

COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

This item has been digitally signed by Douglas Fleming on the date adjacent to the seal.

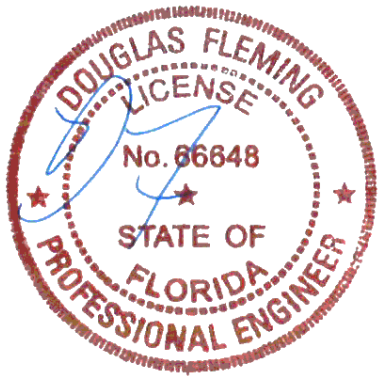
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Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 24-1285
Job Description: Hedrick	
Address:	

Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res.	IntelliVIEW Version: 23.02.01A through 24.01.02 JRef #: 1Y0W2150008
Wind Standard: ASCE 7-22 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 65 truss drawing(s) and 7 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	176.24.0951.30440	A01	2	176.24.0951.55437	A02
3	176.24.1034.26650	A03	4	176.24.0952.36717	A04
5	176.24.0952.50517	A05	6	176.24.0953.00080	A06
7	176.24.0953.08510	A07	8	176.24.0953.18777	A08
9	176.24.1034.44910	A09	10	176.24.1034.48917	A10
11	176.24.0953.45597	A11	12	176.24.0953.52110	A12
13	176.24.0953.58257	A13	14	176.24.1034.10847	A14
15	176.24.1000.07233	B01	16	176.24.1000.12130	B02
17	176.24.1000.14647	B03	18	176.24.1000.53177	B04
19	173.24.1518.48422	C01	20	176.24.1000.54683	C02
21	176.24.1000.56980	C03	22	173.24.1518.48271	C04
23	173.24.1518.48302	D01	24	176.24.1001.00080	D02
25	176.24.1001.01473	D03	26	176.24.1001.02673	D04
27	176.24.1001.04190	D05	28	176.24.1001.35000	D06
29	173.24.1518.48224	E01	30	173.24.1518.48162	E03
31	176.24.1001.39987	J01	32	176.24.1001.43977	J01HJ
33	176.24.1001.47700	J02	34	176.24.1001.51767	J02HJ
35	176.24.1001.54670	J03	36	176.24.1001.57687	J03HJ
37	176.24.1002.00983	J04	38	176.24.1002.04980	J05
39	176.24.1002.07323	J06	40	176.24.1002.09547	J07
41	176.24.1002.12543	J08	42	173.24.1518.48427	J09
43	173.24.1518.48208	J10	44	176.24.1002.16027	J11
45	176.24.1002.18280	J12	46	173.24.1518.48444	J13
47	176.24.0951.01433	PB01	48	176.24.0951.02617	PB02
49	176.24.0951.04763	PB03	50	176.24.0951.23930	PB04



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Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	Job Number: 24-1285
Job Description: Hedrick	
Address:	

Item	Drawing Number	Truss
51	176.24.1002.20990	V01
53	176.24.1002.23547	V03
55	176.24.1003.51183	V05
57	176.24.1003.56050	V07
59	176.24.1003.59853	V09
61	176.24.1004.12467	V11
63	176.24.1004.14680	V13
65	176.24.1004.18633	V15
67	BRCLBSUB0119	
69	PB160220723	
71	VALTN220723	

Item	Drawing Number	Truss
52	176.24.1002.22150	V02
54	176.24.1003.49680	V04
56	176.24.1003.54010	V06
58	176.24.1003.57320	V08
60	176.24.1004.09157	V10
62	176.24.1004.13513	V12
64	176.24.1004.16837	V14
66	160TL	
68	DEFLCAMB1014	
70	VAL180220723	
72	CNNAILSP1014	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C-TW = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

General Notes (continued)

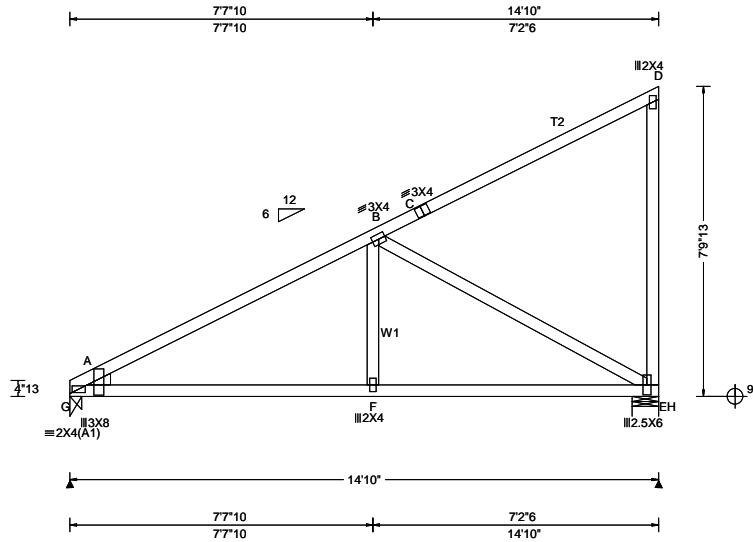
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com

SEQN: 26145 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: A01	Cust: R 215 JRef: 1Y0W2150008 T11 DrwNo: 176.24.0951.30440 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.026 A 999 240 VERT(CL): 0.054 A 999 180 HORZ(LL): 0.014 A - - HORZ(TL): 0.029 A - - Creep Factor: 2.0 Max TC CSI: 0.576 Max BC CSI: 0.706 Max Web CSI: 0.892 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 617 -/- /- /373 /93 /336 H 605 -/- /- /450 /120 /- Wind reactions based on MWFRS G Brg Wid = 3.5 Min Req = 1.5 (Truss) H Brg Wid = 8.0 Min Req = 1.5 (Truss) Bearings G & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. A - B 272 -850

Lumber

Top chord: 2x4 SP M-31; T2 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2; W1 2x4 SP #3;
Lt Wedge: 2x4 SP #3;

Wind

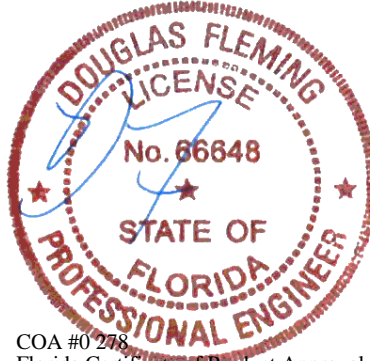
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - F	697 -420	F - E	693 -421

