DATE 05/02/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024457

APPLICANT FRED O. GAYLARD PHONE 386.961.8915

ADDRESS 177 NW BULTHIUS COURT LAKE CITY FL 32055

OWNER ROBIN H. & LISA K.B. ROBERTS PHONE 386.755.5796

ADDRESS 6854 NW LAKE JEFFREY ROAD LAKE CITY FL 32055

CONTRACTOR FRED GAYLARD PHONE 386.961.8915

LOCATION OF PROPERTY LAKE JEFFREY ROAD TO APPROX. 6 1/2 MILES ON THE L, PAST HUNTSVILLE UME CHURCH, NEXT DRIVE AFTER IT.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 92800.00

HEATED FLOOR AREA 1856.00 TOTAL AREA 2492.00 HEIGHT 26.30 STORIES 1

FOUNDATION CONC WALLS B/FRAMED ROOF PITCH 10'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 05-3S-16-01994-000 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 1.80

		CBC1250318		
Culvert Permit No.	Culvert Waiver	Contractor's License Number		Applicant/Owner/Contractor
EXISTING	06-421	BLK	JDK	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE. ONE FOOT ABOVE ROAD. SECTION 2.3.1. OLD SFD WILL BE REPLACED WITH NEW SFD.

Check # or Cash 426

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by | Foundation _____ date/app. by | Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by | Slab _____ date/app. by | Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by | Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by | Heat & Air Duct _____ date/app. by | Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by | C.O. Final _____ date/app. by | Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by | Pool _____ date/app. by

Reconnection _____ date/app. by | Pump pole _____ date/app. by | Utility Pole _____ date/app. by

M/H Pole _____ date/app. by | Travel Trailer _____ date/app. by | Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 465.00 CERTIFICATION FEE $ 12.46 SURCHARGE FEE $ 12.46

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ CULVERT FEE $ **TOTAL FEE** 539.92

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 06-04-88 Date Received 4/26/06 By GT Permit # 24457

Application Approved by - Zoning Official BLK Date 28.04.06 Plans Examiner _____ Date _____

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments SECTION 2.3.1

* OLD SFD WILL BE REPLACED WITH NEW MFD

Applicants Name FRED O. GAYLARD FOG CONTRACTING, INC Phone 386.961.8915

Address 177 NW BUCHULIS CT, LAKE CITY FL 32055

Owners Name ROBIN & LISA K.B. ROBERTS Phone 386.755.5796

911 Address 6854 NW LAKE JEFFERY ROAD, LAKE CITY, FL 32055

Contractors Name FRED GAYLARD Phone 386.961.8915

Address SAME AS ABOVE

Fee Simple Owner Name & Address —

Bonding Co. Name & Address —

Architect/Engineer Name & Address HUEY HAWKINS, P.E. - 6855 SW ELIM CHURCH Rd LAKE CITY, FL 32025

Mortgage Lenders Name & Address MERCANTILE BANK OF LAKE CITY, FL

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 05-3S-16-01994-000 Estimated Cost of Construction 186,000.

Subdivision Name — Lot ____ Block ____ Unit ____ Phase ____

Driving Directions LAKE JEFFERY Rd TO APPROX. 6½ MILES ON THE LEFT PAST THE CHURCH - NUMTSVILLE, - NEXT DRIVE AFTER IT).

Type of Construction BRICK - SFD Number of Existing Dwellings on Property 1

Total Acreage 180 Lot Size — Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 151' Side 71.4' Side 69' Rear 153.2

Total Building Height 26'3½" Number of Stories 1 Heated Floor Area 1856 Roof Pitch 10'12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor) — Contractor Signature

Contractors License Number CBC1250318

Competency Card Number

STATE OF FLORIDA
COUNTY OF COLUMBIA

NOTARY STAMP/SEAL

GALE TEDDER
MY COMMISSION # DD 333586
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

Sworn to (or affirmed) and subscribed before me this 26th day of April 2006.

Personally known ✓ or Produced Identification ____

Notary Signature

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

FORM 600B-04 — **Residential Component Prescriptive Method B** — **NORTH 1 2 3**

Compliance with Method B of Subchapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single-and multiple-family residences of three stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptives listed in this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the code.

PROJECT NAME: AND ADDRESS:	ROBERTS 6854 NW LAKE JEFFERY	BUILDER: FRED GAYLARD	
		PERMITTING OFFICE: COLUMBIA CO.	CLIMATE ZONE: 1 ☐ 2 ☐ 3 ☒
OWNER: ROBIN & LISA ROBERTS		PERMIT NO.: 24457	JURISDICTION NO.: 22 1006

1. New construction including additions which incorporate any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other nonvertical roof glass.
2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the code. Circle the column of the package you have choosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on "Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages," Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

		Please Print	CK
1.	**Compliance package chosen (A-E)**	1. A	✓
2.	**New construction or addition**	2. SFD	✓
3.	**Single-family detached or multiple-family attached**	3. SFD	✓
4.	**If multiple-family–No. of units covered by this submission**	4.	
5.	**Is this a worst case? (yes/no)**	5. NO	✓
6.	**Conditioned floor area (sq. ft.)**	6. 1856	✓
7.	**Predominant eave overhang (ft.)**	7. 2'	✓
		Single Pane / Double Pane	
8.	**Glass type and area:**		
	a. Clear glass	8a. ____ sq. ft. ____ sq. ft.	—
	b. Tint, film or solar screen	8b. ____ sq. ft. 24 sq. ft.	✓
9.	**Percentage of glass to floor area**	9. 13 %	✓
10.	**Floor type, area or perimeter, and insulation:**		
	a. Slab-on-grade (*R*-value)	10a R = 0 180 lin. ft.	✓
	b. Wood, raised (*R*-value)	10b. R = ____ ____ sq. ft.	—
	c. Wood, common (*R*-value)	10c. R = ____ ____ sq. ft.	—
	d. Concrete, raised (*R*-value)	10d. R = ____ ____ sq. ft.	—
	e. Concrete, common (*R*-value)	10e. R = ____ ____ sq. Ft.	—
11.	**Wall type, area and insulation:**		
	a. **Exterior:** 1. Masonry (Insulation R-value)	11a-1 R = ____ ____ sq. ft.	—
	2. Wood frame (Insulation *R*-value)	11a-2 R = 19 1620 sq. ft.	✓
	b. **Adjacent:** 1. Masonry (Insulation *R*-value)	11b-1 R = ____ ____ sq. ft.	—
	2. Wood frame (Insulation *R*-value)	11b-2 R = ____ ____ sq. ft.	—
12.	**Ceiling type, area and insulation:**		
	a. Under attic (Insulation *R*-value)	12a. R = 30 sq. ft. 1856	✓
	b. Single assembly (Insulation *R*-value)	12b. R = ____ ____ sq. ft.	—
13.	**Air distribution system: Duct insulation, location**	13. R = ____ ____	—
	Test report (attach if required)		
14.	**Cooling system:**	14a. Type: CENTRAL	✓
	(Types: central, room unit, package terminal A.C., gas, none)	14b. SEER/EER: 13	✓
		14c. Capacity: 2.5 T.	—
15.	**Heating system:**	15a. Type: HP	—
	(Types: heat pump, elec. strip, nat. gas, LP-Gas, gas h.p., room or PTAC, none)	15b. HSPF/COP/AFUE: 7.9	✓
16.	**Hot water system:**	15c. Capacity: 40	✓
	(Types: elec., nat. gas, LP-gas, solar, heat rec., ded. heat pump, other, none)	16a. Type: ELECT	✓
		16b. EF: .92	✓

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: Fred Gaylard DATE: 5/2/06

I hereby certify that this building is in compliance with the Florida Energy Code:

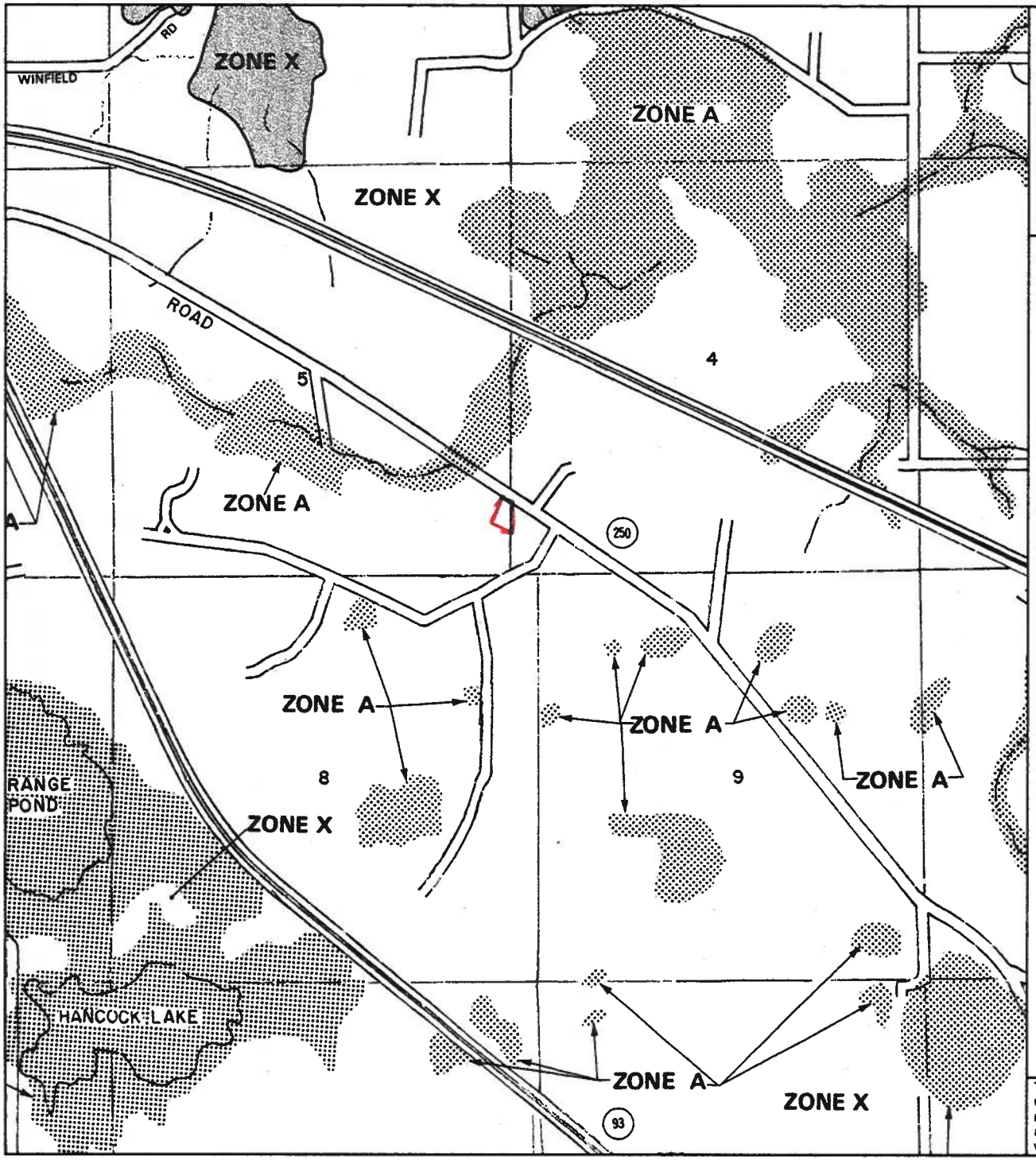
OWNER AGENT: Fred Gaylard DATE: 5/2/06

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____

DATE: _____

0604-88



N

APPROXIMATE SCALE IN FEET

2000 0 2000

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 125 OF 290

PANEL LOCATION

COMMUNITY-PANEL NUMBER
120070 0125 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Futher information about National Flood Insurance Program flood hazard maps is available at www.fema.gov/mit/tsd.

Print Date: 4/28/2006 (printed at scale and type A)

Columbia County Property Appraiser

DB Last Updated: 4/6/2006

2006 Proposed Values

Parcel: 05-3S-16-01994-000 HX

Tax Record | Property Card | Interactive GIS Map | Print

Owner & Property Info

Search Result: 1 of 1

Owner's Name	ROBERTS ROBIN &
Site Address	LAKE JEFFERY
Mailing Address	LISA K B ROBERTS 6854 NW LAKE JEFFERY RD LAKE CITY, FL 32055
Brief Legal	BEG 360 FT N OF SE COR, RUN N 354 FT TO CR-250, NW ALONG R/W 160 FT, SW 386 FT, E 284 FT

Use Desc. (code)	SINGLE FAM (000100)
Neighborhood	5316.00
Tax District	3
UD Codes	MKTA01
Market Area	01
Total Land Area	1.800 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$23,400.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (1)	$34,423.00
XFOB Value	cnt: (2)	$2,500.00
Total Appraised Value		$60,323.00

Just Value		$60,323.00
Class Value		$0.00
Assessed Value		$46,772.00
Exempt Value	(code: HX)	$25,000.00
Total Taxable Value		$21,772.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
11/4/2004	1030/116	WD	I	U	06	$100.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	SINGLE FAM (000100)	1960	Average (05)	1380	2460	$34,423.00
	Note: All S.F. calculations are based on exterior building dimensions.					

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0296	SHED METAL	0	$1,000.00	1.000	18 x 31 x 0	(.00)
0190	FPLC PF	0	$1,500.00	1.000	0 x 0 x 0	(.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000100	SFR (MKT)	1.800 AC	1.00/1.00/1.00/1.00	$13,000.00	$23,400.00

Columbia County Property Appraiser — DB Last Updated: 4/6/2006


(AKA C.R. 250
INTERSTATE 10
053S1601994000
ROBERTS ROBIN &
NW LAKE JEFFERY RD
OLD WINFIELD HUNTSVILLE RD
NW POMPANO CT (Pvt.)
NW PETTIGREW GLN


053S1601994000
ROBERTS ROBIN &

Doc .70

THIS INSTRUMENT PREPARED BY:

MARLIN M. FEAGLE, ESQUIRE
FEAGLE & FEAGLE, ATTORNEYS, P.A.
153 NE Madison Street
Post Office Box 1653
Lake City, Florida 32056-1653
(as to form only)

Florida Bar No. 0173248

Inst:2004024930 Date:11/08/2004 Time:10:36
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1030 P:116

WARRANTY DEED

THIS INDENTURE, made this 4th day of November, 2004, between **ROBIN H. ROBERTS** and his wife, **LISA K. B. ROBERTS**, whose mailing address is 6854 NW Lake Jeffrey Road, Lake City, Florida 32055, parties of the first part, Grantor, and **ROBIN H. ROBERTS** and his wife, **LISA K. B. ROBERTS**, whose mailing address is 6854 NW Lake Jeffrey Road, Lake City, Florida 32055, parties of the second part, Grantee,

W I T N E S S E T H:

That said grantor, for and in consideration of the sum of **TEN AND NO/100** ($10.00) **DOLLARS**, and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs, successors and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

All my right, title and interest in and to the following described property:

TOWNSHIP 3 SOUTH - RANGE 16 EAST

Section 5: Commence at the Southeast corner of Section 5, Township 3 South, Range 16 East, and run North along the East line of said Section 5 a distance of 360 feet for a **POINT OF**

Inst:2004024930 Date:11/08/2004 Time:10:36
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1030 P:117

BEGINNING; and run thence North along said Section line 354 feet; thence run in a Westerly direction 160 feet; thence run South 21°51' West 386 feet, more or less; thence run in an Easterly direction 284 feet, more or less, to the **POINT OF BEGINNING**.

Tax Parcel No.: 05-3S-16-01994-000

TOGETHER WITH all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD the same in fee simple forever.

AND the Grantor hereby covenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has good right and lawful authority to sell and convey said land; that the Grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2003.

IN WITNESS WHEREOF, the said Grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered
in the presence of:

Witness
MARLIN Feagle
Print or type name

Witness
DIANE S. EDENFIELD
Print or type name

ROBIN H. ROBERTS (SEAL)

LISA K. B. ROBERTS (SEAL)

Inst:2004024930 Date:11/08/2004 Time:10:36
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1030 P:118

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 4th day of November, 2004, by **ROBIN H. ROBERTS** and his wife, **LISA K. B. ROBERTS,** who are personally known to me.

Diane S. Edenfield
MY COMMISSION # DD112002 EXPIRES
May 26, 2006
BONDED THRU TROY FAIN INSURANCE, INC.

(NOTARIAL SEAL)

Diane S. Edenfield
Notary Public, State of Florida

My Commission Expires:

Columbia County Property Appraiser

DB Last Updated: 4/6/2006

2006 Proposed Values

Parcel: 05-3S-16-01994-000 HX

Tax Record | Property Card | Interactive GIS Map | Print

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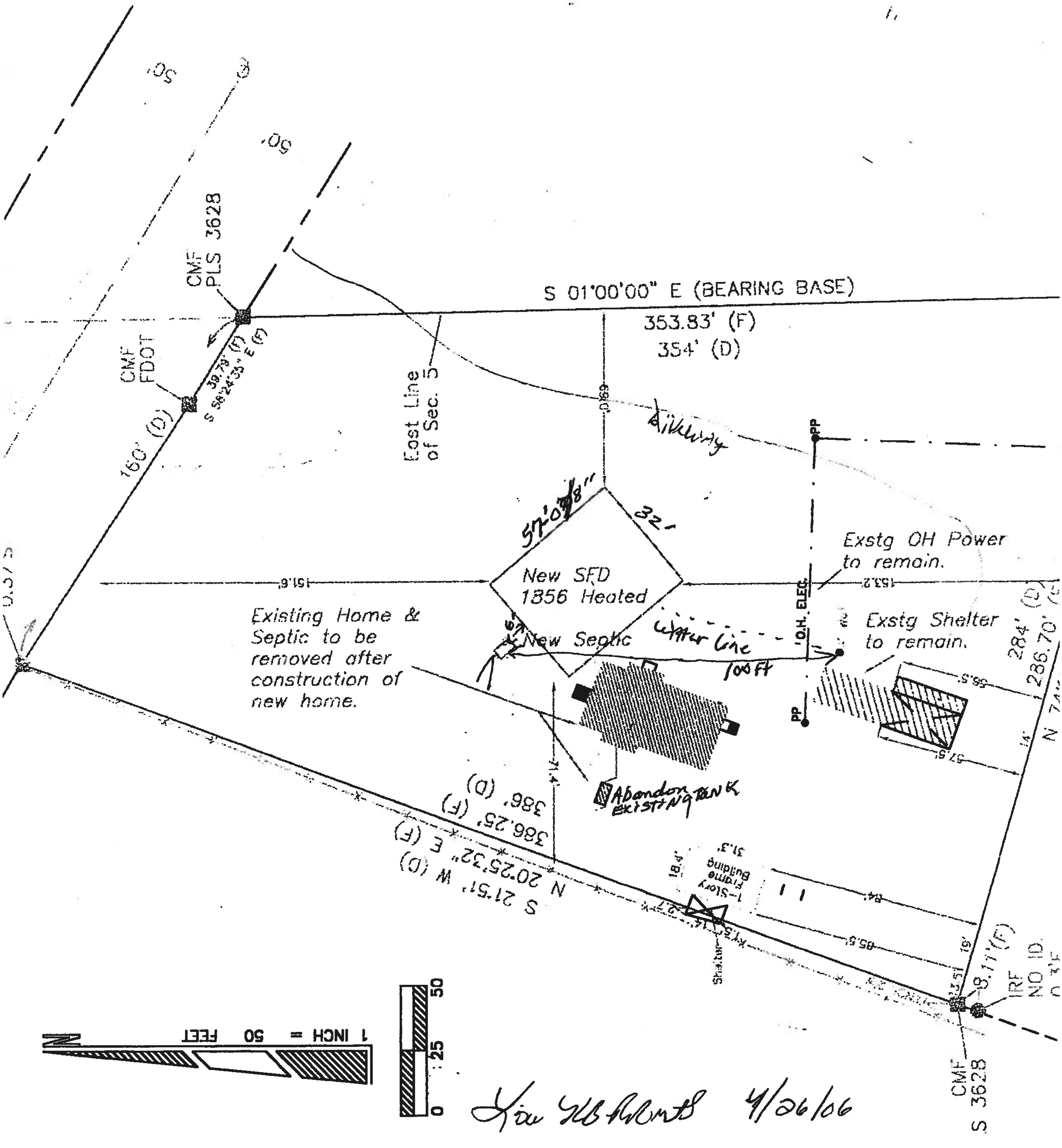
Columbia County Property Appraiser DB Last Updated: 4/6/2006

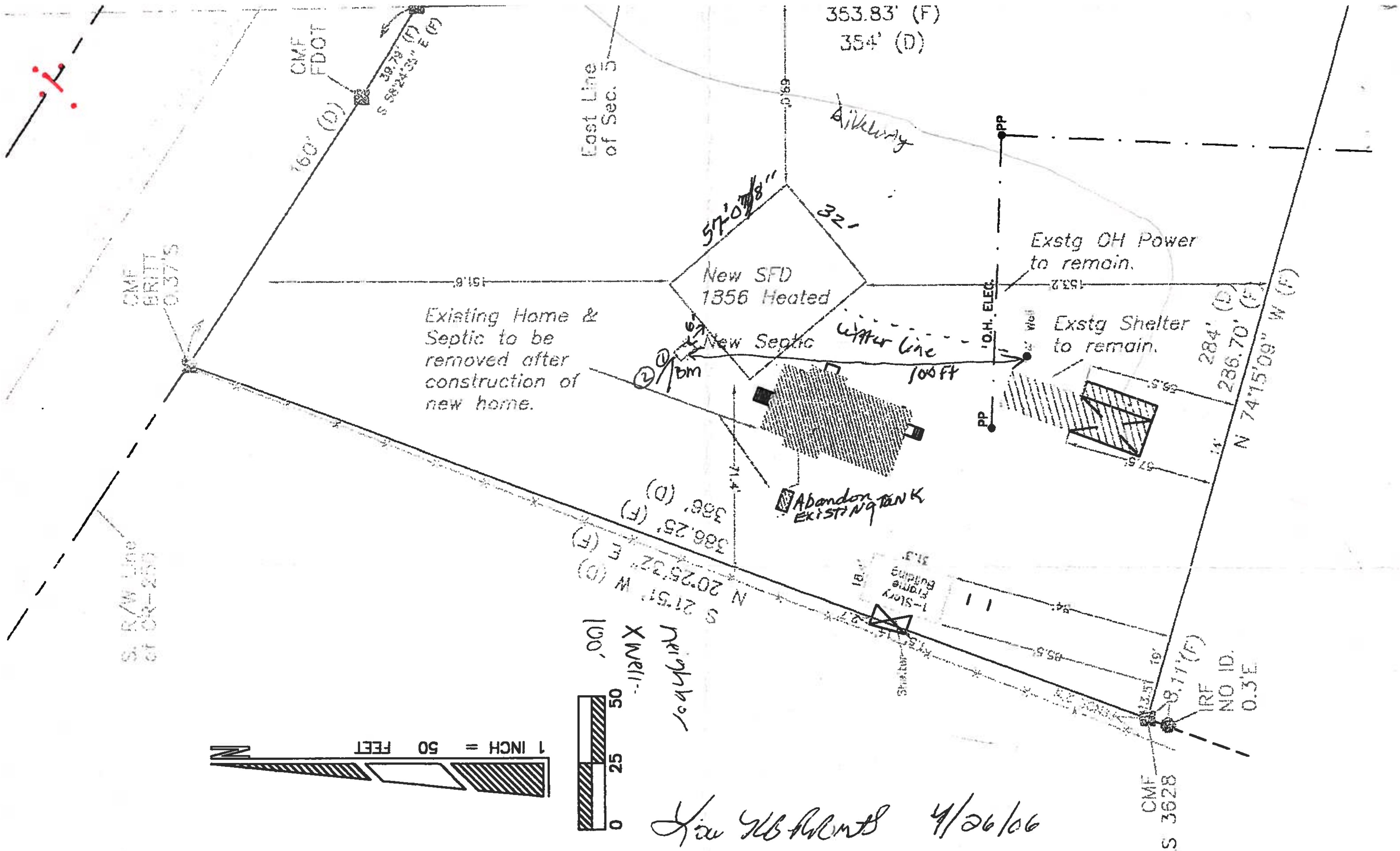
1 of 1

as recorded in O.R. Book 617, Page 687, Columbia County, Florida. COMMENCE at the Southeast corner ...wnship 3 South, Range 16 East and run North along the East line of said Section 5 a distance of 360 ...INT OF BEGINNING and run thence North along said Section line 354 feet, thence run in a Westerly ... feet; thence run South 21 '51' West 386 feet, more or less thence run in an Easterly direction 284 ...less to the POINT OF BEGINNING.

...N FOR ROBIN & LISA ROBERTS



Lisa H B Roberts 4/26/06



353.83' (F)
354' (D)
CMF
FDOT
39.78' (F)
S 58°24'35" E (F)
160' (D)
East Line
of Sec. 5
69.0'
CMF
BRITT
0.37'S
New SFD
1356 Heated
32'
151.6'
153.2'
Exstg OH Power
to remain.
O.H. ELEC.
PP
PP
Exstg Shelter
to remain.
284' (D)
286.70' (F)
N 74°15'09" W (F)
Existing Home &
Septic to be
removed after
construction of
new home.
New Septic
Water Line
100 Ft
Abandon EXISTING TANK
71.4'
388' (D)
386.25' (F)
N 20°25'32" E (F)
S 21°51' W (D)
1-Story
Frame
Building
Shelter
IRF
NO ID.
0.3'E
CMF
S 3628
S. R/W Line
of CR-250
neighbor
X well
100'
0
25
50
1 INCH = 50 FEET

4/26/06

APPROVED Sallie Graddy ES II 5/2/06

06-421

24457

CINCINNATI INSURANCE COMPANY
FLORIDA

THIS ENDORSEMENT CHANGES THE POLICY. PLEASE READ IT CAREFULLY.

