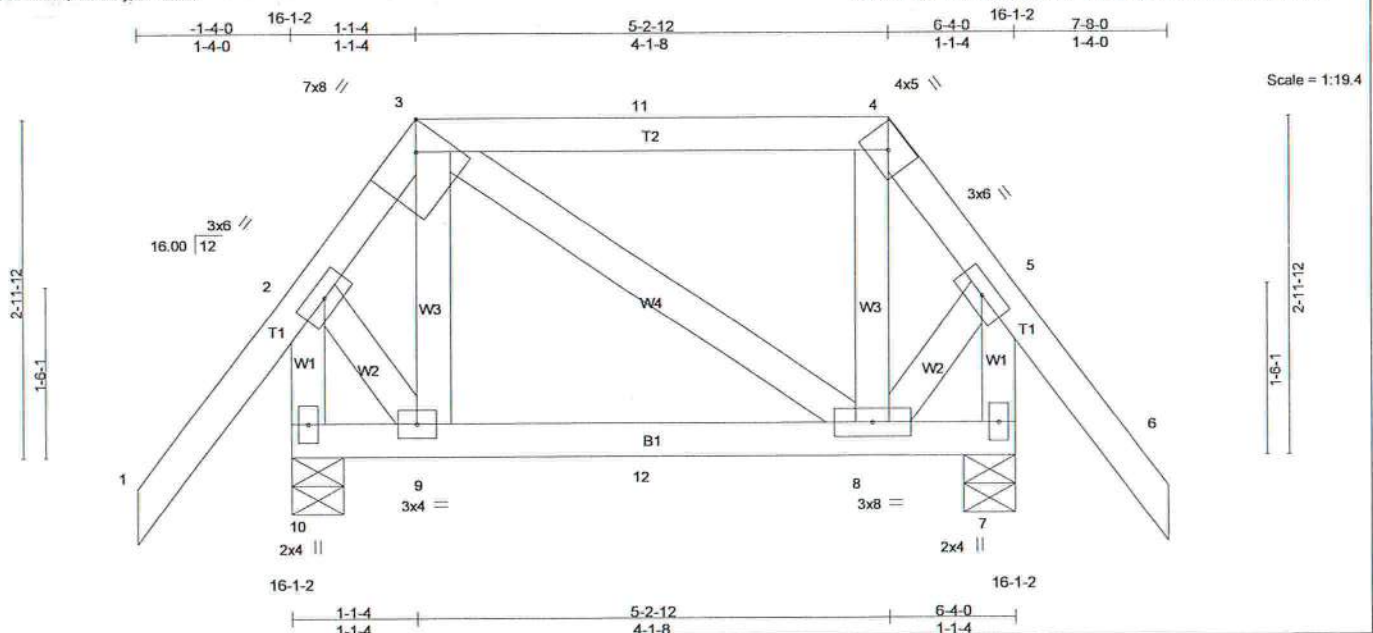


Plate Offsets (X,Y): [3-0-2-13,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.35	Vert(LL)	-0.00	8-9	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.05	Vert(TL)	-0.01	8-9	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.07	Horz(TL)	0.00	7	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.00	9	>999	240		
									Weight: 50 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=213/0-5-8, 7=213/0-5-8
Max Horz 10=119(LC 5)
Max Uplift 10=103(LC 5), 7=117(LC 6)
Max Grav 10=217(LC 9), 7=217(LC 10)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-9=0/264, 5-8=0/256

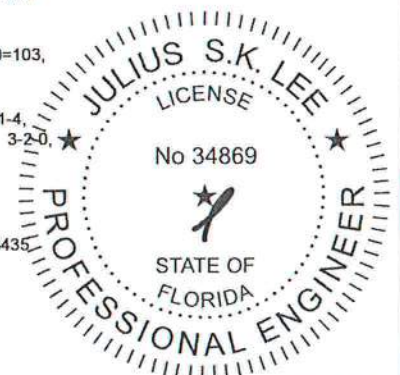
NOTES (11-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=103, 7=117.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 54 lb down and 41 lb up at 1-1-4, and 34 lb down and 19 lb up at 3-2-0, and 70 lb down and 41 lb up at 5-2-12 on top chord, and 22 lb up at 1-1-4, and 14 lb up at 3-2-0, and 22 lb up at 5-2-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-4=-54, 4-5=-54, 5-6=-54, 7-10=-10

Continued on page 2



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MH-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T09	Truss Type HIP	Qty 1	Ply 1	Disosway Custom Residence	I4119380
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Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 3=41(F) 4=41(F) 9=7(F) 8=7(F) 11=19(F) 12=5(F)



October 2, 2009

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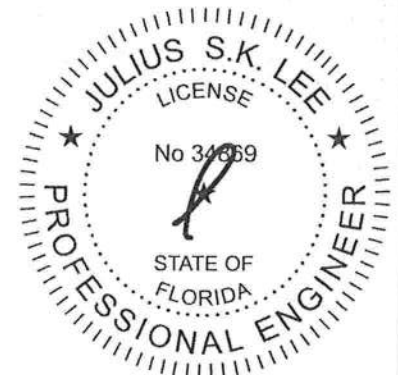
Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119381
315479	T09A	HIP	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard
Concentrated Loads (lb)
Vert: 3=39(B) 8=18(B)



October 2, 2009

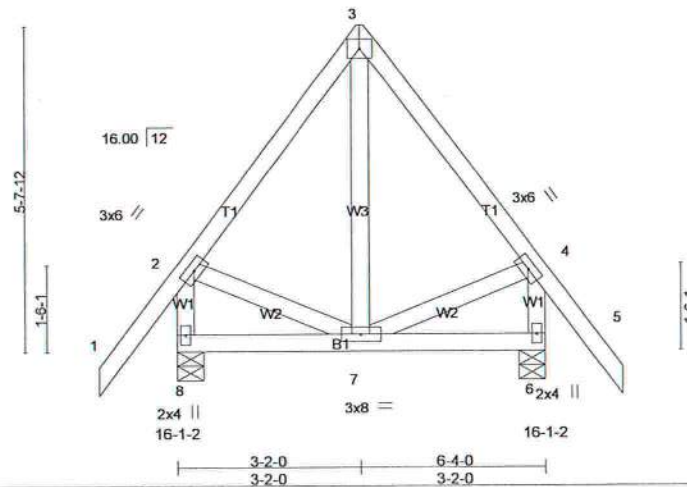
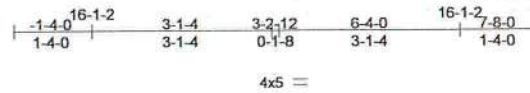
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

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Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence
315479	T10	HIP	1	1	4119382

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.44	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.06	Vert(LL) -0.00 7-8 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.11	Vert(TL) -0.00 7-8 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 6 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.00 7 >999 240		
				Weight: 52 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

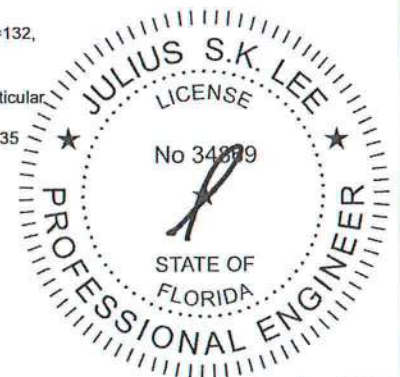
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=273/0-5-8, 6=273/0-5-8
Max Horz 8=236(LC 4)
Max Uplift 8=132(LC 6), 6=132(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-8=261/326, 4-6=261/326
WEBS 2-7=29/323, 4-7=29/323

- NOTES (8-9)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=132, 6=132.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

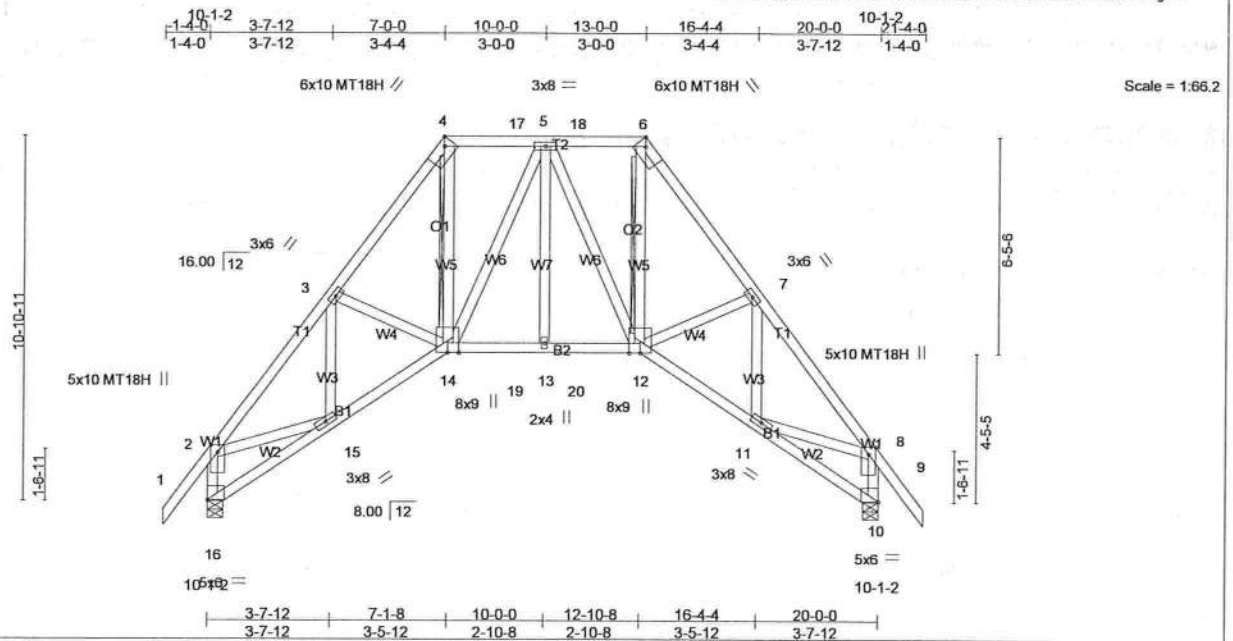
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disoway Custom Residence	I4119383
315479	T11	SPECIAL	1	1		

Builders FrstSource, Lake City, FL 32055

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Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	I4119383
315479	T11	SPECIAL	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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NOTES (15-16)

- 12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 77 lb down and 491 lb up at 7-0-0, 20 lb down and 208 lb up at 9-0-12, and 20 lb down and 208 lb up at 10-11-4, and 117 lb down and 500 lb up at 13-0-0 on top chord, and 409 lb down and 2226 lb up at 7-1-8, 124 lb down and 597 lb up at 9-0-12, and 124 lb down and 597 lb up at 10-11-4, and 409 lb down and 2226 lb up at 12-10-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 13) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 16) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

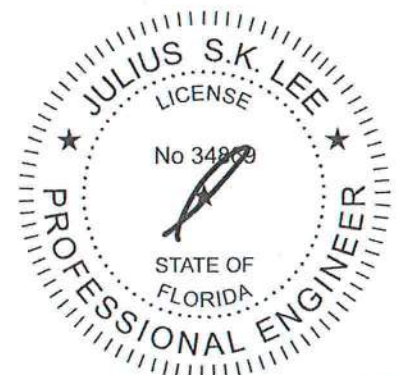
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54, 2-4=-54, 4-6=-54, 6-8=-54, 8-9=-54, 14-16=-10, 12-14=-10, 10-12=-10

Concentrated Loads (lb)

Vert: 4=-77(B) 6=-77(B) 12=-409(B) 14=-409(B) 17=-20(B) 18=-20(B) 19=-124(B) 20=-124(B)



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.

