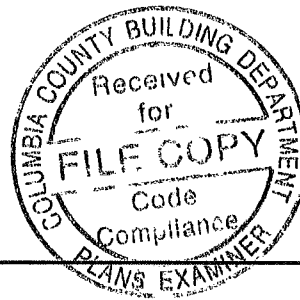


JULIUS LEE PE.



RE: 533050 - GIEBEIG - Lot 8 Unit 3 Mayfair

**1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435**

Site Information:

Project Customer: GIEBEIG HOMES Project Name: 533050 Model: ST. JOHNS 3 BDRM
Lot/Block: 8 Subdivision: MAYFAIR
Address:
City: COLUMBIA CTY State FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name BRIAN TRENT GIEBEIG License # RR282811523
Address 462 SW FAIRLINGTON CT
City: LAKE CITY State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2010/TPI2007 Design Program: MiTek 20/20 7.3
Wind Code: ASCE 7-10 Wind Speed: 130 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 26 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

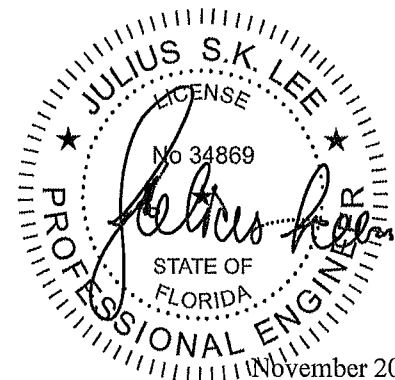
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I7520699	CJ1	11/20/013	18	I7520716	T14	11/20/013
2	I7520700	CJ3	11/20/013	19	I7520717	T15	11/20/013
3	I7520701	CJ5	11/20/013	20	I7520718	T16	11/20/013
4	I7520702	EJ7	11/20/013	21	I7520719	T17	11/20/013
5	I7520703	HJ9	11/20/013	22	I7520720	T18	11/20/013
6	I7520704	T03	11/20/013	23	I7520721	T18G	11/20/013
7	I7520705	T03G	11/20/013	24	I7520722	T19	11/20/013
8	I7520706	T04	11/20/013	25	I7520723	T20	11/20/013
9	I7520707	T05	11/20/013	26	I7520724	T21	11/20/013
10	I7520708	T06	11/20/013				
11	I7520709	T07	11/20/013				
12	I7520710	T08	11/20/013				
13	I7520711	T09	11/20/013				
14	I7520712	T10	11/20/013				
15	I7520713	T11	11/20/013				
16	I7520714	T12	11/20/013				
17	I7520715	T13	11/20/013				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2015.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Job 533050	Truss CJ1	Truss Type JACK	Qty 10	Ply 1	GIEBEIG - Lot 8 Unit 3 Mayfair Job Reference (optional) 7.360 s Sep 27 2012 MiTek Industries, Inc. Tue Nov 19 08:54:20 2013 Page 1 ID:9B5QRIZPhUL0yMYqzVn3hhzz67b-vhlppz8d8GTnvch?2xGQ5pe4037RrqZAq60NYCyHg7X	I7520699
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Builders FirstSource, Lake City FL 32055

Scale = 1/8"

Plate Offsets (X,Y) [2:0-6:0,0-0-14]					
LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	I/defl	L/d
TCLL 20.0	Plates Increase 1.25	TC 0.25	Vert(LL) 0.00 7	>999	240
TCDL 7.0	Lumber Increase 1.25	BC 0.05	Vert(TL) 0.00 7	>999	180
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00 2	n/a	n/a
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)			
			PLATES GRIP MT20 244/190		
			Weight 7 lb FT = 20%		

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

REACTIONS (lb/size) 2=182/0-3-8 (min 0-1-8), 4=-27/Mechanical, 3=-14/Mechanical
Max Horz 2=52(LC 12)
Max Uplift 2=-102(LC 12), 4=-34(LC 2) 3=-18(LC 2)
Max Grav 2=223(LC 2), 4=21(LC 12) 3=11(LC 16)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.

NOTES (7-9)
1) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf h=18ft; Cat. II Exp B Endl GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 2, 34 lb uplift at joint 4 and 18 lb uplift at joint 3.
6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
8) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
9) Truss Design Engineer: Julius Lee, PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

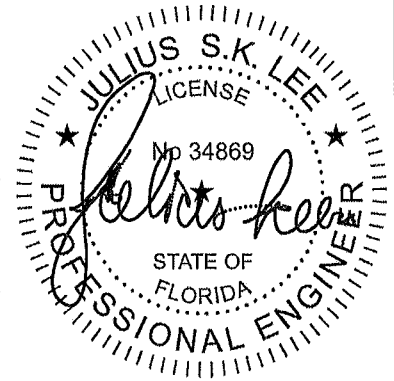
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

November 20,2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435

Job 533050	Truss CJ5	Truss Type JACK	Qty 10	Ply 1	GIEBEIG Lot 8 Unit 3 Mayfair Job Reference (optional)	I7520701																																										
Builders FirstSource Lake City, FL 32055		7.350 s Sep 27 2012 MiTek Industries, Inc. Tue Nov 19 08:54:22 2013 Page 1																																														
ID 9B5QRtZPhUL0yMYqzVn3hhzz87b-r4taOU9tgujU8vrNAMIuBEJQWmtUk3THQItC4yHg7V																																																
Scale = 1/20.3																																																
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td colspan="2">Plate Offsets (X,Y) [2-0-8-0,0-1-2]</td> </tr> <tr> <td style="width:20%;">LOADING (psf)</td> <td style="width:20%;">SPACING 2-0-0</td> <td style="width:10%;">CSI</td> <td style="width:10%;">DEFL</td> <td style="width:10%;">In (loc)</td> <td style="width:10%;">I/defl</td> <td style="width:10%;">L/d</td> <td style="width:20%;">PLATES GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase 1.25</td> <td>TC 0.25</td> <td>Vert(LL) 0.05</td> <td>4-7</td> <td>>999</td> <td>240</td> <td>MT20 244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Lumber Increase 1.25</td> <td>BC 0.18</td> <td>Vert(TL) 0.04</td> <td>4-7</td> <td>>999</td> <td>180</td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Rep Stress Incr YES</td> <td>WB 0.00</td> <td>Horz(TL) -0.00</td> <td>2</td> <td>n/a</td> <td>n/a</td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Code FBC2010/TPI2007</td> <td>(Matrix-M)</td> <td></td> <td></td> <td></td> <td></td> <td>Weight: 19 lb FT = 20%</td> </tr> </table>							Plate Offsets (X,Y) [2-0-8-0,0-1-2]		LOADING (psf)	SPACING 2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES GRIP	TCLL 20.0	Plates Increase 1.25	TC 0.25	Vert(LL) 0.05	4-7	>999	240	MT20 244/190	TCDL 7.0	Lumber Increase 1.25	BC 0.18	Vert(TL) 0.04	4-7	>999	180		BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	2	n/a	n/a		BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)					Weight: 19 lb FT = 20%
Plate Offsets (X,Y) [2-0-8-0,0-1-2]																																																
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES GRIP																																									
TCLL 20.0	Plates Increase 1.25	TC 0.25	Vert(LL) 0.05	4-7	>999	240	MT20 244/190																																									
TCDL 7.0	Lumber Increase 1.25	BC 0.18	Vert(TL) 0.04	4-7	>999	180																																										
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	2	n/a	n/a																																										
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)					Weight: 19 lb FT = 20%																																									
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2			BRACING TOP CHORD BOT CHORD				Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing <div style="border: 1px solid black; padding: 2px; font-size: small;"> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>																																									
REACTIONS (lb/size) 3=79/Mechanical 2=253/0-3-8 (min. 0-1-8), 4=23/Mechanical Max Horz 2=128(LC 12) Max Uplift 3=70(LC 12) 2=-105(LC 12) 4=-26(LC 9) Max Grav 3=97(LC 2) 2=304(LC 2) 4=56(LC 3)																																																
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-281/457 BOT CHORD 2-4=-740/436																																																
NOTES (7-9) 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp B, End. GCpl=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No 2 crushing capacity of 565 psi 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 3, 105 lb uplift at joint 2 and 28 lb uplift at joint 4. 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 8) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package) 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435																																																
LOAD CASE(S) Standard																																																