GENERAL CHANGE ENDORSEMENT

Attached to and forming part of Policy No. CAP5486340
Issued to F.O.G. CONTRACTING INC
Effective Date of Endorsement 07-30-2004

Agency HIB, ROGAL & HAMILTON COMPANY OF 09-155

Additional Premium Due at Endorsement Effective Date $ [redacted]

Subsequent Quarterly Installments Increased by $ [redacted]
Revised Installment Payment $ [redacted]
not Including Auto Premiums

AMENDING POLICY TO EXPIRE 07-30-2006

AMENDING COMMERCIAL GENERAL LIABILITY PREMIUMS PER REVISED GA501 ATTACHED

KG6 06/23/04

IA 407 01 95 ALL OTHER TERMS AND CONDITIONS REMAIN UNCHANGED

THE CINCINNATI INSURANCE COMPANY

A STOCK INSURANCE COMPANY

COMMERCIAL GENERAL LIABILITY COVERAGE PART DECLARATIONS

Attached to and forming part of POLICY NUMBER: CAP 548 63 40 Effective Date: 07-30-2004

Named Insured: IS THE SAME AS IT APPEARS ON THE COMMON POLICY DECLARATION

LIMITS OF INSURANCE

EACH OCCURRENCE LIMIT	$ 1,000,000	
GENERAL AGGREGATE LIMIT	$ 2,000,000	
PRODUCTS-COMPLETED OPERATIONS AGGREGATE LIMIT	$ 2,000,000	
PERSONAL & ADVERTISING INJURY LIMIT	$ 1,000,000	ANY ONE PERSON OR ORGANIZATION
DAMAGE TO PREMISES RENTED TO YOU LIMIT $100,000 limit unless otherwise indicated herein:	$ _____	ANY ONE PREMISES
MEDICAL EXPENSE LIMIT $5,000 limit unless otherwise indicated herein:	$ _____	ANY ONE PERSON

CLASSIFICATION	CODE NO.	PREMIUM BASE (A - Area, B - Payroll, C - Gross Sales, D - Units, E - Other)	RATE		ADVANCE PREMIUM	
			Products / Completed Operations	All Other	Products / Completed Operations	All Other
CONTRACTORS-EXECUTIVE INCL PROD AND/OR COMP OP	91580	B 16,700		[redacted]		[redacted]
CONTRACTORS-SUB WORK	91583	E 400,000	[redacted]	[redacted]	[redacted]	[redacted]

The General Liability Coverage Part is subject to an annual minimum premium.

TOTAL ANNUAL PREMIUM $ [redacted]

FORMS AND / OR ENDORSEMENTS APPLICABLE TO THIS COVERAGE PART:

GA101 10/01 GA369 11/02 GA382 03/02 GA4153 10/01

08-13-2004

TOM GALLAGHER
CHIEF FINANCIAL OFFICER

STATE OF FLORIDA
DEPARTMENT OF FINANCIAL SERVICES
DIVISION OF WORKERS' COMPENSATION

**** CERTIFICATE OF EXEMPTION FROM FLORIDA WORKERS' COMPENSATION LAW ****

CONSTRUCTION INDUSTRY EXEMPTION

This certifies that the individual listed below has elected to be exempt from Florida Workers' Compensation Law.

EFFECTIVE DATE: 09/16/2004 ** EXPIRATION DATE: 09/16/2006

PERSON: GAYLARD FRED O JR

FEIN: 820550363

BUSINESS NAME AND ADDRESS: T O G CONTRACTING INC
1777 NW BULTHUIS CT
LAKE CITY FL 32055

SCOPE OF BUSINESS OR TRADE: 1 - CERTIFIED BUILDING CONTRACTOR

MEETS REISSUANCE REQUIREMENTS

IMPORTANT: Pursuant to Chapter 440.05(14), F.S., an officer of a corporation who elects exemption from this chapter by filing a certificate of election under this section may not recover benefits or compensation under this chapter.

DWC-252 CERTIFICATE OF ELECTION TO BE EXEMPT REVISED 01-04

QUESTIONS? (850) 413-1609

Residential Heat Loss and Heat Gain Calculation

In accordance with ACCA Manual J

Registered For:

[redacted]

For: Robin & Lisa Roberts
6854 NW Lake Jeffrey Road
Lake City, Florida 32055

Design Conditions: Gainesville

Indoor:		**Outdoor:**	
Summer temperature:	74	Summer temperature:	93
Winter temperature:	70	Winter temperature:	31
Relative humidity:	55	Summer grains of moisture:	116
		Daily temperature range:	Medium

Building Component		Sensible Gain (BTUH)	Latent Gain (BTUH)	Total Heat Gain (BTUH)	Total Heat Loss (BTUH)
Whole House	**1,826.2 sq.ft.**	**19,397**	**4,504**	**23,901 (2 tons)**	**21,939**
Main Floor		19,400	4,506	23,906	21,938
Living Room	297 sq.ft.	4,259	1,408	5,667	4,179
Infiltration		319	488	807	982
Duct		710	0	710	545
People	4	1,200	920	2,120	0
Floor	297 sq.ft.	0	0	0	613
SW Wall	109.2 sq.ft.	99	0	99	170
Window	30 sq.ft.	1,218	0	1,218	848
Door	35.6 sq.ft.	370	0	370	639
Ceiling	297 sq.ft.	343	0	343	382
Bedroom #2	163 sq.ft.	1,282	111	1,393	2,251
Infiltration		73	111	184	223
Duct		214	0	214	294
Floor	163 sq.ft.	0	0	0	758
SW Wall	93 sq.ft.	84	0	84	145
Window	15 sq.ft.	609	0	609	424
NW Wall	126 sq.ft.	114	0	114	197
Ceiling	163 sq.ft.	188	0	188	210
Bathroom #2	74 sq.ft.	151	0	151	371
Infiltration		0	0	0	0
Duct		25	0	25	48
Floor	73.8 sq.ft.	0	0	0	158

Building Component		Sensible Gain (BTUH)	Latent Gain (BTUH)	Total Heat Gain (BTUH)	Total Heat Loss (BTUH)
NW Wall	45 sq.ft.	41	0	41	70
Ceiling	73.8 sq.ft.	85	0	85	95
Bedroom #3	153 sq.ft.	1,105	111	1,216	2,202
Infiltration		73	111	184	223
Duct		184	0	184	287
Floor	153 sq.ft.	0	0	0	758
NW Wall	108 sq.ft.	98	0	98	168
NE Wall	93 sq.ft.	84	0	84	145
Window	15 sq.ft.	489	0	489	424
Ceiling	153 sq.ft.	177	0	177	197
Hall/Closets off Library	66 sq.ft.	128	0	128	298
Infiltration		0	0	0	0
Duct		21	0	21	39
Floor	66.4 sq.ft.	0	0	0	120
NE Wall	34.5 sq.ft.	31	0	31	54
Ceiling	66 sq.ft.	76	0	76	85
Library	125 sq.ft.	1,658	111	1,769	1,479
Infiltration		73	111	184	223
Duct		276	0	276	193
Miscellaneous		600	0	600	0
Floor	125 sq.ft.	0	0	0	347
NE Wall	84 sq.ft.	76	0	76	131
Window	15 sq.ft.	489	0	489	424
Ceiling	125 sq.ft.	144	0	144	161
Library WIC Closet	28 sq.ft.	38	0	38	41
Infiltration		0	0	0	0
Duct		6	0	6	5
Floor	28 sq.ft.	0	0	0	0
Ceiling	28 sq.ft.	32	0	32	36
Library Toilet	16 sq.ft.	22	0	22	24
Infiltration		0	0	0	0
Duct		4	0	4	3
Floor	16 sq.ft.	0	0	0	0
Ceiling	16 sq.ft.	18	0	18	21
Library Mechanical	18 sq.ft.	25	0	25	26
Infiltration		0	0	0	0
Duct		4	0	4	3
Floor	18 sq.ft.	0	0	0	0

Building Component		Sensible Gain (BTUH)	Latent Gain (BTUH)	Total Heat Gain (BTUH)	Total Heat Loss (BTUH)
Ceiling	18 sq.ft.	21	0	21	23
Entry Foyer/hall	74 sq.ft.	486	148	634	1,038
Infiltration		97	148	245	298
Duct		81	0	81	135
Floor	74.2 sq.ft.	0	0	0	126
NE Wall	16 sq.ft.	14	0	14	25
Door	20 sq.ft.	208	0	208	359
Ceiling	74.2 sq.ft.	86	0	86	95
Dining Room	169 sq.ft.	2,442	1,031	3,473	1,561
Infiltration		73	111	184	223
Duct		407	0	407	204
People	4	1,200	920	2,120	0
Floor	169 sq.ft.	0	0	0	357
NE Wall	86.7 sq.ft.	78	0	78	135
Window	15 sq.ft.	489	0	489	424
Ceiling	169 sq.ft.	195	0	195	218
Master Bedroom	230 sq.ft.	2,070	222	2,292	3,344
Infiltration		145	222	367	446
Duct		345	0	345	436
Floor	230 sq.ft.	0	0	0	945
NE Wall	106.7 sq.ft.	96	0	96	166
Window	15 sq.ft.	489	0	489	424
SE Wall	133 sq.ft.	120	0	120	207
Window	15 sq.ft.	609	0	609	424
Ceiling	230 sq.ft.	266	0	266	296
Master Bath	94 sq.ft.	1,012	111	1,123	1,277
Infiltration		73	111	184	223
Duct		169	0	169	167
Floor	94 sq.ft.	0	0	0	253
SE Wall	57 sq.ft.	52	0	52	89
Window	15 sq.ft.	609	0	609	424
Ceiling	94 sq.ft.	109	0	109	121
Master WI Closet	51 sq.ft.	210	0	210	829
Infiltration		0	0	0	0
Duct		35	0	35	108
Floor	50.8 sq.ft.	0	0	0	455
SE Wall	61 sq.ft.	55	0	55	95
SW Wall	67.4 sq.ft.	61	0	61	105

Building Component		Sensible Gain (BTUH)	Latent Gain (BTUH)	Total Heat Gain (BTUH)	Total Heat Loss (BTUH)
Ceiling	51 sq.ft.	59	0	59	66
Laundry	48 sq.ft.	604	67	671	813
Infiltration		44	67	111	134
Duct		101	0	101	106
Floor	48 sq.ft.	0	0	0	190
SW Wall	43.2 sq.ft.	39	0	39	67
Window	9 sq.ft.	365	0	365	254
Ceiling	48 sq.ft.	55	0	55	62
Kitchen	220 sq.ft.	3,908	1,186	5,094	2,205
Infiltration		174	266	440	536
Duct		651	0	651	288
People	4	1,200	920	2,120	0
Miscellaneous		1,200	0	1,200	0
Floor	220 sq.ft.	0	0	0	357
SW Wall	65.7 sq.ft.	59	0	59	102
Door	35.6 sq.ft.	370	0	370	639
Ceiling	220 sq.ft.	254	0	254	283
Whole House	**1,826.2 sq.ft.**	**19,397**	**4,504**	**23,901 (2 tons)**	**21,939**

Load calculations are estimates only, actual loads may vary due to weather and construction differences.

User: Public User - Not Associated with Organization -

Need Help ?

Application #:	FL1901-R2
Date Submitted:	08/05/2005
Code Version:	2004
Product Manufacturer:	Simpson Strong-Tie Co.
Address/Phone/email:	2221 Country Lane McKinney, TX 75070 (972) 439-3029
Technical Representative:	Randall Shackelford
Technical Representative Address/Phone/email:	1720 Couch Drive McKinney, TX 75069 (800) 999-5099 rshackelford@strongtie.com
Quality Assurance Representative:	Pat Woodall
Quality Assurance Representative Address/Phone/email:	1720 Couch Drive McKinney, TX 75069 (800) 999-5099 pwoodall@strongtie.com
Category:	Structural Components
Subcategory:	Wood Connectors Anchors
Evaluation Method:	Evaluation Report from a Florida Registered Architect or Florida Professional Engineer

Referenced Standards from the Florida Building Code:

Section	Standard	Year
1715.1.1	ASTM D1761-88	2000
2314.4.4	ASTM D1761-88	2000
2209.1	N. A. Spec for Design of Cold-Formed Steel Structu	2001
2214.3	N. A. Spec for Design of Cold-Formed Steel	2001

2306.1	Structu Nat'l Design Specification for Wood Construction	2001
2314.4.7	Nat'l Design Specification for Wood Construction	2001

Florida Engineer or Architect Name: Jeffrey P. Arneson, P.E.

Florida License: PE- 58544

Quality Assurance Entity: PSI/Pittsburgh Testing Laboratory

Validation Entity: Dole J. Kelley

Authorized Signature: Randall Shackelford
rshackelford@strongtie.com

Evaluation/Test Reports Uploaded: PTID_1901_R2_T_Arneson Certificate of Indep05.pdf
PTID_1901_R2_T_Sim200401R2.pdf

Installation Documents Uploaded:

Product Approval Method: Method 1 Option D

Application Status: Approved
Date Validated: 08/05/2005
Date Approved: 08/24/2005
Date Certified to the 2004 Code:

Page: Go Page 1 / 5 > >|

App/Seq #	Product Model # or Name	Model Description	Limits of Use
1901.1	CMST12	Coiled Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.2	CMST14	Coiled Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.3	CMSTC16	Coiled Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.4	CS16	Coiled Strap Tie	Limitations per attached evaluation report. Must be installed as specified in

			report. OK for HVHZ
1901.5	CS18	Coiled Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.6	CS20	Coiled Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.7	CS22	Coiled Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.8	FHA12	Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.9	FHA18	Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.10	FHA24	Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.11	FHA30	Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.12	FHA6	Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.13	FHA9	Strap Tie	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.14	HETA12	Embedded Truss Anchor	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.15	HETA16	Embedded Truss Anchor	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.16	HETA20	Embedded Truss Anchor	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.17	HETA24	Embedded Truss Anchor	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
1901.18	HETA40	Embedded Truss Anchor	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ
			Limitations per attached

1901.19	HETAL12	Embedded Truss Anchor	evaluation report. Must be installed as specified in report. OK for HVHZ
1901.20	HETAL16	Embedded Truss Anchor	Limitations per attached evaluation report. Must be installed as specified in report. OK for HVHZ

Next

VeriSign Secured VERIFY

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APEXSIM2004-01

Used for Florida State Wide Product Approval

FL1901

Products on this Report which are approved:

Product	Florida #	Product	Florida #	Product	Florida #	Product	Florida #
CMST12	FL1901.1	HHETA20	FL1901.24	MST27	FL1901.48	MSTI26	FL1901.71
CMST14	FL1901.2	HHETA24	FL1901.25	MST37	FL1901.49	MSTI36	FL1901.72
CMSTC16	FL1901.3	HRS12	FL1901.27	MST48	FL1901.50	MSTI48	FL1901.73
CS16	FL1901.4	HRS6	FL1901.28	MST60	FL1901.51	MSTI60	FL1901.74
CS18	FL1901.5	HRS8	FL1901.29	MST72	FL1901.52	MSTI72	FL1901.75
CS20	FL1901.6	LSTA12	FL1901.30	MSTA12	FL1901.53	RPS18	FL1901.76
CS22	FL1901.7	LSTA15	FL1901.31	MSTA15	FL1901.54	RPS22	FL1901.77
FHA12	FL1901.8	LSTA18	FL1901.32	MSTA18	FL1901.55	RPS28	FL1901.78
FHA18	FL1901.9	LSTA21	FL1901.33	MSTA21	FL1901.56	ST12	FL1901.79
FHA24	FL1901.10	LSTA24	FL1901.34	MSTA24	FL1901.57	ST18	FL1901.80
FHA30	FL1901.11	LSTA30	FL1901.35	MSTA30	FL1901.58	ST2115	FL1901.81
FHA6	FL1901.12	LSTA36	FL1901.36	MSTA36	FL1901.59	ST2122	FL1901.82
FHA9	FL1901.13	LSTA9	FL1901.37	MSTA9	FL1901.60	ST22	FL1901.83
HETA12	FL1901.14	LSTI49	FL1901.38	MSTAM24	FL1901.61	ST2215	FL1901.84
HETA16	FL1901.15	LSTI73	FL1901.39	MSTAM36	FL1901.62	ST292	FL1901.85
HETA20	FL1901.16	META12	FL1901.40	MSTC28	FL1901.63	ST6215	FL1901.86
HETA24	FL1901.17	META14	FL1901.41	MSTC40	FL1901.64	ST6224	FL1901.87
HETA40	FL1901.18	META16	FL1901.42	MSTC48B3	FL1901.65	ST6236	FL1901.88
HETAL12	FL1901.19	META18	FL1901.43	MSTC52	FL1901.66	ST9	FL1901.89
HETAL16	FL1901.20	META20	FL1901.44	MSTC66	FL1901.67		
HETAL20	FL1901.21	META22	FL1901.45	MSTC66B3	FL1901.68		
HHETA12	FL1901.22	META24	FL1901.46	MSTC78	FL1901.69		
HHETA16	FL1901.23	META40	FL1901.47	MSTCM40	FL1901.70		

SIMPSON
Strong-Tie

SIMPSON STRONG-TIE COMPANY, INC

Jax Apex Technology, Inc.
4745 Sutton Park Court, Suite 402
Jacksonville, FL 32246/ 904/821-5200

Evaluation reports are the opinion of the evaluation entity, based on the findings, and in no way constitute or imply approval by a local building authority. Apex Technolgy, in review of the data submitted, finds that, in their opinion, the product, material, system, or method of construction specifically identified in this report conforms with or is a suitable alternate to that specified in the Florida Building Code, SUBJECT TO THE LIMITATIONS IN THIS REPORT

Apex Technology has reviewed the data submitted for compliance with the Florida Building Code. Apex Technology is not responsible for any errors or omissions to any documents, calculations, drawings, specifications, tests, or summaries prepared and submitted by the design professional or preparer of record who are listed in the Substantiating Data section of this report.

REPORT NO.: SIM200401

EXPIRES: March 1st, 2006

CATEGORY: Metal Connectors

SUBMITTED BY:
SIMPSON STRONG-TIE COMPANY, INC.
4120 DUBLIN BLVD., SUITE 400
DUBLIN, CA 94568

1. PRODUCT NAME

Strap Ties

LSTA9, LSTA12, LSTA15, LSTA18, LSTA21, LSTA24, LSTA30, LSTA36, MSTA9, MSTA12, MSTA15, MSTA18, MSTA21, MSTA24, MSTA30, MSTA36, MSTC28, MSTC40, MSTC52, MSTC66, MSTC78, MST27, MST37, MST48, MST60, MST72, LSTI49, LSTI73, MSTI26, MSTI36, MSTI48, MSTI60, MSTI72, RPS18, RPS22, RPS28, ST2115, ST292, ST2122, ST2215, ST6215, ST6224, ST6236, ST9, ST12, ST18, ST22, FHA6, FHA9, FHA12, FHA18, FHA24, FHA30.

Coiled Strap Ties

CMST12, CMST14, CMSTC16, CS16, CS18, CS20, CS22

Wood to Masonry Strap Ties

MSTAM24, MSTAM36, MSTCM40

Pre-bent Strap Ties

MSTC48B3, MSTC66B3

Heavy Straps

HRS6, HRS8, HRS12

Embedded Truss Anchors

META12, META14, META16, META18, META20, META22, META24, META40, HETA12, HETA16, HETA20, HETA24, HETA40, HETAL12, HETAL16, HETAL20, HHETA12, HHETA16, HHETA20, HHETA24, HHETA40

2. SCOPE OF EVALUATION

Load Evaluation as a Structural Component using the requirements of the *Florida Building Code*

3. DESCRIPTION:

3.1 LSTA, MSTA Strap Tie. The LSTA and MSTA Strap Tie models are used to provide a tension connection between two wood members. They are 1¼" wide for use on 1½" members. They are installed with 10d common nails. Allowable loads are shown in Table 1. The straps are manufactured from steel meeting ASTM A-653 SS Grade 50, of a thickness as specified in Table 1. They are coated with a G90 galvanized finish.

3.2 MSTC Strap Tie. The MSTC Strap Tie models are used to provide a tension connection between two wood members. They are 3" wide for use on doubled 1½" members or single 4-by or larger members. They have countersunk nail slots for a lower nailing profile, and coined edges for safer handling. They are installed with 16d sinker nails. Allowable loads are shown in Table 2 and 3. The straps are manufactured from steel meeting ASTM A-653 SS Grade 50, of a thickness as specified in Table 2. They are coated with a G90 galvanized finish.

3.3 MST Strap Tie. The MST Strap Tie models are used to provide a tension connection between two wood members. They are $2^{1}/_{16}$" wide for use on doubled 1½" members or 4-by or larger members. They are installed with 16d common nails. To reduce the possibility of splitting when installed on lumber narrower than 3½", install 16d common nails in every other nail hole or 10d×1½" nails in every nail hole and reduce allowable load accordingly. Allowable loads are shown in Table 2 and 3. The MST27 and 37 are manufactured from steel meeting ASTM A-653 SS, Grade 40, and the MST48, MST60 and MST72 are manufactured from steel meeting ASTM A-653 SS, Grade 40 Special, of a thickness as specified in Table 2. They are coated with a G90 galvanized finish.

3.4 LSTI, MSTI Strap Tie. The LSTI and MSTI Strap Tie models are used to provide a tension connection between two wood members. The LSTI models are 3¾" wide and the MSTI models are $2^{1}/_{16}$" wide. They are designed for use on members that could be damaged by large nails, such as I-joist or truss chords. They are installed with 10d×1½" nails. Allowable loads are shown in Table 2 and 3. The straps are manufactured from steel meeting ASTM A-653 SS Grade 40, of a thickness as specified in Table 2. They are coated with a G90 galvanized finish.

3.5 RPS Strap Tie. The RPS Strap Tie models are used to provide a tension connection between two wood members, especially when the top or bottom plates of a wood wall are cut or notched. They are 1½" wide and are installed with 16d common nails. Allowable loads are shown in Table 4. The straps are manufactured from steel meeting ASTM A-653 SS Grade 40, of a thickness as specified in Table 4. They are coated with a G90 galvanized finish.

3.6 ST Strap Tie. The ST Series Strap Tie models are used to provide a tension connection between two wood members. They are manufactured in various widths and lengths. They are installed with 16d common nails. Allowable loads are shown in Table 4. The ST9, ST12, ST18, ST22, ST292 and ST6215 are manufactured from steel meeting ASTM A-653 SS, Grade 33, the ST2122 and ST6224 are manufactured from steel meeting ASTM A-653 SS, Grade 40, and the ST2115, ST2215 and ST6236 are manufactured from steel meeting ASTM A-653 SS, Grade 50, Class 1. The thickness is as specified in Table 4. They are coated with a G90 galvanized finish.

3.7 FHA Strap Tie. The FHA Strap Tie models are used to provide a tension connection between two wood members. They have a corrugated shape with a total width of $1^{7}/_{16}$". They are installed with 16d common nails. Allowable loads are shown in Table 4. The straps are manufactured from steel meeting ASTM A-653 SS Grade 33, of a thickness as specified in Table 4. They are coated with a G90 galvanized finish.

3.8 HRS Heavy Strap Tie. The HRS Strap Tie models are short, thick straps used to provide a tension connection between two wood members. They are 1⅜" wide and are installed with 10d common nails. Allowable loads are shown in Table 4. The straps are manufactured from 12 ga. steel meeting ASTM A-653 SS Grade 33. They are coated with a G90 galvanized finish.

3.9 CS, CMST, CMSTC Coiled Strap Tie. The CS, CMST and CMSTC Coil Strap Tie models are used to provide a tension connection between two wood members. They are packaged in a coil so that the length of strap needed can be cut from the coil on the jobsite. The CS Straps are 1¼" wide for use on 1½" or wider members. They install with either 8d or 10d common nails. The CMST and CMSTC Straps are 3" wide for use on doubled 2-by or 4-by or larger members. The CMST Straps are installed with either 10d or 16d common nails. The CMST has both round and triangle nail holes. If using 16d nails and the wood tends to split, use round holes only. The CMSTC Straps are installed with 16d sinker nails. The CMSTC has coined edges for safer handling. Allowable loads are shown in Table 5. The CS straps are manufactured from steel meeting ASTM A-653 SS Grade 40, of a thickness as specified in Table 5. The CMST and CMSTC straps are manufactured from steel meeting ASTM A-653 SS Grade 50 Class 1, of a thickness as specified in Table 5. They are coated with a G90 galvanized finish.

3.10 MSTAM, MSTCM Strap Tie. The MSTAM and MSTCM Strap Tie models are used to provide a tension connection between wood members and a masonry or concrete structure. The MSTAM Straps are 1¼" wide for use on 1½" and larger members. They are installed with 10d common nails to the wood and either ¼×2¼" Titen Masonry Screws to masonry, or ¼×1¾" Titen Masonry Screws to concrete. The MSTCM Strap is 3" wide for use on doubled 2-by or single 4-by and larger members. They are installed with 16d sinker nails to the wood and either ¼×2¼" Titen Masonry Screws to masonry, or ¼×1¾" Titen Masonry Screws to concrete. The MSTCM Strap has countersunk nail slots for a lower nailing profile and coined edges for safer handling. Allowable loads are shown in Table 6 and 7. The straps are manufactured from steel meeting ASTM A-653 SS Grade 50, Class 1, of a thickness as specified in Table 6. They are coated with a G90 galvanized finish.

3.11 MSTCB3 Pre-bent Strap Tie. The MSTC48B3 and MSTC66B3 Pre-bent Strap Ties are designed to transfer a heavy tension load from framing on an upper story wall to a beam or header on the story below. For example, this could be from shearwall overturning or a large girder truss uplift load. They are installed with 10d common nails,

with a minimum of four nails in the bottom of the beam or header. Allowable loads are shown in Table 8. The straps are manufactured from 14 ga. steel meeting ASTM A-653 SS Grade 50, Class 1. They are coated with a G90 galvanized finish.

3.12 META, HETA, HETAL, HHETA Embedded Truss Anchors. Embedded Truss Anchors are used to anchor a wood member (usually a truss) to a masonry or concrete wall. Embedded truss anchors fasten to a single-ply wood truss with 10d×1½ nails or to a multiple-ply truss with 16d common nails. They are embedded in the masonry or concrete wall to a depth indicated on the side of the anchor (4" for META, HETA, and HETAL, and $5^{1}/_{16}$" for HETAL). The strap portion of the anchor is 1⅛" wide. Allowable loads are shown in Table 9 for single installations and Table 10 for double installations. The anchors are manufactured from steel meeting ASTM A-653 SS Grade 50, Class 1, with the exception of the truss seat of the HETAL which is manufactured from steel meeting ASTM A-653 SS Grade 33. Steel thickness is as specified in Table 9. The Embedded Truss Anchors are coated with a G90 galvanized finish.