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
B - E	358 -793

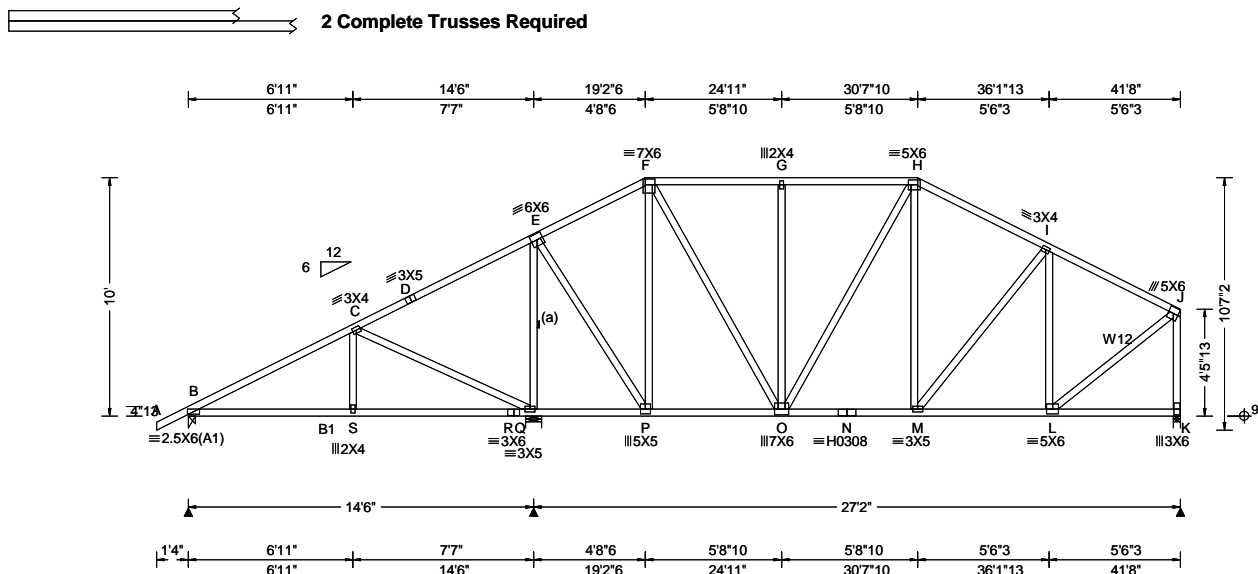


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****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 26464 FROM:	SPEC Ply: 2 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: A02	Cust: R 215 JRef: 1Y0W2150008 T2 DrwNo: 176.24.0951.55437 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.30 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.17 ft ft Loc. from endwall: not in 6.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.074 M 999 240 VERT(CL): 0.146 M 999 180 HORZ(LL): 0.020 L - - HORZ(TL): 0.039 L - - Creep Factor: 2.0 Max TC CSI: 0.383 Max BC CSI: 0.655 Max Web CSI: 0.808 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 620 -/- /- /- /154 /131 Q 6317 -/- /- /- /1023 -/ K 5862 -/- /- /- /966 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) Q Brg Wid = 8.0 Min Req = 2.6 (Truss) K Brg Wid = 3.5 Min Req = 2.4 (Truss) Bearings B, Q, & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;
Webs: 2x4 SP #3; W12 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Nailnote

Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 6.75" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 62 plf at -1.33 to 62 plf at 41.67
BC: From 4 plf at -1.33 to 4 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 13.67
BC: From 10 plf at 13.67 to 10 plf at 41.67
BC: 683 lb Conc. Load at 15.40,17.40,19.40,21.40
23.40,25.40,27.40,29.40,31.40,33.40,35.40,37.40
39.40,41.27

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

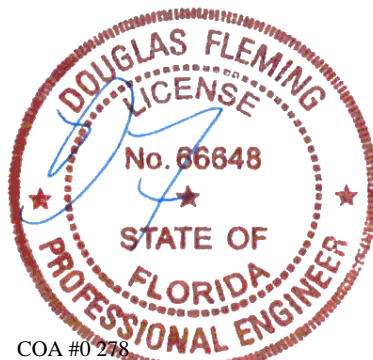
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
P - O	1171 -148	N - M	1949 -274
O - N	1949 -274	M - L	1865 -267

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
Q - E	459 -2606	H - M	869 -113
E - P	2120 -335	L - J	2348 -372
F - P	155 -812	J - K	419 -2439
F - O	1463 -240		