November 20, 2013

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Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 533050	Truss HJ9	Truss Type MONO TRUSS	Qty 5	Ply 1	GIEBEIG - Lot 8 Unit 3 Mayfair Job Reference (optional)	17520703
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Builders FirstSource, Lake City FL 32055 7.350 s Sep 27 2012 MiTek Industries, Inc. Tue Nov 19 08:54:24 2013 Page 1
 ID 9B5QRZPhUL0yMYqzVn3hhzz6?b-nS7KpAB8CVzCOD_mHnKMGfphFgMknalmikMahzyHg7T

Plate Offsets (X,Y) [2-0-3-7,0-0-13]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.55	in (loc) l/defl L/d	MT20	244/190
TCDL 7 0	Plates Increase 1.25	BC 0.54	Vert(LL) 0.09 6-7 >999 240		
BCLL 0 0 *	Lumber Increase 1.25	WB 0.27	Vert(TL) -0 12 6-7 >997 180		
BCDL 5 0	Rep Stress Incr NO	(Matrix-M)	Horz(TL) -0 01 5 n/a n/a		
	Code FBC2010/TPI2007			Weight: 44 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 7-0-4 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=132/Mechanical 2=339/0-5-11 (min 0-1-8) 5=188/Mechanical
 Max Horz 2=188(LC 4)
 Max Uplift 4=124(LC 10), 2=377(LC 4) 5=232(LC 10)
 Max Grav 4=132(LC 1), 2=486(LC 2) 5=239(LC 3)

FORCES (lb) Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.
 TOP CHORD 2-11=-538/556, 11-12=-472/549 3-12=-472/543
 BOT CHORD 2-14=-621/476, 14-15=-621/476 7 15=-621/476 7-16=-621/476, 6-16=-621/476
 WEBS 3-6=-504/658

NOTES (10-12)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp B; Encl GCp=0.18 MWFRS (envelope) gable end zone Lumber DOL=1.60 plate grip DOL=1.60
- 2) Concentrated loads from layout are not present in Load Case(s) #2 Regular Only: #6 MWFRS Wind Left Positive; #7 MWFRS Wind Right Positive #12 MWFRS 1st Wind Parallel Positive; #13 MWFRS 2nd Wind Parallel Positive #14 Live Only: #18 MWFRS Wind Left Positive + Regular #19 MWFRS Wind Right Positive + Regular #20 MWFRS 1st Wind Parallel Positive + Regular #21 MWFRS 2nd Wind Parallel Positive + Regular
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 4, 377 lb uplift at joint 2 and 232 lb uplift at joint 5.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 37 lb up at 1-5-12 37 lb up at 1-5-12 10 lb down and 23 lb up at 4-3-11, 10 lb down and 23 lb up at 4-3-11 and 49 lb down and 75 lb up at 7-1-10 and 49 lb down and 75 lb up at 7-1-10 on top chord, and 19 lb up at 1-5-12 19 lb up at 1-5-12, 12 lb down and 33 lb up at 4-3-11 12 lb down and 33 lb up at 4-3-11 and 42 lb down and 52 lb up at 7 1 10 and 42 lb down and 52 lb up at 7-1 10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