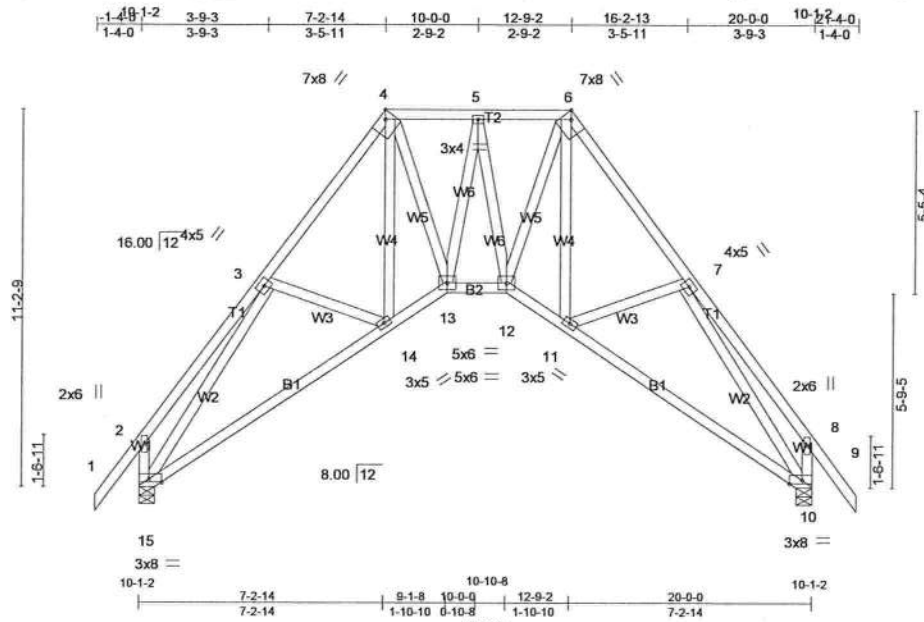
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T12	Truss Type SPECIAL	Qty 1	Ply 1	Disosway Custom Residence	I4119384
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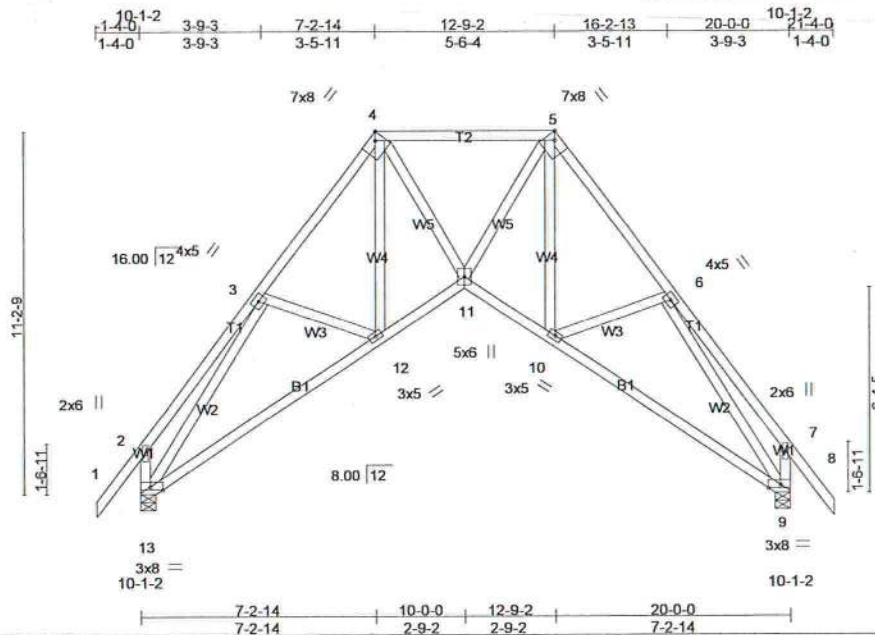
7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:53 2009 Page 1



Job	Truss	Truss Type	Qty	Ply	Disoway Custom Residence	14119385
315479	T13	SPECIAL	4	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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Scale = 1:68.4

Plate Offsets (X,Y): [4:0-2-13,Edge], [5:0-2-13,Edge], [9:0-4-8,Edge], [10:0-0-0-0-0], [13:0-4-8,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.50	Vert(LL)	-0.08 12-13	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.86	Vert(TL)	-0.14 12-13	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.78	Horz(TL)	0.11 9	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.49 12-13	>478	240		
								Weight: 155 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1: 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5.
Rigid ceiling directly applied or 6-5-4 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 13=711/0-5-8, 9=711/0-5-8
Max Horz 13=471(LC 4)
Max Uplift 13=594(LC 6), 9=594(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=149/831, 3-4=841/1198, 4-5=647/658, 5-6=841/1198, 6-7=149/831,
2-13=238/868, 7-9=238/868

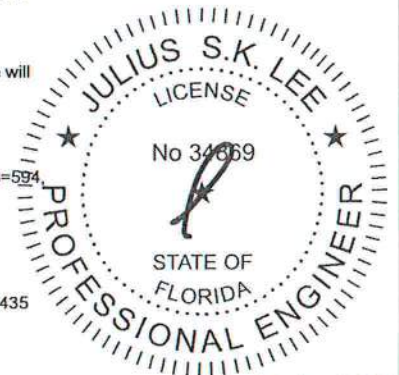
BOT CHORD 12-13=844/602, 11-12=477/744, 10-11=282/685, 9-10=323/565

WEBS 3-12=80/296, 4-12=1141/171, 4-11=64/500, 5-11=371/578, 5-10=1141/171,
6-10=159/296, 3-13=848/493, 6-9=848/557

NOTES (11-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 13, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=594, 9=594.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.

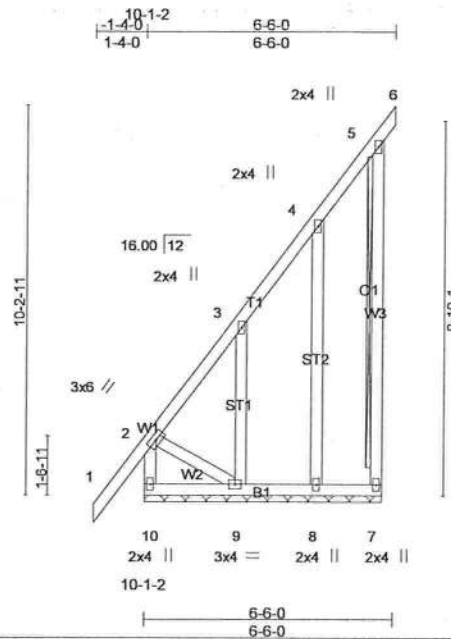
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T14	Truss Type GABLE	Qty 2	Ply 1	Disosway Custom Residence	14119386
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Builders FirstSource, Lake City, FL 32055

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Scale = 1:58.3

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.41	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.03	Vert(LL) 0.00 5 n/r 120		
BCLL 0.0	Lumber Increase 1.25	WB 0.25	Vert(TL) 0.00 5 n/r 90		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 7 n/a n/a		
	Code FBC2007/TPI2002				
				Weight: 65 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-10-12 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 5-7
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

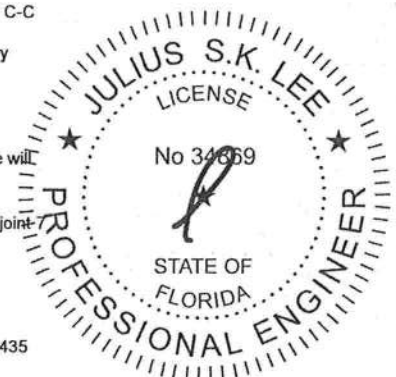
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 6-2-8.
 (lb) - Max Horz 10=496(LC 6)
 Max Uplift All uplift 100 lb or less at joint(s) except 10=102(LC 4), 7=113(LC 5), 8=224(LC 6), 9=417(LC 6)
 Max Grav All reactions 250 lb or less at joint(s) 7, 8, 9 except 10=473(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-10=895/107, 2-3=823/112, 3-4=554/44, 5-7=59/251
 BOT CHORD 9-10=857/5
 WEBS 4-8=105/409, 3-9=104/316, 2-9=6/991

- NOTES** (11-12)
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - 3) Gable requires continuous bottom chord bearing.
 - 4) Gable studs spaced at 2-0-0 oc.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 7) All bearings are assumed to be SYP No.2.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 10, 113 lb uplift at joint 7, 224 lb uplift at joint 8 and 417 lb uplift at joint 9.
 - 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

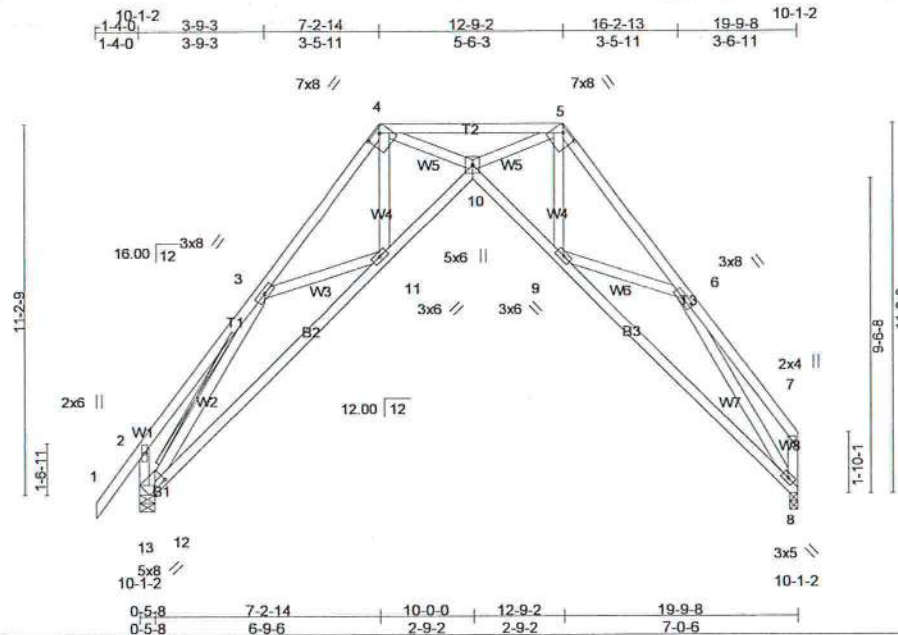
Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence
315479	T15	SPECIAL	9	1	

