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COA #0 278
Florida Certificate of Product Approval #FL1999
07/27/2023

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

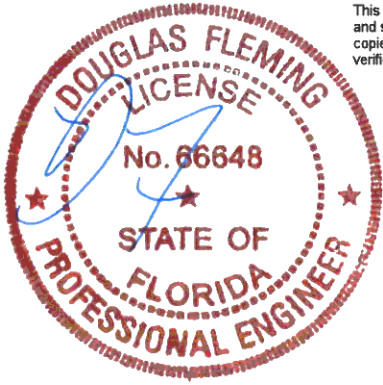
Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 23-9526
Job Description: Karlton	
Address: FL	

Job Engineering Criteria:
Design Code: FBC 7th Ed. 2020 Res.
IntelliVIEW Version: 22.02.00 through 23.01.01B
JRef #: 1XRQ2150001
Wind Standard: ASCE 7-16 Wind Speed (mph): 130
Design Loading (psf): 37.00, 55.00
Building Type: Closed

This package contains general notes pages, 49 truss drawing(s) and 6 detail(s).

Item	Drawing Number	Truss
1	208.23.0620.31811	A1
3	208.23.0803.31643	A2
5	208.23.0803.34280	A4
7	208.23.0803.38040	A6
9	208.23.0803.40970	B1
11	208.23.0804.12710	B2
13	208.23.0620.32593	C1E
15	208.23.0620.32030	C3
17	208.23.0849.22670	D1E
19	208.23.0848.19630	D3
21	208.23.0846.43713	FT2
23	208.23.0845.07380	G1E
25	208.23.0620.31968	G3
27	208.23.0620.31781	G5
29	208.23.0805.06763	H2
31	208.23.0620.32405	H4
33	208.23.0805.37223	H7E
35	208.23.0805.47773	L1
37	208.23.0620.31920	M1E
39	208.23.0805.51313	M3
41	208.23.0620.32078	M4
43	208.23.0620.31733	M5
45	208.23.0620.31873	P1E
47	208.23.0620.32390	P2A
49	208.23.0620.32217	P2E

Item	Drawing Number	Truss
2	208.23.0803.30237	A1E
4	208.23.0803.33047	A3
6	208.23.0803.36577	A5
8	208.23.0803.39700	A6E
10	208.23.0804.11247	B1E
12	208.23.0620.32483	C1
14	208.23.0620.32296	C2
16	208.23.0804.48483	D1
18	208.23.0848.14510	D2
20	208.23.0846.28847	FT1
22	208.23.0844.34403	G1
24	208.23.0620.32187	G2
26	208.23.0620.32452	G4
28	208.23.0620.32545	H1
30	208.23.0805.18760	H3
32	208.23.0805.20487	H7
34	208.23.0805.38600	K1
36	208.23.0620.32139	M1
38	208.23.0805.49757	M2
40	208.23.0620.32108	M3E
42	208.23.0620.32359	M4E
44	208.23.0620.31842	P1
46	208.23.0620.31998	P2
48	208.23.0620.32248	P2B
50	A14015ENC160118	



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Site Information:	Page 2:
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Job Description: Karlton	
Address: FL	

Item	Drawing Number	Truss
51	A14030ENC160118	
53	GBLLETIN0118	
55	160TL	

Item	Drawing Number	Truss
52	BRCLBSUB0119	
54	PB160160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

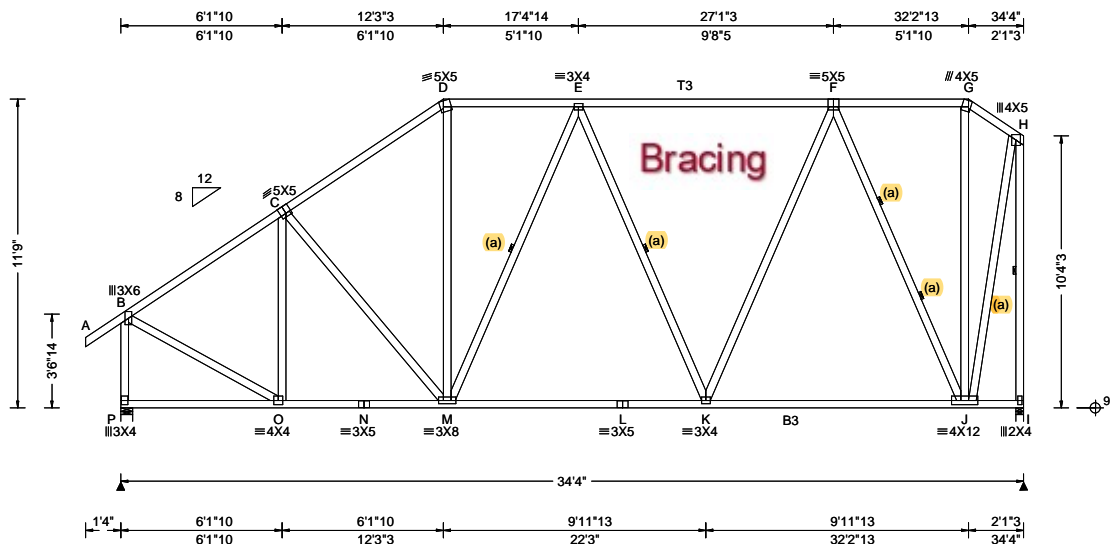
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 568740 / FROM: RFG	COMN Ply: 1 Qty: 7	Job Number: 23-9526 Karlton Truss Label: A1	Cust: R 215 JRef: 1XRQ2150001 T41 DrwNo: 208.23.0620.31811 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 0.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.080 E 999 240 VERT(CL): 0.131 E 999 180 HORZ(LL): 0.030 C - - HORZ(TL): 0.049 C - - Creep Factor: 2.0 Max TC CSI: 0.663 Max BC CSI: 0.864 Max Web CSI: 0.964 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL P 1669 - / - / - / 769 - / 284 I 1670 - / - / - / 666 - / - Non-Gravity Wind reactions based on MWFRS P Brg Wid = 5.5 Min Req = 2.0 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings P & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 22 - 1465 E - F 0 - 1184 C - D 9 - 1524 G - H 0 - 433 D - E 42 - 1186

Lumber

Top chord: 2x4 SP #2; T3 2x4 SP M-31;
Bot chord: 2x4 SP #2; B3 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

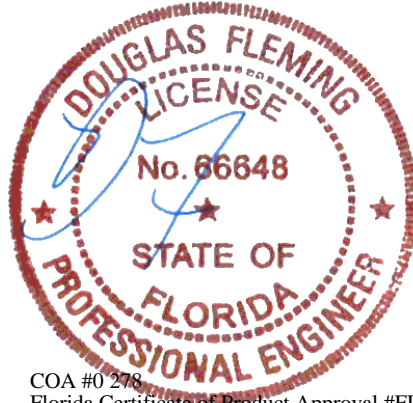
Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

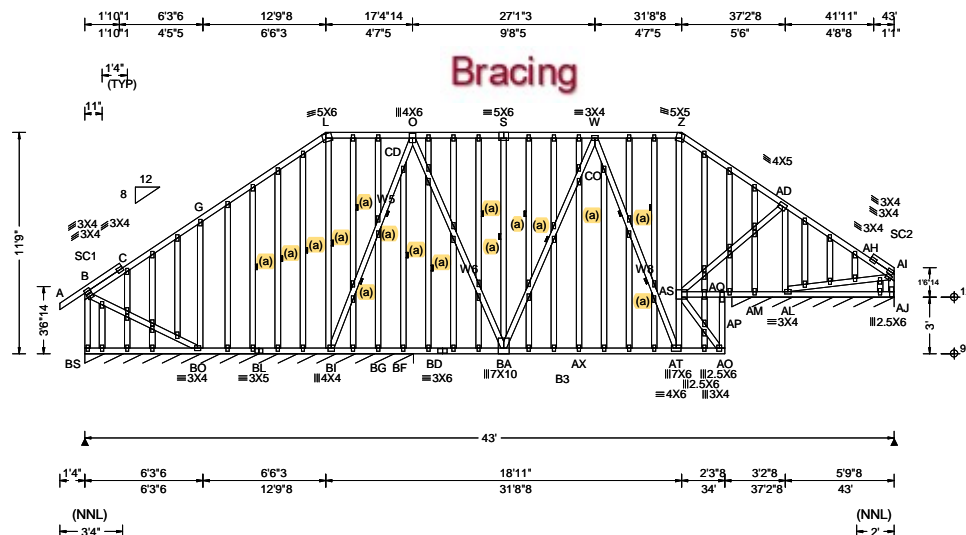
The overall height of this truss excluding overhang is 11-9-0.



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****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.064 U 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.178 U 999 180	BS* 283 -/- -/- -/- /23 -/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.010 AF - -	AJ* 358 -/- -/- -/- /34 -/-
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.022 AO - -	BG -/130
NCBCLL: 10.00	Mean Height: 19.54 ft	Building Code:	Creep Factor: 2.0	AM -/157
Soffit: 2.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.923	AJ -/208
Load Duration: 1.25	BCDL: 3.0 psf	TPI Std: 2014	Max BC CSI: 0.718	Wind reactions based on MWFRS
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.995	BS Brg Wid = 209 Min Req = -
	C&C Dist a: 4.30 ft	FT/RT: 20(0)/10(0)		AJ Brg Wid = 103 Min Req = -
	Loc. from endwall: Any	Plate Type(s):		Bearings BS & AN are a rigid surface.
	GCpi: 0.18			Members not listed have forces less than 375#
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.00.0914.12	Maximum Top Chord Forces Per Ply (lbs)

Lumber	Purlins	Chords	Tens.Comp.	Chords	Tens. Comp.
Top chord: 2x4 SP #2;	In lieu of structural panels use purlins to brace TC @ 24" oc.	C - G	376 -36	W - Z	69 -844
Bot chord: 2x4 SP #2; B3 2x4 SP M-31;		O - S	96 -1229	Z -AD	97 -1108
Webs: 2x4 SP #3; W5 2x4 SP M-31; W6,		S - W	96 -1230		
W8, W9, W10, W11, W12, W13, W14, W15, W16, W17, W18, W19, W20, W21, W22, W23, W24, W25, W26, W27, W28, W29, W30, W31, W32, W33, W34, W35, W36, W37, W38, W39, W40, W41, W42, W43, W44, W45, W46, W47, W48, W49, W50, W51, W52, W53, W54, W55, W56, W57, W58, W59, W60, W61, W62, W63, W64, W65, W66, W67, W68, W69, W70, W71, W72, W73, W74, W75, W76, W77, W78, W79, W80, W81, W82, W83, W84, W85, W86, W87, W88, W89, W90, W91, W92, W93, W94, W95, W96, W97, W98, W99, W100, W101, W102, W103, W104, W105, W106, W107, W108, W109, W110, W111, W112, W113, W114, W115, W116, W117, W118, W119, W120, W121, W122, W123, W124, W125, W126, W127, W128, W129, W130, W131, W132, W133, W134, W135, W136, W137, W138, W139, W140, W141, W142, W143, W144, W145, W146, W147, W148, W149, W150, W151, W152, W153, W154, W155, W156, W157, W158, W159, W160, W161, W162, W163, W164, W165, W166, W167, W168, W169, W170, W171, W172, W173, W174, W175, W176, W177, W178, W179, W180, W181, W182, W183, W184, W185, W186, W187, W188, W189, 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Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	57 plf at	-1.33 to	57 plf at	19.06
TC: From	28 plf at	19.06 to	28 plf at	32.20
TC: From	57 plf at	32.20 to	57 plf at	43.00
BC: From	5 plf at	-1.33 to	5 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	19.00
BC: From	10 plf at	19.00 to	10 plf at	34.00
BC: From	20 plf at	34.00 to	20 plf at	43.00
BC: 208 lb Conc. Load	at 19.06,21.06,23.06,25.06			

26.44,28.44,30.44,32.44

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

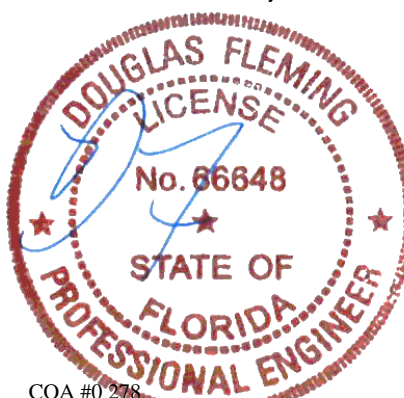
Wind

Wind loads and reactions based on MWERS

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



COA #0 278
Florida Certificate of Product Approval #FL1999
07/27/2023

Maximum Bot Chord Forces Per Ply (lbs)					
Chords Tens.Comp.			Chords Tens. Comp.		
BI-BF	491	-38	BA-AX	1299	-104
BF-BD	485	-38	AX-AT	1299	-104
BD-BA	485	-38	AT-AO	795	-66

Maximum Web Forces Per Ply (lbs)				
Webs	Tens.	Comp.	Webs	Tens. Comp.
BI- O	193	- 2379	AS-AO	108 - 1308
O -BA	1956	- 153	AS-AD	1229 - 93
W -AT	104	- 1366	AP-AO	992 - 81
AT-AS	1760	- 141	AD-AL	158 - 1501

Maximum Gable Forces Per Ply (lbs)					
Gables			Tens. Comp.		
Gables	Tens.	Comp.	Gables	Tens.	Comp.
CD-BF	31	-375	CO-AX	422	-44
S-BA	42	-644			

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

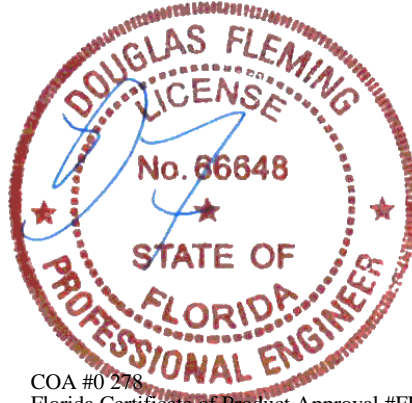
SEQN: 568743	GABL	Ply: 1	Job Number: 23-9526	Cust: R 215 JRef: 1XRQ2150001 T22
FROM: RFG		Qty: 1	Karlton	DrwNo: 208.23.0803.30237
Page 2 of 2			Truss Label: A1E	SSB / DF 07/27/2023

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

The overall height of this truss excluding overhang is 11-9-0.



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07/27/2023

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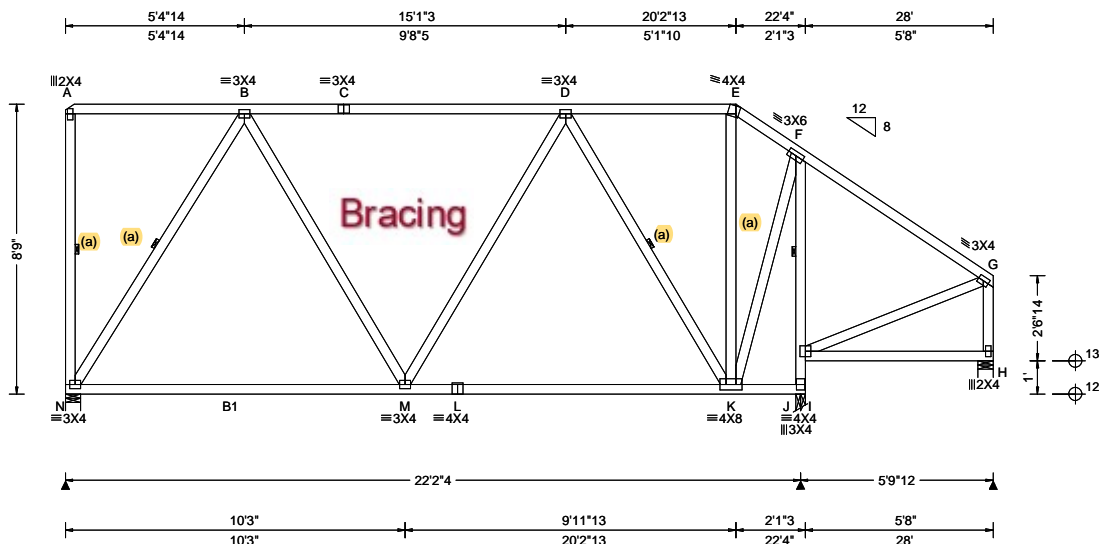
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155 Harlem Ave
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Glenview, IL 60025

SEQN: 568753 FROM: RFG	COMN Ply: 1 Qty: 6	Job Number: 23-9526 Karlton Truss Label: A2	Cust: R 215 JRef: 1XRQ2150001 T58 DrwNo: 208.23.0803.31643 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 18.16 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.030 M 999 240 VERT(CL): 0.050 M 999 180 HORZ(LL): 0.015 H - - HORZ(TL): 0.025 H - - Creep Factor: 2.0 Max TC CSI: 0.927 Max BC CSI: 0.620 Max Web CSI: 0.520 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL N 1068 - / - / - /417 - /99 J 1292 - / - /562 - / - H 203 - / - /130 /18 - Wind reactions based on MWFRS N Brg Wid = 5.5 Min Req = 1.5 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) H Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings N, J, & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

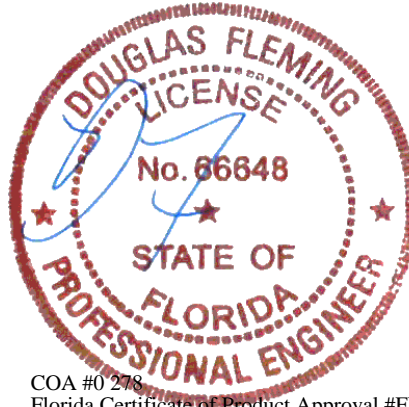
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8'-9".

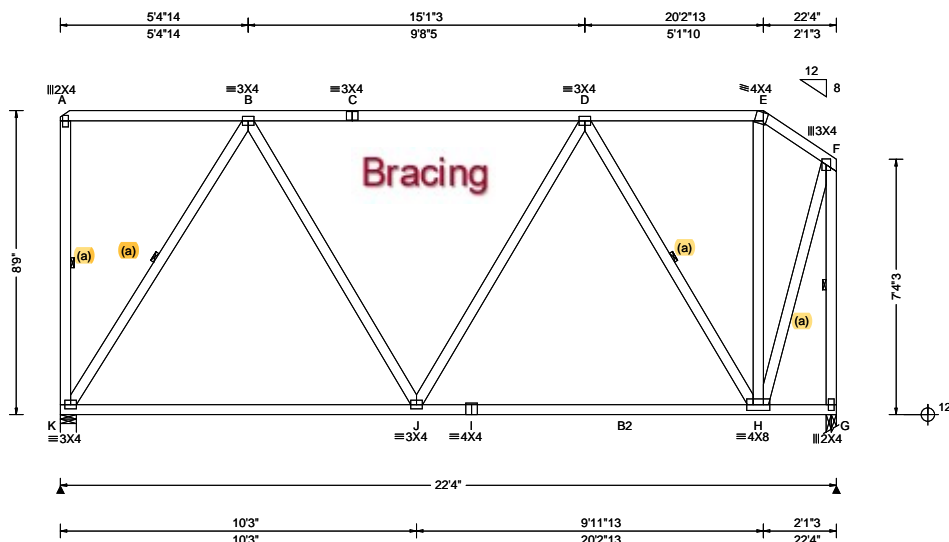


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07/27/2023

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568755 FROM: RFG	COMN Ply: 1 Qty: 5	Job Number: 23-9526 Karlton Truss Label: A3	Cust: R 215 JRef: 1XRQ2150001 T56 DrwNo: 208.23.0803.33047 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.38 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.031 J 999 240 VERT(CL): 0.050 J 999 180 HORZ(LL): 0.013 A - - HORZ(TL): 0.020 A - - Creep Factor: 2.0 Max TC CSI: 0.928 Max BC CSI: 0.621 Max Web CSI: 0.525 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL K 1075 -/- /- /412 -/- /27 G 1048 -/- /- /427 -/- /- Non-Gravity Wind reactions based on MWFRS K Brg Wid = 5.5 Min Req = 1.5 (Truss) G Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings K & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -748 C - D 0 -748

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31; B2 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

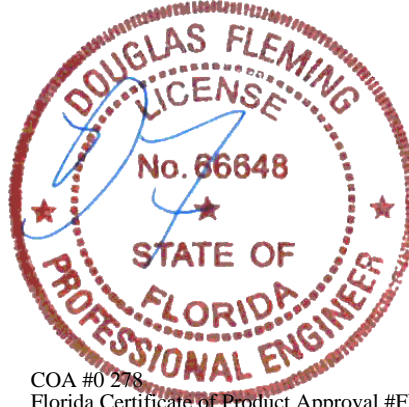
Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8-9-0.