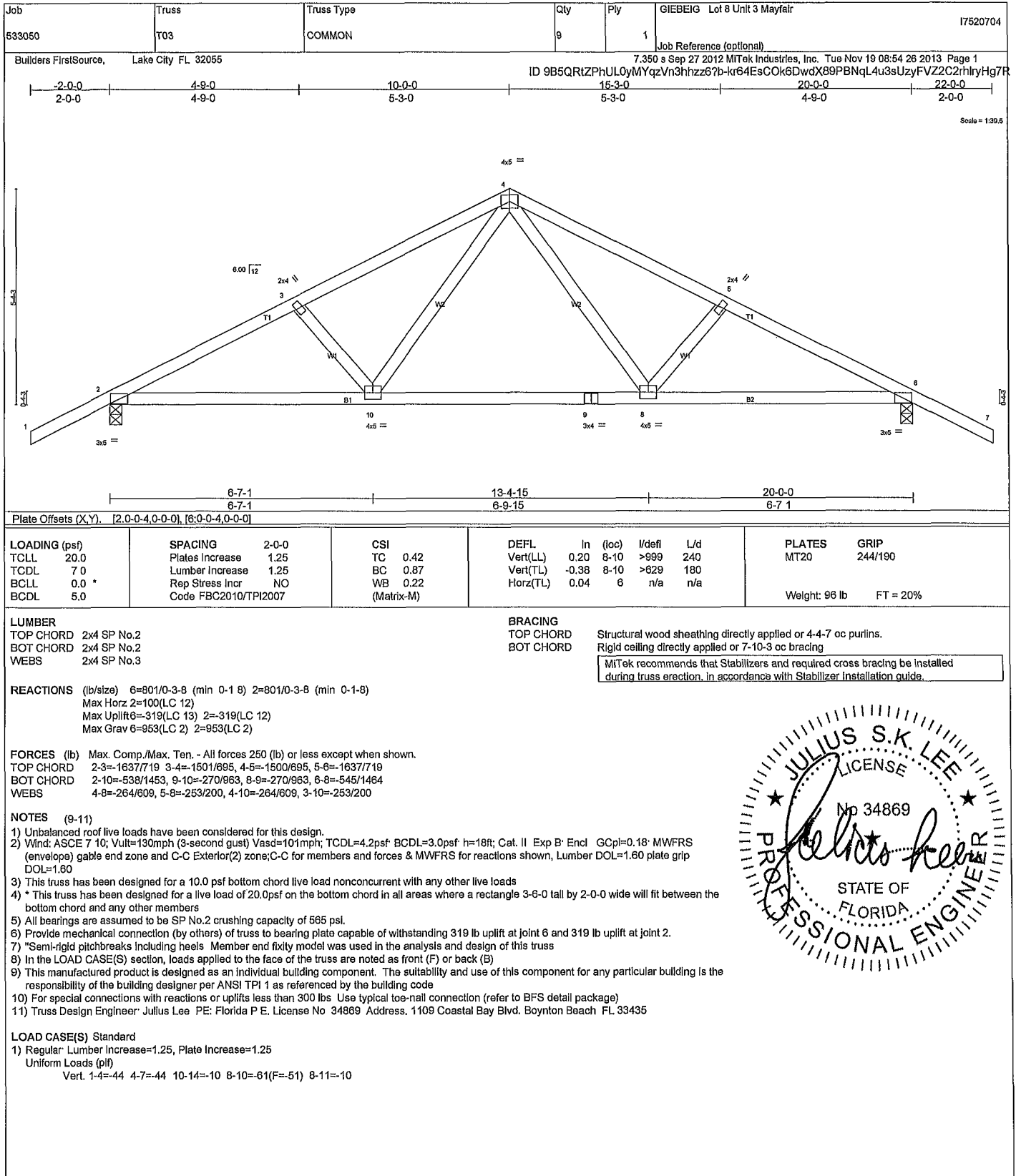
Continued on page 2



November 20, 2013

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 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach FL 33435



November 20,2013

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Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job 533050	Truss T04	Truss Type HIP	Qty 1	Ply 1	GIEBEIG - Lot 8 Unit 3 Mayfair Job Reference (optional)	17520706
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Builders FirstSource Lake City FL 32055 7.350 s Sep 27 2012 MITEK Industries, Inc. Tue Nov 19 08:54:29 2013 Page 1
ID 9B5QRIZPhUL0yMYqzVn3hhzz67b-8QoDsuEG11cVU_tk4KwXzIWTth7PSIBVU04LMayHg70

Scale = 1:56.6

Plate Offsets (X,Y)	[2:0-10-4,0-1-9], [5:0-4-0,0-3-0], [7:0-10-4,0-1-9], [10:0-4-0,0-4-8]
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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.83	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.82	Vert(LL) 0.32 9-10 >999 240		
BCCL 0.0 *	Rep Stress Incr NO	WB 0.87	Vert(TL) -0.49 10-11 >731 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.14 7 n/a n/a		
				Weight: 164 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.1 *Except*
T2 T3, 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied or 5-0-7 oc bracing.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection. In accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1539/0-3-8 (min. 0-2-2), 7=1540/0-3-8 (min. 0-2-2)
Max Horz 2=45(LC 8)
Max Uplift 2=908(LC 8) 7=908(LC 9)
Max Grav 2=1823(LC 2) 7=1823(LC 2)

FORCES (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=3461/1916, 3-16=3068/1766, 16-17=3068/1766, 4-17=3068/1766, 4-18=4045/2211
18-19=4045/2211 19-20=4045/2211 5-20=4045/2211 5-21=3068/1766, 21-22=3068/1766
6-22=3068/1766 6-7=3460/1916
BOT CHORD 2-11=1671/3025 11-23=2116/3981 23-24=2116/3981 24-25=2116/3981 10-25=2116/3981
10-26=2098/3984, 26-27=2098/3984 27-28=2098/3984, 8-28=2098/3984, 7-9=1640/3025
WEBS 3-11=605/1020, 4-11=1187/561 4-10=100/286, 5-10=99/286, 5-9=1190/563, 6-9=605/1019

NOTES (11 13)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3.0psf; h=18ft, Cat. II Exp B Encl. GCpl=0.18 MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=lb) 2=908 7=908.
8) "Semi-rigid" pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 163 lb down and 204 lb up at 7-0-0 85 lb down and 72 lb up at 9-0-12, 85 lb down and 72 lb up at 11-0-12 85 lb down and 72 lb up at 13-0-12 85 lb down and 72 lb up at 15-0-8, 85 lb down and 72 lb up at 17-0-4, 85 lb down and 72 lb up at 19-0-4, and 85 lb down and 72 lb up at 21-0-4, and 163 lb down and 204 lb up at 23-1-0 on top chord and 258 lb down and 286 lb up at 7-0-0, 49 lb down and 44 lb up at 9-0-12 49 lb down and 44 lb up at 11-0-12, 49 lb down and 44 lb up at 13-0-12 49 lb down and 44 lb up at 15-0-8, 49 lb down and 44 lb up at 17-0-4, 49 lb down and 44 lb up at 19-0-4, and 49 lb down and 44 lb up at 21-0-4, and 258 lb down and 286 lb up at 23-0-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note: Visually graded lumber designation SP, represents new lumber design values as per SPIB.
13) Truss Design Engineer Julius Lee, PE, Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
Continued on page 2



November 20,2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB 89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,
1109 Coastal Bay
Boynton Beach, FL 33435

Job 533050	Truss T05	Truss Type HIP	Qty 1	Ply 1	GIEBEIG Lot 8 Unit 3 Mayfair Job Reference (optional)	I7520707
Builders FirstSource Lake City, FL 32055		7 350 s Sep 27 2012 MiTek Industries, Inc. Tue Nov 19 08:54:31 2013 Page 1				
ID 9B5QRtZPhUL0yMYqzVn3hhzz67b-4pwwHZGXyfsDj16Cly727bvrVkwieoMKZSR3yHg7M						
<div style="display: flex; justify-content: space-between;"> <div style="width: 45%;"> <p style="margin: 0;">-2-0-0 4-9-5 9-0-0 15-0-8 21-1-0 26-3-11 30-1-0 32-1-0</p> <p style="margin: 0;">2-0-0 4-9-5 4-2-11 6-0-8 6-0-8 4-2-11 4-9-5 2-0-0</p> </div> <div style="width: 5%; text-align: right;">Scale = 1:50.0</div> </div>						
<div style="display: flex; justify-content: space-between; margin-top: 10px;"> <div style="width: 45%;"> <p style="margin: 0;">9-0-0 9-0-0 15-0-8 21-1-0 30-1-0</p> <p style="margin: 0;">6-0-8 6-0-8 9-0-0</p> </div> <div style="width: 5%;"></div> </div>						
Plate Offsets (X,Y): [2-0-2-10,0-1-8], [8,0-2-10,0-1-8], [11,0-3-0,0-3-0]						
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase 1.25	TC 0.40	Vert(LL) -0 12 10-15	>999	240	
TCDL 7 0	Lumber Increase 1.25	BC 0.58	Vert(TL) -0 24 10-15	>999	180	
BCLL 0 0 *	Rep Stress Incr YES	WB 0.45	Horz(TL) 0.09 8	n/a	n/a	
BCDL 5 0	Code FBC2010/TPI2007	(Matrix-M)				
			PLATES		GRIP	
			MT20		244/190	
			Weight: 154 lb FT = 20%			
<div style="display: flex; justify-content: space-between;"> <div style="width: 45%;"> <p>LUMBER</p> <p>TOP CHORD 2x4 SP No.2</p> <p>BOT CHORD 2x4 SP No.2</p> <p>WEBS 2x4 SP No.3</p> </div> <div style="width: 50%;"> <p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 4-3-11 oc purlins.</p> <p>BOT CHORD Rigid ceiling directly applied or 7-8-11 oc bracing</p> </div> </div>						
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.						
<p>REACTIONS (lb/size) 8=900/0-3-8 (min. 0-1-8) 2=900/0-3-8 (min. 0-1-8)</p> <p>Max Horz 2=59(LC 10)</p> <p>Max Uplift 8=138(LC 13) 2=138(LC 12)</p> <p>Max Grav 8=1071(LC 2) 2=1071(LC 2)</p>						
<p>FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.</p> <p>TOP CHORD 2-3=1740/765, 3-4=1508/661, 4-5=1332/633, 5-6=1332/633, 6-7=1508/660 7-8=1740/765</p> <p>BOT CHORD 2-12=575/1505, 11 12=522/1583 10-11=522/1583 8-10=582/1505</p> <p>WEBS 3-12=258/207 4-12=137/396, 5-12=433/153, 5-10=433/153, 6-10=137/395, 7 10=258/206</p>						
<p>NOTES (9-11)</p> <p>1) Unbalanced roof live loads have been considered for this design</p> <p>2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II Exp B, Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</p> <p>3) Provide adequate drainage to prevent water ponding</p> <p>4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</p> <p>5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.</p> <p>6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi</p> <p>7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 8=138, 2=138.</p> <p>8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.</p> <p>9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.</p> <p>10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)</p> <p>11) Truss Design Engineer: Julius Lee, PE: Florida P E License No. 34869; Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435</p>						
LOAD CASE(S) Standard						



