

GENERAL NOTES

1. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ENGINEER OF DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH THE WORK.
2. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE COMPLETE DESIGN OF THE STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN, OR OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE BUT NOT BE LIMITED TO BRACING, SHORING FOR EARTH BANKS, FORMS, SCAFFOLDING, PLANKING, SAFETY NETS, SUPPORT AND BRACING FOR CRANES AND GIN POLES.
3. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL MEMBERS AS REQUIRED FOR STRUCTURAL STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE STRUCTURAL ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.
4. CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON ROOFS IN EXCESS OF THE POSTED ROOF LIVE LOAD. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO INSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON ROOFS.
5. PLANS, SECTIONS AND DETAILS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.
6. SUBMIT WRITTEN REQUESTS TO THE ENGINEER FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. SPLICING, CUTTING, NOTCHING OR OTHER ALTERATIONS TO STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE STRUCTURAL ENGINEER. ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS, AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR.
7. THE ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK PROVIDED BY THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
8. PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF NORTH FLORIDA PROFESSIONAL SERVICES, INC. IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE OWNER AGAINST DEFECTS OR DEFICIENCIES IN THE WORK OF THE CONTRACTOR.

DESIGN CRITERIA

1. THE DESIGN IS BASED ON, AND ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE (FBC) WITH AMENDMENTS AND DESIGN CODES REFERENCED WITHIN THESE DOCUMENTS. USE THE REFERENCED EDITIONS FROM THE FBC CHAPTER 35 OR THE LATEST EDITIONS IF NOT REFERENCED.
- AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7-22
"MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES"
STRUCTURAL CONCRETE
"BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
THE AMERICAN CONCRETE INSTITUTE (ACI 318-19 AND ACI 350-06)
MASONRY
"BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
THE MASONRY SOCIETY (TMS 402/602-16)
STRUCTURAL STEEL
STEEL CONSTRUCTION MANUAL - FIFTEENTH EDITION BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 360-16)
WOOD
"NATIONAL DESIGN SPECIFICATION" AND SUPPLEMENT (ANS/AWC NDS-18)
2. LIVE LOADS:
ROOF 20 PSF (REDUCIBLE BY CODE)
3. SUPERIMPOSED DEAD LOADS:
ROOFING / CEILING / M.E.P. 5 PSF
4. WIND LOAD DESIGN DATA:
WIND LOADS SHALL BE IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE (REFERENCING ASCE 7-22).
MAIN WIND FORCE RESISTING SYSTEM
WIND DESIGN DATA:
a. ULTIMATE DESIGN WIND SPEED, 3 SECOND GUSTS, VULT. 120 MPH
b. HURRICANE PRONE REGION YES
c. WINDBORNE DEBRIS REGION NO
d. BUILDING RISK CATEGORY II
e. WIND EXPOSURE CATEGORY C
f. WIND TOPOGRAPHIC FACTOR (KZT) 1.0
g. ENCLOSURE CATEGORY PARTIALLY OPEN
h. INTERNAL PRESSURE COEFFICIENT ±0.18
i. MEAN ROOF HEIGHT 15 FEET
j. WIND DIRECTIONALLY FACTOR, KD 0.85
k. VELOCITY PRESSURE COEFFICIENT (KH) 0.85
l. ULTIMATE VELOCITY PRESSURE (QH(ULT)) 31.3 PSF
m. COMPONENT & CLADDING WIND PRESSURES SEE TABLE THIS SHEET
n. DIMENSION "a" 3'-0"
o. GROUND ELEVATION FACTOR, KE 1.0
5. 60-MINUTE RAINFALL INTENSITY 4.5 INCHES PER HOUR.
6. DISTRIBUTE THE MAXIMUM LOAD HUNG FROM ANY STRUCTURAL MEMBERS FOR MEP DUCTWORK, PIPING ETC OVER THE MEMBER'S TRIBUTARY AREA IN A WAY THAT THE DESIGN SUPERIMPOSED DEAD LOADS LISTED IN CONTRACT DOCUMENTS ARE NOT EXCEEDED. THE CONTRACTOR SHALL COORDINATE THE LOADS OF ALL TRADES AND PROVIDE ADDITIONAL SUPPORT OR DISTRIBUTION FRAMING AS REQUIRED TO ACHIEVE THE ALLOWABLE LOAD DISTRIBUTION.
7. STRUCTURAL COMPONENTS ARE NOT DESIGNED FOR VIBRATING EQUIPMENT. MOUNT VIBRATING EQUIPMENT ON VIBRATION ISOLATORS.

FOUNDATIONS

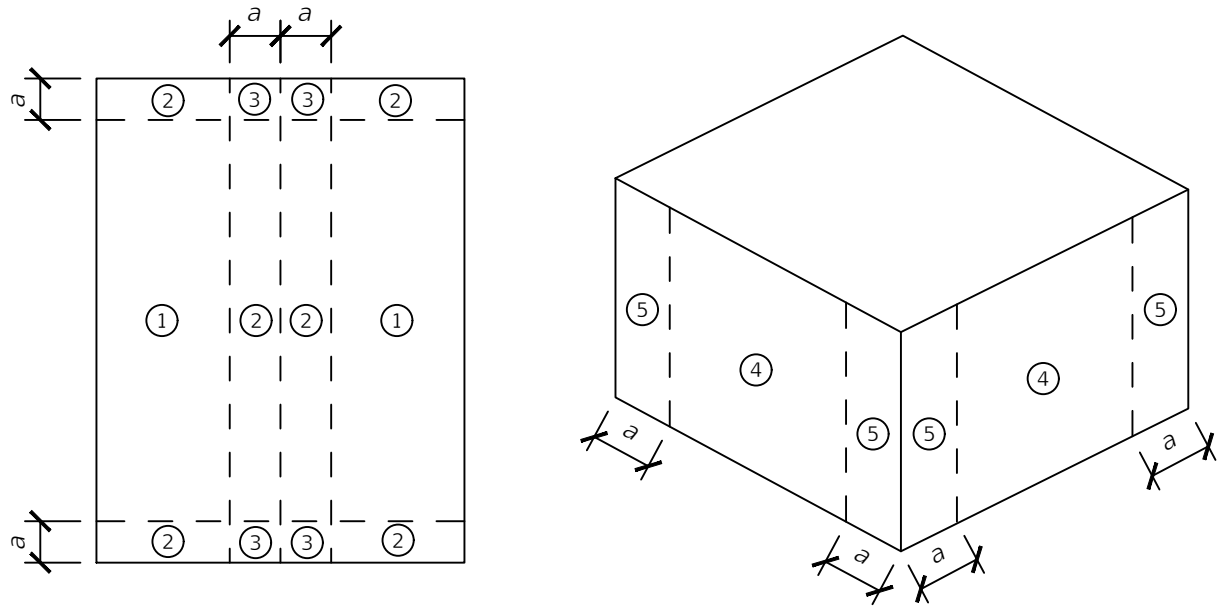
1. BEARING SOILS SHALL BE FREE OF ORGANIC MATERIAL AND MEET THE FBC REQUIREMENTS TO PROVIDE A MINIMUM OF 1,500 PSF SOIL BEARING DESIGN PRESSURES. PER TABLE RA01.4.1 OF THE FLORIDA RESIDENTIAL BUILDING CODE. IT IS THE HOMEOWNERS RESPONSIBILITY TO VERIFY THAT THE SOIL CONDITIONS ARE SUITABLE FOR THESE ASSUMPTIONS. IT IS SUGGESTED THAT PRIOR TO CONSTRUCTION A GEOTECHNICAL INVESTIGATION BE MADE TO VERIFY THE BEARING PRESSURE AND SUBSURFACE CONDITIONS. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT FROM THOSE ASSUMED FOR DESIGN.