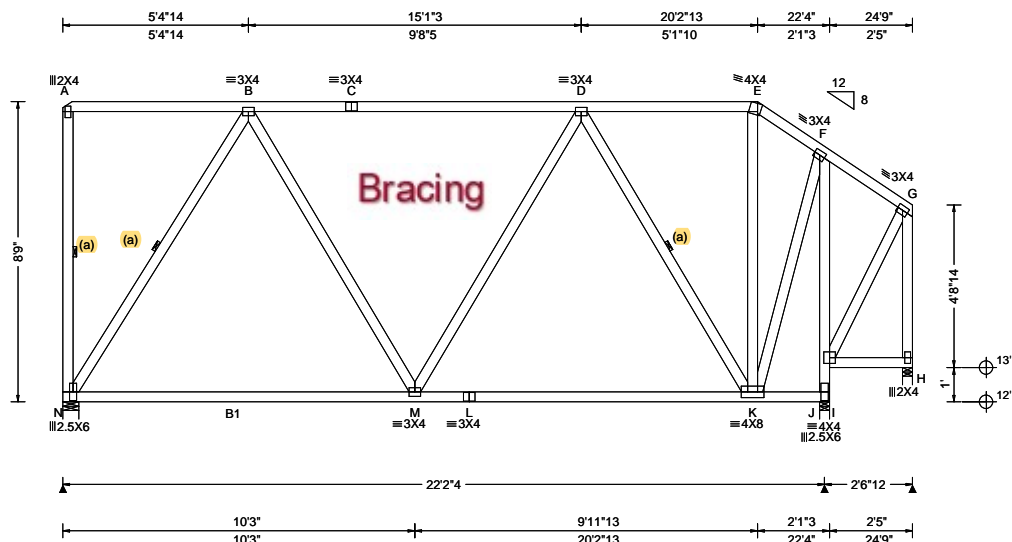


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07/27/2023

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568758 FROM: RFG	COMN Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: A4	Cust: R 215 JRRef: 1XRQ2150001 T12 DrwNo: 208.23.0803.34280 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.24 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.020 D 999 240 VERT(CL): 0.038 M 999 180 HORZ(LL): 0.010 H - - HORZ(TL): 0.020 H - - Creep Factor: 2.0 Max TC CSI: 0.930 Max BC CSI: 0.481 Max Web CSI: 0.595 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL N 854 - / - / - /394 /158 /56 J 923 - / - / - /447 /94 - /- H 123 - / - / - /72 /5 - /- Non-Gravity N Brg Wid = 5.5 Min Req = 1.5 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings N, J, & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

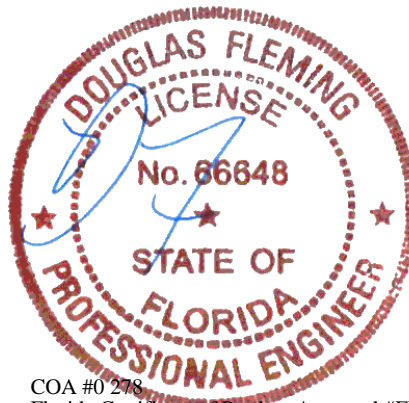
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8'-9".



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07/27/2023

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North Building, 4th Floor
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The diagram illustrates a roof truss system with the following dimensions and member labels:

- Overall Dimensions:**
 - Span: 36'9"
 - Height: 9'9"
 - Roof Slope: 12/8
- Top Chord Members:**
 - D-E: 5X5
 - E-F: 3X4
 - F-G: 3X5
 - G-H: 3X4
 - H-I: 5X5
- Bottom Chord Members:**
 - A-B: 5X5(B2)
 - P-Q: 2X4
 - O-R: 3X5
 - N-S: 3X8
 - M-T: 4X5
 - L-U: 3X4
 - K-V: 4X10
 - J-W: 2.5X6
- Vertical Members:**
 - B-P: 5X5
 - D-N: 5X5
 - E-M: 3X4
 - F-L: 3X5
 - G-K: 3X4
 - H-J: 5X5
- Diagonal Bracing Members:**
 - C-N: 5X5
 - E-M: 3X4
 - F-L: 3X5
 - G-K: 3X4
 - H-J: 5X5
- Labels and Notes:**
 - Truss sections are labeled T3 and T4.
 - Members are labeled with (a) for bracing.
 - Dimensions are given in feet and inches (e.g., 6'3"6, 12'3"3).

Lumber	C - D	515 - 2063	G - H	274 - 856
Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;	D - E	467 - 1649	H - I	286 - 1085
Bot chord: 2x4 SP M-31;	E - F	497 - 1788		
Webbs: 2x4 SP #3;				
Lt Wedge: 2x4 SP #3;				
				Maximum Bot Chord Forces Per Ply (lbs)

	Chords	Tens.Comp.	Chords	Tens.	Comp.
Bracing					
(a) Continuous lateral restraint equally spaced on member.	B - P	1879 - 472	N - M	1853	- 488
	P - O	1879 - 473	M - L	1853	- 488
	O - N	1879 - 473	L - K	1467	- 393

Loading	Maximum Web Forces Per Ply (lbs)					
	Webs	Tens.Comp.	Webs	Tens. Comp.		
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.	D. N.	617	173	G. K.	294	1004

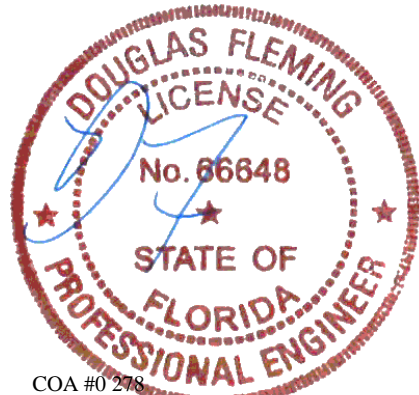
Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

D - N	817	-179	G - K	391	-1204
N - E	216	-402	K - I	1303	-298
L - G	645	-61	I - J	405	-1668


Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 8-9-0.



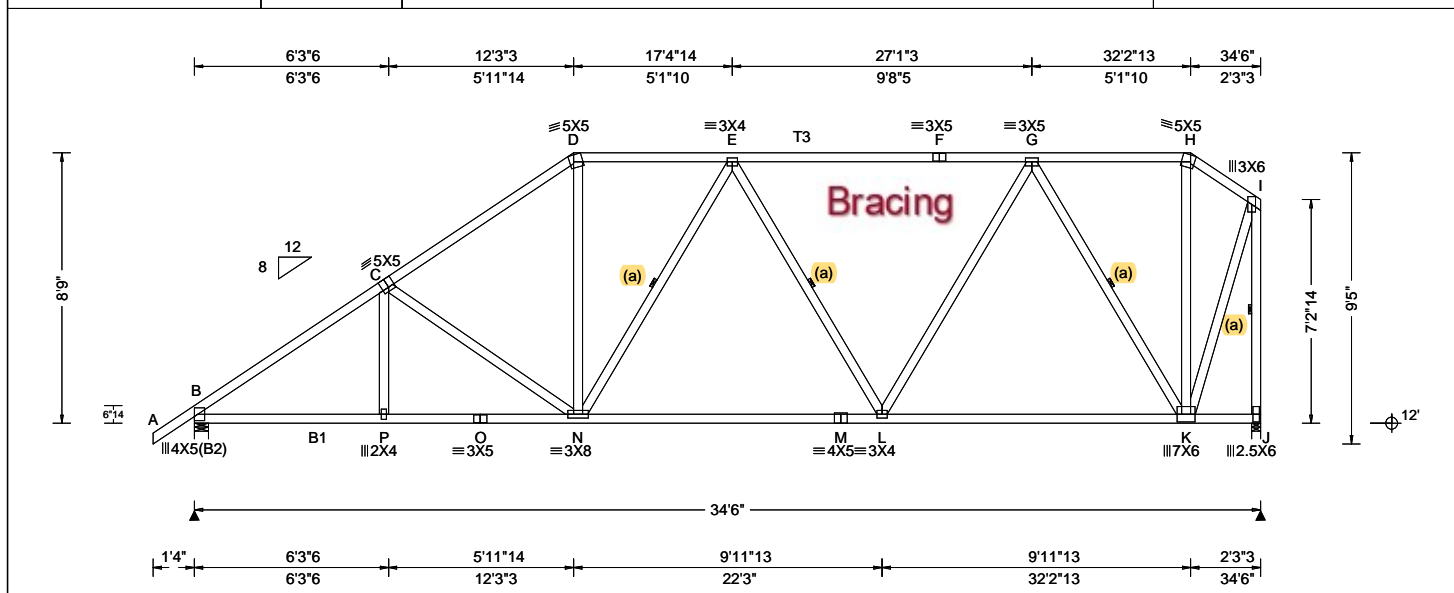
COA #0 278
Florida Certificate of Product Approval #FL1999
07/27/2023

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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 568781 FROM: RFG	COMN Ply: 1 Qty: 2	Job Number: 23-9526 Karlton Truss Label: A6	Cust: R 215 JRef: 1XRQ2150001 T14 DrwNo: 208.23.0803.38040 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.22 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.45 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.107 E 999 240 VERT(CL): 0.183 E 999 180 HORZ(LL): 0.044 K - - HORZ(TL): 0.074 K - - Creep Factor: 2.0 Max TC CSI: 0.875 Max BC CSI: 0.970 Max Web CSI: 0.672 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1559 - / - / - / 772 / 152 / 181 J 1590 - / - / - / 625 / 181 / - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.8 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 449 - 2204 F - G 426 - 1517 C - D 474 - 1884 G - H 151 - 461 D - E 432 - 1498 H - I 159 - 584 E - F 426 - 1517

Lumber

Top chord: 2x4 SP #2; T3 2x4 SP M-31;
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

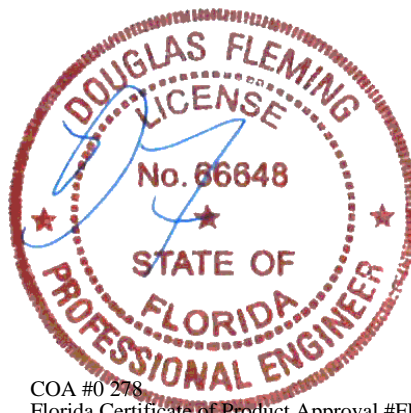
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8'-9-0.

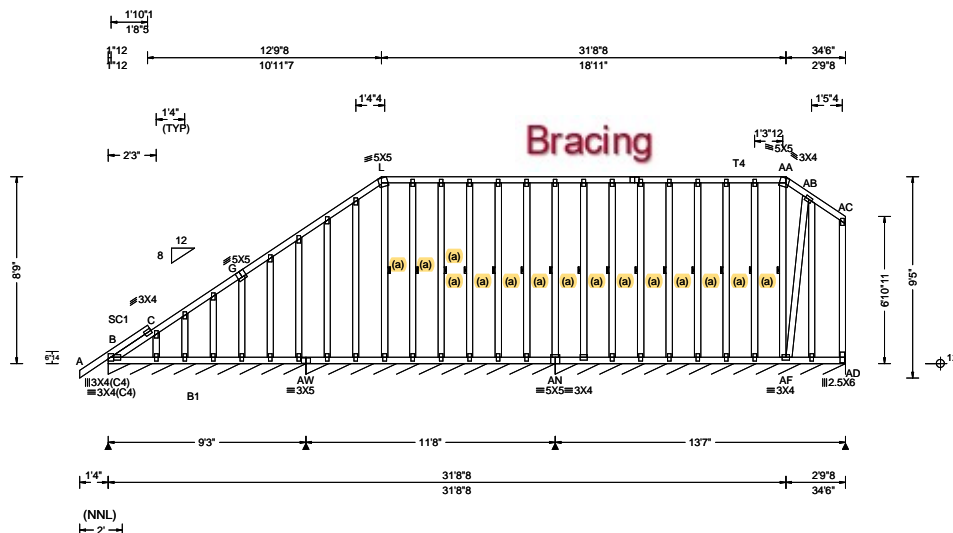


COA #0 278
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155 Harlem Ave
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Glenview, IL 60025

SEQN: 568778 FROM: RFG	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: A6E	Cust: R 215 JRef: 1XRQ2150001 T5 DrwNo: 208.23.0803.39700 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.22 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.45 ft Loc. from endwall: not in 10.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 M 999 240 VERT(CL): 0.003 M 999 180 HORZ(LL): -0.003 J - - HORZ(TL): 0.004 J - - Creep Factor: 2.0 Max TC CSI: 0.190 Max BC CSI: 0.071 Max Web CSI: 0.113 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B* 118 - / - /56 /12 /22 AW*143 - / - /44 /13 - AN*131 - / - /38 /11 - Wind reactions based on MWFRS B Brg Wid = 111 Min Req = - AW Brg Wid = 140 Min Req = - AN Brg Wid = 163 Min Req = - Bearings B, AW, & AN are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

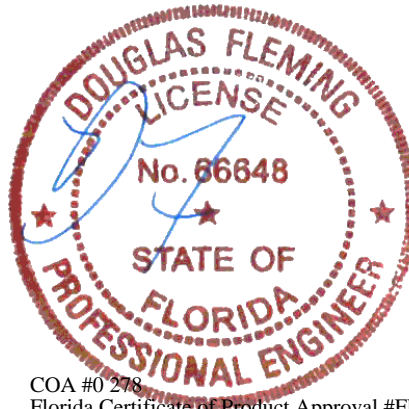
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 8-9-0.

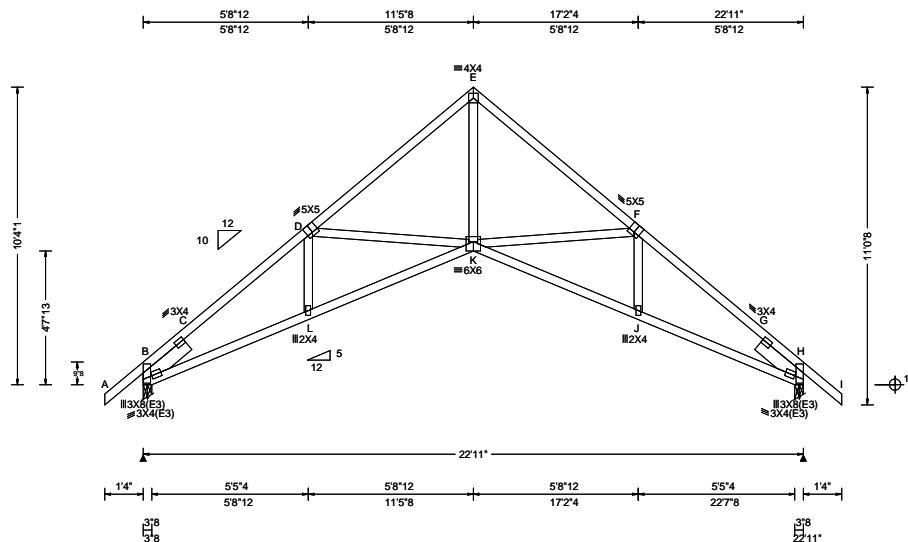


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07/27/2023

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568767 FROM: RFG	COMN Ply: 1 Qty: 3	Job Number: 23-9526 Karlton Truss Label: B1	Cust: R 215 JRRef: 1XRQ2150001 T15 DrwNo: 208.23.0803.40970 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 17.01 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.103 K 999 240 VERT(CL): 0.202 K 999 180 HORZ(LL): 0.117 H - - HORZ(TL): 0.229 H - - Creep Factor: 2.0 Max TC CSI: 0.344 Max BC CSI: 0.453 Max Web CSI: 0.507 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1000 - / - / 544 / 112 / 241 H 1000 - / - / 544 / 112 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 295 - 1938 E - F 186 - 1414 C - D 295 - 1876 F - G 294 - 1876 D - E 196 - 1414 G - H 303 - 1938

Lumber

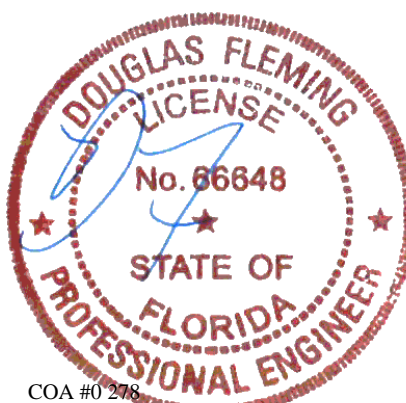
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.956'
Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.956'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 10-4-1.

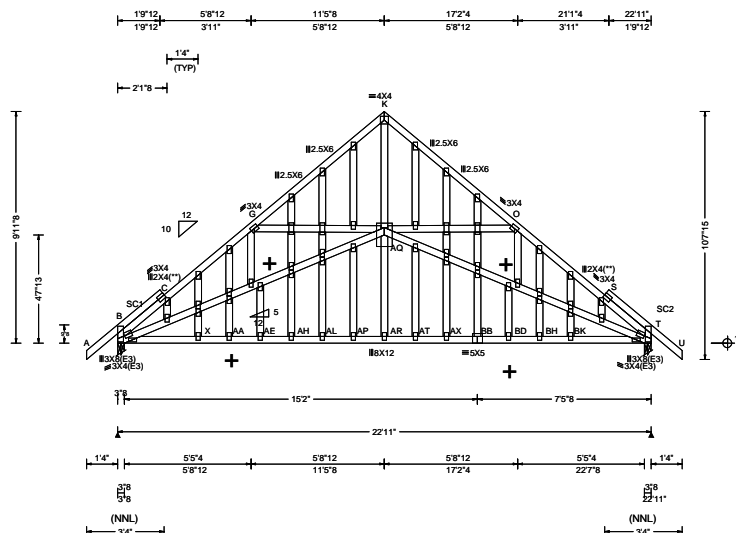


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07/27/2023

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568771 FROM: RFG	GABL Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: B1E	Cust: R 215 JRRef: 1XRQ2150001 T55 DrwNo: 208.23.0804.11247 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.82 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.049 AJ 999 240 VERT(CL): 0.124 AJ 999 180 HORZ(LL): 0.027 I - - HORZ(TL): 0.068 I - - Creep Factor: 2.0 Max TC CSI: 0.324 Max BC CSI: 0.254 Max Web CSI: 0.553 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1472 -/- /- /559 /132 /265 T 1470 -/- /- /559 /132 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.6 (Truss) T Brg Wid = 3.5 Min Req = 1.6 (Truss) Bearings B & T are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 240 - 1713 K - O 140 - 655 C - G 326 - 1636 O - S 325 - 1629 G - K 140 - 655 S - T 240 - 1707

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Filler 2x4 SP #2;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0' span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

Laterally brace BC at 24" oc in lieu of rigid ceiling.
Laterally brace BC above filler at 24" oc.

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 9-11-8.

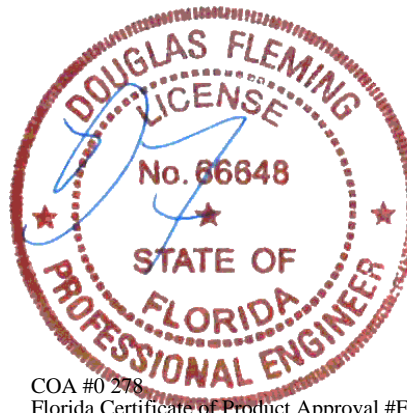
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - X	1451 -95	AQ - O	326 - 825
X - AA	1450 -95	AR - AT	1453 -96
AA - AE	1450 -95	AT - AX	1454 -96
G - AQ	326 -830	AX - BB	1454 -96
AE - AH	1454 -96	BB - BD	1454 -96
AH - AL	1454 -96	BD - BH	1450 -95
AL - AP	1454 -96	BH - BK	1450 -95
AP - AR	1453 -96	BK - T	1451 -96

Note: The Project Engineer shall provide for endwall stability per the Florida Building Code. The top of the wall below this truss shall be laterally braced as specified by the Project Engineer. This truss will not provide lateral support of the end wall.

Bottom chord to be laterally braced for out of plane wind loads

+ Member to be laterally braced for out of plane wind loads

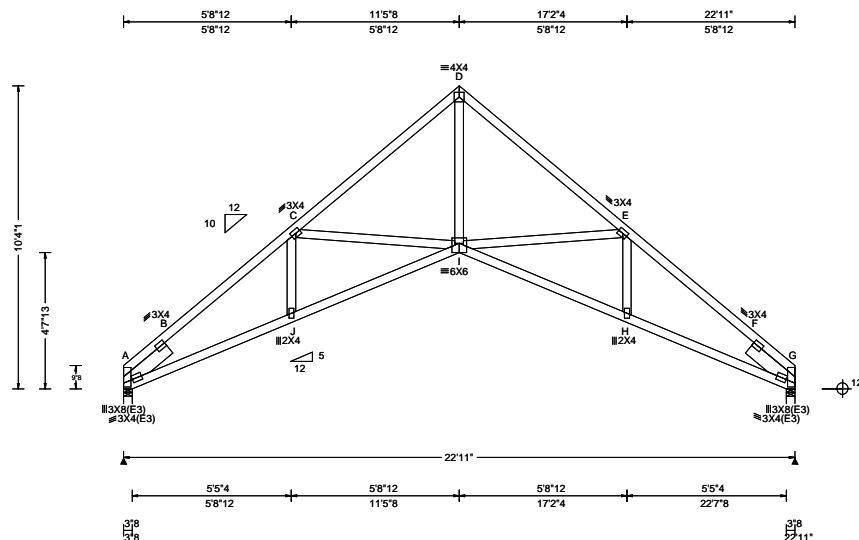


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07/27/2023

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568769 FROM: RFG	COMN Ply: 1 Qty: 7	Job Number: 23-9526 Karlton Truss Label: B2	Cust: R 215 JRRef: 1XRQ2150001 T2 DrwNo: 208.23.0804.12710 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 17.57 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.100 I 999 240 VERT(CL): 0.199 I 999 180 HORZ(LL): 0.113 G - - HORZ(TL): 0.226 G - - Creep Factor: 2.0 Max TC CSI: 0.323 Max BC CSI: 0.459 Max Web CSI: 0.515 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL A 915 - / - / - / 484 / 78 / 199 G 915 - / - / - / 484 / 78 / - Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) G Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & G Fcperp = 565psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 202 - 1985 D - E 130 - 1428 B - C 193 - 1903 E - F 193 - 1903 C - D 130 - 1428 F - G 202 - 1985

Lumber

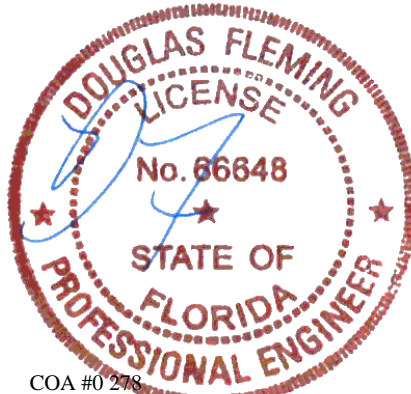
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.956'
Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.956'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 10-4-1.