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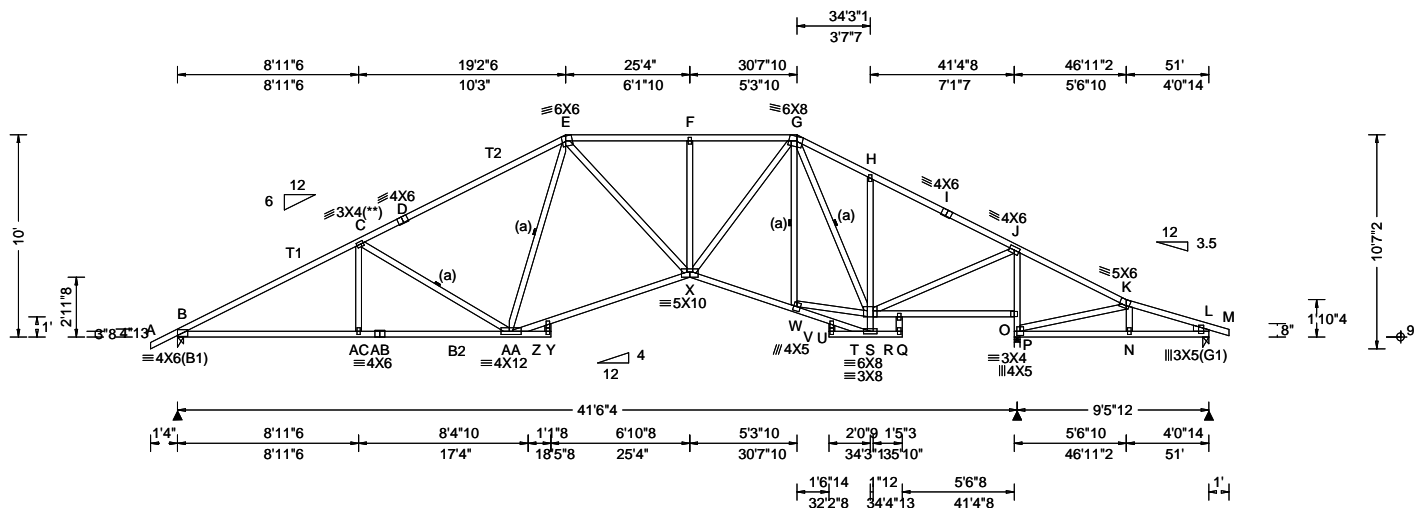
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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34735 FROM:	COMN Qty: 2	Ply: 1 Qty: 2	Job Number: 24-1285 Hedrick Truss Label: A03	Cust: R 215 JRRef: 1Y0W2150008 T13 DrwNo: 176.24.1034.26650 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.30 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.10 ft ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.193 Y 999 240 VERT(CL): 0.390 Y 999 180 HORZ(LL): 0.077 S - - HORZ(TL): 0.158 S - - Creep Factor: 2.0 Max TC CSI: 0.864 Max BC CSI: 0.882 Max Web CSI: 0.820 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1790 -/- /- /1135 -/- /292 O 2269 -/- /- /1295 -/- /- L 364 -/- /- /220 /51 -/- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 2.1 (Truss) O Brg Wid = 3.5 Min Req = 2.7 (Truss) L Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, O, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber Top chord: 2x4 SP #2; T1,T2 2x4 SP M-31; Bot chord: 2x4 SP #2; B2 2x4 SP M-31; Webs: 2x4 SP #3; Rt Stub Wedge: 2x4 SP #3;	Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)	B - C 347 -3086 G - H 322 -1689 C - D 214 -2356 H - I 182 -1632 D - E 255 -2298 I - J 163 -1727 E - F 116 -2450 J - K 386 -3 F - G 116 -2450 K - L 176 -474
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Bracing (a) Continuous lateral restraint equally spaced on member.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - AC 2663 -271 Z - X 2124 0 AC-AB 2660 -272 X - W 1652 0 AB-AA 2660 -272 O - N 410 -130 AA- Z 2070 0 N - L 420 -125
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Plating Notes All plates are 2X4 except as noted. (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - AA 339 -807 S - J 1772 0 E - X 685 0 J - P 196 -2002 X - G 1458 0 P - O 168 -2084 W - G 0 -420 O - K 174 -593 W - S 1546 0
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Purlins In lieu of structural panels use purlins to brace all flat TC @ 24" oc.	Additional Notes WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
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Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.	Professional Engineer Seal DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER COA #0248 Florida Certificate of Product Approval #FL1999 06/24/2024
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WARNING **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information. Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org	ALPINE AN ITW COMPANY 155 Harlem Ave North Building, 4th Floor Glenview, IL 60025
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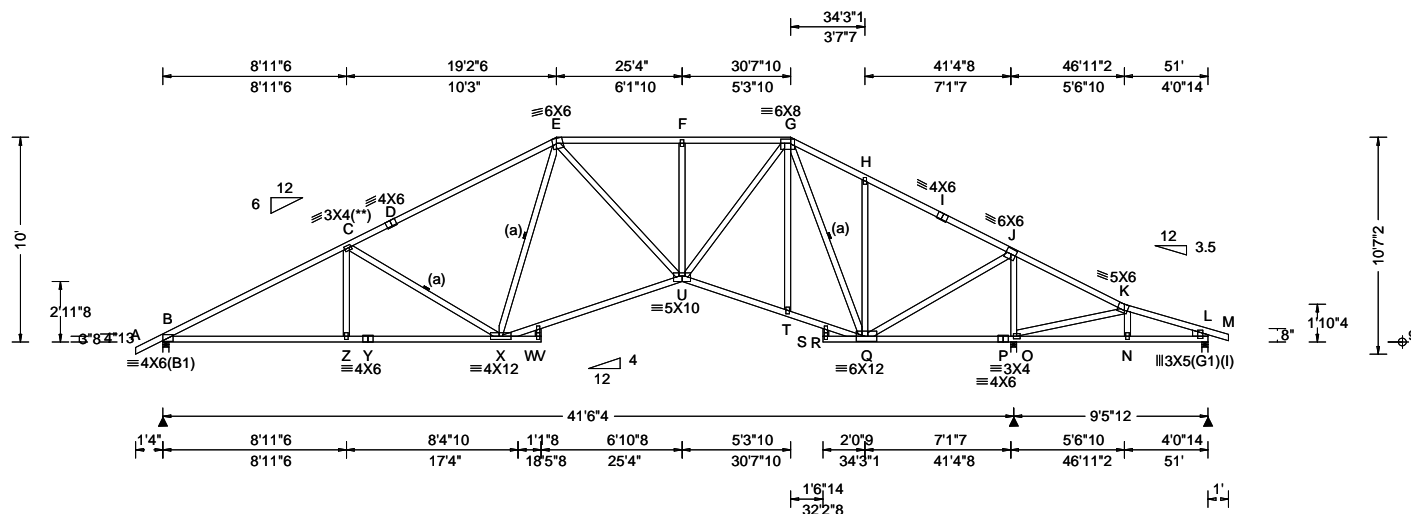
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ALPINE
AN ITW COMPANY

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34739 FROM:	COMN Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: A05	Cust: R 215 JRRef: 1Y0W2150008 T19 DrwNo: 176.24.0952.50517 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.30 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.10 ft ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.164 V 999 240 VERT(CL): 0.337 V 999 180 HORZ(LL): 0.067 Q - - HORZ(TL): 0.138 Q - - Creep Factor: 2.0 Max TC CSI: 0.677 Max BC CSI: 0.383 Max Web CSI: 0.561 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 1808 -/- /- /1096 /328 /292 O 3333 -/- /- /1503 /588 -/ L 200 -/144 -/- /62 /114 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) O Brg Wid = 3.5 Min Req = 2.4 (Truss) L Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, O, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;
Rt Stub Wedge: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 62 plf at -1.33 to 62 plf at 46.93	
TC: From 61 plf at 46.93 to 61 plf at 52.00	
BC: From 4 plf at -1.33 to 4 plf at 0.00	
BC: From 20 plf at 0.00 to 20 plf at 18.46	
BC: From 21 plf at 18.46 to 21 plf at 33.33	
BC: From 20 plf at 33.33 to 20 plf at 51.00	
BC: From 4 plf at 51.00 to 4 plf at 52.00	
TC: 400 lb Conc. Load at 29.48,46.40	

Plating Notes

All plates are 2X4 except as noted.

(l) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

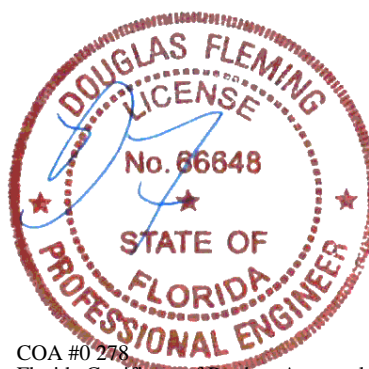
Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - Z	2698 -471	T - R	1661 -283
Z - Y	2696 -472	R - Q	1641 -280
Y - X	2696 -472	Q - P	199 -1144
X - W	2112 -384	P - O	199 -1144
W - U	2168 -389	O - N	58 -573
U - T	1680 -287	N - L	63 -563