WOOD

1. STRUCTURAL FRAMING PLANS DEPICT THE PRIMARY STRUCTURAL FRAMING SYSTEM. CONTRACTOR SHALL PROVIDE SECONDARY AND MISCELLANEOUS FRAMING AS REQUIRED TO COMPLETE THE PROJECT.
2. DRESSED SEASONED LUMBER: S4S, 19% MAXIMUM MOISTURE CONTENT AT TIME OF DRESSING.
- 2.1. COLUMNS AND STUD FRAMING: SOUTHERN PINE NO.2 OR STRONGER.
- 2.2. LINTELS, FLOOR JOISTS AND BEAMS: SOUTHERN PINE, NO.2 GRADE
- 2.3. WOOD IN CONTACT WITH CONCRETE OR MASONRY OR EXPOSED TO WEATHER: ABOVE GRADE PRESSURE-TREATED (AWPA-UC3A OR UC3B) OR GROUND CONTACT RATED PRESSURE TREATED (AWPA-UC4A). GROUND CONTACT RATED WOOD IS RECOMMENDED AT THE CRAWLSPACE AND DECK AREAS (IF PRESENT). USE HOT-DIP GALVANIZED NAILS IN PRESSURE TREATED WOOD.
3. STRUCTURAL PANELS
- 3.1. WALL PANELS: 1/2" APA RATED SHEATHING.
- 3.2. ROOF PANELS: 1/2" APA RATED SHEATHING.
4. WOOD SHEAR WALLS
- 4.1. PANELS SHALL BE ORIENTED WITH THE LONG DIMENSION IN THE VERTICAL DIRECTION.
- 4.2. SOLID 2x BLOCKING SHALL BE PROVIDED AT UNSUPPORTED, HORIZONTAL PANEL EDGES.
- 4.3. NAIL PANELS WITH 8d HOT-DIPPED GALVANIZED RINGSHANK NAILS SPACED AT 6" AT THE PERIMETER OF THE PANELS AND AT 12" AT INTERMEDIATE SUPPORTS. UON.
- 4.4. DOUBLE 2x FRAMING STUDS SHALL BE USED AT THE ENDS OF EACH SHEAR WALL. UON.
- 4.5. CONNECTIONS FOR STRUCTURAL TIMBER: GALVANIZED STRONG-TIE CONNECTORS BY THE SIMPSON STRONG TIE COMPANY OR APPROVED EQUAL.
5. LAMINATED VENEER LUMBER (LVL) SHALL BE WEYERHAUSER/TRUS JOIST MICROLAM LVL (OR EQUAL) WITH FB NOT LESS THAN 2,600 PSI AND MINIMUM 2.0E.
6. BOLTED CONNECTIONS SHALL CONSIST OF ASTM A307 BOLTS, FASTENED TO A SNUG-TIGHT CONDITION.

WIND PRESSURE DIAGRAM

1. DESIGN WIND PRESSURES TO BE USED IN THE DESIGN OF ALL COMPONENTS AND CLADDING ELEMENTS. PRESSURES INDICATED IN TABLE ARE SERVICE LOADS. MULTIPLY TABULATED VALUES BY 0.6 FOR ALLOWABLE STRESS DESIGN (ASD) AND BY 1.0 FOR LOAD AND RESISTANCE FACTOR DESIGN (LRFD).
2. REFER TO WIND PRESSURE DIAGRAM FOR ZONE LOCATIONS AND EXTENTS.
3. POSITIVE PRESSURES ACT TOWARD COMPONENT SURFACES AND NEGATIVE PRESSURES ACT AWAY FROM COMPONENT SURFACES.



COMPONENTS AND CLADDING WIND PRESSURES ON ROOF AND WALLS (PSF)								
ZONE	1, 2, 3	1	2	3	4	5		
TRIB AREA	(+)	(-)	(-)	(-)	(+)	(-)	(+)	(-)
10	21	-58	-77	-101	31	-34	31	-42
20	19	-50	-67	-87	30	-33	30	-39
50	17	-39	-53	-68	29	-31	29	-36
100	15	-31	-43	-53	27	-30	27	-33
200	13	-18	-31	-53	25	-28	25	-30
500	13	-18	-31	-53	23	-26	23	-26

WINDOWS, DOORS, AND ROOFING

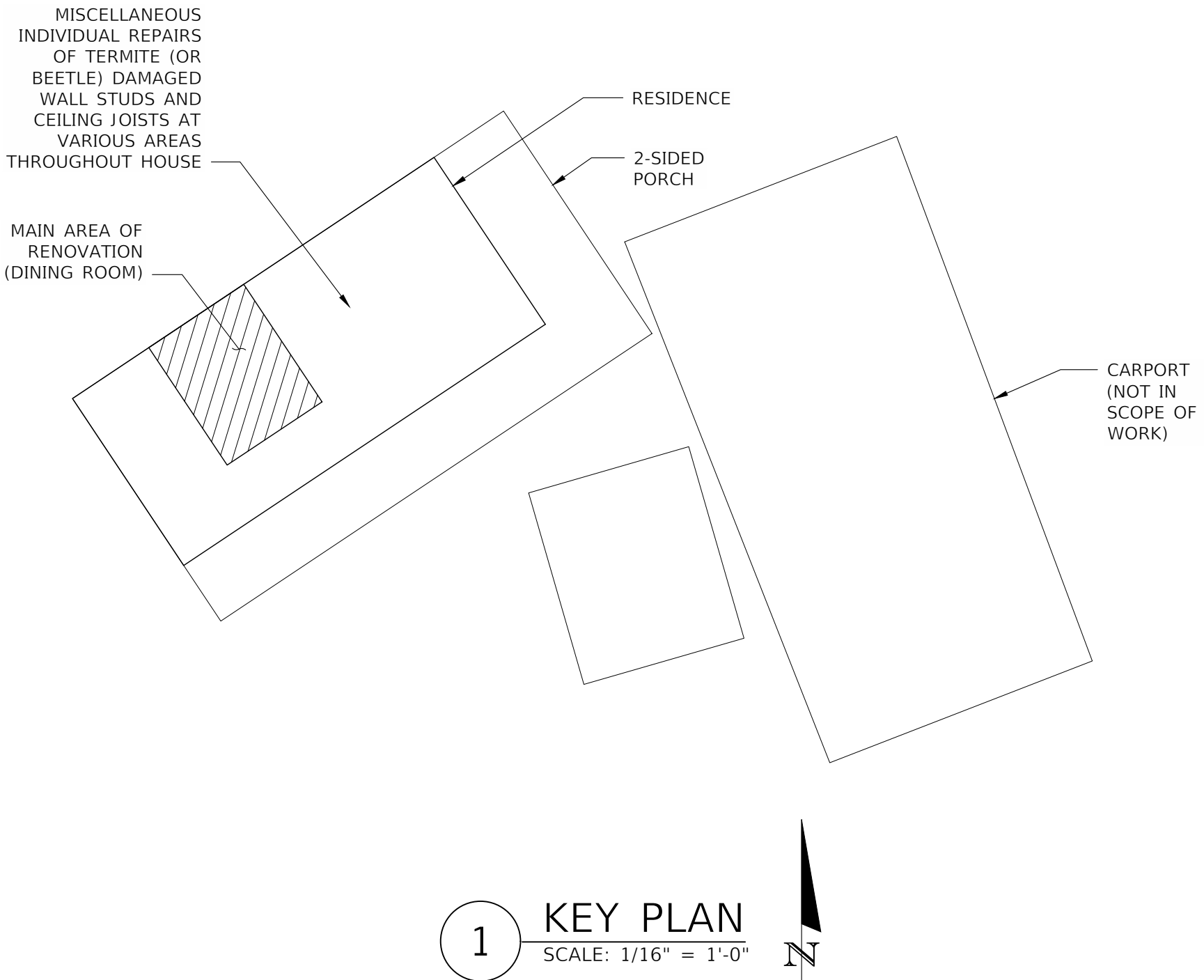
1. FOR THE SELECTION OF WINDOW, DOOR, AND ROOFING PRODUCTS, TABULATED VALUES ARE NORMALLY MULTIPLIED BY 0.6 PRIOR TO COMPARISON WITH THE POSITIVE AND NEGATIVE PRESSURE RATINGS PROVIDED IN EACH FLORIDA PRODUCT APPROVAL. IT IS RECOMMENDED THAT THE MANUFACTURER'S REPRESENTATIVE REVIEW THESE DRAWINGS FOR VERIFICATION. THE TRIBUTARY AREA FOR ROOFING PRODUCTS IS TYPICALLY BASED ON 10 SQUARE FEET, AND FOR DOORS AND WINDOWS IT IS BASED ON THE SURFACE AREA OF THE WALL OPENINGS.