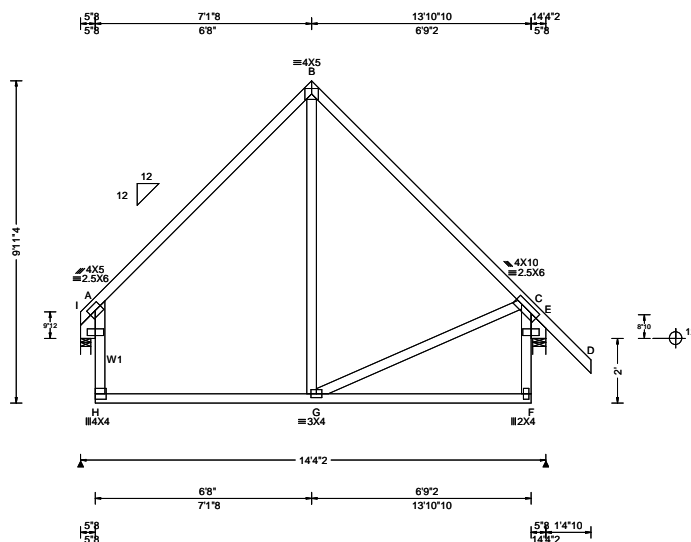


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07/27/2023

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568785 / FROM: RFG	SPEC Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: C1	Cust: R 215 JRef: 1XRQ2150001 T6 DrwNo: 208.23.0620.32483 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.64 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.094 B 999 240 VERT(CL): 0.167 G 995 180 HORZ(LL): 0.181 F - - HORZ(TL): 0.352 F - - Creep Factor: 2.0 Max TC CSI: 0.771 Max BC CSI: 0.350 Max Web CSI: 0.788 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I 553 - / - / 293 / 47 / 190 E 664 - / - / 358 / 61 / - Wind reactions based on MWFRS I Brg Wid = 3.8 Min Req = 1.5 (Support) E Brg Wid = 4.9 Min Req = 1.5 (Support) Bearings I & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 221 -464 B - C 245 -449

Lumber

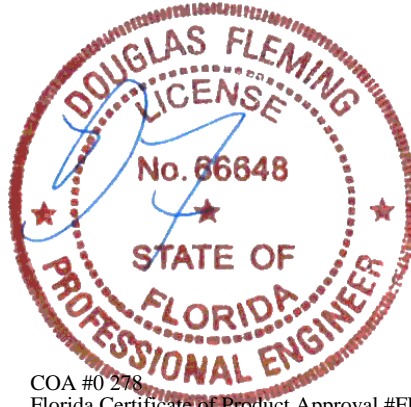
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1 2x4 SP #2;
Lt Bearing Leg: 2x6 SP 2400f-2.0E;
Rt Bearing Leg: 2x6 SP 2400f-2.0E;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-11-4.

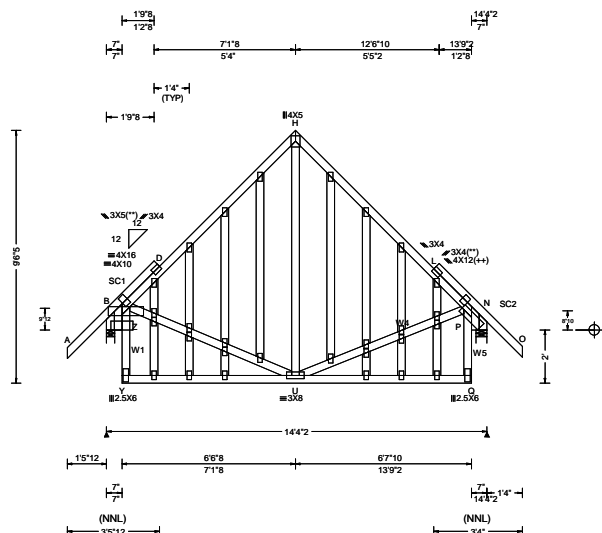


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07/27/2023

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568793 / FROM: RFG	GABL Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: C1E	Cust: R 215 JRef: 1XRQ2150001 T4 DrwNo: 208.23.0620.32593 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.43 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.080 S 999 240 VERT(CL): 0.207 S 745 180 HORZ(LL): -0.053 K - - HORZ(TL): 0.136 K - - Creep Factor: 2.0 Max TC CSI: 0.534 Max BC CSI: 0.649 Max Web CSI: 0.963 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1012 -/- /- /382 /81 /228 P 994 -/- /- /374 /76 -/- Wind reactions based on MWFRS B Brg Wid = 3.8 Min Req = 1.5 (Support) P Brg Wid = 4.9 Min Req = 1.5 (Support) Bearings B & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - D 146 -630 H - L 312 -593 D - H 315 -590 L - N 75 -639

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1,W5 2x4 SP M-31;
W4 2x4 SP #2;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;
Lt Bearing Leg: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Plating Notes
All plates are 2X4 except as noted.
(++) - This plate works for both joints covered.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

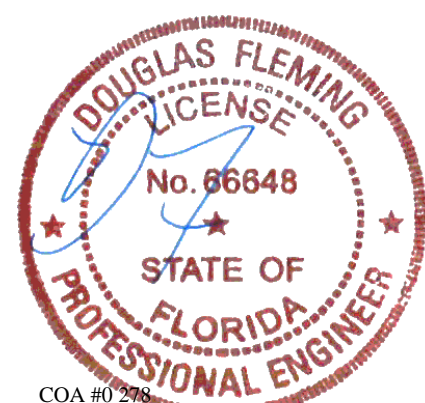
Purlins
In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
The overall height of this truss excluding overhang is 9-6-5.

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - Z	127 -441	U - N	670 -38
B - U	670 0		

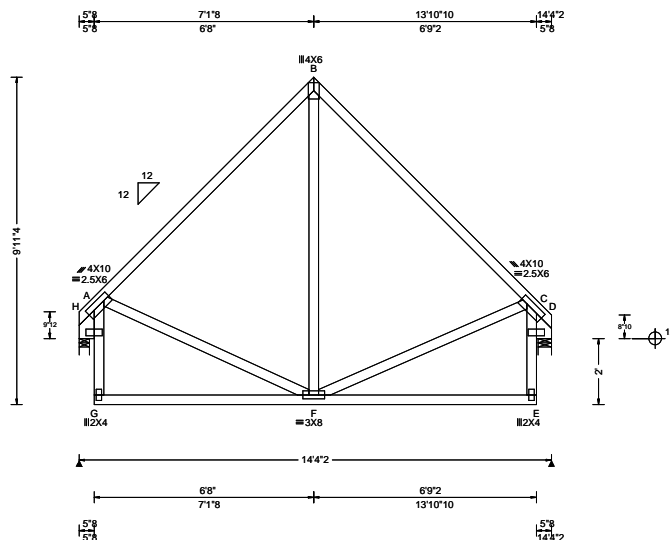


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07/27/2023

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568787 / FROM: RFG	SPEC Ply: 1 Qty: 2	Job Number: 23-9526 Karlton Truss Label: C2	Cust: R 215 JRef: 1XRQ2150001 T3 DrwNo: 208.23.0620.32296 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.33 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.007 F 999 240 VERT(CL): 0.015 F 999 180 HORZ(LL): 0.025 C - - HORZ(TL): 0.049 C - - Creep Factor: 2.0 Max TC CSI: 0.478 Max BC CSI: 0.409 Max Web CSI: 0.777 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 559 -/- /- /298 /53 /155 D 567 -/- /- /301 /54 -/ Wind reactions based on MWFRS H Brg Wid = 3.8 Min Req = 1.5 (Support) D Brg Wid = 4.9 Min Req = 1.5 (Support) Bearings H & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 158 -462 B - C 147 -465

Lumber

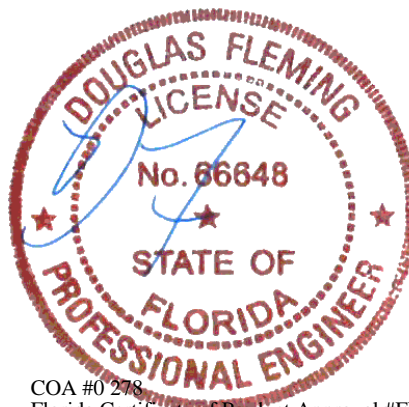
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Bearing Leg: 2x6 SP 2400f-2.0E;
Rt Bearing Leg: 2x6 SP 2400f-2.0E;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-11-4.

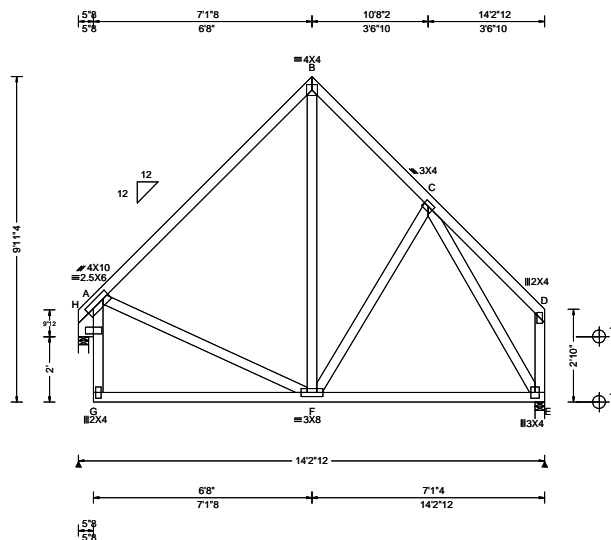


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07/27/2023

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Glenview, IL 60025

SEQN: 568789 / FROM: RFG	SPEC Ply: 1 Qty: 3	Job Number: 23-9526 Karlton Truss Label: C3	Cust: R 215 JRef: 1XRQ2150001 T21 DrwNo: 208.23.0620.32030 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.38 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 F 999 240 VERT(CL): 0.017 F 999 180 HORZ(LL): 0.024 D - - HORZ(TL): 0.044 D - - Creep Factor: 2.0 Max TC CSI: 0.464 Max BC CSI: 0.467 Max Web CSI: 0.870 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 590 -/- /- /301 /46 /154 E 626 -/- /- /302 /42 -/ Wind reactions based on MWFRS H Brg Wid = 3.8 Min Req = 1.5 (Support) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings H & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 159 -502 B - C 176 -415

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Bearing Leg: 2x6 SP 2400f-2.0E;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

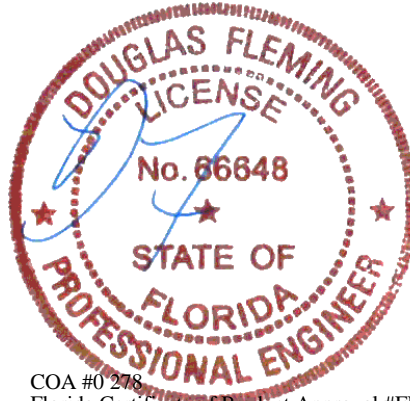
Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9'-11-4".

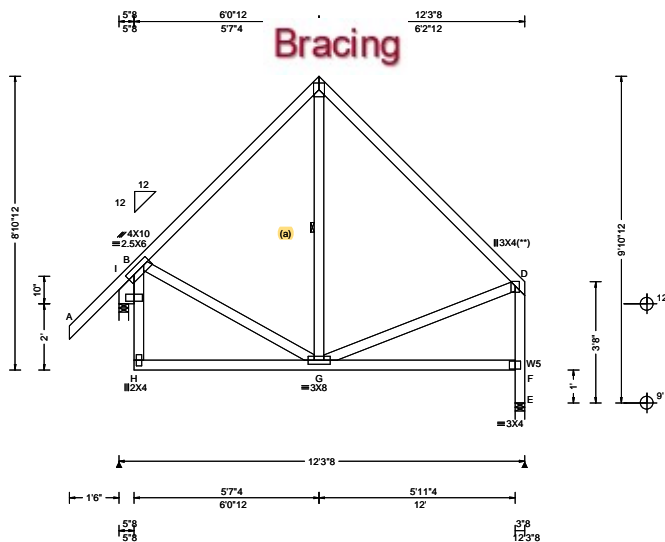


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155 Harlem Ave
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Glenview, IL 60025

SEQN: 4879 FROM: RFG	SPEC Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: D1	Cust: R 215 JRef: 1XRQ2150001 T7 DrwNo: 208.23.0804.48483 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.12 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.008 G 999 180 HORZ(LL): 0.015 F - - HORZ(TL): 0.021 F - - Creep Factor: 2.0 Max TC CSI: 0.450 Max BC CSI: 0.312 Max Web CSI: 0.514 VIEW Ver: 23.01.01B.0621.10	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I 589 - / - / 324 / 48 / 196 E 475 - / - / 255 / 39 / - Wind reactions based on MWFRS I Brg Wid = 3.5 Min Req = 1.5 (Support) E Brg Wid = 3.5 Min Req = 1.5 (Support) Bearings I & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 210 -379 C - D 169 -390

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W5 2x4 SP 2400f-2.0E;
Lt Bearing Leg: 2x6 SP #3;

Bracing

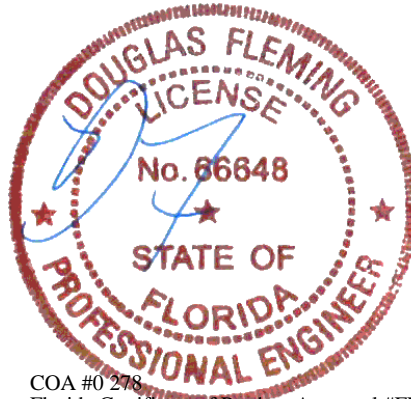
(a) Continuous lateral restraint equally spaced on member.

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Drop leg is not designed to resist any lateral loading from wind pressure on the wall. End vertical does not provide support for wall.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.055 G 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.139 G 988 180	B 903 /- /- /356 /111 /238
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.057 G - -	L 806 /- /- /327 /94 /-
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.144 G - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 3.5 Min Req = 1.5 (Support)
Soffit: 2.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.551	L Brg Wid = 3.5 Min Req = 1.5 (Support)
Load Duration: 1.25	BCDL: 3.0 psf	TPI Std: 2014	Max BC CSI: 0.483	Bearings B & L are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.752	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: Any	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE	VIEW Ver: 23.01.01B.0621.10	B - D 140 - 545 F - I 277 - 507
	Wind Duration: 1.60			D - F 255 - 507

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[illegible]

Lumber	Maximum Web Forces Per Ply (lbs)			
	Webs	Tens.Comp.	Webs	Tens. Comp.
Top chord: 2x4 SP #2;	H - A	246 - 617	E - C	115 - 435
Bot chord: 2x4 SP #2;	E - D	102 - 482		
Webs: 2x4 SP #3; W5 2x4 SP 2400f-2.0E;				
Lt Bearing Leg: 2x6 SP #3;				

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

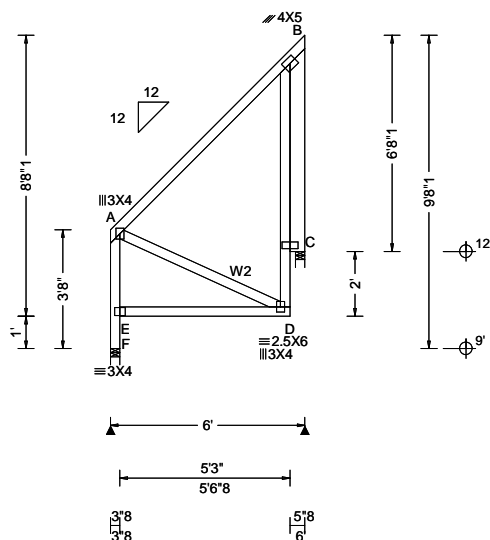
Drop leg is not designed to resist any lateral loading from wind pressure on the wall. End vertical does not provide support for wall.



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SEQN: 4882 FROM: RFG	SPEC Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: D3	Cust: R 215 JRef: 1XRQ2150001 T27 DrwNo: 208.23.0848.19630 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.67 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 A 681 240 VERT(CL): 0.005 D 999 180 HORZ(LL): -0.052 B - - HORZ(TL): 0.064 B - - Creep Factor: 2.0 Max TC CSI: 0.394 Max BC CSI: 0.303 Max Web CSI: 0.747 VIEW Ver: 23.01.01B.0621.10	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 232 - / - /147 /4 /157 C 232 - / - /245 /186 - Wind reactions based on MWFRS F Brg Wid = 3.5 Min Req = 1.5 (Support) C Brg Wid = 3.5 Min Req = 1.5 (Support) Bearings F & C are a rigid surface. Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. E - D 137 -467

Lumber

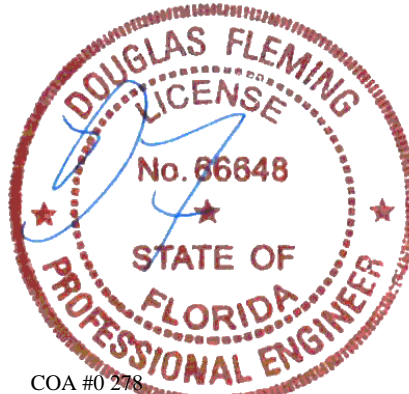
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2; W2 2x4 SP #3;
Rt Bearing Leg: 2x6 SP 2400f-2.0E;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Drop leg is not designed to resist any lateral loading from wind pressure on the wall. End vertical does not provide support for wall.

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - D	458 -124	B - C	1019 -748

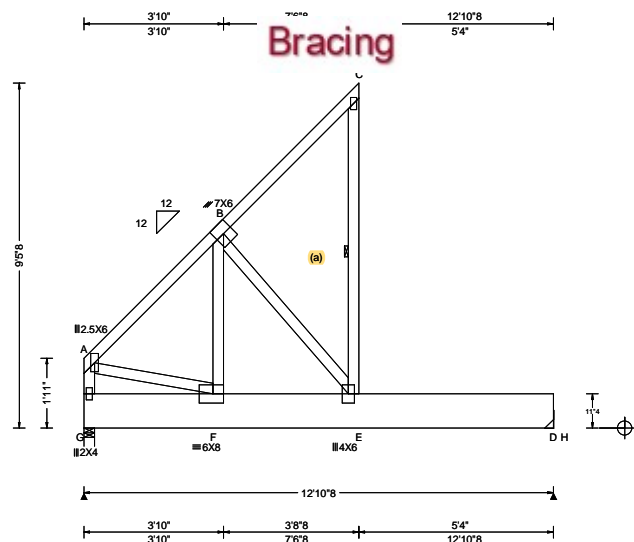


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568815 FROM: RFG	MONO Qty: 2	Ply: 1	Job Number: 23-9526 Karlton Truss Label: FT1	Cust: R 215 JRef: 1XRQ2150001 T19 DrwNo: 208.23.0846.28847 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.178 C 868 480 VERT(CL): 0.302 C 511 360 HORZ(LL): 0.200 C - - HORZ(TL): 0.340 C - - Creep Factor: 2.0 Max TC CSI: 0.629 Max BC CSI: 0.604 Max Web CSI: 0.597 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 1076 -/- /- /246 -/- /130 H 1206 -/- /- /195 /25 -/- Wind reactions based on MWFRS G Brg Wid = 3.5 Min Req = 1.5 (Truss) H Brg Wid = - Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. A - B 0 -995

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x12 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 108 plf at 0.00 to 108 plf at 7.54
BC: From 10 plf at 0.00 to 10 plf at 7.54
BC: From 110 plf at 7.54 to 110 plf at 12.88
BC: 803 lb Conc. Load at 7.69

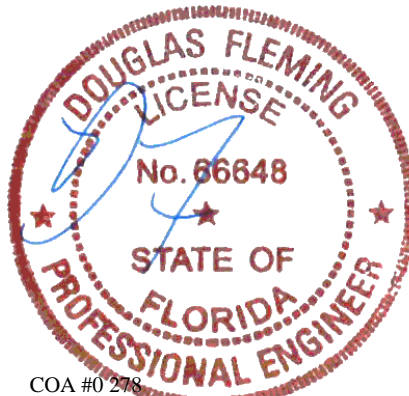
Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9'-5.8".

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

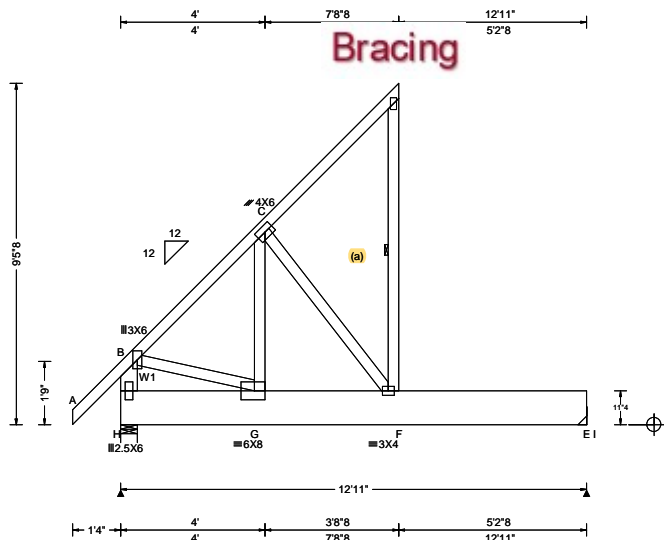


COA #0 278
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07/27/2023

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568813 FROM: RFG	COMN Ply: 1 Qty: 3	Job Number: 23-9526 Karlton Truss Label: FT2	Cust: R 215 JRef: 1XRQ2150001 T54 DrwNo: 208.23.0846.43713 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.175 D 883 480 VERT(CL): 0.294 D 527 360 HORZ(LL): 0.193 D - - HORZ(TL): 0.324 D - - Creep Factor: 2.0 Max TC CSI: 0.576 Max BC CSI: 0.594 Max Web CSI: 0.668 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL H 1230 -/- /- /323 -/- /150 I 1210 -/- /- /197 /26 -/- Non-Gravity Wind reactions based on MWFRS H Brg Wid = 5.5 Min Req = 1.5 (Truss) I Brg Wid = - Min Req = - Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 0 - 1026

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x12 SP 2400f-2.0E;
Webs: 2x4 SP #3; W1 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 108 plf at -1.33 to 108 plf at 7.71
BC: From 6 plf at -1.33 to 6 plf at 0.00
BC: From 10 plf at 0.00 to 10 plf at 7.71
BC: From 110 plf at 7.71 to 110 plf at 12.92
BC: 803 lb Conc. Load at 7.85

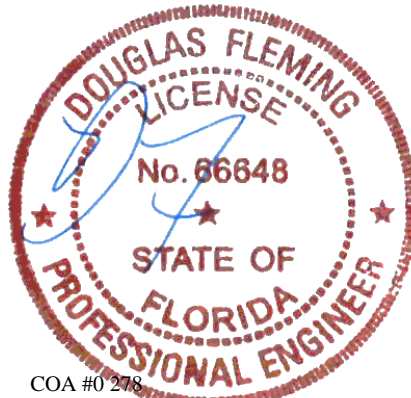
Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9'-5-8.

