DATE 02/1	4/2008			uilding Permit on Premises During Co	estmustion	PERMIT
A DDL ICANE	C + N / + N / T		Prominently Posted	(S T)		000026755
APPLICANT		HA HARRINGTON	170 DMD 229	PHONE LAKE CITY	386.719.7143	FL 32055
ADDRESS OWNER	2109 ISAAC CC	W US HWY 90, STE	170, PMB 338	PHONE	386.719.7143	<u>1L</u> <u>32033</u>
ADDRESS	153	SW ASHEVILLE WA	AV	- LAKE CITY	360.717.7143	FL 32024
CONTRACTO		AC BRATKOVICH		PHONE	386.719.7143	
LOCATION O			-341.TL TO C-242.TL	TO ASHEVILLE RD,TF	-	
		THE 3RD.				
TYPE DEVEL	OPMENT	SFD/UTILITY	ES*	TIMATED COST OF CO	NSTRUCTION	121550.00
HEATED FLO	OOR AREA	1735.00	TOTAL ARE	EA 2431.00	HEIGHT 2	2.00 STORIES <u>1</u>
FOUNDATIO	N CONC	WALL	S FRAMED F	ROOF PITCH 7'12	FL	OOR CONC
LAND USE &	ZONING	PRD		MAX	. HEIGHT 3	5
Minimum Set l	Back Requir	ments: STREET-F	RONT 25.00	REAR	15.00	SIDE 10.00
NO. EX.D.U.	0	FLOOD ZONE	XPP	DEVELOPMENT PERI	MIT NO.	-
PARCEL ID	25-4S-16-	03124-103	SUBDIVISIO	N HICKORY COVE		
LOT 3	BLOCK	PHASE _	UNIT	тотл	AL ACRES 0.	32
			CBC059323	110	hl a	
Culvert Permit	No.	Culvert Waiver Co	ontractor's License Nur	nber / man	Applicant/Owner/	/Contractor
WAIVER	5.150	08-0152-N	BLK		ТН	N
Driveway Con	nection	Septic Tank Number	LU & Zoni	ng checked by App	proved for Issuance	e New Resident
COMMENTS:	1 FOOT A	ABOVE ROAD.				
					Check # or C	ash 9884
Mytanii II Alliana J		FOR BU	ILDING & ZONIN	NG DEPARTMENT	ONLY	(footer/Slab)
Temporary Pov	wer		Foundation		Monolithic	
		date/app. by		date/app. by		date/app. by
Under slab rou	igh-in plumb				Sheathing/	Nailing date/app. by
Framing		date/app		date/app. by bove slab and below woo	d floor	date/app. by
	date/ap		Rougn-in plumbing a	bove stab and below woo	u 11001	
Electrical roug	gh-in					date/app. by
			Heat & Air Duct		Peri, beam (Linte	70
		date/app. by	Heat & Air Duct	date/app. by	Peri. beam (Linte	70
Permanent pow		date/app. by	C.O. Final		Peri. beam (Linte	date/app. by
	da		C.O. Final	date/app. by		el)
M/H tie downs,	da , blocking, e	date/app. by	C.O. Finaldate/app	date/app. by	Culvert	date/app. by
	da, blocking, e	date/app. by ate/app. by lectricity and plumbing	C.O. Finaldate/app	date/app. by	Culvert	date/app. by date/app. by date/app. by
M/H tie downs, Reconnection M/H Pole	da , blocking, e	date/app. by ate/app. by lectricity and plumbing date/app. by	C.O. Final date/app Pump pole date date date date	p. by Utility Po	Pool	date/app. by date/app. by date/app. by
M/H tie downs, Reconnection M/H Pole	da, blocking, e	date/app. by ate/app. by lectricity and plumbing date/app. by	C.O. Final date/app Pump pole date date date date	date/app. by p. by Utility Po	Pool	date/app. by date/app. by date/app. by
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M/H tie downs, Reconnection M/H Pole	da , blocking, e ate/app. by	date/app. by tte/app. by lectricity and plumbing date/app. by Trav	C.O. Final date/app Pump pole date vel Trailer CERTIFICATION FE	p. by Utility Po	Pool ole date/app. by Re-roof	date/app. by date/app. by date/app. by date/app. by date/app. by
M/H tie downs, Reconnection M/H Pole da BUILDING PE	ate/app. by ERMIT FEE 0.00	date/app. by tte/app. by lectricity and plumbing date/app. by Trav \$ 610.00 ZONING	C.O. Final date/app Pump pole date vel Trailer CERTIFICATION FE	date/app. by p. by Utility Polyapp. by date/app. by E \$ 12.16	Pool	date/app. by date/app. by date/app. by date/app. by date/app. by EFEE \$ 12.16
M/H tie downs, Reconnection M/H Pole dz BUILDING PE	ate/app. by ERMIT FEE 0.00 CLOPMENT	date/app. by tte/app. by lectricity and plumbing date/app. by Trav \$ 610.00 ZONING	C.O. Final date/app Pump pole date vel Trailer CERTIFICATION FE CERT. FEE \$ 50.00 DD ZONE FEE \$ 25.00	date/app. by Dility Postapp. by date/app. by EE\$ 12.16	Culvert Pool le date/app. by Re-roof SURCHARGI WAST TOT	date/app. by date/app. by date/app. by date/app. by date/app. by EFEE \$ 12.16 EFEE \$ 709.32

PERMIT

PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Warranty Deed

This Indenture, made this, January 3 Between

Inst:200812002030 Date:1/31/2008 Time:4:31 PM Doc Stamp-Deed: 59.50

DC,P.DeWitt Cason,Columbia County Page 1 of 1

FRONTIER CAPITAL, LLC, a Florida Limited Liability Company whose post office address is: 426 SW Commerce Dr., Suite 130, Lake City, FL 32025; Grantor and ISAAC CONSTRUCTION, LLC, a Florida Limited Liability Company whose post office address is: PMB 338, 2109 W. US Highway 90, Suite 170, Lake City, Florida 32025, Grantee,

(Whenever used herein the term "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth, that the grantor, for and in consideration of the sum of TEN AND NO/100 DOLLARS (\$10.00) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz:

Lot 3, HICKORY COVE, A PLANNED RESIDENTIAL DEVELOPMENT, as per the plat thereof recorded in Plat Book 9, Pages 11-13, of the Public Records of Columbia County, Florida.

The above described property does not constitute the homestead property of the grantors described herein.

N.B. Subject to an existing Mortgage to Mercantile Bank, recorded in the Public Records of Columbia County, Florida, which the grantor herein agrees to pay the existing balance thereon.

Subject to taxes for the current year, covenants, restrictions and easements of record, if any.

Parcel Identification Number: 03124-103

And the said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

In Witness Whereof, the said Grantor has caused this instrument to be executed in its name by its duly authorized officer and caused its corporate seal to be affixed the day and year first above written.

EDONTIED GADIO

	PROVIDENCAPITAL, ELC
Signed and Sealed in Our Presence:	By: Mulls m
Callielle .	CHARLES S. SPARKS, MANAGING MEMBER
Witness Print Name: Aaron Nickelson	
Berbara Chabst.	
Witness Print Name: Baybona Cluds V	
1 — 1:	By: DOC SMANNING W
Samantha Farington	ISAAC BRATKOVICH, MANAGING MEMBER
Witness Print Name Duran tha Harrington	
Callell	
Witness Print Name: Acron Wickelson	
State of Florida	
State of Florida	

County of

Columbia

The foregoing instrument was acknowledged before me this 3151 day of January, 2008, by CHARLES S. SPARKS AND ISAAC BRATKOVICH as the Managing Members of FRONTIER CAPITAL, LLC a Limited Liability Company existing under the laws of the State of Florida, on behalf of the company. He/She is personally known to me or has produced DL as identification.

Barbara C. Webster Commission # 00329279 Expires July 2, 2008

AHN, WELGHE

Columbia County Building Department Culvert Waiver

Culvert Waiver No. 000001556

DATE: 02/14/2008 BUILDING PERMIT NO.	26755	000001	
APPLICANT SAMANTHA HARRINGTON	PHONE	386.719.7143	
ADDRESS 2109 W US HWY 90,STE 170 PMB 338	LAKE CITY		32055
OWNER ISAAC CONSTRUCTION	PHONE	386.719.7143	
ADDRESS 153 SW ASHEVILLE WAY	LAKE CITY	FL	32024
CONTRACTOR ISAAC BRATKOVICH	PHONE	386.719.7143	
LOCATION OF PROPERTY 90-W TO C-341,TL TO C-242,TL	TO ASHEVILLE RD,	TR AND IT'S	
THE 3RD LOT ON L.			
SUBDIVISION/LOT/BLOCK/PHASE/UNITHICKORY COVE		3	
PARCEL ID # 25-4S-16-03124-103			
HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY	COMPLY WITH TH	E DECICION OF TH	IE COLUMBIA
	COMILET WITH THE	DECIDION OF TE	E COLUMBIA
COUNTY PURLIC WORKS DEBARTMENT IN CONNECTION WI	TH THE HEREIN PR	OPOSED APPLICA	TION.
	TH THE HEREIN PR	ROPOSED APPLICA	ATION.
	TH THE HEREIN PR	ROPOSED APPLICA	ATION.
A SEPARATE CHECK IS REQUIRED		t Paid 50.00	
SIGNATURE X James James			
	Amount		
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC PUBLIC WORKS DEPARTME	Amount	t Paid _50.00	
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC PUBLIC WORKS DEPARTME HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION	Amount	t Paid _50.00	
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC PUBLIC WORKS DEPARTME HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION	Amount ONT USE ONLY ON AND DETERMINE	t Paid _50.00	0
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC PUBLIC WORKS DEPARTME HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION CULVERT WAIVER IS:	Amount ONT USE ONLY ON AND DETERMINE	t Paid 50.00	0
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC PUBLIC WORKS DEPARTME HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION CULVERT WAIVER IS: APPROVED	Amount ONT USE ONLY ON AND DETERMINE	t Paid 50.00	0
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC PUBLIC WORKS DEPARTME HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION CULVERT WAIVER IS: APPROVED COMMENTS:	Amount ONT USE ONLY ON AND DETERMINE	NED THAT THE	0
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC PUBLIC WORKS DEPARTME HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION CULVERT WAIVER IS: APPROVED COMMENTS:	Amount INT USE ONLY ION AND DETERMIT NOT APPROV	Paid 50.00 NED THAT THE VED - NEEDS A	0

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

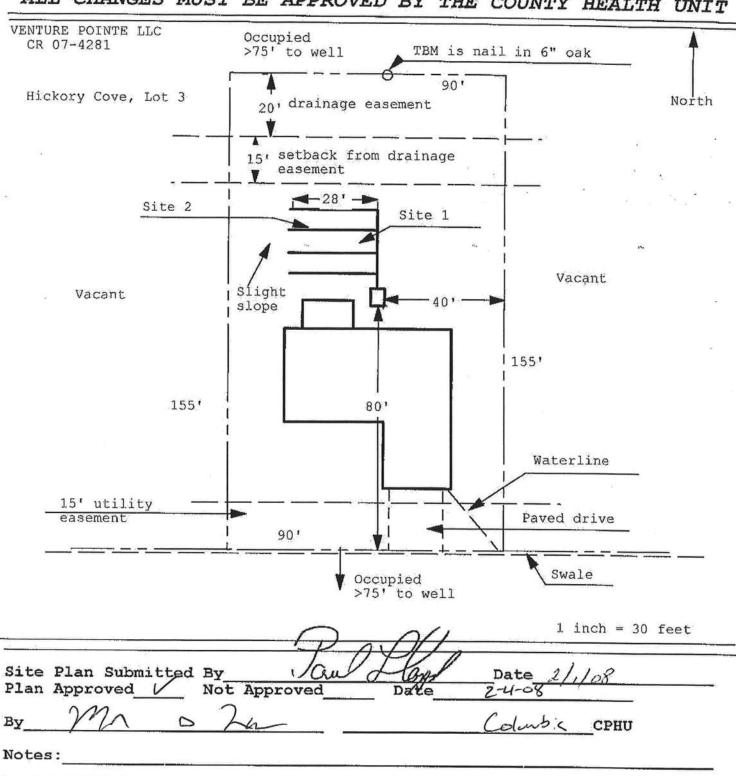
Phone: 386-758-1008 Fax: 386-758-2160





Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: 08-015

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



COLUMBIA COUNTY 9-1-1 ADDRESSING / GIS DEPARTMENT

P. O. Box 1787, Lake City, FL 32056-1787 Telephone: (386) 758-1125 * Fax: (386) 758-1365 * E-mail: ron_croft@columbiacountyfla.com

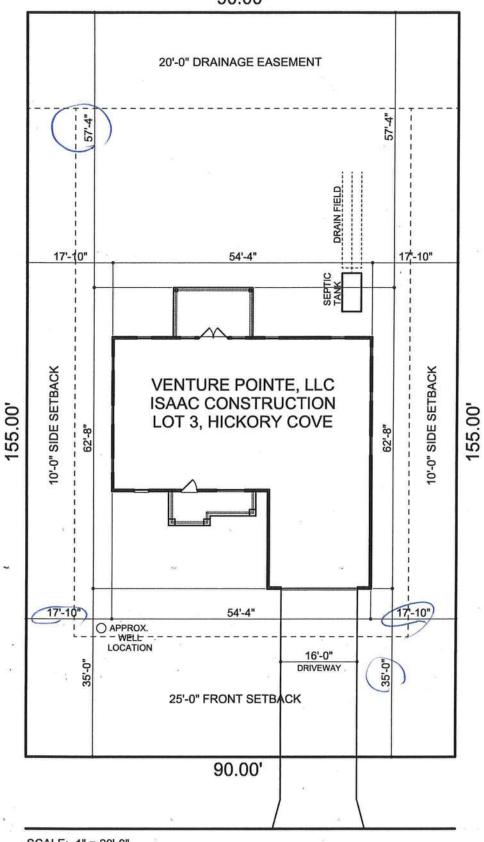
ADDRESS ASSIGNMENT DATA

The Columbia County Board of County Commissioners has passed Ordinance 2001-9, which provides for a uniform numbering system. A copy of this ordinance is available in the Clerk of Court records, located in the courthouse. This new numbering system will increase the efficiency of POLICE, FIRE AND EMERGENCY MEDICAL vehicles responding to calls within Columbia County by immediately identifying the location of the caller.

Residential or Other Structure on Parcel Number: 25-4S-16-03124-103 (LOT 3 HICKORY COVE)

Address Assignments: 153 SW ASHEVILLE WAY, LAKE CITY, FL, 32024

Any questions concerning this information should be referred to the Columbia County 9-1-1 Addressing / GIS Department at the address or telephone number above.

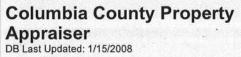


SCALE: 1" = 20'-0"

SW ASHVILLE WAY

Print

Next >>



2008 Proposed Values

Search Result: 3 of 14

Tax Record

Property Card

<< Prev

Interactive GIS Map

Parcel: 25-4S-16-03124-103

Owner & Prope	rty Info		
Owner's Name	FRONTIER C	APITAL	
Site Address	ASHEVILLE		
Mailing Address	P O BOX 356 LAKE CITY, F		
Use Desc. (code)	VACANT (00	0000)	
Neighborhood	25416.00	Tax District	2
UD Codes	MKTA01	Market Area	01
Total Land	0.320 ACRES	3	

LOT 3 HIGKORY COVE.



Dra	norty	0.	Accor	cmo	nt l	/51	1100
ELC.	perty	OL.	HSSE:	Sille	TTAY	CIL	ues

Mkt Land Value	cnt: (1)	\$17,000.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (0)	\$0.00
XFOB Value	cnt: (0)	\$0.00
Total Appraised Value		\$17,000.00

Just Value	\$17,000.00
Class Value	\$0.00
Assessed Value	\$20,000.00
Exempt Value	\$0.00
Total Taxable Value	\$20,000.00

Sales History

Area

Description

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
			NONE	·		

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
			NONE			·

Extra Features & Out Buildings

Code	Desc	Year Bit	Value	Units	Dims	Condition (% Good)
				NONE		

ı	Land Breakd	own				
	Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
	000000	VAC RES (MKT)	1.000 LT - (.320AC)	1.00/1.00/1.00/1.00	\$17,000.00	\$17,000.00

Columbia County Property Appraiser

DB Last Updated: 1/15/2008

<< Prev

3 of 14

Next >>

HTD AREA 48.888 E-RAIE 1000 INDEX 25416.00 DIST 2 PUSE 000000 VACGE RCM 86.888 E-RAIE 1000 INDX STR 25-45-16 RCM AVE ACCE 100 INDX STR 25-45-16 RCM AVE ACCE 100 INDX STR 25-45-16 RCM AVE ACCE 100 INDX STR 25-45-16 RCM	LOT 3 HICKORY COVE.	OVE.		FRONTIER CAPITAL P O BOX 3566	a	5-4S-16-03124-103		00
STATE ABCT	1	1		1	0	APPR	1/15/2008 17:40 11/29/2007 CM	BY JEFF
THE COORSE ROAD CRY.	BUSE MOD EXW *	BATH FIXT BDRM	AE?	HTD RCN %GOC	48.888	25416.00 .000	2 PC STR 25- 4S- MKT AREA 01 (PUD1	000000 VACANT
STATES S	RCVR *	C-W*		1	WAY		8	
P	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	PMTR		+ +- +- +			# CNDO # SUBD # BLK	
ALL DEPR	HTTP A/C	FUNC		+ ++ ++			# LOI # MAP#	
11	QUAL	DEPR UD-1		** **				
HE UD-5 HO TO	SIZE	12-5 18-5 18-5 18-5 18-5 18-5 18-5 18-5 18		- #- #		,	# BLD(3 TRAVERSE
DO UD-5 ND B AS UD-6 AS UD-7 B A-AREA & E-AREA & SUB VALUE # 185UED TALL EXTRA PEATURES LAND DESC CONE ROAD UD1 UD2 PROUT PILLD CK: TAND DESC CONE ROAD UD1 UD2 PROUT PILLD CK: TAND DESC CONE ROAD UD1 UD3 PRONT DEPTH FIELD CK: TAND DESC CONE ROAD UD1 UD3 PRONT DEPTH FIELD CK: TAND DESC CONE ROAD UD1 UD3 PRONT DEPTH FIELD CK: TAND DESC CONE ROAD UD1 UD3 PRONT DEPTH PIELD CK: TAND DESC CONE ROAD UD1 UD3 PRONT	ARCH	3BE		+ ++ +			H= 44= 1	
AS UD-8 ND-9 RAD RAD RAD RAD RAD RAD RAD RA	KTCH	9-66		+ ** +				
ND ## NUMBER ## NUMBER ## NUMBER ## NUMBER ## NUMBER ## ISSUED ## NUMBER ## ISSUED ## PRICE ## NUMBER ## NUMBE	CLAS	8-6-6 8-6-6		- 11- 11			· · ·	
FAL EXTRA FEATURES LAND DESC LODE TOPO UTIL (UD2 (UD4 BACK DT 1.00 1.00 1.00 LT 17000.000 17000.00 LT 17000.000 LT 17000.0	A-AREA	E-AREA	SUB VALU	· ++ ++ EI			DESC	AMT ISSUED
## GRANTOR ## GRANTEE							PAGE	ALE
TAL EXTRA FEATURES LAND DESC LEN WID HGHT QTY QL YR ADJ LAND DESC ZONE ROAD {UD1 }UD2 {UD4 BACK DT ADJUSTWENTS UNITS UT PRICE ADJ UT PR SPCD % %GOOD XFOB CODE TOPO UTIL {UD2 }UD4 BACK DT ADJUSTWENTS UNITS UT PRICE ADJ UT PR LAND VA UNITS UT PRICE ADJ UT PR LAND VA 1,001.001.001.001.001.001.001.001.001.00				+ + +			# GRANTEE	
BN CODE DESC LEN WID HGHT QTY QL YR ADJ UNITS UT PRICE ADJ UT PR SPCD % %GOOD XFOB LAND DESC ZONE ROAD {UD1 {UD3 FRONT DEPTH FIELD CK: CODE TOPO UTIL {UD2 {UD4 BACK DT ADJUSTWENTS UNITS UT PRICE ADJ UT PR LAND VA 00020 0003 1.00 1.00 1.00 LT 17000.000 17000.00 17000.00	TOTAL			- #			# GRANTOR GRANTEE	
LAND DESC ZONE ROAD {UD1 {UD3 FRONT DEPTH FIELD CK: CODE TOPO UTIL {UD2 {UD4 BACK DT ADJUSTMENTS UNITS UT PRICE ADJ UT PR LAND 1.00 1.00 1.00 L.00 LT 17000.000 17000.00 1.00 2.00 1.00 L.00 LT 17000.00	CODE	ATURES DESC		WID HGHT QTY QL	FIELD CK: ADJ	Б	PR SPCD	%GOOD XFOB
	CODE CODE	S RS	ROAD UTIL 0007 0003		FIELD CK: ADJUSTMENTS 1.00 1.00 1.00		UT PRICE AD	PR LAND

Project Name:

Isaac Construction

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

Venture Pointe LLC - Model 1735

City, State: Owner: Climate Zone:	Lot: 3, Sub: Hic Lake City, FL 32 Spec House North	Rory Cove, Plat: 025-	Permitting Office: Permit Number: Jurisdiction Number:	ZZ/600	
New construction Single femile		New _	12. Cooling systems	G 22.01 P. 4	
 Single family or r Number of units. 		Single family	a. Central Unit	Cap: 32.0 kBtu/hr	-
 Number of units, Number of Bedro 		1 —	h N/A	SEER: 13.00	-
5. Is this a worst cas	07-07:00	3	b. N/A		-
6. Conditioned floor		No 1735 ft²	- N/A		
	rea: (Label reqd. by 13-1		c. N/A		-
a. U-factor:			13. Heating systems		-
	uble DEFAULT) 7a.(Db	Description Area	a. Electric Heat Pump	Cap: 32.0 kBtu/hr	
b. SHGC:	able DEFACET) 7a.(DE	ble Default) 153.0 ft-	a. Electric Heat Fump	HSPF: 7.70	-
(or Clear or Tint	t DEFAULT) 7b.	(Clear) 153.0 ft ²	b. N/A	11311.7.70	1
8. Floor types	(BEINGEL) 70.	(Clear) 155.0 II	U. IVIA		-
a. Slab-On-Grade E	dge Insulation	R=5.0, 194.0(p) ft	c. N/A		-
b. N/A	8				-
c. N/A		_	14. Hot water systems		
Wall types		_	a. Electric Resistance	Cap: 50.0 gallons	1
a. Frame, Wood, Ex	terior	R=13.0, 1083.0 ft ²		EF: 0.90	=
b. Frame, Wood, Ac	ljacent	R=13.0, 150.0 ft ²	b. N/A		
c. N/A	Z.	_			
d. N/A		-	c. Conservation credits		
e. N/A			(HR-Heat recovery, Solar		
Ceiling types			DHP-Dedicated heat pump)		
a. Under Attic		R=30.0, 1850.0 ft ²	15. HVAC credits	PT,	_
b. N/A		_	(CF-Ceiling fan, CV-Cross ventilation	n,	
c. N/A		_	HF-Whole house fan,		
Ducts(Leak Free)		_	PT-Programmable Thermostat,		
a. Sup: Unc. Ret: U	nc. AH: Interior	Sup. R=6.0, 25.0 ft	MZ-C-Multizone cooling,		
b. N/A		_	MZ-H-Multizone heating)		
		_			
Glas	ss/Floor Area: 0.0	Total as-built p	points: 18135 points: 23498	S	
I hereby certify that	the plans and specific	cations covered by	Review of the plans and	THE STA	

this calculation are in compliance with the Florida Energy specifications covered by this Code. calculation indicates compliance with the Florida Energy Code. PREPARED BY: Before construction is completed this building will be inspected for compliance with Section 553.908 I hereby certify that this building, as designed, is in compliance Florida Statutes. with the Florida Energy Code. OWNER/AGENT: _____ BUILDING OFFICIAL: DATE:

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCPB v4.5.2)

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

BASE		AS-BUILT		
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area		verhang t Len Hgt Area X S	SPM X SOF:	= Points
.18 1735.0 18.59 5806.0	1.Double, Clear	W 1.5 8.0 60.0	38.52 0.96	2214.0
	NOTE AND	W 11.5 8.0 40.0	38.52 0.46	702.0
¥1		E 1.5 8.0 40.0	42.06 0.96	1611.0
		S 1.5 8.0 8.0	35.87 0.92	264.0
	5.Double, Clear	S 1.5 8.0 5.0	35.87 0.92	165.0
	As-Built Total:	153.0		4956.0
WALL TYPES Area X BSPM = Points	Туре	R-Value Area	X SPM =	Points
Adjacent 150.0 0.70 105.0	1. Frame, Wood, Exterior	13.0 1083.0	1.50	1624.5
Exterior 1083.0 1.70 1841.1	2. Frame, Wood, Adjacent	13.0 150.0	0.60	90.0
Base Total: 1233.0 1946.1	As-Built Total:	1233.0		1714.5
DOOR TYPES Area X BSPM = Points	Туре	Area	X SPM =	Points
Adjacent 18.0 2.40 43.2	1.Exterior Insulated	20.0	4.10	82.0
Exterior 20.0 6.10 122.0	2.Adjacent Insulated	18.0	1.60	28.8
Base Total: 38.0 165.2	As-Built Total:	38.0		110.8
CEILING TYPES Area X BSPM = Points	Туре	R-Value Area X SF	PM X SCM =	Points
Under Attic 1735.0 1.73 3001.6	1. Under Attic	30.0 1850.0 1.7	73 X 1.00	3200.5
Base Total: 1735.0 3001.6	As-Built Total:	1850.0		3200.5
FLOOR TYPES Area X BSPM = Points	Туре	R-Value Area	X SPM =	Points
Slab 194.0(p) -37.0 -7178.0	1. Slab-On-Grade Edge Insulation	5.0 194.0(p	-36.20	-7022.8
Raised 0.0 0.00 0.0		v		
Base Total: -7178.0	As-Built Total:	194.0		-7022.8
INFILTRATION Area X BSPM = Points		Area 2	X SPM =	Points
1735.0 10.21 17714.3		1735.0	10.21	17714.3

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

	BASE		AS-BUILT	
Summer Ba	se Points: 2	1455.2	Summer As-Built Points:	20673.3
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	= Cooling Points
21455.2	0.3250	6972.9	(sys 1: Central Unit 32000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS) 20673 1.00 (1.09 x 1.000 x 0.91) 0.260 0.950 20673.3 1.00 0.992 0.260 0.950	5065.0 5065.0

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

BASE		AS-	BUILT			
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Type/SC C	Overhang Ornt Len	Hgt Area X	(WPM X	WOF	= Points
.18 1735.0 20.17 6299.0	1.Double, Clear	W 1.5	8.0 60.0	20.73	1.01	1257.0
	2.Double, Clear	W 11.5	8.0 40.0	20.73	1.20	995.0
	3.Double, Clear	E 1.5	8.0 40.0	18.79	1.02	766.0
	4.Double, Clear	S 1.5			1.04	110.0
e e	5.Double, Clear	S 1.5	8.0 5.0	13.30	1.04	69.0
	As-Built Total:		153.0			3197.0
WALL TYPES Area X BWPM = Points	Туре	R-	Value Are	a X WPN	<i>1</i> =	Points
Adjacent 150.0 3.60 540.0	1. Frame, Wood, Exterior	0	13.0 1083.0	3.40		3682.2
Exterior 1083.0 3.70 4007.1	2. Frame, Wood, Adjacent		13.0 150.0	3.30		495.0
Base Total: 1233.0 4547.1	As-Built Total:		1233.0			4177.2
DOOR TYPES Area X BWPM = Points	Туре		Area	X WPN	1 =	Points
Adjacent 18.0 11.50 207.0	1.Exterior Insulated		20.0	8.40		168.0
Exterior 20.0 12.30 246.0	2.Adjacent Insulated		18.0	8.00		144.0
Base Total: 38.0 453.0	As-Built Total:		38.0			312.0
CEILING TYPES Area X BWPM = Points	Туре	R-Value	Area X V	VPM X WO	CM =	Points
Under Attic 1735.0 2.05 3556.8	1. Under Attic	5 8 8	30.0 1850.0	2.05 X 1.00		3792.5
Base Total: 1735.0 3556.8	As-Built Total:		1850.0			3792.5
FLOOR TYPES Area X BWPM = Points	Туре	R-	Value Area	a X WPN	1 =	Points
Slab 194.0(p) 8.9 1726.6	Slab-On-Grade Edge Insulat	tion	5.0 194.0(p	7.60		1474.4
Raised 0.0 0.00 0.0			sometime recently was the			anno tiú i stieri).
Base Total: 1726.6	As-Built Total:		194.0	7.		1474.4
INFILTRATION Area X BWPM = Points			Area	X WPM	1 =	Points
1735.0 -0.59 -1023.6			1735	.0 -0.59		-1023.6

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

	BASE		AS-BUILT
Winter Base	Points:	15558.8	Winter As-Built Points: 11929.
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Heatin Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)
15558.8	0.5540	8619.6	(sys 1: Electric Heat Pump 32000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Int(AH),R6.0 11929.5 1.000 (1.069 x 1.000 x 0.93) 0.443 0.950 4989.6 11929.5 1.00 0.994 0.443 0.950 4989.6

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 3, Sub: Hickory Cove, Plat: , Lake City, FL, 32025- PERMIT #:

	E	BASE						Α	S-BUII	_T		
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier	Credit Multiplie	
3		2635.00		7905.0	50.0	0.90	3		1.00	2693.56	1.00	8080.7
					As-Built To	otal:						8080.7

				CODE	C	OMPLI	ANCE	Sī	TATUS	S	2		
BASE													
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+		+		=	
6973	×	8620		7905		23498	5065		4990		8081		18135

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 3, Sub: Hickory Cove, Plat: , Lake City, FL, 32025-

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	-
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

Tested sealed ducts must be certified in this house.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 88.8

The higher the score, the more efficient the home.

Spec House, Lot: 3, Sub: Hickory Cove, Plat: , Lake City, FL, 32025-

	ARC		:50			
1.	New construction or existing	New		12. Cooling systems		
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 32.0 kBtu/hr	-
3.	Number of units, if multi-family	1			SEER: 13.00	
4.	Number of Bedrooms	3		b. N/A		-
5.	Is this a worst case?	No	1			
6.	Conditioned floor area (ft2)	1735 ft ²		c. N/A	5 6	-
7.	Glass type 1 and area: (Label reqd.	by 13-104.4.5 if not default)	11.			-
8	a. U-factor:	Description Area		13. Heating systems	3	
	(or Single or Double DEFAULT)	7a. (Dble Default) 153.0 ft ²		a. Electric Heat Pump	Cap: 32.0 kBtu/hr	
ł	o. SHGC:				HSPF: 7.70	
	(or Clear or Tint DEFAULT)	7b. (Clear) 153.0 ft ²		b. N/A		
8.	Floor types	V-1-1-1	_			
8	a. Slab-On-Grade Edge Insulation	R=5.0, 194.0(p) ft		c. N/A		
ł	o. N/A		_			
(. N/A			14. Hot water systems		
9.	Wall types			a. Electric Resistance	Cap: 50.0 gallons	
8	a. Frame, Wood, Exterior	R=13.0, 1083.0 ft2			EF: 0.90	
ŀ	o. Frame, Wood, Adjacent	R=13.0, 150.0 ft ²		b. N/A		
C	:. N/A		_			
C	I. N/A			c. Conservation credits		
e	. N/A			(HR-Heat recovery, Solar		
10.	Ceiling types			DHP-Dedicated heat pump)		
a	. Under Attic	R=30.0, 1850.0 ft ²		15. HVAC credits	PT,	
t	o. N/A		_	(CF-Ceiling fan, CV-Cross ventilation,	•	
C	. N/A			HF-Whole house fan,		
11.	Ducts(Leak Free)			PT-Programmable Thermostat,		
а	. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 25.0 ft		MZ-C-Multizone cooling,		
b	o. N/A		_	MZ-H-Multizone heating)		
				en valoritation en entrecon construire en 150 a 60 a 60 a 70 a 70 a 70 a 70 a 70 a 7		
	ertify that this home has complie				THE CT	
Co	nstruction through the above en	ergy saving features which	h will b	e installed (or exceeded)	OF	À
	this home before final inspection					Be
	sed on installed Code compliant			100 miles 100 mi	A MARINE	112
	ilder Signature:		Date:		RS TO THE REPORT OF THE REPORT	DA.
Ad	dress of New Home:		City/F	L Zip:	THE TRUST	

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar Mesignation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

City/FL Zip:

COD WE TE

Energy Code Compliance

Duct System Performance Report

Project Name:

Venture Pointe LLC - Model 1735

Address:

City, State:

Lake City, FL 32025-

Owner:

Spec House

Climate Zone:

North

Builder:

Permitting Office:

Permit Number:

Jurisdiction Number:

Isaac Construction

Columbia County

Total Duct System Leakage Test Results

CFM2	25 Total Duct Leal	kage Test Values	
Line	System	Duct Leakage Total	Duct Leakage to Outdoors
1	System1	cfm25(tot)	cfm25(out)
2	System2	cfm25(tot)	cfm25(out)
3	System3	cfm25(tot)	cfm25(out)
4	System4	cfm25(tot)	cfm25(out)
5	Total House Duct System Leakage	Sum lines 1-4 Divide by (Total Conditioned Floor Area) =(Q _n ,tot) Receive credit if Q _n ,tot≤ 0.03	Sum lines 1-4

I hereby certify that the above duct testing performance results demonstrate compliance with the Florida Energy Code requirements in accordance with Section 610.1.A.1, Florida Building Code, Building Volume, Chapter 13 for leak free duct system credit.

Signature: _____
Printed Name: _____
Florida Rater Certification #: _____

DATE:

Florida Building Code requires that testing to confirm leak free duct systems be performed by a Class 1 Florida Energy Gauge Certified Energy Rater. Certified Florida Class 1 raters can be found at: http://energygauge.com/search.htp



BUILDING OFFICIAL: ______

Columbia County Building Department

NOTICE TO PERMITEE: (Pursuant to SS 713.135)

AS A CONDITION OF THE ISSUANCE OF A PERMIT, YOU MUST PROVIDE A COPY OF THIS NOTICE TO THE

PROPERTY OWNER.

Permitee, Printed Name

Permitee Signature

Date

STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION

FLORIDA'S CONSTRUCTION LIEN LAW

PROTECT YOURSELF AND YOUR INVESTMENT

According to Florida Law, those who work on your property or provide materials, and are not paid-infull, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, **even if you have paid your contractor in full.**

This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

This document explains Florida Statute 713, Part 1, as it pertains to home construction and remodeling, and provides tips on how you can avoid construction liens on your property.

Protecting Yourself

If you hire a contractor and the improvements cost more than \$2,500, you should know the following:

- You may be liable if you pay your contractor and he then fails to pay his suppliers or contractors.
 There is a way to protect yourself: a Release of lien. Before you make any payment, be sure you
 receive this waiver from suppliers and subcontractors covering the materials used and work
 performed.
- Request from the contractor, via certified or registered mail, a list of all subcontractors and suppliers who have a contract with the contractor to provide services or materials to your property.
- If your contract calls for partial payments before the work is completed, gets a Partial Release of Lien covering all workers and materials used to that point.
- Before you make the last payment to your contractor, obtain and affidavit that specifies all
 unpaid parties who performed labor, services or provided materials to your property. Make
 sure that your contractor obtains releases from these parties before you make the final
 payment.
- Always file a Notice of Commencement before beginning a home construction or remodeling
 project. The local authority that issues building permits is required to provide this form. You
 must record the form with the Clerk of the Circuit Court in the county where the property being
 improved is located. Also post a certified copy at the job site. (In lieu of a certified copy, you
 may post an affidavit stating that a Notice of commencement has been recorded. Attach a copy
 of the Notice of commencement to the affidavit.)

Page 2 of 4 FLORIDA'S CONSTRUCTION LILEN LAW

 In addition, the building department is prohibited from performing the first inspecti9on if the Notice of Commencement is not also filed with the building department. You can also supply a notarized statement that the Notice has been filed, with a copy attached.

DBPR Customer Contact Center 1940 North Monroe Street Tallahassee, Florida 32399-1027

Website: http://www.myflolrida.com/dbpr/

Phone

850 487-1395 Fax: 850 488-1830

Email

CallCenter@dbpr.state.fl.us

INTERNET

www.MyFlorida.com

The Notice of Commencement notes the intent to begin improvements, the location of the property, description of the work and the amount of bond (if any). It also identifies the property owner, contractor, surety, lender and other pertinent information. Failure to record a Notice of Commencement or incorrect information of the Notice could contribute to your having to pay twice for the same work or materials.

Whose Responsibility Is It To Get These Releases?

You can stipulate in the agreement with your contractor that he must provide all releases of lien. If it is not a part of the contract, however, or you act as your own contractor, YOU must get the releases. If you borrow money to pay for the improvements and the lender pays the contractor(s) directly, instruct the lender to get releases before making any payments. If your lender then fails to follow the legal requirements, the lending institution may be responsible to you for any loss.

What Can Happen If I Don't Get Releases of Lien?

You will not be able to sell your property unless all outstanding liens are paid. Sometimes a landowner can even be forced to sell his property to satisfy a lien.

Who Can Claim a Lien on My property?

Contractors, laborers, material suppliers, subcontractors and professionals such as architects, landscape architects, interior designers, engineers or land surveyors all have the right to file a claim of lien for work or materials. Always get a release of lien from anyone who does work on your home.

Additional Tips on Home Construction

- Verify that your contractor is properly licensed. Information regarding licensing can be found below.
- If you intend to get financing, consult with your lender or an attorney before recording your
 Notice of Commencement.
- Insist that the contractor/remodeler secures a building permit and adheres to all building codes and ordinances.

Information All Construction Contracts Should Contain

- The contractor's name, address, telephone number and contractor's license number.
- A precise description of work and materials to be supplied. The contract should specify the
 grade of construction, flooring and trim materials to be used. Don't accept the phrase "or
 equivalent", the contract should specify appliance models and alternates for models not
 available.
- A beginning date.
- A completion date.
- A complete list of companies or individuals supplying the contractor with labor or materials. Be sure they are insured so you are protected against theft or damage to their supplies or work.
- Financing information and the payment schedule.
- All necessary building permits or licenses.
- · Agreement regarding site clean-up and debris disposal.
- All warranty agreements.

Ask for explanations and clarifications of legal terms or confusing language. Be sure you understand completely what you are signing: **Remember**, promises are difficult to enforce unless they are in writing. Even for small jobs, have a written contract spelling out the details. Be wary of anyone who says, "We don't need to bother putting it in writing." Some contractors require a down payment of 10-30 percent of the total and an additional payment at the halfway point. Pay only when the work is done to your satisfaction and you have releases of lien as described above. If the completion date if critical, like a swimming pool planned for summertime use, link payment to on-time performance. Changes to a contract after construction has begun can cost you.

Specify in the contract how changes are to be handled and insist that all change orders be in writing and signed by both you and the contractor.

Cancellation of Contracts

Some home repair/improvement contracts can be canceled in writing (preferably by certified mail? Without penalty or obligation by midnight of the third business day after signing. They include:

- Those signed anywhere other than the seller's normal place of business.
- Those signed as a result of door-to-door solicitation, except emergency home repairs.
- Those paid on an installment basis. Other contracts are binding as soon as they are signed, so be sure before you sign.

Page 4 of 4 FLORIDA'S CONSTRUCTION LIEN LAW

Things You Should Know Before Starting

The most frequently cited complaints concerning home remodeling; home improvements and home repair are cost overruns, missed deadlines and inferior workmanship. Another persistent problem is "fly-bynight" contractors who take deposits or payments before finishing or starting work. When you need something done to your home, choose a contractor carefully. Be wary of door-to-door salespeople and telephone solicitors promising "this-month-only" bargains. Make sure your contractor is properly licensed and insured. The Construction Lien Law is complex and cannot be covered completely in this document. We recommend that whenever a specific problem arises, you consult an attorney.

To register a complaint (or to learn if Complaints have been filed against a prospective contractor)

Call:

Florida Department of Business and Professional Regulation, Customer Contact Center 850 487-1395

Email:

CallCenter@dbpr.state.fl.us

Write:

Florida Department of Business and Professional Regulation 1940 North Monroe Street Tallahassee, Florida 32399-1027

Or go online to: www.MyFlorida.com

Click on Business and Professional Licenses

To check al license on the Internet 24 hours a day, please visit www.MyFlorida.com and click on Business and Professional Licenses, then Search for a Licensee.

License verification is available 24/7 by calling our Customer Contract Center at 850 487-1395 You may also contact your local building department or the Better Business Bureau.