ROOF OVERHANG PRESSURES
(WHERE NOT TABULATED ABOVE)

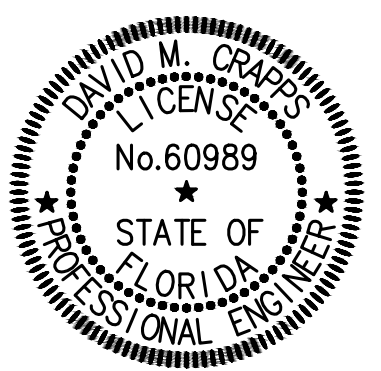
1. ROOF OVERHANG PRESSURES ARE DETERMINED BY SUMMING THE ABSOLUTE VALUE OF THE NEGATIVE ROOF ZONE (1, 2, OR 3) PRESSURE AND THE POSITIVE WALL ZONE (4 OR 5) PRESSURE, BASED ON THE APPLICABLE TRIBUTARY AREA. THE SUM IS THEN MULTIPLIED BY .1 TO GET THE CORRESPONDING ROOF OVERHANG UPLIFT PRESSURE.

SYMBOLS AND ABBREV.

ALT	ALTERNATE/ALTERNATIVE	K	KIP = 1000 LB
ACI	AMERICAN CONCRETE INSTITUTE	KO	KNOCK OUT
AFF	ABOVE FINISHED FLOOR		
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LGTH	LENTGH
ASIS	AMERICAN IRON AND STEEL INSTITUTE	LLH	LONG LEG HORIZONTAL
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	LLV	LONG LEG VERTICAL
AWS	AMERICAN WELDING SOCIETY	LONG	LONGITUDINAL
AB	ANCHOR BOLTS		
ARCH	ARCHITECTURE/ARCHITECTURAL	MANUF	MANUFACTURE/MANUFACTURER
ASTM	AMERICAN SOCIETY OF TESTING MATERIALS	M B	MASONRY BEAM
AWS	AMERICAN WELDING SOCIETY	MATL	MATERIAL
		MAX	MAXIMUM
BB	BOND BEAM	MECH	MECHANICAL
BFB	BOTTOM FLANGE BRACE	MEZZ	MEZZANINE
BP	BASE PLATE/BEARING PLATE	MIN	MINIMUM
BRG	BEARING	MISC	MISCELLANEOUS
BM	BEAR	MO	MASONRY OPENING
BLK	BLOCK	MTL	METAL
B/	BOTTOM OF		
BLDG	BUILDING	N I C	NOT IN CONTRACT
		NOM	NOMINAL
CANT	CANTILEVER	N T S	NOT TO SCALE
CL	CENTERLINE	N W T	NORMAL WEIGH TOPPING
CLR	CLEAR/CLEARANCE		
COL	COLUMN	O C	ON CENTER
CB	CONCRETE BEAM	OPNG	OPENING
CC	CONCRETE COLUMN	OPP	OPPOSITE
CMU	CONCRETE MASONRY UNIT		
CONT	CONTINUOUS	PAF	POWER ACTUATED FASTENER
CONN	CONNECTION	PL	PLATE
CONST	CONSTRUCTION	PLY	PLYWOOD
CJ	CONSTRUCTION JOINT	PSF	POUNDS PER SQUARE FOOT
	CONTRACTION JOINT / CONTROL JOINT	PSI	POUNDS PER SQUARE INCH
DET	DETAIL	PC	PRECAST CONCRETE
DEPT	DEPARTMENT	PRE-ENG	PRE-ENGINEERED
DBA	DEFORMED BAR ANCHOR	PREFAB	PREFABRICATED
DFT	DRY FILM THICKNESS	PROJ	PROJECTION
DIA	DIAMETER	PT	PRESSURE TREATED
DIM	DIMENSION	PW	PANEL WIDTH
DIST	DISTANCE		
DN	DOWN	REF	REFERENCE
DR	DRAIN	REINF	REINFORCING
DWG	DRAWING	R C P	REINFORCED CONCRETE PIPE
		REQD	REQUIRED
EA	EACH	R W	RETAINING WALL
EE	EACH END	R D	ROOF DRAIN
EF	EACH FACE		
EJ	EXPANSION JOINT	SCHED	SCHEDULE
EL	ELEVATION	SIM	SIMILAR
EMB	EMBEDMENT	SPC	SPACE/SPACES
ENGR	ENGINEER	SPECS	SPECIFICATIONS
EOS	EDGE OF SLAB	SPP	SPRUCE PINE FUR
EQ	EQUAL	SQ	SQUARE
ES	EACH SIDE	S A	STUD ANCHOR
EW	EACH WAY	S S	STAINLESS STEEL
EXIST	EXISTING	STD	STANDARD
EXP	EXPANSION	STL	STEEL
EXT	EXTERIOR	STRUC	STRUCTURAL
		SYM	SYMMETRICAL
FBC	FLORIDA BUILDING CODE	SP	STEPPED FOOTING
FL	FULL LENGTH WELD, WELD ENTIRE DIST.	SYP	SOUTHERN YELLOW PINE
F V	FIELD VERIFY		
F F	FINISHED FLOOR	THK	THICK
FLR	FLOOR	THD	THREAD/THREADED
F D	FLOOR DRAIN	T B	TIE BEAM
FTG	FOOTING	T & B	TOP AND BOTTOM
		T & G	TONGUE AND GROOVE
GA	GAGE/GAUGE	T.O.C.	TOP OF CONCRETE
GALV	GALVANIZED	T.O.S.	TOP OF STEEL
GC	GENERAL CONTRACTOR	TRANS	TRANSVERSE
GLU-LAM	GLUE LAMINATED	TYP	TYPICAL
		T/	TOP OF
HAS	HEADED ANCHOR STUD	UNO	UNLESS NOTED OTHERWISE
HC	HOLLOW CORE		
HK	HOOK	VERT	VERTICAL
HORIZ	HORIZONTAL	VOL	VOLUME
HP	HIGH POINT		
HSS	HOLLOW STRUCTURAL SECTION	WF	WALL FOOTING
HT	HEIGHT	WPF	WATERPROOF
		WWF	WELDED WIRE FABRIC
ID	INSIDE DIAMETER	WH	WEEP HOLE
IF	INSIDE FACE	WT	WEIGHT
INT	INTERIOR	WF	WIDE FLANGE
		W /	WITH
JT	JOINT	W / O	WITHOUT
JST	JOIST	WD	WOOD
		WP	WORKING POINT