November 20,2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

November 20, 2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIU-7473 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	GIEBEIG - Lot 8 Unit 3 Mayfair	17520711
533050	T09	MONO HIP	1	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055

7.350 s Sep 27 2012 MITek Industries Inc. Tue Nov 19 08:54:38 2013 Page 1

-2-0-0 7-0-0 12-4-0 18-6-7 24-9-0 30-1-0
2-0-0 7-0-0 5-4-0 6-2-7 6-2-9 5-4-0

Scale = 1:54.3

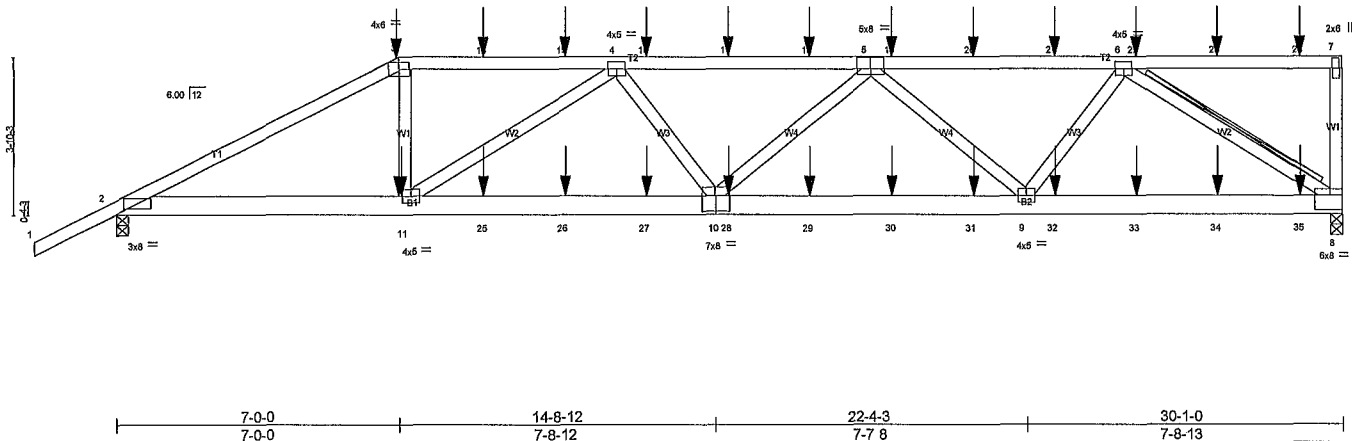


Plate Offsets (X,Y): [2:0-10-4-0-1-9], [5:0-4-0-0-3-4], [8:Edge,0-4-4], [10:0-4-0-0-4-8]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.89	Vert(LL) 0.29	10-11	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.81	Vert(TL) -0.45	10	>792	180		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.86	Horz(TL) 0.12	8	n/a	n/a		
BCDL 5.0	Code FBC2010/TP12007	(Matrix-M)					Weight: 173 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except*
T1 2x4 SP No.1
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-11 oc purlins, except end verticals
Rigid ceiling directly applied or 5-1 15 oc bracing
T Brace: 2 X 6 SYP No.2 6-8
Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c. with 3in minimum end distance.
Brace must cover 90% of web length

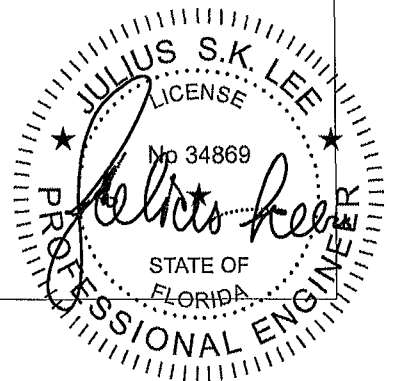
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=1517/0-3-8 (min. 0-2-2) 2=1507/0-3-8 (min. 0-2-2)
Max Horz 2=116(LC 8)
Max Uplift 8=885(LC 5) 2=846(LC 8)
Max Grav 8=1796(LC 2) 2=1786(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3376/1804, 3-14=-2991/1654, 14-15=-2991/1654 4-15=-2991/1654 4-16=-3883/2009, 16-17=-3883/2009 17 18=-3883/2009, 5-18=-3883/2009 5-19=-2912/1462 19-20=-2912/1462 20-21=-2912/1462, 6-21=-2912/1462
BOT CHORD 2-11=-1631/2950, 11-25=-2015/3875 25-26=-2015/3875, 26-27=-2015/3875, 10-27=-2015/3875, 10-28=-1878/3731 28-29=-1878/3731 29-30=-1878/3731, 30-31=-1878/3731 9-31=-1878/3731 9-32=-1123/2279 32-33=-1123/2279 33-34=-1123/2279, 34-35=-1123/2279 8-35=-1123/2279
WEBS 3-11=-550/983, 4-11=-1146/468 5-10=-194/342 5-9=-1108/562, 6-9=-596/1112, 6-8=-2699/1331

NOTES (12-14)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4 2psf BCDL=3.0psf h=18ft; Cat. II Exp B Encl, GCpl=0.18, MWFRS (envelope) Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=885, 2=846.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 163 lb down and 204 lb up at 7-0-0, 85 lb down and 72 lb up at 9-0-12, 85 lb down and 72 lb up at 11-0-12, 85 lb down and 72 lb up at 13-0-12, 85 lb down and 72 lb up at 15-0-12, 85 lb down and 72 lb up at 17-0-12, 85 lb down and 72 lb up at 19-0-12, 85 lb down and 72 lb up at 21-0-12, 85 lb down and 72 lb up at 23-0-12, 85 lb down and 72 lb up at 25-0-12, and 85 lb down and 72 lb up at 27-0-12, and 85 lb down and 72 lb up at 29-0-12 on top chord and 258 lb down and 286 lb up at 7-0-0, 49 lb down and 44 lb up at 9-0-12, 49 lb down and 44 lb up at 11-0-12, 49 lb down and 44 lb up at 13-0-12, 49 lb down and 44 lb up at 15-0-12, 49 lb down and 44 lb up at 17-0-12, 49 lb down and 44 lb up at 19-0-12, 49 lb down and 44 lb up at 21-0-12, 49 lb down and 44 lb up at 23-0-12, 49 lb down and 44 lb up at 25-0-12, and 49 lb down and 44 lb up at 27-0-12, and 49 lb down and 44 lb up at 29-0-12 on bottom chord The design/selection of such connection device(s) is the responsibility of others.