Project Information for:

L267271

Builder:

Isaac Construction

Lot:

Subdivision:

Hickory Cove Columbia

County:

Truss Count:

Design Program: MiTek 20/20 6.3 **Building Code:** FBC2004/TPI2002

Truss Design Load Information: Gravity:

Wind:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

> Isaac P. Bratkovich Florida License No. CBC059323 Address: 1005 Southwest Walter Avenue Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1924498	CJ1	1/9/08
2	J1924499	CJ3	1/9/08
3	J1924500	CJ5	1/9/08
4	J1924501	EJ7	1/9/08
5	J1924502	EJ7A	1/9/08
6	J1924503	EJ7B	1/9/08
7	J1924504	EJ7C	1/9/08
8	J1924505	EJ7D	1/9/08
9	J1924506	EJ7G	1/9/08
10	J1924507	HJ9	1/9/08
11	J1924508	T01	1/9/08
12	J1924509	T01G	1/9/08
13	J1924510	T02	1/9/08
14	J1924511	T02G	1/9/08
15	J1924512	T03	1/9/08
16	J1924513	T04	1/9/08
17	J1924514	T05	1/9/08
18	J1924515	T06	1/9/08
19	J1924516	T06G	1/9/08
20	J1924517	T07	1/9/08
21	J1924518	T08	1/9/08
22	J1924519	T09	1/9/08
23	J1924520	T09G	1/9/08
24	J1924521	T10G	1/9/08
25	J1924522	T11G	1/9/08
26	J1924523	T12	1/9/08
27	J1924524	T13	1/9/08





Project Information for: L267271

Builder:

Isaac Construction

Lot:

3

Subdivision:

Hickory Cove Columbia

County: Truss Count:

27

Design Program: MiTek 20/20 6.3 **Building Code:**

FBC2004/TPI2002

Truss Design Load Information: Gravity:

Wind Standard: ASCE 7-02

Wind Exposure: B

Roof (psf): 42.0 Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

Isaac P. Bratkovich Florida License No. CBC059323

Address: 1005 Southwest Walter Avenue Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

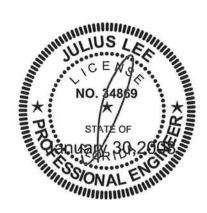
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1924498	CJ1	1/9/08
2	J1924499	CJ3	1/9/08
3	J1924500	CJ5	1/9/08
4	J1924501	EJ7	1/9/08
5	J1924502	EJ7A	1/9/08
6	J1924503	EJ7B	1/9/08
7	J1924504	EJ7C	1/9/08
8	J1924505	EJ7D	1/9/08
9	J1924506	EJ7G	1/9/08
10	J1924507	HJ9	1/9/08
11	J1924508	T01	1/9/08
12	J1924509	T01G	1/9/08
13	J1924510	T02	1/9/08
14	J1924511	T02G	1/9/08
15	J1924512	T03	1/9/08
16	J1924513	T04	1/9/08
17	J1924514	T05	1/9/08
8	J1924515	T06	1/9/08
19	J1924516	T06G	1/9/08
20	J1924517	T07	1/9/08
21	J1924518	T08	1/9/08
22	J1924519	T09	1/9/08
23	J1924520	T09G	1/9/08
24	J1924521	T10G	1/9/08
25	J1924522	T11G	1/9/08
26	J1924523	T12	1/9/08
27	J1924524	T13	1/9/08





Job Truss Truss Type Qty Ply J1924498 CJ₁ JACK 1 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 09 11:39:55 2008 Page 1

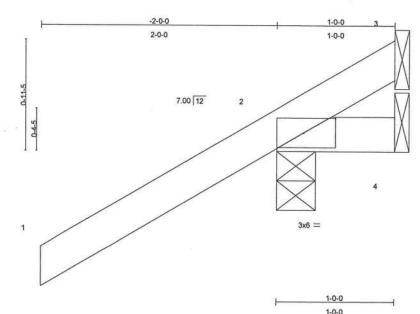


Plate Of	fsets (X,)	(): [2:0-3-3,0-1-8]										
LOADIN	IG (psf) 20.0	SPACING Plates Increase	2-0-0 1.25	CSI	0.27	DEFL Vert(LL)	in -0.00	(loc)	l/defl >999	L/d 360	PLATES	GRIP
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240	MT20	244/190
BCLL BCDL	10.0 5.0	* Rep Stress Incr Code FBC2004/TF	YES PI2002	WB (Mat	0.00 rix)	Horz(TĹ)	0.00	3	n/a	n/a	Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=257/0-4-0, 4=5/Mechanical, 3=-91/Mechanical

Max Horz 2=101(load case 6)

Max Uplift 2=-296(load case 6), 4=-11(load case 4), 3=-91(load case 1) Max Grav 2=257(load case 1), 4=14(load case 2), 3=137(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-78/87

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 9,2008

Scale = 1:9.2

🔬 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply		
	CJ1	JACK	4	1		J1924498
				120	Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

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NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2, 11 lb uplift at joint 4 and 91 lb uplift at joint 3.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Plenda PE No. 24868 1 109 Coastal Bay Blvd

January 9,2008



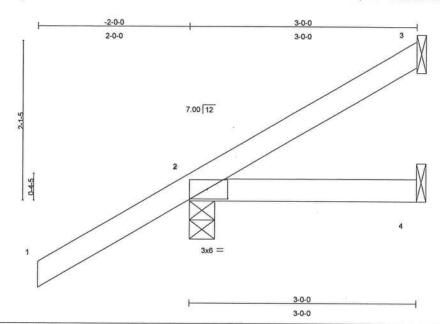
 Job
 Truss
 Truss Type
 Qty
 Ply

 CJ3
 JACK
 4
 1

 Job Reference (optional)
 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=251/0-4-0, 4=14/Mechanical

Max Horz 2=154(load case 6)

Max Uplift 3=-30(load case 7), 2=-237(load case 6), 4=-33(load case 4) Max Grav 3=31(load case 4), 2=251(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-65/15

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.12

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Les Truss Cesign Engineer Florida PE No. 34868 1109 Ceastel Bay Blvd

January 9,2008

Scale = 1:14.3

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply		
	CJ3	JACK	4	1	J192	4499
			17.0	- 1	Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

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NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 3, 237 lb uplift at joint 2 and 33 lb uplift at joint 4.

LOAD CASE(S) Standard

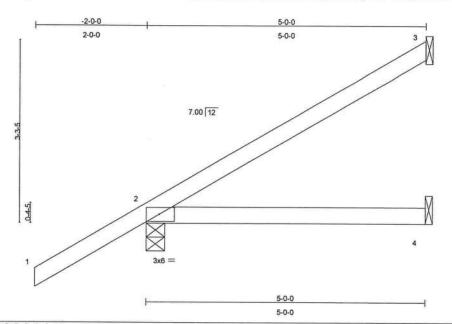
January 9,2008



Job Truss Ply Truss Type Qty J1924500 CJ5 **JACK** Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>671	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)						Weight: 20 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=296/0-4-0, 4=24/Mechanical

Max Horz 2=207(load case 6)

Max Uplift 3=-95(load case 6), 2=-252(load case 6), 4=-56(load case 4) Max Grav 3=102(load case 1), 2=296(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-81/40

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 9,2008

Scale = 1:19.5

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building occe. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Jóp	Truss	Truss Type	Qty	Ply		
	CJ5	JACK	4	1		J1924500
					Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

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NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 3, 252 lb uplift at joint 2 and 56 lb uplift at joint 4.

LOAD CASE(S) Standard

January 9,2008



Job Truss Ply Truss Type Qty J1924501 EJ7 MONO TRUSS 9 1 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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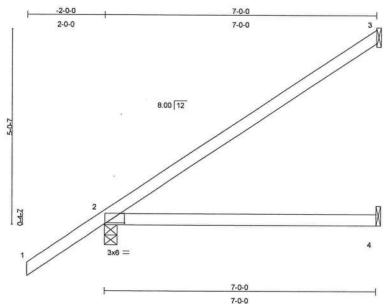


Plate Of	fsets (X, Y	(): [2:0-6-3,0-0-6]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.08	2-4	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.16	2-4	>497	240	20	211110
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat		1.0.2(,2)	0.00		1110	1110	Weight: 27 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or

6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=153/Mechanical, 2=352/0-4-0, 4=45/Mechanical

Max Horz 2=215(load case 6)

Max Uplift 3=-102(load case 6), 2=-120(load case 6)

Max Grav 3=153(load case 1), 2=352(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-130/66

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.59

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 Coltinuet shiping and 120 lb uplift at joint 2.

January 9,2008

Scale = 1:28 0

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building does. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, Wi 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, Wi 53719



Jóp	Truss	Truss Type	Qty	Ply		1.4120200000000
	EJ7	MONO TRUSS	9	1		J1924501
L					Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

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LOAD CASE(S) Standard

Julius Lee Truss Cosign Engineer Florida PE No. 24869 1109 Coestel Bay Blvd

January 9,2008





Job Truss Truss Type Qty Ply J1924502 EJ7A **SPECIAL** 1 Job Reference (optional)

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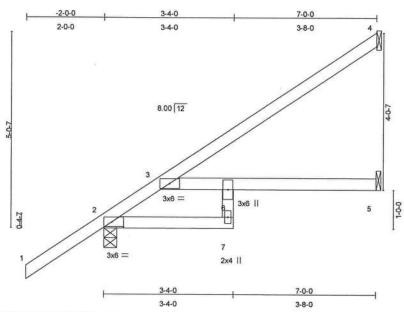


Plate Offsets (X,Y): [2:0-3-9,0-1-8] LOADING (psf) **SPACING** 2-0-0 CSI DEFL L/d **PLATES** in (loc) I/defl GRIP TCLL 20.0 Plates Increase 1.25 TC 0.30 Vert(LL) 0.09 5-6 >858 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.53 Vert(TL) -0.175-6 >492 240 BCLL 10.0 Rep Stress Incr WB YES 0.00 Horz(TL) 0.05 5 n/a n/a BCDL Code FBC2004/TPI2002 (Matrix) Weight: 31 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

6-7 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=133/Mechanical, 2=364/0-4-0, 5=71/Mechanical

Max Horz 2=215(load case 6)

Max Uplift 4=-86(load case 6), 2=-113(load case 6), 5=-2(load case 6) Max Grav 4=133(load case 1), 2=364(load case 1), 5=111(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-278/0, 3-4=-106/57

BOT CHORD 2-7=-118/177, 6-7=0/50, 3-6=-177/118, 5-6=0/0

JOINT STRESS INDEX

2 = 0.52, 3 = 0.69, 6 = 0.57 and 7 = 0.43

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb Complified in the 13 lb uplift at joint 2 and 2 lb uplift at joint 5.

January 9,2008

Scale = 1:28.0

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters show for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Jóp	Truss	Truss Type	Qty	Ply	
	EJ7A	SPECIAL	4	1	J1924502
	VC				Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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LOAD CASE(S) Standard

January 9,2008



Truss Job Ply Truss Type Qty J1924503 EJ7B SPECIAL 1 Job Reference (optional)

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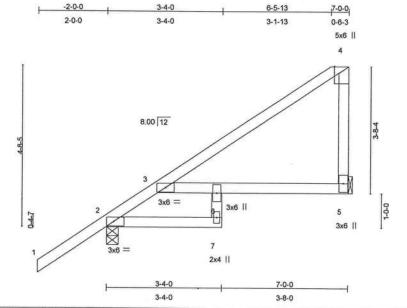


Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-2-6,Edge]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.05	5-6	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.08	5-6	>988	240	111120	211/10
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mati	rix)			-		111.00	Weight: 35 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

6-7 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 5=195/Mechanical, 2=350/0-4-0

Max Horz 2=213(load case 6)

Max Uplift 5=-89(load case 6), 2=-120(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-240/0, 3-4=-144/18, 4-5=-116/138 BOT CHORD 2-7=-100/139, 6-7=0/48, 3-6=-83/54, 5-6=-47/56

JOINT STRESS INDEX

2 = 0.49, 3 = 0.62, 4 = 0.41, 5 = 0.26, 6 = 0.44 and 7 = 0.36

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb Complified ining and 120 lb uplift at joint 2.

January 9,2008

Scale = 1:31.5

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Jóp	Truss	Truss Type	Qty	Ply		Marie Sol Control Herman Bernard
	EJ7B	SPECIAL	1	1		J1924503
				20.	Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

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LOAD CASE(S) Standard

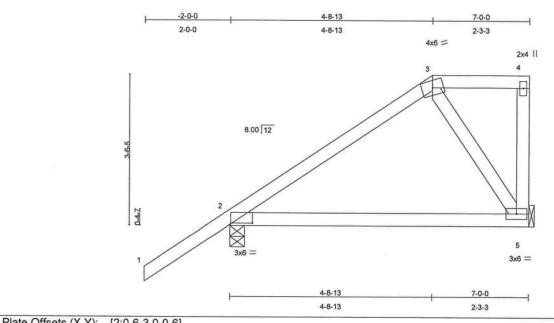
January 9,2008



Job Truss Truss Type Qty Ply J1924504 EJ7C MONO HIP 1 Job Reference (optional)

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.04	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.08	2-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3 BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=195/Mechanical, 2=350/0-4-0

Max Horz 2=168(load case 6)

Max Uplift 5=-57(load case 5), 2=-143(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-196/39, 3-4=-35/0, 4-5=-26/44

BOT CHORD

2-5=-91/100

WEBS

3-5=-164/186

JOINT STRESS INDEX

2 = 0.60, 3 = 0.21, 4 = 0.23 and 5 = 0.41

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 9,2008



jop	Truss	Truss Type	Qty	Ply		
	EJ7C	MONO HIP	1	1		J1924504
	12.00mm (12.00mm)				Job Reference (optional)	

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NOTES

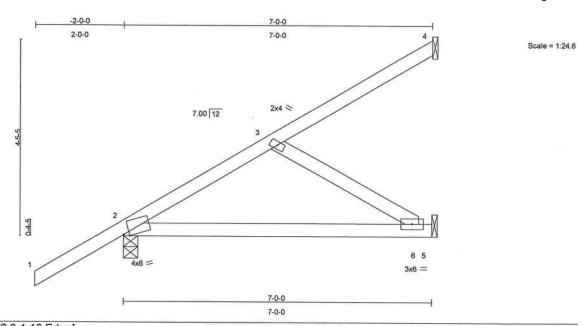
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 5 and 143 lb uplift at joint 2.

LOAD CASE(S) Standard



Jóp	Truss	Truss Type	Qty	Ply		5 9559254.5
	EJ7D	MONO TRUSS	2	1		J1924505
					Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	0.18	2-6	>462	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.11	2-6	>755	240		_ , ,, , , ,
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mati	rix)			8	100000	33.00.00	Weight: 32 lb	

 LUMBER

 TOP CHORD
 2 X 4 SYP No.2

 BOT CHORD
 2 X 4 SYP No.1D

 WEBS
 2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=80/Mechanical, 2=352/0-4-0, 5=118/Mechanical

Max Horz 2=188(load case 6)

Max Uplift 4=-51(load case 6), 2=-216(load case 6), 5=-115(load case 6) Max Grav 4=80(load case 1), 2=352(load case 1), 5=128(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-239/68, 3-4=-57/31

BOT CHORD 2-6=-226/165, 5-6=0/0

WEBS 3-6=-192/263

JOINT STRESS INDEX

2 = 0.80, 3 = 0.14 and 6 = 0.07

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

86 All hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Truss Design Engineer Florida PE No. 34868 1 100 Ceastal Bay Blvd Boynton Beach, Ft. 33435

January 9,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply		
	EJ7D	MONO TRUSS	2	1		J1924505
		The second secon		51	Job Reference (optional)	

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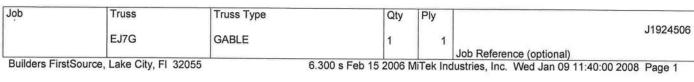
NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 4, 216 lb uplift at joint 2 and 115 lb uplift at joint 5.

LOAD CASE(S) Standard

Ne Lee Useligh Engineer Inda PE No. 34866 93 Castel Bay Blyd Vnton Uesch, FL 33436





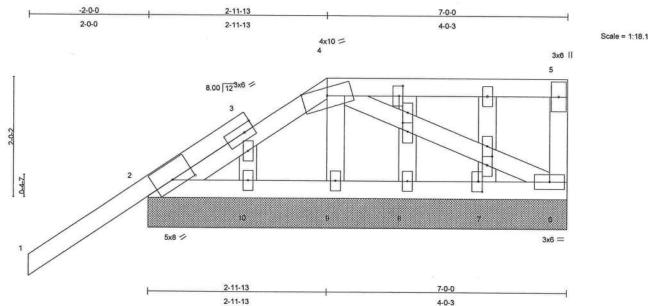


Plate Of	ffsets (X,Y): [2:0-4-5,0-1-12], [7	[2:0-4-5,0-1-12], [7:0-2-0,0-0-12], [12:0-1-12,0-1-0], [13:0-2-0,0-0-12], [14:0-1-12,0-1-0]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.01	1	n/r	120	MT20	244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.04	1	n/r	90	20	2111100	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.06	Horz(TL)	-0.00	6	n/a	n/a			
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)		(3,0,3,3)			1110	Weight: 42 lb		

LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins, except end verticals. **WEBS** 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc **OTHERS** 2 X 4 SYP No.3 bracing.

2=532/7-0-0, 6=261/7-0-0, 9=335/7-0-0, 7=-28/7-0-0, 8=17/7-0-0, REACTIONS (lb/size)

10=-39/7-0-0

Max Horz 2=162(load case 6)

Max Uplift 2=-364(load case 6), 6=-194(load case 4), 9=-170(load case 5), 7=-28(load

case 1), 10=-39(load case 1)

Max Grav 2=532(load case 1), 6=261(load case 1), 9=335(load case 1), 7=41(load

case 4), 8=40(load case 2), 10=66(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-45/123, 2-3=-214/129, 3-4=-63/83, 4-5=-84/81, 5-6=-219/212

BOT CHORD 2-10=-121/113, 9-10=-121/113, 8-9=-99/86, 7-8=-99/86, 6-7=-99/86

WEBS 4-9=-308/257, 4-6=-3/19 esign Chaineer PE No. 34888 restal Bay Blyd n Geach, ft Do406

JOINT STRESS INDEX

2 = 0.73, 3 = 0.00, 3 = 0.29, 4 = 0.43, 5 = 0.32, 6 = 0.29, 7 = 0.00, 8 = 0.00, 9 = 0.14, 10 = 0.00, 11 = 0.00, 12 = 0.00, 0.00, 13 = 0.00, 14 = 0.00, 14 = 0.00, 15 = 0.00 and 15 = 0.00

Continued on page 2



Job	Truss	Truss Type	Qty	Ply		
	EJ7G	GABLE	1	1		J1924506
		ii.			Job Reference (optional)	

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NOTES

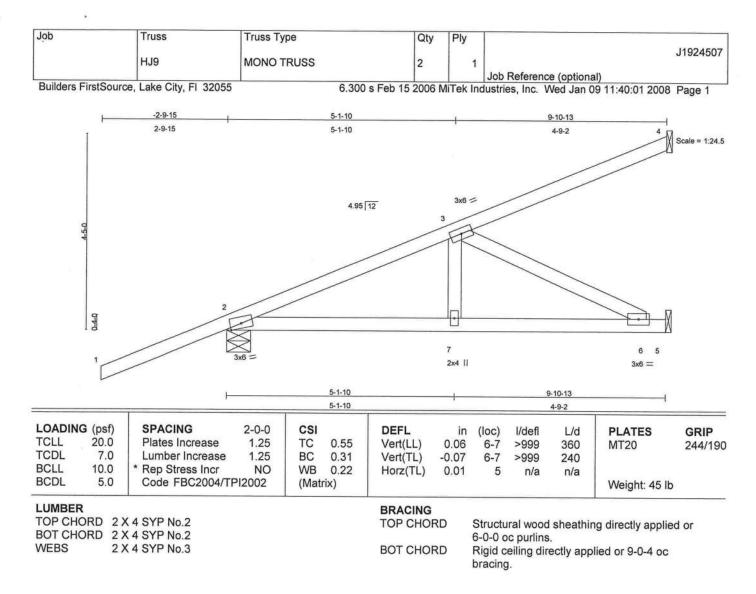
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 364 lb uplift at joint 2, 194 lb uplift at joint 6, 170 lb uplift at joint 9, 28 lb uplift at joint 7 and 39 lb uplift at joint 10.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25. Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-114(F=-60), 4-5=-114(F=-60), 2-6=-10





REACTIONS (lb/size) 4=233/Mechanical, 2=458/0-6-7, 5=251/Mechanical

Max Horz 2=317(load case 5)

Max Uplift 4=-219(load case 5), 2=-384(load case 5), 5=-219(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/58, 2-3=-512/220, 3-4=-112/65

BOT CHORD

2-7=-419/446, 6-7=-419/446, 5-6=0/0

WEBS

3-7=-99/192, 3-6=-498/468

JOINT STRESS INDEX

2 = 0.85, 3 = 0.20, 6 = 0.14 and 7 = 0.14

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 4, 384 lb uplift at joint 2 and 219 lb uplift at joint 5.

Julius Les Truss Design Engineer Flonda PE No. 34869 1 109 Coestel Bay Blvd Bovnton Beach, FL 35436

Continued on page 2



Íop	Truss	Truss Type	Qty	Ply		
	HJ9	MONO TRUSS	2	1		J1924507
				100	Job Reference (optional)	

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NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

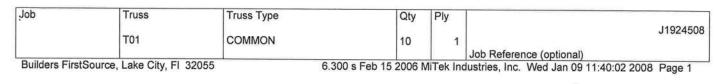
LOAD CASE(S) Standard

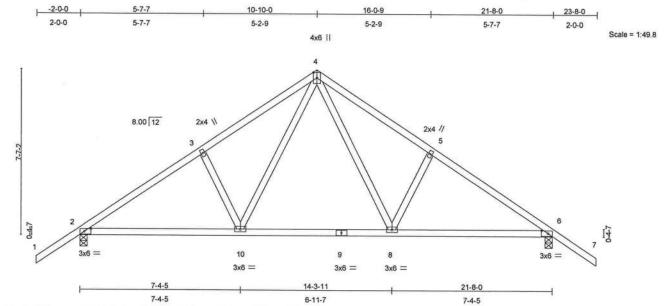
1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)







LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	0.18	8-10	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.31	8-10	>813	240	20	211110
BCLL	10.0	* Rep Stress Incr	NO	WB	0.32	Horz(TL)	0.03	6	n/a	n/a	1	
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)			70		11154	Weight: 113 lb	

LUMBER

Plate Offsets (X,Y):

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-3-4 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=1008/0-4-0, 6=1008/0-4-0

Max Horz 2=200(load case 5)

Max Uplift 2=-297(load case 6), 6=-297(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-1368/585, 3-4=-1232/641, 4-5=-1232/641, 5-6=-1368/585, 6-7=0/60

BOT CHORD 2-10=-294/1050, 9-10=-85/726, 8-9=-85/726, 6-8=-294/1050

[2:0-3-9,0-1-8], [6:0-3-9,0-1-8]

WEBS 3-10=-220/214, 4-10=-279/561, 4-8=-279/561, 5-8=-220/214

JOINT STRESS INDEX

2 = 0.69, 3 = 0.33, 4 = 0.56, 5 = 0.33, 6 = 0.69, 8 = 0.46, 9 = 0.66 and 10 = 0.46

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 9,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply		H HIJ SUCH PRINCES (SEE) PARLICUS (SE
	T01	COMMON	10	1		J1924508
			6000	1.012	Job Reference (optional)	

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NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 297 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

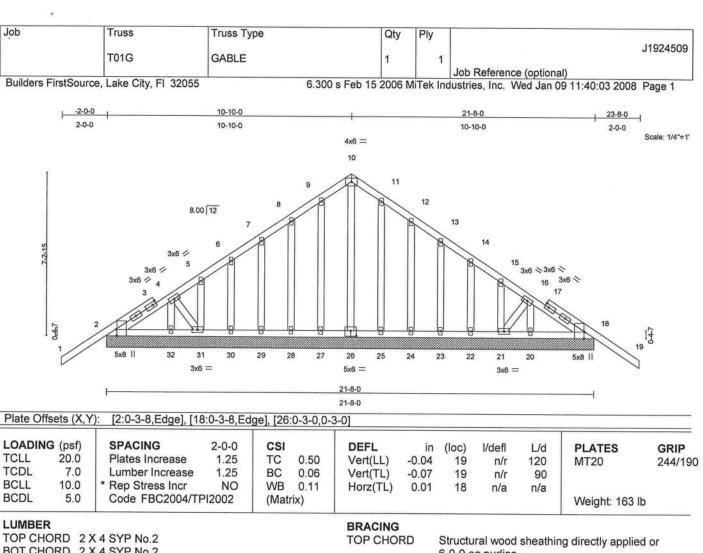
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

Julius Les Trues Design Engineer Florida PE No. 34866 I 199 Cessial Bay Blvd Boynion Beach, FL 30436





BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

OTHERS 2 X 4 SYP No.3

BOT CHORD 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

2=500/21-8-0, 18=500/21-8-0, 26=145/21-8-0, 27=164/21-8-0,

22=164(load case 11), 21=196(load case 11), 20=145(load case 1)

28=166/21-8-0, 29=166/21-8-0, 30=164/21-8-0, 31=194/21-8-0, 32=145/21-8-0, 25=164/21-8-0, 24=166/21-8-0, 23=166/21-8-0, 22=164/21-8-0, 21=194/21-8-0, 20=145/21-8-0

Max Horz 2=247(load case 5)

Max Uplift 2=-301(load case 6), 18=-335(load case 7), 27=-84(load case 6), 28=-117(load case 6), 29=-110(load case 6), 30=-105(load case 6), 31=-201(load case 6), 25=-77(load case 7), 24=-119(load case 7), 23=-110(load case 7), 22=-105(load case 7), 21=-220(load case 7)

Max Grav 2=500(load case 1), 18=500(load case 1), 26=145(load case 1), 27=165(load case 10), 28=166(load case 10), 32=145(load case 1), 25=165(load case 11), 24=166(load case 11), 23=166(load case 1),

Julius Les Truss Design Engineer Flonds PE No. 34868 Flonds PE No. 34868 Bonton Besch. FL 33436

Continued on page 2

REACTIONS (lb/size)



Yop	Truss	Truss Type	Qty	Ply		
	T01G	GABLE	1	1	J1924	509
					Job Reference (optional)	

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-23/123, 2-3=-149/173, 3-4=-153/165, 4-5=-154/144, 5-6=-121/141, 6-7=-88/137, 7-8=-62/158, 8-9=-63/197.