1 KEY PLAN
SCALE: 1/16" = 1'-0"

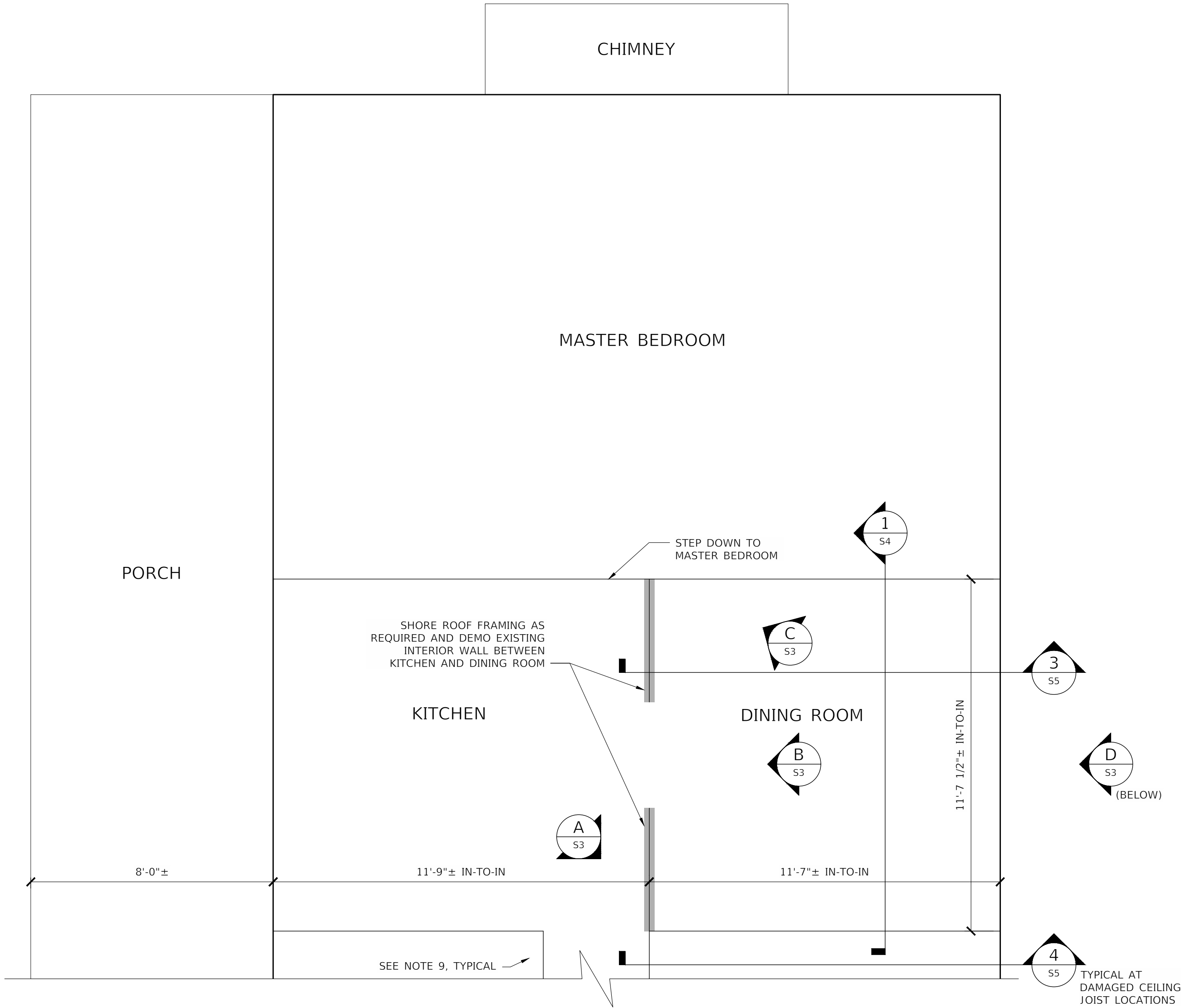


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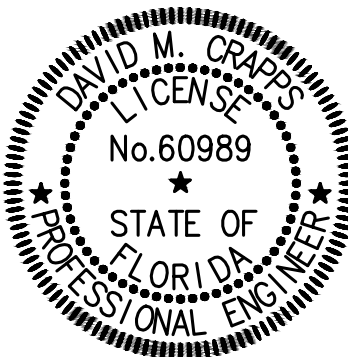
REVISIONS			NORTH FLORIDA PROFESSIONAL SERVICES, INC. P.O. BOX 3823 LAKE CITY, FL 32056 PH. 386-752-4675 LIC NO. LB8356	2551 BLAIRSTONE PINES DR. TALLAHASSEE, FL 32301 WWW.NFPS.NET CA# 29011	JOB NUMBER: L241211KOT EOR: DAVID M. CRAPPS P.E. NO.: 60989	STRUCTURAL GENERAL NOTES KOTILA RESIDENCE - RENOVATION 660 NE HARRINGTON COURT LAKE CITY, COLUMBIA COUNTY, FLORIDA	SHEET NO. S1
DATE	DESCRIPTION						

PARTIAL FLOOR PLAN NOTES

1. LOCATE EXISTING UTILITIES PRIOR TO EXCAVATION FOR ANY NEW FOUNDATION PIER (SEE DETAIL 1/54).
2. NEW FOUNDATIONS SHALL BE ALLOWED TO SETTLE PRIOR TO COMMENCEMENT OF WOOD-FRAMED CONSTRUCTION.
3. MAINTAIN POSITIVE SLOPE FOR FINISHED GRADE AWAY FROM NEW FOUNDATIONS PER CODE.
4. FIELD VERIFY DIMENSIONS AS REQUIRED.
5. PROVIDE TERMITE TREATMENT OF SUB-GRADE.
6. SPIKE MULTIPLE PLY BEAMS TOGETHER WITH 2 ROWS OF 10d GALV. COMMON NAILS @12" O.C. STAGGERED, PER PLY.
7. WALL SHEATHING SHALL BE NOT LESS THAN 1/2" APA RATED PLYWOOD OR OSB WITH 8d GALV x 2-1/2" LONG RINGSHANK NAILS @6" O.C. AT PANEL EDGES AND @12" O.C. IN FIELD OF PANEL. PROVIDE 2x SOLID BLOCKING AT HORIZONTAL PANEL EDGES.
8. AT STUD PACKS, SPIKE STUD PLIES TOGETHER WITH 10d GALV. COMMON NAILS @6" O.C. STAGGERED, PER PLY.
9. CONTRACTOR TO FIELD VERIFY STUDS, CEILING JOISTS, AND RAFTERS IN THE HOUSE FOR ANY SIGNS OF TERMITE / BEETLE DAMAGE. REPAIR DAMAGED MATERIALS PER DETAIL 3/55 SIMILAR FOR RAFTERS AND 455 FOR CEILING JOISTS. AT DAMAGED STUDS, SISTER NEW 2x4 STUD WITH 10d HOT DIPPED GALVANIZED COMMON NAILS @8" O.C. STAGGERED, AND WITH SIMPSON A342Z AT EACH END.