Continued on page 2



November 20, 2013

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Julius Lee PE,
1109 Coastal Bay
Boynton Beach FL 33435

Job 533050	Truss T10	Truss Type MONO HIP	Qty 1	Ply 1	GIEBEIG Lot 8 Unit 3 Mayfair Job Reference (optional)	I7520712
Builders FirstSource, Lake City FL 32055		7.350 s Sep 27 2012 MiTek Industries, Inc. Tue Nov 19 08:54:37 2013 Page 1				
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<div style="display: flex; justify-content: space-between;"> -2-0-0 2-0-0 4-9-5 4-9-5 9-0-0 4-2-11 15-11-9 6-11-9 23-1-7 7-1-14 30-1-0 6-11-9 </div>						
Scale = 1/8" = 1'-0"						

Job	Truss	Truss Type	Qty	Ply	GIEBEIG - Lot 8 Unit 3 Mayfair	17520716
533050	T14	SPECIAL	2	1	Job Reference (optional)	

Builders FirstSource, Lake City FL 32055

7.350 s Sep 27 2012 MITek Industries, Inc. Tue Nov 19 08:54.43 2013 Page 1
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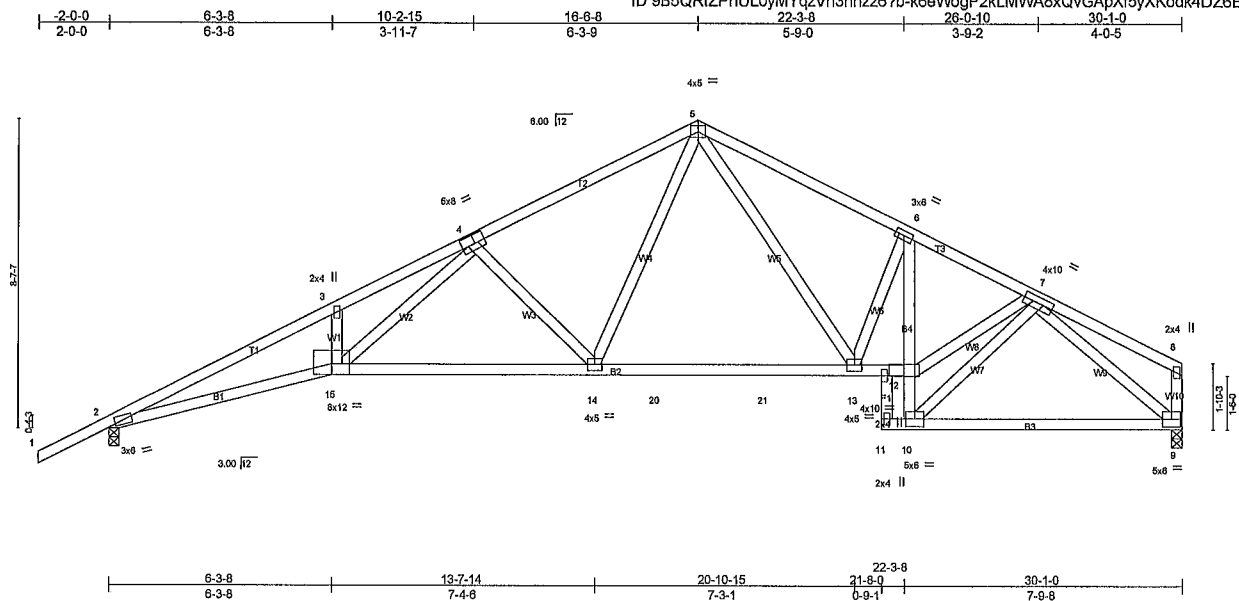


Plate Offsets (X,Y), [4,0-3-8,0-3-0], [16,0-2-0,0-0-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.49	In (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.76	Vert(LL) 0.25 14-15 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.64	Vert(TL) -0.50 14-15 >724 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0.27 9 n/a n/a		
	Code FBC2010/TPI2007			Weight: 176 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-11-0 oc purlins, except end
BOT CHORD 2x4 SP No.2 *Except*	verticals.
B4, 2x4 SP No.3	Rigid ceiling directly applied or 5-4-5 oc bracing. Except.
WEBS 2x4 SP No.3 *Except*	9-5-0 oc bracing 10-12
W10: 2x4 SP No.2	

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=943/0-3-8 (min. 0-1-8) 9=859/0-3-8 (min. 0-1-8)
Max Horz 2=130(LC 12)
Max Uplift 2=171(LC 12) 9=127(LC 13)
Max Grav 2=1074(LC 2), 9=957(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-3339/1354, 3-4=-3233/1480, 4-5=-1698/752 5-6=-1707/778, 6-7=-1698/728
BOT CHORD 2-15=-1224/3011 14-15=-733/1907 14-20=-327/1114 20-21=-327/1114, 13-21=-327/1114,
12-13=-545/1497 10-12=-341/881 9-10=-395/987
WEBS 4-15=-635/1403, 4-14=-739/429, 5-14=-290/739 5-13=-248/556, 6-13=-284/227 7 12=-585/1519
7-10=-1171/520, 7-9=-1274/519

NOTES (9-11)

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp B Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
- 5) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 2=171 9=127
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) For special connections with reactions or uplifts less than 300 lbs Use typical toe-nail connection (refer to BFS detail package)
- 11) Truss Design Engineer: Julius Lee PE, Florida P E License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



November 20,2013

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Julius Lee PE,
1109 Coastal Bay
Boynton Beach FL 33435