4. MATERIALS

4.1 Steel. Steel specifications for each product listed in this evaluation report shall be as indicated in the previous section. In addition to the standard G90 finish, some products are available with a G185 finish, indicated as Z-Max. Allowable loads published in this report will apply to G185 products as well as G90 products.

4.2 Wood. Wood members to which these connectors are fastened shall be solid sawn lumber, glued-laminated lumber, or structural composite lumber having dimensions consistent with the connector dimensions shown in Tables1 through 4. Unless otherwise noted, lumber shall be Southern Pine or Douglas Fir-Larch having a minimum specific gravity of 0.50. Where indicated by SPF, lumber shall be Spruce-Pine-Fir having a minimum specific gravity of 0.42.

4.3 Nails and Bolts. Unless noted otherwise, nails shall be common nails. Nails shall comply with ASTM F 1667 and shall have the minimum bending yield strengths F_{yb}:

Nail Pennyweight	Nail Shank Diameter (inch)	F_{yb} (psi)
10d Common	0.148	90,000
16d Sinker	0.148	90,000
16d Common	0.162	90,000

Fasteners for galvanized connectors in pressure-preservative treated wood shall be hot-dipped zinc coated galvanized steel, except where otherwise permitted by the treatment manufacturer. Fasteners for stainless steel connectors shall be stainless steel.

4.4 Concrete/Masonry. Concrete and Masonry design specifications shall be the stricter of the specifications by the engineer of record, the Florida Building Code minimum standards, or the following:

Material	Specification	Minimum Compressive Strength
Concrete, f'c	-	2500 psi
Masonry, f'm	ASTM E447	1500 psi
Masonry Unit	ASTM C90	1900 psi
Mortar	ASTM C270 Type S	1800 psi (or by proportions)
Grout	ASTM C476	2000 psi (or by proportions)

5. INSTALLATION

Installation shall be in accordance with this report and the most recent edition of the Simpson Strong-Tie *Wood Construction Connectors* catalog. Information in this report supercedes any conflicting information between information provided in this report and the catalogue, the information in this report supercedes the catalogue.

6. SUBSTANTIATING DATA

Test data submitted by Testing Engineers Inc. and Product Testing, Inc., and signed and sealed calculations provided by Simpson Strong-Tie performed in accordance with the 2001 Florida Building Code.

7. FINDINGS

The connectors listed in this evaluation report comply with the Florida Building Code when installed in accordance with this report. Maximum allowable loads shall not exceed the allowable loads listed in this report.

8. LIMITATIONS:

1. Maximum allowable loads shall not exceed the allowable loads listed in this report. Allowable loads listed in this report are based on allowable stress design. The loads in this report are not applicable to Load and Resistance Factor Design.
2. Capacity of wood members is not covered by this report. Capacity of wood members must be checked by the building designer.
3. Allowable loads for more than one direction for a single connection cannot be added together. A design load which can be divided into components in the directions given must be evaluated as follows:
   (Design Uplift/Allowable Uplift) + (Design Lateral Parallel to Plate/Allowable Lateral Parallel to Plate) + (Design Lateral Perp. to Plate/Allowable Lateral Perp. to Plate) < 1.0

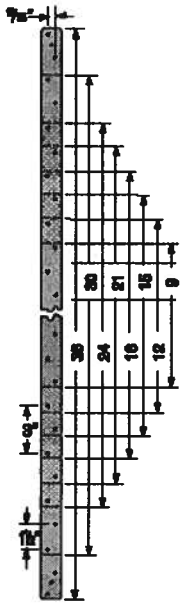
9. ALLOWABLE LOADS:

The pages that follow reference the allowable loads for the aforementioned products.

TABLE 1 ALLOWABLE TENSION LOADS									
Model No.	Ga	DF/SP (133)		DF/SP(160)		SPF (133)		SPF(160)	
		Nails	*Load*	*Nails*	*Load*	*Nails*	*Load*	*Nails*	*Load*
LSTA9	20	8-10d	645	8-10d	775	8-10d	555	8-10d	665
LSTA12		10-10d	805	10-10d	970	10-10d	695	10-10d	830
LSTA15		12-10d	970	12-10d	1160	12-10d	830	12-10d	1000
LSTA18		14-10d	1130	14-10d	1235	14-10d	970	14-10d	1165
LSTA21		16-10d	1235	14-10d	1235	16-10d	1110	16-10d	1235
LSTA24		16-10d	1235	14-10d	1235	18-10d	1235	16-10d	1235
LSTA30	18	22-10d	1640	18-10d	1640	22-10d	1555	20-10d	1640
LSTA36		22-10d	1640	18-10d	1640	24-10d	1640	20-10d	1640
MSTA9		8-10d	650	8-10d	780	8-10d	565	8-10d	680
MSTA12		10-10d	815	10-10d	975	10-10d	705	10-10d	850
MSTA15		12-10d	975	12-10d	1170	12-10d	850	12-10d	1020
MSTA18		14-10d	1140	14-10d	1365	14-10d	990	14-10d	1185
MSTA21		16-10d	1300	16-10d	1560	16-10d	1130	16-10d	1355
MSTA24		18-10d	1465	18-10d	1640	18-10d	1270	18-10d	1525
MSTA30	16	22-10d	1835	22-10d	2050	22-10d	1585	22-10d	1900
MSTA36		26-10d	2050	22-10d	2050	26-10d	1870	24-10d	2050

Notes:

1. Loads do not include a stress increase on the strength of the steel. Where noted, loads include a 33% or 60% load duration increase on the strength of the fasteners in wood. No further increases are permitted. Reduce loads where other loads govern.
2. 10d×1½" nails may be substituted where 10d common nails are specified provided the load is reduced by multiplying table loads by 0.80.
3. Use half of the nails specified in each of the two members being connected.

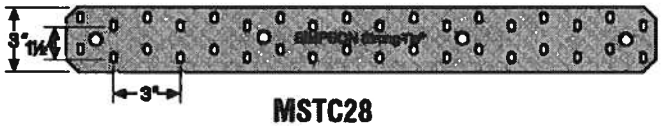


LSTA and MSTA
(Pilot holes not shown)

TABLE 2 ALLOWABLE TENSION LOADS									
Model No.	Ga	DF/SP (133)		DF/SP(160)		SPF (133)		SPF(160)	
		Nails	*Load*	*Nails*	*Load*	*Nails*	*Load*	*Nails*	*Load*
MSTC28	16	36-16d Sinker	3000	36-16d Sinker	3600	36-16d Sinker	2590	36-16d Sinker	3110
MSTC40		52-16d Sinker	4335	48-16d Sinker	4745	52-16d Sinker	3745	52-16d Sinker	4495
MSTC52		58-16d Sinker	4745	48-16d Sinker	4745	64-16d Sinker	4610	56-16d Sinker	4745
MSTC66	14	70-16d Sinker	5860	58-16d Sinker	5860	80-16d Sinker	5860	66-16d Sinker	5860
MSTC78		70-16d Sinker	5860	58-16d Sinker	5860	80-16d Sinker	5860	66-16d Sinker	5860
MST27	12	30-16d	3140	30-16d	3770	30-16d	2740	30-16d	3290
MST37		42-16d	4935	42-16d	5080	42-16d	3835	42-16d	4605
MST48		52-16d	5310	44-16d	5310	54-16d	4930	50-16d	5310
MST60	10	62-16d	6950	58-16d	6950	68-16d	6800	58-16d	6950
MST72		62-16d	6950	52-16d	6950	70-16d	6950	58-16d	6950
LSTI49	18	32-10d×1½	2580	32-10d×1½	3100	32-10d×1½	2220	32-10d×1½	2660
LSTI73		48-10d×1½	3870	44-10d×1½	4210	48-10d×1½	3330	48-10d×1½	3995
MSTI26	12	26-10d×1½	2355	26-10d×1½	2830	26-10d×1½	2045	26-10d×1½	2455
MSTI36		36-10d×1½	3265	36-10d×1½	3915	36-10d×1½	2830	36-10d×1½	3400
MSTI48		48-10d×1½	4350	48-10d×1½	5080	48-10d×1½	3775	48-10d×1½	4530
MSTI60		58-10d×1½	5080	48-10d×1½	5080	60-10d×1½	4720	54-10d×1½	5080
MSTI72		58-10d×1½	5080	48-10d×1½	5080	66-10d×1½	5080	54-10d×1½	5080

Notes:

1. Loads do not include a stress increase on the strength of the steel. Where noted, loads include a 33% or 60% load duration increase on the strength of the fasteners in wood. No further increases are permitted. Reduce loads where other loads govern.
2. 10d×1½" nails may be substituted where 16d sinker nails are specified provided the load is reduced by multiplying table loads by 0.80. 10d common nails may be substituted where 16d sinker nails are specified with no load reduction. 10d common or 16d sinker nails may be substituted where 16d common nails are specified provided the load is reduced by multiplying table loads by 0.85.
3. Use half of the nails specified in each of the two members being connected.



MSTC28

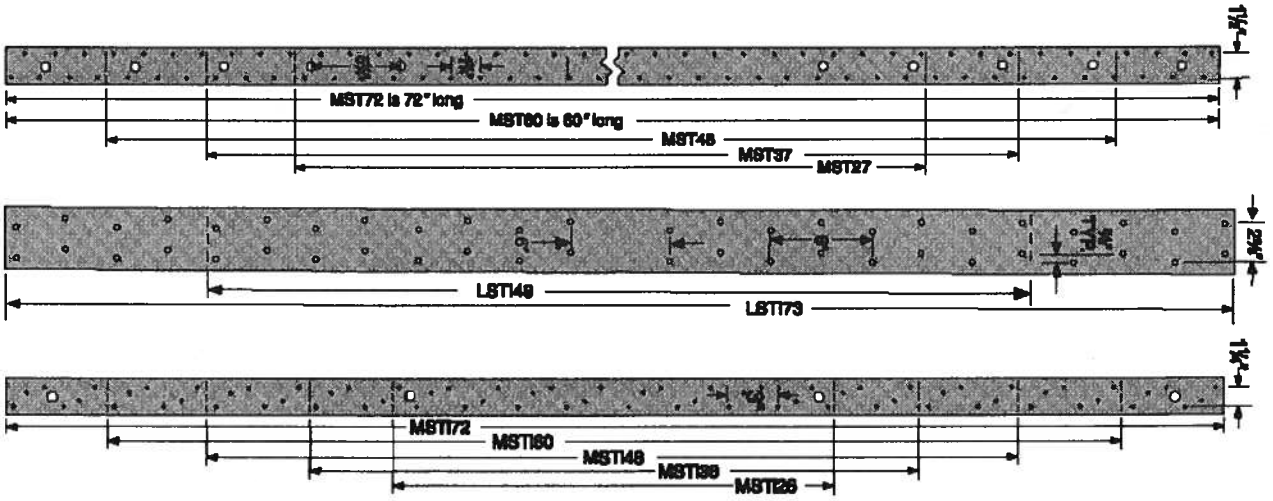


TABLE 3 ALLOWABLE UPLIFT LOADS FLOOR TO FLOOR CLEAR SPAN									
Model No.	Clear Span	Douglas Fir/Southern Pine				Spruce-Pine-Fir			
		133		160		133		160	
MST37 Clear Span	18"	20-16d	2095	20-16d	2510	20-16d	1825	20-16d	2190
	16"	22-16d	2305	22-16d	2765	22-16d	2010	22-16d	2410
MST48 Clear Span	18"	32-16d	3370	30-16d	3695	32-16d	2925	32-16d	3505
	16"	34-16d	3580	30-16d	3695	34-16d	3105	34-16d	3695
MST60 Clear Span	18"	44-16d	4830	36-16d	4830	46-16d	4600	42-16d	4830
	16"	44-16d	4830	36-16d	4830	48-16d	4800	42-16d	4830
MST72 Clear Span	18"	56-16d	6420	52-16d	6945	56-16d	5600	56-16d	6720
	16"	56-16d	6420	52-16d	6945	56-16d	5600	56-16d	6720
MSTI36 Clear Span	18"	14-10d×1½	1270	14-10d×1½	1525	14-10d×1½	1100	14-10d×1½	1320
	16"	16-10d×1½	1450	16-10d×1½	1740	16-10d×1½	1260	16-10d×1½	1510
MSTI48 Clear Span	18"	26-10d×1½	2355	26-10d×1½	2830	26-10d×1½	2045	26-10d×1½	2455
	16"	28-10d×1½	2540	28-10d×1½	3045	28-10d×1½	2205	28-10d×1½	2645
MSTI60 Clear Span	18"	38-10d×1½	3445	38-10d×1½	4135	38-10d×1½	2990	38-10d×1½	3585
	16"	40-10d×1½	3625	40-10d×1½	4350	40-10d×1½	3145	40-10d×1½	3775
MSTI72 Clear Span	18"	50-10d×1½	4535	48-10d×1½	5080	50-10d×1½	3935	50-10d×1½	4720
	16"	52-10d×1½	4715	48-10d×1½	5080	52-10d×1½	4090	52-10d×1½	4910

Notes:

1. Loads do not include a stress increase on the strength of the steel. Where noted, loads include a 33% or 60% load duration increase on the strength of the fasteners in wood. No further increases are permitted. Reduce loads where other loads govern.
2. 10d×1½" nails may be substituted where 16d sinker nails are specified provided the load is reduced by multiplying table loads by 0.77. 10d common nails may be substituted where 16d sinker nails are specified with no load reduction. 10d common or 16d sinker nails may be substituted where 16d common nails are specified provided the load is reduced by multiplying table loads by 0.85.
3. Use half of the nails specified in each of the two members being connected.

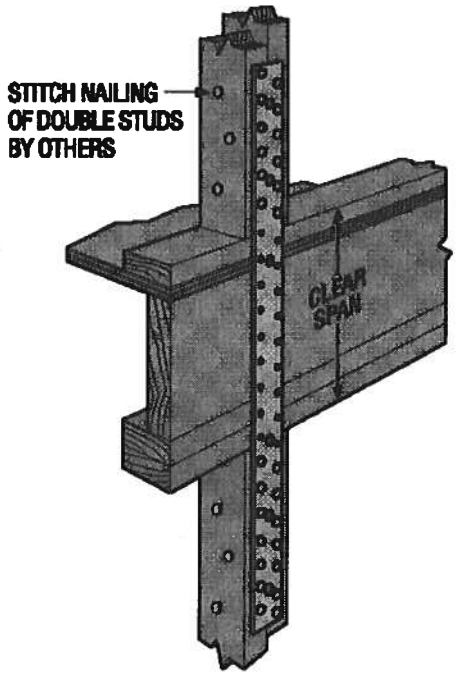
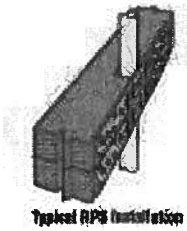
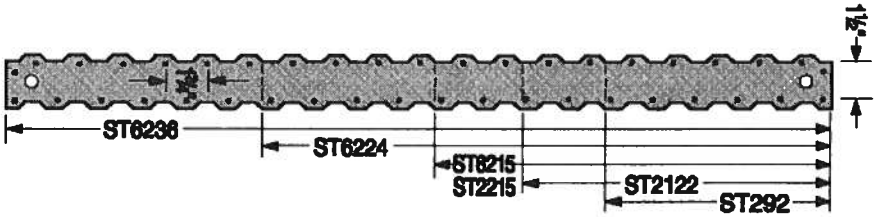


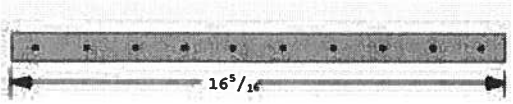
TABLE 4 ALLOWABLE TENSION LOADS									
Model No.	Ga	DF/SP (133)		DF/SP(160)		SPF (133)		SPF(160)	
		Nails	*Load*	*Nails*	*Load*	*Nails*	*Load*	*Nails*	*Load*
RPS18	16	12-16d	1150	12-16d	1380	12-16d	990	12-16d	1190
RPS22		12-16d	1150	12-16d	1380	12-16d	990	12-16d	1190
		16-16d	1535	16-16d	1845	16-16d	1325	16-16d	1585
RPS28		12-16d	1150	12-16d	1380	12-16d	990	12-16d	1190
		16-16d	1535	16-16d	1785	16-16d	1325	16-16d	1585
ST292	20	12-16d	1120	12-16d	1265	12-16d	970	12-16d	1160
ST2122		16-16d	1505	14-16d	1530	16-16d	1290	16-16d	1530
ST2115		8-16d	660	6-16d	660	10-16d	660	8-16d	660
ST2215		20-16d	1875	18-16d	1875	20-16d	1625	20-16d	1875
ST6215	16	20-16d	1895	20-16d	2095	20-16d	1640	20-16d	1970
ST6224		28-16d	2540	24-16d	2540	28-16d	2315	26-16d	2540
ST9		8-16d	755	8-16d	910	8-16d	655	8-16d	785
ST12		10-16d	945	10-16d	1135	10-16d	820	10-16d	985
ST18		14-16d	1325	14-16d	1420	14-16d	1150	14-16d	1380
ST22		16-16d	1420	14-16d	1420	18-16d	1420	16-16d	1420
ST6236	14	40-16d	3845	34-16d	3845	40-16d	3465	38-16d	3845
FHA6	12	8-16d	810	8-16d	975	8-16d	705	8-16d	845
FHA9		8-16d	810	8-16d	975	8-16d	705	8-16d	845
FHA12		8-16d	810	8-16d	975	8-16d	705	8-16d	845
FHA18		8-16d	810	8-16d	975	8-16d	705	8-16d	845
FHA24		8-16d	810	8-16d	975	8-16d	705	8-16d	845
FHA30		8-16d	810	8-16d	975	8-16d	705	8-16d	845
HRS6		6-10d	525	6-10d	630	6-10d	455	6-10d	545
HRS8		10-10d	875	10-10d	1050	10-10d	760	10-10d	910
HRS12		14-10d	1225	14-10d	1465	14-10d	1065	14-10d	1275

Notes:

1. Loads do not include a stress increase on the strength of the steel. Where noted, loads include a 33% or 60% load duration increase on the strength of the fasteners in wood. No further increases are permitted. Reduce loads where other loads govern.
2. 10d common or 16d sinker nails may be substituted where 16d common nails are specified provided the load is reduced by multiplying table loads by 0.85.
3. Use half of the nails specified in each of the two members being connected.







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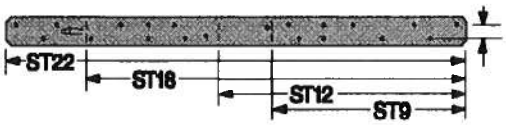


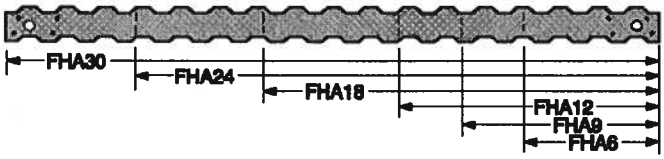
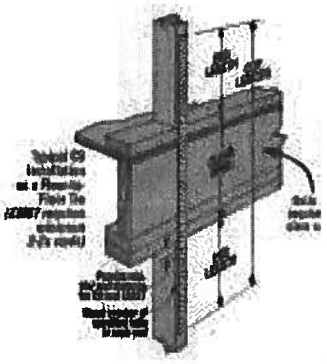


TABLE 5
ALLOWABLE TENSION LOAD AND NUMBER OF NAILS REQUIRED

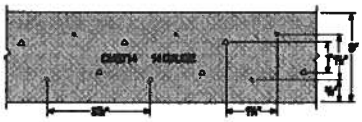
Model No.	Allowable Load	Req'd # of Nails DF/SP		Req'd # of Nails SPF	
		133	*160*	*133*	*160*
CMST12	9235	86-16d	72-16d	100-16d	82-16d
		100-10d	82-10d	114-10d	94-10d
CMST14	6490	66-16d	56-16d	76-16d	64-16d
		76-10d	64-10d	88-10d	74-10d
CMSTC16	4745	58-16d Sinkers	48- 16d Sinkers	66- 16d Sinkers	56- 16d Sinkers
CS16	1705	22-10d	18-10d	26-10d	22-10d
		26-8d	22-8d	30-8d	26-8d
CS18	1370	18-10d	16-10d	20-10d	18-10d
		22-8d	18-8d	26-8d	22-8d
CS20	1030	14-10d	12-10d	16-10d	14-10d
		18-8d	14-8d	20-8d	16-8d
CS22	845	12-10d	10-10d	14-10d	12-10d
		14-8d	12-8d	16-8d	14-8d



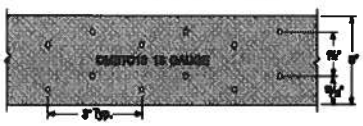
Notes:

1. Loads do not include a stress increase on the strength of the steel. Where noted, loads include a 33% or 60% load duration increase on the strength of the fasteners in wood. No further increases are permitted. Reduce loads where other loads govern.
2. 16d sinker nails may be substituted for 10d common nails with no load reduction.
3. Use half of the nails specified in each of the two members being connected.
4. Provide a minimum of 1⅛" end distance from the last nail to the end of the wood member.

CS16 Hole Pattern *(all other CS straps similar)*



CMST14 Hole Pattern *(CMST12 similar)*

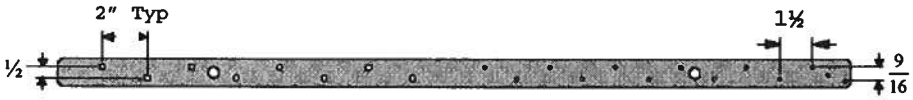


CMSTC16 Hole Pattern

TABLE 6 ALLOWABLE TENSION LOADS									
Model No.	Ga	DF/SP (133)		DF/SP (160)		SPF (133)		SPF(160)	
		Nails	*Load*	*Nails*	*Load*	*Nails*	*Load*	*Nails*	*Load*
MSTAM24	18	9-10d	1465	8-10d	1500	9-10d	1270	9-10d	1500
MSTAM36	16	12-10d	1870	10-10d	1870	13-10d	1870	11-10d	1870
MSTCM40	16	26-16d Sinker	4250	22-16d Sinker	4250	26-16d Sinker	3745	25-16d Sinker	4250

Notes:

1. Loads do not include a stress increase on the strength of the steel. Where noted, loads include a 33% or 60% load duration increase on the strength of the fasteners in wood. No further increases are permitted. Reduce loads where other loads govern.
2. Minimum edge distance is 1½" for Titen Masonry Screws.



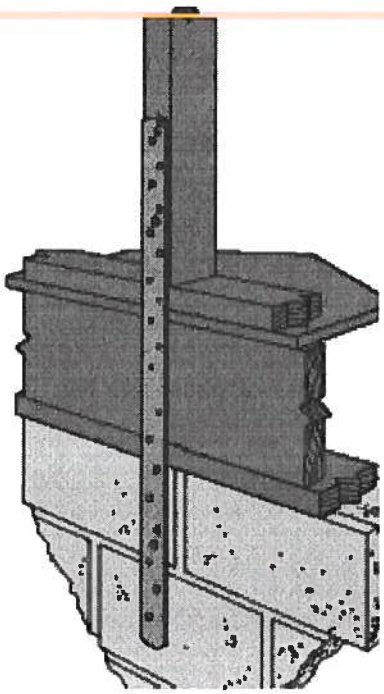
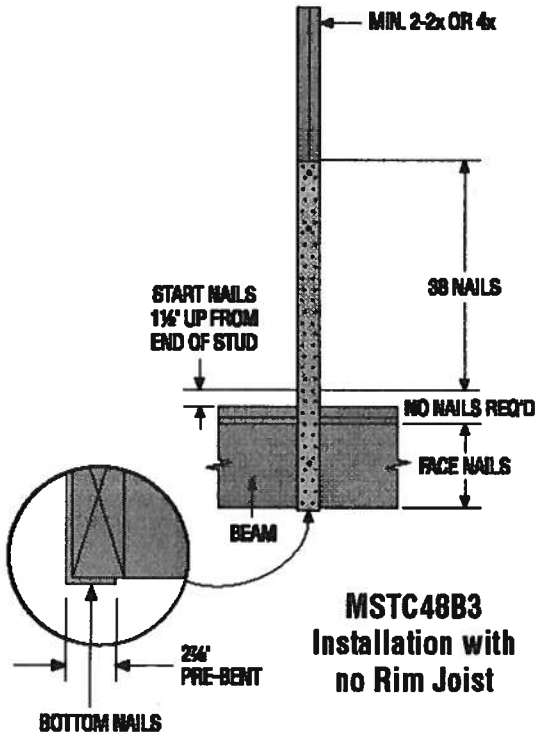


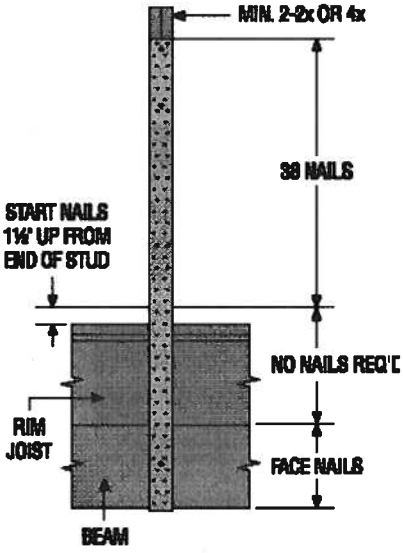
TABLE 8 ALLOWABLE UPLIFT/TENSION LOADS							
Model No.	Beam Minimum Dimensions		Fasteners			Allowable Tension Loads (133/160)	
			Beam		Studs/ Post		
	Width	Depth	Face	Bottom		DF/SP	SPF
MSTC48B3	3"	9¼"	12-10d	4-10d	38-10d	3930	3380
MSTC66B3	3½"	11¼"	14-10d			4440	3820

Notes:

1. Using fewer than 38 nails in the studs/post will reduce the capacity of the connection. To calculate a reduced capacity use 129 lbs. per nail for DFL/SP or 112 lbs. per nail for SPF
2. Nails in studs/post shall be installed symmetrically. Nails may be installed over the entire length of the strap over the studs/post.
3. The 3" wide beam may be double 2-by members.
4. MSTC48B3 and MSTC66B3 installed over sheathing up to ½" thick will achieve 0.85 of the table loads.