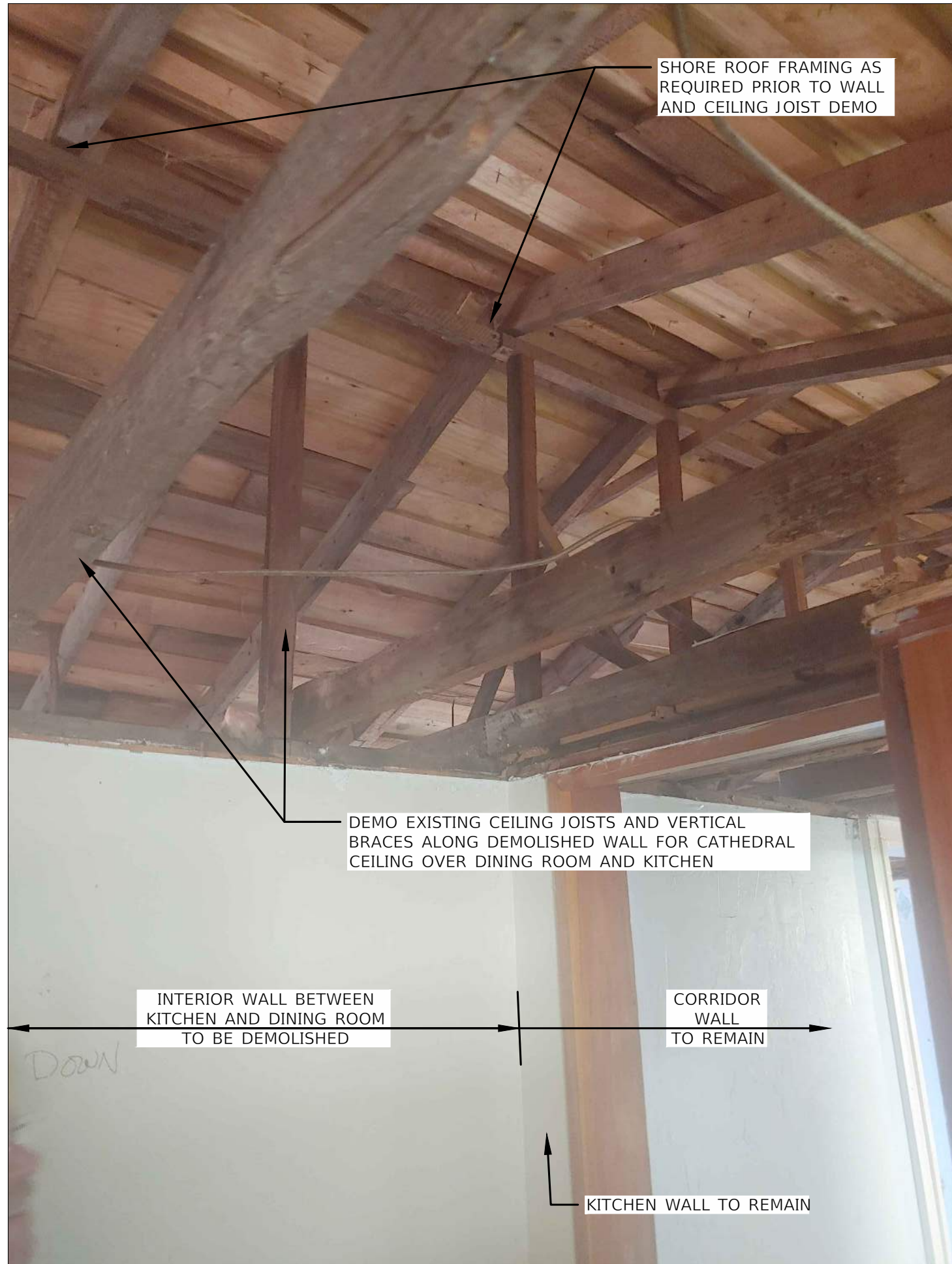


1 PARTIAL FLOOR PLAN
SCALE: 3/16" = 1'-0"



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DATE	DESCRIPTION				
					S2



A KITCHEN AREA REPAIR PHOTO DETAIL
SCALE: N.T.S.



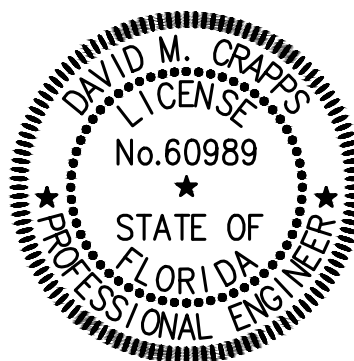
B DINING ROOM AREA REPAIR PHOTO DETAIL
SCALE: N.T.S.



D CRAWLSPACE REPAIR PHOTO DETAIL
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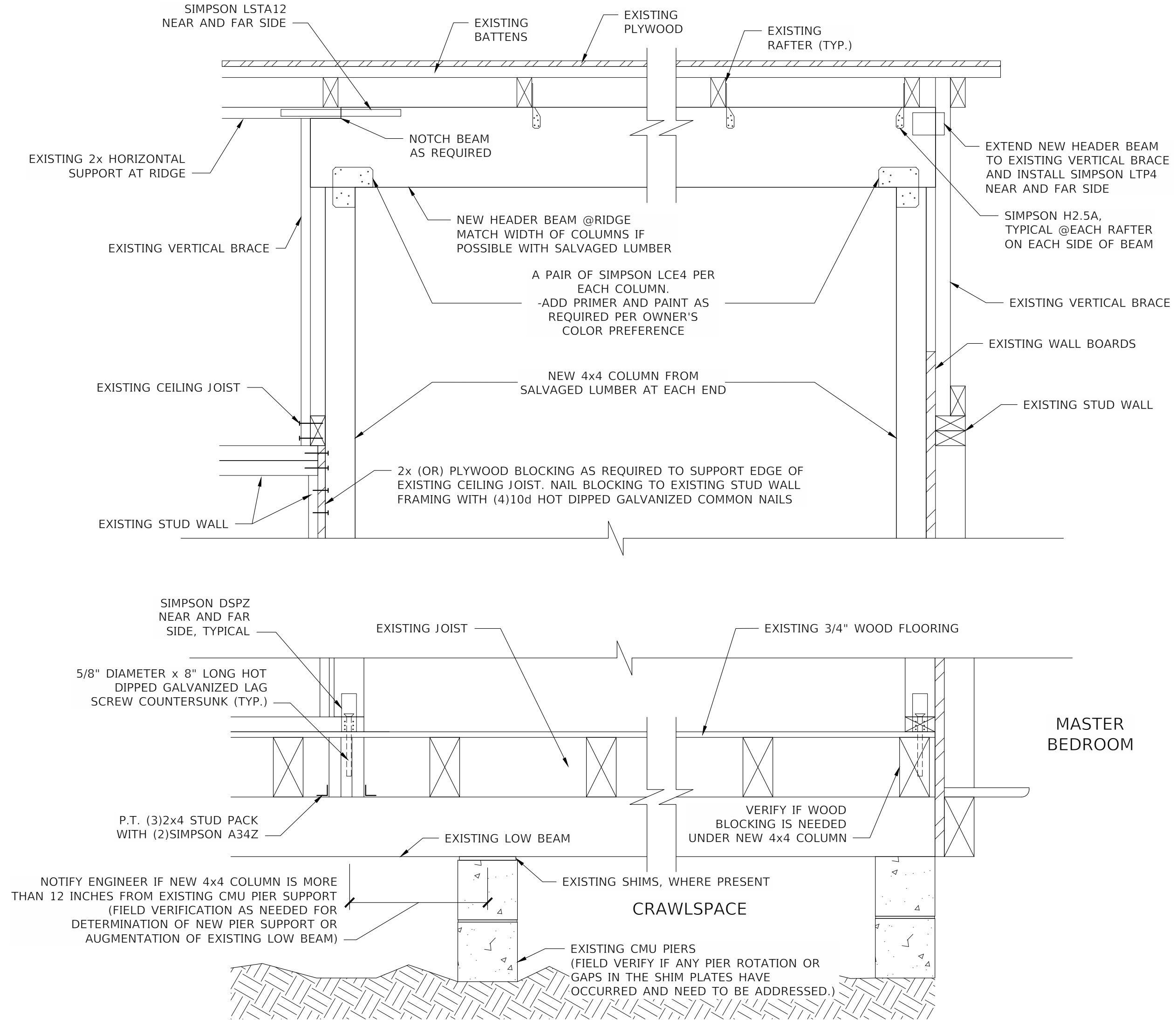