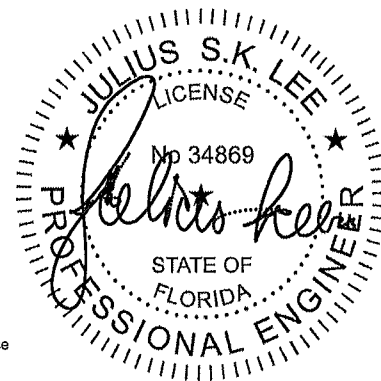
Job 533050	Truss T16	Truss Type SPECIAL	Qty 1	Ply 1	GIEBEIG - Lot 8 Unit 3 Mayfair Job Reference (optional)	17520718																																																						
Builders FirstSource, Lake City, FL 32055		7 350 s Sep 27 2012 MiTek Industries, Inc. Tue Nov 19 08:54:46 2013 Page 1																																																										
ID 9B5QRtZPhUL0yMYqzVn3hzz57b-BhKeQISx0GI41bg7aOkW9HJTBYqqxSW7p9hThyHg77																																																												
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:20%;">Plate Offsets (X,Y)</td> <td>[3,0-3-0,0-3-0], [4,0-6-0,0-2-8], [6,0-3-0,0-3-0], [7,0-2-10,0-1 8]</td> </tr> </table>							Plate Offsets (X,Y)	[3,0-3-0,0-3-0], [4,0-6-0,0-2-8], [6,0-3-0,0-3-0], [7,0-2-10,0-1 8]																																																				
Plate Offsets (X,Y)	[3,0-3-0,0-3-0], [4,0-6-0,0-2-8], [6,0-3-0,0-3-0], [7,0-2-10,0-1 8]																																																											
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:10%;">CSI</td> <td style="width:10%;">DEFL</td> <td style="width:10%;">in (loc)</td> <td style="width:10%;">l/defl</td> <td style="width:10%;">L/d</td> <td style="width:10%;">PLATES</td> <td style="width:10%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase 1.25</td> <td>TC 0.46</td> <td>Vert(LL) -0.29</td> <td>12-13</td> <td>>999</td> <td>240</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Lumber Increase 1.25</td> <td>BC 0.73</td> <td>Vert(TL) -0.63</td> <td>12-13</td> <td>>571</td> <td>180</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Rep Stress Incr YES</td> <td>WB 0.61</td> <td>Horz(TL) 0.21</td> <td>7</td> <td>n/a</td> <td>n/a</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Code FBC2010/TPI2007</td> <td>(Matrix-M)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td colspan="7"></td> <td colspan="2">Weight: 158 lb FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	TCLL 20.0	Plates Increase 1.25	TC 0.46	Vert(LL) -0.29	12-13	>999	240	MT20	244/190	TCDL 7.0	Lumber Increase 1.25	BC 0.73	Vert(TL) -0.63	12-13	>571	180			BCLL 0.0 *	Rep Stress Incr YES	WB 0.61	Horz(TL) 0.21	7	n/a	n/a			BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)														Weight: 158 lb FT = 20%	
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<div style="display: flex; justify-content: space-between;"> <div style="width: 45%;"> <p>LUMBER</p> <p>TOP CHORD 2x4 SP No.2</p> <p>BOT CHORD 2x4 SP No.2</p> <p>WEBS 2x4 SP No.3</p> </div> <div style="width: 45%;"> <p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 2-10-8 oc purlins.</p> <p>BOT CHORD Rigid ceiling directly applied or 5-5-4 oc bracing.</p> </div> </div>																																																												
<p>REACTIONS (lb/size) 1=807/0-3-8 (min. 0-1-8) 7=905/0-3-8 (min. 0-1-8)</p> <p>Max Horz 1=89(LC 12)</p> <p>Max Uplift 1=-141(LC 12) 7=-153(LC 13)</p> <p>Max Grav 1=957(LC 2) 7=1077(LC 2)</p>																																																												
<p>FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.</p> <p>TOP CHORD 1-2=-3200/1333, 2-3=-3202/1448, 3-4=-1386/644, 4-5=-1168/613, 5-6=-1346/628 6-7=-1744/741</p> <p>BOT CHORD 1 13=-1167/2895, 12-13=-683/1791 11 12=-324/1163, 10-11=-324/1163, 9-10=-549/1498, 7-8=-548/1499</p> <p>WEBS 3-13=-635/1420, 3-12=-785/438, 4-12=-194/523, 5-10=-136/341 6-10=-447/265</p>																																																												
<p>NOTES (10-12)</p> <p>1) Unbalanced roof live loads have been considered for this design</p> <p>2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II, Exp B' Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60</p> <p>3) Provide adequate drainage to prevent water ponding</p> <p>4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</p> <p>5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members</p> <p>6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi</p> <p>7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface</p> <p>8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=141 7=153.</p> <p>9) Semi-rigid pitchbreaks including heels. Member end fixity model was used in the analysis and design of this truss.</p> <p>10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code</p> <p>11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)</p> <p>12) Truss Design Engineer Julius Lee PE: Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435</p>																																																												
<p>LOAD CASE(S) Standard</p>																																																												

November 20, 2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435



Job 533050	Truss T18	Truss Type COMMON	Qty 1	Ply 1	GIEBEIG - Lot 8 Unit 3 Mayfair Job Reference (optional)	17520720
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Builders FirstSource, Lake City FL 32055
7,350 s Sep 27 2012 MiTek Industries, Inc. Tue Nov 19 08:54:48 2013 Page 1

ID 9B5QRiZPhUL0yMYqzVn3hhzz87b-44RPnNTBYt?oGvqNhpm_ElosyLf2PVoiGTArXeyHg75

Scale = 1:23.3

Plate Offsets (X,Y): [2-0-6-0,0-0-14], [4-0-6-0,0-0-14]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.25	Vert(LL) 0.02	6-12 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.11	Vert(TL) -0.01	6-9 >999 180	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.05	Horz(TL) 0.00	4 n/a n/a	
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)			
Weight: 36 lb FT = 20%						

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 2=313/0-3-8 (min. 0-1-8) 4=313/0-3-8 (min. 0-1-8)

Max Horz 2=-51(LC 13)

Max Uplift 2=-134(LC 12) 4=-134(LC 13)

Max Grav 2=375(LC 2) 4=375(LC 2)

FORCES (lb) - Max. Comp./Max. Terf. All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-296/386, 3-4=-296/386

BOT CHORD 2-6=-281/241 4-6=-296/261

NOTES (8-10)

- Unbalanced roof live loads have been considered for this design
- Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4 2psf; BCDL=3.0psf; h=18ft, Cat. II Exp B, Encl GCpi=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 2=134, 4=134.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- For special connections with reactions or uplifts less than 300 lbs Use typical toe-nail connection (refer to BFS detail package)
- Truss Design Engineer: Julius Lee PE: Florida P E License No 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

November 20,2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435

Job 533050	Truss T19	Truss Type COMMON	Qty 3	Ply 1	GIEBEIG - Lot 8 Unit 3 Mayfair Job Reference (optional)	I7520722
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Builders FirstSource Lake City, FL 32055
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ID 9B5QRIZPhUL0yMYqzVn3hhzz67b-1TZ9G3VR4UFWVD_mpEoSJ7ICR9LRtPGbjnfycSyHg73

Scale = 1:19.6

Plate Offsets (X,Y). [2.0-6.0,0-0-14], [4.0-6.0,0-0-14]									
LOADING (psf)		SPACING		CSI		DEFL		PLATES GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.02	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	0.02		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.00		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)					
								Weight: 33 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins

Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=221/0-3-8 (min 0-1 B) 2=317/0-3-8 (min 0-1-8)

Max Horz 2=68(LC 16)

Max Uplift 4=100(LC 8) 2=135(LC 12)

Max Grav 4=262(LC 2) 2=379(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-312/406, 3-4=-325/476

BOT CHORD 2-5=-321/247 4-5=-569/407

NOTES (8-10)

1) Unbalanced roof live loads have been considered for this design

2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp B, Encl GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members

5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (I=lb) 4=100, 2=135.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) For special connections with reactions or uplifts less than 300 lbs Use typical toe-nail connection (refer to BFS detail package)

10) Truss Design Engineer: Julius Lee PE: Florida P.E. License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard

November 20,2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.