MSTC48B3 Installation with no Rim Joist



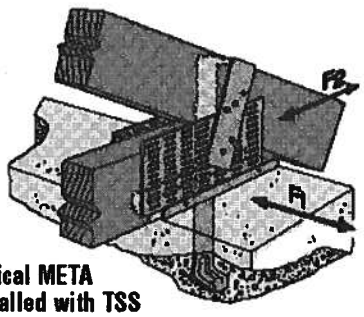
MSTC66B3 Installation with Rim Joist

TABLE 9 ALLOWABLE LOADS

Model No.	Ga	H	Fasteners and Uplift								Lateral Loads	
			160 Load Duration Increase				133 Load Duration Increase				133/160	
			1 Ply So. Pine Truss		2 or 3 Ply So. Pine Truss		1 Ply So. Pine Truss		2 or 3 Ply So. Pine Truss		F_1 (parallel to wall)	F_2 (perpen. to wall)
			Fasteners	Load	Fasteners	Load	Fasteners	Load	Fasteners	Load		
META12	18	8	7-10d×1½	1450	6-16d	1450	7-10d×1½	1240	7-16d	1450	280	725
META14		10	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META16		12	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META18		14	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META20		16	6-10d×1½	1270	5-16d	1245	8-10d×1½	1415	6-16d	1250	280	725
			7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META22		18	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META24		20	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META40		36	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
HETA12	16	8	7-10d×1½	1520	7-16d	1780	7-10d×1½	1265	7-16d	1475	280	725
HETA16		12	9-10d×1½	1810	8-16d	1810	10-10d×1½	1810	9-16d	1810	280	725
HETA20		16	8-10d×1½	1735	7-16d	1780	9-10d×1½	1630	8-16d	1690	280	725
			9-10d×1½	1810	8-16d	1810	10-10d×1½	1810	9-16d	1810	280	725
HETA24		20	9-10d×1½	1810	8-16d	1810	10-10d×1½	1810	9-16d	1810	280	725
HETA40		36	9-10d×1½	1810	8-16d	1810	10-10d×1½	1810	9-16d	1810	280	725
HHETA12	14	8	7-10d×1½	1565	7-16d	1820	7-10d×1½	1305	7-16d	1520	435	815
HHETA16		12	10-10d×1½	2235	9-16d	2235	12-10d×1½	2235	11-16d	2235	435	815
HHETA20		16	9-10d×1½	2010	8-16d	2080	11-10d×1½	2050	10-16d	2170	435	815
			10-10d×1½	2235	9-16d	2235	12-10d×1½	2235	11-16d	2235	435	815
HHETA24		20	10-10d×1½	2235	9-16d	2235	12-10d×1½	2235	11-16d	2235	435	815
HHETA40		36	10-10d×1½	2235	9-16d	2235	12-10d×1½	2235	11-16d	2235	435	815
HETAL12	16	7	10-10d×1½	1085	10-16d	1270	10-10d×1½	905	10-16d	1055	415	1100
HETAL16		11	14-10d×1½	1810	13-16d	1810	15-10d×1½	1810	14-16d	1810	415	1100
HETAL20		15	14-10d×1½	1810	13-16d	1810	15-10d×1½	1810	14-16d	1810	415	1100

Notes:

1. Loads do not include a stress increase on the strength of the steel. No further increases are permitted. Reduce loads where other loads govern.
2. Five nails must be installed into the truss seat of the HETAL
3. Parallel-to-plate load towards face of HETAL is 1975 lbs.
4. Except for HETAL straps, lateral loads are based on a minimum installation of 12 nails and the strap wrapped over the heel
5. Minimum f'c is 2,000psi
6. It is acceptable to use a reduced number of fasteners in a product provided that there is a reduction in load capacity. The load per nail can be approximated by dividing the allowable load by the number of fasteners. This concept applies to all member sizes. There should be a minimum of 4 nails installed in the strap.

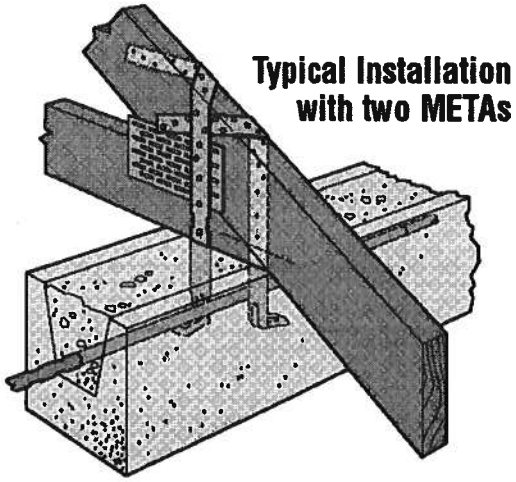


Typical META Installed with TSS

TABLE 10 ALLOWABLE LOADS FOR DOUBLE EMBEDDED TRUSS ANCHORS										
Double Embedded Anchor Installation Into Grouted CMU Bond Beam										
Model No.	Uplift - 160 Load Duration Increase				Uplift - 133 Load Duration Increase				Lateral Loads 133/160	
	1 Ply Southern Pine Truss		2 or 3 Ply Southern Pine Truss		1 Ply Southern Pine Truss		2 or 3 Ply Southern Pine Truss		F_1 (parallel to wall)	F_2 (perpen. to wall)
	Fasteners	Load	Fasteners	Load	Fasteners	Load	Fasteners	Load		
META	10-10d×1½	1985	14-16d	1900	12-10d×1½	1985	14-16d	1900	1210	1160
HETA	10-10d×1½	2035	12-16d	2500	12-10d×1½	2035	14-16d	2500	1225	1520
HHETA	10-10d×1½	2035	12-16d	2500	12-10d×1½	2035	14-16d	2500	1225	1520

Notes:

1. Minimum f'c is 2,500psi.
2. Install with spoons facing outward and spaced no more than 1/8" wider than the truss width.
3. Install half of the required number of fasteners in each strap.
4. For uplift loads for poured concrete tie beam applications with 2 or 3 ply trusses, increase the META load by 35%, the HETA load by 8%, and the HHETA load by 34%. Listed lateral loads apply to concrete applications.
5. Lateral loads apply only to anchors spaced a minimum of 3" apart.



Typical Installation with two METAs

10. CODE REFERENCES

Florida Building Code 2001 Edition

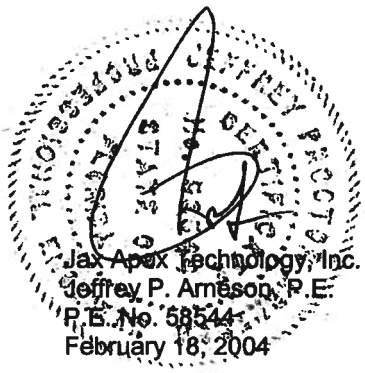
Section 103.7	Alternate Materials and Methods
Chapter 1703	Structural Tests and Inspections
Chapter 21	Masonry
Chapter 22	Steel
Chapter 23	Wood

11. IDENTIFICATION

Each connector covered by this report shall be stamped with the manufacturer's name and/or trademark and the product name.

12. PERIOD OF ISSUANCE

The content of this report expires on March 1st, 2006. For information on this report, contact Apex Technolgy. 904/821-5200



Jax Apex Technology, Inc.
Jeffrey P. Arneson, P.E.
P.E. No. 58544
February 18, 2004

Product Approval

USER: Public User

FL #	FL5190
Application Type	New
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	Wheeling Corrugating Company
Address/Phone/Email	1134 Market Street Wheeling, WV 26003
Authorized Signature	James L. Buckner, P.E. jimmy@cbuckinc.net
Technical Representative	David W. Boltz
Address/Phone/Email	1134 Market Street Wheeling, WV 26003 boltzdw@wpsc.com
Quality Assurance Representative	
Address/Phone/Email	
Category	Roofing
Subcategory	Metal Roofing
Compliance Method	Evaluation Report from a Florida Regis Licensed Florida Professional Engineer ☑ Evaluation Report - Hardcopy Rece
Florida Engineer or Architect Name who developed the Evaluation Report	James L. Buckner

Florida License	PE-31242
Quality Assurance Entity	Underwriters Laboratories Inc.
Validated By	Warren W. Schaefer, P.E.
Certificate of Independence	
Referenced Standard and Year (of Standard)	**Standard** UL 580 with 1998 Revisions
Equivalence of Product Standards Certified By	
Sections from the Code	1507.4
Product Approval Method	Method 1 Option D
Date Submitted	09/01/2005
Date Validated	09/23/2005
Date Pending FBC Approval	10/01/2005
Date Approved	10/11/2005

Summary of Products

FL #	Model, Number or Name	Description
5190.1	1- "5-V"	Minimum 29 Gauge Steel, Maximu Attached to Wood Deck
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure = -52.5 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.		**Installation Instructions** Verified By: **Evaluation Reports** PTID_5190_T_1-5V_24in_29GaSteelOnWood_EVA PTID_5190_T_2-5V_24in_26GaSteelOnWood_EVA PTID_5190_T_3-CenturyDrain_36in_29GaSteelOn' PTID_5190_T_4-CenturyDrain_36in_26GaSteelOn' PTID_5190_T_5-RPanel_36in_29GaSteelOnWood_ PTID_5190_T_6-RPanel_36in_26GaSteelOnWood_ PTID_5190_T_7-LocSeam_16in_26GaSteelOnWoo PTID_5190_T_8-LocSeam_12in_26GaSteelOnWoo PTID_5190_T_NS-CertOfIndepAr

5190.2	2- "5-V"	Minimum 26 Gauge Steel, Panel Attached to Wood D
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure = -90 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.		**Installation Instruction** Verified By: **Evaluation Reports**
5190.3	3- "Centurydrain"	Minimum 29 Gauge Steel, Panel Attached to Wood D
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure = -52.5 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.		**Installation Instruction** Verified By: **Evaluation Reports**
5190.4	4- "Centurydrain"	Minimum 26 Gauge Steel, Panel Attached to Wood D
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure = -70 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.		**Installation Instruction** Verified By: **Evaluation Reports**
5190.5	5- "R-Panel"	Minimum 29 Gauge Steel, Panel Attached to Wood D

Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure = -52.5 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.	**Installation Instruction** Verified By: **Evaluation Reports**

5190.6	6- "R-Panel"	Minimum 26 Gauge Steel, Panel Attached to Wood D

Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure = -87.5 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.	**Installation Instruction** Verified By: **Evaluation Reports**

5190.7	7- "Loc-Seam"	Minimum 26 Gauge Steel, Attached to Wood Deck

Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure = -52.5 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.	**Installation Instruction** Verified By: **Evaluation Reports**

5190.8	8- "Loc-Seam"	Minimum 26 Gauge Steel, Panel Attached to Wood D

Limits of Use (See Other) **Approved for use in HVHZ:**	**Installation Instruction** Verified By:

Approved for use outside HVHZ: **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure = -70 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.	**Evaluation Reports**

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Product Approval Accepts:

Product Approval

USER: Public User

FL #	FL5984
Application Type	New
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	Wheeling Corrugating Company
Address/Phone/Email	1134 Market Street Wheeling, WV 26003
Authorized Signature	James L. Buckner, P.E. jimmy@cbuckinc.net
Technical Representative	David W. Boltz
Address/Phone/Email	1134 Market Street Wheeling, WV 26003 boltzdw@wpsc.com
Quality Assurance Representative	
Address/Phone/Email	
Category	Roofing
Subcategory	Metal Roofing
Compliance Method	Evaluation Report from a Florida Regis Licensed Florida Professional Engineer ☑ Evaluation Report - Hardcopy Rece
Florida Engineer or Architect Name who developed the Evaluation Report	James L. Buckner

Florida License	PE-31242
Quality Assurance Entity	Underwriters Laboratories Inc.
Validated By	Daniel G. Farabaugh
Certificate of Independence	
Referenced Standard and Year (of Standard)	**Standard** UL580 with 1998 Revisions
Equivalence of Product Standards Certified By	
Sections from the Code	1507.4
Product Approval Method	Method 1 Option D
Date Submitted	12/19/2005
Date Validated	01/04/2006
Date Pending FBC Approval	01/09/2006
Date Approved	02/07/2006

Summary of Products

FL #	Model, Number or Name	Description
5984.1	1- "5-V"	Minimum 26 Gauge Steel, Maximu Coverage) Panel Attached to Woo Flats)
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure with Screw Rows 6" o.c. = 108.5 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105 or 106. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.		**Installation Instructions** Verified By: **Evaluation Reports** PTID_5984_T_1-5V_24in_26GaSteelOnWood_EVA PTID_5984_T_2-CenturyDrain_36in_26GaSteelOn' PTID_5984_T_3-RPanel_36in_26GaSteelOnWood_ PTID_5984_T_CertOfIndepAndQ/

5984.2	2- "Centurydrain"	Minimum 26 Gauge Steel, (Net Coverage) Panel Atta
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure with Screw Rows 12" o.c. = 93.5 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105 or 106. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.		**Installation Instruction** Verified By: **Evaluation Reports**
5984.3	3- "R-Panel"	Minimum 26 Gauge Steel, (Net Coverage) Panel Atta
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Design Uplift Pressure with Screw Rows 12" o.c. = 130.5 psf. The required design wind loads shall be determined for each project. The maximum fastener spacing listed herein shall not be exceeded. All rational analysis computations shall be prepared by a qualified design professional, as required by Florida Building Code, Section 105 or 106. This product is not approved for use in the High Velocity Hurricane Zone. Refer to Evaluation Report.		**Installation Instruction** Verified By: **Evaluation Reports**

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Product Approval Accepts:


Bobby
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Product Approval

USER: Public User

Product Approval Menu > Product or Application Search > Application List > **Application Detail**

FL #	FL4334-R1
Application Type	Revision
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	Masonite International
Address/Phone/Email	One North Dale Mabry Suite 950 Tampa, FL 33609 (615) 441-4258 sschreiber@masonite.com
Authorized Signature	Steve Schreiber sschreiber@masonite.com
Technical Representative Address/Phone/Email	
Quality Assurance Representative Address/Phone/Email	
Category	Exterior Doors
Subcategory	Swinging Exterior Door Assemblies
Compliance Method	Certification Mark or Listing
Certification Agency	National Accreditation & Management

Referenced Standard and Year (of Standard)	**Standard** ASTM E1300 ASTM E1300 TAS 201 TAS 202 TAS203
Equivalence of Product Standards Certified By	
Sections from the Code	Section 2612 HVHZ Pl
Product Approval Method	Method 1 Option A
Date Submitted	09/08/2005
Date Validated	10/16/2005
Date Pending FBC Approval	10/01/2005
Date Approved	10/19/2005

Summary of Products

FL #	Model, Number or Name	Description
4334.1	Fiberglass Side-hinged Door Units - Impact Rated	6'-8" Opaque I/S and O/S
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 3'-0" x 6'-8" max nominal size Max DP = +/- 70.0 When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0120-05 for additional information.		**Certification Agency Ce** **Installation Instruction** PTID_4334_R1_I__68 Gl PTID_4334_R1_I__68 Op PTID_4334_R1_I__80 Gl PTID_4334_R1_I__80 Op Verified By:
4334.2	Fiberglass Side-hinged Door Units - Impact Rated	8'-0" Opaque I/S and O/S
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:**		**Certification Agency Ce** **Installation Instruction** Verified By:

Design Pressure: +/- **Other:** Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 3'-0" x 8'-0" max nominal size Max DP = +/- 70.0 When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0121-05 for additional information.		
4334.3	Fiberglass Side-hinged Door Units - Impact Rated	8'-0" Opaque O/S Door w/
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 8'-0" max nominal size Max DP = +55.0 / -50.5 When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0121-05 for additional information.		**Certification Agency Ce** **Installation Instruction** Verified By:
4334.4	Fiberglass Side-hinged Door Units - Impact Rated	8'-0" Opaque I/S Door w/
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 8'-0" max nominal size Max DP = +50.5 / -50.5 When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0121-05 for additional information.		**Certification Agency Ce** **Installation Instruction** Verified By:

4334.5	Fiberglass Side-hinged Door Units - Impact Rated	6'-8" Opaque O/S Door w/
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 6'-8" max nominal size Max DP = +55.0 / -50.5 When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0120-05 for additional information.		**Certification Agency Ce** **Installation Instruction** Verified By:
4334.6	Fiberglass Side-hinged Door Units - Impact Rated	6'-8" Opaque I/S Door w/
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 6'-8" max nominal size Max DP = +50.5 / -50.5 When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0120-05 for additional information.		**Certification Agency Ce** **Installation Instruction** Verified By:
4334.7	Fiberglass Side-hinged Door Units - Impact Rated	8'-0" Glazed I/S and O/S Sidelites
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other		**Certification Agency Ce** **Installation Instruction** Verified By:

Structures, does not exceed the design pressures listed. 12'-0" x 8'-0" max nominal size Max DP = +50.0 / -50.0 When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0123-05 for additional information.		
4334.8	Fiberglass Side-hinged Door Units - Impact Rated	6'-8" Glazed I/S and O/S Sidelites
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 6'-8" max nominal size Max DP = +60.0 / -60.0 When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0122-05 for additional information.		**Certification Agency Ce** **Installation Instruction** Verified By:

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Codes and Standards
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Tallahassee, Florida 32399-2100
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Product Approval Accepts:

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Product Approval

USER: Public User

Product Approval Menu > Product or Application Search > Application List > **Application Detail**

FL #	FL5438
Application Type	New
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	MI Windows and Doors
Address/Phone/Email	650 W Market St Gratz, PA 17030 (717) 365-3300 ext 2564 bdoyle@mihp.com
Authorized Signature	Brandon Doyle bdoyle@mihp.com
Technical Representative	
Address/Phone/Email	
Quality Assurance Representative	
Address/Phone/Email	
Category	Windows
Subcategory	Single Hung
Compliance Method	Certification Mark or Listing
Certification Agency	American Architectural Manufacturers
Referenced Standard and Year (of	**Standard**

Standard)	ANSI/AAMA/NWWDA 101/I.S.2
Equivalence of Product Standards Certified By	
Product Approval Method	Method 1 Option A
Date Submitted	09/22/2005
Date Validated	10/14/2005
Date Pending FBC Approval	10/07/2005
Date Approved	10/17/2005

Summary of Products

Go to Page GO

FL #	Model, Number or Name	Description
5438.1	165 Triple with Continuous Head and Sill	106x72 Insulated SSB An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-20* DP-31.4 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** PTID_5438_I_165 SH Fla Fastener Schedule.pdf PTID_5438_I_650 SH Fla Fastener Schedule.pdf PTID_5438_I_740-744 S - Fastener Schedule.pdf PTID_5438_I_AAMA Cha Windows.pdf PTID_5438_I_Installatior BetterBilt Nail Fin Alum W PTID_5438_I_Installatior BetterBilt Nail Fin Vinyl W PTID_5438_I_Installatior Nail Fin Alum Windows.pd PTID_5438_I_Installatior Nail Fin Vinyl Windows.pd Verified By:
5438.2	165/3000 Fin Frame Oriel	47x89 Insulated 3/16" An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-30 DP-42.7 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.3	165/3000 Fin Frame Oriel	40x90 Insul SSB Annealed

		Fixed
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-35* DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.4	165/3000 Flange Frame Beveled Buck	53x72 Single Glazed 3/16
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-35 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.5	165/3000 Flange Frame Oriel	47x89 Insulated 3/16" An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-25 DP-34.7 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.6	165/3000 Flange Frame Oriel	36x88 Insulated SSB Ann Annealed Fixed
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-35* DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.7	3540 Fin Frame	36x74 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-40* DP-47 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.8	3540 Fin Frame	44x72 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:**		**Certification Agency Ce** **Installation Instruction** Verified By:

Impact Resistant: **Design Pressure:** +/- **Other:** R-40 DP-47.2 Per manufacturers installation instructions.		
5438.9	3540 Fin Frame Triple with Continuous Head and Sill	108x72 Insulated SSB An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-35* DP-50 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.10	4340 Fin Frame	36x62 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-40* DP-55 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.11	4340 Fin Frame	36x60 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-40* DP-55 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.12	4340 Fin Frame	36x74 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-40* DP-47 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.13	4340 Fin Frame	36x72 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-40* DP-50 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.14	455 Fin Frame	48x84 Insulated DSB Ann

Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-50 DP-50 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.15	455 Fin Frame	54x90 Insulated DSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-35 DP-50 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.16	650 Fin Frame	53x90 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-30 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.17	650 Fin Oriel	48x84 Insulated 3/16" An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-35 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.18	650 Flange Frame	48x84 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-35 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.19	650 Flange Frame Oriel	48x84 Insulated 3/16" An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-35 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:

5438.20	740/3740 Fin Frame	52x71 Single Glazed DSB
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-45 DP-45 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:

Go to Page GO!

Back Next

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436
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Product Approval Accepts:

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☐	☑	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☐	☑	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☐	☑	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☐	☑	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
		Elevations including:
☐	☑	a) All sides
☐	☑	b) Roof pitch
☐	☑	c) Overhang dimensions and detail with attic ventilation

d) Location, size and height above roof of chimneys.
e) Location and size of skylights
f) Building height
e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned.
b) Shear walls identified.
c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 **(see attach forms).**
d) Show safety glazing of glass, where required by code.
e) Identify egress windows in bedrooms, and size.
f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, **(Please circle applicable type).**
g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
b) All posts and/or column footing including size and reinforcing
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel.

Roof System:

a) Truss package including:
   1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
   2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including:
   1. Rafter size, species and spacing
   2. Attachment to wall and uplift
   3. Ridge beam sized and valley framing and support details
   4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall
   1. All materials making up wall
   2. Block size and mortar type with size and spacing of reinforcement
   3. Lintel, tie-beam sizes and reinforcement
   4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
   5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
   6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
   7. Fire resistant construction (if required)
   8. Fireproofing requirements
   9. Shoe type of termite treatment (termiticide or alternative method)
   10. Slab on grade
      a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
   11. Indicate where pressure treated wood will be placed
   12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☐ ☑ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. ~~Crawl~~ space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☐ ☑ **Plumbing Fixture layout**

Electrical layout including:

☐ ☑ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☐ ☑ b) Ceiling fans

☐ ☑ c) Smoke detectors

☐ ☑ d) Service panel and sub-panel size and location(s)

☐ ☑ e) Meter location with type of service entrance (overhead or underground)

☐ ☑ f) Appliances and HVAC equipment

☐ ☑ g) Arc Fault Circuits (AFCI) in bedrooms

☐ ☑ h) Exhaust fans in bathroom

HVAC information

☐ ☑ a) Energy Calculations (dimensions shall match plans)

☐ ☑ b) Manual J sizing equipment or equivalent computation

☐ ☐ c) Gas System Type (LP or Natural) Location and BTU demand of equipment

☐ ☐ **Disclosure Statement for Owner Builders**

☐ ☑ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☐ ☐ **Private Potable Water** Existing

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** **If the project is to be located within the city limits of the Town of Fort White**, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. **(386) 497-2321**

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit **($25.00)** must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver **($50.00)**. All culvert waivers are sent to the Columbia County Public Works Department for approval or denial. **If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.**

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

Location:_____ **Project Name:**_____

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	✓		
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung	✓		
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass -through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding	✓		
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles			
2. Underlayments			
3. Roofing Fasteners	✓		
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor	✓		
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof	✓		
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

Contractor or Contractor's Authorized Agent Signature

Print Name

Date

Location

Permit # **(FOR STAFF USE ONLY)**

Columbia County 9-1-1 Addressing / GIS Department
P.O. Box 1787, Lake City, FL 32056
Telephone: (386) 758-1125 * Fax: (386) 758-1365 * E-mail: ron_croft@columbiacountyfla.com

9-1-1 Address Request Form

NOTE: ADDRESS ASSIGNMENT MAY REQUIRE UP TO 10 WORKING DAYS. IF THE ADDRESSING DEPARTMENT NEEDS TO CONDUCT ON SITE GPS LOCATION IDENTIFICATION, ADDITIONAL TIME MAY BE REQUIRED.

Date of Request: _____

Requester Last Name: _____

First Name: _____

Contact Telephone Number: _____

(Cell Phone Number if Provided): _____

Requested for Self: _____ or Requested for Company: _____
(check one)
If Address is Requested by a Company, Provide Name of Requesting Company:

_____

Parcel Identification Number: ____ - ____ - ____ - _____

If in Subdivision, Provide Name Of Subdivision:

_____

Phase or Unit Number (if any): _____ Block Number (if any): _____

Lot Number: _____

Attach Site Plan or you may use back of Request Form for Site Plan:

Requirements for Site Plan Are Listed on Back of Request From:
(NOTE: Site Plan Does NOT have to be a survey or to scale; FURTHER a Environmental Health Dept. Site Plan showing only a 210 by 210 cutout of a property will NOT suffice for Addressing Requirements.)