9-10=-62/211, 10-11=-62/210, 11-12=-63/184, 12-13=-62/131, 13-14=-62/85, 14-15=-62/52, 15-16=-64/52,

16-17=-33/47, 17-18=-149/53, 18-19=-23/123

BOT CHORD 2-32=-56/141, 31-32=-56/141, 30-31=-29/206, 29-30=-29/206, 28-29=-29/206, 27-28=-29/206, 26-27=-29/206,

25-26=-29/206, 24-25=-29/206, 23-24=-29/206, 22-23=-29/206, 21-22=-29/206, 20-21=-13/129, 18-20=-13/129

10-26=-132/8, 9-27=-152/92, 8-28=-153/125, 7-29=-152/117, 6-30=-153/117, 5-31=-148/122, 4-32=-142/16,

11-25=-152/85, 12-24=-153/127, 13-23=-152/116, 14-22=-153/118, 15-21=-148/119, 16-20=-142/22,

4-31=-36/105, 16-21=-36/120

JOINT STRESS INDEX

2 = 0.69, 3 = 0.00, 3 = 0.32, 4 = 0.41, 5 = 0.33, 6 = 0.33, 7 = 0.33, 8 = 0.33, 9 = 0.33, 10 = 0.27, 11 = 0.33, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.41, 17 = 0.00, 17 = 0.32, 17 = 0.32, 18 = 0.69, 20 = 0.33, 21 = 0.40, 22 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.33,

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 301 lb uplift at joint 2, 335 lb uplift at joint 18, 84 lb uplift at joint 27, 117 lb uplift at joint 28, 110 lb uplift at joint 29, 105 lb uplift at joint 30, 201 lb uplift at joint 31, 77 lb uplift at joint 25, 119 lb uplift at joint 24, 110 lb uplift at joint 23, 105 lb uplift at joint 22 and 220 lb uplift at joint 21.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

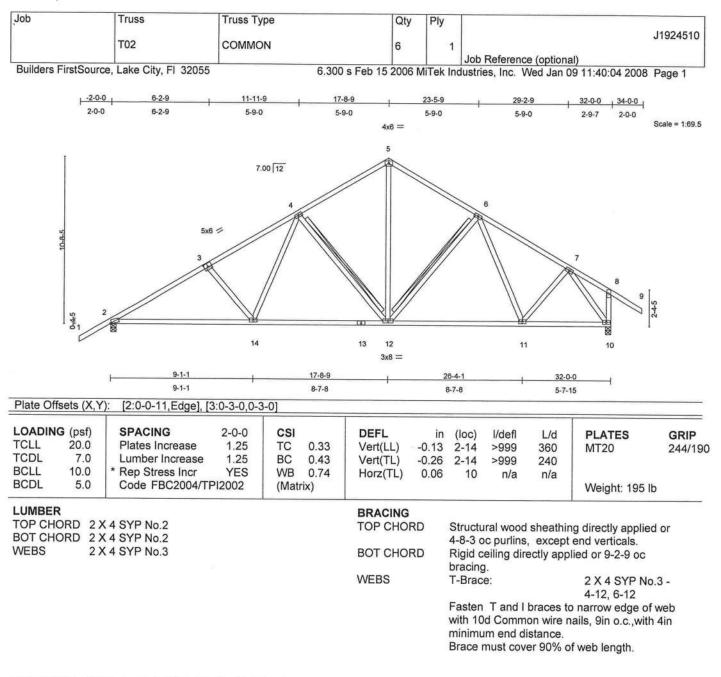
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-10=-114(F=-60), 10-19=-114(F=-60), 2-18=-10

Julius Les Truss Design Engineer Plonda PE No. 34989 1 100 Ceastal Bay Blvd Bovnton Besch, Et. 33436





REACTIONS (lb/size) 2=1131/0-4-0, 10=1130/0-4-0

Max Horz 2=316(load case 5)

Max Uplift 2=-319(load case 6), 10=-293(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-1663/739, 3-4=-1458/730, 4-5=-973/593, 5-6=-975/594,

6-7=-1035/559, 7-8=-31/119, 8-9=0/58, 8-10=-164/226

BOT CHORD 2-14=-468/1354, 13-14=-267/1069, 12-13=-267/1069, 11-12=-197/891,

10-11=-165/646

3-14=-262/236, 4-14=-135/365, 4-12=-480/338, 5-12=-386/592, 6-12=-248/227,

6-11=-161/65, 7-11=-20/289, 7-10=-1155/453

JOINT STRESS INDEX

WEBS

NT STRESS INDEX
2 = 0.76, 3 = 0.44, 4 = 0.40, 5 = 0.51, 6 = 0.40, 7 = 0.36, 8 = 0.32, 10 = 0.41, 11 = 0.47, 12 = 0.56, 13 = 0.35 and 14 = 0.47

January 9,2008 Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



ʻJop	Truss	Truss Type	Qty	Ply		
	T02	COMMON	6	1		J1924510
	Albert Aber				Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

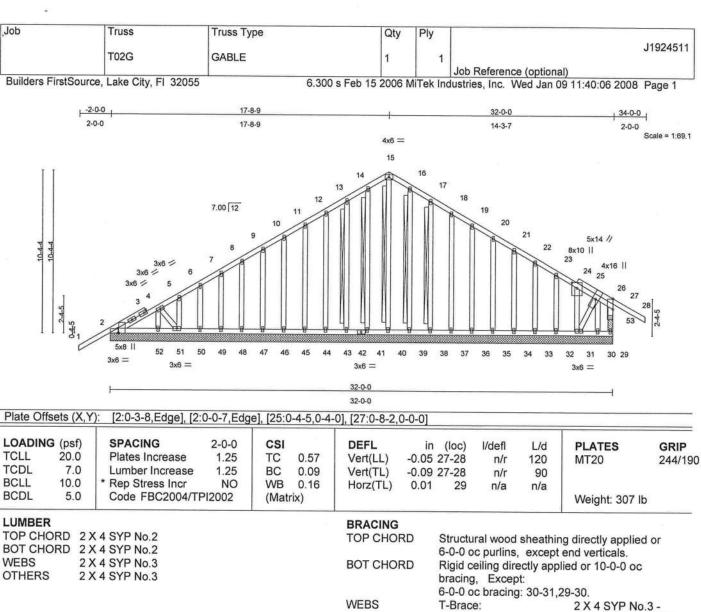
4) All plates are 3x6 MT20 unless otherwise indicated.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 319 lb uplift at joint 2 and 293 lb uplift at joint 10.

LOAD CASE(S) Standard





T-Brace:

15-40, 14-41, 13-43,

16-39, 17-38

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

EL BOADS



Ýop	Truss	Truss Type	Qty	Ply		HISSNESS CONTRACTOR
	T02G	GABLE	1	1		J1924511
					Job Reference (optional)	

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REACTIONS (lb/size)

2=501/32-0-0, 29=477/32-0-0, 40=153/32-0-0, 41=164/32-0-0, 43=166/32-0-0, 44=165/32-0-0, 45=165/32-0-0, 46=165/32-0-0, 47=165/32-0-0, 48=165/32-0-0, 49=165/32-0-0, 50=166/32-0-0, 51=185/32-0-0, 52=174/32-0-0, 39=164/32-0-0, 38=166/32-0-0, 37=165/32-0-0, 36=165/32-0-0, 35=165/32-0-0, 34=165/32-0-0, 33=165/32-0-0, 32=170/32-0-0, 31=68/32-0-0, 30=52/32-0-0

Max Horz 2=391(load case 5)

Max Uplift 2=-276(load case 6), 29=-281(load case 7), 40=-22(load case 5), 41=-64(load case 5), 43=-114(load case 6), 44=-103(load case 6), 45=-102(load case 6), 46=-103(load case 6), 47=-103(load case 6), 48=-103(load case 6), 49=-104(load case 6), 50=-99(load case 6), 51=-203(load case 6), 39=-49(load case 7), 38=-116(load case 7), 37=-104(load case 7), 36=-102(load case 7), 35=-103(load case 7), 34=-103(load case 7), 33=-102(load case 7), 32=-113(load case 7), 31=-227(load case 7), 30=-20(load case 5)

Max Grav 2=501(load case 1), 29=477(load case 1), 40=172(load case 7), 41=165(load case 10), 43=167(load case 10), 44=165(load case 1), 45=165(load case 1), 46=165(load case 10), 47=165(load case 1), 48=165(load case 1), 48=165(load case 1), 49=165(load case 1), 50=166(load case 10), 51=186(load case 10), 52=174(load case 1), 39=165(load case 11), 38=167(load case 11), 37=165(load case 1), 36=165(load case 1), 35=165(load case 1), 31=83(load case 5), 30=167(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-21/112, 2-3=-249/251, 3-4=-266/245, 4-5=-280/230, 5-6=-253/228, 6-7=-225/224, 7-8=-197/221, 8-9=-169/217, 9-10=-142/214, 10-11=-114/218, 11-12=-86/247, 12-13=-58/276, 13-14=-52/311, 14-15=-51/321, 15-16=-51/321, 16-17=-52/311, 17-18=-51/265, 18-19=-51/226, 19-20=-51/187, 20-21=-51/147, 21-22=-51/108,

22-23=-52/69, 23-24=-48/48, 24-25=-35/49, 25-26=-61/49, 26-27=-102/143, 27-28=-21/110, 29-53=-497/371, 27-52=-497/371, 27-52=-497/371, 27-52=-497/371, 27-52=-497/371, 27-53=-497/371, 27-53=-497/372

27-53=-497/372

BOT CHORD

2-52=-104/151, 51-52=-104/151, 50-51=-69/193, 49-50=-69/193, 48-49=-69/193, 47-48=-69/193, 46-47=-69/193, 45-46=-69/193, 44-45=-69/193, 43-44=-69/193, 42-43=-69/193, 41-42=-69/193, 40-41=-69/193, 39-40=-69/193, 38-39=-69/193, 37-38=-69/193, 36-37=-69/193, 35-36=-69/193, 34-35=-69/193, 33-34=-69/193, 32-33=-69/193, 32-33=-69/193, 36-37

31-32=-69/193, 30-31=-29/89, 29-30=-29/90

WEBS

15-40=-172/30, 14-41=-152/72, 13-43=-153/122, 12-44=-152/111, 11-45=-152/110, 10-46=-152/111, 9-47=-152/111, 8-48=-152/111, 7-49=-152/111, 6-50=-154/111, 5-51=-142/112, 4-52=-165/12, 16-39=-152/57, 17-38=-153/124, 18-37=-152/112, 19-36=-152/110, 20-35=-152/111, 21-34=-152/111, 22-33=-152/110, 23-32=-156/118, 25-31=-131/51, 26-30=-236/36, 26-31=-107/238, 4-51=-36/121

JOINT STRESS INDEX

2 = 0.66, 2 = 0.21, 3 = 0.00, 3 = 0.34, 3 = 0.34, 4 = 0.40, 5 = 0.33, 6 = 0.33, 7 = 0.33, 8 = 0.33, 9 = 0.33, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.26, 16 = 0.33, 17 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.00, 25 = 0.25, 26 = 0.25, 27 = 0.59, 29 = 0.54, 29 = 0.00, 30 = 0.33, 31 = 0.45, 32 = 0.33, 33 = 0.33, 34 = 0.33, 35 = 0.33, 36 = 0.33, 37 = 0.33, 38 = 0.33, 39 = 0.33, 40 = 0.33, 41 = 0.33, 42 = 0.15, 43 = 0.33, 44 = 0.33, 45 = 0.33, 46 = 0.33, 47 = 0.33, 48 = 0.33, 49 = 0.33, 50 = 0.33, 51 = 0.38, 52 = 0.33, 53 = 0.00

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Less Truss Design Engineer Florida PE No. 24868 1109 Coastal Bay Blvd Boynton Beach, FL 23435

Continued on page 3



jop	Truss	Truss Type	Qty	Ply		
	T02G	GABLE	1	1		J1924511
					Job Reference (optional)	

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NOTES

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 276 lb uplift at joint 2, 281 lb uplift at joint 29, 22 lb uplift at joint 40, 64 lb uplift at joint 41, 114 lb uplift at joint 43, 103 lb uplift at joint 44, 102 lb uplift at joint 45, 103 lb uplift at joint 46, 103 lb uplift at joint 47, 103 lb uplift at joint 48, 104 lb uplift at joint 49, 99 lb uplift at joint 50, 203 lb uplift at joint 51, 49 lb uplift at joint 39, 116 lb uplift at joint 38, 104 lb uplift at joint 37, 102 lb uplift at joint 36, 103 lb uplift at joint 33, 113 lb uplift at joint 32, 227 lb uplift at joint 31 and 20 lb uplift at joint 30.
 In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

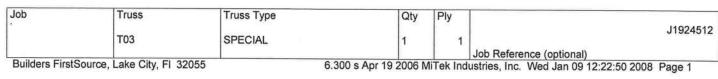
LOAD CASE(S) Standard

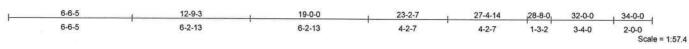
 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

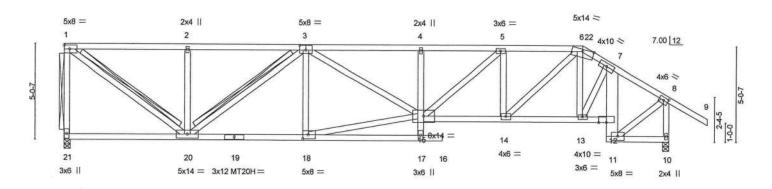
Vert: 1-15=-114(F=-60), 15-27=-114(F=-60), 27-28=-114(F=-60), 2-29=-10

Julius Lee Truse Design Engineer Flonda PE No. 34889 1 109 Geestal Bay Blvd Bovnton Besch. FL 33435









6-6-5	12-9-3	19-0-0	20-0-9	27-4-14	₁ 28-8-0 ₁	32-0-0
6-6-5	6-2-13	6-2-13	1-0-0	7-4-14	1-3-2	3-4-0
		A SAN SAN SAN SAN SAN SAN SAN SAN SAN SA				

Plate Of	ffsets (X,Y):	[3:0-4-0,0-3-0], [8:0	-2-14,0-2-0], [18:0-3	3-8,0-2-8]							
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.75	Vert(LL)	-0.29	16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.55	16	>695	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.95	Horz(TL)	0.20	10	n/a	n/a		1017110
BCDL			(Matrix)				,,,		7110	Weight: 218 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

4-17 2 X 4 SYP No.3, 12-15 2 X 4 SYP No.1D

7-11 2 X 8 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3 *Except*

1-21 2 X 4 SYP No.2, 15-18 2 X 4 SYP No.2

BRACING

TOP CHORD

....

BOT CHORD WEBS Structural wood sheathing directly applied or 2-4-2 oc purlins, except end verticals.

Rigid ceiling directly applied or 5-3-7 oc bracing. T-Brace: 2 X 4 SYP No.3 - 1-21,

1-20, 3-20

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 21=2202/0-4-0, 10=2319/0-4-0

Max Horz 21=180(load case 4)

Max Uplift 21=-772(load case 4), 10=-674(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-21=-2121/802, 1-2=-2460/856, 2-3=-2460/856, 3-4=-4534/1553, 4-5=-4549/1550,

5-22=-3739/1240, 6-22=-3739/1240, 6-7=-2633/795, 7-8=-1811/583, 8-9=0/58,

8-10=-2235/655

BOT CHORD 20-21=-210/82, 19-20=-1415/3614, 18-19=-1415/3614, 17-18=-103/349, 16-17=0/0,

15-17=0/181, 4-15=-614/335, 14-15=-1421/3738, 13-14=-876/2273, 12-13=-734/1958,

11-12=-927/293, 7-12=-979/296, 10-11=-15/49

WEBS 1-20=-1029/2964, 2-20=-765/421, 3-20=-1450/476, 3-18=-421/302, 15-18=-1329/3309,

3-15=-374/1080, 5-15=-409/1070, 6-14=-729/1959, 6-13=-298/233, 7-13=-291/630,

8-11=-564/1830, 5-14=-1170/529

Julius Lee Truss Cesign Engineer Florida PE No. 24869 1 100 Caastal Bay Blvd

JOINT STRESS INDEX

1 = 0.65, 2 = 0.34, 3 = 0.67, 4 = 0.37, 5 = 0.65, 6 = 0.80, 7 = 0.36, 8 = 0.86, 10 = 0.82, 11 = 0.51, 12 = 0.77, 13 = 0.54, 14 = 0.85angary 9,2008Continued on page 32 18 = 0.76, 19 = 0.83, 20 = 0.87 and 21 = 0.46

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Jop	Truss	Truss Type	Qty	Ply		
	Т03	SPECIAL	1	1		J1924512
					Job Reference (optional)	

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NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

2) Provide adequate drainage to prevent water ponding.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All plates are MT20 plates unless otherwise indicated.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 772 lb uplift at joint 21 and 674 lb uplift at joint 10.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

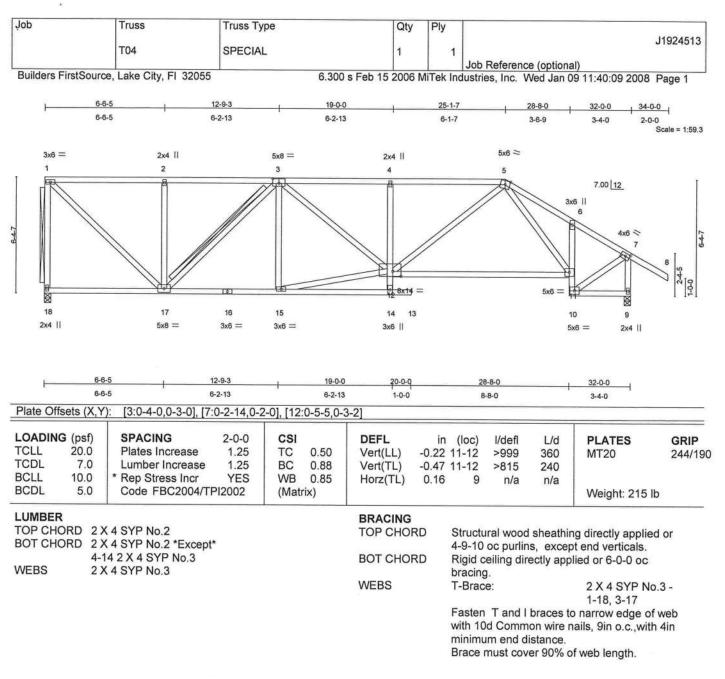
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-22=-117(F=-63), 6-22=-54, 6-8=-54, 8-9=-54, 17-21=-22(F=-12), 16-17=-22(F=-12), 13-15=-22(F=-12), 12-13=-85(F=-75), 10-11=-85(F=-75)

Julius Lee Truss Cesion Engineer Floride PE No. 24888 1100 Caestal Bay Blvd Boynton Besch, FL 20426





REACTIONS (lb/size) 18=1015/0-4-0, 9=1141/0-4-0

Max Horz 18=-239(load case 7)

Max Uplift 18=-338(load case 5), 9=-233(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-18=-979/551, 1-2=-887/475, 2-3=-887/475, 3-4=-1571/804, 4-5=-1592/806,

5-6=-1123/577, 6-7=-933/470, 7-8=0/58, 7-9=-1246/634

BOT CHORD 17-18=-23/321, 16-17=-421/1300, 15-16=-421/1300, 14-15=-85/23, 13-14=0/0,

12-14=0/127, 4-12=-339/242, 11-12=-270/1059, 10-11=-403/167, 6-11=-196/122,

9-10=-32/89

WEBS 1-17=-637/1190, 2-17=-360/261, 3-17=-576/323, 3-15=-154/99, 12-15=-371/1294,

3-12=-127/354, 5-12=-259/685, 5-11=-209/223, 7-10=-330/937

Indius Lee Truss Design Engineer Flonds PE No. 34Ees 1 109 Coasial Eay Blvd

Continued on page 2



Job	Truss	Truss Type	Qty	Ply		
	T04	SPECIAL	1	1		J1924513
	10000				Job Reference (optional)	

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JOINT STRESS INDEX

1 = 0.73, 2 = 0.33, 3 = 0.34, 4 = 0.34, 5 = 0.42, 6 = 0.67, 7 = 0.73, 9 = 0.73, 10 = 0.70, 11 = 0.79, 12 = 0.44, 14 = 0.29, 15 = 0.69, 16 = 0.47, 17 = 0.55 and 18 = 0.66

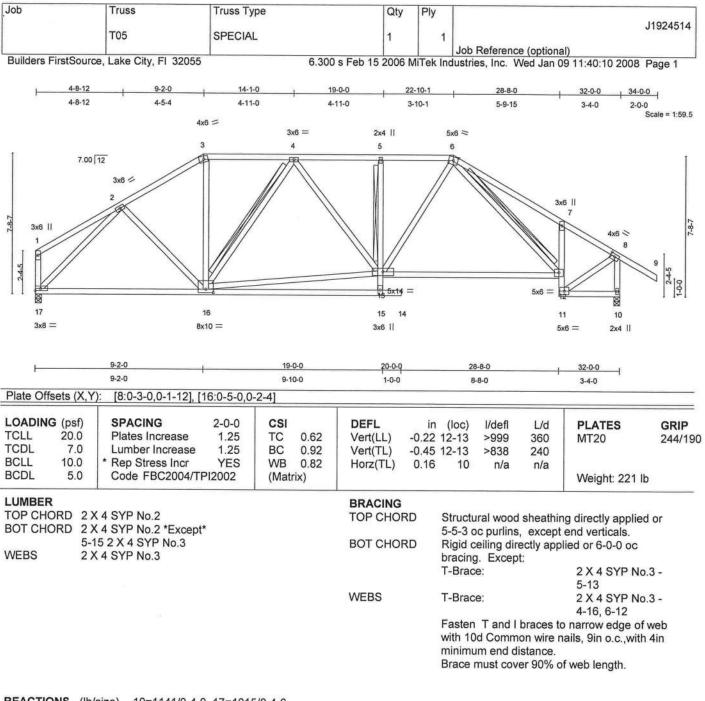
NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 338 lb uplift at joint 18 and 233 lb uplift at joint 9.