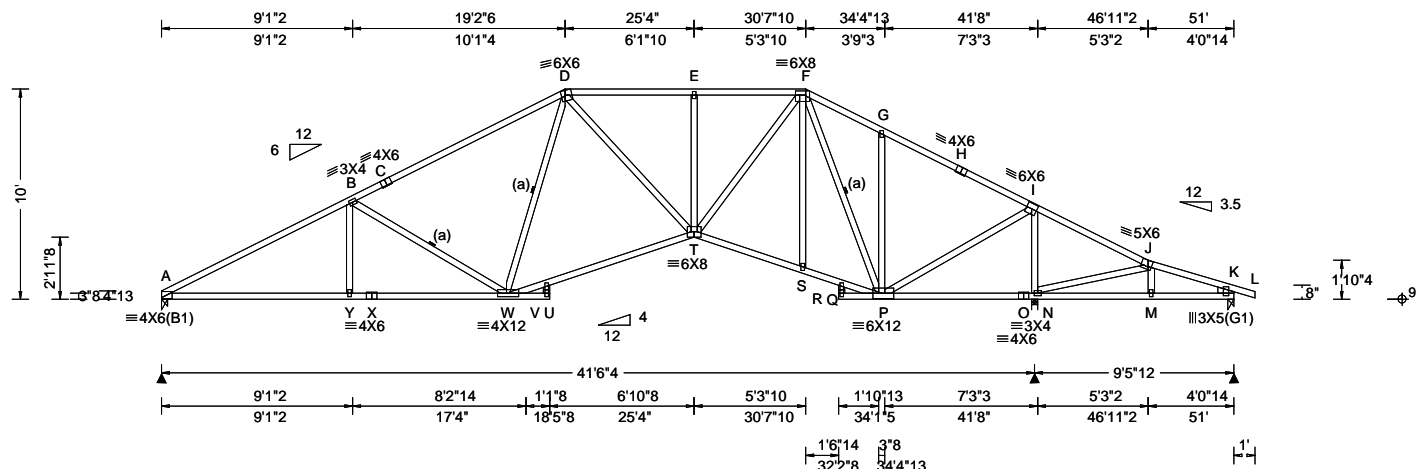
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - X	334 -808	G - Q	262 -1460
E - U	744 -97	Q - J	2515 -436
U - G	1549 -257	J - O	605 -2927
F - U	144 -405	O - K	180 -1024

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ALPINE
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34730 FROM:	SPEC	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: A06	Cust: R 215 JRRef: 1Y0W2150008 T43 DrwNo: 176.24.0953.00080 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.10 ft ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.163 U 999 240 VERT(CL): 0.335 U 999 180 HORZ(LL): 0.066 P - - HORZ(TL): 0.137 P - - Creep Factor: 2.0 Max TC CSI: 0.669 Max BC CSI: 0.382 Max Web CSI: 0.559 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 1712 -/- /- /1013 /298 /268 N 3320 -/- /- /1491 /590 -/ K 203 -/139 -/- /74 /98 -/ Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) N Brg Wid = 3.5 Min Req = 2.4 (Truss) K Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A, N, & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;
Rt Stub Wedge: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 62 plf at 0.00 to 62 plf at 19.20
TC: From 61 plf at 19.20 to 61 plf at 30.64
TC: From 62 plf at 30.64 to 62 plf at 46.93
TC: From 61 plf at 46.93 to 61 plf at 52.00
BC: From 20 plf at 0.00 to 20 plf at 18.46
BC: From 21 plf at 18.46 to 21 plf at 33.33
BC: From 20 plf at 33.33 to 20 plf at 51.00
BC: From 4 plf at 51.00 to 4 plf at 52.00
TC: 400 lb Conc. Load at 29.48,46.41

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

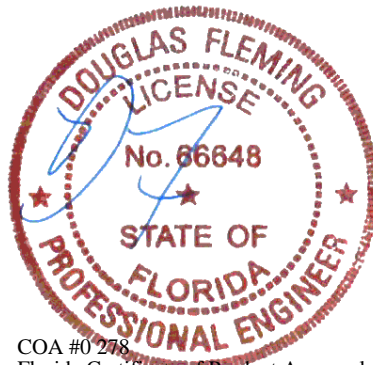
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - Y	2704 -708	S - Q	1655 -289
Y - X	2702 -710	Q - P	1636 -285
X - W	2702 -710	P - O	303 -1152
W - V	2104 -494	O - N	303 -1152
V - T	2160 -500	N - M	258 -560
T - S	1675 -293	M - K	261 -550

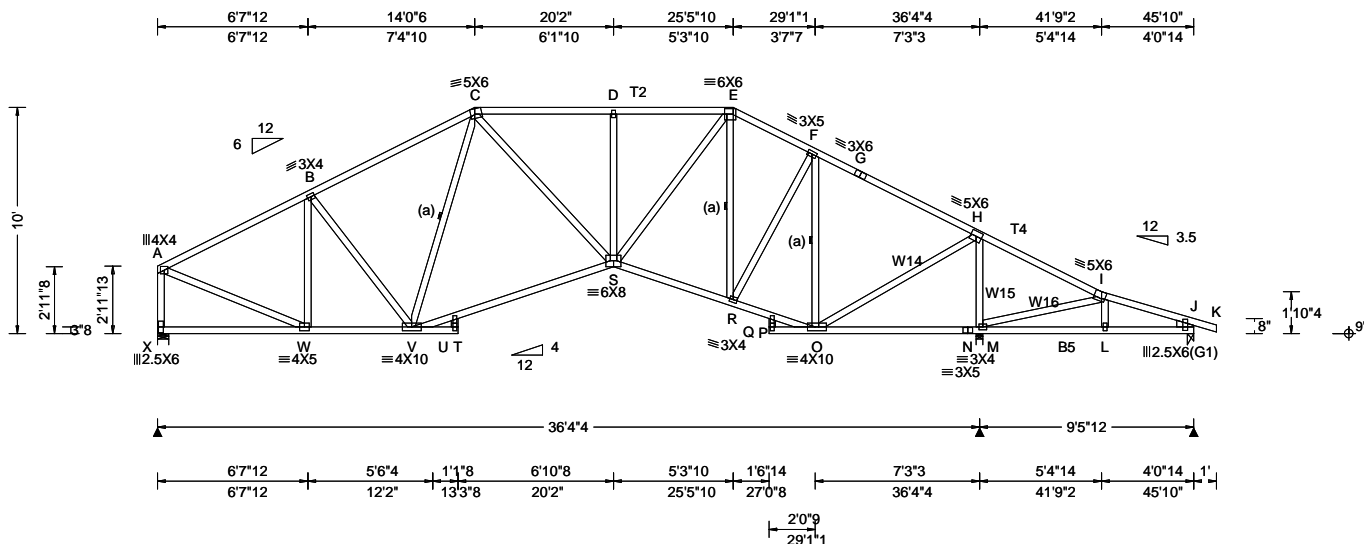
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - W	312 -843	F - P	267 -1558
D - T	831 -121	P - I	2501 -538
T - F	1538 -434	I - N	771 -2913
E - T	267 -451	N - J	178 -1025