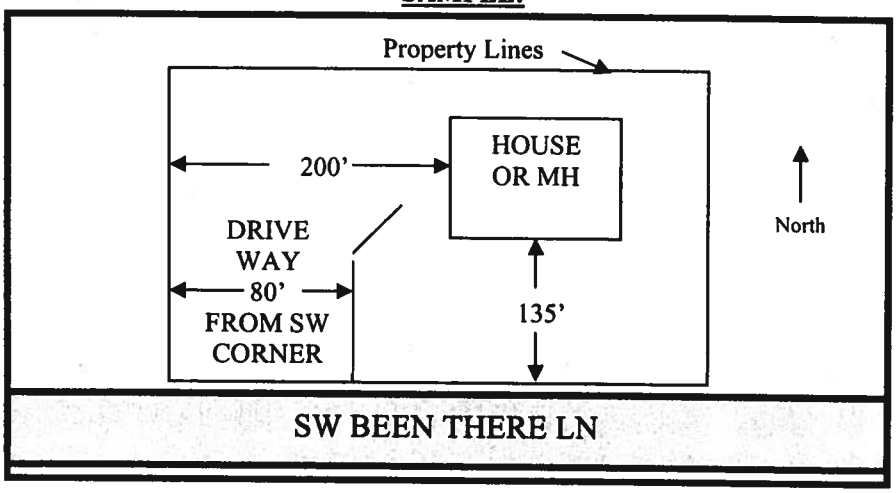
Addressing / GIS Department Use Only:

Date Received: _____ Date Assigned: _____

ID Number: _____

1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

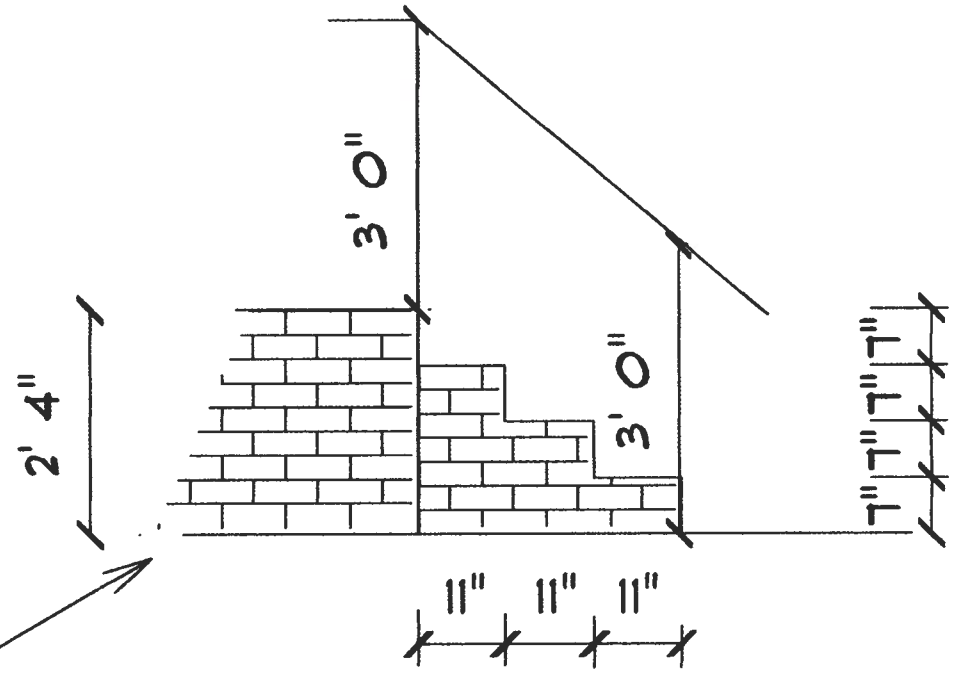
SAMPLE:



SITE PLAN BOX:

ROBIN & LISA ROBERTS
STEP DETAILS
ADDENDUM TO PLANS
DATED 3/06

STEPS TO BE CONCRETE FORMED AND PLACED W/ BRICK FINISH.



PORCH

HANDRAILS: MATERIAL AND DESIGN TO BE SELECTED BY OWNER, SELECTION WILL BE IN ACCORDANCE WITH F.B.C. SECTION 1007.5.3
INSTALLED HANDRAIL SHALL PROJECT HORIZONTIALLY 12" AT 36" ABOVE TOP TREAD AND SHALL CONTINUE TO FOLLOW SLOPE FOR 11" PAST BOTTOM RISER.

24457

Note: Used to obtain FL474; See cover sheet for applicable products

TABLE 2 - ALLOWABLE LOADS FOR BC

MODEL NO.	DIMENSIONS[1]						FASTENERS (EACH SIDE)[2]		ALLOWABLE LOADS[3,5]		
	W_1	W_2	L_1	L_2	H_1	H_2	SURFACE A	SURFACE B	MAX UPLIFT	LATERAL[4] NORMAL	LATERAL[4] MAXIMUM
BC4	3 9/16	3 9/16	2 7/8	2 7/8	3	3	3 - 16d	3 - 16d	980	805	1000
BC4S	3 9/16	5 1/2	4 7/8	2 7/8	3 1/2	2 1/2	6 - 16d	3 - 16d	980	805	1000
BC4R	4	4	4	4	3	3	6 - 16d	6 - 16d	980	805	1000
BC6	5 1/2	5 1/2	4 3/8	4 3/8	3 3/8	3 3/8	6 - 16d	6 - 16d	1330	1610	2000
BC6R	6	6	6	6	3	3	6 - 16d	6 - 16d	1330	1610	2000
BC8	7 1/2	7 1/2	7 1/2	7 1/2	4	4	6 - 16d	6 - 16d	1800	1610	2000
BC8R	8	8	8	8	4	4	6 - 16d	6 - 16d	1800	1610	2000

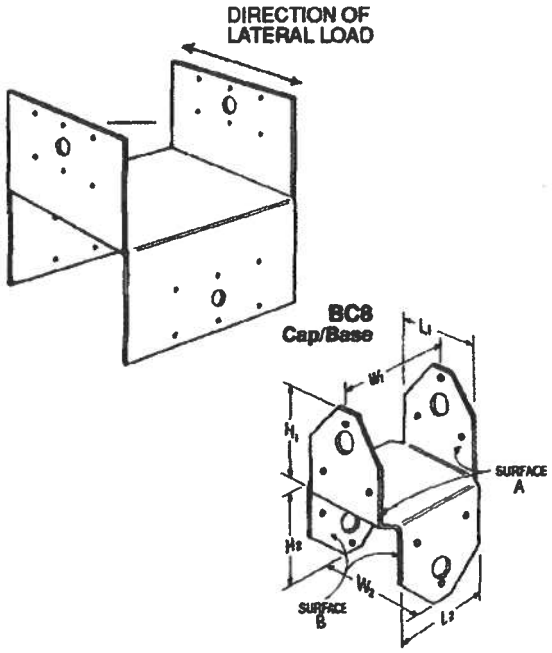
[1] Dimensions are in inches.

[2] Fasteners are the minimum number of common nails required.

[3] Design loads are in pounds.

[4] Normal loads have $C_D = 1$ and are adjusted using the design loads C_D up to the maximum load.

[5] Loads apply to solid wood members with G of 0.5 and greater.



Post Cap to Header

24457

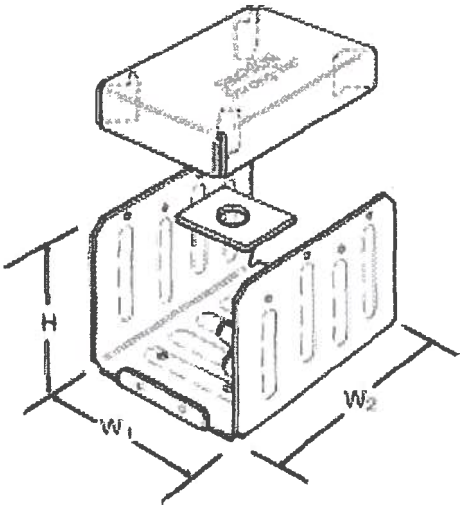
Note: Used to obtain FL474; See cover sheet for applicable products

TABLE 1 - ABE46 AND ABE66 Adjustable post base (lbs)[1,2,3]

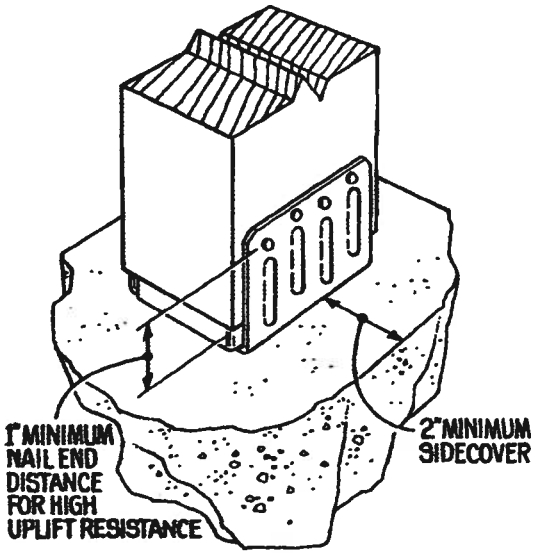
MODEL NO.	NOMINAL POST SIZE	MATERIAL		DIMENSIONS			FASTENERS		ALLOWABLE LOADS	
		BASE	STRAP	W_1	W_2	L	ANCHOR DIA	POST NAILS[4]	UPLIFT (133%)	DOWN (100%)
ABE46	4 x 6	12 ga.	16 ga.	$3^9/_{16}$	$5^3/_8$	$5^7/_{16}$	$^5/_8$	8 - 16d	810	8800
ABE66	6 x 6	12 ga.	14 ga.	5½	5½	$5^7/_{16}$	$^5/_8$	8 - 16d	900	16,665

SI: 1 inch = 25.4 mm, 1 lbf = 4.5 N

1. Down loads shall not be increased for short-term loading.
2. Uplift loads have been increased 33% for wind or earthquake loading with no further increase allowed. Loads shall be reduced by 33% for normal loading such as in cantilever construction.
3. Minimum average compressive strength of concrete shall be 2000 psi.
4. Nails are 16d common - 0.162 dia x 3½" long.



ABE 46



Typical ABE Installation

Post base to Slab

24457

2) TMD.

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF: Columbia

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement:

1. Description of Property: Long Legal — 6864 NW Lake Jeffrey Road — Northwest Columbia County, Lake City, Fl 32055

2. General Description of Improvements: Residential Construction

3. Name and Address of Owner: Robin H. Roberts & Lisa K. Roberts, 6864 NW Lake Jeffrey Road, Lake City, Fl 32055

   Interest in Property: Fee Simple

   Name and Address of Fee Simple Titleholder (if other than owner): N/A

4. Name and Address of Contractor: F.O.G. Contracting, Inc., 177 NW Bulthute Court, Lake City, Fl 32055

5. Name and Address of Surety on payment bond, if any, and amount of such bond: N/A

   Amount of Bond: $0

6. Name and Address of Lender:

   MERCANTILE BANK
   425 22nd Avenue North
   St. Petersburg, FL 33704

   Attention: Sara Lopez

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Sharon Feagle, Deputy Clerk
Date 05-02-2006

CLERK CIRCUIT COURT COLUMBIA COUNTY, FLORIDA

Inst:2006010597 Date:05/02/2006 Time:13:51
DC,P.DeWitt Cason,Columbia County B:1082 P:1024

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13 (1)(b), Florida Statutes:

   MERCANTILE BANK
   425 22nd Avenue North
   St. Petersburg, FL 33704

   Attention: Sara Lopez

8. In addition to himself, Owner designates _____ of _____ to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

OWNER: Robin H. Roberts & Lisa K. Roberts

STATE OF FLORIDA
COUNTY OF: Columbia

I HEREBY CERTIFY that before me personally appeared Robin H. Roberts and Lisa KB Roberts to me personally known or who has produced Drivers License as identification, known to me to be the person described in and who executed the foregoing instrument, and severally acknowledged the execution thereof to be his free act and deed, for the uses and purposes therein expressed.

WITNESS my hand and official seal at Lake City, said County and State, this 26rh day of April, 2005.

Notary Public
Print Name: _____

My Commission Expires:

TERRY MCDAVID
MY COMMISSION # DD 500788
EXPIRES: January 16, 2010
Bonded Thru Notary Public Underwriters

EXHIBIT "A"

COMMENCE AT THE SOUTHEAST CORNER OF SECTION 5, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA, AND RUN NORTH ALONG THE EAST LINE OF SAID SECTION 5, A DISTANCE OF 360 FEET FOR A POINT OF BEGINNING; AND RUN THENCE NORTH ALONG SAID SECTION LINE, 354 FEET; THENCE RUN IN A WESTERLY DIRECTION 160 FEET; THENCE RUN SOUTH 21 DEG. 51 MIN. WEST, 386 FEET, MORE OR LESS; THENCE RUN IN AN EASTERLY DIRECTION 284 FEET, MORE OR LESS, TO THE POINT OF BEGINNING.

Inst:2006010597 Date:05/02/2006 Time:13:51
_____DC,P.DeWitt Cason,Columbia County B:1082 P:1025

24457

ASPEN
PEST CONTROL, INC.

Lake City (386) 755-3611
Gainesville (352) 494-5751
Fax (386) 755-3885
Toll Free 1-800-616-4707

Certificate of Compliance for Termite Protection

(as required by Florida Building Code (FBC) 1816.1.7)

Aspen Pest Control, Inc.
(386) 755-3611
State License # - JB109476
State Certification # - JF104376

6854 NW Lake Jeffery Rd. Lake City, Fl. (Roberts)
address of Treatment or Lot/Block of Treatment

Soil Barrier

Method of Termite Prevention Treatment – Soil Barrier, Wood Treatment, Bait System, Other

Horizontal, Vertical, Void and Exterior Treatment

Description of Treatment

The above named structure has received a complete treatment for the prevention of subterranean termites. Treatment was done in accordance with the rules and laws established by the Florida Department of Agriculture and Consumer Services.

Celia Dryden

Authorized Signature

2-19-13

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 05-3S-16-01994-000 **Building permit No.** 000024457

Use Classification SFD/UTILITY **Fire:** 0.00

Permit Holder FRED GAYLARD **Waste:** 0.00

Owner of Building ROBIN H. & LISA K.B. ROBERTS **Total:** 0.00

Location: 6854 NW LAKE JEFFREY RD, LAKE CITY, FL

Date: 01/29/2007

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
 Document ID:1SWQ487-Z0227130152

Truss Fabricator:	Anderson Truss Company
Job Identification:	6-176--F.O.G. Contracting Roberts -- , **
Truss Count:	6
Model Code:	Florida Building Code 2004
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.24.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 32.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Notes:

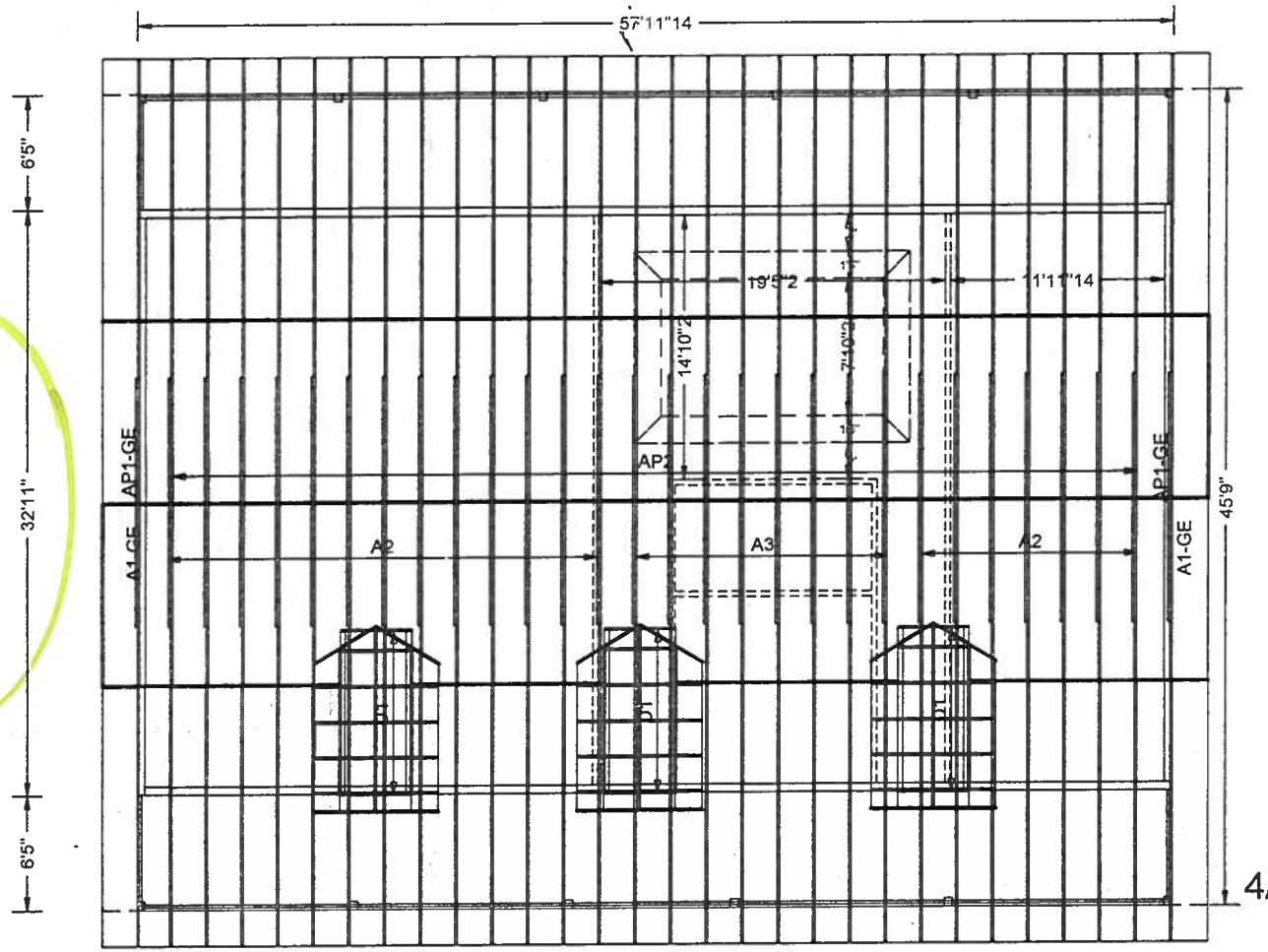
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: A11015EE-GBLLETIN-BRCLBSUB-PIGBACKB-

#	Ref	Description	Drawing#	Date
1	20726--A1-GE		06117042	04/27/06
2	20727--A2		06117043	04/27/06
3	20728--A3		06117044	04/27/06
4	20729--D1		06117041	04/27/06
5	20730--AP1-GE		06117045	04/27/06
6	20731--AP2		06117046	04/27/06

Seal Date: 04/27/2005

-Truss Design Engineer-
Arthur R. Fisher
Florida License Number: 59687
1950 Marley Drive
Haines City, FL 33844



4/27/06

#6-176 F.O.G. CONTRACTING - ROBIN AND LISA ROBERTS

Scale: 3/32" = 1'

897-4815 Fred

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

(A) 1x4 SP #3 or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

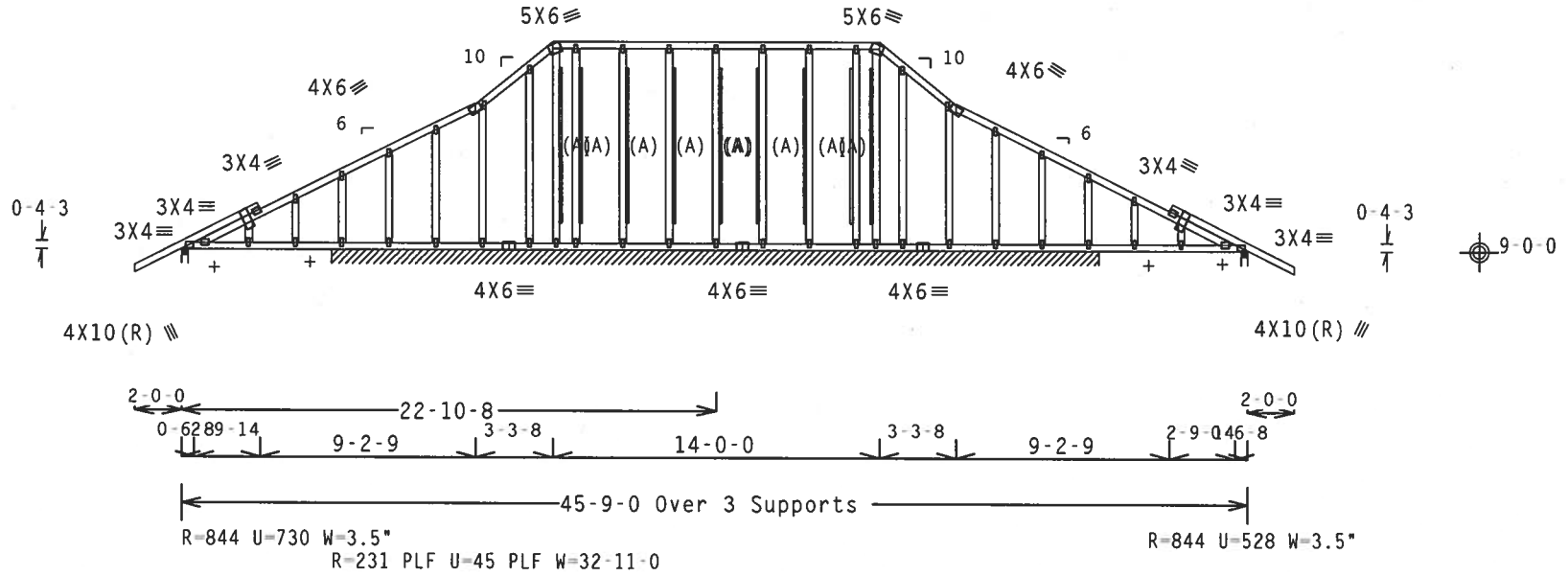
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Apr '06

TC LL	20.0 PSF	REF	R487-- 20726
TC DL	10.0 PSF	DATE	04/27/06
BC DL	10.0 PSF	DRW	HCUSR487 06117042
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN-	100053 REV
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SW0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC -	From	62 PLF	at -2.00	to	62 PLF	at 12.58
TC -	From	66 PLF	at 12.58	to	66 PLF	at 15.88
TC -	From	62 PLF	at 15.88	to	62 PLF	at 29.88
TC -	From	66 PLF	at 29.88	to	66 PLF	at 31.53
TC -	From	199 PLF	at 31.53	to	212 PLF	at 33.17
TC -	From	208 PLF	at 33.17	to	257 PLF	at 39.33
TC -	From	62 PLF	at 39.33	to	62 PLF	at 47.75
BC -	From	4 PLF	at -2.00	to	4 PLF	at 0.00
BC -	From	20 PLF	at 0.00	to	20 PLF	at 45.75
BC -	From	4 PLF	at 45.75	to	4 PLF	at 47.75

TC - 248 LB Conc. Load at 39.33

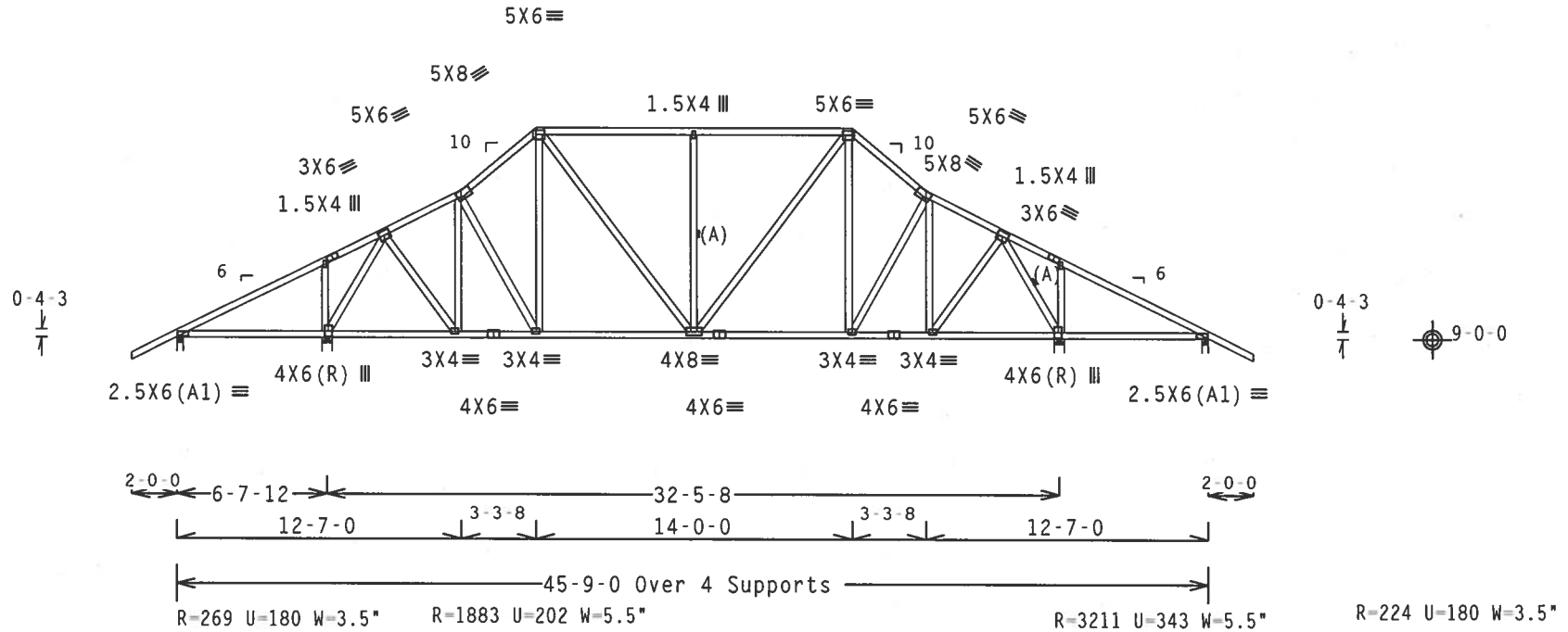
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.[illegible] QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 27 '06

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

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TC LL	20.0 PSF	REF	R487-- 20727
TC DL	10.0 PSF	DATE	04/27/06
BC DL	10.0 PSF	DRW	HCUSR487 06117043
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	100019
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SWQ487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	62 PLF	at	-2.00	to	62 PLF	at 12.58
TC - From	66 PLF	at	12.58	to	66 PLF	at 15.88
TC - From	62 PLF	at	15.88	to	62 PLF	at 29.88
TC - From	66 PLF	at	29.88	to	66 PLF	at 31.53
TC - From	199 PLF	at	31.53	to	212 PLF	at 33.17
TC - From	208 PLF	at	33.17	to	257 PLF	at 39.33
TC - From	62 PLF	at	39.33	to	62 PLF	at 47.75
BC - From	4 PLF	at	-2.00	to	4 PLF	at 0.00
BC - From	20 PLF	at	0.00	to	20 PLF	at 8.88
BC - From	24 PLF	at	8.88	to	24 PLF	at 10.38
BC - From	20 PLF	at	10.38	to	20 PLF	at 18.22
BC - From	24 PLF	at	18.22	to	24 PLF	at 19.72
BC - From	20 PLF	at	19.72	to	20 PLF	at 45.75
BC - From	4 PLF	at	45.75	to	4 PLF	at 47.75