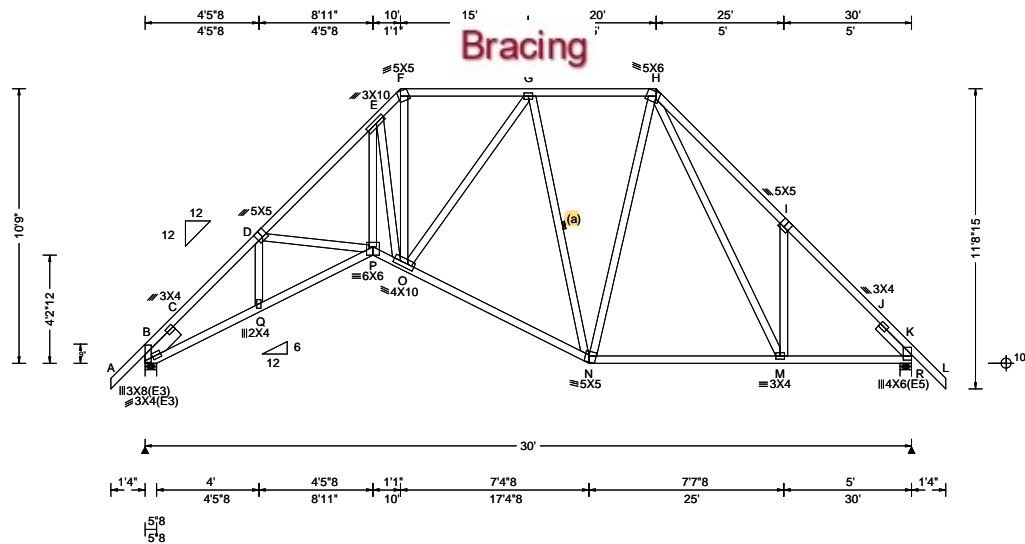
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.135 P 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.269 P 999 180	B 1313 /- /- /708 /112 /266
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.121 K - -	R 1296 /- /- /701 /114 /-
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.241 K - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.08 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 5.5 Min Req = 1.5 (Truss)
Soffit: 2.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.596	R Brg Wid = 5.5 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 3.0 psf	TPI Std: 2014	Max BC CSI: 0.596	Bearings B & R are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.989	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: Any	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18			B - C 495 -2512 G - H 381 -850
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.00.0914.12	

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.729'
Rt Slider: 2x4 SP #3; block length = 1.749'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

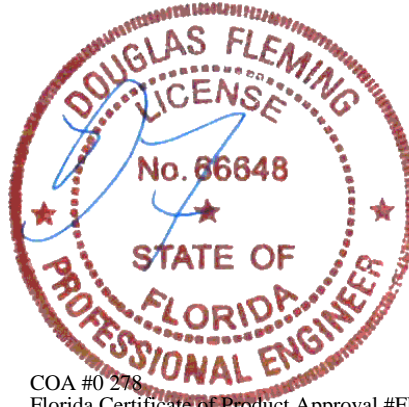
In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 10-9-0.



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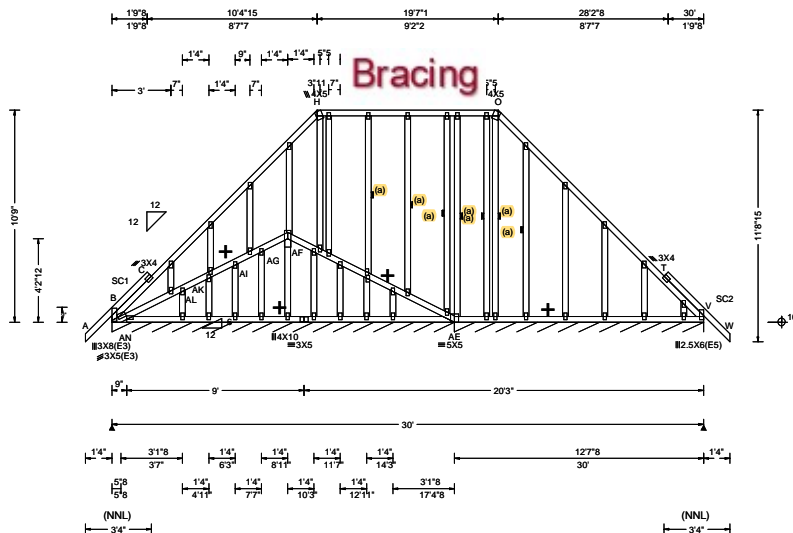
Maximum Bot Chord Forces Per Ply (lbs)					
Chords	Tens.Comp.		Chords	Tens. Comp.	
B - Q	1798	- 254	O - N	1146	- 105
Q - P	1827	- 259	N - M	779	- 37
P - O	1621	- 151	M - K	961	- 135

Maximum Web Forces Per Ply (lbs)					
Webs	Tens.Comp.		Webs	Tens. Comp.	
P - E	1507	- 169	G - N	149	- 664
E - O	257	- 1416	N - H	375	- 57
F - O	1025	- 265	H - M	412	- 221
O - G	421	- 114			

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SEQN: 568866 FROM: RFG	GABL Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: G1E	Cust: R 215 JRef: 1XRQ2150001 T46 DrwNo: 208.23.0845.07380 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.08 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.007 K 999 240 VERT(CL): 0.019 K 999 180 HORZ(LL): -0.006 C - - HORZ(TL): 0.009 C - - Creep Factor: 2.0 Max TC CSI: 0.195 Max BC CSI: 0.214 Max Web CSI: 0.117 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B* 155 - / - /54 /11 /17 AE* 125 - / - /55 /7 - Wind reactions based on MWFRS B Brg Wid = 208 Min Req = - AE Brg Wid = 151 Min Req = - Bearings B & AE are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Filler 2x4 SP #2;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0' span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

Laterally brace BC at 24" oc in lieu of rigid ceiling.
Laterally brace BC above filler at 24" oc.

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

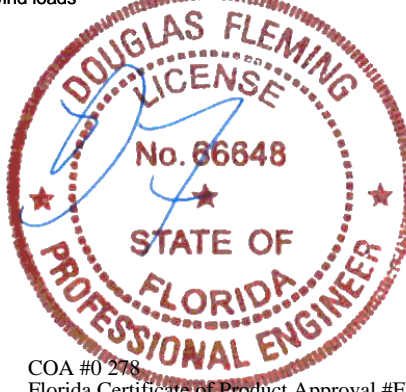
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 10-9.0.

+ Member to be laterally braced for out of plane wind loads

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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07/27/2023

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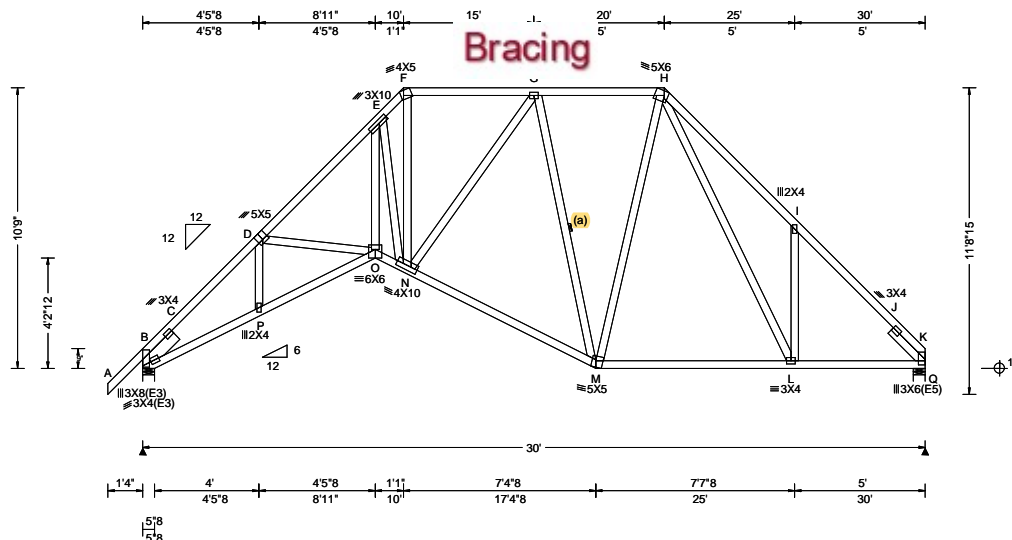
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568851 / FROM: RFG	COMN Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: G2	Cust: R 215 JRRef: 1XRQ2150001 T8 DrwNo: 208.23.0620.32187 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.08 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.134 O 999 240 VERT(CL): 0.269 O 999 180 HORZ(LL): 0.120 K - - HORZ(TL): 0.242 K - - Creep Factor: 2.0 Max TC CSI: 0.597 Max BC CSI: 0.597 Max Web CSI: 0.991 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1315 - / - / /708 /113 /248 Q 1207 - / - / /638 /104 - / - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) Q Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings B & Q are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 471 -2516 G - H 332 -854 C - D 458 -2457 H - I 486 -1471 D - E 472 -2218 I - J 313 -1451 E - F 484 -1743 J - K 305 -1503 F - G 357 -1235

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.729'
Rt Slider: 2x4 SP #3; block length = 1.749'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

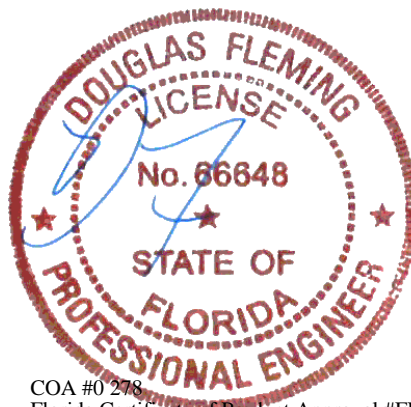
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 10-9-0.



COA #0278
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07/27/2023

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	1801 -298	N - M	1150 -163
P - O	1830 -304	M - L	782 -77
O - N	1625 -206	L - K	973 -152

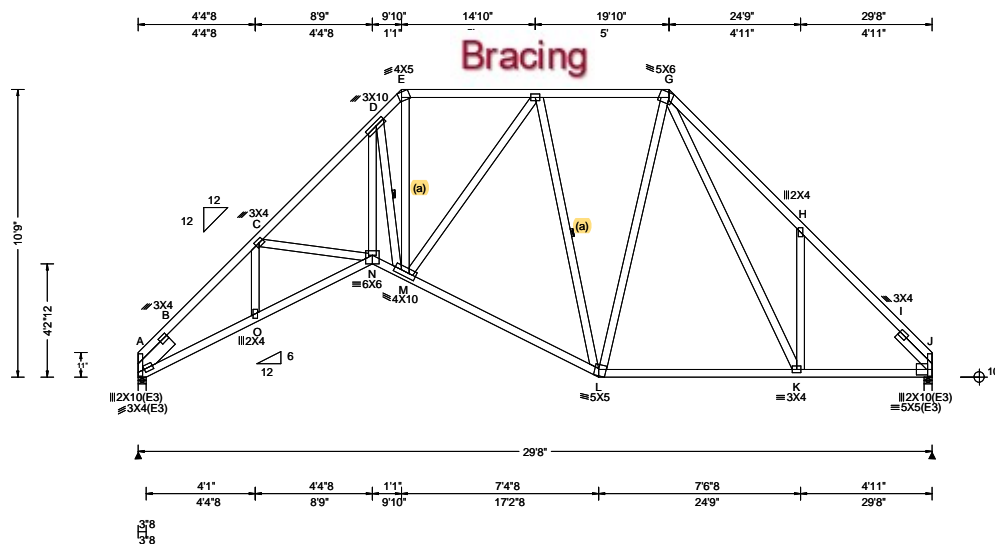
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
O - E	1510 -224	N - G	421 -89
E - N	294 -1418	G - M	188 -664
F - N	1028 -245	H - L	432 -172

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568854 / FROM: RFG	COMN Ply: 1 Qty: 6	Job Number: 23-9526 Karlton Truss Label: G3	Cust: R 215 JRRef: 1XRQ2150001 T43 DrwNo: 208.23.0620.31968 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 18.33 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.156 N 999 240 VERT(CL): 0.295 N 999 180 HORZ(LL): 0.141 J - - HORZ(TL): 0.267 J - - Creep Factor: 2.0 Max TC CSI: 0.761 Max BC CSI: 0.685 Max Web CSI: 0.599 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL A 1267 - / - / - / 666 - / 218 J 1319 - / - / - / 656 - / - Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) J Brg Wid = 3.5 Min Req = 1.6 (Truss) Bearings A & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 1 - 2564 F - G 82 - 934 B - C 0 - 2497 G - H 217 - 1578 C - D 0 - 2300 H - I 12 - 1595 D - E 18 - 1818 I - J 43 - 1652 E - F 0 - 1287

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.724'
Rt Slider: 2x4 SP #3; block length = 1.837'

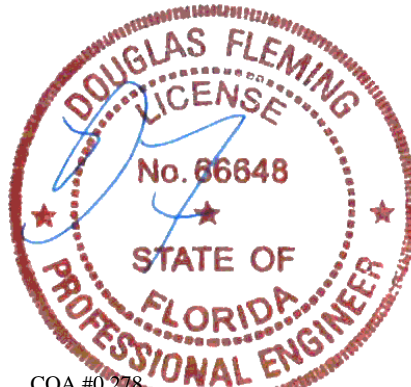
Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 10-9-0.

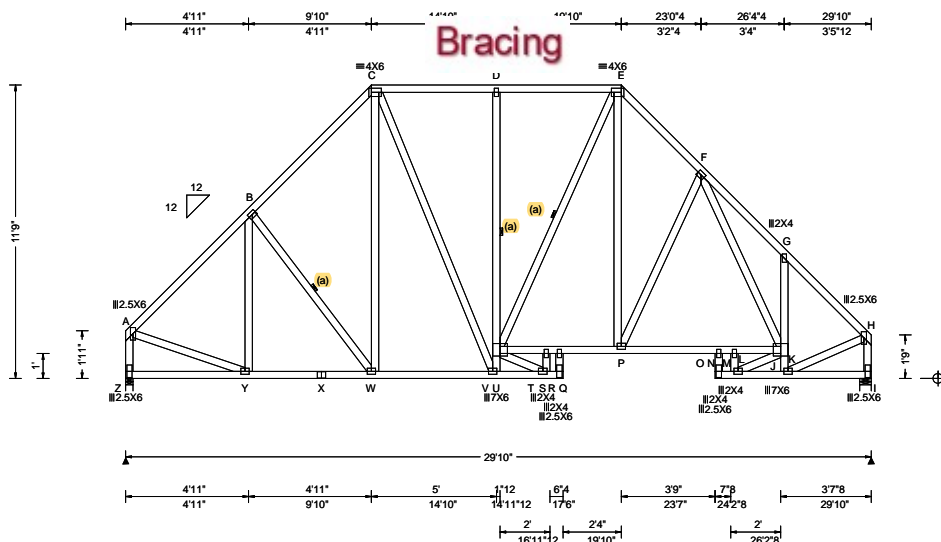


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568856 / FROM: RFG	COMN Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: G4	Cust: R 215 JRRef: 1XRQ2150001 T45 DrwNo: 208.23.0620.32452 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.75 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.146 O 999 240 VERT(CL): 0.275 O 999 180 HORZ(LL): 0.080 I - - HORZ(TL): 0.160 I - - Creep Factor: 2.0 Max TC CSI: 0.299 Max BC CSI: 0.792 Max Web CSI: 0.780 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Z 1190 -/- /- /615 -/- /215 I 1190 -/- /- /617 -/- /- Wind reactions based on MWFRS Z Brg Wid = 3.5 Min Req = 1.5 (Truss) I Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings Z & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 278 -1188 E - F 413 -1191 B - C 386 -1099 F - G 393 -1242 C - D 354 -796 G - H 257 -1174 D - E 357 -804

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

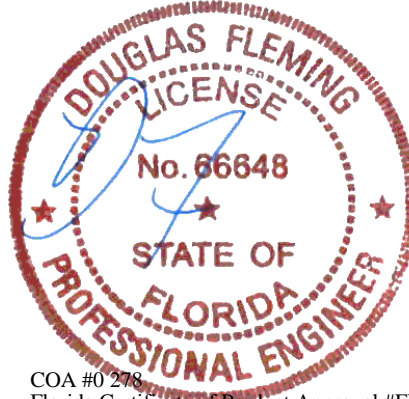
End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 11-9-0.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

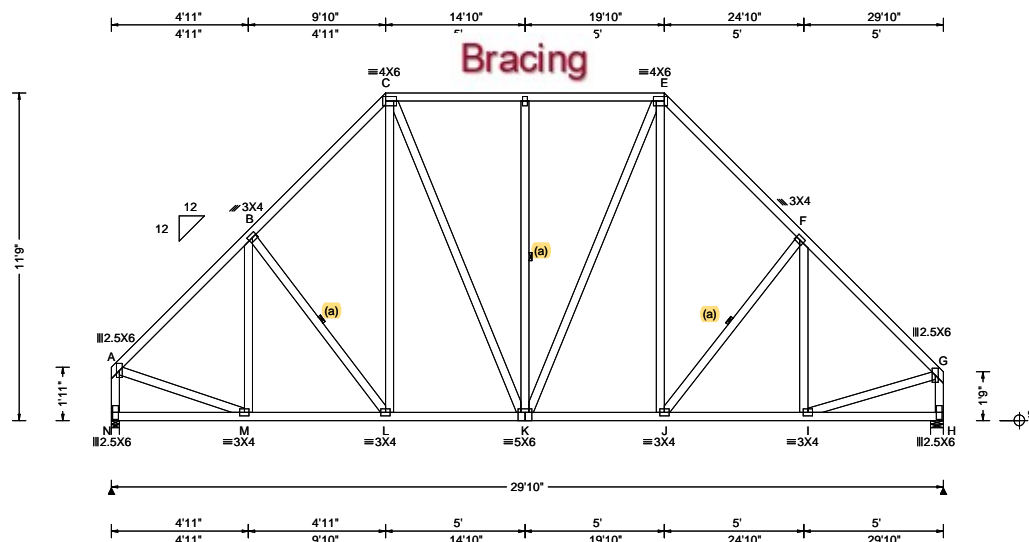


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568858 / FROM: RFG	COMN Ply: 1 Qty: 1	Job Number: 23-9526 Karlton Truss Label: G5	Cust: R 215 JRRef: 1XRQ2150001 T47 DrwNo: 208.23.0620.31781 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.75 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.033 D 999 240 VERT(CL): 0.066 D 999 180 HORZ(LL): 0.013 B - - HORZ(TL): 0.027 B - - Creep Factor: 2.0 Max TC CSI: 0.310 Max BC CSI: 0.310 Max Web CSI: 0.314 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL N 1190 - / - / - / 615 - / 215 H 1190 - / - / - / 617 - / - Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.5 (Truss) H Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings N & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 278 - 1187 D - E 354 - 790 B - C 387 - 1101 E - F 388 - 1112 C - D 354 - 790 F - G 282 - 1220

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

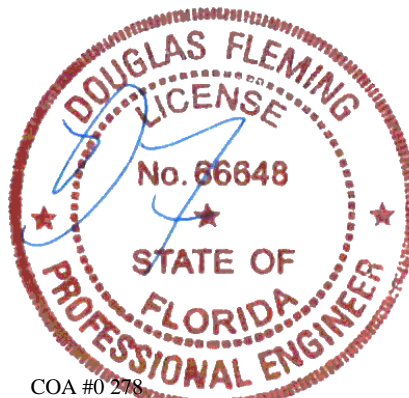
Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 11-9-0.

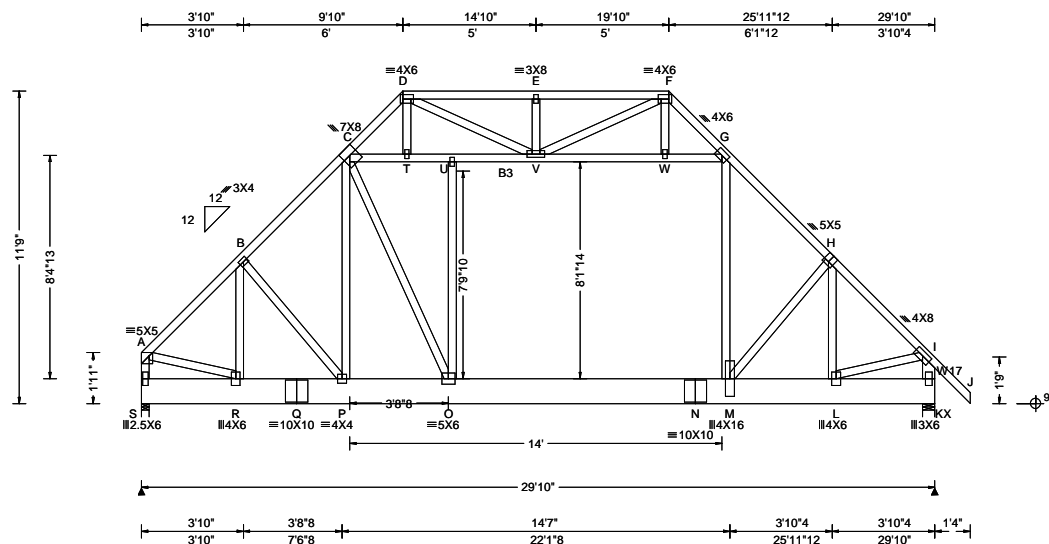


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07/27/2023

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568838 / FROM: RFG	ATIC Qty: 6	Ply: 1 Qty: 6	Job Number: 23-9526 Karlton Truss Label: H1	Cust: R 215 JRRef: 1XRQ2150001 T52 DrwNo: 208.23.0620.32545 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.75 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.096 O 999 240 VERT(CL): 0.197 O 999 180 HORZ(LL): 0.048 C - - HORZ(TL): 0.108 C - - Creep Factor: 2.0 Max TC CSI: 0.295 Max BC CSI: 0.412 Max Web CSI: 0.588 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL S 2204 -/- /- /642 -/- /254 X 2279 -/- /- /707 -/- /- Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.8 (Truss) X Brg Wid = 5.5 Min Req = 1.9 (Truss) Bearings S & X are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x12 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W17 2x6 SP 2400f-2.0E;

Plating Notes

All plates are 2X4 except as noted.