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34725 FROM:	SPEC	Ply: 1 Qty: 3	Job Number: 24-1285 Hedrick Truss Label: A07	Cust: R 215 JRef: 1Y0W2150008 T51 DrwNo: 176.24.0953.08510 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.58 ft ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.131 T 999 240 VERT(CL): 0.308 T 999 180 HORZ(LL): 0.053 O - - HORZ(TL): 0.109 O - - Creep Factor: 2.0 Max TC CSI: 0.746 Max BC CSI: 0.740 Max Web CSI: 0.583 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL X 1531 -/- /- /850 /267 /256 M 2923 -/- /- /1327 /517 -/ J 289 -/- /- /108 /53 -/ Wind reactions based on MWFRS X Brg Wid = 6.0 Min Req = 1.8 (Truss) M Brg Wid = 3.5 Min Req = 2.0 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings X, M, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #2; T2,T4 2x4 SP M-31;
Bot chord: 2x4 SP #2; B5 2x4 SP M-31;
Webs: 2x4 SP #3; W14,W15,W16 2x4 SP M-31;
Rt Stub Wedge: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 62 plf at 0.00 to 62 plf at 14.03
TC: From 61 plf at 14.03 to 61 plf at 25.47
TC: From 62 plf at 25.47 to 62 plf at 41.76
TC: From 61 plf at 41.76 to 61 plf at 46.83
BC: From 20 plf at 0.00 to 20 plf at 13.29
BC: From 21 plf at 13.29 to 21 plf at 28.17
BC: From 20 plf at 28.17 to 20 plf at 45.83
BC: From 4 plf at 45.83 to 4 plf at 46.83
TC: 400 lb Conc. Load at 24.31,41.25

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
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See "WARNING" note below.

Maximum Bot Chord Forces Per Ply (lbs)

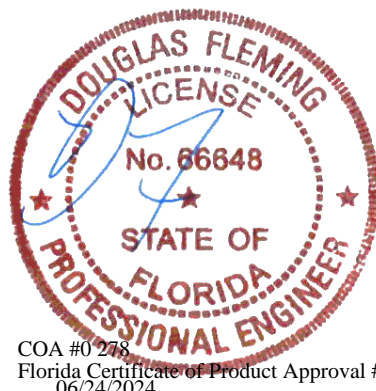
Chords	Tens.Comp.	Chords	Tens. Comp.
W - V	1456 -337	R - P	1097 -187
V - U	1597 -337	P - O	1082 -185
U - S	1651 -340	O - N	214 -831
S - R	1514 -266	N - M	214 -831

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - X	445 -1470	R - E	190 -901
A - W	1530 -405	R - F	925 -152
W - B	209 -480	F - O	325 -1265
V - C	146 -393	O - H	2063 -414
C - S	934 -170	H - M	655 -2504
D - S	295 -475	M - I	184 -1035
S - E	1182 -334		

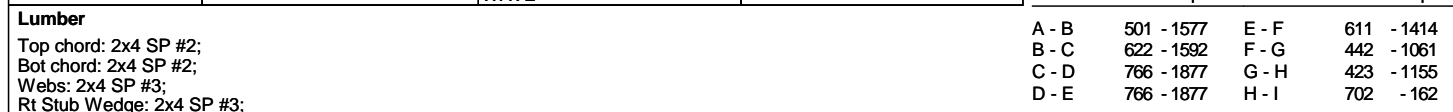
Plating Notes
All plates are 2X4 except as noted.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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Purlins	Maximum Web Forces Per Ply (lbs)
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.	Webs Tens.Comp. Webs Tens. Comp.

Additional Notes

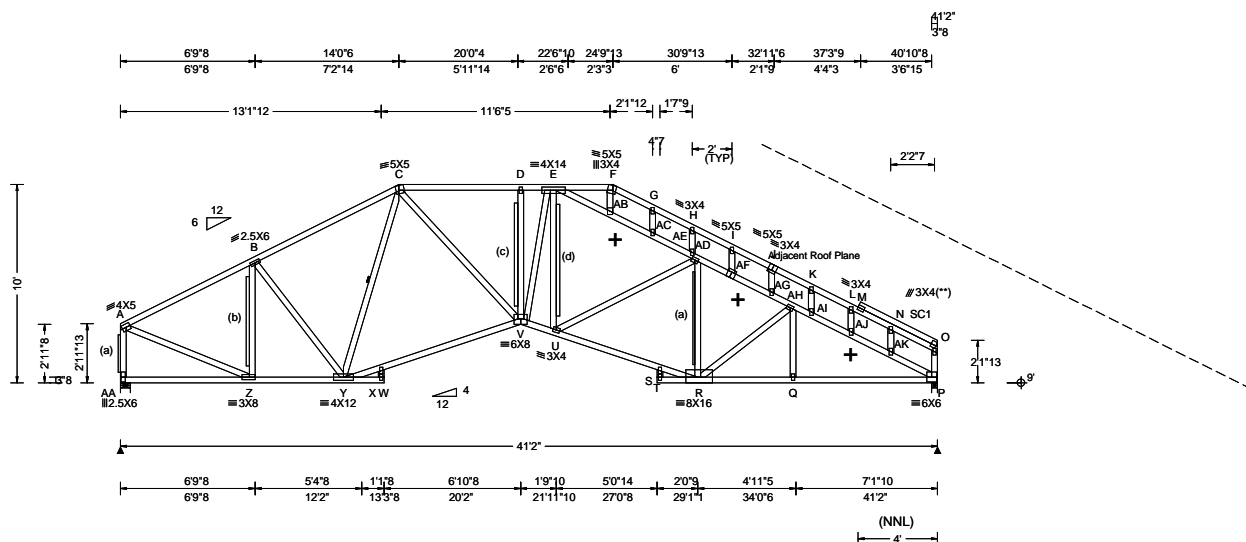
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point)



COA #0278
Florida Certificate of Product Approval #FL1999
06/24/2024

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.12 ft ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.234 F 999 240 VERT(CL): 0.484 F 999 180 HORZ(LL): 0.170 P - - HORZ(TL): 0.351 P - - Creep Factor: 2.0 Max TC CSI: 0.702 Max BC CSI: 0.856 Max Web CSI: 0.889 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL AA 1703 -/- /- /976 -/- /265 P 1703 -/- /- /1002 -/- /- Wind reactions based on MWFRS AA Brg Wid = 6.0 Min Req = 2.0 (Truss) P Brg Wid = 3.5 Min Req = 2.0 (Truss) Bearings AA & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 247 - 1893 C - D 219 - 2635 B - C 306 - 1983 D - E 219 - 2635

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Plating Notes
All plates are 2X4 except as noted.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

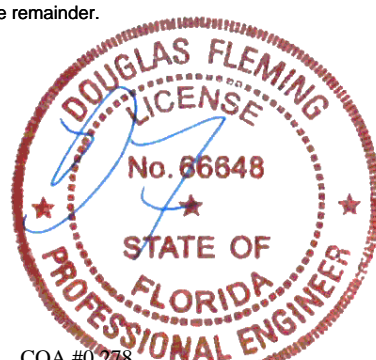
Purlins
In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/566.
Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)
+ Member to be laterally braced for out of plane wind loads

Gable Reinforcement
(a) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(b) 2x6 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(c) 2x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(d) 2x6 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
Z - Y	1646 -22	U - S	2581 -121
Y - X	1887 0	S - R	2530 -121
X - V	1943 0	R - Q	2730 -480
V - U	2572 0	Q - P	2735 -479