TC - 248 LB Conc. Load at 39.33

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

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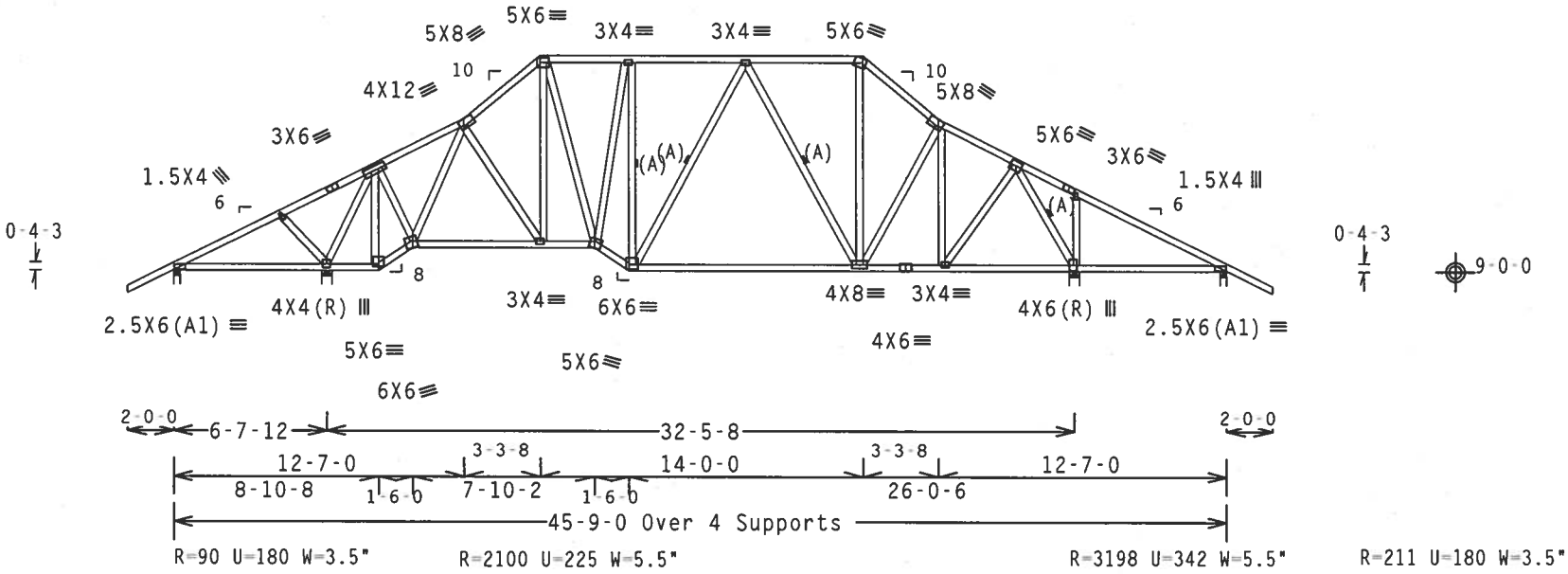

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.12 TY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE

Alpine Engineered Products, Inc.
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FL Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Apr 27 '06

TC LL	20.0 PSF	REF	R487-- 20728
TC DL	10.0 PSF	DATE	04/27/06
BC DL	10.0 PSF	DRW	HCUSR487 06117044
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	100033
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SWQ487_Z02

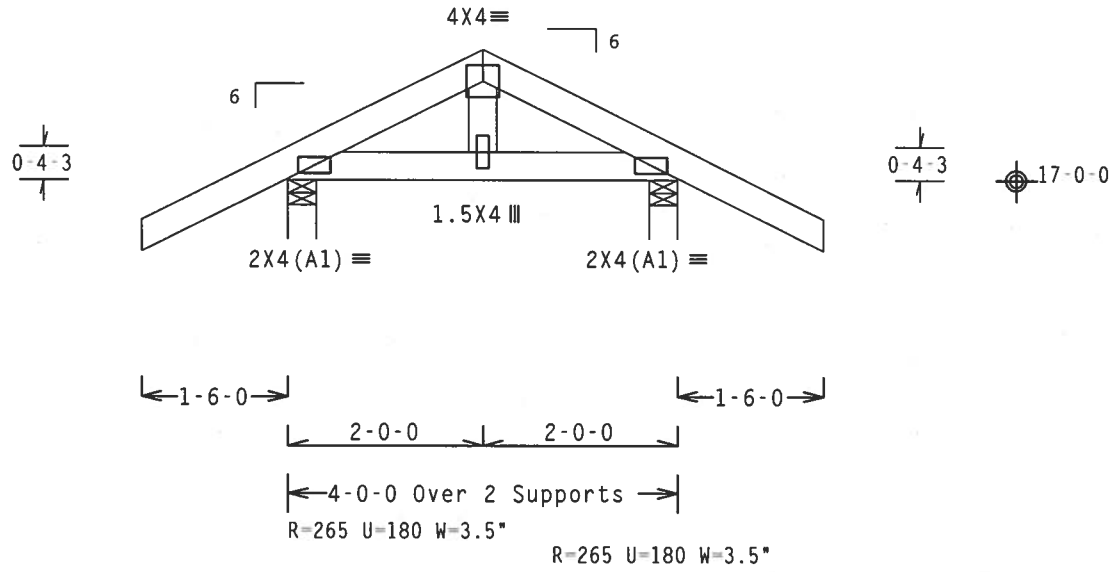
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 17.47 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 TY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 27 '06

TC LL	20.0 PSF	REF R487-- 20729
TC DL	10.0 PSF	DATE 04/27/06
BC DL	10.0 PSF	DRW HCUSR487 06117041
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 99999
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SWQ487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

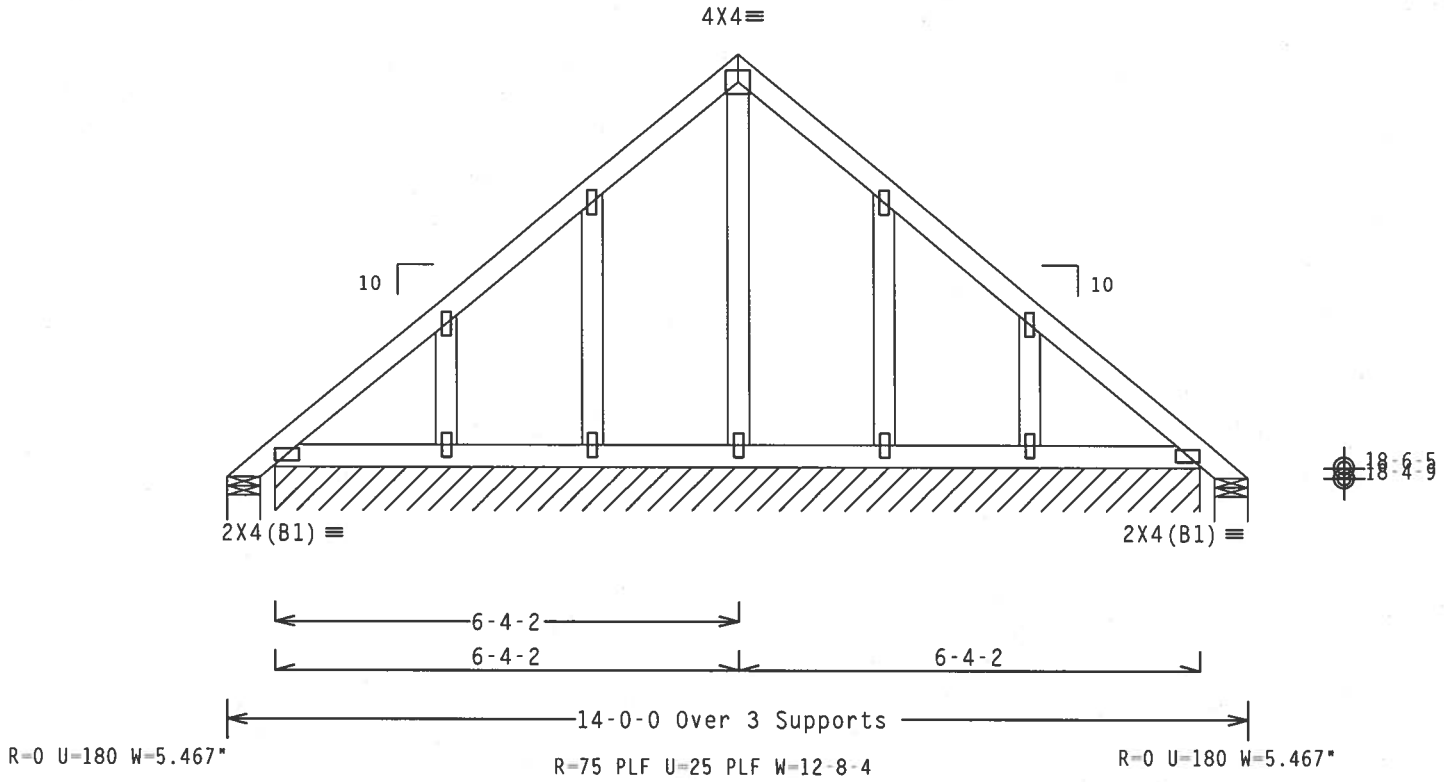
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24"O.C., UNLESS OTHERWISE SPECIFIED

110 mph wind, 21.30 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cg/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 27,06

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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TC LL	20.0 PSF	REF	R487-- 20730
TC DL	10.0 PSF	DATE	04/27/06
BC DL	2.0 PSF	DRW	HCUSR487 06117045
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	32.0 PSF	SEQN-	100058
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SWQ487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

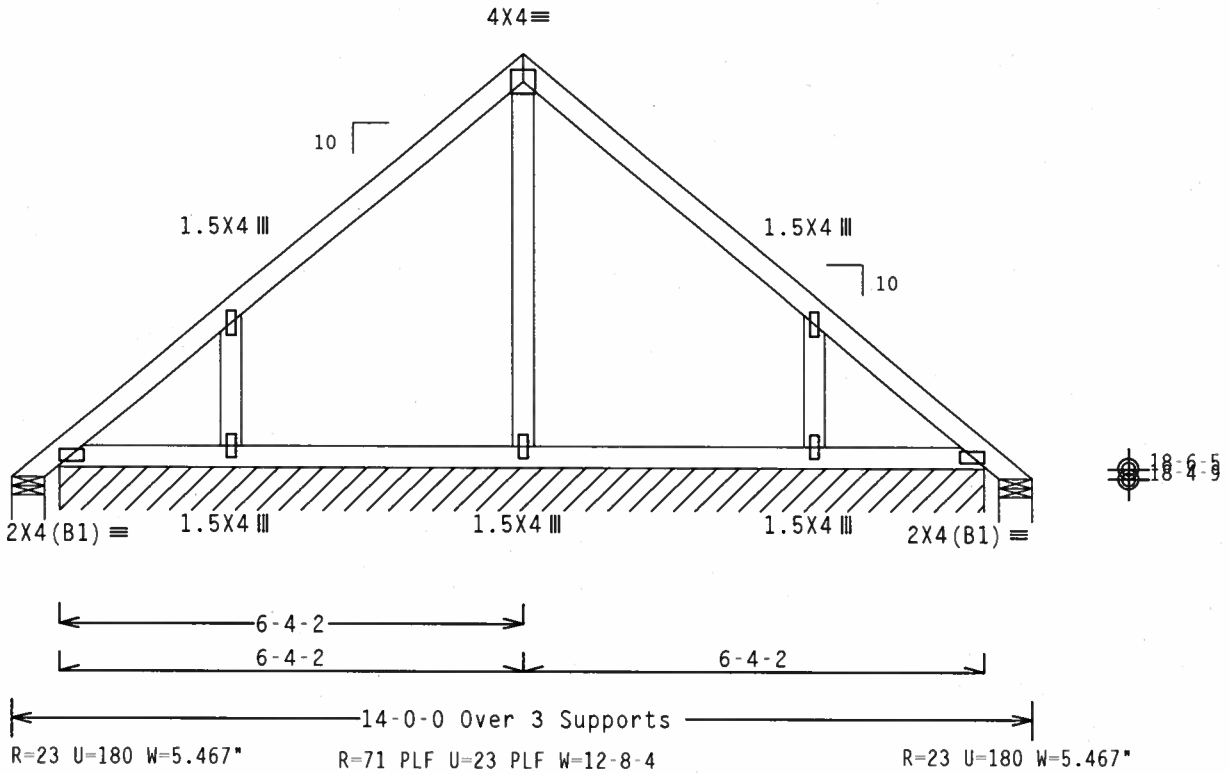
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24"O.C., UNLESS OTHERWISE SPECIFIED

110 mph wind, 21.30 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 27 '06

TC LL	20.0 PSF	REF	R487-- 20731
TC DL	10.0 PSF	DATE	04/27/06
BC DL	2.0 PSF	DRW	HCUSR487 06117046
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	32.0 PSF	SEQN-	99990
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SWQ487_Z02

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

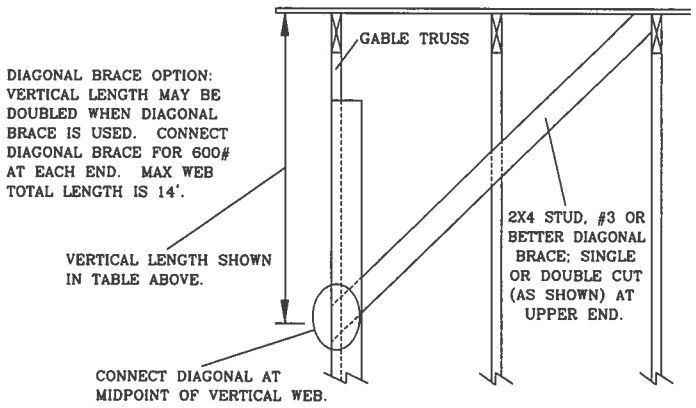
LIVE LOAD DEFLECTION CRITERIA IS L/240.

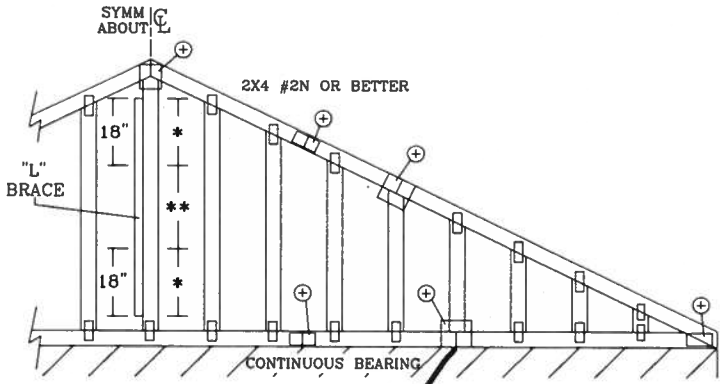
PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

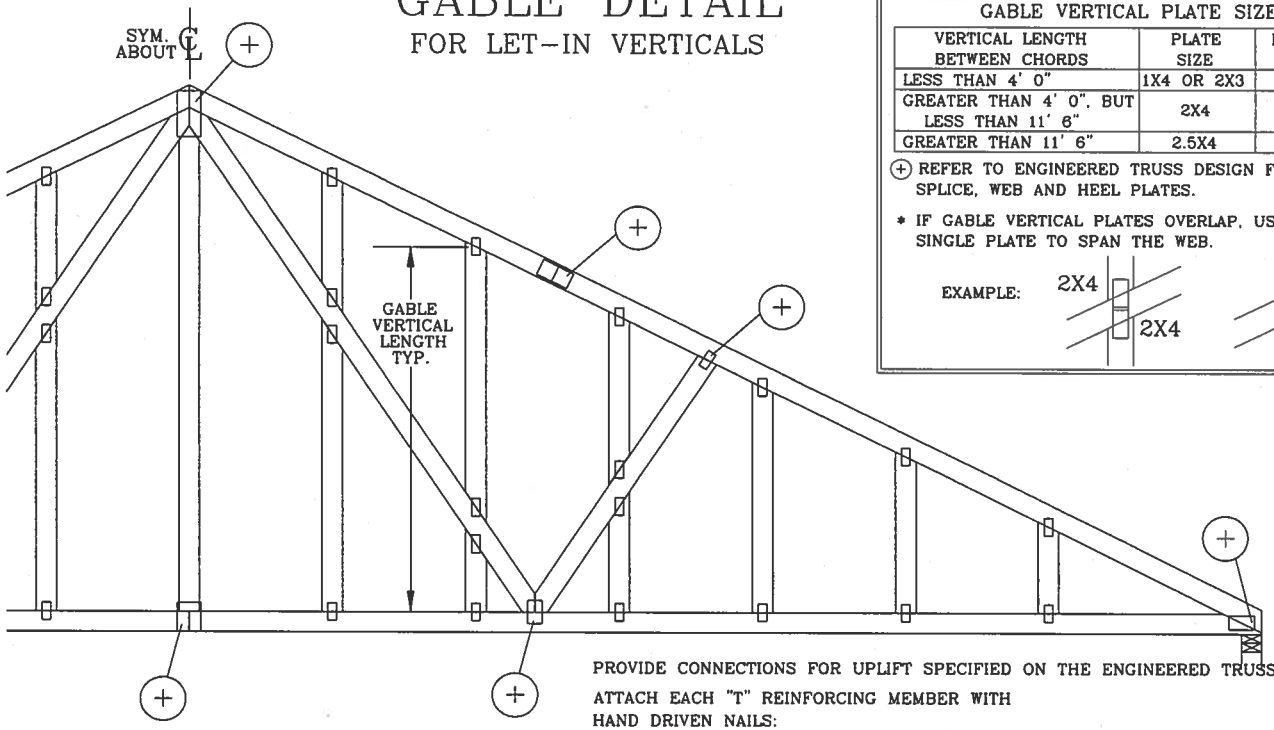
Ap... Dr...

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11015
DATE	04/15/05
DRWG	A11015EE0405
-ENG	

GABLE DETAIL
FOR LET-IN VERTICALS

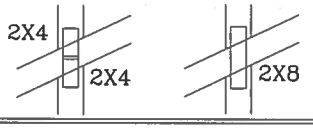


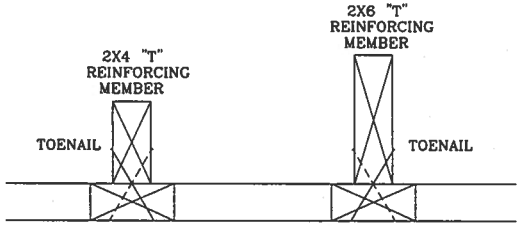
GABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:





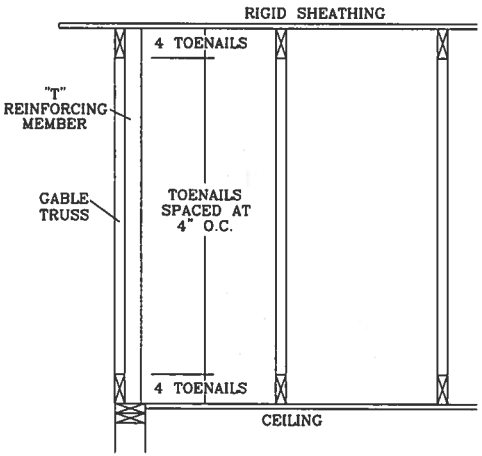
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.
GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.
ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN1103, A10015EN1103, A09015EN1103, A08015EN1103, A07015EN1103
A11030EN1103, A10030EN1103, A09030EN1103, A08030EN1103, A07030EN1103
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC1103, A12015EC1103, A11015EC1103, A10015EC1103, A08515EC1103
A13030EC1103, A12030EC1103, A11030EC1103, A10030EC1103, A08530EC1103
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0405, A12015EE0405, A11015EE0405, A10015EE0405, A08515EE0405,
A13030EE0405, A12030EE0405, A11030EE0405, A10030EE0405, A08530EE0405

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

ARTHUR R. KARMER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Apr 2

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

ALPINE
ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

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MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"

REF	LET-IN VERT
DATE	04/14/05
DRWG	GBLLETIN0405
-ENG	DLJ/KAR

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

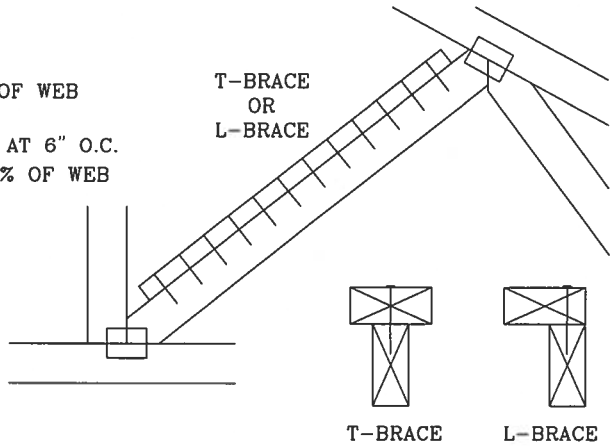
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

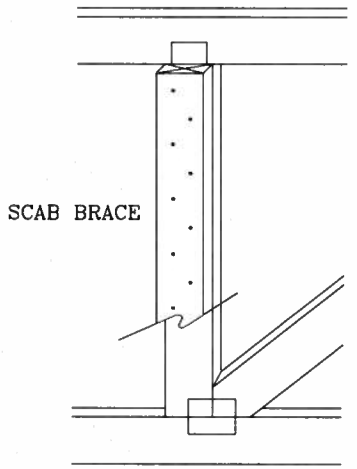
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE
ATTACH WITH 16d NAILS AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d OR .128"x3" GUN NAILS AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE
ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 11/26/03
BC DL	PSF	DRWG BRCLBSUB1103
BC LL	PSF	-ENG MLH/KAR
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

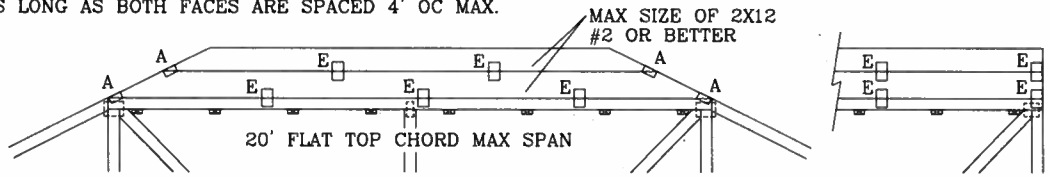
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

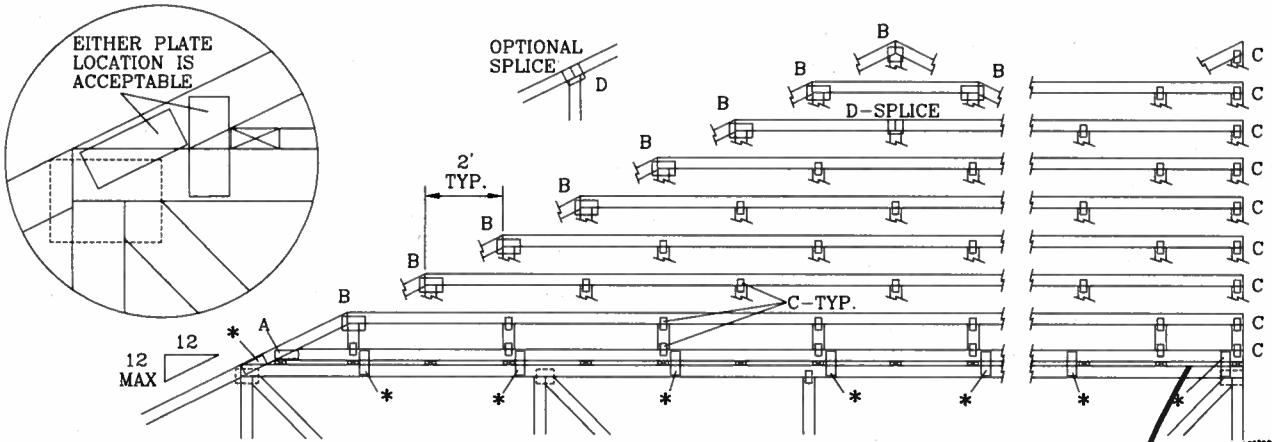
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

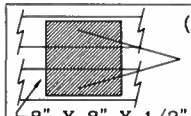
130 MPH WIND, 30' MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.



(4) 6d BOX (0.099"X 2.",MIN) NAILS.

8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET.

(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

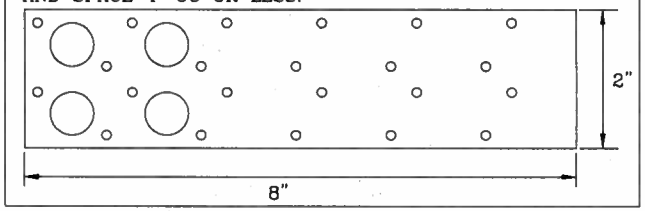
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5",MIN) NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5",MIN) NAILS AT 4" OC.