LOAD CASE(S) Standard

Julium Les Truse Design Engineer Florida PE No. 24669 Florida Eay Blvd Goymon Besch. Ft. 23425





REACTIONS (lb/size) 10=1141/0-4-0, 17=1015/0-4-0

Max Horz 17=-221(load case 4)

Max Uplift 10=-260(load case 7), 17=-207(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-152/94, 2-3=-1112/620, 3-4=-914/587, 4-5=-1231/708, 5-6=-1241/705,

6-7=-1215/684, 7-8=-937/486, 8-9=0/58, 1-17=-169/117, 8-10=-1246/647

BOT CHORD 16-17=-322/832, 15-16=-30/61, 14-15=0/0, 13-15=0/176, 5-13=-237/142,

12-13=-244/1028, 11-12=-417/186, 7-12=-324/239, 10-11=-41/97

2-16=-106/201, 3-16=-132/312, 4-16=-501/230, 4-13=-43/171, 6-13=-193/488,

6-12=-192/202, 2-17=-1099/541, 8-11=-366/963, 13-16=-325/1104

Julius Les Truss Ossign Engineer Florida PE No. 24869 I 109 Ceasial Bay Blvd Boynton Beach, FL 99495

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply		
	T05	SPECIAL	1	1		J1924514
L					Job Reference (optional)	

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JOINT STRESS INDEX

1 = 0.27, 2 = 0.37, 3 = 0.43, 4 = 0.41, 5 = 0.33, 6 = 0.46, 7 = 0.71, 8 = 0.71, 10 = 0.76, 11 = 0.70, 12 = 0.79, 13 = 0.53, 15 = 0.69, 16 = 0.30 and 17 = 0.52

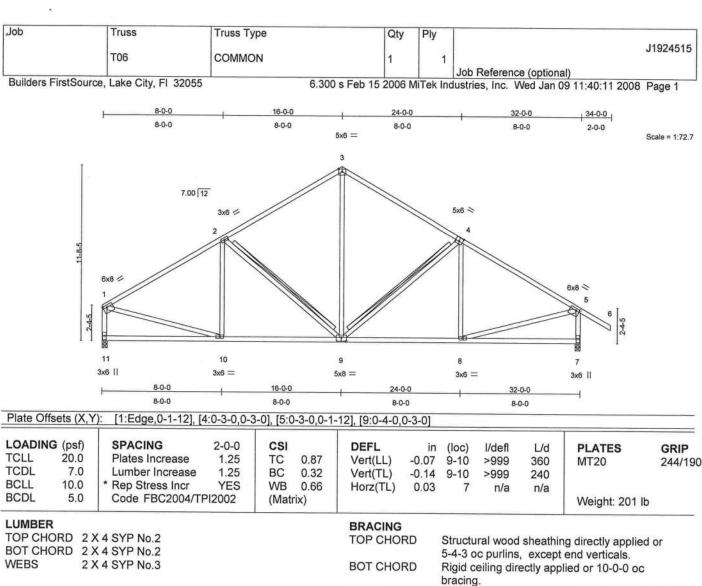
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 10 and 207 lb uplift at joint 17.

LOAD CASE(S) Standard

Julius Lew Truss Cissian Engineer Flonda FE No. 34889 1 109 Ceastal Bay Blvd Boynton Besch, Ft. 33436





WEBS

T-Brace:

2 X 4 SYP No.3 - 2-9, 4-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 11=1011/0-4-0, 7=1134/0-4-0

Max Horz 11=-331(load case 4)

Max Uplift 11=-213(load case 6), 7=-299(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-1188/562, 2-3=-956/577, 3-4=-955/577, 4-5=-1178/570, 5-6=0/58, TOP CHORD

1-11=-966/481, 5-7=-1092/604

BOT CHORD 10-11=-302/356, 9-10=-221/938, 8-9=-217/927, 7-8=0/97

WEBS 2-10=-144/132, 2-9=-333/252, 3-9=-285/475, 4-9=-321/259, 4-8=-151/115,

1-10=-288/823, 5-8=-240/858

JOINT STRESS INDEX

1 = 0.89, 2 = 0.40, 3 = 0.71, 4 = 0.78, 5 = 0.88, 7 = 0.39, 8 = 0.46, 9 = 0.43, 10 = 0.46 and 11 = 0.39

Continued on page 2



Job	Truss	Truss Type	Qty	Ply		
	T06	COMMON	1	1		J1924515
					Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 11 and 299 lb uplift at joint 7.

LOAD CASE(S) Standard





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8x10 = 8x10 = 3 7.00 12 3x6 < 6 5x6 = 5x6 <> 15 14 13 12 11 10 3x6 II 3x6 =3x6 = 5x8 = 3x6 = 3x6 = 3x6 11 5-2-7 10-1-7 16-0-0 21-10-10 26-9-9 5-2-7 4-10-15

[3:0-4-1,Edge], [4:0-1-8,0-1-0], [5:0-4-1,Edge], [6:0-0-0,0-0-0], [7:0-2-10,0-2-8], [12:0-4-0,0-3-0], [16:0-2-0,0-3-8], Plate Offsets (X,Y): [18:0-2-0,0-1-8]

5-10-10

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.75	Vert(LL)	0.09	12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.15	11-12	>999	240	111120	2111100
BCLL	10.0	* Rep Stress Incr	NO	WB	0.83	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mati					,,,,	1110	Weight: 254 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

5-10-10

Structural wood sheathing directly applied or 4-0-13 oc purlins, except end verticals. Except:

2 Rows at 1/3 pts 3-5

4-10-15

BOT CHORD WEBS

Rigid ceiling directly applied or 6-9-12 oc bracing. T-Brace: 2 X 4 SYP No.3 - 4-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 15=1523/0-4-0, 9=1646/0-4-0

Max Horz 15=-292(load case 4)

Max Uplift 15=-641(load case 6), 9=-758(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1658/834, 2-3=-1840/1016, 3-4=-1892/1116, 4-5=-1892/1116, 5-6=-1832/1018,

6-7=-1642/852, 7-8=0/58, 1-15=-1487/755, 7-9=-1614/882

14-15=-262/291, 13-14=-789/1368, 12-13=-867/1545, 11-12=-732/1539, 10-11=-504/1350 **BOT CHORD**

9-10=-10/61

2-14=-507/292, 2-13=-326/362, 3-13=-197/259, 3-12=-495/672, 4-12=-904/651,

5-12=-494/678, 5-11=-201/262, 6-11=-330/368, 6-10=-515/266, 1-14=-646/1398,

7-10=-601/1429

JOINT STRESS INDEX

WEBS

1 = 0.75, 2 = 0.42, 3 = 0.73, 4 = 0.37, 4 = 0.46, 5 = 0.73, 6 = 0.41, 7 = 0.75, 9 = 0.31, 10 = 0.81, 11 = 0.37, 12 = 0.42, 13 = 0.37, 14 = 0.46, 0.81, 15 = 0.31, 16 = 0.34, 17 = 0.26, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34 and bentiary 9,2008 Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply		
	T06G	HIP	1	1		J1924516
D 311 E	Course Laboration El 00	2075			Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 641 lb uplift at joint 15 and 758 lb uplift at joint 9.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

 Truss designed for wind loads in plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-141(F=-87), 5-7=-54, 7-8=-54, 9-15=-10

Julius Lee Truse Design Engineer Ponda PE No. 34868 1409 Geestal Bay Blvd Gryston Design Bay Blvd





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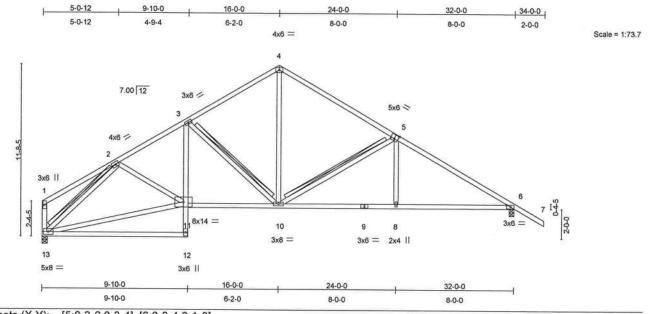


Plate Offsets (X,Y): [5:0-3-0,0-3-4], [6:0-3-4,0-1-8] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.75 Vert(LL) -0.15 12-13 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.71 Vert(TL) -0.27 12-13 >999 240 BCLL 10.0 * Rep Stress Incr YES WB 0.62 Horz(TL) 0.10 6 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 197 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-12 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-6-8 oc purlins, except end verticals.

Rigid ceiling directly applied or 9-8-7 oc bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -

3-10, 5-10, 2-13

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 13=1010/0-4-0, 6=1135/0-4-0

Max Horz 13=-281(load case 4)

Max Uplift 13=-211(load case 6), 6=-302(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-170/101, 2-3=-1463/697, 3-4=-1097/603, 4-5=-1132/595, 5-6=-1648/704,

6-7=0/54, 1-13=-186/127

12-13=0/110, 11-12=0/143, 3-11=-73/259, 10-11=-337/1222, 9-10=-413/1323,

8-9=-413/1323, 6-8=-413/1325

WEBS 3-10=-485/299, 4-10=-330/634, 5-10=-534/348, 5-8=0/258, 2-13=-1388/621,

11-13=-388/1019, 2-11=0/185

JOINT STRESS INDEX

BOT CHORD

1 = 0.32, 2 = 0.41, 3 = 0.72, 4 = 0.83, 5 = 0.71, 6 = 0.72, 8 = 0.33, 9 = 0.42, 10 = 0.56, 11 = 0.36, 12 = 0.69 and 13 = 0.64

January 9,2008 Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conne Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erec and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



allia Lee 1988 Cesign Engineer Iorda PE No. 34860 199 Ceastal Rey Blod ovnton Gesch, FL 33435

.Job	Truss	Truss Type	Qty	Ply		
	T07	SPECIAL	3	1		J1924517
	10.7 M2.10		١		Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

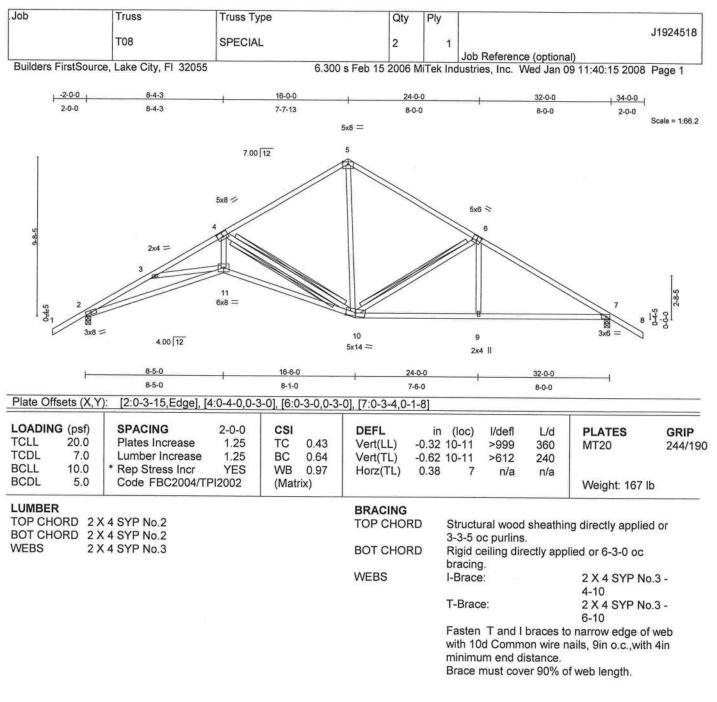
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 lb uplift at joint 13 and 302 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida FE No. 34868 1 100 Ceestal Bay Elvid Boynton Beach Et 23435





REACTIONS (lb/size) 2=1130/0-4-0, 7=1130/0-4-0

Max Horz 2=-260(load case 4)

Max Uplift 2=-312(load case 6), 7=-312(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/52, 2-3=-3340/1297, 3-4=-3225/1210, 4-5=-1099/585, 5-6=-1147/606.

6-7=-1634/701, 7-8=0/54

BOT CHORD 2-11=-999/2917, 10-11=-905/3030, 9-10=-410/1311, 7-9=-409/1312 **WEBS**

4-10=-2328/864, 5-10=-299/595, 6-10=-518/335, 6-9=0/244, 4-11=-525/1946,

3-11=-35/120

JOINT STRESS INDEX

Continued 55 3ag 23, 4 = 0.84, 5 = 0.98, 6 = 0.80, 7 = 0.73, 9 = 0.33, 10 = 0.73 and 11 = 0.77



Job	Truss	Truss Type	Qty	Ply		
	T08	SPECIAL	2	1		J1924518
					Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

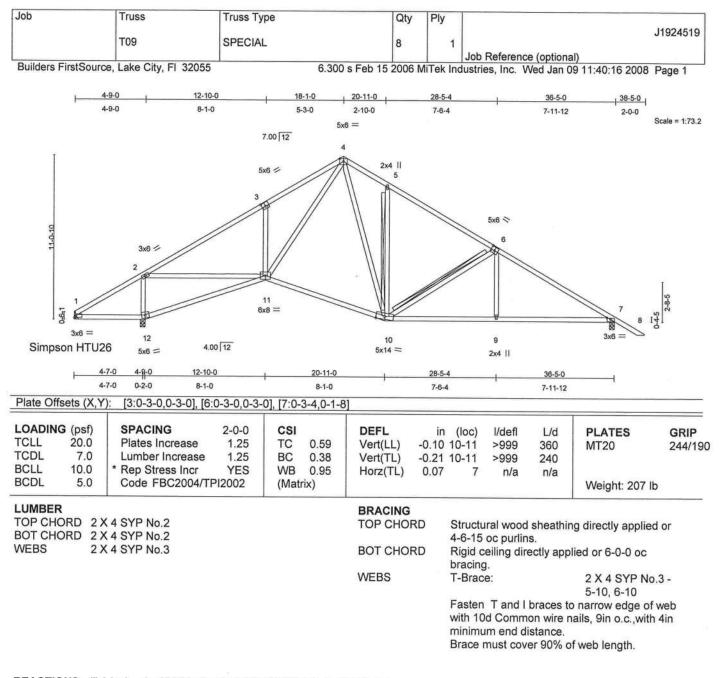
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 2 and 312 lb uplift at joint 7.

LOAD CASE(S) Standard





REACTIONS (lb/size) 1=-223/Mechanical, 12=1577/0-4-0, 7=1061/0-4-0

Max Horz 1=-316(load case 4)

Max Uplift 1=-223(load case 1), 12=-447(load case 6), 7=-309(load case 7) Max Grav 1=36(load case 6), 12=1577(load case 1), 7=1061(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-217/638, 2-3=-1292/497, 3-4=-1263/700, 4-5=-990/718, 5-6=-1043/568,

6-7=-1535/672, 7-8=0/47

1-12=-454/274, 11-12=-563/334, 10-11=-15/740, 9-10=-396/1228, 7-9=-396/1229

WEBS 2-12=-1324/634, 2-11=-426/1540, 3-11=-396/366, 4-11=-228/588, 4-10=-435/504,

5-10=-304/272, 6-10=-506/347, 6-9=0/243

JOINT STRESS INDEX

1 = 0.54, 2 = 0.73, 3 = 0.71, 4 = 0.29, 5 = 0.33, 6 = 0.82, 7 = 0.76, 9 = 0.33, 10 = 0.31, 11 = 0.64 and 12 = 0.66

Continued on page 2

BOT CHORD

January 9,2008



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Job	Truss	Truss Type	Qty	Ply		
	Т09	SPECIAL	8	1		J1924519
					Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 1, 447 lb uplift at joint 12 and 309 lb uplift at joint 7.

LOAD CASE(S) Standard





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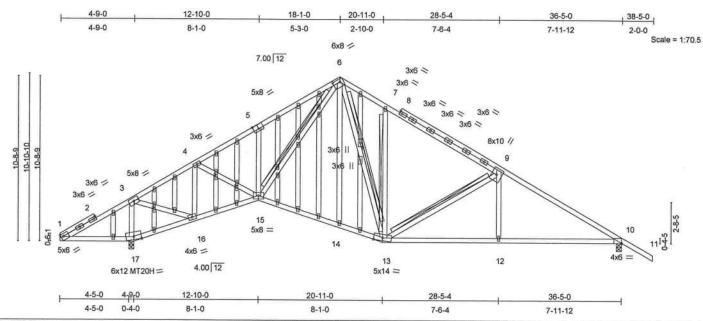


Plate Of	fsets (X,Y):	[1:Edge,0-2-0], [5:0-	-4-0,0-3-4]	[6:0-5-4	,0-2-4], [1	0:0-0-0,0-0-4]	[27:0-	1-12,0-1	-0], [30:0)-1-12,0-1	-0], [35:0-1-8,0-1-0	0]
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	000	14-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.25	14-15	>999	240	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.75	Horz(TL)	0.17	10	n/a	n/a	III LOTT	1017140
BCDL 5.0 Code FBC2004/TF		PI2002	(Mat		()		,,,	1000	1114	Weight: 301 lb		

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

LUMBED

BRACING TOP CHORD

Structural wood sheathing directly applied or 3-2-6 oc purlins.

BOT CHORD WEBS Rigid ceiling directly applied or 5-11-6 oc bracing. T-Brace: 2 X 4 SYP No.3 - 6-15,

6-13, 7-13, 9-13

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 17=2862/0-4-0, 10=1483/0-4-0

Max Horz 17=-387(load case 4)

Max Uplift 17=-2050(load case 6), 10=-891(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-657/572, 2-3=-782/763, 3-4=-2008/1399, 4-5=-2869/2082, 5-6=-2913/2249,

6-7=-1847/1560, 7-8=-1702/1396, 8-9=-1873/1389, 9-10=-2381/1508, 10-11=0/54

BOT CHORD 1-17=-508/624, 16-17=-701/878, 15-16=-987/1688, 14-15=-789/1544, 13-14=-791/1540, 12-13=-1119/1973, 10-12=-1118/1975

WEBS 3-17=-2556/2125, 5-15=-680/706, 6-15=-1126/1587, 6-13=-394/362, 7-13=-436/403,

9-13=-519/372, 9-12=0/254, 4-16=-1387/1118, 4-15=-585/876, 3-16=-1772/2345.