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
A-AA	226 -1642	AE-R	5 -424
A-Z	1732 -164	AE-AF	309 -2636
Z-B	116 -558	R-AH	504 -402
Y-C	0 -506	AF-AG	306 -2663
C-V	1215 0	AG-AH	344 -2706
V-E	858 0	AH-AI	526 -2950
E-AB	9 -2705	AI-AJ	583 -2997
AB-AC	0 -2729	AJ-AK	576 -3030
AC-AD	8 -2747	AK-P	563 -3075
AD-AE	41 -2837		



COA #0 278
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06/24/2024

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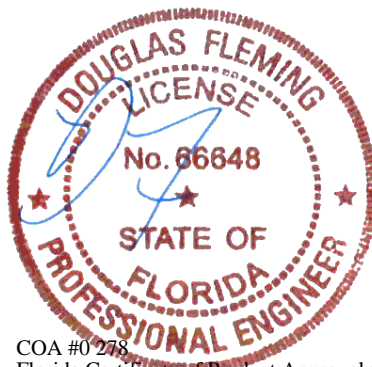
SEQN: 26539	GABL	Ply: 1	Job Number: 24-1285	Cust: R 215 JRef: 1Y0W2150008 T48
FROM:		Qty: 1	Hedrick	DrwNo: 176.24.1034.44910
Page 2 of 2			Truss Label: A09	NW / DF 06/24/2024

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



COA #0278

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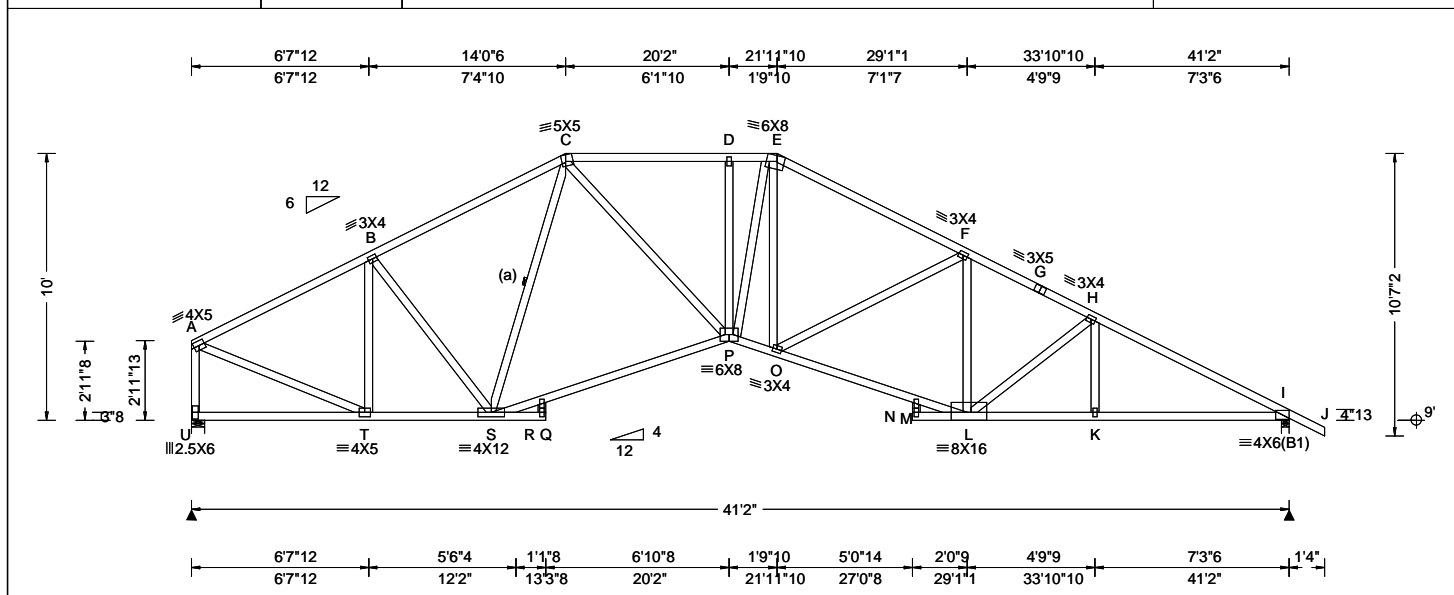
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26245 FROM:	COMN Qty: 1	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: A10	Cust: R 215 JRef: 1Y0W2150008 T39 DrwNo: 176.24.1034.48917 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.12 ft ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.191 Q 999 240 VERT(CL): 0.420 Q 999 180 HORZ(LL): 0.091 I - - HORZ(TL): 0.188 I - - Creep Factor: 2.0 Max TC CSI: 0.774 Max BC CSI: 0.841 Max Web CSI: 0.656 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL U 1695 -/- /- /963 -/- /270 I 1800 -/- /- /1117 -/- /- Non-Gravity Wind reactions based on MWFRS U Brg Wid = 6.0 Min Req = 2.0 (Truss) I Brg Wid = 3.5 Min Req = 2.1 (Truss) Bearings U & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

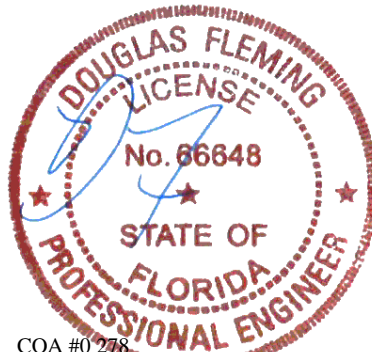
Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point)