I4119387

Builders FrstSource, Lake City, FL 32055

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Scale = 1:66.9

Plate Offsets (X,Y): [4:0-4-12,0-1-8], [5:0-4-12,0-1-8], [13:0-6-10,0-1-6]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.39	Vert(LL)	-0.13	10	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.32	Vert(TL)	-0.24	10	>954	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.88	Horz(TL)	0.54	8	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.19	10	>999	240		
									Weight: 144 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3 *Except*
 W1,W8: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-8-2 oc purlins, except end verticals, and 2-0-0 oc purlins (4-0-10 max.): 4-5.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 3-12
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 13=333/0-5-8, 12=1057/0-5-8, 8=604/0-3-0
 Max Horz 13=476(LC 5)
 Max Uplift 13=483(LC 2), 12=312(LC 5), 8=194(LC 6)
 Max Grav 13=209(LC 5), 12=1057(LC 1), 8=604(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

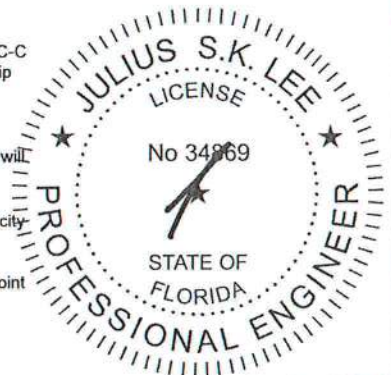
TOP CHORD 2-3=23/432, 3-4=1175/865, 4-5=2097/1592, 5-6=1214/617, 2-13=138/549
 BOT CHORD 12-13=430/374, 11-12=1085/837, 10-11=915/924, 9-10=575/952, 8-9=459/848
 WEBS 3-12=1253/778, 3-11=0/437, 4-11=275/95, 4-10=1105/1584, 5-10=1367/1561,
 6-9=219/451, 6-8=1140/613

NOTES (13-14)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 8.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 483 lb uplift at joint 13, 312 lb uplift at joint 12 and 194 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

Continued on page 2

October 2, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 315479	Truss T15	Truss Type SPECIAL	Qty 9	Ply 1	Disosway Custom Residence	14119387
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Builders FirstSource, Lake City, FL 32055

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13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

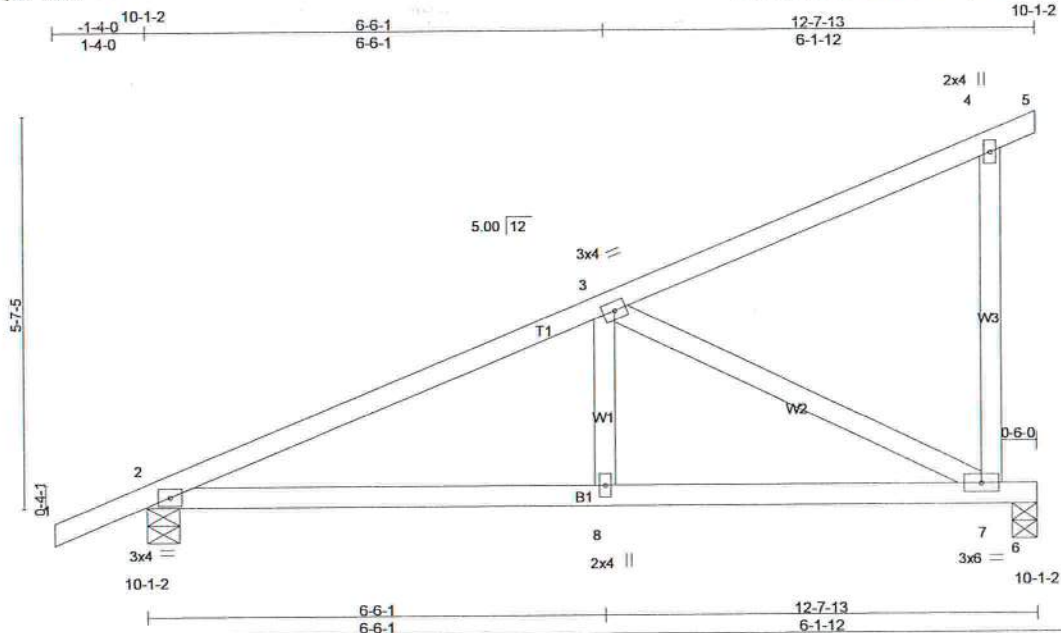
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MJ-T473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disoway Custom Residence	I4119388
315479	T16	MONO TRUSS	1	1	Job Reference (optional)	

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LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.32	Vert(LL)	-0.04	7-8	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.35	Vert(TL)	-0.09	7-8	>999	240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.36	Horz(TL)	-0.01	6	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Wind(LL)	0.09	7-8	>999	240		
	Code FBC2007/TPI2002							Weight: 61 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 7-8-13 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=481/0-5-8, 6=396/0-4-4
Max Horz 2=263(LC 6)
Max Uplift 2=228(LC 6), 6=219(LC 6)

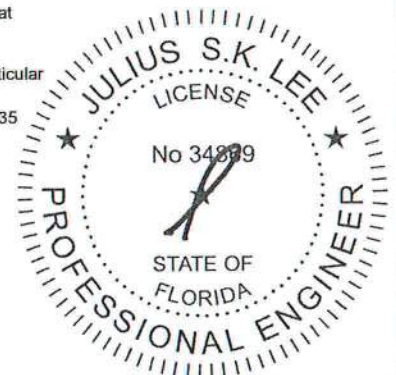
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=628/323, 4-7=150/253
BOT CHORD 2-8=596/518, 7-8=596/518
WEBS 3-7=550/632

NOTES (7-8)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 2 and 219 lb uplift at joint 6.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

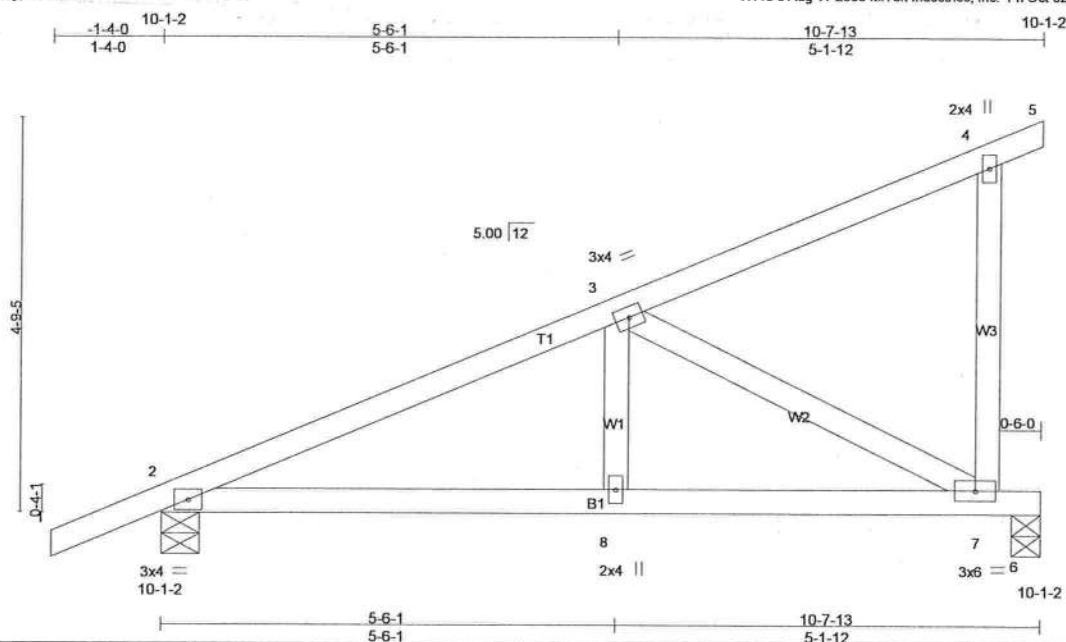
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T17	Truss Type MONO TRUSS	Qty 1	Ply 1	Disosway Custom Residence	I4119389
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LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES MT20	GRIP 244/190
TCLL 20.0	Plates Increase 1.25	TC 0.27	Vert(LL) -0.02 7-8 >999 360		
TCDL 7.0	Lumber Increase 1.25	BC 0.30	Vert(TL) -0.05 7-8 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.20	Horz(TL) -0.01 6 n/a n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.06 7-8 >999 240		Weight: 51 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-4-14 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=419/0-5-8, 6=331/0-4-4
Max Horz 2=227(LC 6)
Max Uplift 2=208(LC 6), 6=182(LC 6)

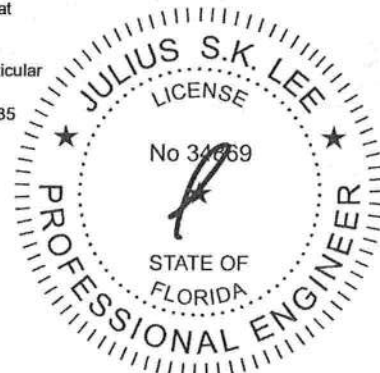
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=513/270
BOT CHORD 2-8=510/420, 7-8=510/420
WEBS 3-7=446/540

NOTES (7-8)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 208 lb uplift at joint 2 and 182 lb uplift at joint 6.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

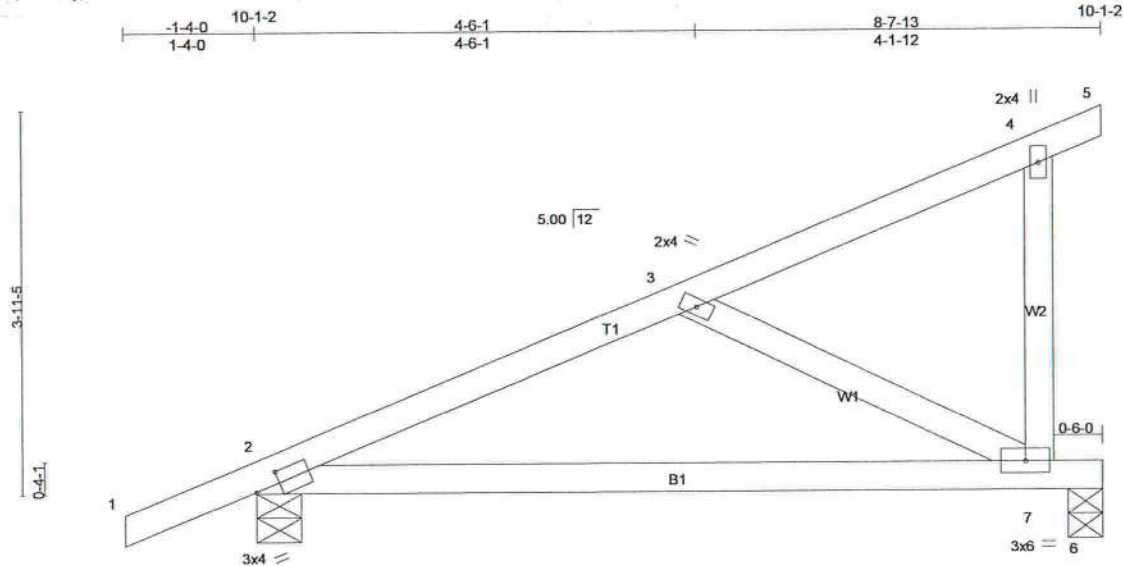
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