6-14=0/102

Julius Lee Truss Design Engineer Florida Fiz No. 34888 1108 Coastal Bay Blvd Boynton Beach, FL 33435

January 9,2008

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply		
	T09G	GABLE	8	1		J1924520
) (Company of the Company of the Com			Job Reference (optional)	

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JOINT STRESS INDEX

 $1 = 0.43, \ 2 = 0.00, \ 2 = 0.40, \ 2 = 0.40, \ 3 = 0.64, \ 4 = 0.66, \ 5 = 0.74, \ 6 = 0.88, \ 7 = 0.34, \ 8 = 0.00, \ 8 = 0.38, \ 8 = 0.35, \ 8 =$

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2050 lb uplift at joint 17 and 891 lb uplift at joint 10.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-6=-141(F=-87), 6-8=-141(F=-87), 8-11=-54, 1-17=-10, 15-17=-10, 13-15=-10, 10-13=-10

Julius Lee fruse Design Engineer fonda PE No. 34868 | 100 Geastel Ray Blyd | 100 Geastel Ry Blyd | 100 Geach, FL 99436



Job Truss Truss Type Qty Ply J1924521 T10G **GABLE** 1 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 12:28:04 2008 Page 1 -2-0-0 3-10-0 7-8-0 9-8-0 2-0-0 3-10-0 3-10-0 2-0-0 Scale = 1:20.5 4x6 = 8.00 12 2x4 || 2x4 || 3x6 / 3x6 > 0-6-5 0-6-5 3x6 [] 2x4 || 2x4 II 8 2x4 II 7-8-0 3-10-0

Plate Of	fsets (X,Y)	[2:0-1-6,0-1-11], [2:0-2-4,0-8-5], [6:0-1-6,0-1-11], [6:0-2-4,0-8-5]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.01	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-8	>999	240		2111100
BCLL	10.0	* Rep Stress Incr	NO	WB	0.03	Horz(TL)	0.00	6	n/a	n/a		
BCDL			(Mat	O Sall Control					,,,,	Weight: 45 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

OTHERS 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0

3-10-0

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=701/0-3-8, 6=701/0-3-8

Max Horz 2=-84(load case 4)

Max Uplift 2=-641(load case 6), 6=-641(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-92/129, 2-3=-410/548, 3-4=-300/476, 4-5=-300/476, 5-6=-410/548, 6-7=-92/129

2-8=-248/252, 6-8=-248/252 **BOT CHORD**

WEBS 4-8=-151/88

JOINT STRESS INDEX

2 = 0.76, 2 = 0.49, 3 = 0.00, 3 = 0.55, 4 = 0.39, 5 = 0.00, 5 = 0.55, 6 = 0.76, 6 = 0.49, 8 = 0.08, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

January 9,2008

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply			
	T10G	GABLE	1	1		J1924521	
					Job Reference (optional)		

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NOTES

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable studs spaced at 1-4-0 oc.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 641 lb uplift at joint 2 and 641 lb uplift at joint
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

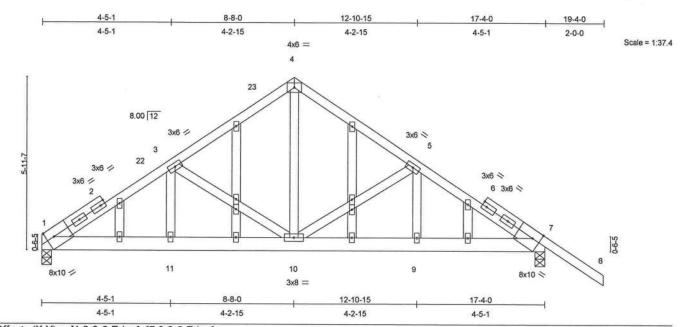
Vert: 2-6=-10, 1-4=-114(F=-60), 4-7=-114(F=-60)





Builders FirstSource, Lake City, FI 32055

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LOADIN	G (not)		SPACING	2-0-0	CSI		DEFL	1	(1)	114-5	1.64	DIATEO	ODID
	**			1000	1000000		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.58	Vert(LL)	0.07	10-11	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.08	9-10	>999	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.41	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Mat	rix)	, , , ,					Weight: 125 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D **WEBS** 2 X 4 SYP No.3

OTHERS 2 X 4 SYP No.3 BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-5

BOT CHORD Rigid ceiling directly applied or 7-4-12 oc bracing.

REACTIONS (lb/size) 1=1240/0-4-0, 7=1656/0-4-0

Max Horz 1=-221(load case 3)

Max Uplift 1=-939(load case 5), 7=-1326(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2013/1493, 2-22=-1953/1493, 3-22=-1893/1494, 3-23=-1483/1147, 4-23=-1193/1009,

4-5=-1451/1131, 5-6=-1939/1472, 6-7=-2061/1519, 7-8=-47/120

BOT CHORD 1-11=-1256/1656, 10-11=-1256/1656, 9-10=-1156/1618, 7-9=-1156/1618 **WEBS** 3-11=-180/202, 3-10=-668/611, 4-10=-786/928, 5-10=-621/551, 5-9=-180/198

JOINT STRESS INDEX

1 = 0.48, 2 = 0.00, 2 = 0.45, 2 = 0.46, 3 = 0.43, 4 = 0.69, 5 = 0.43, 6 = 0.00, 6 = 0.46, 6 = 0.45, 7 = 0.48, 9 = 0.34, 10 = 0.57, 11 = 0.34, 10 = 0.57, 11 = 0.34, 10 = 0.57, 11 = 0.34, 10 = 0.57, 1112 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 19 = 0.34, 20 = 0.34 and 21 = 0.34

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

January 9,2008

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply		W. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.
	T11G	GABLE	1	1		J1924522
					Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

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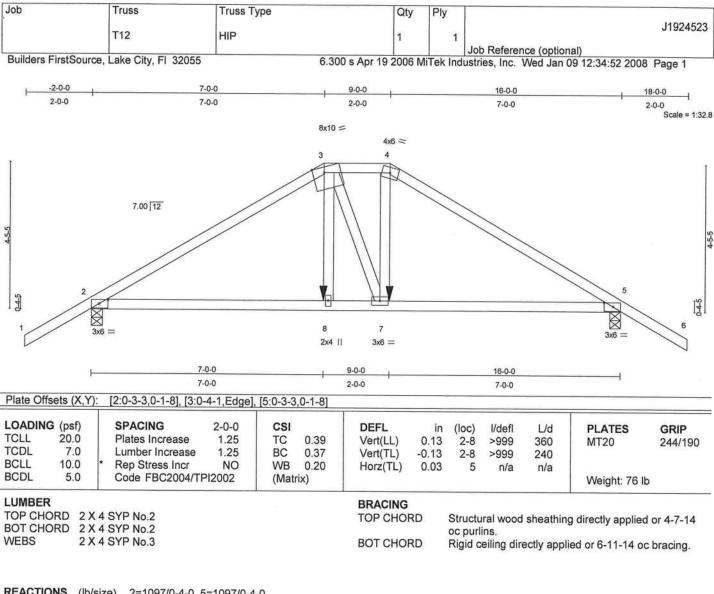
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 939 lb uplift at joint 1 and 1326 lb uplift at
- 9) Girder carries tie-in span(s): 4-5-0 from 0-0-0 to 17-4-0
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-22=-54, 22-23=-141(F=-87), 4-23=-114(F=-60), 4-8=-114(F=-60), 1-7=-49(F=-39)





REACTIONS (lb/size) 2=1097/0-4-0, 5=1097/0-4-0

Max Horz 2=-113(load case 3)

Max Uplift 2=-670(load case 5), 5=-670(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-1587/865, 3-4=-1297/802, 4-5=-1590/867, 5-6=0/54

BOT CHORD 2-8=-754/1276, 7-8=-768/1294, 5-7=-683/1279 WEBS 3-8=-390/520, 3-7=-176/165, 4-7=-489/633

JOINT STRESS INDEX

2 = 0.70, 3 = 0.50, 4 = 0.69, 5 = 0.71, 7 = 0.41 and 8 = 0.38

NOTES

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 670 lb uplift at joint 2 and 670 lb uplift at joint 5.

Confirmed arries by end with 7-0-0 end setback.

January 9,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conner Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occil. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply		#WWW.
ĺ	T12	HIP	1	1		J1924523
	Sauras I also City El 2				Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

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NOTES

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

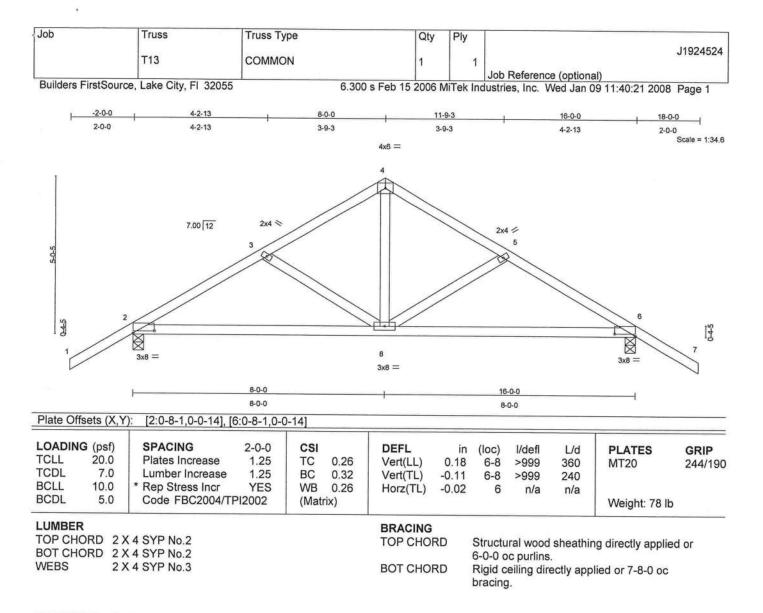
Vert: 1-3=-54, 3-4=-72(F=-18), 4-6=-54, 2-8=-10, 7-8=-69(F=-59), 5-7=-10

Concentrated Loads (lb)

Vert: 8=-411(F) 7=-411(F)

Julius Les Truss Design Engineer Florida PE No. 34869 1169 Coestal Bay Blyd Boyston Bassel





REACTIONS (lb/size) 2=618/0-4-0, 6=618/0-4-0

Max Horz 2=129(load case 5)

Max Uplift 2=-402(load case 6), 6=-402(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/54, 2-3=-724/861, 3-4=-532/787, 4-5=-532/787, 5-6=-724/861, 6-7=0/54 TOP CHORD

BOT CHORD 2-8=-608/566, 6-8=-608/566

WEBS 3-8=-193/200, 4-8=-653/330, 5-8=-193/200

JOINT STRESS INDEX

2 = 0.75, 3 = 0.12, 4 = 0.26, 5 = 0.12, 6 = 0.75 and 8 = 0.16

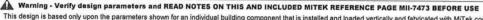
NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continues age assumed to be SYP No.2 crushing capacity of 565.00 psi





Job	Truss	Truss Type	Qty	Ply		
	T13	COMMON	1	1		J1924524
					Job Reference (optional)	

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 09 11:40:21 2008 Page 2

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 402 lb uplift at joint 2 and 402 lb uplift at joint 6.

LOAD CASE(S) Standard

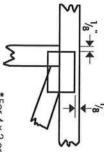


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless plates to both sides of truss and Dimensions are in inches. Apply dimensions indicate otherwise. securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



This symbol indicates the connector plates. required direction of slots in

PLATE SIZE

4 × 4

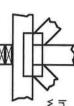
dimension is the length paralle to slots. perpendicular to slots. Second The first dimension is the width

LATERAL BRACING



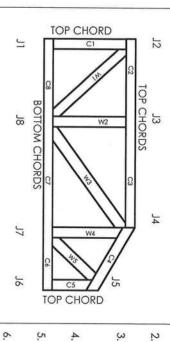
continuous lateral bracing. Indicates location of required

BEARING



which bearings (supports) occur. Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

ICBO

3907, 4922

96-31, 96-67

BOCA

9667, 9432A

SBCCI

WISC/DILHR

960022-W, 970036-N

561





MiTek Engineering Reference Sheet: MII-7473

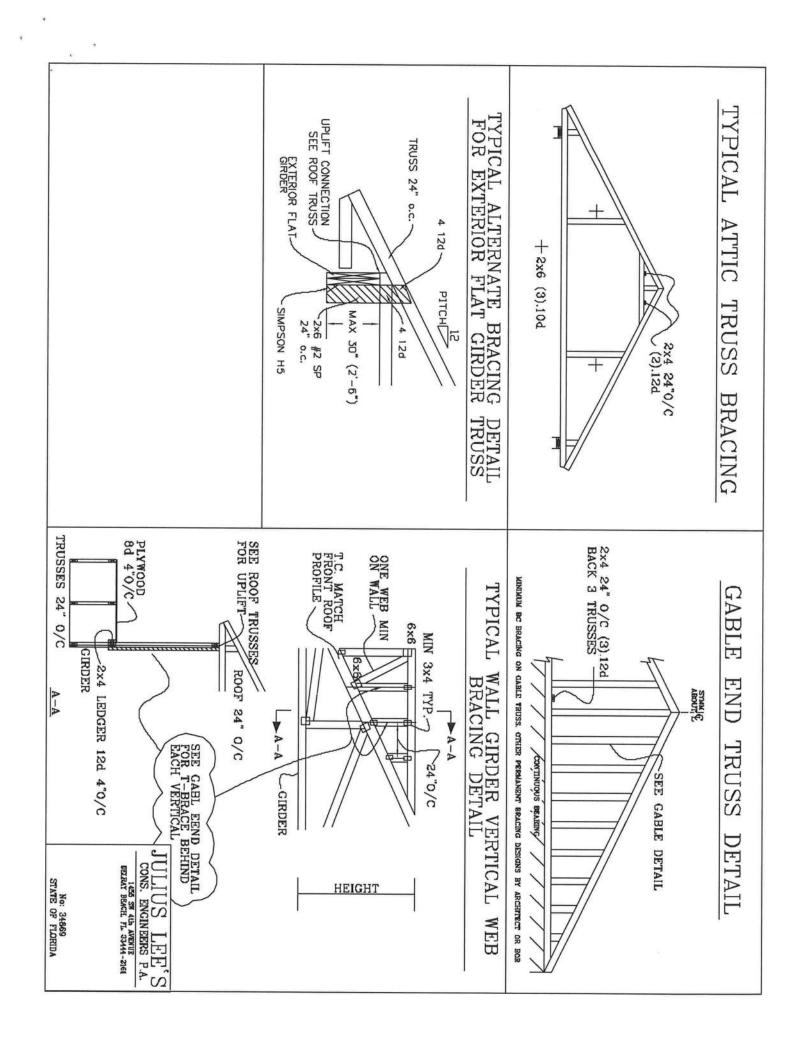
Damage or Personal Injury Failure to Follow Could Cause Property General Safety Notes

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 12 Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- 7 Camber is a non-structural consideration and practice is to camber for dead load deflection is the responsibility of truss fabricator. General
- 00 shown indicate minimum plating requirements. Plate type, size and location dimensions
- grade specified in all respects, equal to or better than the Lumber shall be of the species and size, and
- Top chords must be sheathed or purlins provided at spacing shown on design.
- 11. Boffom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Anchorage and / or load transferring others unless shown connections to trusses are the responsibility of
- Do not overload roof or floor trusses with stacks of construction materials.
- Do not cut or after truss member or plate engineer. without prior approval of a professional
- Care should be exercised in handling erection and installation of trusses

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		MAX GABLE VERTICAL LENGTH
		TOWN LENGTH IS 14: CABLE VERTICAL SPACING SPECIES GABLE VERTICAL SPECIES GABLE VERTICAL SPECIES GABLE VERTICAL SPECIES SPECIES MAX GABLE VERTICAL SPECIES SPECIES MAX GABLE VERTICAL SPECIES SPEC
		STAN STAN STAN STAN STAN STAN STAN STAN
	HEVARNONGHE BRACING. REFE PLATE (HSTITUTI OF AMERICA, 63 THESE FUNCTION STRUCTURAL PA	
	RUSSES REQUIRE R TO BCS) 1-G2 R TO BCS) 1-G3 R TO BCSS 01-M R UNICESS 01-M RELS AND BOTTO	
	EXTREME CARE BUILDING CONFID OR, NUTTE FOOL N, MUISON, VI 3 CHORD SHALL H	A CROUP A CROU
	ENT SAFETY INFE ENT SAFETY INFE MADISON, VI. 53 37(9) FOR SAFET J. TUP CHORD SH AVE A PROPERLY	# (1) 2X4 "L" BR # CROUP A CRO # 6: 11" 7 # 6: 11" 7 # 6: 11" 7 # 6: 11" 7 # 7: 11" 8 # 8: 8" 8 # 8: 9" 9 9 8 # 8: 9" 9 9 9 8 # 8: 9" 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9
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	WARNING* TRUSSES REDUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST 1-OG (BUILING) COMPONENT SAFETY MEDMANION, PUBLISED BY TRY (TRUSS PLANE (USTITUEE, 583 IDDNORTHO BA, DUTTE ROM, HANDLING, VET SAFETY FAMILY NEW ACCOUNT RUSS COLUMNIO OF MAREILA, 6300 ENTEMPRISE, UM, HANDSIN, VI 53719) FOR SAFETY FAMILTICES REDURE TO PRETENTING THESE TUNCTIONS, UNICESS OFFERSY MEDICATED, 1019 DORD SHALL HAVE PROPERLY ATTACKED STRUCTURAL PANELS AND INDITION CHORG SHALL HAVE A PROPERLY ATTACKED CEILING.	22X4 17 BB 3 3 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
ST		FOR MAN (1) (1) (1) (1) (1) (1) (1) (1) (1) (1)
No: 34869 STATE OF FLORIDA	JLIUS LEE'S CONS. ENGINEERS P.A. DELLAN BEACH, P.C. 33441-2161	THE NOLUCE IN EXCES OF THE STACE OF THE STAC
	EF.S.	A T. BRACE • (2) 238 T. P A GROUP B GROUP A O II 2 II I
MAX. TOT. LD.		HEACE TO THE LICE
LD. 60 PSF	-	
	REF ASC DATE 11, DRWG MIEK -ENG	BRACING GROUP SPECIES AND GRADES: GROUP A: SPRICE-PINE-PIR FI / 42 STANDARD STUD STUD STANDARD GROUP B: HEM-PIR
	REF ASCEY-02-GAB13015 DATE 11/26/03 DRWC MIEK SID CABLE 15 E HT -ENG	CIES AND GRADES: P.A: HEM-PIR A2 STUD SOUTHERN PINE A3 STUD SOUTHERN PINE A4 A5 CETAIL NOTES: ERIA IS L/240. PEON 4 0°. OVERHANC, DR 12° OVERHANCH A1 A1 A1 A1 A1 A1 A1 A1 A1 A
	E HT	ASB SESS OF CO. AND SESS OF CO

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WINNED THEN DIAGONAL BRACE IS USED. CONNECT DIACONAL BRACE FOR SEG AT EACH END. MAX WEB TOTAL LENGTH IS 14*. **GABLE** MAX VERTICAL LENGTH VERTICAL LENGTH IN TABLE ABOVE. SPACING SPECIES 12" 24" O.C. 16 O.C. O.C. MODPOINT OF VERTICAL WEB. GABLE VERTICAL SPF SPF SPF DFL DFL DFL SP H SP H SP H ASCE STUD STANDARD #2 #3 NAOHS STANDARD STANDARD GRADE STANDARD **\$1** / STANDARD STANDARD #2 #3 STUD STUD \$ to to BRACE 7-02: MAYARONGIN TRUSSES REDURE EXTREME CARE IN FARRICATING, HANDLOG, SUPPING, DISTALLING MO BACCHO. BEER TO BEST 1-43 KRULDING CHEPOKET SAFETY (HEDRATIDO), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 383 THORPOKET SAFETY BANDISON, HE SAFETY RACIOTES PRIZE TO PERCHANIS CHACHE THE RESERVENCE, 6300 ENTERPRIZE LIM, MODISON, HE SAFETY RACIFICES PRIZE TO PERCHANIS CHACHE TO PERCHANIS THE SAFETY BANCE PROPERTY ATTACHED RESERVED ATTACHED STRUCTURAL PANCE PROPERTY ATTACHED STRUCTURAL PANCE GABLE TRUSS BRACES 4 4 4 0 0 0 4 3 3 0 8 9 130 OR DOUBLE CUT (AS SHOWN) ZX4 SP OR DF-L #Z OH BETTER DIAGONAL BRACE; SINGLE GROUP A (1) 1X4 "L" BRACE . MPH 9 0 GROUP B 6' 10" WIND 4. 9. 5 5 6 6 GROUP A (1) 2X4 "L" BRACE • (2) 2X4 "L" BRACE •• SPEED, 0 0 0 REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH SI WINTE 30 GROUP A MEAN 10 10 EX4 MEN OR BETTER CONTINUOUS GROUP B 10. 1. HEIGHT, • SNIBVÉE ULIUS LEE'S cons. ENGINEERS P.A. (1) 2X6 12 11 12, 11, 11. 5 11. 4 12. 11. GROUP A DELRAY BEACH, FL. 33444-2161 No: 34869 STATE OF FLORIDA ENCLOSED, "L" BRACE . GROUP B 12, 10, 12 GROUP A (2) 13' 3" 10, 10, 14. 0 2XB r. _ MAX. MAX 14' 0" 12' 11' CROUP B BRACE 12' 6' 12' 6' 14' 0° II 14' 0" 14 0 0 0 14' 0" 13, 5, 12' 3" 12' 3" 10' 7" 14' 0" 14. 0 TOT. SPACING 1.00, F ATTACH EACH 'L' BRACE WITH 104 NAILS. 18 FOR (1) 'L' BRACE: SPACE NAILS AT 2' O.C. BETWEEN ZONES. 4* FOR (2) 'L' BRACES: SPACE NAILS AT 3' O.C. IN 18' END ZONES AND 6' O.C. BETWEEN ZONES. CARLE END SUPPORTS LOAD FROM 4: 0" PROVIDE UPLAT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LIVE LOAD DEPLECTION CRITERIA IS L/240. MEMBER LENGTH. L' BRACING MUST BE A MINIMUM OF 80% OF WEB SPRUCE-PINE-FIR #1 / #2 STANDARD #3 STUD DOUGLAS FIR-LARCH #3 STUD STANDARD PLYWOOD OVERHANG. BRACING GROUP SPECIES SOUTHERN PINE EXPOSURE 60 CABLE TRUSS GREATER THAN 4' D', BUT LESS THAN 11' 6" GREATER THAN 11' 6" VERTICAL LENGTH 24.0 REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES. CABLE VERTICAL PLATE SIZES PSF REF DATE DWG MITEK STD GABLE 30' E HT HEM-FIR FI & BIR GROUP GROUP DETAIL 0 DOUGLAS FIR-LARCH 11/26/03 STANDARD ASCE7-02-GAB13030 Ŗ A. 42 STUD NO SPLICE AND HEM-PIR 200 NOTES: 22 2 GRADES:



TOP CHORD 2X4 \$2 OR BETTER
BOT CHORD 2X4 \$2 OR BETTER
WEBS 2X4 \$3 OR BETTER

PIGGYBACK DETAIL

TYPE

SPANS

뒫

8

12

8 3

52'

4X8

2.5X4

2.5X4

9XG

5X8

5X6

REFER TO SEALED DESIGN FOR DASHED PLATES

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE DMITTED. ATTACH VERTICAL WERS TO

PIGCYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDC, LOCATED ANYWHERE IN ROOF, I MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF 110 MPH WIND, 30' MEAN HGT, FBG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

m 0

4XS OR 3X6 TRULOX AT 4'
HOTATED VERTICALLY

00,

0 8 >

1.5X3

1.5X4

1.5X4

1.5X4 5X6

584

5X5

535

FRONT FACE (5,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. EITHER PLATE LOCATION IS ACCEPTABLE MAX C 12 20' FLAT TOP CHORD MAX SPAN TYP. 图 B 8 MAX SIZE OF ZXIZ 要 -TYP. D-SPLICE

WEB LENGTH	REQUIRED BRACING
o' To 7'9"	NO BRACING
7'9" TO 10'	1x4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 164 NAILS AT 4" OC.