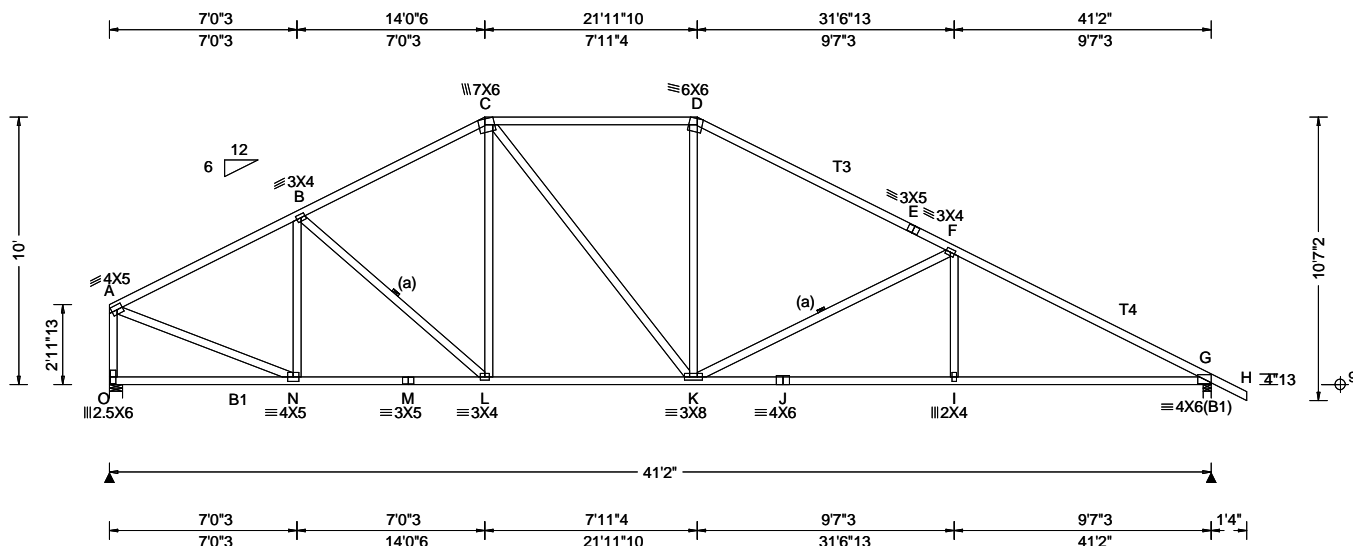


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06/24/2024

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26241 FROM:	COMN Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: A11	Cust: R 215 JRRef: 1Y0W2150008 T40 DrwNo: 176.24.0953.45597 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.12 ft ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.114 K 999 240 VERT(CL): 0.232 K 999 180 HORZ(LL): 0.045 G - - HORZ(TL): 0.093 G - - Creep Factor: 2.0 Max TC CSI: 0.810 Max BC CSI: 0.629 Max Web CSI: 0.657 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL O 1687 - / - / - / 950 / 37 / 270 G 1792 - / - / - / 1106 / 92 / - Wind reactions based on MWFRS O Brg Wid = 6.0 Min Req = 2.0 (Truss) G Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings O & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2; T3, T4 2x4 SP M-31;
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

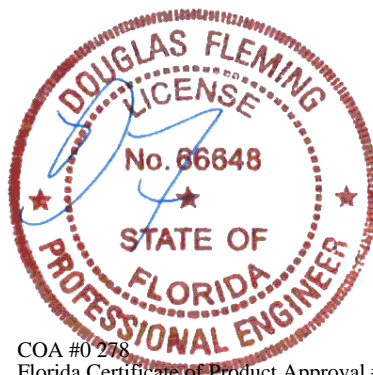
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
N - M	1651 -351	K - J	2623 -615
M - L	1651 -351	J - I	2623 -615
L - K	1638 -320	I - G	2626 -613

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - O	498 -1629	K - D	493 -3
A - N	1723 -467	K - F	291 -904
N - B	248 -475	F - I	398 0

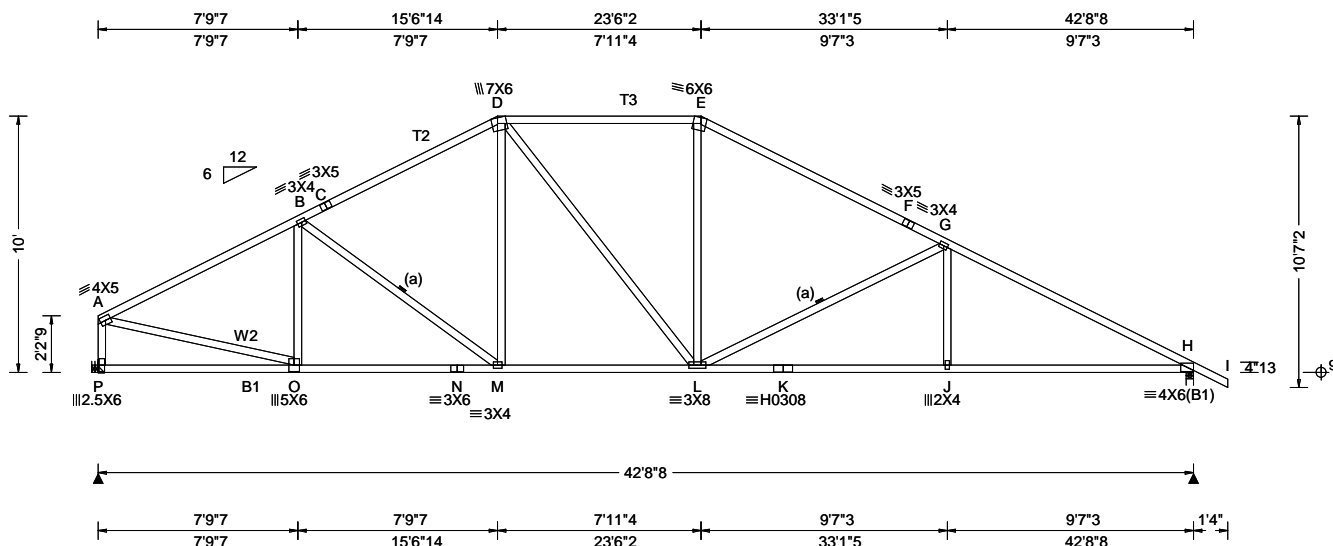


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26236 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: A12	Cust: R 215 JRRef: 1Y0W2150008 T38 DrwNo: 176.24.0953.52110 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.27 ft ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.128 L 999 240 VERT(CL): 0.263 L 999 180 HORZ(LL): 0.051 H - - HORZ(TL): 0.105 H - - Creep Factor: 2.0 Max TC CSI: 0.812 Max BC CSI: 0.780 Max Web CSI: 0.821 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL P 1751 -/- /- /1004 /309 /273 H 1856 -/- /- /1138 /331 -/ Wind reactions based on MWFRS P Brg Wid = - Min Req = - H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 970 -2318 E - F 1081 -2270 B - C 1012 -2148 F - G 1041 -2310 C - D 1045 -2120 G - H 1291 -3195 D - E 1076 -1948

Lumber

Top chord: 2x4 SP M-31; T2,T3 2x4 SP #2;
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;
Webs: 2x4 SP #3; W2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

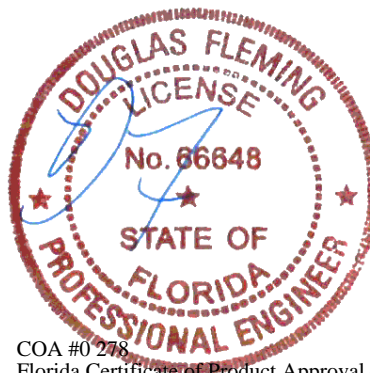
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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SEQN: 26541	SPEC	Ply: 1	Job Number: 24-1285	Cust: R 215 JRef: 1Y0W2150008 T59
FROM:		Qty: 5	Hedrick	DrwNo: 176.24.0953.58257
Page 2 of 2			Truss Label: A13	NW / DF 06/24/2024

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

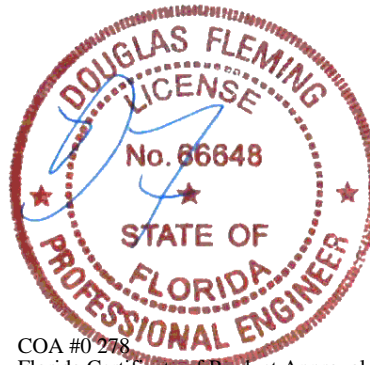
(J) Hanger Support Required, by others

Bearing N (0', 9') HUS26

Supporting Member: (2)2x6 SP 2400f-2.0E

(14) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member.