C DINING ROOM AREA REPAIR PHOTO DETAIL
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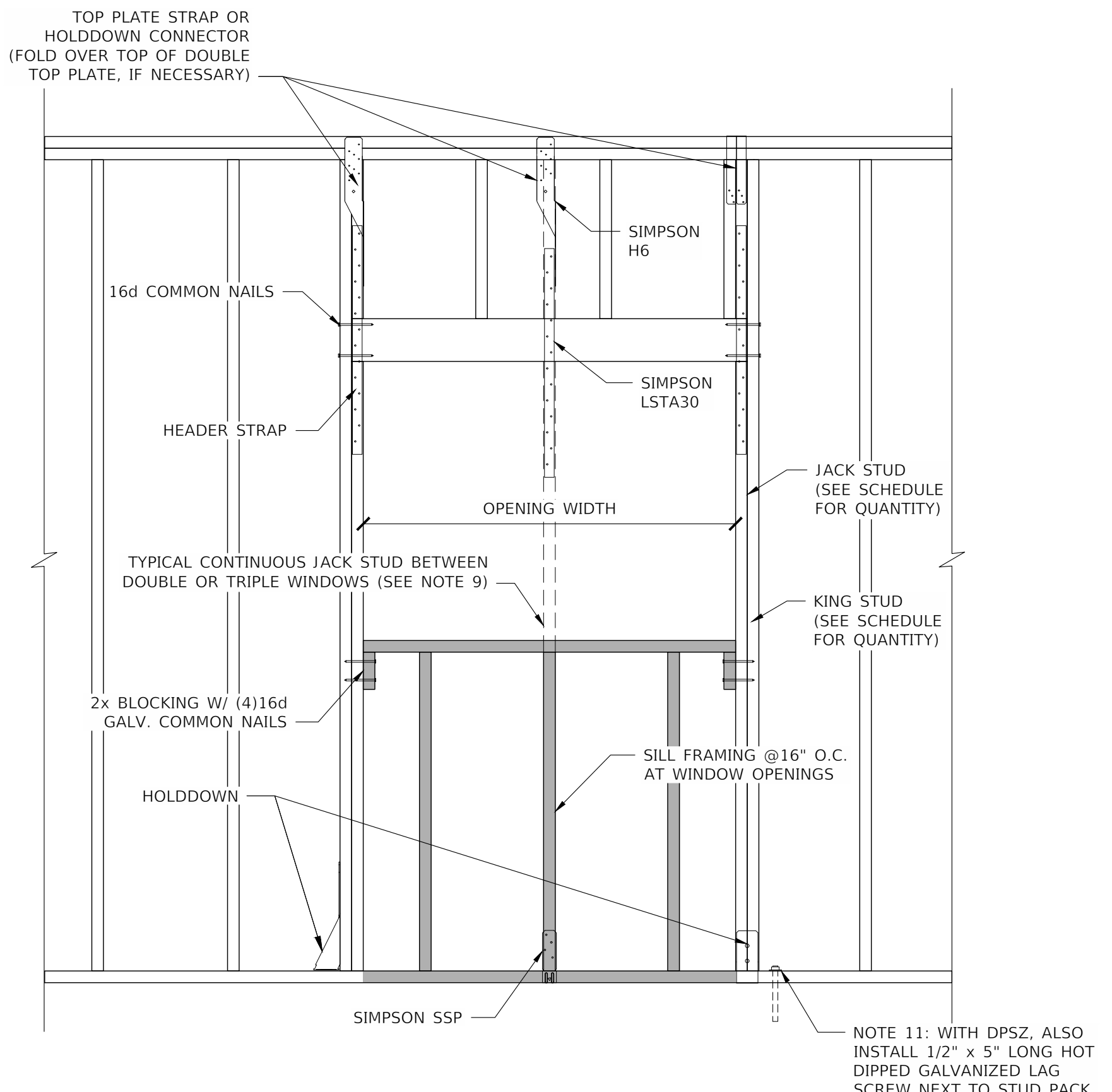


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DATE	DESCRIPTION					



1 ELEVATION AT NEW DINING ROOM/KITCHEN WALL OPENING
SCALE: N.T.S.



HEADER SCHEDULE						
MAXIMUM OPENING WIDTH	HEADER SIZE	JACK STUDS	KING STUDS	SIMPSON STRONG-TIE CONNECTORS		
				TOP PLATE	HEADER	HOLDDOWN
SINGLE 3'-0"	(2)2x6	(1)2x4	(1)2x4	H6	LSTA30	DSPZ (SEE NOTE 11)
TRIPLE 3'-0"	(2)2x10	(1)2x4	(2)2x4	H6	LSTA30	DSPZ (SEE NOTES 9 & 11)

- NOTES
- ALL LUMBER SHALL BE SOUTHERN YELLOW PINE NO.2 OR BETTER.
 - USE PLYWOOD SPACERS BETWEEN HEADER PLIES AS REQUIRED TO MATCH STUD DEPTH.
 - NAIL STUD PACKS TOGETHER WITH 10d GALVANIZED NAILS @6" O.C. STAGGERED
 - INTERIOR NON-LOAD BEARING WALL HEADERS SHALL NOT BE LESS THAN CODE MINIMUMS.
 - USE 8d COMMON GALVANIZED NAILS ON SIMPSON H6, FULLY NAILED.
 - USE 10d COMMON GALVANIZED NAILS ON SIMPSON LSTA30, FULLY NAILED.
 - SIMPSON CS16 MAY BE SUBSTITUTED FOR LSTA30.
 - INSTALL HEADER STRAPS FROM THE INSIDE FACE OF WALL. IF THE TABLE CALLS FOR 2 HEADER STRAPS AT ONE JACK STUD, INSTALL 1 STRAP ON THE INSIDE FACE OF WALL AND 1 ON THE EXTERIOR FACE OF WALL. STAGGER NAIL LOCATIONS AS REQUIRED.
 - WHERE ADJACENT WINDOWS ARE SEPARATED BY A JAMB STUD, ATTACH TOP OF STUD TO CONTINUOUS HEADER WITH SIMPSON LSTA30. ATTACH BOTTOM OF STUD TO BOTTOM PLATE WITH SIMPSON SSP.
 - IF THE FOUNDATION PLAN CALLS FOR A HOLDDOWN AT A WALL OR DOOR JAMB, THE HOLDDOWN ON PLAN GOVERNS OVER THE HOLDDOWN SHOWN IN THE HEADER SCHEDULE.