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1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	Disosway Custom Residence	14119390
315479	T18	MONO TRUSS	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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Scale = 1:22.7

Plate Offsets (X,Y): [2-0-3-0,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.54	Vert(LL)	-0.12	2-7	>825	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.26	Vert(TL)	-0.22	2-7	>440	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.11	Horz(TL)	-0.01	6	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.07	2-7	>999	240		
									Weight: 39 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-4-14 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=356/0-5-8, 6=265/0-4-4
Max Horz 2=191(LC 6)
Max Uplift 2=189(LC 6), 6=143(LC 6)

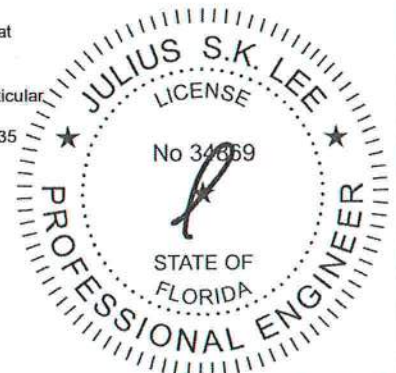
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=349/211
BOT CHORD 2-7=419/281
WEBS 3-7=270/431

NOTES (7-8)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint 2 and 143 lb uplift at joint 6.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

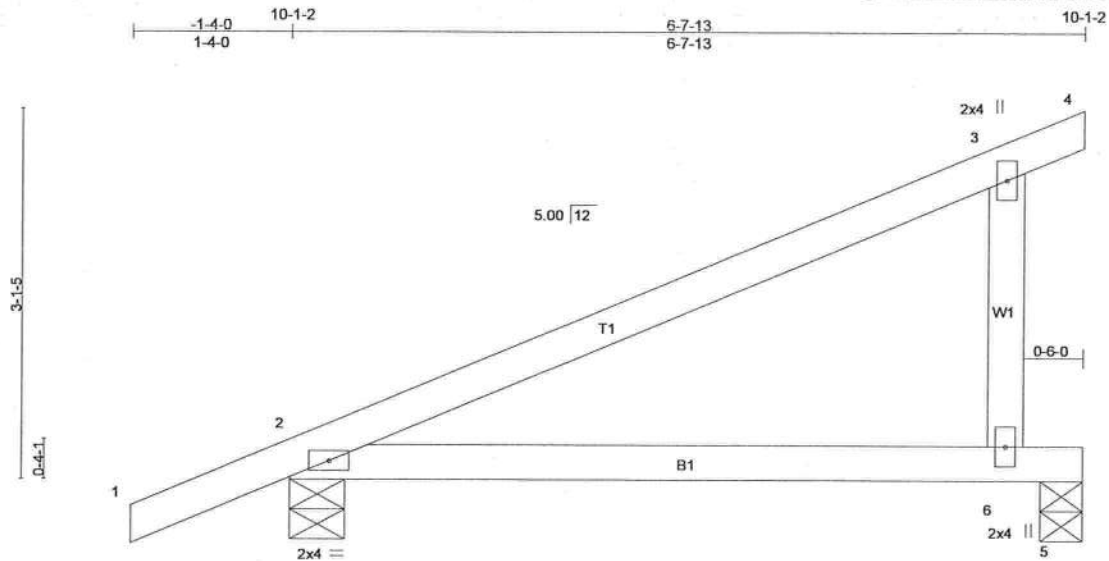
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T19	Truss Type MONO TRUSS	Qty 1	Ply 1	Disoway Custom Residence	I4119391
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Aug 17 2009 MITek Industries, Inc. Fri Oct 02 11:55:57 2009 Page 1



Scale = 1:18.6

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.47	Vert(LL)	-0.08	2-6	>940	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.30	Vert(TL)	-0.16	2-6	>481		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.09	2-6	>863		
								Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

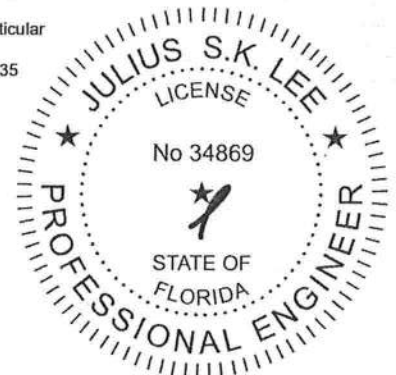
REACTIONS (lb/size) 2=295/0-5-8, 5=199/0-4-4
Max Horz 2=155(LC 6)
Max Uplift 2=170(LC 6), 5=104(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-6=181/342

NOTES (7-8)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 2 and 104 lb uplift at joint 5.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

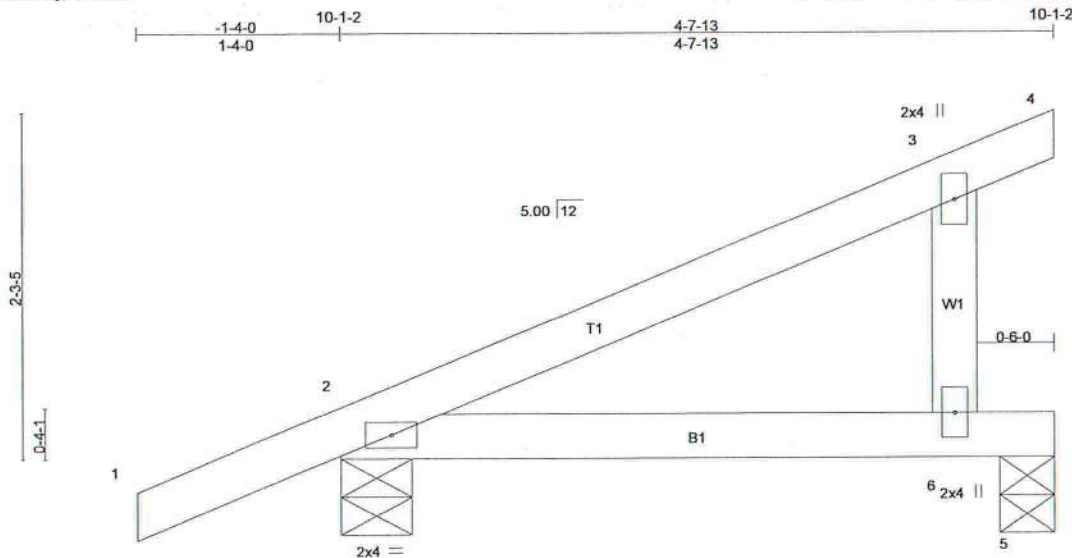
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T20	Truss Type MONO TRUSS	Qty 1	Ply 1	Disosway Custom Residence	I4119392
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Builders FirstSource, Lake City, FL 32055

7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:57 2009 Page 1



Scale = 1:14.5

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.20	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.14	Vert(LL) -0.02 2-6 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.03 2-6 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.03 2-6 >999 240		
				Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

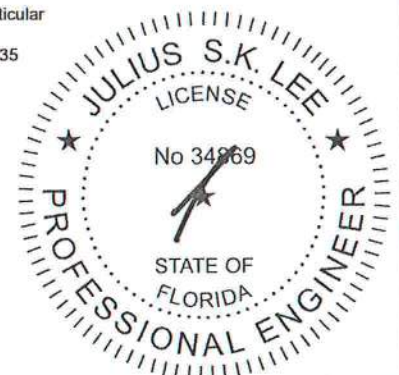
REACTIONS (lb/size) 2=235/0-5-8, 5=130/0-4-4
Max Horz 2=119(LC 6)
Max Uplift 2=155(LC 6), 5=70(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (7-8)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 2 and 70 lb uplift at joint 5.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 2, 2009

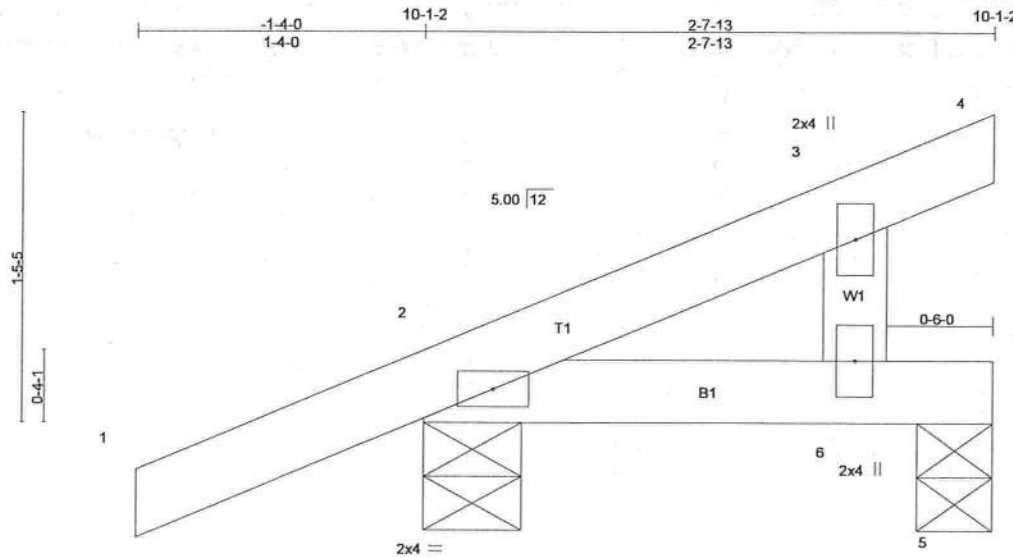
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 315479	Truss T21	Truss Type MONO TRUSS	Qty 1	Ply 1	Disosway Custom Residence	I4119393
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Builders FirstSource, Lake City, FL 32055

7.140 s Aug 17 2009 MiTek Industries, Inc. Fri Oct 02 11:55:58 2009 Page 1



Scale = 1:10.3

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.20	Vert(LL)	-0.00	6	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.06	Vert(TL)	-0.00	2-6	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00		n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.00	6	>999	240		
									Weight: 11 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-7-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

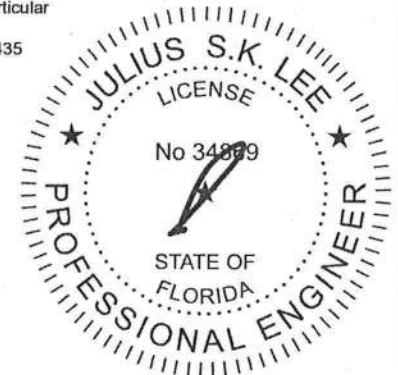
REACTIONS (lb/size) 2=185/0-5-8, 5=52/0-4-4
Max Horz 2=82(LC 6)
Max Uplift 2=151(LC 6), 5=42(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (7-8)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 151 lb uplift at joint 2 and 42 lb uplift at joint 5.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



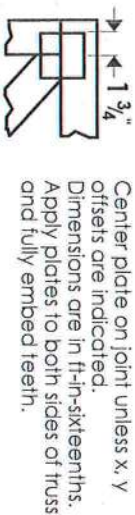
October 2, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MI-T473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



0-1/16"

Center plate on joint unless X, Y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.