* PICGYBACK SPECIAL PLATE

ATTACH TRUGOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUGOX INFORMATION.

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		ENACING. REFER TO EXIL 1-03 GUILDING COMPONENT SAFETY DECIMATION, PUBLICHED BY TO! CIRCLES AND PARTICLES. REFER TO EXIL 1-03 GUILDING COMPONENT SAFETY DECIMATION, PUBLICHED BY TO! CIRCLES PARTICLES PROPERLY AND PRICE VOID HRISS COUNCIL OF ARERCA, AGO DETERRISE UN, MANDEN, VI. 33799 FOR SAFETY PRACTICES PRICE TO PORTOROMO. THE SE FINCTIONS. LALESS OTHERWINE DAIGNET AND FOR SAFETY PRACTICES PROPERLY ATTACHED STRUCTURAL PARCLS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGIO CEILNIG.	TATIACH FIGGEBACK WITH 3AB TRULUX OR ALFINE FIGGEBACK SPECIAL PLATE.
STATE OF FLORIDA		CONS. ENGINEERS P.A. DELEAN BEACH, FL. 33444-2161	THIS DRAW
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	MAX LOADING 55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC.	ING REPLACES DRAWINGS
		DATE 09/12/07 DRWGMITEK STD PIGG	63

VALLEY TRUSS DETAIL

TOP CHORD BOT CHORD SEEA 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 2X4 SP #3 OR BETTER. OR BETTER.

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEICHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9". UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING,

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

ENGINEERS' SEALED

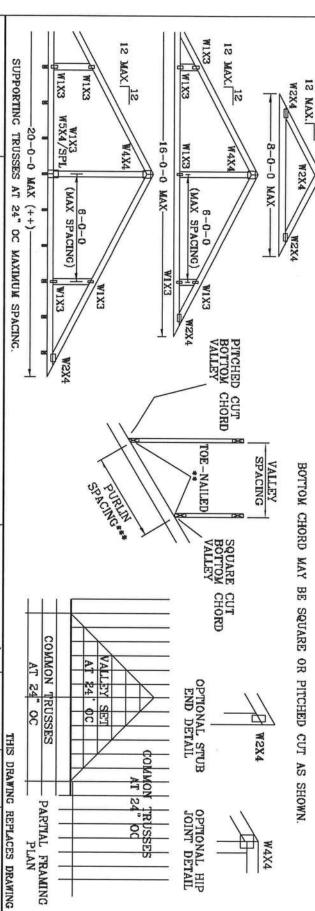
DESIGN.

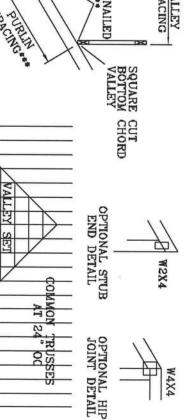
* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

4-0-0 MAX

+ LARGER SPANS MAY BE NOT EXCEED 12'0". BUILT AS LONG AS THE VERTICAL HEIGHT DOES





STATE OF FLORIDA	No: 34869			1455 SW 4th AVENUE	INEERS P.A.	LIUS LEE'S
SPA	DUR	TOT	BC	BC	TC	TC
SPACING	UR.FAC. 1.25	TOT. LD.	F	PL	DL	F
	5	32	0	CT.	~2	20
24"	1.25	40	0	5	15	88
		PSF	PSF	PSF	PSF	PSF
			-ENG JL	DRWG	DATE	REF
			JL	VALTRUSS1103	11/26/03	VALLEY DETAIL

SECURING MENTAL SECURIS EXTENDE TARE (N FABRICATING, MANDLING, SKIPPING, INSTALLING AND BACING REFER TO EXIL 1-20 GUILLING COPPINDIN SAFETY BAPDANDUN, PUBLICATED BY TRY (TRASS PLATE INSTITUE, SEO COORDERID INS, SUITE 200, MANISON, V. 153779) AND WITCH AVOID TRASS COLURCIUM FRANCIA, ASID ENTERPRISE NA, MANISON, VI 153779 FIRE SAFETY PRACTICES PRINT TO PERFORM THE SAFETY PARCLES AND BUTTON CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARCLES AND BUTTON CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARCLES AND BUTTON CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

 \overline{c}

A105

CONS.

DELRAY BEACH, I'L 3544

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

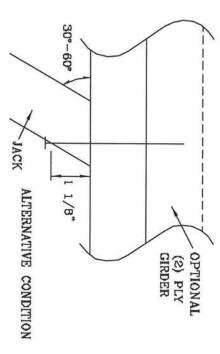
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

ALL VALUE	5	.4	မ	N	TOE-NAILS	NUMBER OF
S MAY BE	493#	394#	296#	187#	1 PLY	SOUTHE
ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR	639#	511#	383#	256#	2 PLIES	SOUTHERN PINE
ED BY APP	452#	361#	271#	181#	1 PLY	DOUGLAS
ROPRIATE	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
DURATION	390#	312#	234#	156#	1 PLY	
OF LOAD F	507#	406#	304#	203#	2 PLIES	HEM-FIR
ACTOR.	384#	307#	230#	154#	1 PLY	SPRUCE
	496#	397#	298#	199#	2 PLIES	SPRUCE PINE FIR

OPTIONAL (2) PLY GIRDER



THIS DRAWING REPLACES DRAWING 784040

JACK

_						
			STRUCTURAL PANELS AND BUTTON COURD SHALL HAVE A PROPERTY ATTACHED RIGID CEILING	PLATE INSTITUTE, 383 D'ONGERIO DE, SUITE 200, NADISON, ME 33719) AND VICA (MOD) TRUSS COLNCIL. OF ANCRICA, 6300 ENTEDERISE, LM, MADISON, ME 33719) TOR SAFETY PRACFICES PRIER TO PERFORMING THESE FINITITUE INTERES TORS DUSTE INDICATES TOR CONTROL MANY PRACFICES PRIER TO PERFORMING		
STATE OF FLORIDA	No: 34869			DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
SPACING	DUR.	TOT. LD.	BC LL	BC L	TC DL	TC LL
ING	DUR. FAC.	IJ.		DL	Ē	L
	1.00					
		PSF	PSF	PSF	PSF	PSF
			-ENG JL	DRWG	DATE	REF
			JL	CNTONAIL1103	09/12/07	TOE-NAIL

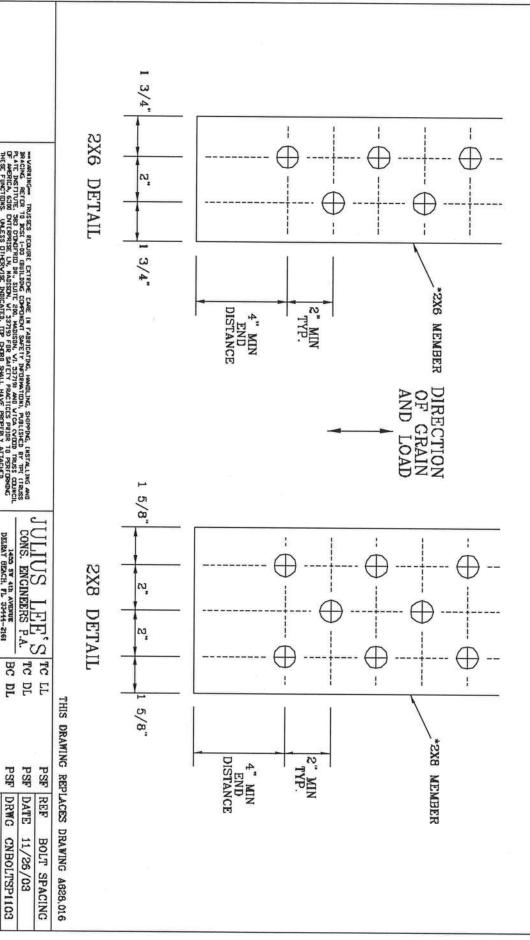
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PATTERNS SHOWN BELOW. APPLIED

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



c CONS.

DELRAY BEACH, FL 33444-2161

BC LL

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IL

PSF

PSF PSF

DATE

11/26/03

DRWG -ENG

CNBOLTSP1103

No: 34869 STATE OF FLORIDA

SPACING DUR. FAC. TOT. LD.

TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\(\phi \)).

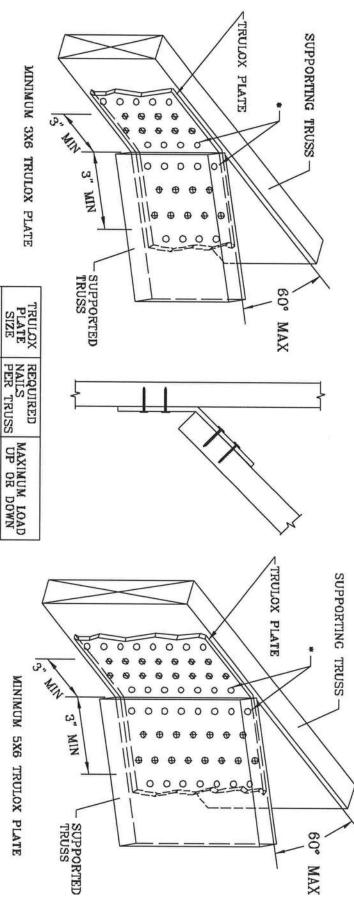
* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO PINE DOUGLAS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HAMILING, SHIPPING, 1910 INC.31 HOR GUILLING, DEPORTATION PROLITING THE REPORT OF THE PROMITION PROLITING IN THE 28D MAINTEN, VI. 33719 AND VICA KNOW ON THE AREA MAINTEN, VI. 33719 FOR SAFETY PRACTICES PRICE TO THE PROLITING SHIPPING INC. SAFETY PRACTICES PRICE NO. MALESS (THERVISE MILICATER), TO PORDE SHALL HAVE REPORTED.

3X6

15

350#

SOITIOR

LEE'S

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

1455 SW 44h AVENUE DELBAY BEACH, FL 35444-2161

DATE

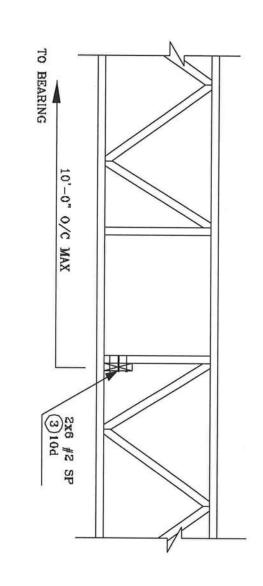
TRULOX 11/26/03

DRWG

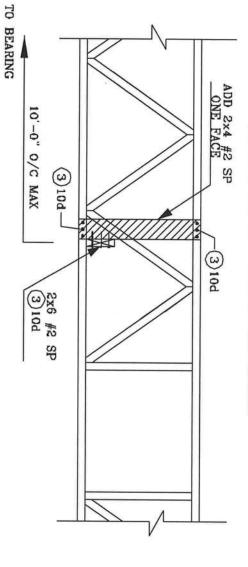
CNTRULOX1103

No: 34869 STATE OF FLORIDA

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP

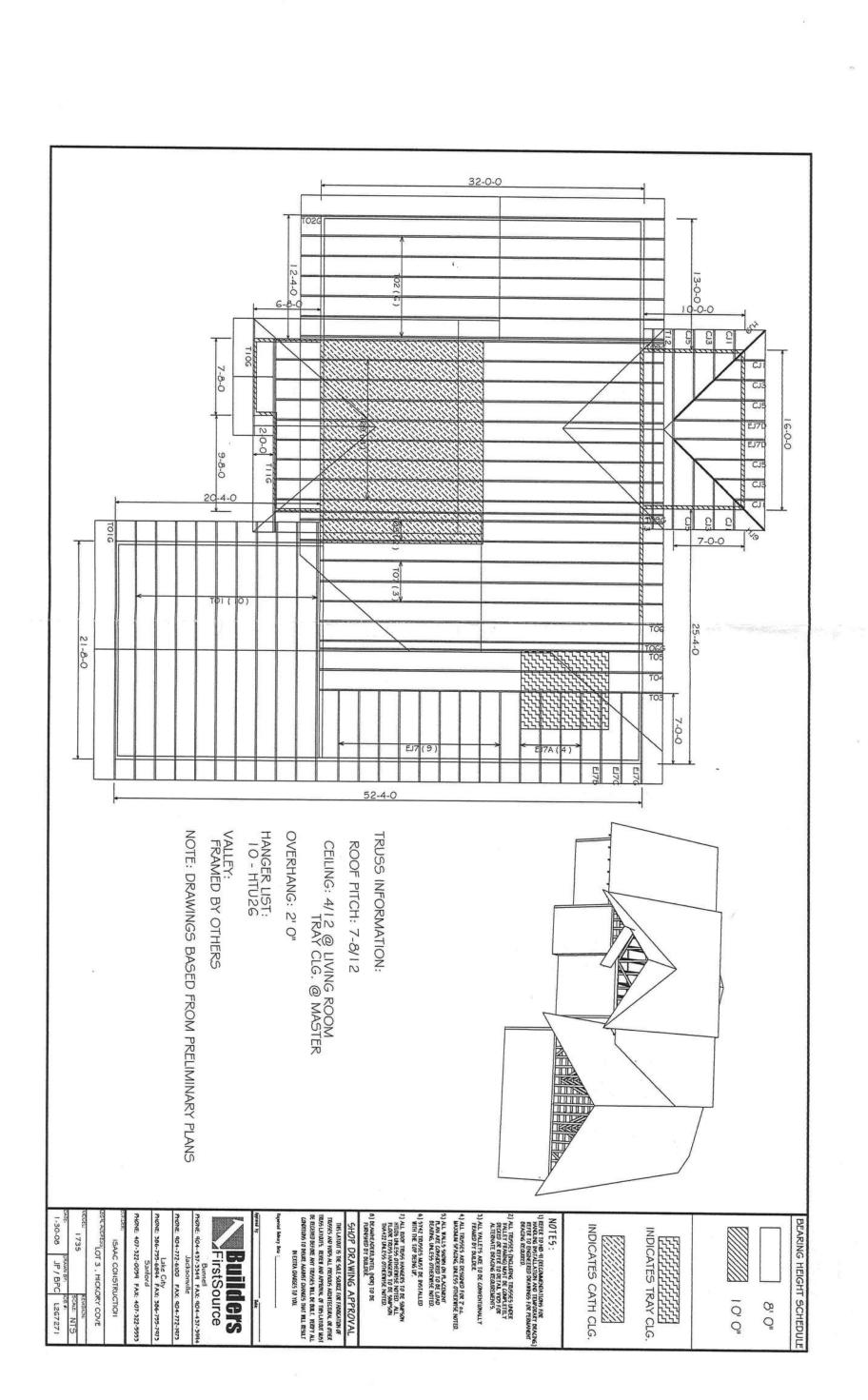


JULIUS LEE'S CONS. ENGINEERS P.A.

1455 SW 415 AVENUE
DELEAY BEACH, FL 3344-2161

No: 34869 STATE OF FLORIDA

ty



PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval

number for any of the applicable listed products.

Category/Subcategory	Manufacturer	Product Description	
1. EXTERIOR DOORS			Approval Number
A. SWINGING	PLASTPRO IN	C 3068 + 6068 Fiberalass	
B. SLIDING	CAPITAL!	2008 + 6068 Fiberglass	4760.162
C. SECTIONAL	Raynor	Classia Southand Comment	7055.1
D. ROLL UP	Janus	Classic Sectional Garage Door	
E. AUTOMATIC		model 3100-Rolling Shoot Du	R FL-2274
F. OTHER			
2. WINDOWS			
A. SINGLE HUNG	000:1-1		
B. HORIZONTAL SLIDER	CAPITAL	48 x 84	6029.7
C. CASEMENT	capital	126 x59	6024.4
D. DOUBLE HUNG	30 M		(400.4.)
E. FIXED	Danvid	single hung windows	PL1369
F. AWNING	capital	96×72	6028/20
G. PASS THROUGH			400.000
H. PROJECTED			/
			· · · · · · · · · · · · · · · · · · ·
I. MULLION			
J. WIND BREAKER			
K. DUAL ACTION			
OTHER			
. PANEL WALL			
. SIDING	Alcoa	winth aidean	5
. SOFFITS	ASI Building Pro	rinyl siding	FL1621
. EIFS	AST BUILTING THE	. Aluminum & vinuy soffit	FL55461&2
STOREFRONTS		0	
CURTAIN WALLS			
WALL LOUVER			
GLASS BLOCK			
MEMBRANE			
GREENHOUSE			
OTHER			
OTTIEN	*		-
ROOFING PRODUCTS			
ASPHALT SHINGLES	The older		
	Tamko	30-YEAR Shingles asphalt	PL673
UNDERLAYMENTS ROOFING		3.2 (2)	10013
STENERS			
ION-STRUCTURAL			
METAL ROOFING		,	
WOOD SHINGLES AND		3.8	
SHAKES			
ROOFING TILES	ě.		
ROOFING			
JLATION			
VATERPROOFING			

ROOF SYSTEMS	**		
J. MODIFIED BITUMEN			
K. SINGLE PLY ROOF SYSTEMS	. *	•	
L. ROOFING SLATE		2/	
M. CEMENTS-ADHESIVES COATINGS		2	

Category/Subcategory	Manufacturer	Product Description	Approval Numbe
N. LÍQUID APPLIED ROOF SYSTEMS			Approval Number
O. ROOF TILE ADHESIVE			
P. SPRAY APPLIED POLYURETHANE ROOF	* *	2	
Q. OTHER			
	*		
5. SHUTTERS	•		
A. ACCORDION			
B. BAHAMA	s" e		
C. STORM PANELS	9		
D. COLONIAL	1,		
E. ROLL-UP			
F. EQUIPMENT			
G. OTHERS			
6. SKYLIGHTS			
A. SKYLIGHT			
			N
B. OTHER		,	
. STRUCTURAL			
COMPONENTS			
. WOOD CONNECTORS/			
ANCHORS	Simpson Steony	Ward con mentale / suchor o	FL1474
. TRUSS PLATES	Alpine Engineered	Produkt - Alpine Truss Plates	91999
. LINGINEERED		TOUGH - SAIPINE TRAUS PHATES	84999
UMBER	LPEWP	laminated Beams I Joist	PL ISII
. RAILING		, , , , , , , , , , , , , , , , ,	101311
COOLERS-FREEZERS			
CONCRETE			
ADMIXTURES	A A		
MATERIAL			
INSULATION FORMS			
PLASTICS			
DECK-ROOF			
WALL			
SHEDS			
OTHER			
NEW EXTERIOR	****	,	
ENVELOPE	10		-
ODUCTS		3	
	FS.0		

The products listed below did not demonstrate product approval at plan review. I understand that at the time of products, the following information must be available to the inspector on the jobsite; 1) copy of the product appropriate characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacture requirements. Further, I understand these products may have to be removed if approval cannot be demonstrate inspection.	roval, 2) the per ers installation
inspection.	