Loading

Attic room loading from 7-10-0 to 21-10-0: Live Load:
40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF,
Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Collar-tie braced with continuous lateral bracing at 24"
oc. or rigid ceiling.

Wind

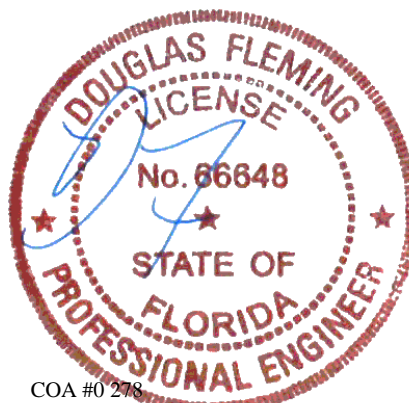
Wind loads based on MWFRS with additional C&C
member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is
11-9-0.



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
R - Q	1492 -111	O - N	1817 0
Q - P	1492 -111	N - M	1817 0
P - O	1616 -43	M - L	1545 0

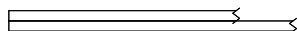
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - S	0 -2070	U - V	155 -1061
A - R	1543 0	V - W	153 -1163
R - B	10 -593	V - F	639 0
P - C	412 -107	W - G	154 -1183
C - T	156 -1095	G - M	1143 -29
C - O	711 -168	M - H	583 -122
D - T	409 -39	H - L	33 -941
D - V	467 0	L - I	1521 0
T - U	155 -1064	I - K	36 -2143

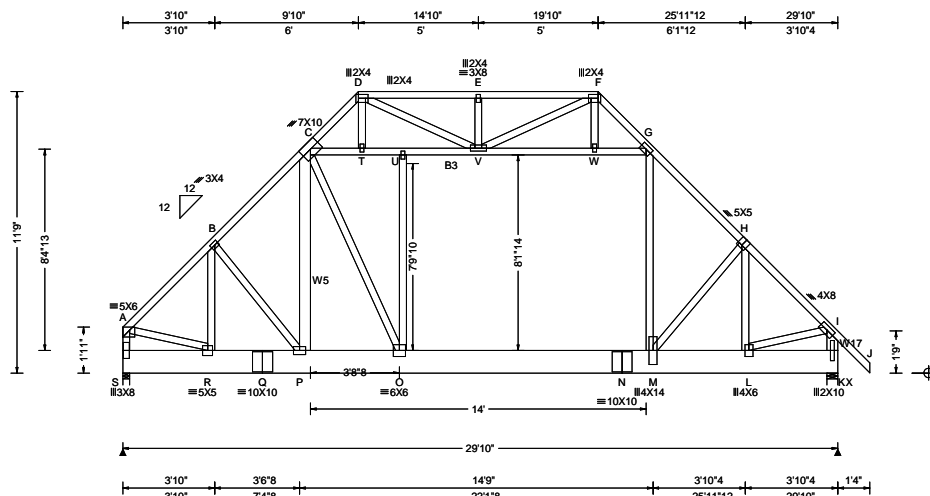
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568918 FROM: RFG	SPEC Ply: 2 Qty: 1	Job Number: 23-9526 Karlton Truss Label: H2	Cust: R 215 JRRef: 1XRQ2150001 T23 DrwNo: 208.23.0805.06763 SSB / DF 07/27/2023
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2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.08 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.111 O 999 240 VERT(CL): 0.169 O 999 180 HORZ(LL): 0.058 C - - HORZ(TL): 0.089 C - - Creep Factor: 2.0 Max TC CSI: 0.368 Max BC CSI: 0.436 Max Web CSI: 0.644 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL S 3826 -/- /- /- /218 -/ X 3646 -/- /- /- /191 -/ Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.6 (Truss) X Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings S & X are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 110 -1880 E - F 93 -1047 B - C 119 -2116 F - G 74 -799 C - D 74 -1000 G - H 96 -2232 D - E 93 -1047 H - I 89 -1788

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x12 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W5,W17 2x6 SP 2400f-2.0E;

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @10.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

Plating Notes

All plates are 4X6 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24"
oc. or rigid ceiling.

Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is
11-9-0.

Special loads

----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 108 plf at 0.00 to 108 plf at 31.17
PLT: From 28 plf at 7.83 to 28 plf at 7.59
PLT: From 20 plf at 7.83 to 20 plf at 21.83
PLT: From 100 plf at 7.83 to 100 plf at 21.83
BC: From 10 plf at 0.00 to 10 plf at 29.83
BC: From 6 plf at 29.83 to 6 plf at 31.17
BC: 893 lb Conc. Load at 7.67
BC: 163 lb Conc. Load at 7.83,21.83
BC: 883 lb Conc. Load at 16.79

It is the responsibility of the Building Designer and
Truss Fabricator to review this drawing prior to
cutting lumber to verify that all data,including
dimensions and loads, conform to the architectural
plans/specifications and fabricator's truss layout.

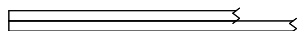


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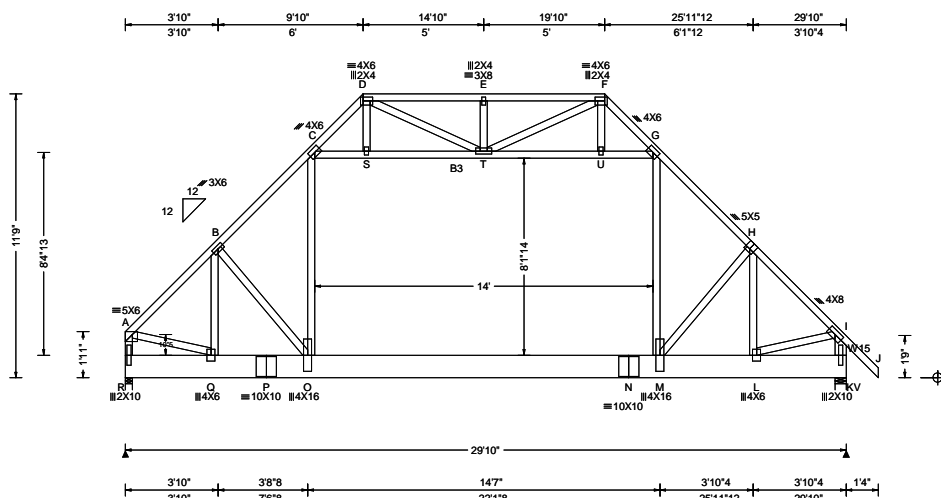
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568920 FROM: RFG	ATIC Ply: 2 Qty: 1	Job Number: 23-9526 Karlton Truss Label: H3	Cust: R 215 JRRef: 1XQR2150001 T1 DrwNo: 208.23.0805.18760 SSB / DF 07/27/2023
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2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.08 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.068 O 999 240 VERT(CL): 0.097 M 999 180 HORZ(LL): -0.033 G - - HORZ(TL): 0.062 C - - Creep Factor: 2.0 Max TC CSI: 0.363 Max BC CSI: 0.385 Max Web CSI: 0.610 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL R 3646 -/- /- /- /148 -/ V 3662 -/- /- /- /170 -/ Wind reactions based on MWFRS R Brg Wid = 3.5 Min Req = 1.5 (Truss) V Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings R & V are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x12 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W15 2x6 SP 2400f-2.0E;

Nailnote
Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @10.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

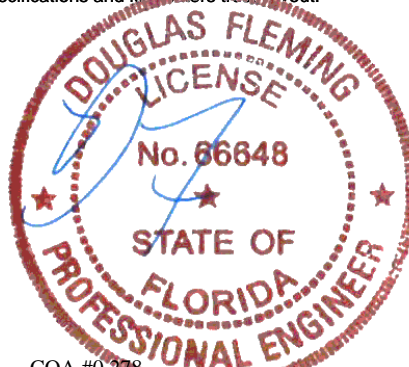
Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 11-9-0.

Special loads
-----(Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 108 plf at 0.00 to 108 plf at 31.17
PLT: From 28 plf at 7.83 to 28 plf at 7.59
PLT: From 20 plf at 7.83 to 20 plf at 21.83
PLT: From 100 plf at 7.83 to 100 plf at 21.83
BC: From 10 plf at 0.00 to 10 plf at 29.83
BC: From 6 plf at 29.83 to 6 plf at 31.17
BC: 163 lb Conc. Load at 7.83,21.83
BC: 1293 lb Conc. Load at 13.00
BC: 318 lb Conc. Load at 16.79

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricator's layout.



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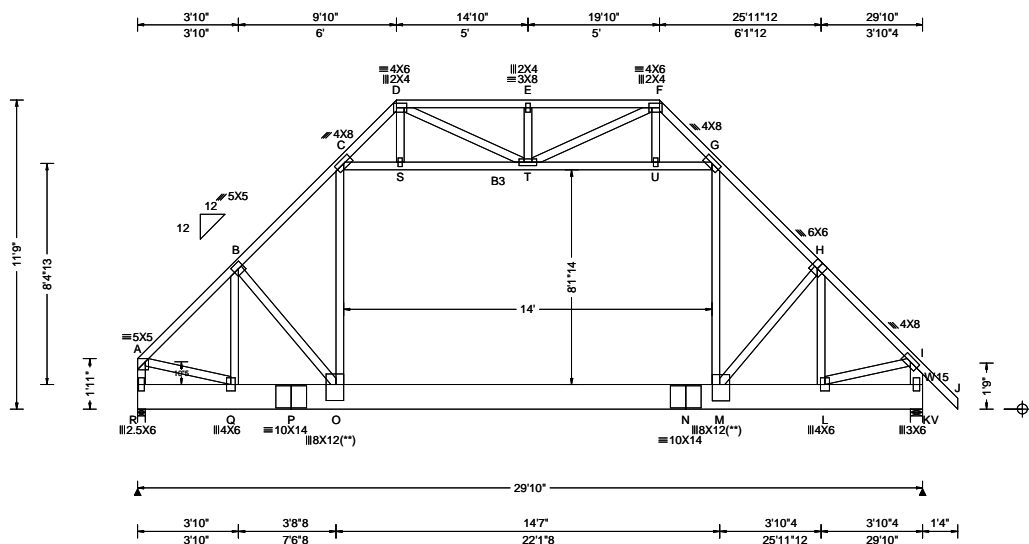
Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
Q - P	1258 -48	N - M	1530 -45
P - O	1258 -48	M - L	1252 -49
O - N	1530 -45		

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
A - R	75 -1730	T - U	0 -1031
A - Q	1282 -49	T - F	388 -49
Q - B	26 -949	U - G	0 -1047
B - O	481 0	G - M	987 0
O - C	996 0	M - H	492 0
C - S	0 -1038	H - L	22 -937
D - T	385 -49	L - I	1224 -48
S - T	0 -1023	I - K	87 -1741

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Glenview, IL 60025

SEQN: 568713 / FROM: RFG	ATIC Qty: 2	Ply: 1	Job Number: 23-9526 Karlton Truss Label: H4	Cust: R 215 JRRef: 1XRQ2150001 T51 DrwNo: 208.23.0620.32405 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.08 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.064 M 999 240 VERT(CL): 0.140 M 999 180 HORZ(LL): 0.031 C - - HORZ(TL): 0.069 C - - Creep Factor: 2.0 Max TC CSI: 0.289 Max BC CSI: 0.262 Max Web CSI: 0.586 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL R 2204 - / - / - / 642 - / 254 V 2279 - / - / - / 707 - / - Non-Gravity Wind reactions based on MWFRS R Brg Wid = 3.5 Min Req = 1.8 (Truss) V Brg Wid = 5.5 Min Req = 1.9 (Truss) Bearings R & V are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 8 - 2140 E - F 0 - 1048 B - C 60 - 2660 F - G 0 - 903 C - D 0 - 907 G - H 55 - 2658 D - E 0 - 1048 H - I 16 - 2221

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x12 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W15 2x6 SP 2400f-2.0E;

Plating Notes

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Attic room loading from 7-10-0 to 21-10-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

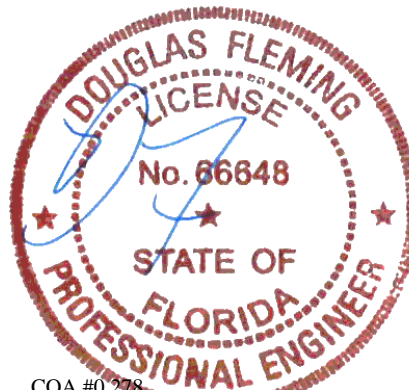
Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 11-9-0.

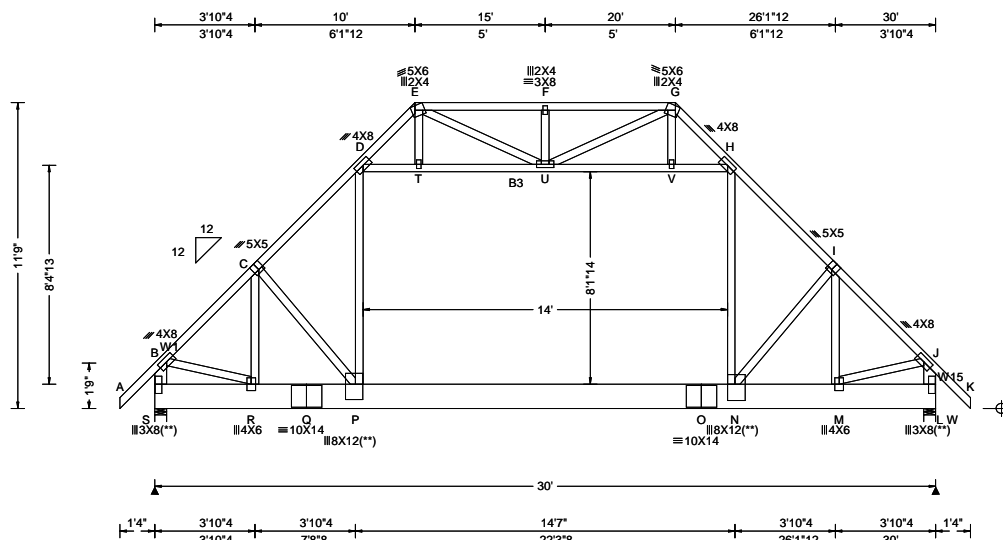


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Glenview, IL 60025

SEQN: 568682 FROM: RFG	SPEC	Ply: 1 Qty: 4	Job Number: 23-9526 Karlton Truss Label: H7	Cust: R 215 JRRef: 1XRQ2150001 T50 DrwNo: 208.23.0805.20487 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.08 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.063 P 999 240 VERT(CL): 0.138 P 999 180 HORZ(LL): 0.035 D - - HORZ(TL): 0.076 D - - Creep Factor: 2.0 Max TC CSI: 0.290 Max BC CSI: 0.262 Max Web CSI: 0.584 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL S 2295 -/- /- /790 -/- /273 W 2290 -/- /- /806 -/- /- Wind reactions based on MWFRS S Brg Wid = 5.5 Min Req = 1.9 (Truss) W Brg Wid = 5.5 Min Req = 1.9 (Truss) Bearings S & W are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 42 -2235 F - G 0 -1049 C - D 73 -2683 G - H 0 -905 D - E 0 -908 H - I 72 -2683 E - F 0 -1049 I - J 34 -2232

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x12 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W1, W15 2x6 SP 2400f-2.0E;

Plating Notes

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Attic room loading from 8-0-0 to 22-0-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

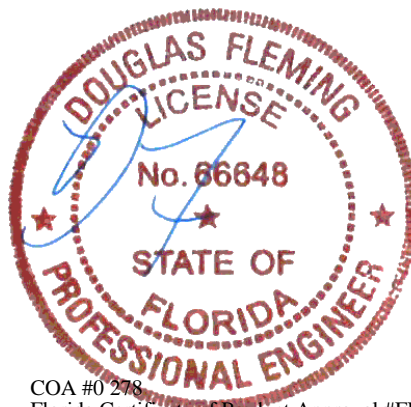
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 11-9-0.



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Maximum Bot Chord Forces Per Ply (lbs)

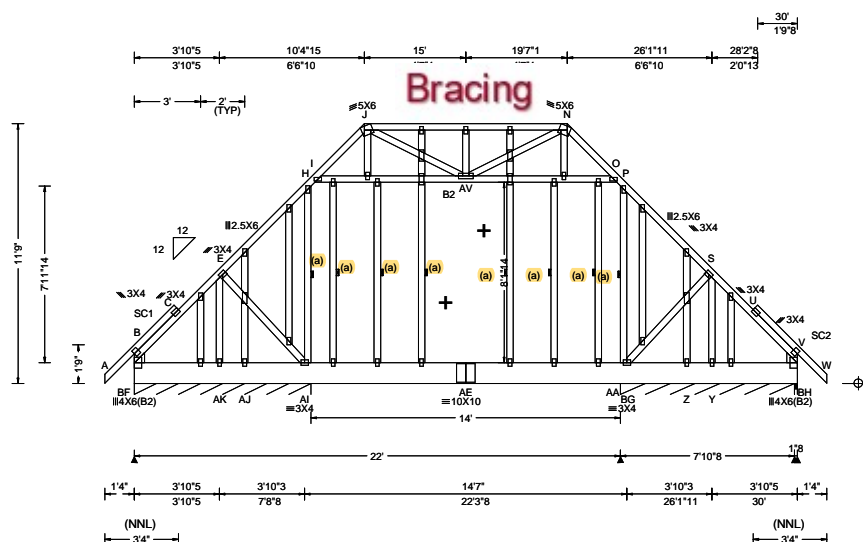
Chords	Tens.Comp.	Chords	Tens. Comp.
R - Q	1555 -86	O - N	1812 0
Q - P	1555 -86	N - M	1553 0
P - O	1812 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - S	71 -2154	U - V	187 -1191
B - R	1534 0	U - G	472 0
R - C	60 -940	V - H	188 -1213
C - P	600 -143	H - N	1242 -26
P - D	1236 -16	N - I	606 -150
D - T	178 -1212	I - M	70 -947
E - U	472 0	M - J	1532 0
T - U	176 -1191	J - L	64 -2152

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.08 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.049 AX 999 240 VERT(CL): 0.084 M 999 180 HORZ(LL): 0.012 Q - - HORZ(TL): 0.023 Q - - Creep Factor: 2.0 Max TC CSI: 0.321 Max BC CSI: 0.263 Max Web CSI: 0.518 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL BF* 307 -/- /- /115 /25 /39 BG* 223 -/- /- /96 /16 /- BH 759 -/- /- /278 /77 /- AJ -/-315 Z -/-317 Wind reactions based on MWFRS BF Brg Wid = 96.0 Min Req = - BG Brg Wid = 94.5 Min Req = - BH Brg Wid = 1.5 Min Req = 1.5 (Truss) Bearings BF, BG, & BH are a rigid surface. Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x12 SP 2400f-2.0E; B2 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;
Lt Wedge: 2x6 SP 2400f-2.0E;
Rt Wedge: 2x6 SP 2400f-2.0E;

Wind
Wind loads based on MWFRS.
Wind loading based on both gable and hip roof types.

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

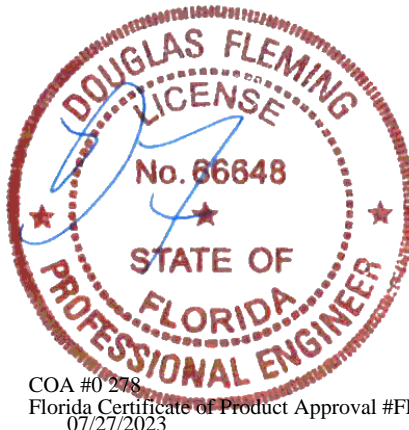
Loading
Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.
Attic room loading from 8-0-0 to 22-0-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.
+ Member to be laterally braced for out of plane wind loads

Maximum Top Chord Forces Per Ply (lbs)					
Chords	Tens.Comp.	Chords	Tens.	Comp.	
B - C	299 -680	L - N	67	-1386	
C - E	68 -616	N - O	68	-1201	
E - H	63 -889	O - P	66	-1050	
H - I	65 -1042	P - S	63	-892	
I - J	67 -1188	S - U	44	-620	
J - L	67 -1386	U - V	246	-682	

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
B - AK	394 -142	AE-AA	565 -96
AK-AI	392 -141	AA-Y	392 -68
AI-AE	565 -96	Y - V	395 -68

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
AK- E	29 -403	AV- N	671 -32
AI- H	46 -529	P-AA	47 -547
J-AV	679 -32	S - Y	29 -404
L-AV	22 -449		



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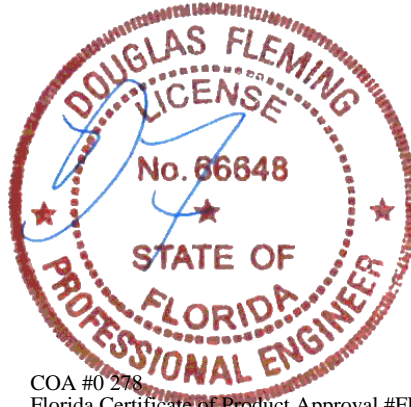
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FROM: RFG		Qty: 1	Karlton	DrwNo: 208.23.0805.37223
Page 2 of 2			Truss Label: H7E	SSB / DF 07/27/2023

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

The overall height of this truss excluding overhang is 11-9-0.