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Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435

Job 533050	Truss T21	Truss Type COMMON	Qty 2	Ply 1	GIEBEIG - Lot 8 Unit 3 Mayfair Job Reference (optional)	17520724
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Builders FirstSource, Lake City FL 32055

7.350 s Sep 27 2012 MiTek Industries, Inc. Tue Nov 19 08:54:52 2013 Page 1
ID 9B5QRtZPhUL0yMYqzVn3hhzz67b-zrhwhIWic6VEIW78wfrwOYzVLz_?LICIB583gLyHg7

Plate Offsets (X,Y) [2 0-2-10,0-1-8], [4,0-2-10,0-1-8]								
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	In (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) 0.11	6-12	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.30	Vert(TL) 0.09	6-12	>999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.08	Horz(TL) 0.01	4	n/a	n/a		
BCDL 5.0	Code FBC2010/TP12007	(Matrix-M)						
							Weight: 56 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 2=466/0-3-8 (min. 0-1-8) 4=466/0-3-8 (min. 0-1-8)

Max Horz 2=48(LC 11)

Max Uplift 2=-156(LC 9) 4=-156(LC 8)

Max Grav 2=556(LC 2), 4=556(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-619/914, 3-4=-821/918

BOT CHORD 2-6=-1157/852, 4-6=-1177/863

WEBS 3-6=-317/209

NOTES (8-10)

- Unbalanced roof live loads have been considered for this design
- Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp B Encl. GCp=0.18 MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 2=156 4=156.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- Truss Design Engineer- Julius Lee, PE, Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 8-8-11 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



November 20, 2013

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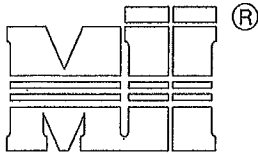
Julius Lee PE,
1109 Coastal Bay
Boynton Beach FL 33435

August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2

MiTek Industries, Chesterfield, MO Page 1 of 1



MiTek Industries, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

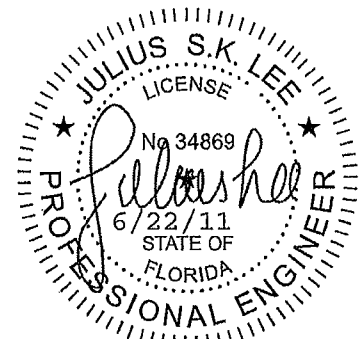
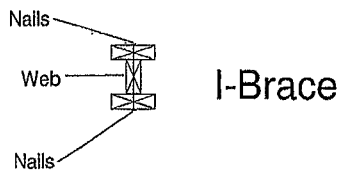
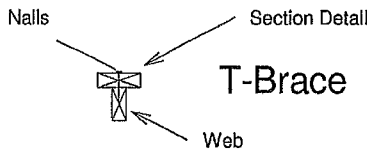
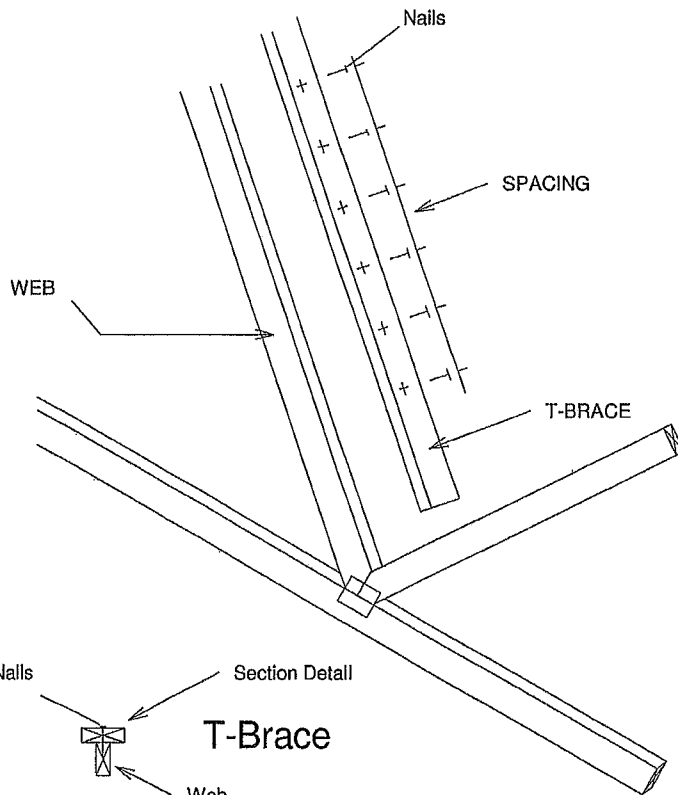
Brace Size for One-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.

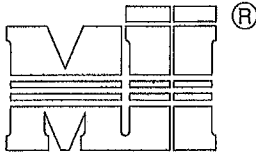


1109 COASTAL BAY
BOYNTON BC, FL 33435

JANUARY 1, 2009

LATERAL TOE-NAIL DETAIL

ST-TOENAIL_SP



MiTek Industries, Inc.

MiTek Industries, Chesterfield, MO Page 1 of 1

NOTES

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
 APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

EXAMPLE

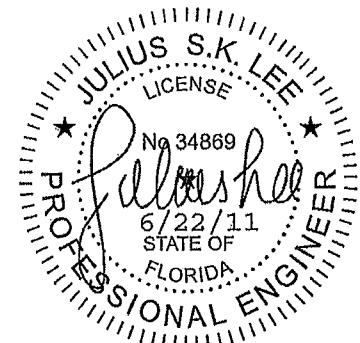
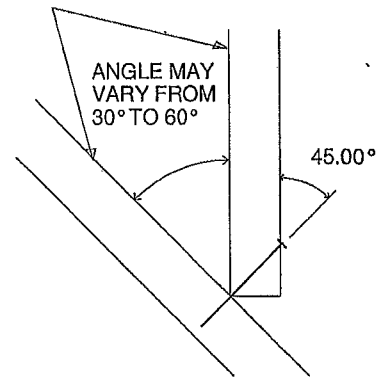
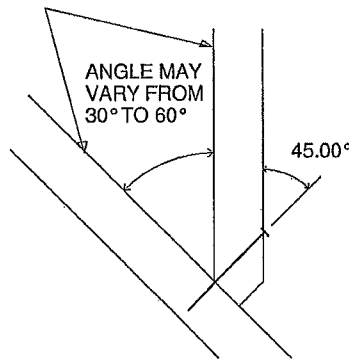
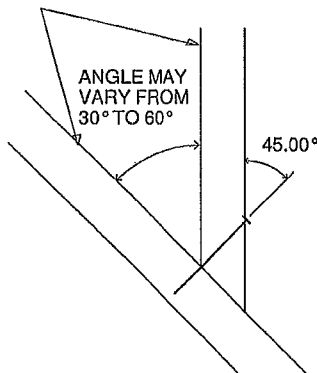
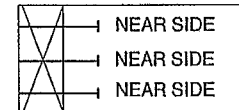
(3) - 16d NAILS (.162" diam x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