2 TYPICAL LOAD BEARING HEADER DETAIL
SCALE: N.T.S.

DAVID M. CRAPPS
LICENSE
No.60989
STATE OF FLORIDA
PROFESSIONAL ENGINEER

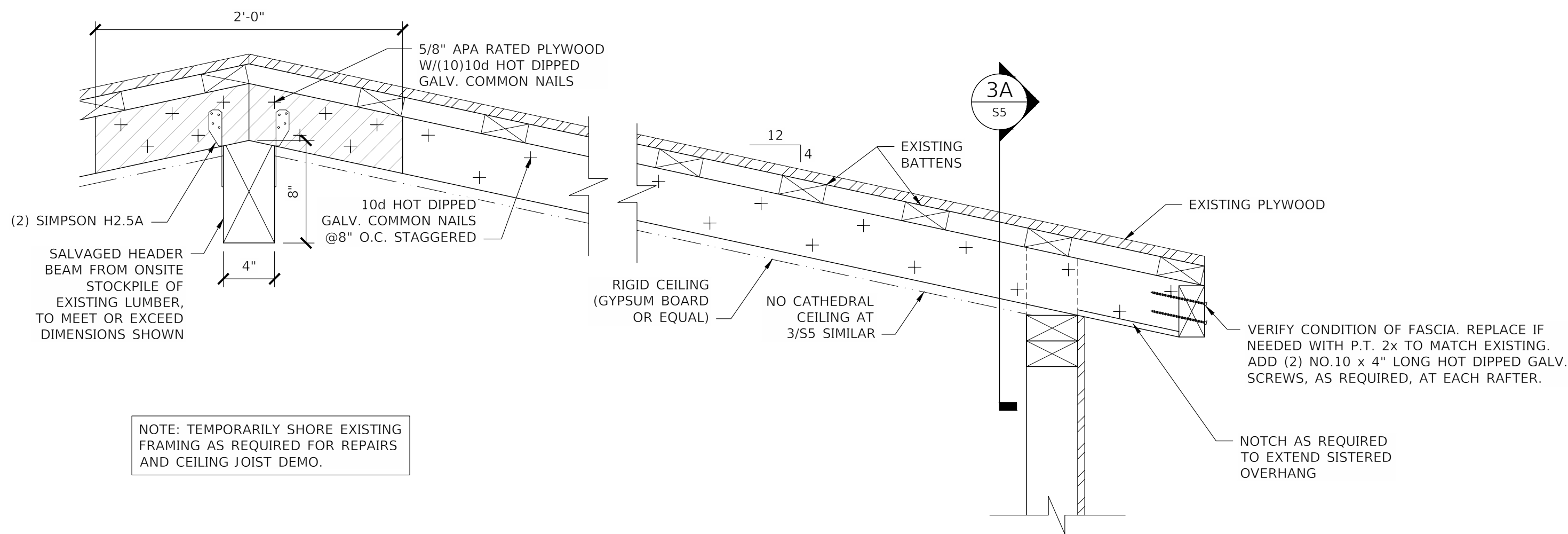
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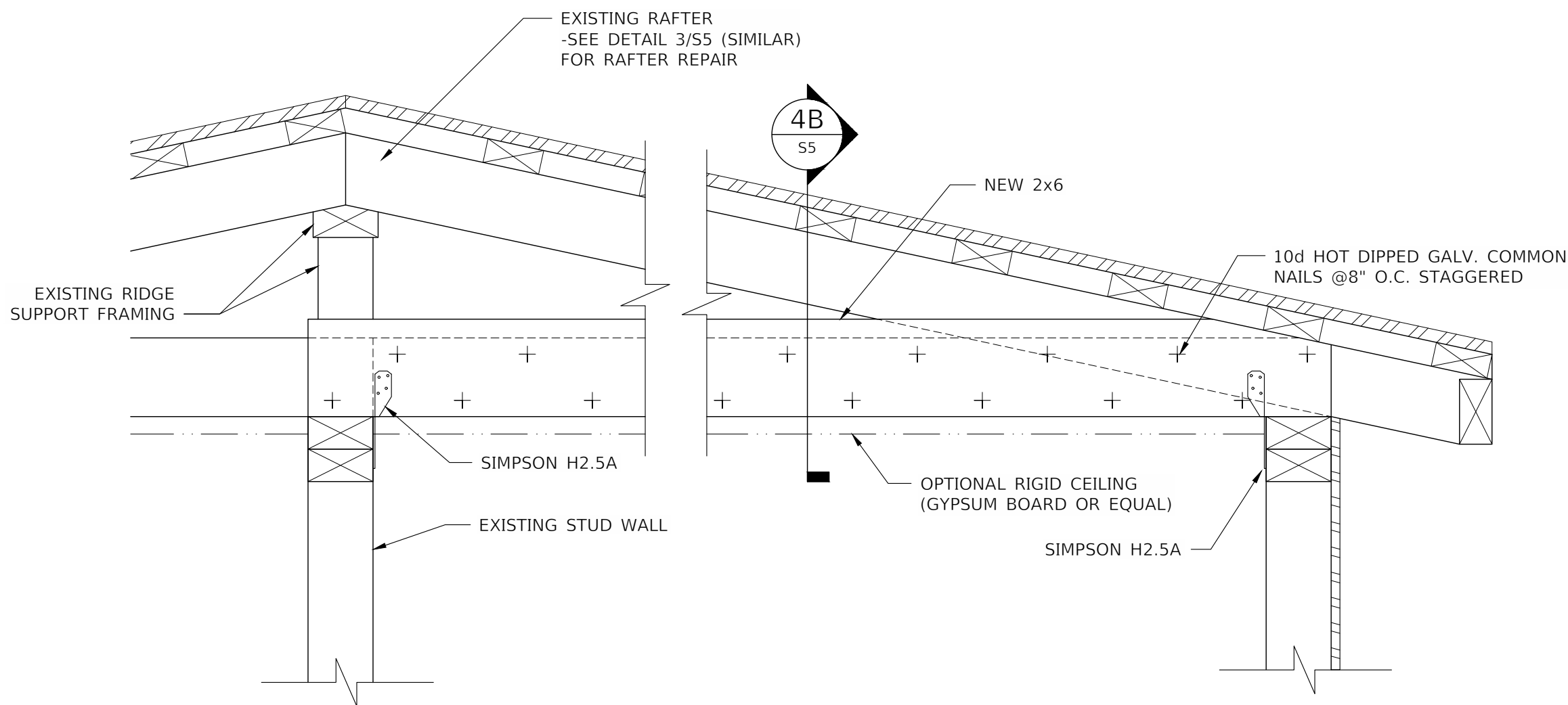
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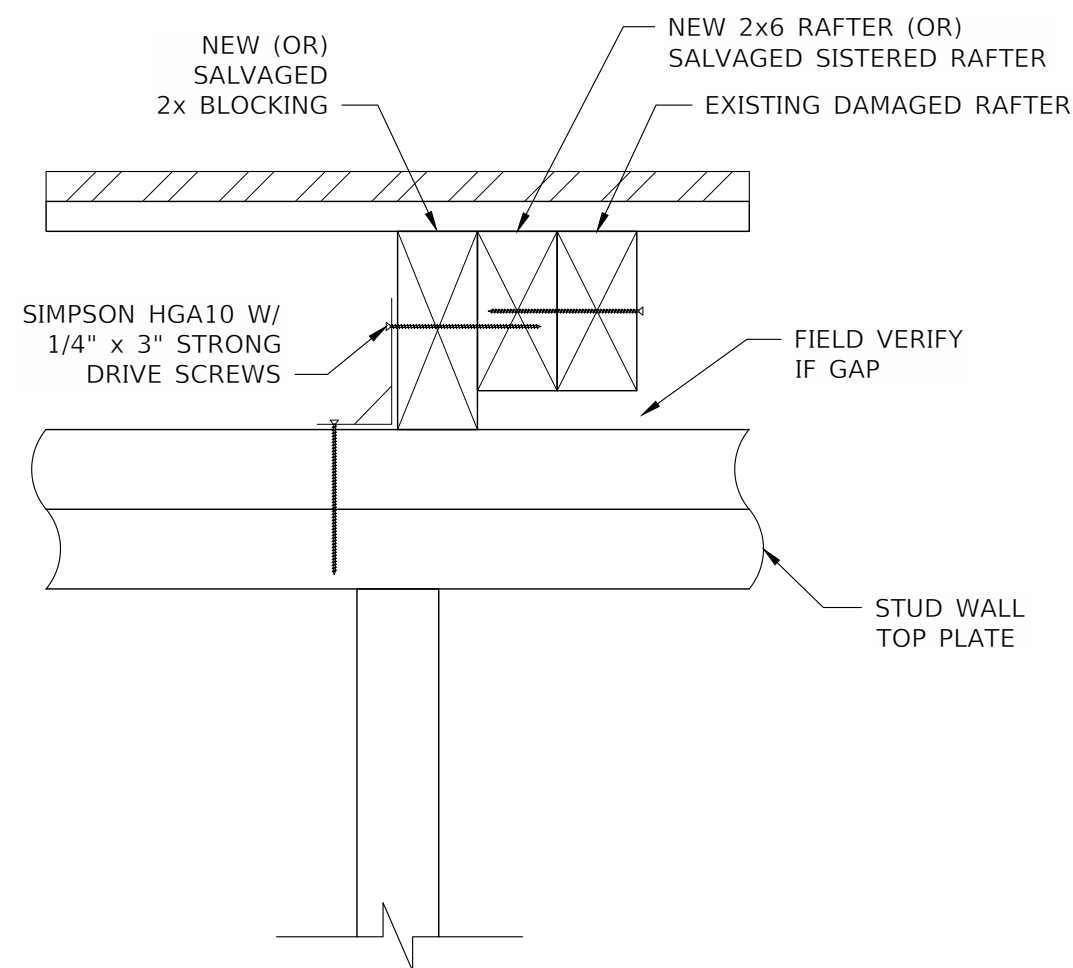
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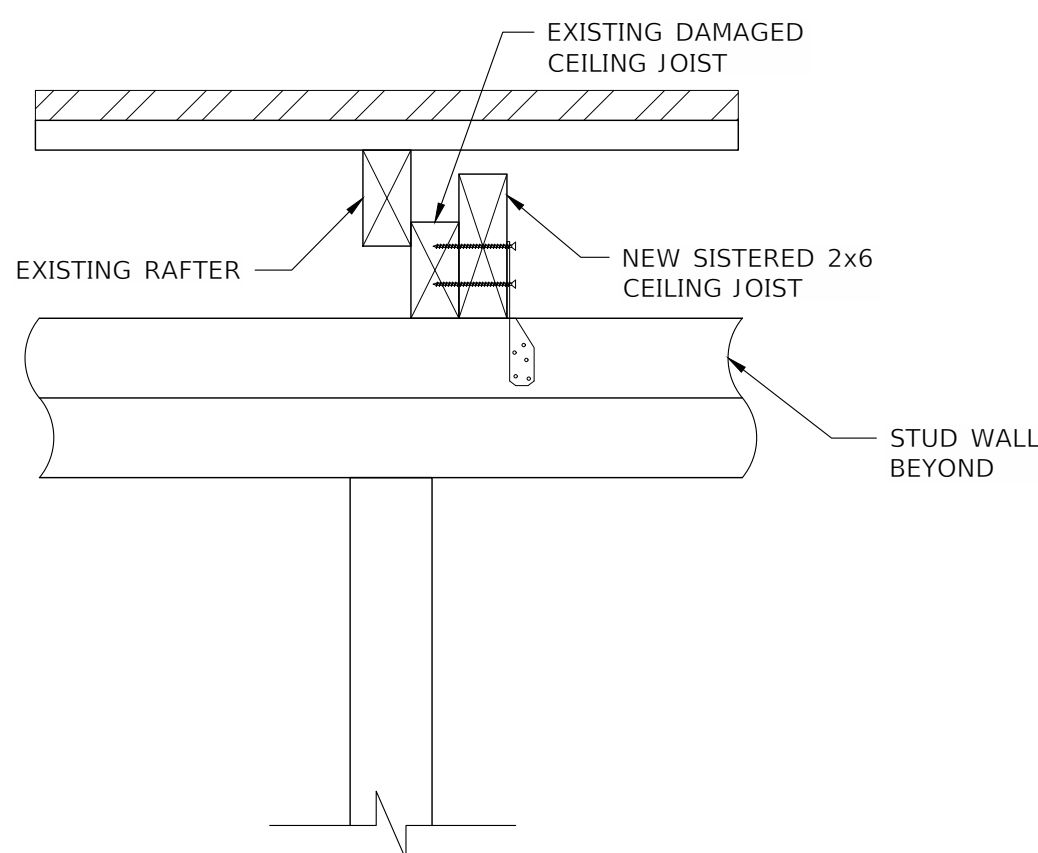
3 ELEVATION AT NEW DINING ROOM/KITCHEN CATHEDRAL CEILING
SCALE: N.T.S.