*** PIGGYBACK SPECIAL PLATE**

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

ALPINE

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POMPANO BEACH, FLORIDA

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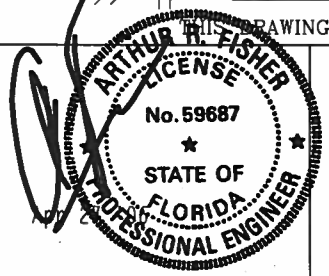


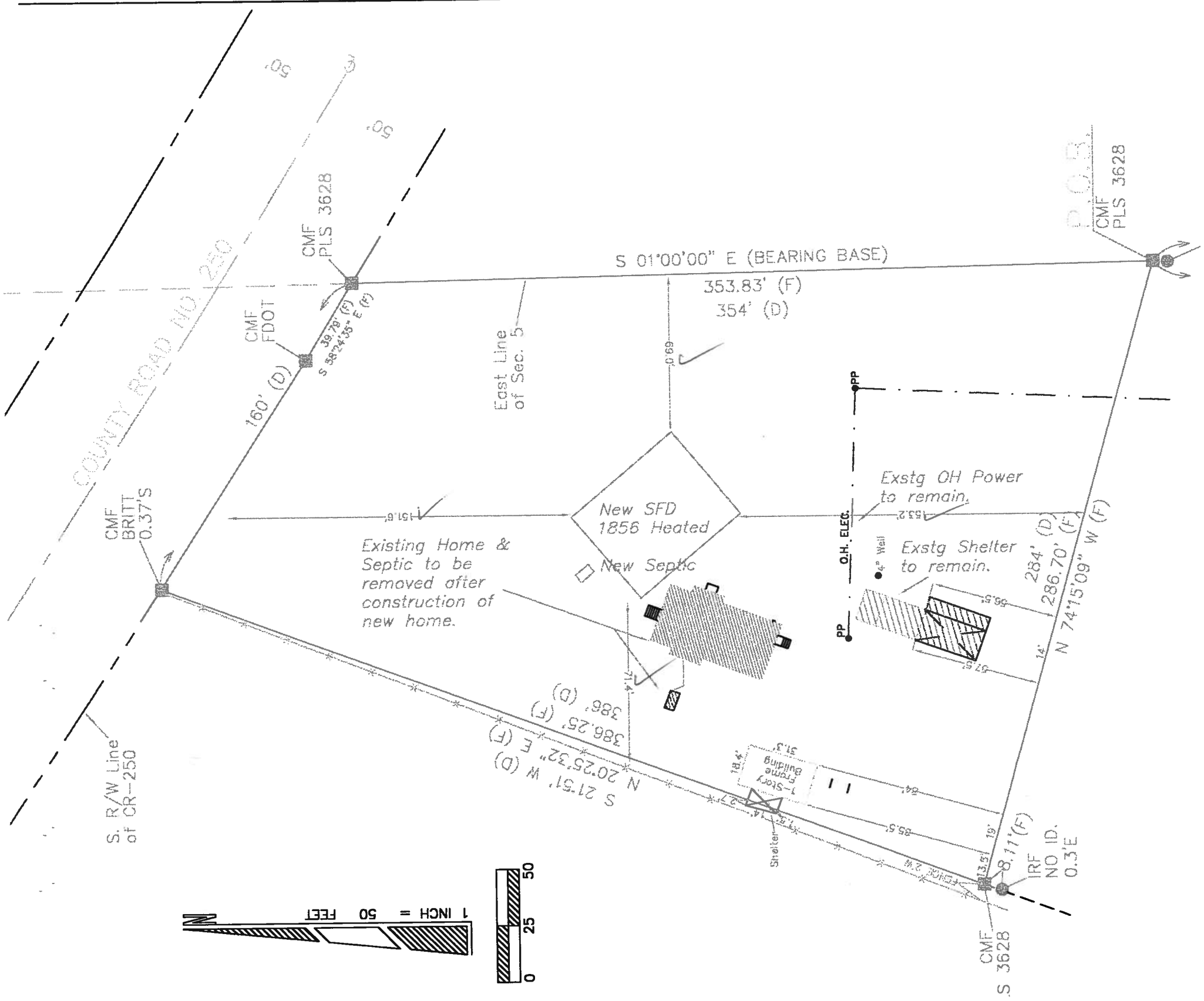
MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF	PIGGYBACK
DATE	04/14/05
DRWG	PIGBACKB0405
-ENG	DLJ/KAR

TOWNSHIP 3 SOUTH – RANGE 16 EAST

DESCRIPTION as recorded in O.R. Book 617, Page 687, Columbia County, Florida: COMMENCE at the Southeast corner of Section 5, Township 3 South, Range 16 East and run North along the East line of said Section 5 a distance of 360 feet for a POINT OF BEGINNING and run thence North along said Section line 354 feet, thence run in a Westerly direction 160 feet; thence run South 21 °51' West 386 feet, more or less thence run in an Easterly direction 284 feet more or less to the POINT OF BEGINNING.

SITE PLAN FOR ROBIN & LISA ROBERTS



OFFICE

ROBIN & LISA ROBERTS RESIDENCE

INDEX TO SHEETS

ENGINEER'S CERTIFICATE

WINLOAD ENGINEER: Huey Hawkins, PE No 33665 - 6855 SW Elim Church Rd., Fort White, FL 32038 - 386 397 -3991

CERTIFICATION: These plans (sheets 1 -12) and winload engineering, comply with Florida Building Code Res. 2004 Sec. R301.2.1, to the best of my knowledge.

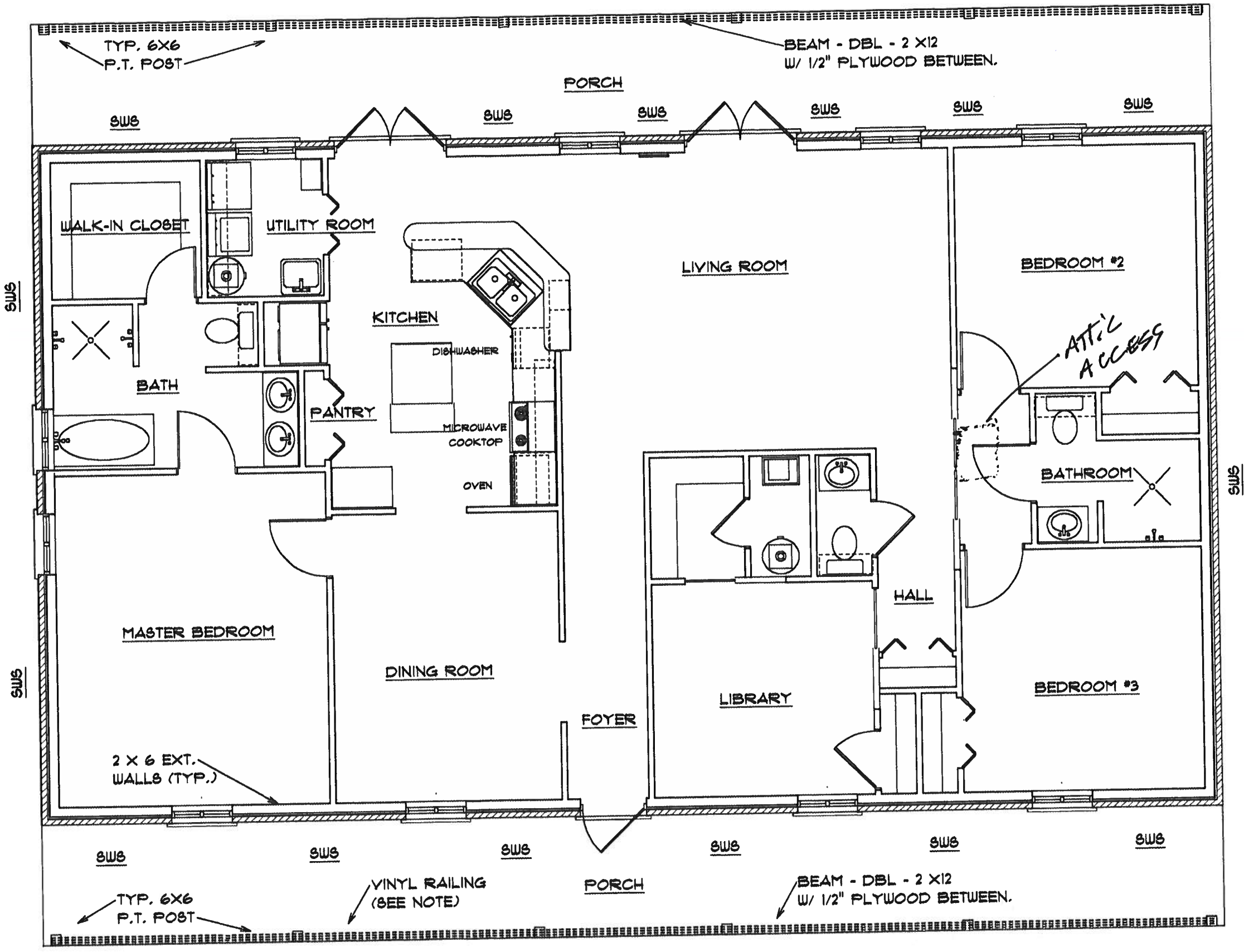
SIGNED _____ DATE: 4/9/2006

DATE 3/06
REVISED
DRAWING 12 EACH
SCALE VARIES
DRAWN BY FRED GAYLARD
APPROVED

HEADERS AT OPENINGS ARE MIN. DBL 2X10 U.N.O.

SWS = SHEAR WALL SEGMENT LYING BETWEEN ADJACENT OPENINGS.

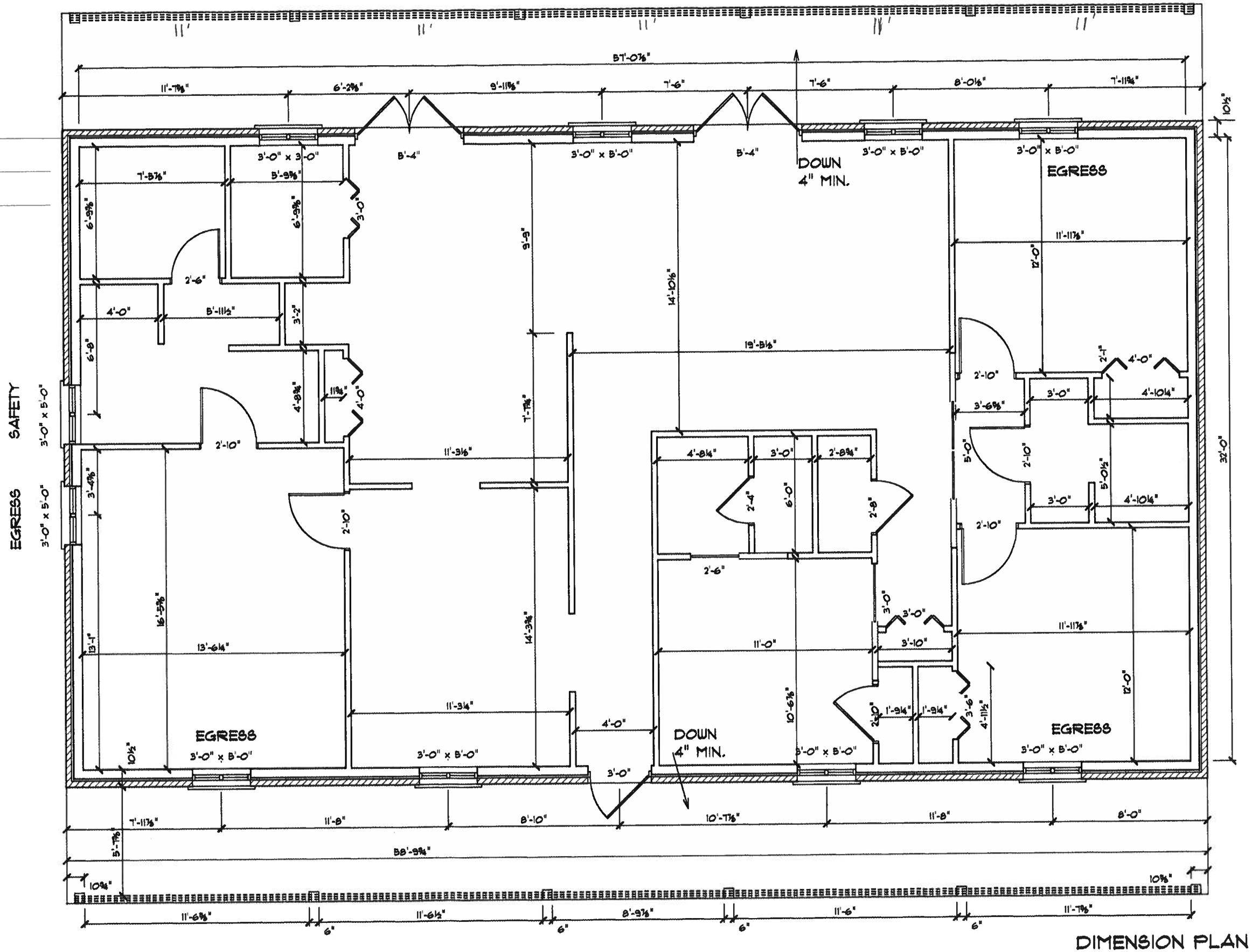
RAILING NOTES:
1 - 36" HEIGHT
2 - 4" MAX. CLEAR BETWEEN BALLISTERS
3 - BOTTOM RAIL 2" MAX. A.F.F.