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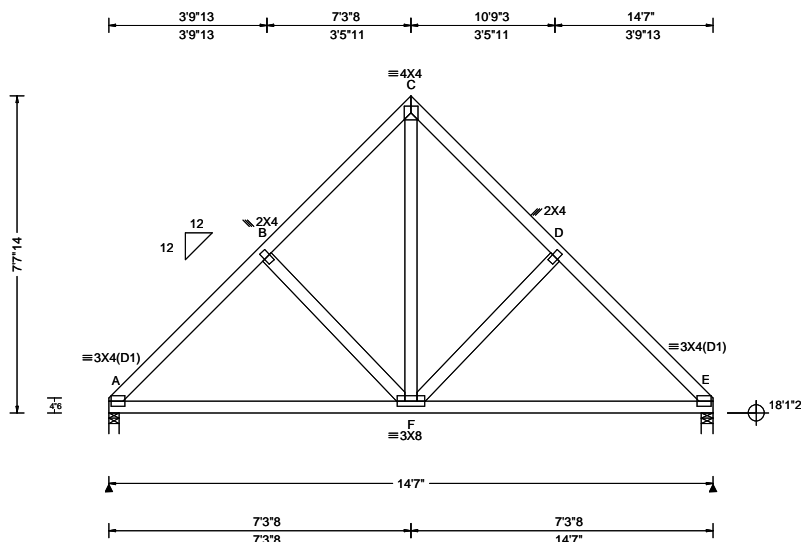
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568824 FROM: RFG	COMN Ply: 1 Qty: 3	Job Number: 23-9526 Karlton Truss Label: K1	Cust: R 215 JRef: 1XRQ2150001 T59 DrwNo: 208.23.0805.38600 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 22.10 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.010 F 999 240 VERT(CL): 0.019 F 999 180 HORZ(LL): 0.005 B - - HORZ(TL): 0.010 B - - Creep Factor: 2.0 Max TC CSI: 0.140 Max BC CSI: 0.463 Max Web CSI: 0.168 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 581 /- /- /306 /9 /165 E 583 /- /- /307 /9 /- Wind reactions based on MWFRS A Brg Wid = 3.0 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 167 -649 C - D 199 -505 B - C 199 -505 D - E 166 -648

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

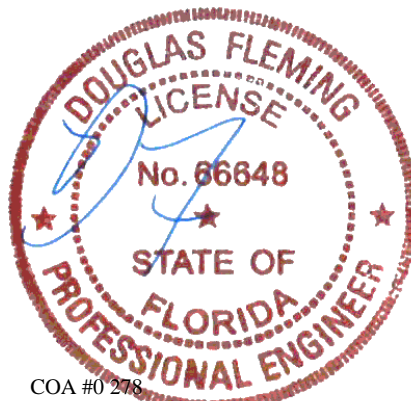
The overall height of this truss excluding overhang is 7-7-14.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - F	416 -78	F - E	414 -59

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
C - F	421 -167

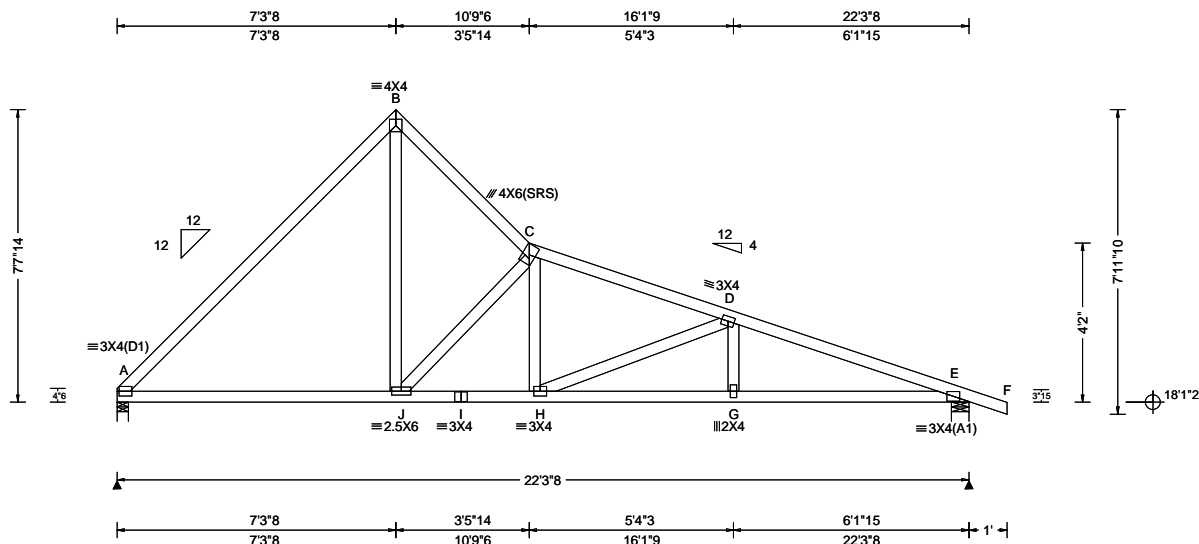


COA #0 278
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07/27/2023

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568688 FROM: RFG	COMN Ply: 1 Qty: 5	Job Number: 23-9526 Karlton Truss Label: L1	Cust: R 215 JRRef: 1XRQ2150001 T38 DrwNo: 208.23.0805.47773 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.92 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.091 D 999 240 VERT(CL): 0.163 D 999 180 HORZ(LL): 0.049 B - - HORZ(TL): 0.088 B - - Creep Factor: 2.0 Max TC CSI: 0.718 Max BC CSI: 0.656 Max Web CSI: 0.483 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL A 973 -/- /- /438 /78 /168 E 939 -/- /- /437 /108 -/ Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 222 - 1101 C - D 314 - 1456 B - C 266 - 1025 D - E 448 - 2068

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

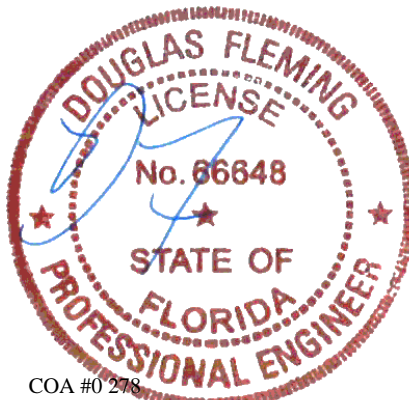
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 7-7-14.



COA #0 278
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07/27/2023

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - J	680 -8	H - G	1915 -372
J - I	1314 -171	G - E	1920 -370
I - H	1314 -171		

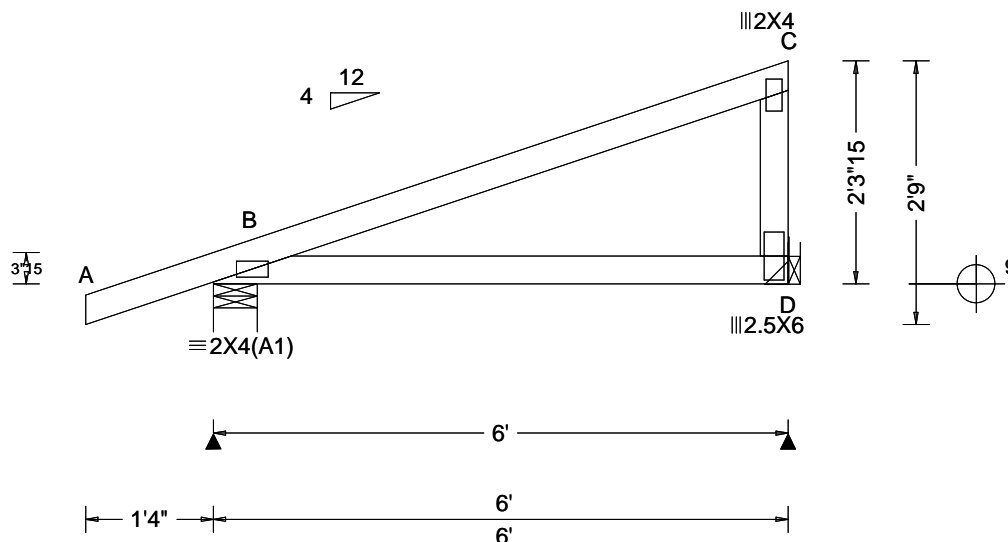
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - J	996 -199	H - D	215 -631
J - C	277 -934		

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568736 / FROM: RFG	MONO Ply: 1 Qty: 8	Job Number: 23-9526 Karlton Truss Label: M1	Cust: R 215 JRef: 1XRQ2150001 T16 DrwNo: 208.23.0620.32139 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.008 B - - HORZ(TL): 0.015 B - - Creep Factor: 2.0 Max TC CSI: 0.390 Max BC CSI: 0.308 Max Web CSI: 0.171 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 319 - / - /163 /35 /56 D 208 - / - /107 /33 - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=5'9" uses the following

support conditions: 5'9"

Bearing D (5'9", 9') LUS24

Supporting Member: (1)2x4 SP M-31

(4) 0.148"x3" nails into supporting

member,

(2) 0.148"x3" nails into supported

member.

Additional Notes

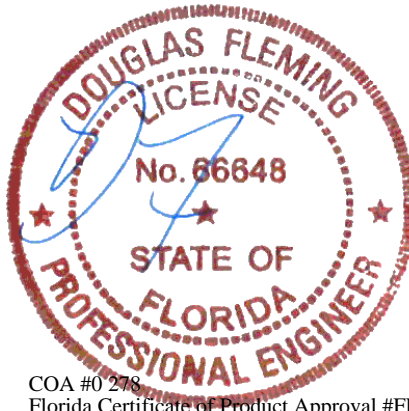
The overall height of this truss excluding overhang is 2-3-15.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278

Florida Certificate of Product Approval #FL1999
07/27/2023

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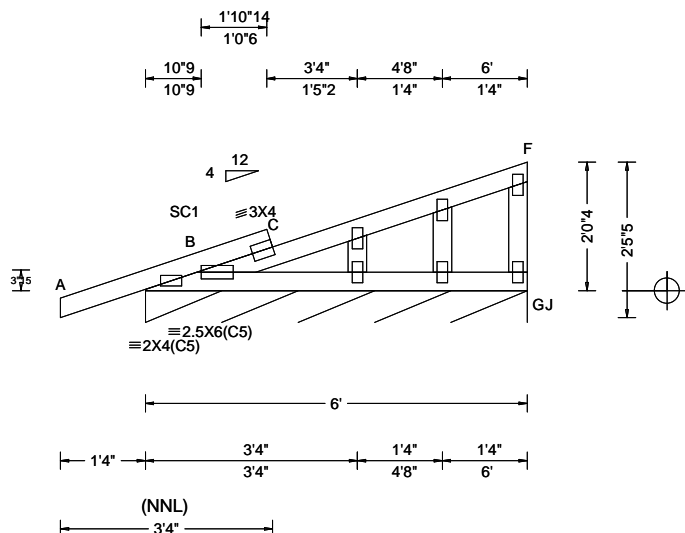
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568738 / FROM: RFG	GABL Ply: 1 Qty: 2	Job Number: 23-9526 Karlton Truss Label: M1E	Cust: R 215 JRef: 1XRQ2150001 T18 DrwNo: 208.23.0620.31920 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.002 C 999 240 VERT(CL): 0.004 C 999 180 HORZ(LL): -0.001 F - - HORZ(TL): 0.001 F - - Creep Factor: 2.0 Max TC CSI: 0.207 Max BC CSI: 0.052 Max Web CSI: 0.031 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J* 100 /- /- /48 /47 /21 Wind reactions based on MWFRS J Brg Wid = 72.0 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

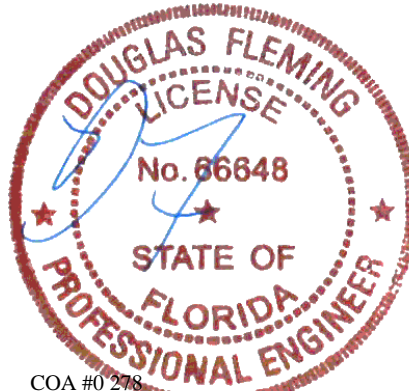
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 2-0-4.



COA #0278
Florida Certificate of Product Approval #FL1999
07/27/2023

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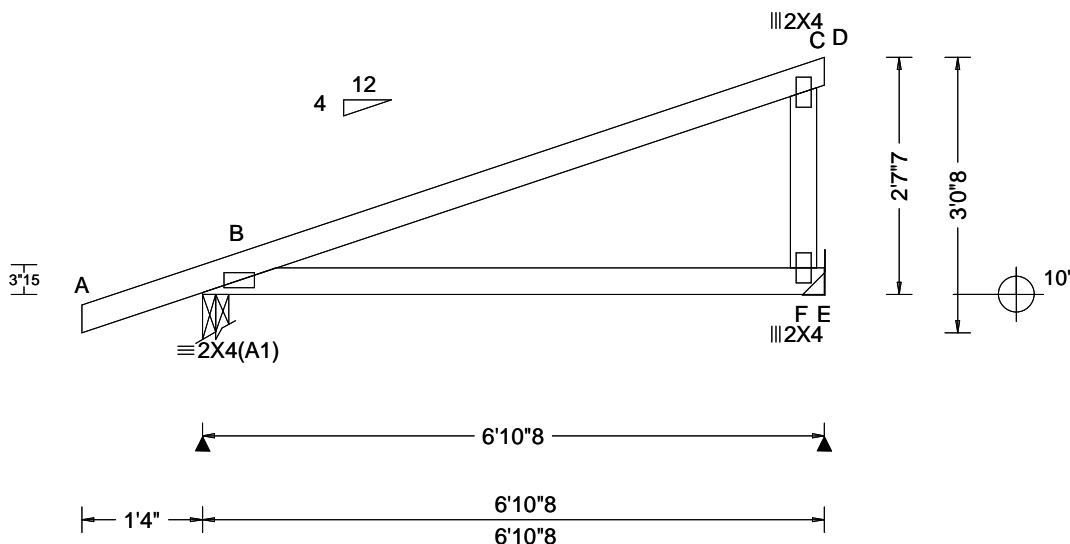
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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568783 FROM: RFG	MONO Ply: 1 Qty: 12	Job Number: 23-9526 Karlton Truss Label: M2	Cust: R 215 JRef: 1XRQ2150001 T24 DrwNo: 208.23.0805.49757 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.012 B - - HORZ(TL): 0.022 B - - Creep Factor: 2.0 Max TC CSI: 0.522 Max BC CSI: 0.406 Max Web CSI: 0.185 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 348 - / - /177 /23 /47 E 245 - / - /126 /13 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

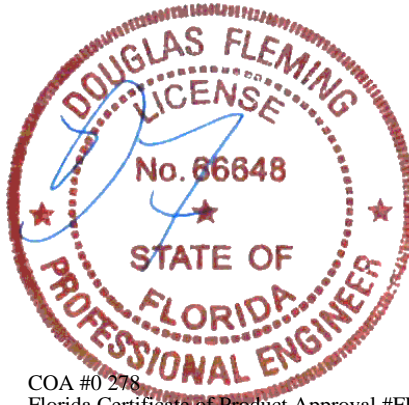
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2'-7".

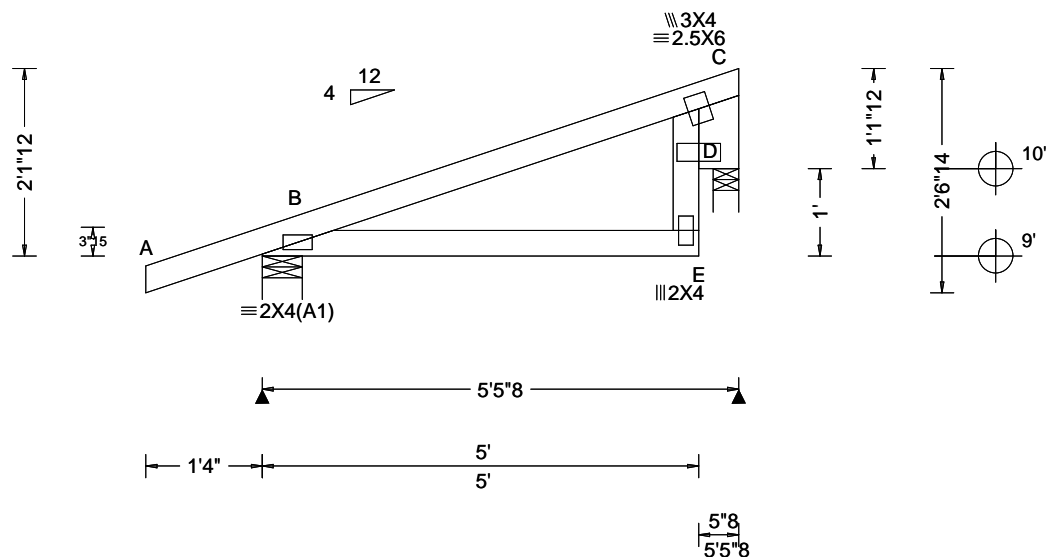


COA #0 278
Florida Certificate of Product Approval #FL1999
07/27/2023

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568860 FROM: RFG	SPEC Ply: 1 Qty: 6	Job Number: 23-9526 Karlton Truss Label: M3	Cust: R 215 JRef: 1XRQ2150001 T11 DrwNo: 208.23.0805.51313 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 B 999 240 VERT(CL): 0.016 B 999 180 HORZ(LL): 0.004 B - - HORZ(TL): 0.006 B - - Creep Factor: 2.0 Max TC CSI: 0.195 Max BC CSI: 0.200 Max Web CSI: 0.260 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 293 - / - / 149 / 32 / 52 D 184 - / - / 90 / 33 / - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) D Brg Wid = 3.5 Min Req = 1.5 (Support) Bearings B & D are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - D 513 -378

Lumber

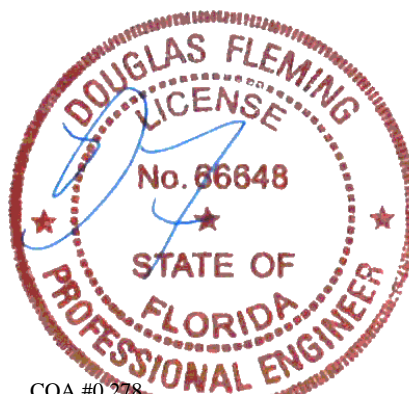
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x6 SP 2400f-2.0E;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2'-1-12.

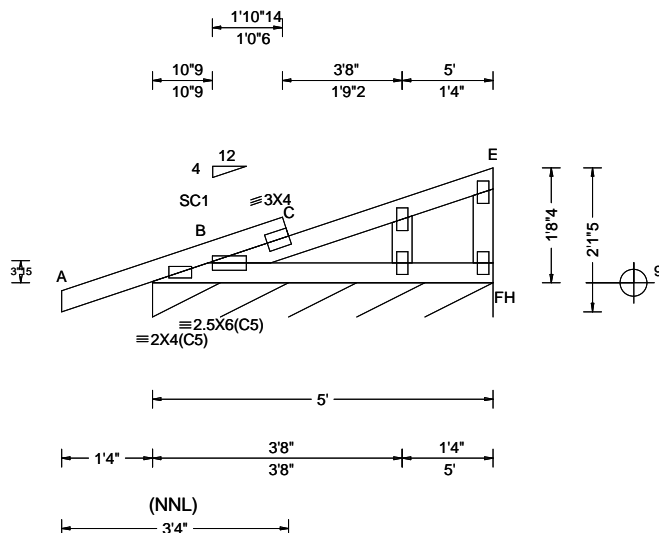


COA #0 278
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07/27/2023

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568885 / FROM: RFG	GABL Ply: 1 Qty: 2	Job Number: 23-9526 Karlton Truss Label: M3E	Cust: R 215 JRef: 1XRQ2150001 T33 DrwNo: 208.23.0620.32108 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 C 999 240 VERT(CL): 0.007 C 999 180 HORZ(LL): -0.001 C - - HORZ(TL): 0.001 E - - Creep Factor: 2.0 Max TC CSI: 0.207 Max BC CSI: 0.068 Max Web CSI: 0.033 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H* 102 /- /- /49 /53 /23 Wind reactions based on MWFRS H Brg Wid = 60.0 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

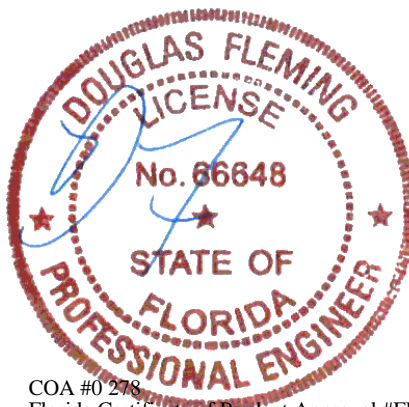
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 1-8-4.

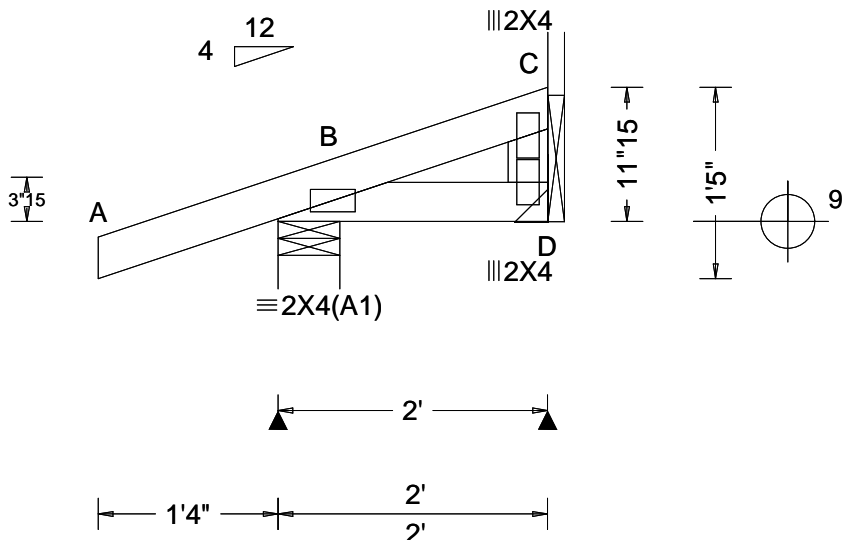


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07/27/2023

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568700 / FROM: RFG	MONO Ply: 1 Qty: 7	Job Number: 23-9526 Karlton Truss Label: M4	Cust: R 215 JRef: 1XRQ2150001 T40 DrwNo: 208.23.0620.32078 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.138 Max BC CSI: 0.020 Max Web CSI: 0.014 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 194 - / - /100 /35 /24 D 39 - / - /26 /7 - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

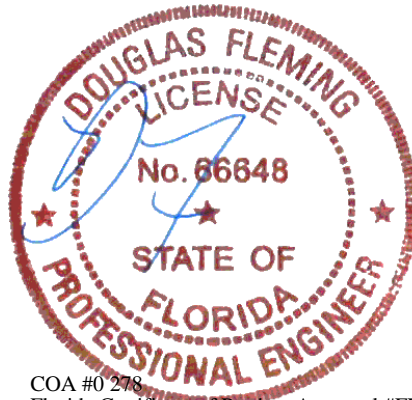
(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 0-11-15.