SIDE VIEW

3 NAILS



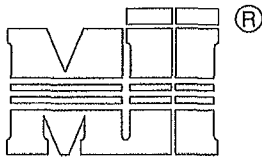
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FEBRUARY 14, 2012

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

MITek Industries, Chesterfield, MO

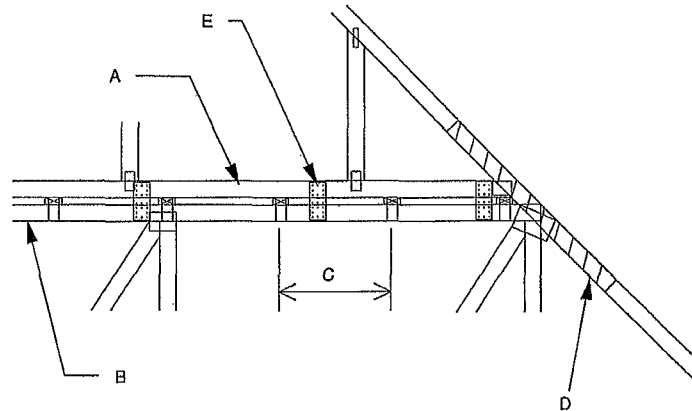


MITek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.80

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES)
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

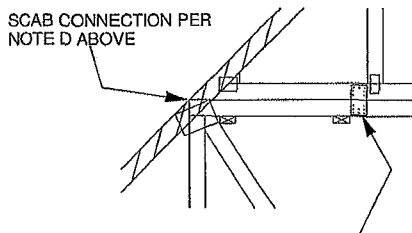
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH
- D - 2 X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND
- 1 WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



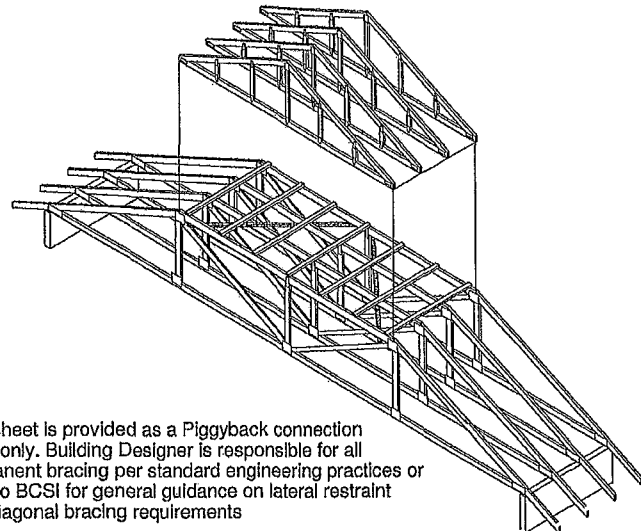
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING

SCAB CONNECTION PER
NOTE D ABOVE

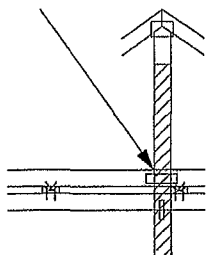


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE



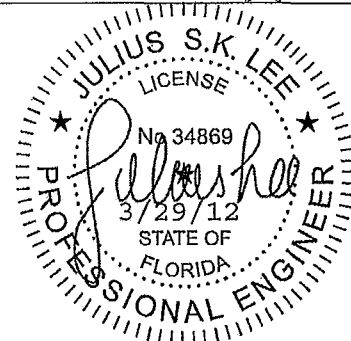
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK

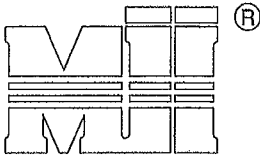


FOR LARGE CONCENTRATED LOADS APPLIED
TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN



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MiTek Industries, Inc.

ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD

Trusses @ 24" o.c.

HORIZONTAL BRACE
(SEE SECTION A-A)2x6 DIAGONAL BRACE SPACED 48" O.C.
ATTACHED TO VERTICAL WITH (4) - 16d
COMMON WIRE NAILS AND ATTACHED
TO BLOCKING WITH (5) - 10d COMMONS.

Roof Sheathing

1'-3"
Max.

IT IS THE RESPONSIBILITY OF THE BLDG DESIGNER OR
THE PROJECT ENGINEER/ARCHITECT TO DESIGN THE
CEILING DIAPHRAGM AND ITS ATTACHMENT TO THE
TRUSSES TO RESIST ALL OUT OF PLANE LOADS THAT
MAY RESULT FROM THE BRACING OF THE GABLE ENDS

NAIL DIAGONAL BRACE TO
PURLIN WITH TWO 16d NAILS2X 4 PURLIN FASTENED TO FOUR TRUSSES
WITH TWO 16d NAILS EACH FASTEN PURLIN
TO BLOCKING W/ TWO 16d NAILS (MIN)Diag. Brace
at 1/3 points
if needed

PROVIDE 2x4 BLOCKING BETWEEN THE TRUSSES
SUPPORTING THE BRACE AND THE TWO TRUSSES
ON EITHER SIDE AS NOTED. TOENAIL BLOCKING
TO TRUSSES WITH (2) - 10d NAILS AT EACH END
ATTACH DIAGONAL BRACE TO BLOCKING WITH
(5) - 10d COMMON WIRE NAILS.

End Wall

CEILING SHEATHING

BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

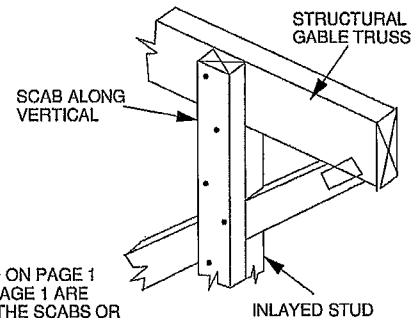
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED.

METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE
FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE
FOLLOWING NAILING SCHEDULE.

METHOD 2 : ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL
MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING
NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE
AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)

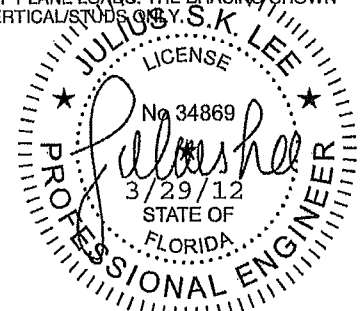


MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1
ALL BRACING METHODS SHOWN ON PAGE 1 ARE
VALID AND ARE TO BE FASTENED TO THE SCABS OR
VERTICAL STUDS OF THE STANDARD GABLE TRUSS
ON THE INTERIOR SIDE OF THE STRUCTURE.

STRUCTURAL
GABLE TRUSS

AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST
BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM
CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN
IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.

NOTE : THIS DETAIL IS TO BE USED ONLY FOR
STRUCTURAL GABLES WITH INLAYED
STUDS TRUSSES WITHOUT INLAYED
STUDS ARE NOT ADDRESSED HERE

STANDARD
GABLE TRUSS

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