For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 X 4

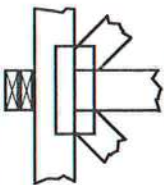
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

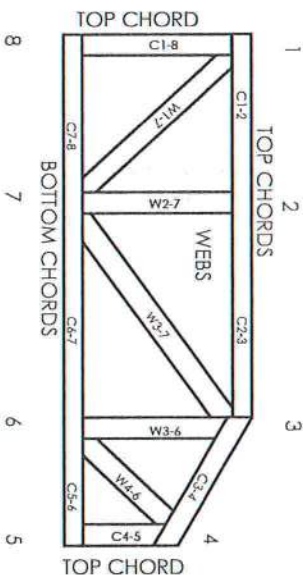


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by ANSI/TP11.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP11.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP11 Quality Criteria.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

#2 HIP OR COMMON TRUSS

TYP
AX

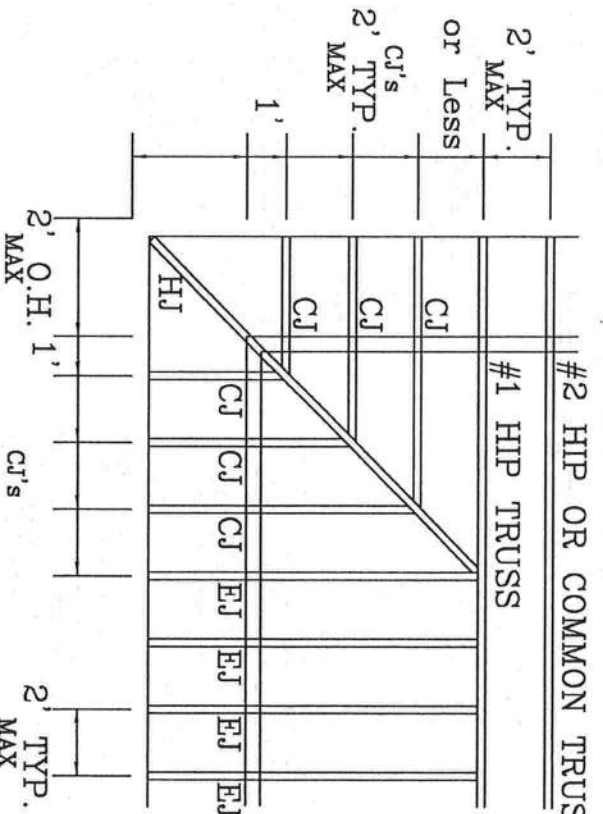
1

UPLIFT: 400# or Less

UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND
SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

UPLEFT: 400# or Less

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)



HIPJACK

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED

BC LIVE LOAD IS NON CONCURRENT 10%

7'0" MAX

THE DRAWINGS AND TRUSSES REQUIRING EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RC1-10-3 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 388 ENTERPRISE DR., SUITE 200, MADISON, WI 53719 AND VITA (WOOD TRUSS COLLECTOR OF AMERICA, 6500 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

PRODUCTS: FURNISH A COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, AND FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPECIFICATION BY NDS AND/OR AISC. APPLY CONNECTOR PLATES TO EACH JOINT OF TRUSS AND UNLESS OTHERWISE SPECIFIED BY NDS AND/OR AISC, GALV. STEEL. APPLY PLATES TO EACH JOINT OF TRUSS AND UNLESS OTHERWISE SPECIFIED BY NDS AND/OR AISC, GALV. STEEL. ANY INSPECTION OF PLATES FOLLOWED BY CD SHALL BE AT THIS DESIGN. POSITION PER DRAWINGS 1604-2.

PROFESSIONAL ENGINEERING RESPONSIBILITY: I, THE ENGINEER, HEREBY CERTIFY THAT I AM A LICENSED PROFESSIONAL ENGINEERING RESPONSIBILITY AND ONLY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE DESIGNER, PER ANSI/TPI 1 SEC. 2

JOINTS-FLEX
CONS. ENGINEERS, P.A.
1465 ST. 404 AVENUE

Series	20	10*	5
MAX	MAX	MAX	MAX
PSTF	PSTF	PSTF	PSTF

REF	7'MAX STBK CS
DATE	Jun./27/2008

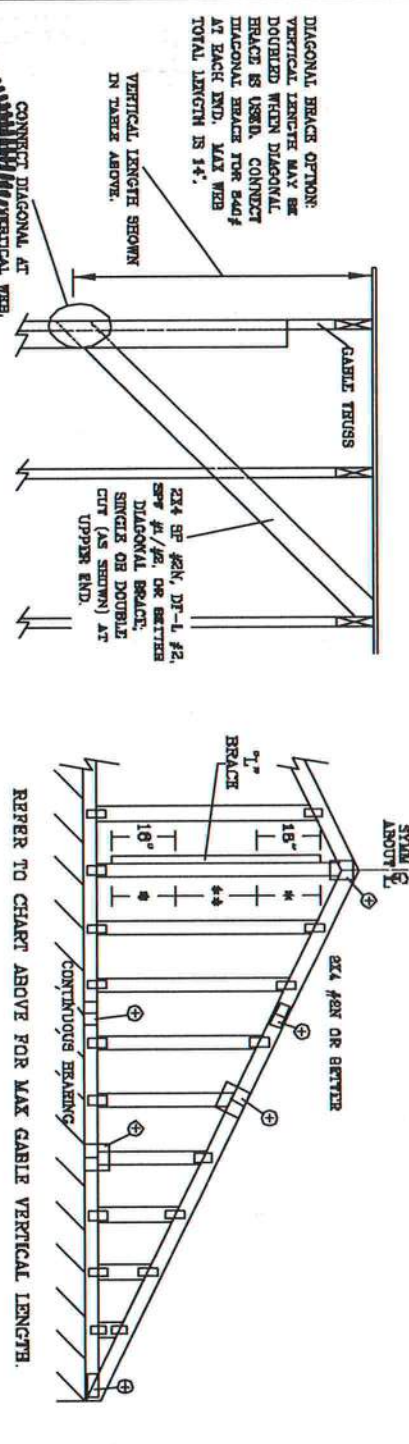
—ENG

REVIEWED
By Julius Iee at 10:52 am, Jun 27, 2008



ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO		(1) 1X4 "L" BRACE		(1) 2X4 "L" BRACE		(2) 2X4 "L" BRACE		(1) 2X6 "L" BRACE		(2) 2X8 "L" BRACE	
CABLE VERTICAL SPACING	SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"	13' 3"
			3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	10' 1"	12' 11"	12' 11"	12' 11"
			3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	10' 1"	12' 11"	12' 11"	12' 11"
			3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"	13' 11"
16" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"	13' 3"
			3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	10' 1"	12' 11"	12' 11"	12' 11"
			3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	10' 1"	12' 11"	12' 11"	12' 11"
			3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"	13' 11"
24" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"	13' 3"
			3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	10' 1"	12' 11"	12' 11"	12' 11"
			3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	10' 1"	12' 11"	12' 11"	12' 11"
			3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"	13' 11"



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS L/740.	
PROVIDE UPLIFT CONNECTIONS FOR 136 PSF OVER CONTINUOUS BEARING (6 PSF TO DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4' 0" OUTDOORS WITH 8' 0" OVERHANG, OR 12' PLYWOOD OVERHANG.	
ATTACH EACH "L" BRACE WITH 104 NAILS.	
* FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
* FOR (2) "L" BRACES, SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.	
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	

REVIEWED
 By Julius Lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1455 SW 4th Avenue
 DEER BEACH, FL 33441-8161

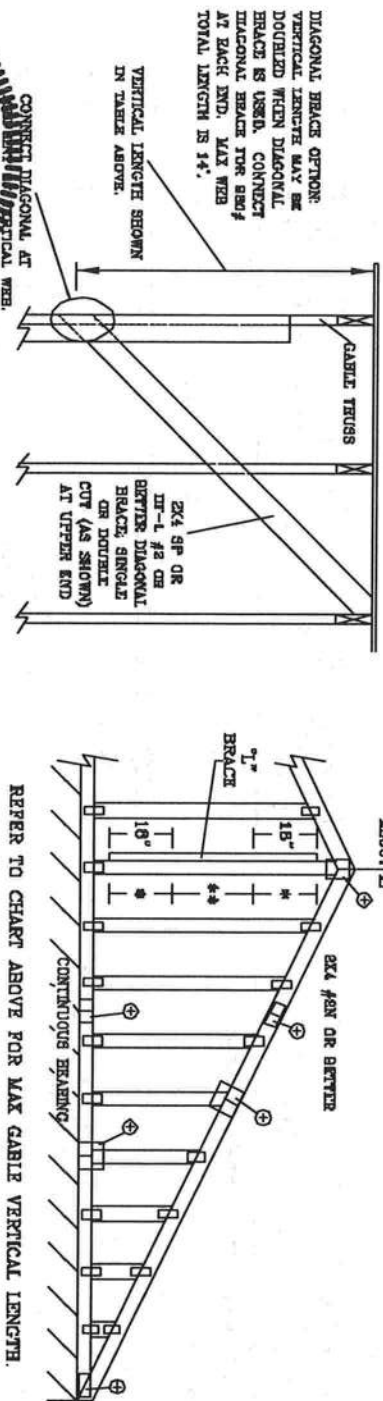
No. 34869
 STATE OF FLORIDA

MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

REF ASCE 7-02-CAR13015
 DATE 11/26/03
 DRWG MTRK STD CABLE 15 E BT
 -ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO		(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE *		(2) 2x8 "L" BRACE *	
CABLE VERTICAL SPACING	SPECIES	GRADE	BRACE	NO	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A
12" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 8"	6' 8"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 3"	12' 3"	12' 7"
	SPF	#3	3' 1"	4' 5"	4' 5"	6' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"
	HF	STUD	3' 1"	4' 6"	4' 5"	5' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"
	SP	STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"	10' 7"	10' 7"
	SP	#1	3' 6"	5' 6"	5' 6"	5' 11"	6' 8"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"	13' 2"	13' 2"
16" O.C.	SPF	#1 / #2	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 0"	9' 4"	9' 4"	12' 3"	12' 3"	12' 3"	12' 3"
	SPF	#3	3' 3"	4' 8"	4' 8"	6' 10"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 3"	12' 3"	12' 3"
	HF	STUD	3' 3"	4' 8"	4' 8"	6' 10"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 3"	12' 3"	12' 3"
	SP	STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"	10' 10"	10' 10"
	SP	#1 / #2	3' 8"	6' 4"	6' 4"	7' 6"	7' 6"	8' 11"	9' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
24" O.C.	SPF	#1 / #2	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"	10' 10"	10' 10"
	SP	#1 / #2	3' 8"	6' 4"	6' 4"	7' 6"	7' 6"	8' 11"	9' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS L/240.	
PROVIDE UPLIFT CONNECTIONS FOR 150 PSF OVER CONTINUOUS BEAMING (6 PSF TO DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4' 0" OUTDOCKERS WITH 8' 0" OVERHANG, OR 12' PLYWOOD OVERHANG.	
ATTACH EACH "L" BRACE WITH 104 NAILS.	
* FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C.	
* FOR (2) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (3) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (4) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (5) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (6) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (7) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (8) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (9) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (10) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (11) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (12) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (13) "L" BRACES, SPACE NAILS AT 8" O.C.	
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* FOR (96) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (97) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (98) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (99) "L" BRACES, SPACE NAILS AT 8" O.C.	
* FOR (100) "L" BRACES, SPACE NAILS AT 8" O.C.	



DESIGNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTING. REFER TO EIGHT 1-800 CALLING CONCRETE SAFETY INFORMATION, PUBLISHED BY THE TRUSS INSTITUTE, 582 BUNKER RD., SUITE 200, NASHVILLE, TN 37219, AND VITA (WOOD TRUSS COASTAL) THESE FUNCTIONS UNLESS OTHERWISE NOTED. THESE TRUSSES MUST BE PROPERLY ATTACHED TO THE STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2401

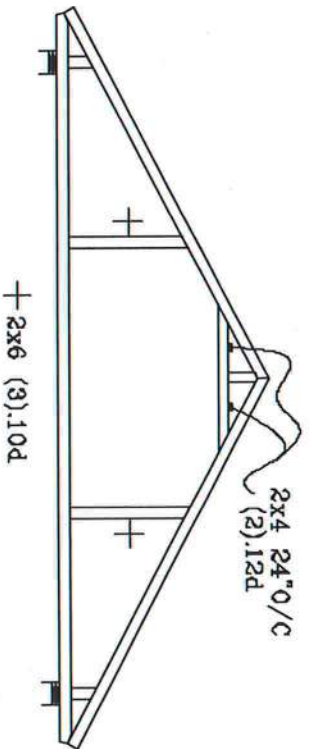
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

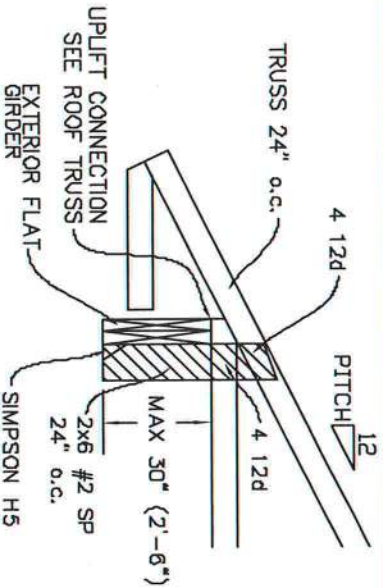
REF ASCE 7-02-GAB10030
DATE 11/26/03
DWG. BY: STD. GAB. 50 2 17
-ENG-

REVIEWED
By Julius Lee at 12:00 pm, Jun 11, 2008

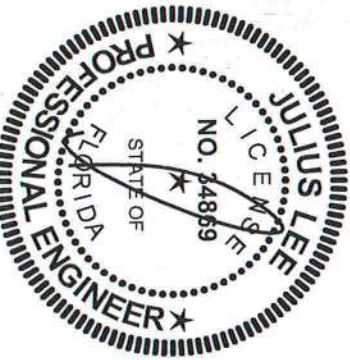
TYPICAL ATTIC TRUSS BRACING



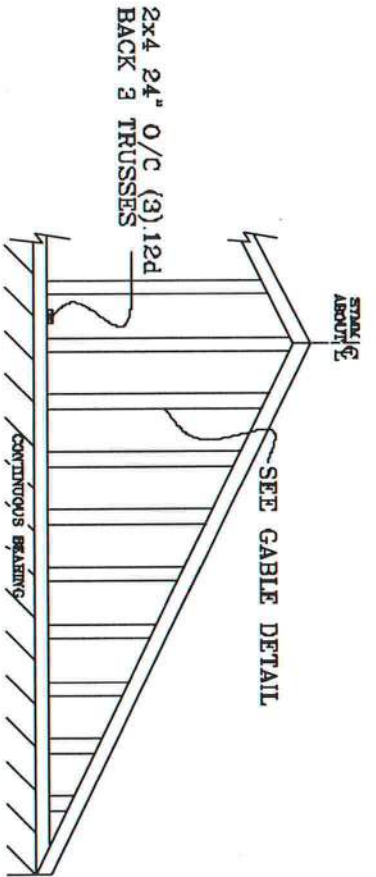
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

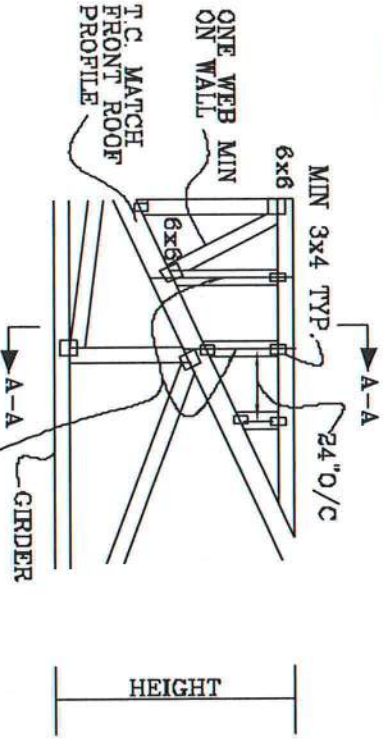


GABLE END TRUSS DETAIL

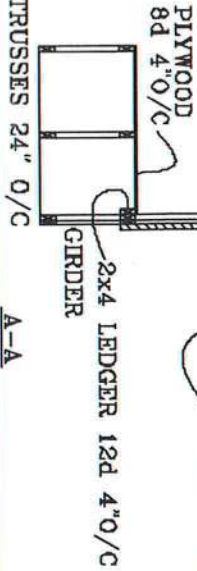


MINIMUM BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



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DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES

SPACE PIGGYBACK VERTICALS AT 4' OC MAX

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

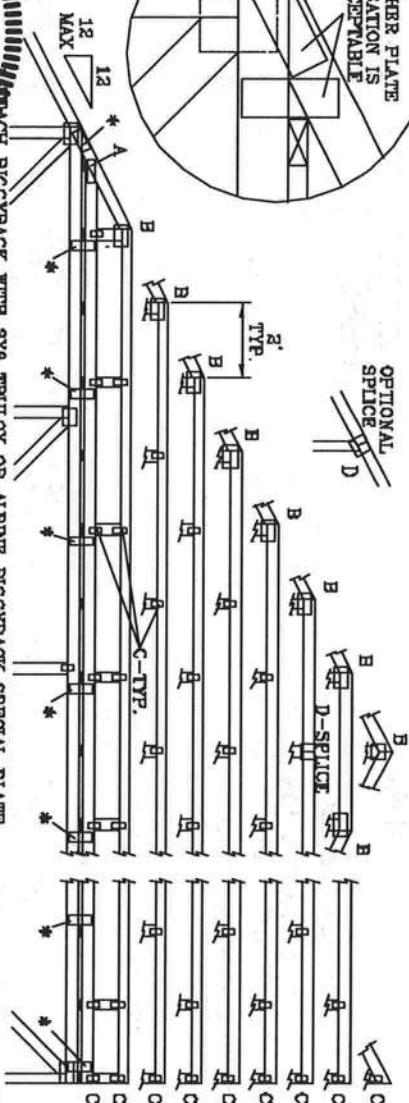
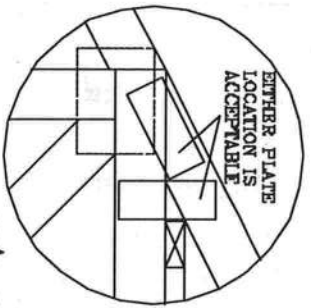
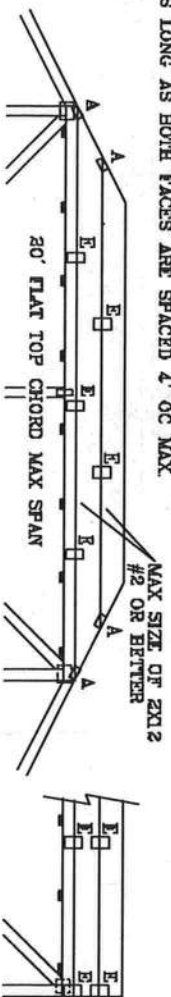
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
110 MPH WIND, 30' MEAN HGT, ENG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT I, EXP C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

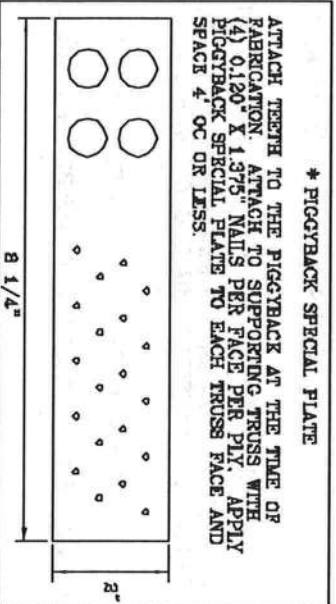


THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

JOINT TYPE	SPANS UP TO		
	30'	34'	38'
A	2X4	2.5X4	2.5X4
B	4X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4
D	5X4	5X6	5X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY		

ATTACH TRUSS PLATES WITH (B) 0.120" X 1.375" NAILS OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	WEB BRACING CHART
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4' OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 10d NAILS AT 4' OC.

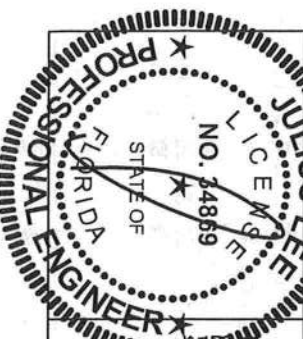


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ORLANDO, FL 32811-2161

MAX LOADING
55 PSF AT
1.33 DUR. FAC.
50 PSF AT
1.25 DUR. FAC.
47 PSF AT
1.15 DUR. FAC.