APPLICANT SIGNATURE

DATE

L:/GENERAL/STATEPROD.XLS

Residential System Sizing Calculation

Summary Project Title:

Spec House

Lake City, FL 32025-

Project Title: Venture Pointe LLC - Model 1735 Code Only Professional Version Climate: North

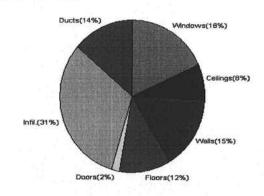
1/31/2008

				1/01/200	J
Location for weather data: Gaine	sville - Def	aults: Latitu	ude(29) Altitude(152 ft.) Temp Ran	ge(M)	
Humidity data: Interior RH (50%) Outdoor	wet bulb (7	7F) Humidity difference(54gr.)	- x x x	
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	26946	Btuh	Total cooling load calculation	35263	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	118.8	32000	Sensible (SHR = 0.75)	89.3	24000
Heat Pump + Auxiliary(0.0kW)	118.8	32000	Latent	95.5	8000
			Total (Electric Heat Pump)	90.7	32000

WINTER CALCULATIONS

Winter Heating Load (for 1735 sqft)

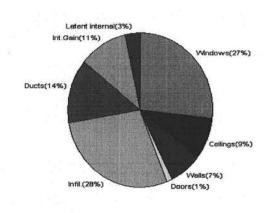
Load component			Load	
Window total	153	sqft	4925	Btuh
Wall total	1233	sqft	4049	Btuh
Door total	38	sqft	492	Btuh
Ceiling total	1850	sqft	2180	Btuh
Floor total	194	sqft	3173	Btuh
Infiltration	208	cfm	8433	Btuh
Duct loss			3694	Btuh
Subtotal		- 1	26946	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			26946	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1735 sqft)

Load component			Load	
Window total	153	sqft	9487	Btuh
Wall total	1233	sqft	2485	Btuh
Door total	38	sqft	372	Btuh
Ceiling total	1850	sqft	3064	Btuh
Floor total			0	Btuh
Infiltration	182	cfm	3390	Btuh
Internal gain			3780	Btuh
Duct gain			4309	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			26887	Btuh
Latent gain(ducts)			518	Btuh
Latent gain(infiltration)			6658	Btuh
Latent gain(ventilation)	0	Btuh		
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			8375	Btuh
TOTAL HEAT GAIN			35263	Btuh





Version 8 For Florida residences only EnergyGauge® System Sizing PREPARED BY:

DATE: 1/31/1

EnergyGauge® FLRCPB v4.5.2

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Spec House

Project Title:

Lake City, FL 32025-

Venture Pointe LLC - Model 1735

Code Only **Professional Version**

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/31/2008

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	60.0	32.2	1931 Btuh
2	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
3	2, Clear, Metal, 0.87	E	40.0	32.2	1288 Btuh
4	2, Clear, Metal, 0.87	S	8.0	32.2	258 Btuh
5	2, Clear, Metal, 0.87	S	5.0	32.2	161 Btuh
	Window Total		153(sqft)		4925 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1083	3.3	3557 Btuh
2	Frame - Wood - Adj(0.09)	13.0	150	3.3	493 Btuh
	Wall Total		1233		4049 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		18	12.9	233 Btuh
200	Door Total		38		492Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1850	1.2	2180 Btuh
	Ceiling Total		1850		2180Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	194.0 ft(p)	16.4	3173 Btuh
	Floor Total		194		3173 Btuh
			Envelope Su	ıbtotal:	14819 Btuh
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqf	t) CFM=	
	Natural	0.80	15615 1233	208.2	8433 Btuh
Ductload		3	(D	LM of 0.159)	3694 Btuh
All Zones		Sen	sible Subtotal Al	l Zones	26946 Btuh

WHOLE HOUSE TOTALS

Subtatal Canaible	26046 Ptub
Ventilation Sensible	26946 Btuh 0 Btuh
Total Btuh Loss	26946 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Project Title:

Lake City, FL 32025-

Venture Pointe LLC - Model 1735

Code Only Professional Version

Climate: North

1/31/2008

EQUIPMENT

1. Electric Heat Pump

#

32000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details Project Title: Code C

Spec House

Lake City, FL 32025-

Venture Pointe LLC - Model 1735

Code Only Professional Version

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/31/2008

Component	Loads	for Zone #1:	Main
		SECTION ASSESSED.	Manager.
	1-		

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	60.0	32.2	1931 Btuh
2	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
3	2, Clear, Metal, 0.87	E	40.0	32.2	1288 Btuh
4	2, Clear, Metal, 0.87	S	8.0	32.2	258 Btuh
5	2, Clear, Metal, 0.87	S	5.0	32.2	161 Btuh
	Window Total		153(sqft)		4925 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1083	3.3	3557 Btuh
2	Frame - Wood - Adj(0.09)	13.0	150	3.3	493 Btuh
	Wall Total		1233		4049 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		18	12.9	233 Btuh
	Door Total		38		492Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1850	1.2	2180 Btuh
	Ceiling Total		1850		2180Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	194.0 ft(p)	16.4	3173 Btuh
	Floor Total		194		3173 Btuh
		Z	Zone Envelope Su	btotal:	14819 Btuh
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqff) CFM=	
	Natural	0.80	15615 1233	208.2	8433 Btuh
Ductload	Pro. leak free, Supply(R6.0-Attic), Return(R6.0-Attic) (DLM of 0.159)				3694 Btuh
Zone #1		Sens	sible Zone Subto	otal	26946 Btuh

WHOLE HOUSE TOTA	.s	
	Subtotal Sensible Ventilation Sensible Total Btuh Loss	26946 Btuh 0 Btuh 26946 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Project Title: Venture Pointe LLC - Model 1735

Lake City, FL 32025-

Code Only Professional Version

Climate: North

1/31/2008

EQUIPMENT

1. Electric Heat Pump

#

32000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default) (HTM - ManualJ Heat Transfer Multiplier)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

ACCA MRMURL 3

Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Spec House

Project Title:

Code Only Professional Version

Lake City, FL 32025-

Venture Pointe LLC - Model 1735

Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/31/2008

Component Loads for Whole House

	Type*	Overhang			Window Area(sqft)			H	MTH	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	60.0	0.0	60.0	29	80	4771	Btuh
2	2, Clear, 0.87, None,N,N	W	11.5f	8ft.	40.0	40.0	0.0	29	80	1158	
3	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	40.0	0.0	40.0	29	80	3181	100 500 00 00 00 00
4	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	8.0	8.0	0.0	29	34	232	
5	2, Clear, 0.87, None, N, N	S	1.5ft	8ft.	5.0	5.0	0.0	29	34	145	
	Window Total		4		153 (sqft)				9487	Btuh
Walls	Туре		R-Va	lue/U	-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	108	33.0		2.1	2259	Btuh
2	Frame - Wood - Adj			13.0/	0.09	15	0.0		1.5	226	Btuh
	Wall Total					123	33 (sqft)			2485	Btuh
Doors	Type						(sqft)		HTM	Load	
1	Insulated - Exterior						0.0		9.8	196	Btuh
2	Insulated - Adjacent						3.0		9.8	176	
	Door Total				38 (sqft)			372	Btuh		
Ceilings	Type/Color/Surface		R-Va	lue		Area(sqft)		нтм	Load		
1	Vented Attic/DarkShingle			30.0		1850.0			1.7	3064	Btuh
8	Ceiling Total			2000000				1.7	3064		
Classo			D \/-	de la constant			0 (sqft)		LITAA		Dluii
Floors	Туре		R-Va					HTM	Load		
1	Slab On Grade			5.0			94 (ft(p))		0.0	0	
	Floor Total					194	.0 (sqft)			0	Btuh
	-					Е	nvelope	Subtota	l:	15408	Btuh
nfiltration	Туре		А	СН	Volum	e(cuft)	wall area	(saft)	CFM=	Load	
	SensibleNatural		15.00	0.70		15615	1233	(-4.4)	208.2	3390	Btuh
Internal			Occup	ants		Btuh/od	cupant	-	Appliance	Load	
gain				6		X 23			2400	3780	Btuh
						S	ensible E	Envelope	e Load:	22578	Btuh
Duct load							(DG	M of 0.1	91)	4309	Btuh
						Sei	nsible L	oad All	Zones	26887	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Venture Pointe LLC - Model 1735

Code Only Professional Version Climate: North

1/31/2008

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	22578	Btu
	Sensible Duct Load	4309	Btu
	Total Sensible Zone Loads	26887	Btu
	Sensible ventilation	0	Btu
	Blower	0	Btu
Whole House	Total sensible gain	26887	Btu
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6658	Btu
	Latent ventilation gain	0	Btu
	Latent duct gain	518	Btu
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btu
	Latent other gain	0	Btu
	Latent total gain	8375	Btu
	TOTAL GAIN	35263	Btu

EQUIPMENT		
1. Central Unit	#	32000 Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Spec House

Project Title:

Venture Pointe LLC - Model 1735

Code Only Professional Version

Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/31/2008

Component Loads for Zone #1: Main

	Type*		Over	hang	Win	Vindow Area(sqft) HTM		Load			
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hqt	Gross		Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	60.0	0.0	60.0	29	80	4771	Btuh
2	2, Clear, 0.87, None,N,N	W	11.5f	8ft.	40.0	40.0	0.0	29	80	1158	Btuh
3	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	40.0	0.0	40.0	29	80	3181	Btuh
4	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	8.0	8.0	0.0	29	34	232	
5	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	5.0	5.0	0.0	29	34		Btuh
	Window Total				153 (sqft)				9487	Btuh
Walls	Туре		R-Va	alue/U	I-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	108	83.0		2.1	2259	Btuh
2	Frame - Wood - Adj			13.0/	0.09	15	0.0		1.5	226	Btuh
	Wall Total					123	1233 (sqft)			2485	
Doors	Туре					Area	(sqft)		HTM	Load	
1	Insulated - Exterior					20	0.0		9.8	196	Btuh
2	Insulated - Adjacent					18	8.0		9.8	176	Btuh
	Door Total					3	38 (sqft)			372	Btuh
Ceilings	Type/Color/Surface		R-Va	alue			(sqft)		HTM	Load	
1	Vented Attic/DarkShingle		30.0 1850.0			1.7	3064	Btuh			
	Ceiling Total						50 (sqft)			3064	
Floors	Туре		R-Va	alue			ize		НТМ	Load	
1	Slab On Grade			5.0		1	94 (ft(p))		0.0		Btuh
	Floor Total						.0 (sqft)			-	Btuh
							one Enve	elope Su	ubtotal:	15408	
nfiltration	Туре		Α	СН	Volum	e(cuft)	wall area	(saft)	CFM=	Load	
	SensibleNatural		•	0.70	7 010111	15615	1233	(04.1)	182.2	3390	Btuh
Internal			Occup	ants		Btuh/o	ccupant	-	Appliance	Load	
gain			•	6		X 23			2400	3780	Btuh
						S	ensible E	Envelope	e Load:	22578	Btuh
Duct load	Prop. leak free, Supply(R6.0-A	ttic), f	Return	n(R6.0-	Attic)		(DGM c	of 0.191)	4309	Btuh
							Sensib	le Zone	Load	26887	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Project Title: Venture Pointe LLC - Model 1735

Lake City, FL 32025-

Code Only **Professional Version** Climate: North

1/31/2008

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	22578	Btuh
	Sensible Duct Load	4309	Btuh
	Total Sensible Zone Loads	26887	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	26887	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6658	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	518	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	8375	Btuh
	TOTAL GAIN	35263	Btuh

EQUIPMENT		
1. Central Unit	#	32000 Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value) (BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

Residential Window Diversity

MidSummer

Spec House

K. 27 . * . * .

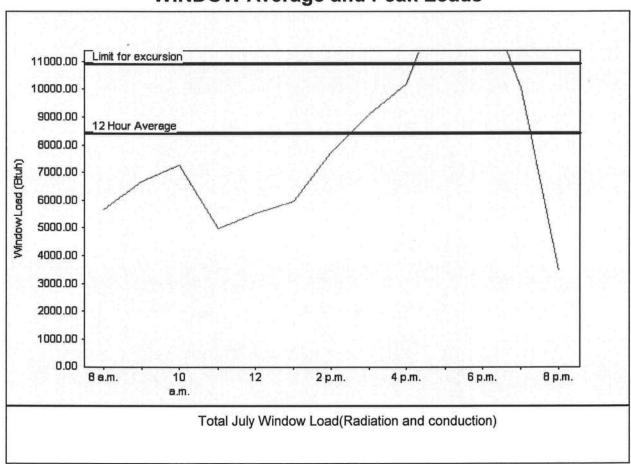
Lake City, FL 32025-

Project Title: Venture Pointe LLC - Model 1735 Code Only Professional Version Climate: North

1/31/2008

Weather data for: Gainesville - Defa	aults			
Summer design temperature	92	F	Average window load for July	8409 Btuh
Summer setpoint	75	F	Peak window load for July	14151 Btu
Summer temperature difference	17	F	Excusion limit(130% of Ave.)	10932 Btu
Latitude	29	North	Window excursion (July)	3219 Btuh

WINDOW Average and Peak Loads



This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences on
PREPARED BY:
DATE:



BOTTOM CHORD FILLER REPAIR

26755

.YAM

RECOMMENDED REPAIR PROCEDURE

- MEASURE DISTANCE FOR NEW LENGTH OF FILLER.
- N APPLY NEW 2X4 STUD GRADE OR BETTER VERTICAL NAILS 0.131" DIA x 3.0" OR LARGER, (I.E. 10d OR SCAB TO BOTTOM CHORD AND FILLER WITH (3) EACH END OF VERTICAL. 16d COMMON, SINKER, GUN, OR 16d BOX NAILS) TO
- 3 CAREFULLY REMOVE EFFECTED CONNECTOR PLATES. USE CARE NOT TO DAMAGE THE REMAINING CONNECTOR PLATES OR LUMBER IN ANY WAY.
- TRIM FILLER TO LENGTH, AT EDGE OF NEW VERTICAL

- MAXIMUM BOTTOM CHORD LOAD IS 10 PSF
- REMOVED. SEE NOTE #3.
- ++ FIELD APPLIED FILLER

15:15PM

2008 .0

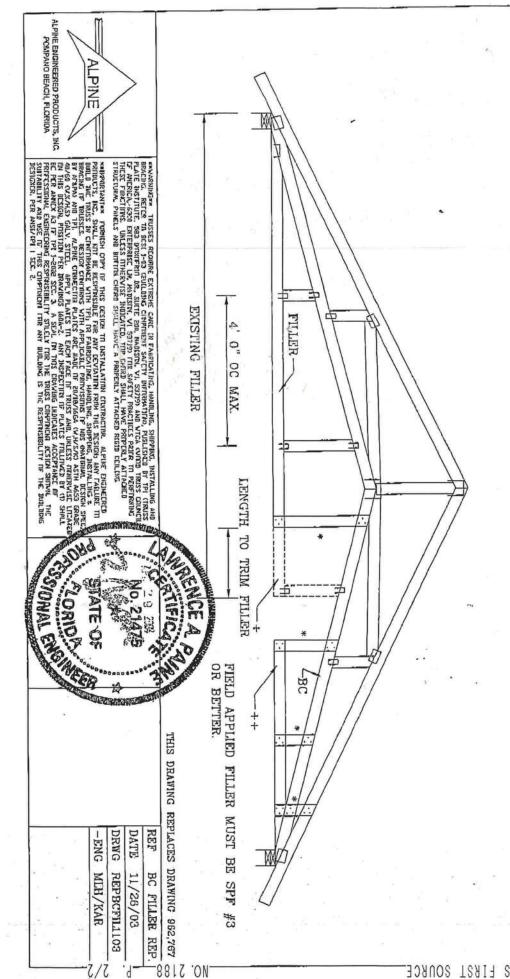
2X4 STUD GRADE OR BETTER VERTICAL SCAB. NAILS WITH A MIN. 0.131" DIA. X 3.0" LENGTH. AFTACH TO BOTTOM CHORD AND FILLER WITH (3)

FILLER DIMENSIONS, PLACEMENT, AND WEBBING. REFER TO ENGINEER'S SEALED DESIGN

BNILDERS

SONKCE

FIRST



'ON

NOTICE OF COMMENCEMENT FORM COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and inaccordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Tax Parcel ID Number 25-45-16-03)24-703 Permit Number 600026755
1. Description of property: (legal description of the property and street address or 911 address) 90 W to CR 341. Turn Left to CR 242, Turn Left to Asheville Rd Turn Right and it's the 3rd lot on left.
153 S.W. Asheville I Nav. Lake City. Fl 32024
2. General description of improvement: Resendital Home
2. Constant decompliant of improvement. <u>ICC Spring 1 to 1. 110 mile</u>
3. Owner Name & Address Isaac Construction, LLC
2109 WUS Hay 90 Suffer PMB#338Interest in Property
4. Name & Address of Fee Simple Owner (if other than owner):
5. Contractor Name Tagac Construction, LLC Phone Number 386-719-7143
Address 2109 W 45 Hwy 90 Suite 170 PMB#338 lake City Fl. 32055
6. Surety Holders Name NIA Phone Number
Address
Amount of Bond
7. Lender Name Gateway Bank of Central FL. Phone Number 352-231-1200
Address 2210 SE 17th Street, Ocola, Fl 34471
8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be
served as provided by section 718.13 (1)(a) 7; Florida Statutes:
Name Inst;200812005667 Date:3/24/2008 Time:11:13 AM
Address
9. In addition to himself/herself the owner designates of
to receive a copy of the Lien Notice as provided in Section 713.13 (1) –
(a) 7. Phone Number of the designee
10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)
THE OWNER MUST SIGN THE NOTICE OF COMMENCEMENT AND NO ONE ELSE MAY BE PERMITTED TO SIGN IN HIS/HER STEAD. Signature of Owner
Sworn to (or affirmed) and subscribed before day of 24th Day of Mud., 2008. NOTARY STAMP/SEAL Signature of Notary Signature of Notary
Bonded Troy Fain · Insurance, Inc. 800-385-7019

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA. 26755

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

HENDER HOLDER HELDER HER HER HER HER HER HER HER HER HER H
Section 1: General Information (Treating Company Information)
Company Name: _Aspan Past Control, Inc.
Company Address: P.O. Box 1785 City State Zip Zip
Company Business License No Company Phone No Company Phone No Company Phone No
FHA/VA Case No. (if any)
Section 2: Builder Information
Company Name: Tsuar Zuns + Company Phone No
Section 3: Property Information
Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip)
Type of Construction (More than one box may be checked)
Section 4: Treatment Information
Date(s) of Treatment(s) 3-26-0%
Brand Name of Product(s) Used
EPA Registration No. 53 % % 3 - 1 % 9
Approximate Final Mix Solution % Linear ft Linear ft. of Masonry Voids
Approximate Size of Treatment Area: Sq. ft Linear ft Linear ft. of Masonry Voids Approximate Total Gallons of Solution Applied 2 4 5
Was treatment completed on exterior?
Service Agreement Available?
Note: Some state laws require service agreements to be issued. This form does not preempt state law.
Attachments (List)
Comments
Name of Applicator(s) 5700 Bronner Certification No. (if required by State law)
The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.
Authorized Signature At Back Brown Date 3- 26-04
Authorized Signature Date 7- 24-0-6

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)



Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 25-4S-16-03124-103

64.20 Fire:

Building permit No. 000026755

Waste: 167.50

231.70 Total:

153 SW ASHEVILLE WAY, LAKE CITY, FL

Location:

Date: 12/16/2008

Owner of Building ISAAC CONSTRUCTION

Permit Holder ISAAC BRATKOVICH

Use Classification SFD/UTILITY

Building Inspector

POST IN A CONSPICUOUS PLACE Business Places Only) WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

<u>YOU ARE HEREBY NOTIFIED</u> as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit. **Owners Signature** CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit. Contractor's License Number 131 0593 Contractor's Signature (Permitee) **Columbia County** Competency Card Number Affirmed under penalty of perjury to by the Contractor and subscribed before me this 31 day of or Produced Identification Personally known Barbara C. Webster SEAL: Commission # DD329279

Page 2 of 2 (Both Pages must be submitted together.)

State of Florida Notary Signature (For the Contractor)

Revised 11-30-07

Expires July 2, 2008

For Office Use Only Application # 0801-180 Date Received 1/3 By Permit # 1556 - 26755 Zoning Official BLK Date 12.02.08 Flood Zone Application FEMA Map # 200 Zoning PRO Land Use L.D. Elevation N/A MFE River N/A Plans Examiner Date 2-7-05
Comments/ CNOC DEH Deed or PA Site Plan State Road Info Parent Parcel #
□ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Septic Permit No Fax 386-719-4757
Name Authorized Person Signing Permit Samantha Harrington Phone 386-719-9143
Address 209 W. US Hwy 90 Suite 170 PMB 338 Lake City, FL 32055
Owners Name ISAAC (onstruction Phone
911 Address 153 SW Asheville Way lake City fe 32024
Contractors Name Isaac Construction, LC Phone 386-719-7443
Address 2109 W 45 Hay 90 Suite 170 PMB 338 Lake City, FL 32055
Fee Simple Owner Name & Address
Bonding Co. Name & Address
Architect/Engineer Name & Address Will Myers/Mark Disasway
Mortgage Lenders Name & Address
Gircle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy
on the second period company, the control of the second control of
Property ID Number 25 - 45 - 16 - 03/24 - 103 Estimated Cost of Construction 1/2, 775.00
Subdivision Name Hickory Cove Lot 3 Block Unit Phase
Driving Directions 90 to Sister Welcome at 242 take a
left On Ashville Zood in 3rd on left
Number of Existing Dwellings on Property
Construction of Residenta home Total Acreage 320 Lot Size 90 65
Do you need a - <u>Culvert Permit</u> or <u>Culvert Waiver</u> or <u>Have an Existing Drive</u> Total Building Height <u>22++</u>
Actual Distance of Structure from Property Lines - Front 35 Side Side Rear_ 59.49
Number of Stories Heated Floor Area
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.
Page 1 of 2 (Both Pages must be submitted together.) Revised 11-30-07
sec Back over