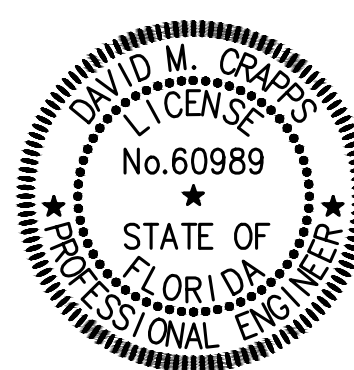
4 TYPICAL CEILING JOIST REPAIR DETAIL
SCALE: N.T.S.



3A SECTION AT DINING ROOM/KITCHEN CATHEDRAL CEILING
SCALE: N.T.S.



4B TYPICAL SECTION AT CEILING JOIST REPAIR
SCALE: N.T.S.



This item has been digitally signed and sealed by
David M Crapps
on the date adjacent to the seal. Printed copies of
this document are not considered signed and
sealed and the signature must be verified on any
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REVISIONS	
DATE	DESCRIPTION



NORTH FLORIDA PROFESSIONAL SERVICES, INC.
P.O. BOX 3823
LAKE CITY, FL 32056
PH. 386-752-4675
LIC NO. LB8356

2551 BLAIRSTONE PINES DR.
TALLAHASSEE, FL 32301
WWW.NFPS.NET
CA# 29011

JOB NUMBER:
L241211KOT
EOR:
DAVID M. CRAPPS
P.E. NO.:
60989

STRUCT. SECTIONS & DETAILS
KOTILA RESIDENCE - RENOVATION
660 NE HARRINGTON COURT
LAKE CITY, COLUMBIA COUNTY, FLORIDA

SHEET
NO.

S5