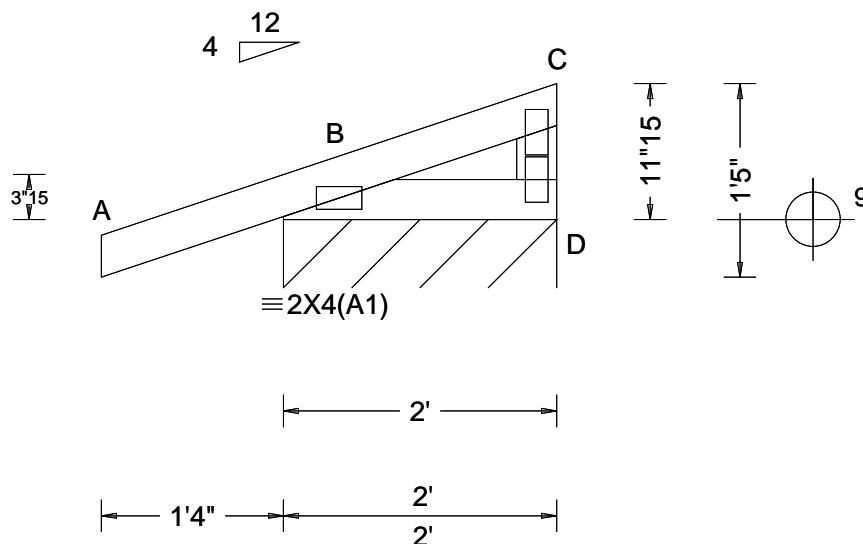


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568871 / FROM: RFG	GABL Ply: 1 Qty: 2	Job Number: 23-9526 Karlton Truss Label: M4E	Cust: R 215 JRef: 1XRQ2150001 T32 DrwNo: 208.23.0620.32359 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.156 Max BC CSI: 0.022 Max Web CSI: 0.017 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 127 /- /- /63 /75 /30 Wind reactions based on MWFRS D Brg Wid = 24.0 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

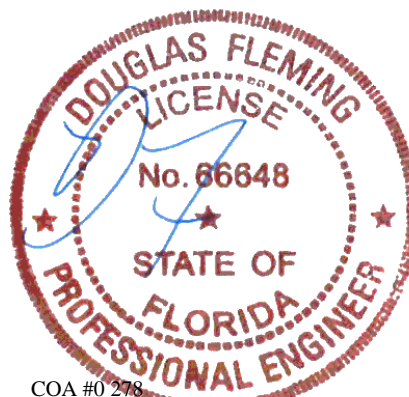
Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

The overall height of this truss excluding overhang is 0-11-15.



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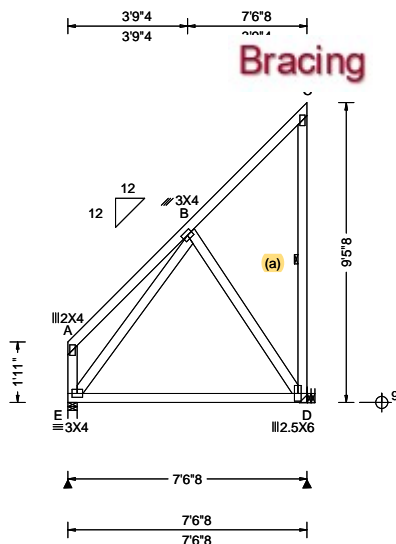
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568684 / FROM: RFG	MONO Ply: 1 Qty: 5	Job Number: 23-9526 Karlton Truss Label: M5	Cust: R 215 JRef: 1XRQ2150001 T25 DrwNo: 208.23.0620.31733 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.003 B 999 180 HORZ(LL): -0.003 C - - HORZ(TL): 0.004 C - - Creep Factor: 2.0 Max TC CSI: 0.246 Max BC CSI: 0.643 Max Web CSI: 0.220 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 301 - / - / 149 - / 178 D 301 - / - / 270 / 150 - Wind reactions based on MWFRS E Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

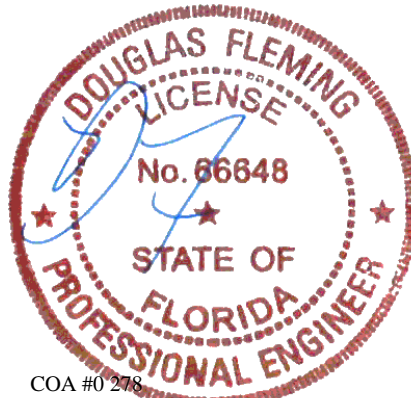
Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9'-5-8".

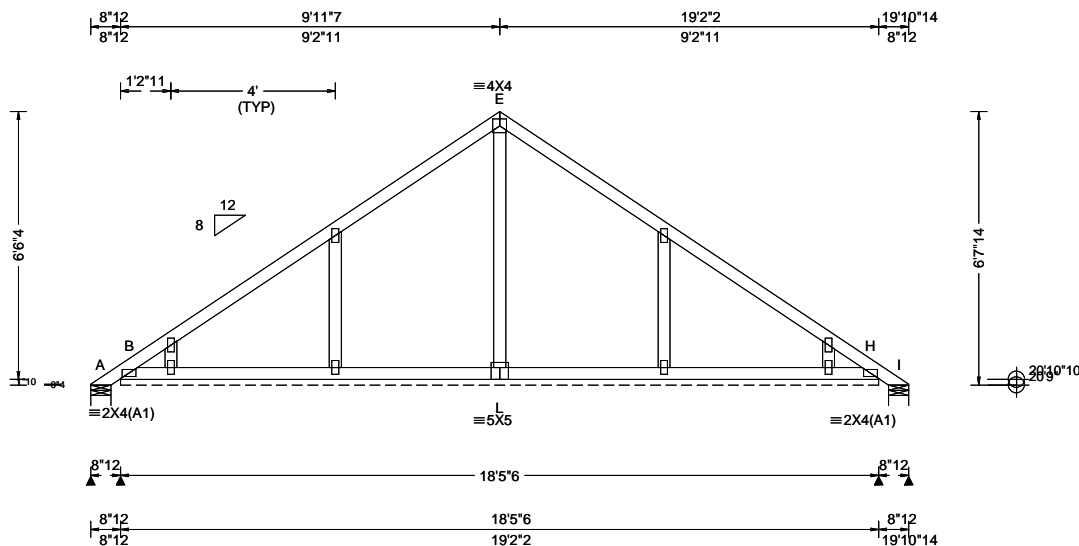


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155 Harlem Ave
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Glenview, IL 60025

SEQN: 568842 / FROM: RFG	COMN Ply: 1 Qty: 24	Job Number: 23-9526 Karlton Truss Label: P1	Cust: R 215 JRRef: 1XRQ2150001 T13 DrwNo: 208.23.0620.31842 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.54 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.15 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 E 999 240 VERT(CL): 0.002 E 999 180 HORZ(LL): 0.002 F - - HORZ(TL): 0.002 F - - Creep Factor: 2.0 Max TC CSI: 0.194 Max BC CSI: 0.056 Max Web CSI: 0.116 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 21 - / - /85 /70 /130 B* 62 - / - /43 /13 - I 21 - / - /14 /3 - Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 221 Min Req = - I Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

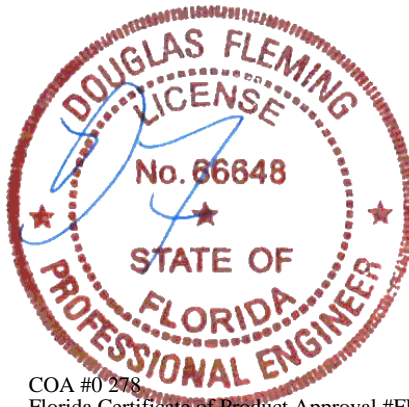
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 6'-7-14.

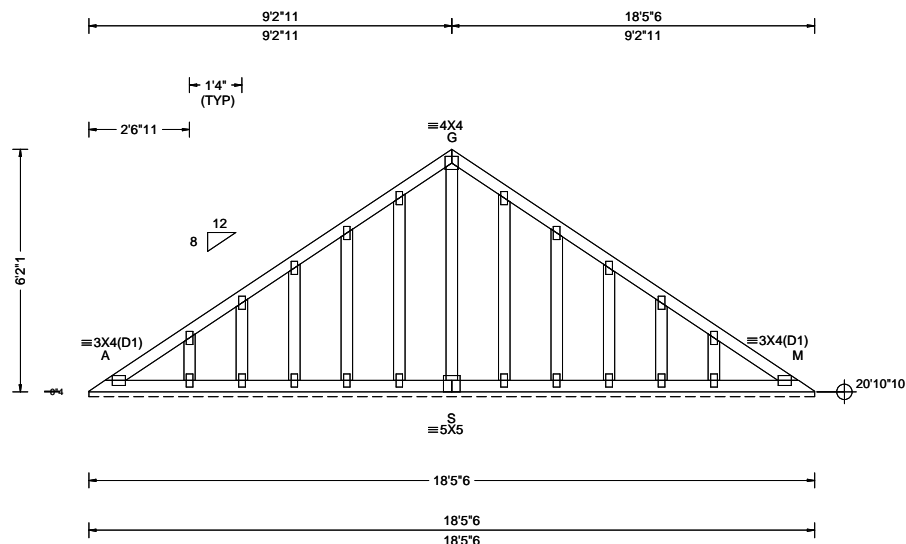


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568883 / FROM: RFG	GABL Ply: 1 Qty: 2	Job Number: 23-9526 Karlton Truss Label: P1E	Cust: R 215 JRef: 1XRQ2150001 T10 DrwNo: 208.23.0620.31873 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.54 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.15 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 A 999 240 VERT(CL): 0.006 A 999 180 HORZ(LL): -0.001 M - - HORZ(TL): 0.002 A - - Creep Factor: 2.0 Max TC CSI: 0.129 Max BC CSI: 0.113 Max Web CSI: 0.054 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A* 175 -/- /35 -/- Wind reactions based on MWFRS A Brg Wid = 221 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS.

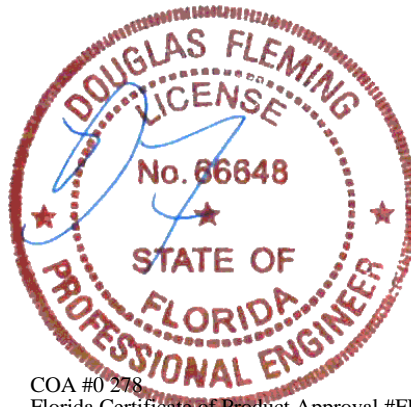
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 6'-3-11".

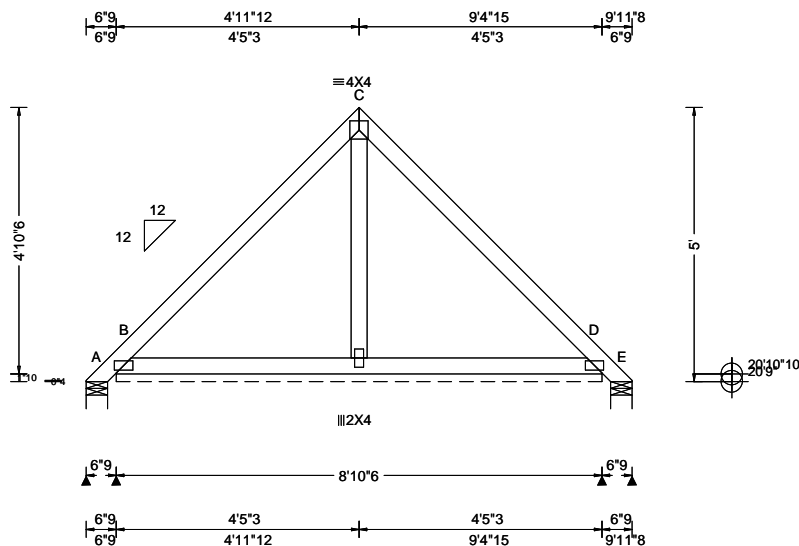


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07/27/2023

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568873 / FROM: RFG	COMN Ply: 1 Qty: 22	Job Number: 23-9526 Karlton Truss Label: P2	Cust: R 215 JRef: 1XRQ2150001 T36 DrwNo: 208.23.0620.31998 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 18.33 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): 0.002 D - - HORZ(TL): 0.002 D - - Creep Factor: 2.0 Max TC CSI: 0.222 Max BC CSI: 0.094 Max Web CSI: 0.033 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-162 /- /166 /235 /107 B* 106 /- /- /74 /41 /- E - /-162 /- /130 /154 /- Wind reactions based on MWFRS A Brg Wid = 4.7 Min Req = 1.5 (Truss) B Brg Wid = 106 Min Req = - E Brg Wid = 4.7 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

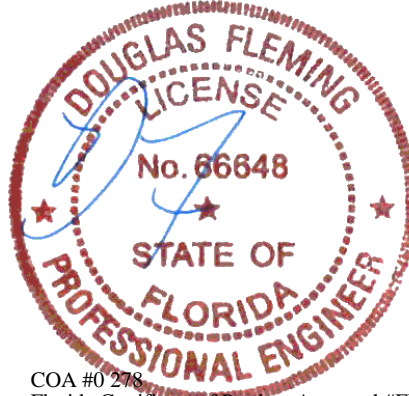
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 5'-0".

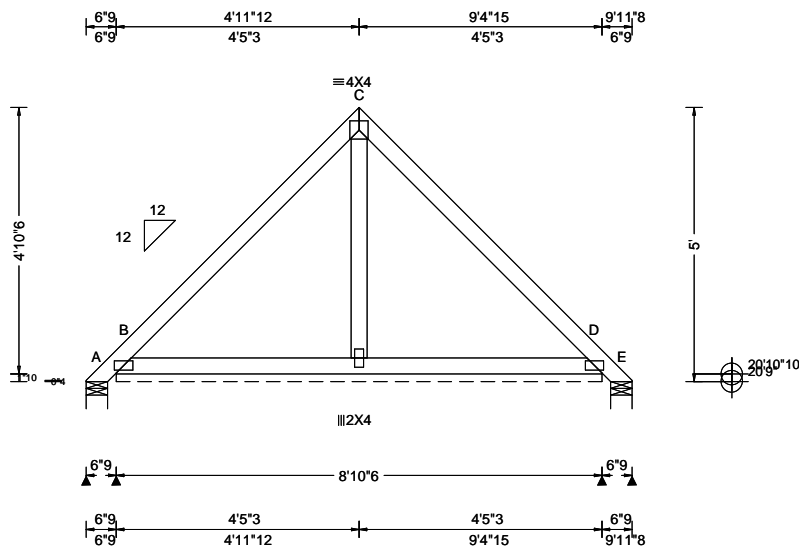


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07/27/2023

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568875 / FROM: RFG	ATIC Qty: 2	Ply: 1 Qty: 2	Job Number: 23-9526 Karlton Truss Label: P2A	Cust: R 215 JRRef: 1XRQ2150001 T9 DrwNo: 208.23.0620.32390 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 18.25 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): -0.001 D - - HORZ(TL): 0.002 D - - Creep Factor: 2.0 Max TC CSI: 0.222 Max BC CSI: 0.087 Max Web CSI: 0.033 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-162 /- /166 /233 /107 B* 105 /- /- /72 /17 /- E - /-162 /- /87 /153 /- Wind reactions based on MWFRS A Brg Wid = 4.7 Min Req = 1.5 (Truss) B Brg Wid = 106 Min Req = - E Brg Wid = 4.7 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS.

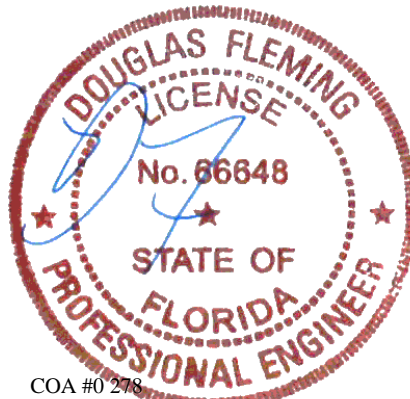
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 5'-0".

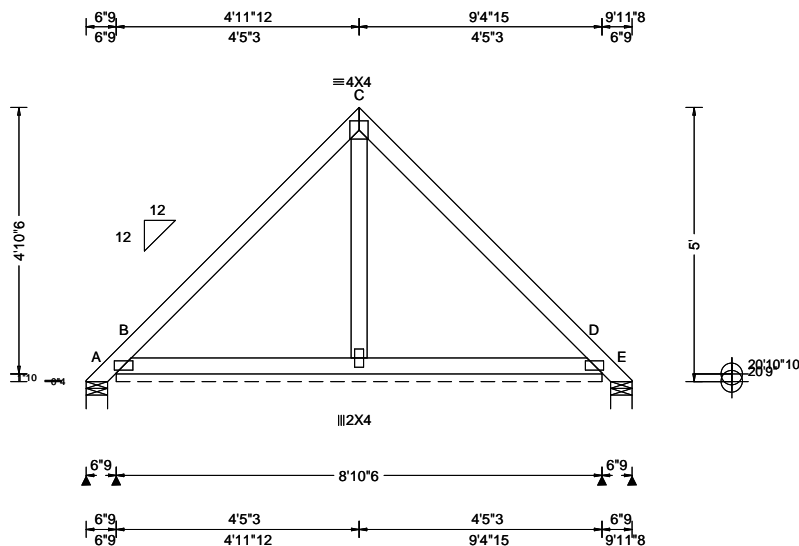


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568877 / FROM: RFG	ATIC Ply: 1 Qty: 2	Job Number: 23-9526 Karlton Truss Label: P2B	Cust: R 215 JRef: 1XRQ2150001 T39 DrwNo: 208.23.0620.32248 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 18.25 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): -0.001 D - - HORZ(TL): 0.002 D - - Creep Factor: 2.0 Max TC CSI: 0.222 Max BC CSI: 0.087 Max Web CSI: 0.033 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-162 /- /166 /233 /107 B* 105 /- /- /72 /17 /- E - /-162 /- /87 /153 /- Wind reactions based on MWFRS A Brg Wid = 4.7 Min Req = 1.5 (Truss) B Brg Wid = 106 Min Req = - E Brg Wid = 4.7 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS.

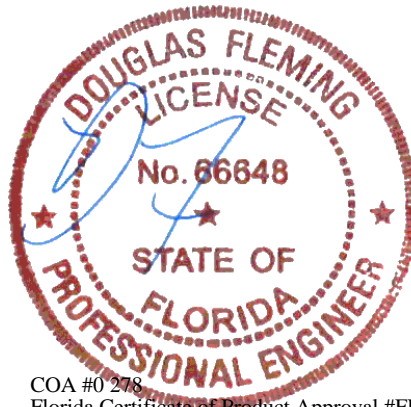
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 5'-0".

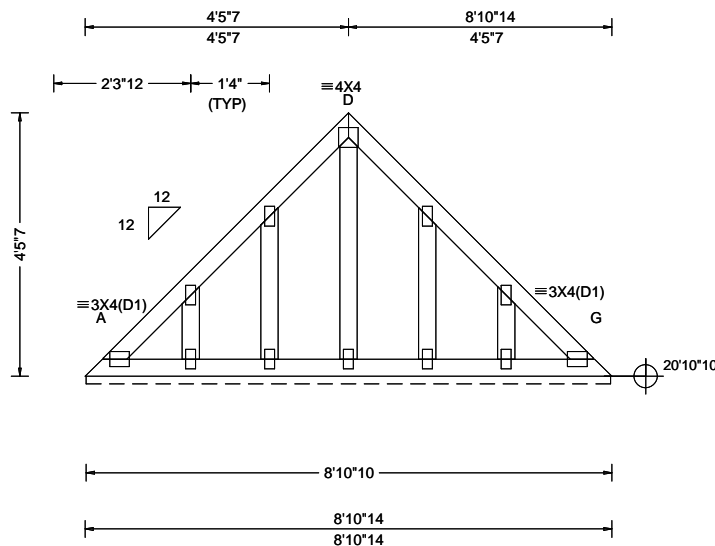


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 568879 / FROM: RFG	GABL Ply: 1 Qty: 2	Job Number: 23-9526 Karlton Truss Label: P2E	Cust: R 215 JRef: 1XRQ2150001 T31 DrwNo: 208.23.0620.32217 SSB / DF 07/27/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.26 ft TCDL: 4.2 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 10.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 G 999 240 VERT(CL): 0.002 G 999 180 HORZ(LL): 0.000 F - - HORZ(TL): 0.002 F - - Creep Factor: 2.0 Max TC CSI: 0.078 Max BC CSI: 0.065 Max Web CSI: 0.026 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A* 172 -/- /34 -/- Wind reactions based on MWFRS A Brg Wid = 106 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

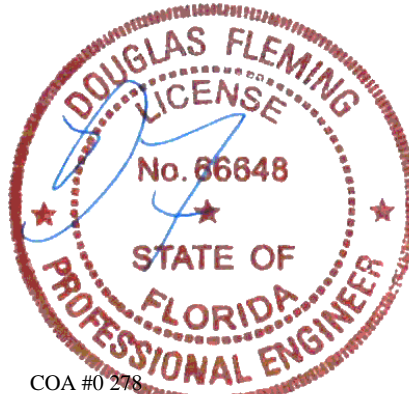
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 4-7-1.