REF PIGGYBACK
DATE 09/12/07
DRWG/ITERK STD PIGGY
-ENG JL

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



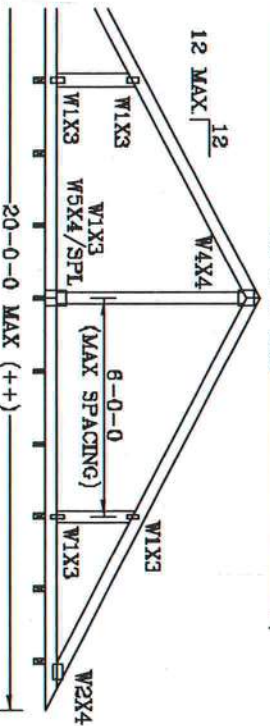
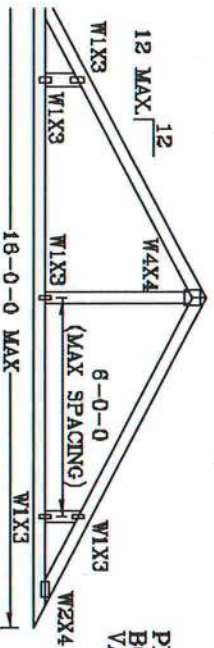
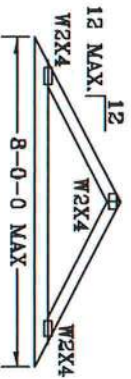
VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(?) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

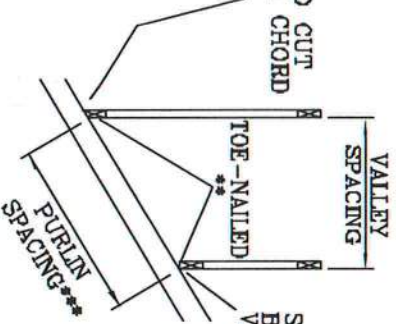
* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

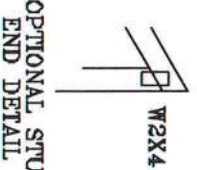
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15 MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=6 PSF.



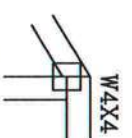
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS
BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES
NOT EXCEED 12'0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



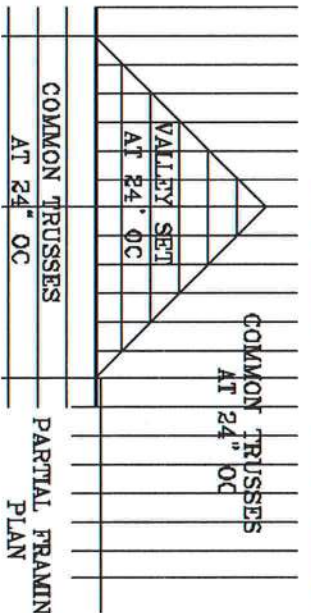
SQUARE CUT
BOTTOM CHORD
VALLEY



OPTIONAL STUB
END DETAIL



OPTIONAL HIP
JOINT DETAIL



COMMON TRUSSES
AT 24" OC

PARTIAL FRAMING
PLAN

OVERVIEWING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND
BRACING. REFER TO ECT-1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS
INSTITUTE, 5820 DOWNSIDE DR., SUITE 200, WASHINGTON, VA 22779 AND VITA CYCLO TRUSS COUNCIL
IN AMERICA, 6200 ENTERPRISE LN, NATION, VT 55759 FOR SAFETY PRACTICES PRIOR TO PERFORMING
THESE FUNCTIONS. TRUSSES OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED
STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIBBON CEILING.

JULIUS LEE'S
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DEALITY BEACH, FL 33444-2814

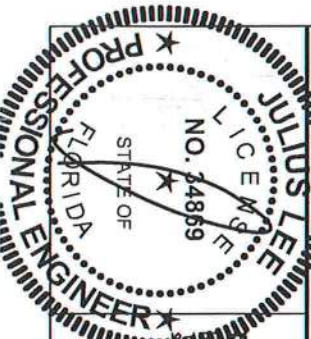
THIS DRAWING REPLACES DRAWING A105

TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		

DUR.FAC.	1.25	1.25
SPACING	24"	

No. 34869
STATE OF FLORIDA

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

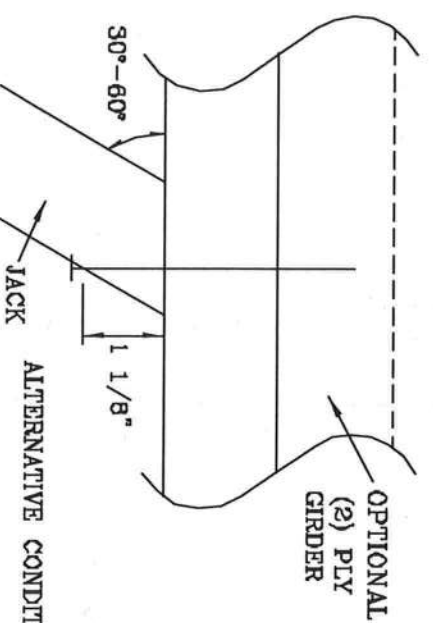
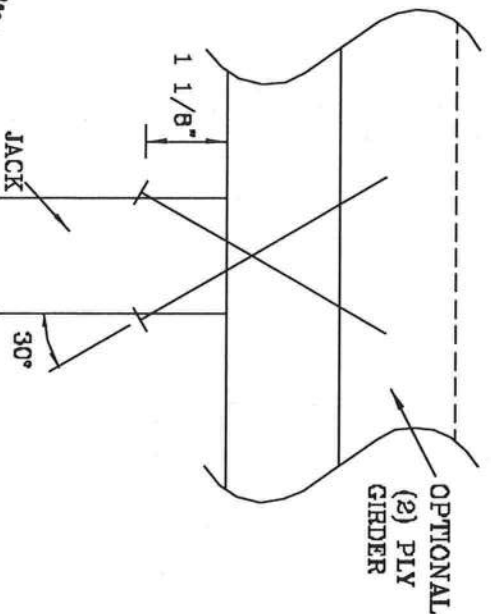
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

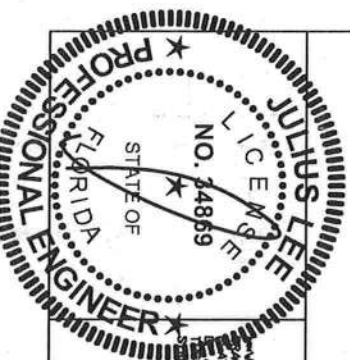
NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	187#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	498#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-83 CHORDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS ASSOCIATION, 588 PARKER RD., SUITE 200, NATION, VA 22079 AND APCA (WOOD TRUSS COUNCIL) PUBLICATIONS. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS ARE IN INCHES. ALL DIMENSIONS SHALL HAVE A PROPERLY ATTACHED ROUNDED

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1405 EY 4TH AVENUE
DELMAR BEACH, FL 33441-2101

No. 34869
STATE OF FLORIDA

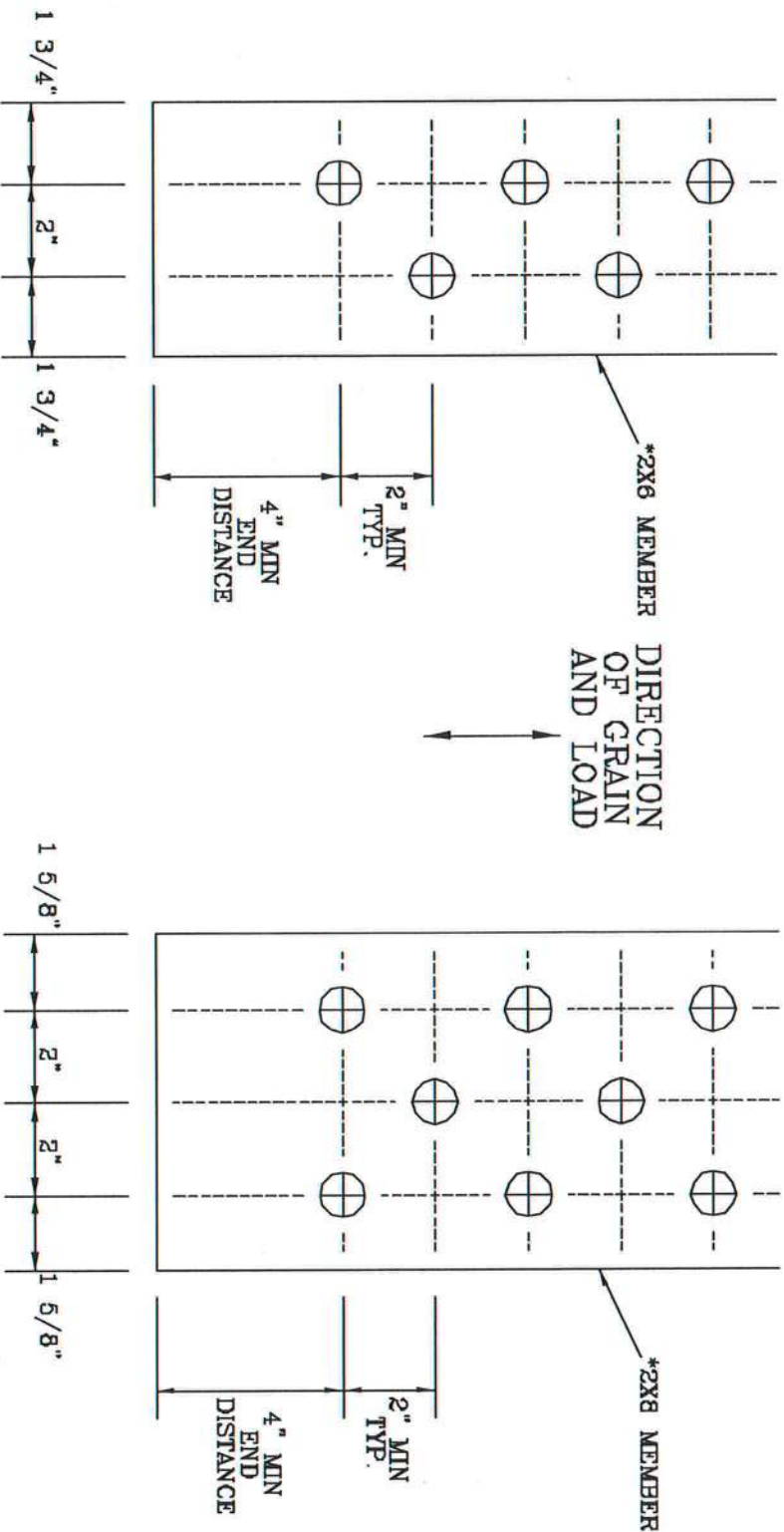
TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-83 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS ASSOCIATION, 588 GUNPOWDER DR., SUITE 200, WASHINGTON, VA 22799 AND THE NATIONAL WOOD FRAMEWORK ASSOCIATION, 6500 W. 12TH AVE., SUITE 100, DENVER, CO 80202. THE INFORMATION CONTAINED HEREIN IS FOR GENERAL INFORMATION ONLY. IT IS NOT TO BE USED AS A BASIS FOR DESIGN OR CONSTRUCTION. THE USER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND EDITION CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
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1400 ST. KEN AVENUE
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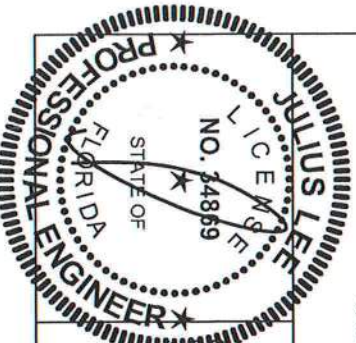
TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/28/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC IL	PSF	-ENG	JL
TOT. LD.	PSF		

REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

No. 34869
STATE OF FLORIDA

DUR. FAC.
SPACING



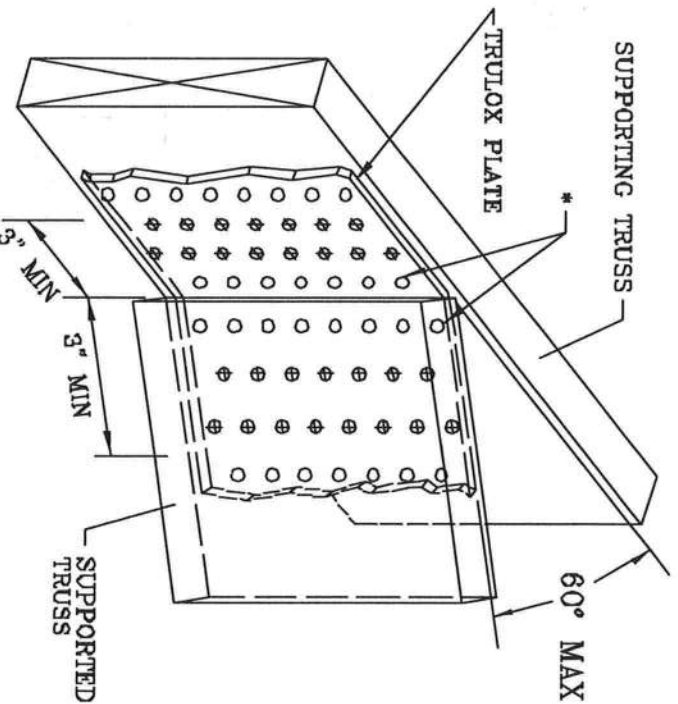
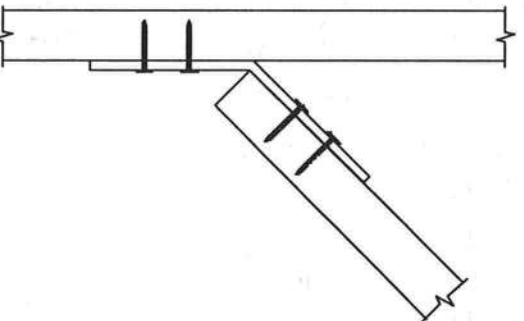
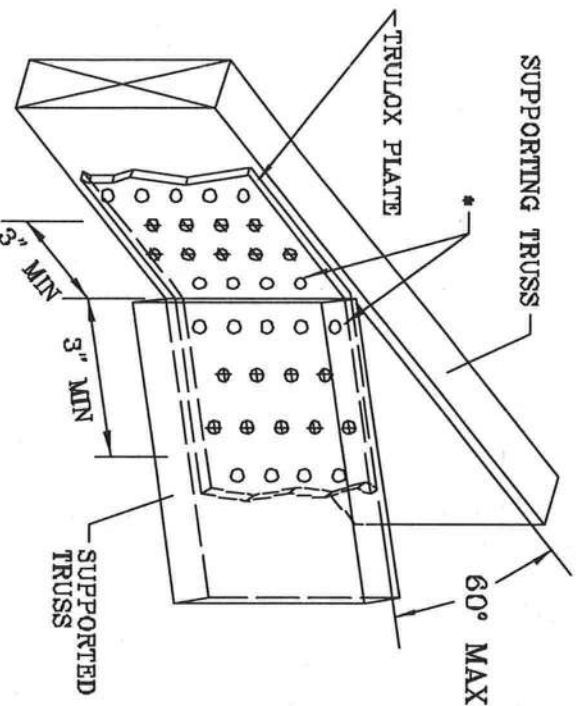
TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

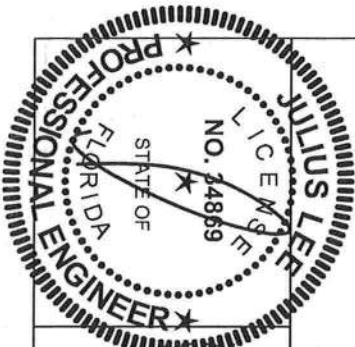


MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	15	990#

MINIMUM 5X6 TRULOX PLATE

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,988/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 2021-1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 383 DOWNSIDE DR., SUITE 200, NORTON, VA 22655 AND VITA TRUSS COUNCIL, 6500 ENTERPRISE LN, HANOVER, VA 22975 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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No: 34889
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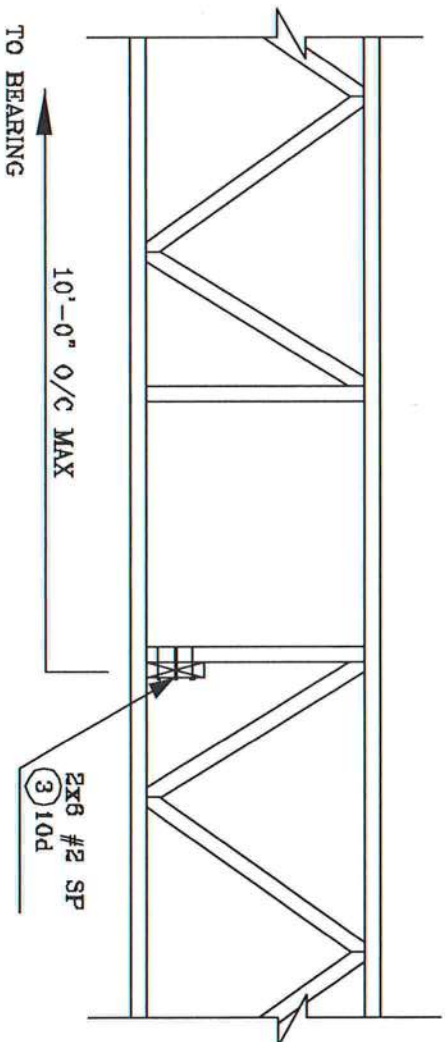
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DATE 11/26/03

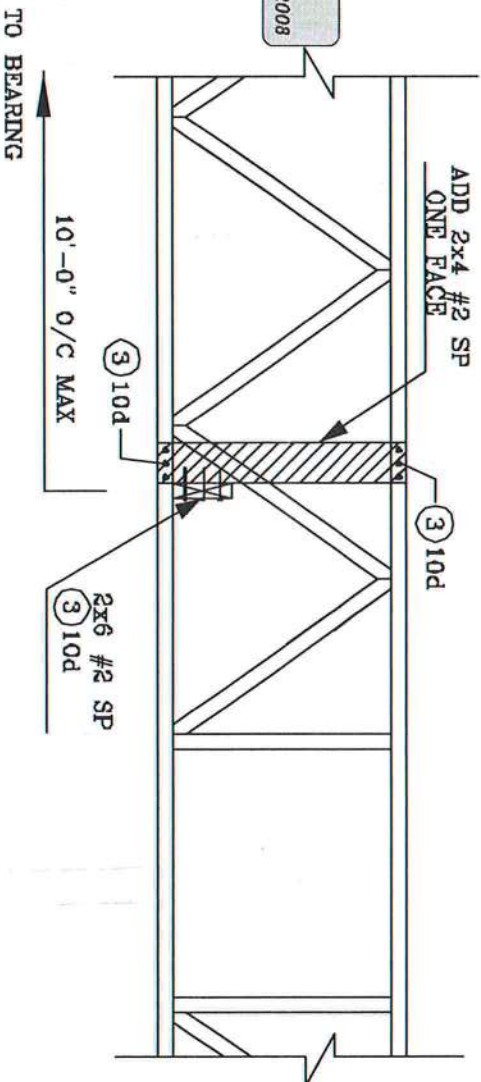
DRWG CNTRULOX1103

-ENG JL

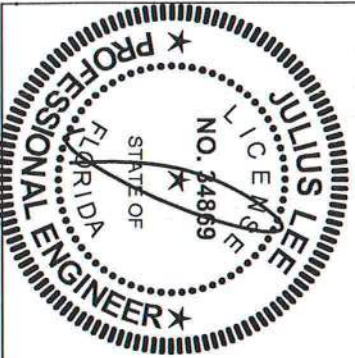
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008



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No: 34869
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MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/4" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 1/16" maximum.

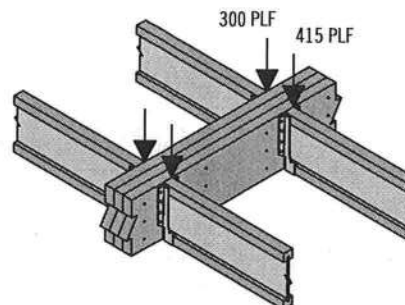
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



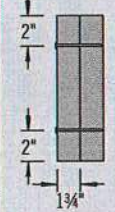
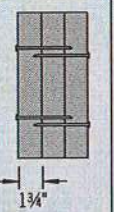

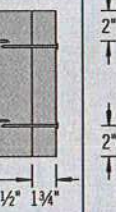
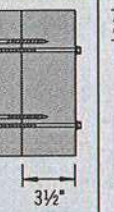
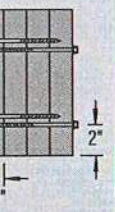
First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
							
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 ⁽¹⁾	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽²⁾
3 3/8" or 5" TrussLok™	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽²⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽²⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

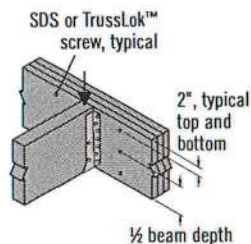
(2) 6" long screws required.

(3) 5" long screws required.

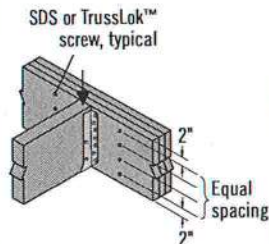
(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

Connections

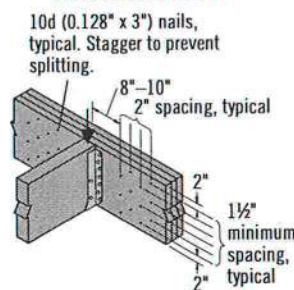
4 or 6 Screw Connection



8 Screw Connection

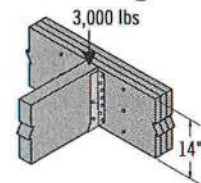


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

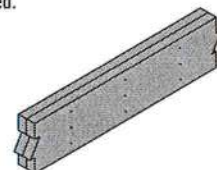
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6