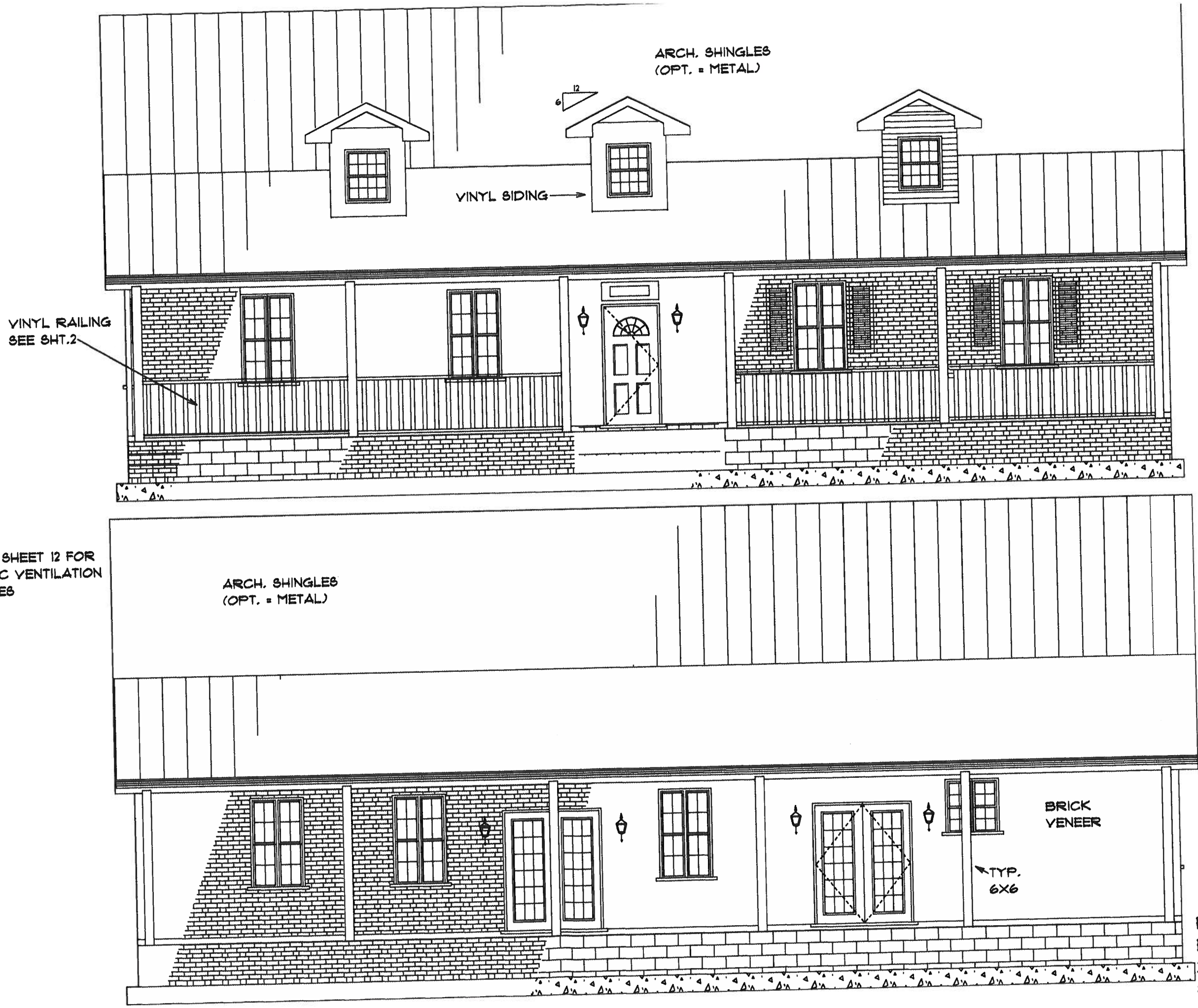
4-10-06

FLOOR PLAN
3/16" = ONE FOOT
SHEET 2

LIVING AREA 1856 SF
FRONT PORCH 318 SF
BACK PORCH 318 SF
TOTAL AREA 2492 SF

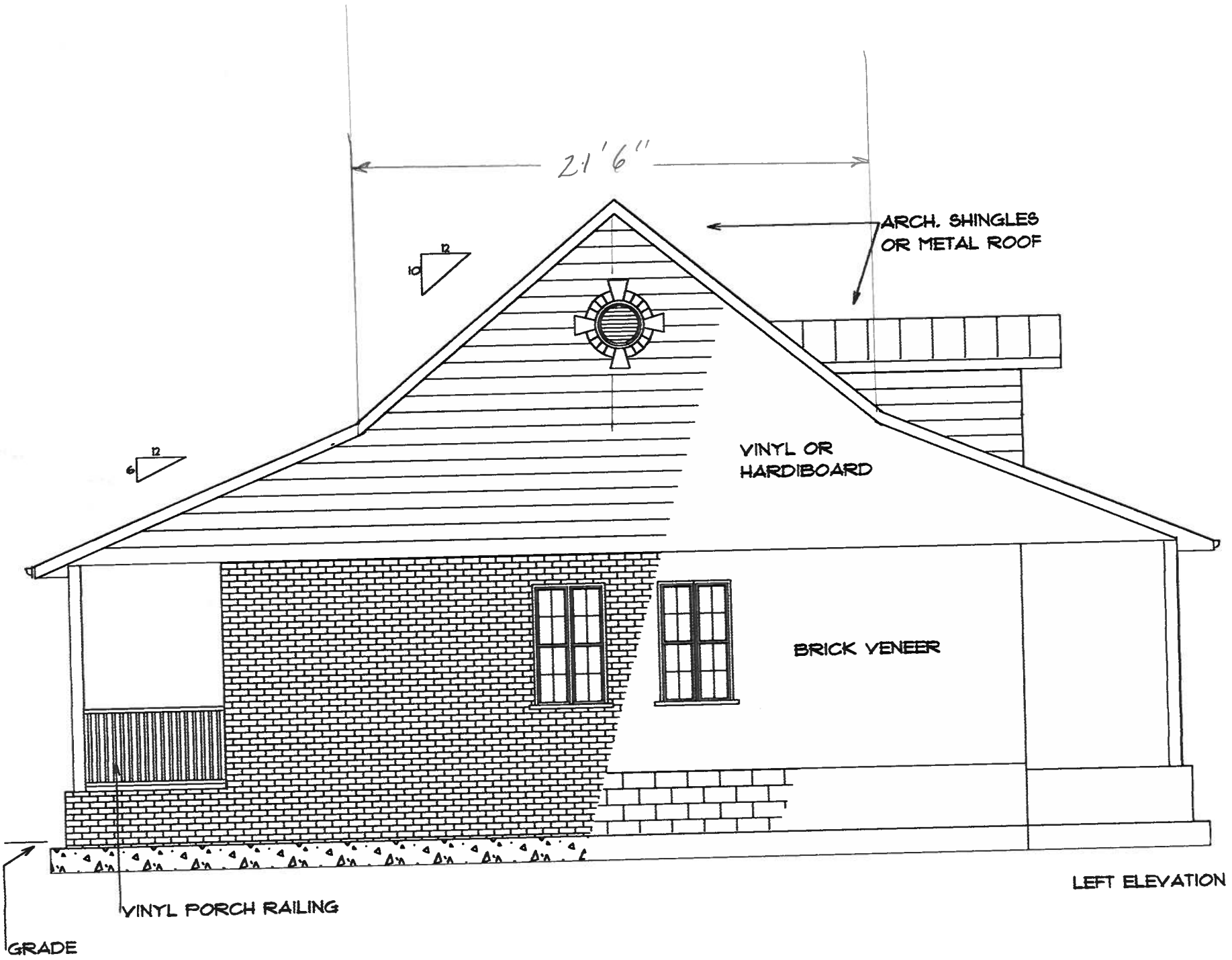


DIMENSION PLAN
3/16" = ONE FOOT
SHEET 3

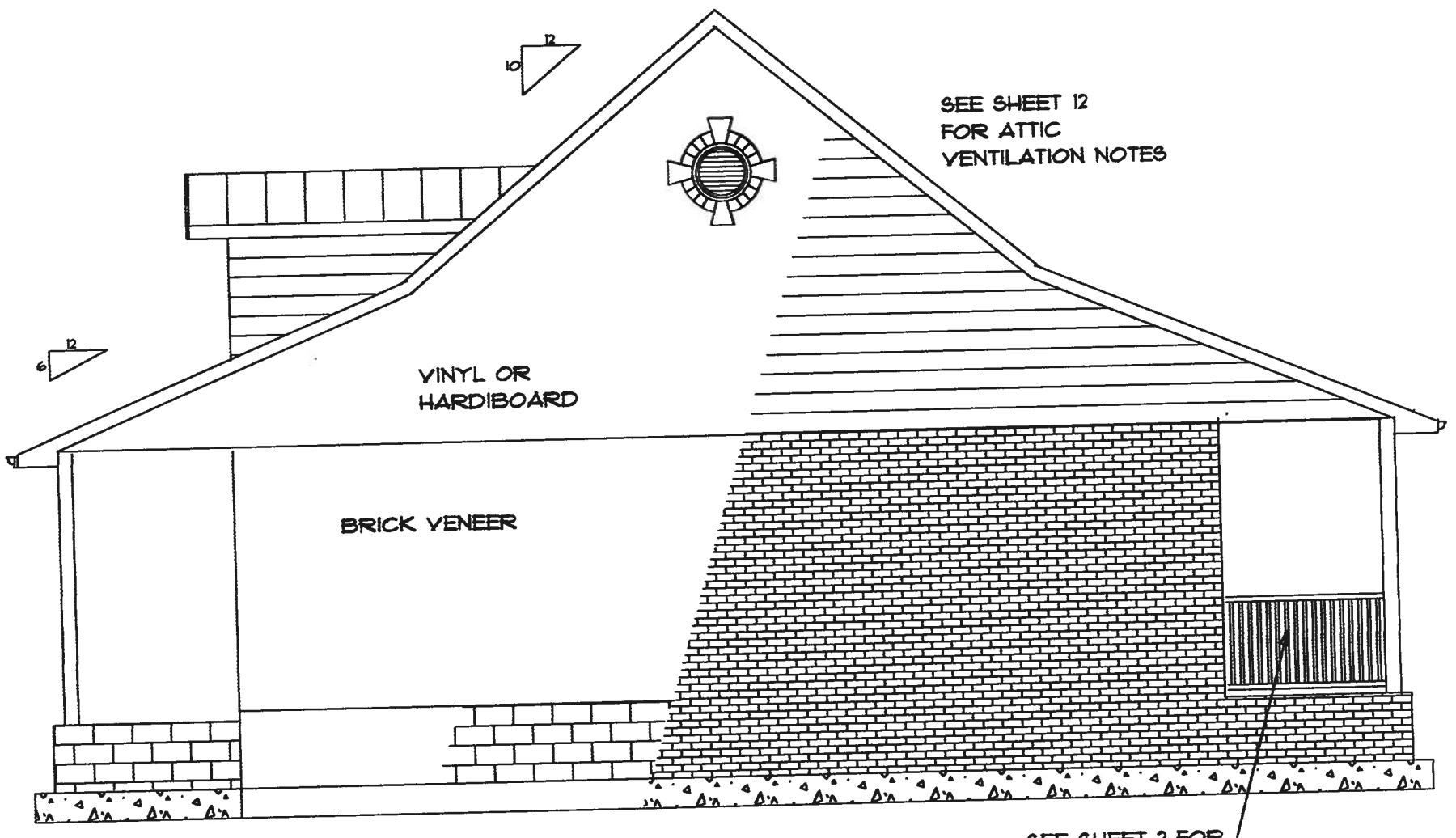


SEE SHEET 12 FOR ATTIC VENTILATION NOTES

FRONT/REAR ELEVATIONS
3/16" = ONE FOOT
SHEET 4

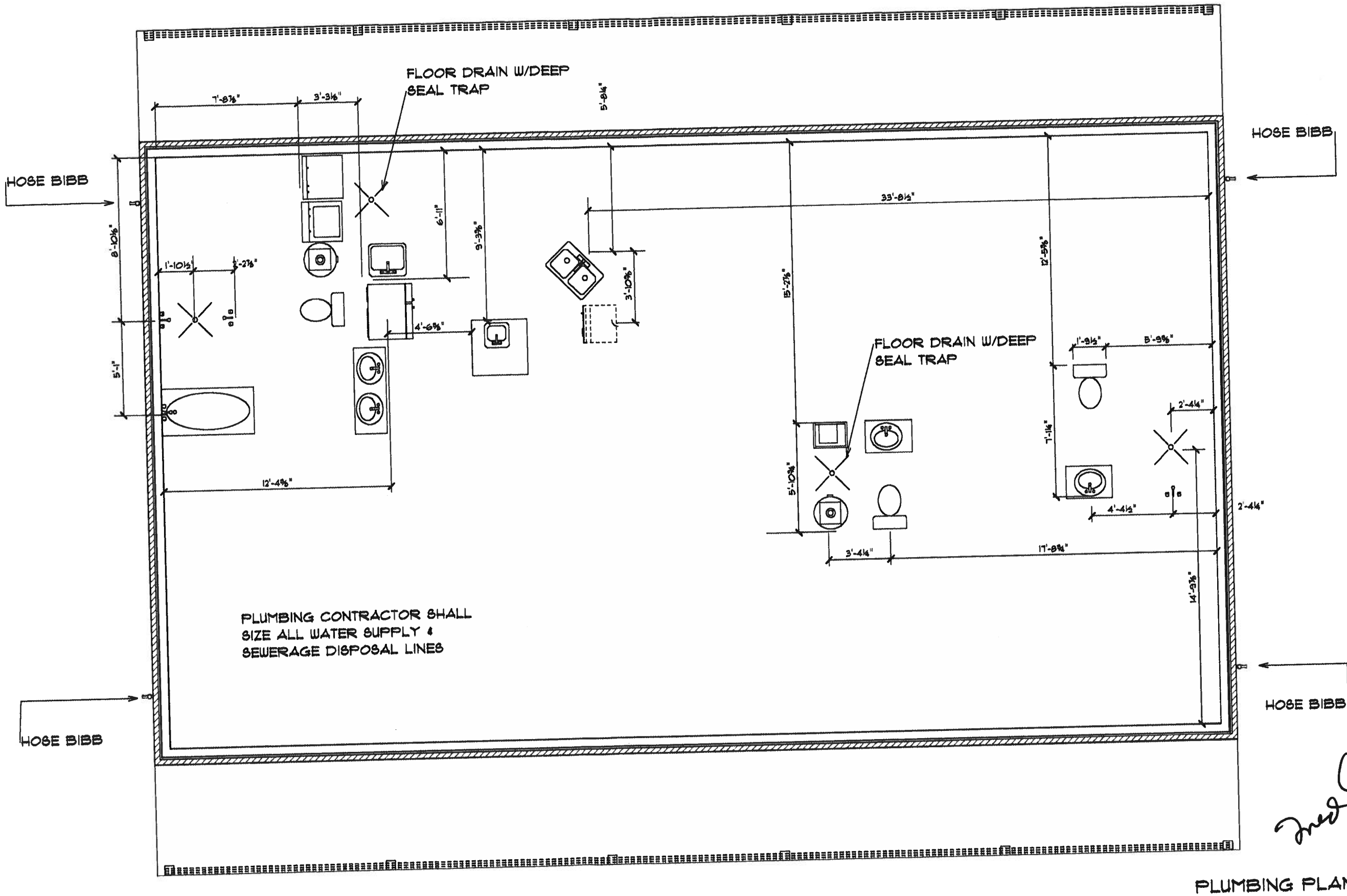


LEFT ELEVATION

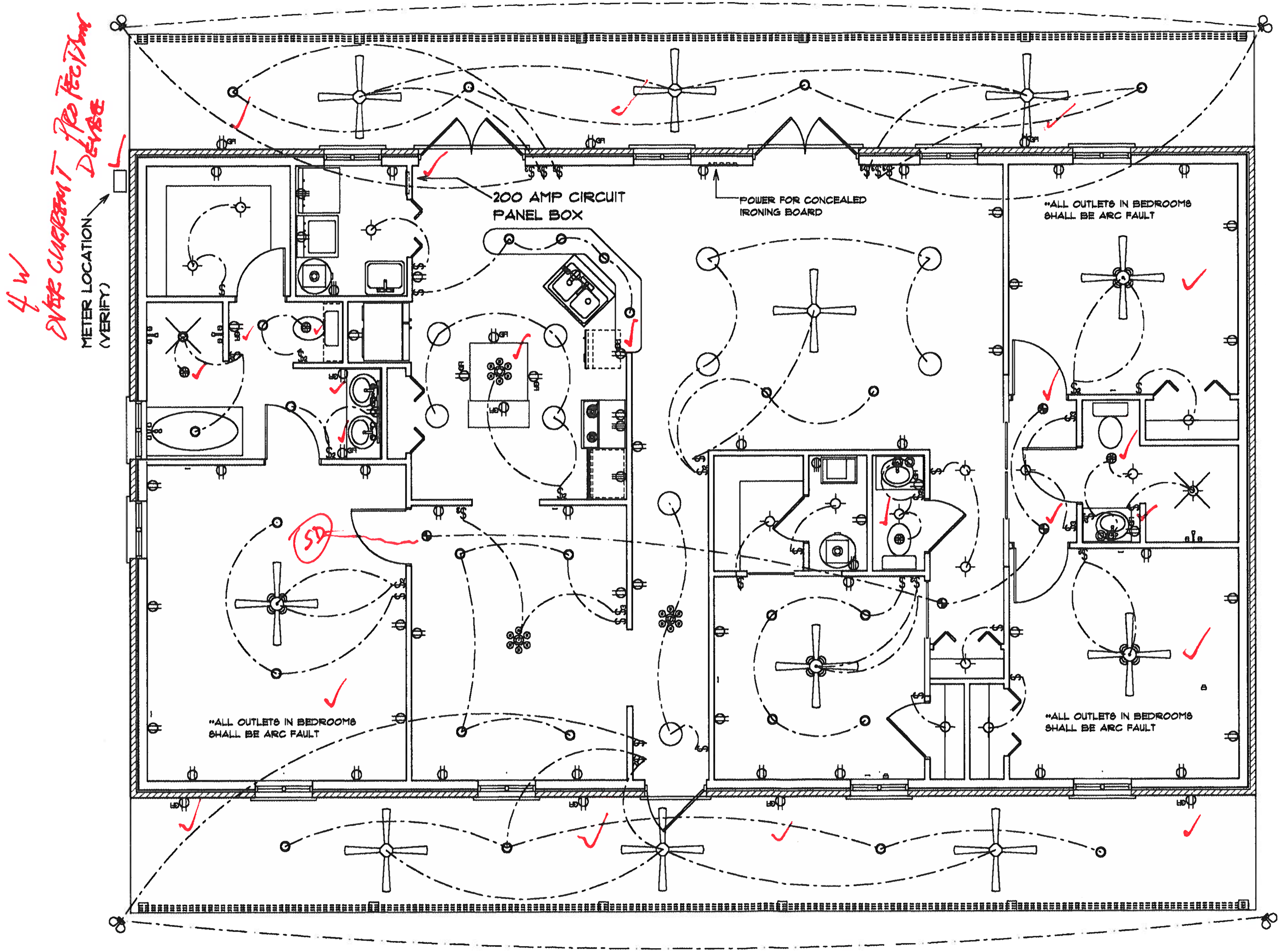


RIGHT ELEVATION

SIDE ELEVATIONS
SHEET 5



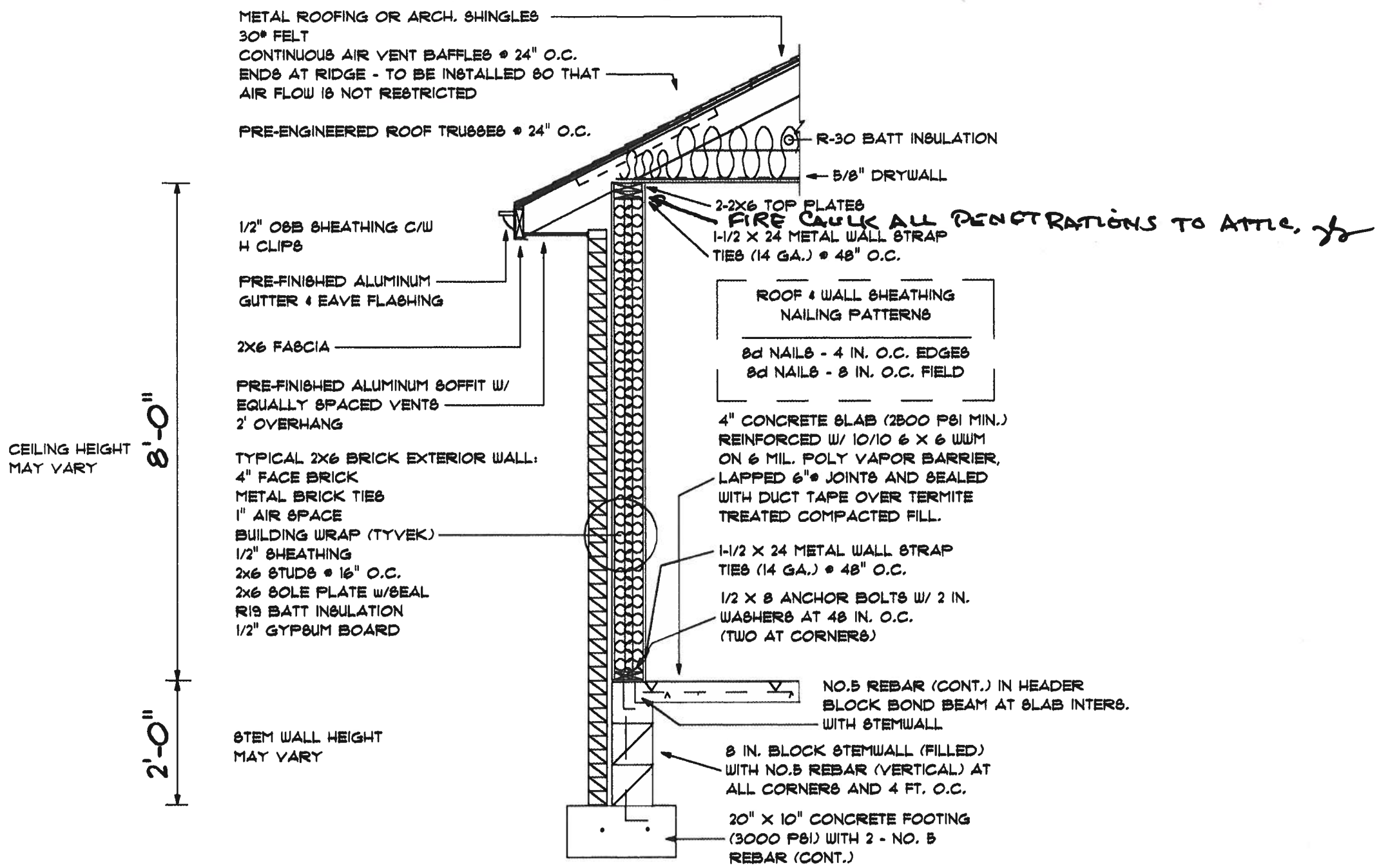
PLUMBING PLAN
3/16" = ONE FOOT
SHEET 6



	Electrical
	exterior ceiling fan
	ceiling fan w/light kit
	recessed can light
	chandelier
	double spotlight
	can light w/trim
	vanity bar light
	vanity wall light
	200 Amp elec panel
	telephone jack
	exhaust fan
	ceiling light
	outlet
	outlet 220v
	outlet gfi
	smoke detector
$	switch
$3	switch 3 way
$2	switch double
	computer jack

WIRE APPLIANCES PER MANUFACTURER'S SPECS.

ELECTRICAL PLAN
3/16" = ONE FOOT
SHEET 1



TYP. WALL SECTION

WALL SECTION
SCALE - 1/2" = 1'-0"
SHEET 8

4-10-06

FF = 2'-4" ABOVE
TOP OF FOOTING

TYP. SLAB

VERIFY DIMENSIONS
W/ FLOOR PLAN

SITE ANALYSIS IS
NOT A PART OF
THIS PLAN.

SEE TYP. WALL
SECTION - SHT. 8

73'-10 1/4"

4" CONC. SLAB (2500 PSI.MIN.)
REINFORCED WITH SYNTHETIC FIBERS
ON 6 MIL. POLYETHYLENE VAPOR
BARRIER, LAPPED 6" @ JOINTS AND
SEALED WITH TAPE OVER TERMITE
TREATED COMPACTED FILL
REINFORCED WITH 10/10 6X6 WWM

FF = 2'-8" ABOVE
TOP OF FOOTING

*SEE PLUMBING PLAN FOR
INFORMATION RELATIVE
TO FIXTURE LOCATIONS

VERIFY INTERIOR BEARING
REQUIREMENTS W/TRUSS
PLAN (BY SUPPLIER)

FF = 2'-4" ABOVE
TOP OF FOOTING

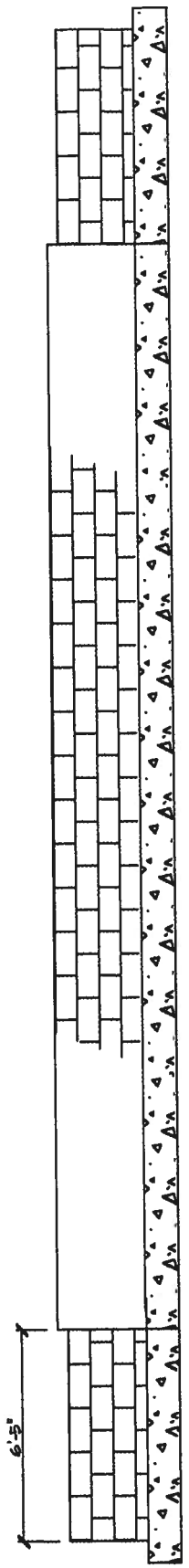
TYP. SLAB

6'-5"

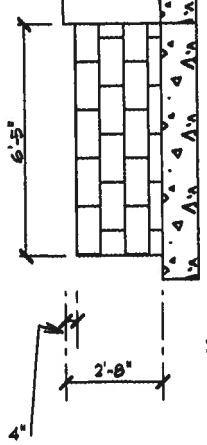
32'-11"

6'-5"

87'-11⅞"

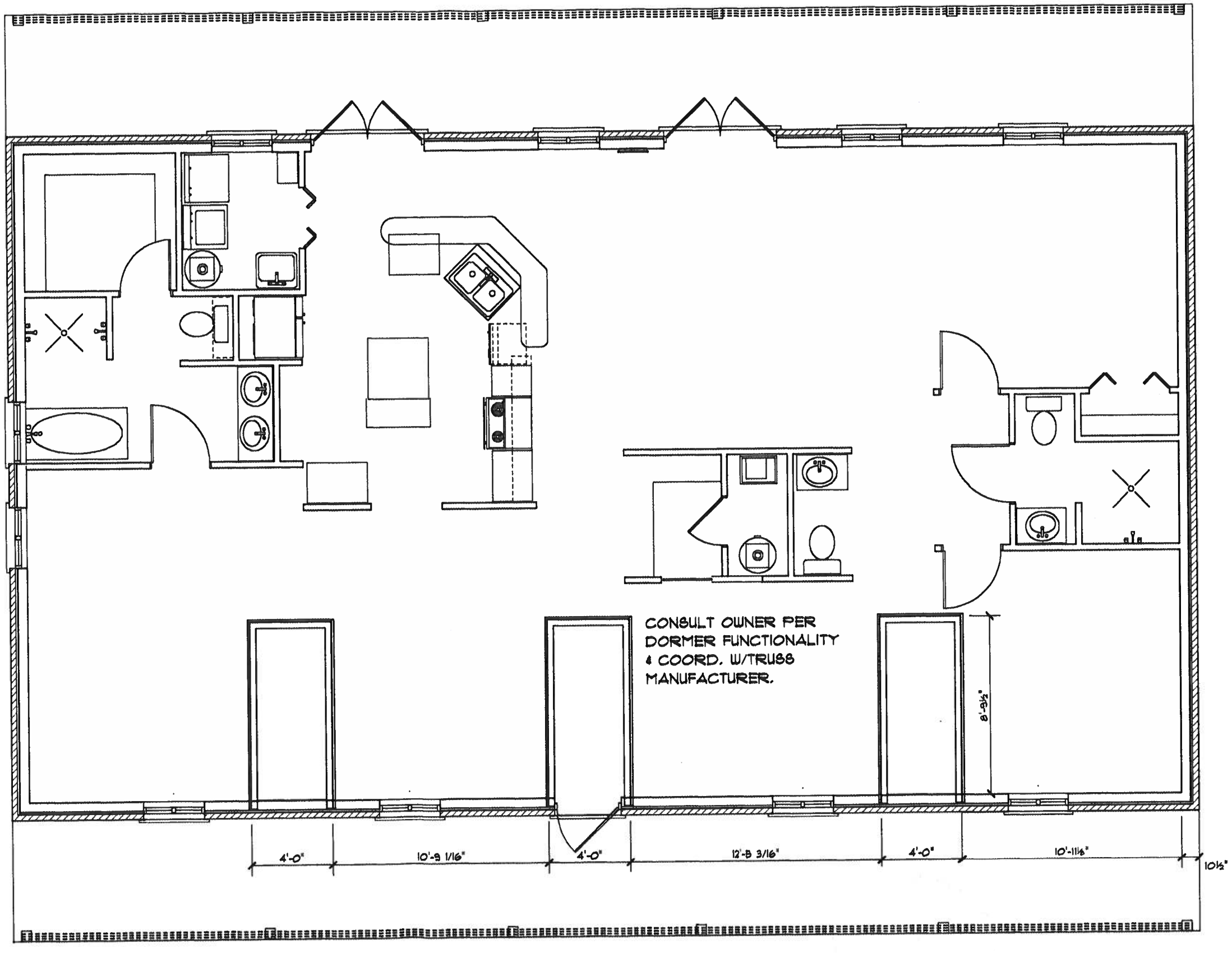


SEE WALL SECTION
FOR REINFORCEMENT
AND CONCRETE
INFORMATION

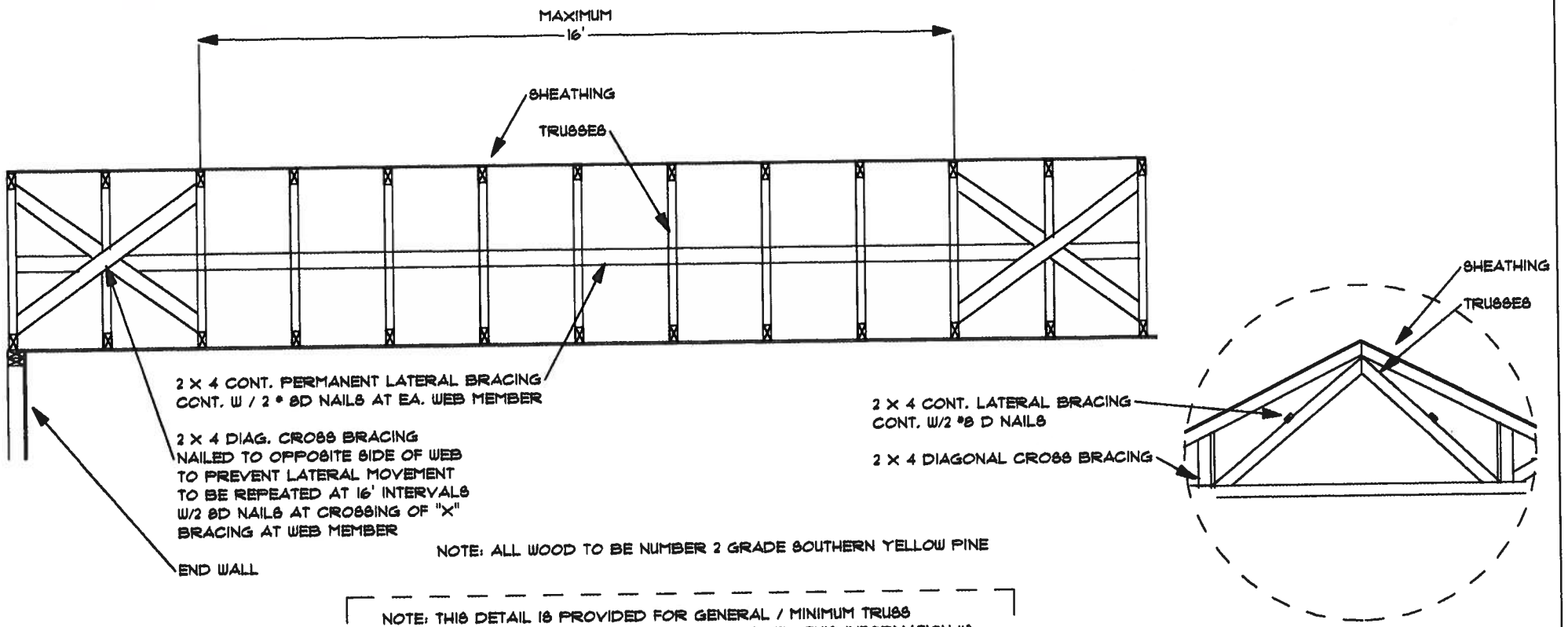


FOUNDATION PLAN
3/16" = ONE FOOT
SHEET 9

4-10-06



DORMER LOCATIONS
3/16" = ONE FOOT
SHEET 10

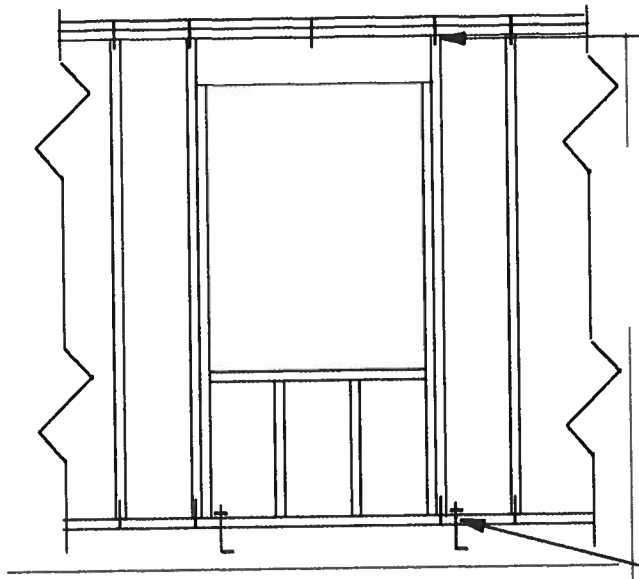


NOTE: ALL WOOD TO BE NUMBER 2 GRADE SOUTHERN YELLOW PINE

NOTE: THIS DETAIL IS PROVIDED FOR GENERAL / MINIMUM TRUSS BRACING DATA ONLY. THE BUILDER SHALL VERIFY THIS INFORMATION W\ WITH THE ROOF TRUSS PLAN (BY SUPPLIER). ROOF TRUSS SUPPLIER'S REQUIREMENTS SHALL SUPERCEDE THIS DATA WHERE MORE STRIGENT.

TYP. PERMANENT TRUSS BRACING DIAGRAM

SCALE: NONE



NOTE:
OPENINGS TO 5' WIDTH USE (1) 8P4
OPENINGS 5' TO 7' USE (2) 8P4

NOTE:
ANCHOR BOLTS MAY BE LOCATED AT EITHER SIDE OF KING STUDS. PLATE MUST BE CONT. BETWEEN BOLT AND KING STUDS

ANCHOR BOLTS MUST BE WITHIN 12" OF KING STUDS

1/2" X 10" ANCHOR BOLTS AT 32" OC TYPICAL

TYPICAL HEADER STRAPING DETAIL

ROOF SYSEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC.SEC. R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MAUNFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR TH LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

WALL STUD TABLE

STUDS ARE SPRUCE/PINE/FIR AT 16 IN. O.C.

1 - 2 X 4	TO 10 FT. WALL HEIGHT
2 - 2 X 4	TO 12.5 FT. WALL HEIGHT
3 - 2 X 4	TO 16.5 FT WALL HEIGHT
1 - 2 X 6	TO 16 FT WALL HEIGHT
2 - 2 X 6	TO 21 FT WALL HEIGHT

TOP PLATE IS 2 - 2 X 4 (OR 2 X 6) SPRUCE/PINE/FIR

BOTTOM PLATE IS 2 X 4 (OR 2 X 6) PRESS. TEATED BEARING ON FOUNDATION WALL.

TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQ. FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TOP CONNECTOR*			BOTTOM CONNECTOR*	
< 415	H2.5	10-8d	415	NO SPECIAL CONNECTOR REQ.	
< 750	H16	6-10d, 1-1/2"	750	NO SPECIAL CONNECTOR REQ.	
< 905	H10	16-8d, 1-1/2"	905	NO SPECIAL CONNECTOR REQ.	
< 1250	H16	10-10d, 1-1/2"	1250	SPH4 W/10-10d, 1-1/2" + 1/2" AB	970
< 1245	HTS20	24-10d, 1-1/2"	1245	LTT19 W/8-16d, + 1/2" AB	1080
< 2490	2-HTS20	24-10d, 1-1/2"	2490	HD2A-2.5", 5/8" AB	2565

UPLIFT GREATER THAN 2500 LBS. REQUIRES ENGINEERING APPROVAL

*MODEL NUMBERS SHOWN ARE FOR SIMPSON CONNECTORS. THESE ARE LISTED AS EXAMPLES & NOT FOR ENDORSEMENT. OTHER MANUFACTURER'S CONNECTORS WITH SUFFICIENT LOAD CAPACITY MAY BE SUBSTITUTED. FOLLOW MANUFACT. INSTRUCTIONS.

4-10-06

FOUNDATION PLAN
NO SCALE
SHEET 11

ENGINEER'S NOTES:

~This report establishes the minimum requirements for wind load stability. It is the owner/builder's responsibility to provide materials and construction techniques, which comply with FBC requirements for the stated wind velocity. It is the builder's responsibility to provide a continuous load path from trusses to foundation.

~Since truss engineering was not complete at the time of this analysis, it is the builder's responsibility to select uplift connections based on truss engineering uplift and provide footings for interior bearing walls. Builder is to furnish truss engineering to wind load engineer for review of truss reactions on the building structure. Strap 2x6 rafters with min uplift connection 415lb each end, 2x8 rafters 700lb each end.

~Site analysis and preparation information is not part of this plan and is the responsibility of the owner. All foundations and footings are designed for stable soil conditions with 1000 psf bearing capacity. It is the owner's / builder's responsibility to verify soil and clean fill ar compaced to provide 1000 psf minimum bearing capacity or to request foundation design based on actual site conditions.

~Manufacturers and product number for connectors, anchors, and reinforcement are listed for example, not endoresement. An equivalent device of th same or other manufacturer can be substituted for any device listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to achieve rated loads.

~Anchor bolts -A-307. Minimum embedment: 7" in concrete or reinforced bond beam, 15" in grouted CMU.

~Concrete - Minimum compressive strength, Fc = 2500 psi.

~Rebar - Grade 40 deformed bars, Fy = 36ksi. All laps 40*Db (25" for #5) unless otherwise specified.

~Nails - All nails are common nails unless otherwise specified or accepted by FBC test reports as having equal structural values.

GENERAL NOTES

1) See this sheet and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.

2) All concrete used to be 2500 PSI strength or greater.

3) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.

4) Windows to be alum. framed and double glazed. Sizes shown ar nominal and may vary with manufacturer.

5) Roof Truss design is the responsibility of the supplier.

6) The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.

7) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.

8) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall ve provided with corrosion-resistant wire mesh, with 1 / 8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reductd to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of th required ventilation provided by eave or cornice vents.

DESIGN DATA

WIND LOADS ARE PER FLORIDA BUILDING CODE 2001, SECTION 1606.2
(FOR ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED AND GABLE - SHAPED ROOFS HAVING A MEAN ROOF HEIGHT NOT EXCEEDING THE LEAST HORIZONTAL DIMENSION OF THE BUILDING OR 60FT; NOT SITED ON THE UPPER HALF OF A HILL OR ESCARPMENT 60FT IN EXPOSURE B, 30FT IN EXPOSURE C AND >10% SLOPE AND UNOBSTRUCTED UPWINGD FOR 50X HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE.
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.) BASIC WIND SPEED = 110 MPH
2.) WIND IMPORTANCE FACTOR = 1
3.) BUILDING CATEGORY = II
4.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
5.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES: (DOORS & WINDOWS) +21.8 ? -29.1 PSF GARAGE DOOR (N /A) 9X7 +19.3 / -22.2 PSF; 16X7, +18.5 / -20.9
DESIGN LOADS
FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)
30 PSF (ATTICS WITH STORAGE)
10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF 20 PSF (FLAT OR <4:12)
16 PSF (4:12 TO <12:12)
12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
NO SNOW LOAD
WIND LOADS ARE PER FLORIDA BUILDING CODE 2001, SECTION 1606.2

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF TH WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WINDSPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORATY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. SELECT UPLIFT CONNECTIONS AND PROVIDE FOOTINGS BASED ON TRUSS ENGINEERING REACTIONS, FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS.

4-10-06

ENGNR NOTES
NO SCALE
SHEET 12