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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
			Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Standard	#1	4' 6"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
			#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
	DFL	Stud	#1	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
			#2	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	10' 2"	10' 7"	11' 10"	14' 0"	14' 0"	14' 0"
			Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	10' 2"	10' 7"	11' 10"	14' 0"	14' 0"	14' 0"
	16" O.C.	SPF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 5"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	Stud	#1	4' 9"	7' 4"	7' 9"	9' 9"	10' 3"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 9"	7' 4"	7' 9"	9' 9"	10' 3"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
	DFL	#1 / #2	#1	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	12" O.C.	Standard	#1	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
	12" O.C.	Stud	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	12" O.C.	Standard	#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:				Group B:			
Spruce-Pine-Fir		Hem-Fir		Douglas Fir-Larch		Southern Pine***	
#1 / #2	Standard	#2	Stud	#3	Stud	#3	Stud
#3	Stud	#3	Stud	#1	Stud	#1	Stud

Group B:			
Hem-Fir		Douglas Fir-Larch	
#1 & Btr	#1	#1	#2
#1	#1	#1	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c.

in 18' end zones and 4' o.c. between zones.

** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

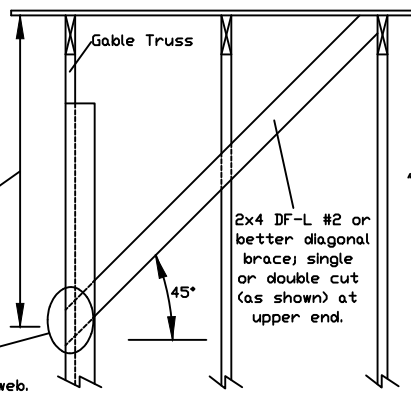
+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

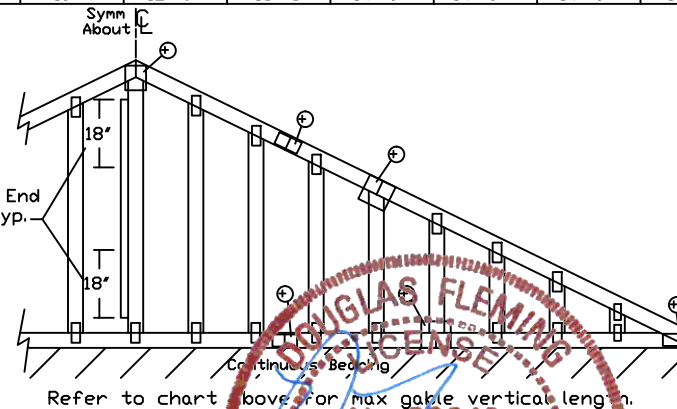
Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



'L' Brace End Zones, typ.



Refer to chart above for max gable vertical length.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

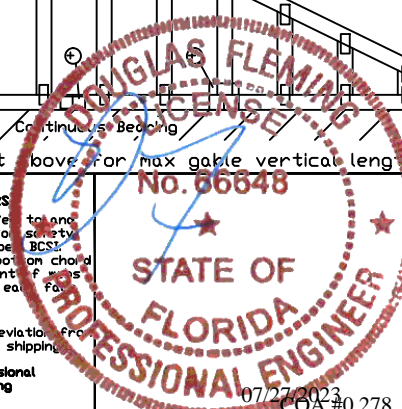
A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:

ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-16-GAB14015

DATE 01/26/2018

DRWG A14015ENC160118

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

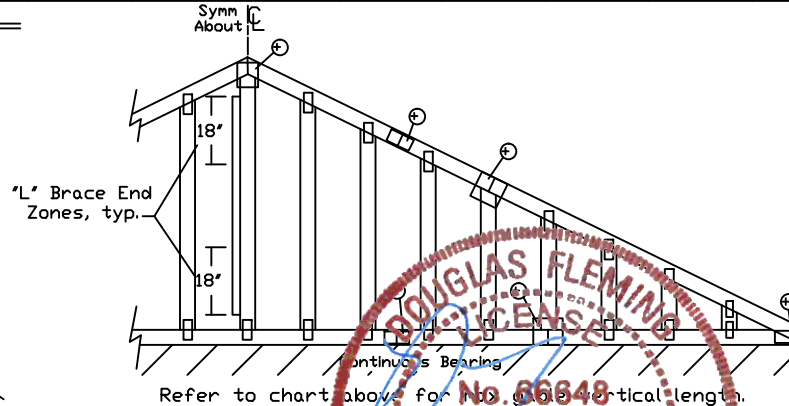
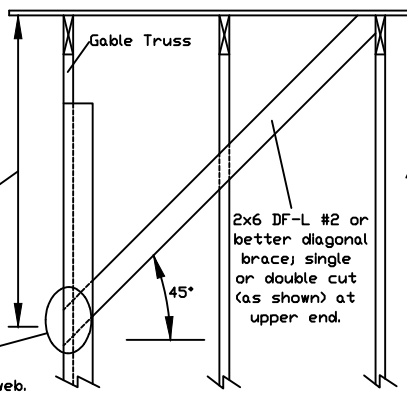
Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
			#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
			Stud	3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Standard	#1	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
			#2	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
			Stud	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
	SP	#1	#1	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
			#2	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
			Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Standard	#1	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
			#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	#1	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"
			Stud	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	#1	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
			#2	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
			Stud	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
		Standard	#1	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#1 / #2	#1	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	#1	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 525# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c.

in 18' end zones and 4' o.c. between zones.

** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

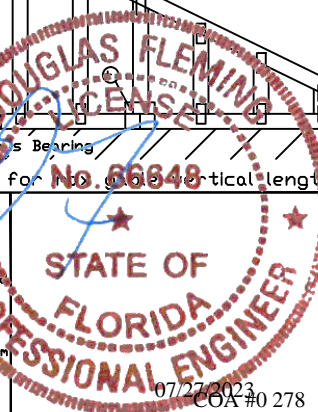
Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4
+ Refer to common truss design for peak, splice, and heel plates.	

Refer to the Building Designer for conditions not addressed by this detail.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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 For more information see this job's general notes page and these web sites:
 ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-16-GAB14030

DATE 01/26/2018

DRWG A14030ENC160118

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

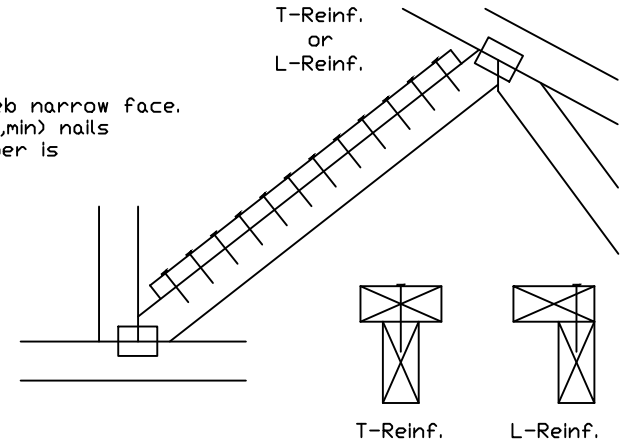
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(✕)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(✕)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(✕) Center scab on wide face of web. Apply (1) scab to each face of web.

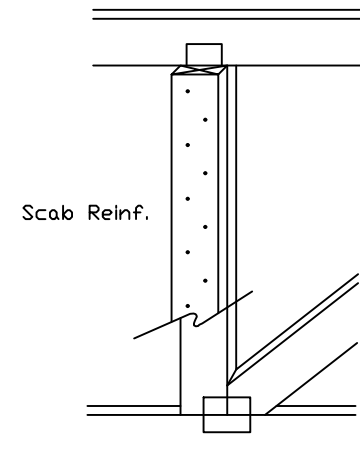
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

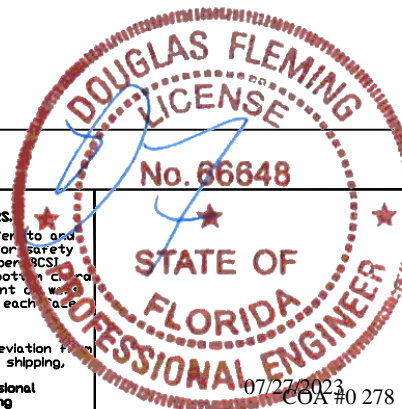
WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

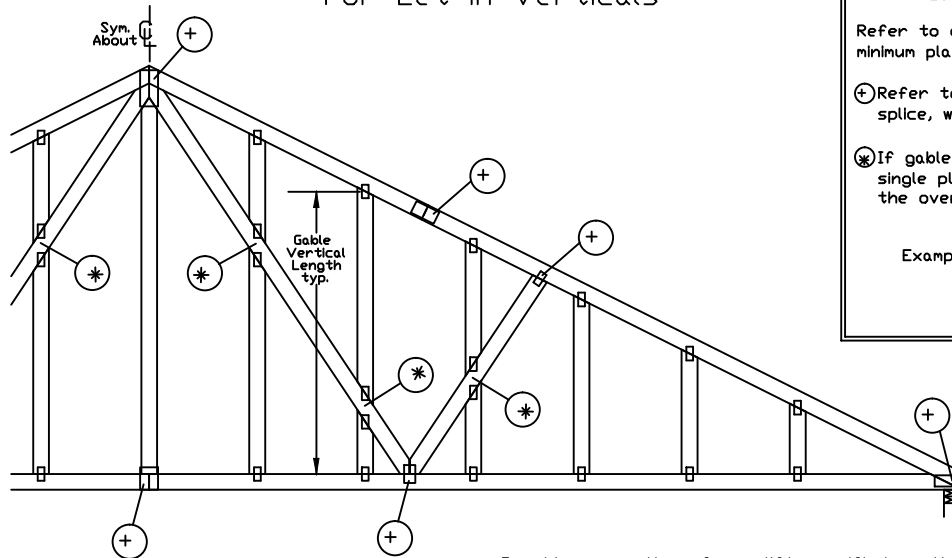
For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



TC LL	PSF	REF	CLR Subst.
TC DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

07/27/2023
COA #0278

Gable Detail For Let-in Verticals



Gable Truss Plate Sizes

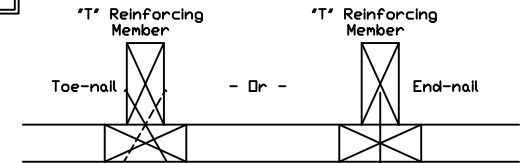
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example: 2X4 2X4

'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (From Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148"x3",min) Nails at 4' o.c. plus

(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x3",min) Toenails at 4' o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

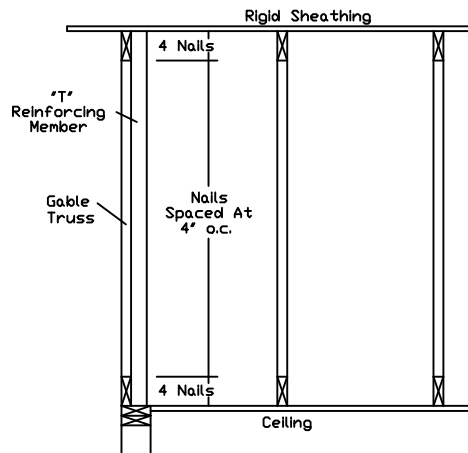
ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLER

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of trusses shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each pair of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

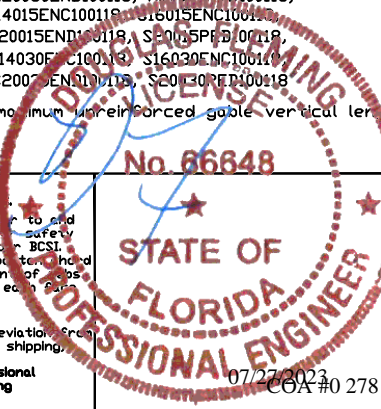
A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this Job's general notes page and these web sites:

ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

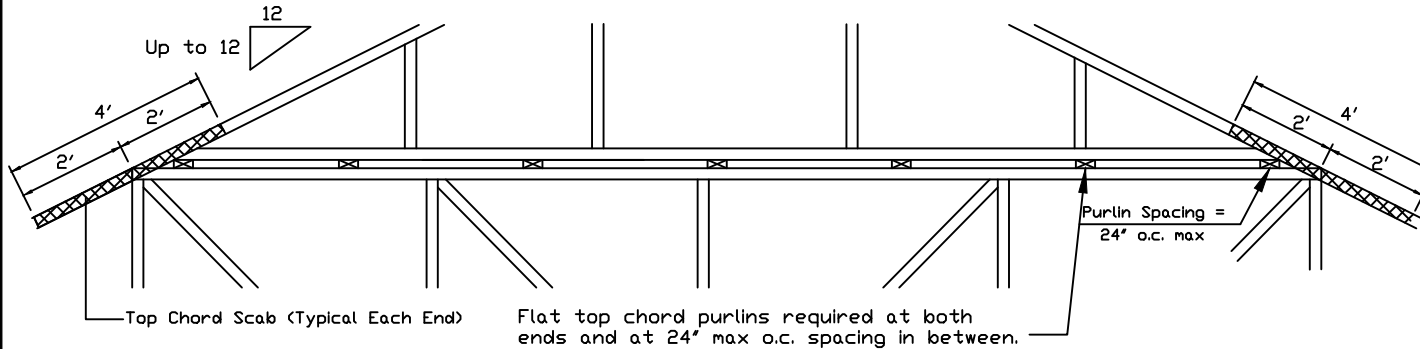
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

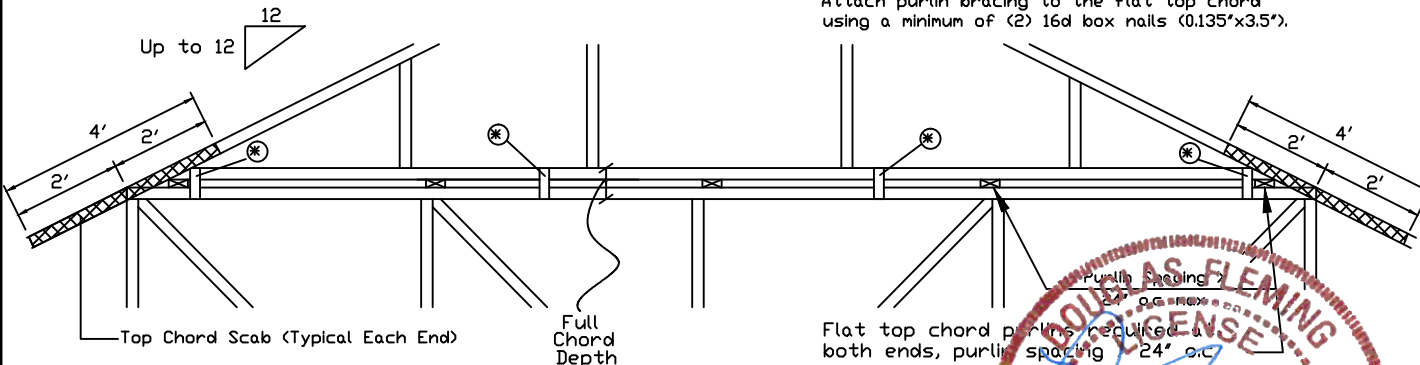


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

* In addition, provide connection with one of the following methods:

Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.
2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.
28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

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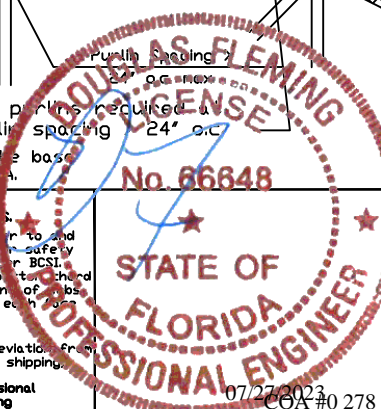
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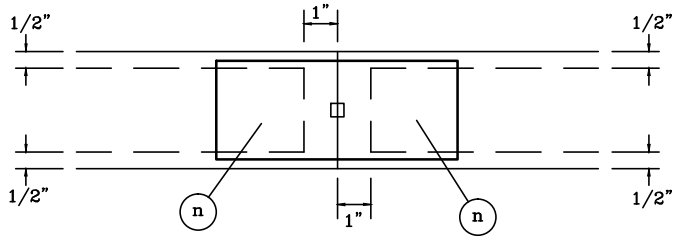


REF PIGGYBACK
DATE 01/02/2018
DRWG PB160160118

SPACING 24.0"

TRULOX INFORMATION DETAIL

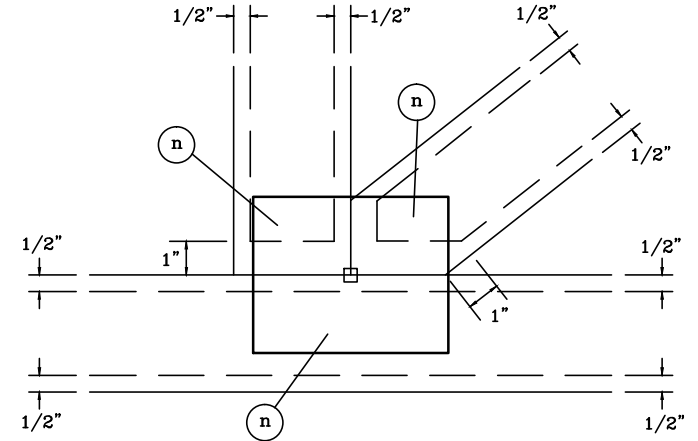
TYPICAL OFF PANEL SPLICE



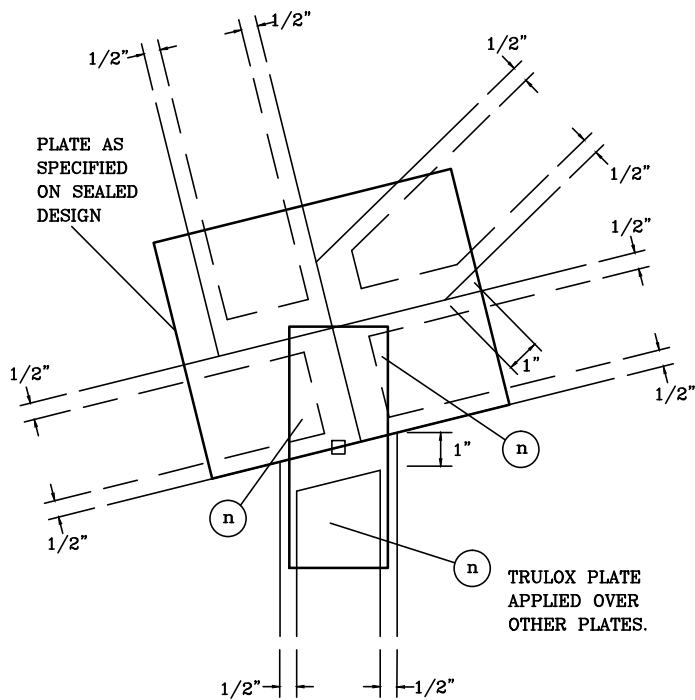
DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LUMBER.

TYPICAL PANEL POINT WITHOUT SPLICE

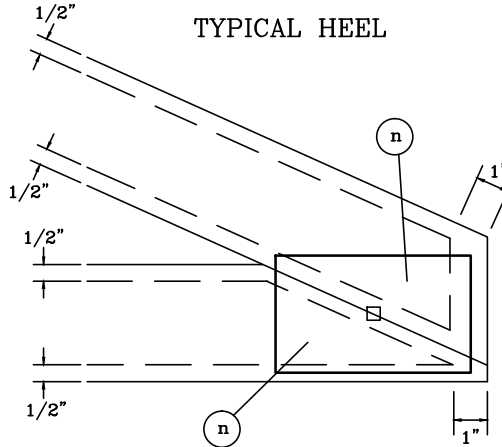


TYPICAL FILLER

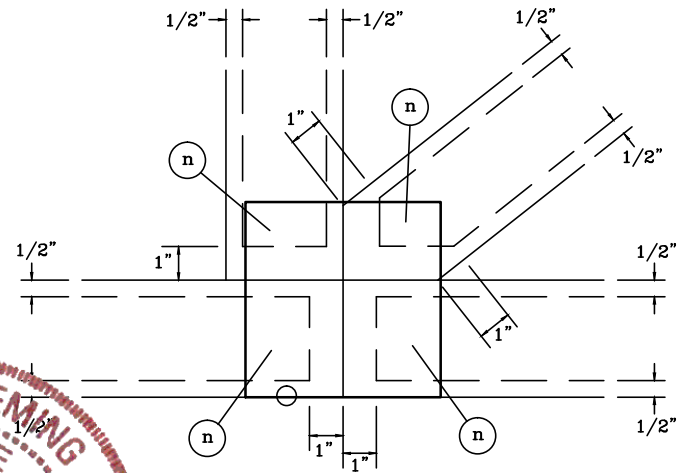


TRULOX PLATE APPLIED OVER OTHER PLATES.

TYPICAL HEEL



TYPICAL PANEL POINT SPLICE

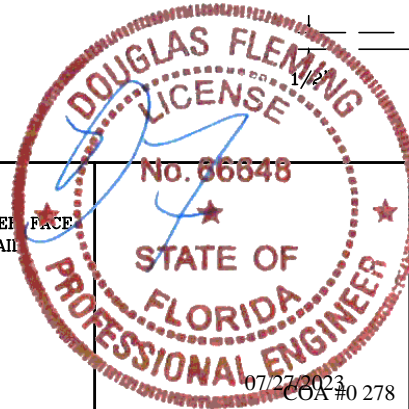


NOTES:

- (n) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.
- LOCATES PLATE CORNER OR FLUSH EDGE.
- LOCATES PLATE CENTER.



155 Harlem Ave
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07/27/2023
COX #0 278

TRULOX PLATING

160
TL

PAGE 1 OF 1

DATE 10/01/14