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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26543	GABL	Ply: 2	Job Number: 24-1285	Cust: R 215 JRef: 1Y0W2150008 T27
FROM:		Qty: 1	Hedrick	DrwNo: 176.24.1034.10847
Page 2 of 2			Truss Label: A14	NW / DF 06/24/2024

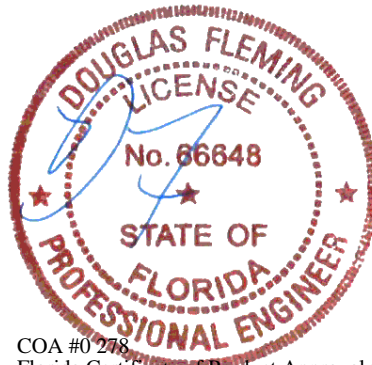
Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Gable Reinforcement

- (a) 2x3 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (b) 2x6 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (c) 2x4 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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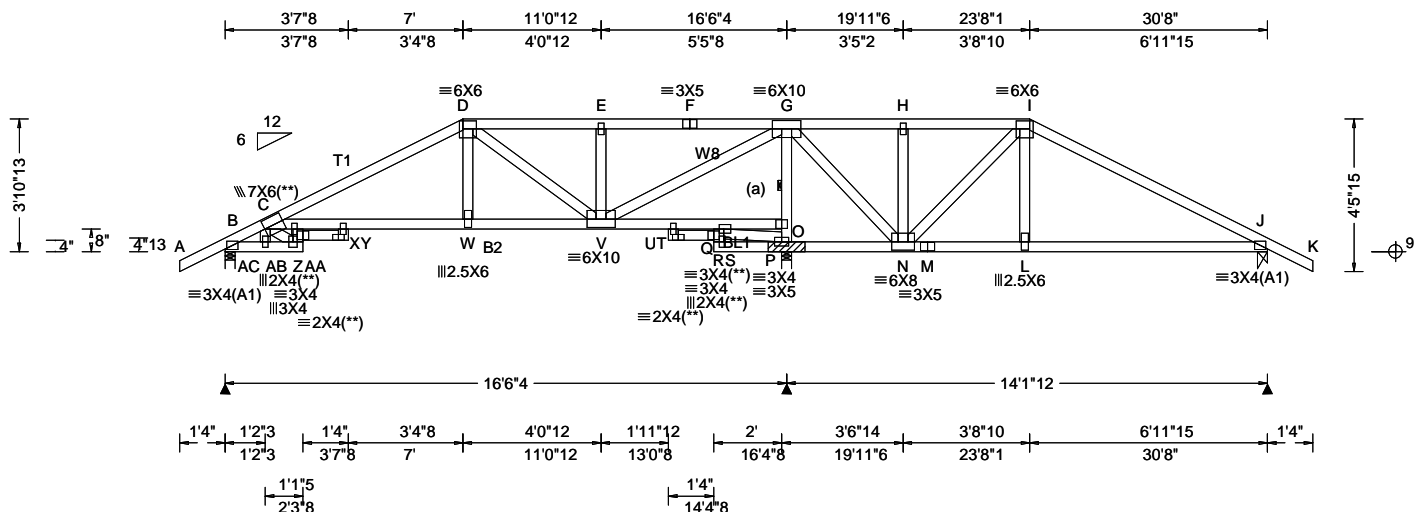
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.07 ft ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.199 Y 979 240 VERT(CL): 0.400 Y 487 180 HORZ(LL): 0.058 V - - HORZ(TL): 0.118 V - - Creep Factor: 2.0 Max TC CSI: 0.838 Max BC CSI: 0.591 Max Web CSI: 0.839 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 1116 -/- /- /- /259 -/ P 4262 -/- /- /- /926 -/ J 909 -/- /- /- /195 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) P Brg Wid = 3.5 Min Req = - J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, P, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber	Purlins
Top chord: 2x4 SP #2; T1 2x4 SP M-31; Bot chord: 2x4 SP #2; B2 2x4 SP M-31; Webs: 2x4 SP #3; W8 2x4 SP #2;	In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

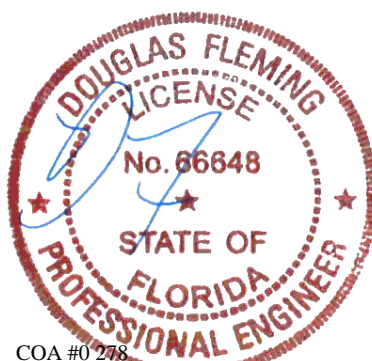
Bracing	Wind
(a) Continuous lateral restraint equally spaced on member.	Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

Special Loads	Bearing Block(s)
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 62 plf at -1.33 to 62 plf at 7.00 TC: From 31 plf at 7.00 to 31 plf at 23.67 TC: From 62 plf at 23.67 to 62 plf at 32.00 BC: From 4 plf at -1.33 to 4 plf at 0.00 BC: From 20 plf at 0.00 to 20 plf at 7.03 BC: From 10 plf at 7.03 to 10 plf at 23.64 BC: From 20 plf at 23.64 to 20 plf at 30.67 BC: From 4 plf at 30.67 to 4 plf at 32.00 TC: 94 lb Conc. Load at 9.06,11.06,13.06 TC: 190 lb Conc. Load at 15.06,15.60,17.61,19.61 21.61 BC: 897 lb Conc. Load at 7.03,23.64 BC: 181 lb Conc. Load at 9.06,11.06,13.06 BC: 130 lb Conc. Load at 15.06,15.60,17.61,19.61 21.61	Brg blocks:0.131"x3", min. nails brg x-loc #blocks length/blk #nails/blk wall plate 2 16.375' 1 13" 8 Rigid Surface Brg block to be same size and species as chord. Refer to drawing C9NAILSP1014 for more information.

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
B -AC 959 -261 V - T 294 -1670 C - Z 1486 -347 T - Q 243 -1480 AC-AB 940 -253 P - N 247 -1500 Z - X 1519 -364 N - M 1080 -219 X - W 1813 -425 M - L 1080 -219 W - V 1880 -436 L - J 1036 -214

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
C -AB 220 -761 Q - P 221 -1442 AB-AA 600 -164 P - O 901 -3440 Z -AA 494 -131 G - O 947 -3611 D - W 1338 -219 G - N 2122 -412 D - V 216 -1358 N - I 254 -1419 E - V 193 -395 L - I 1072 -131 V - G 2783 -630

Plating Notes
All plates are 2X4 except as noted.
(**) 6 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.



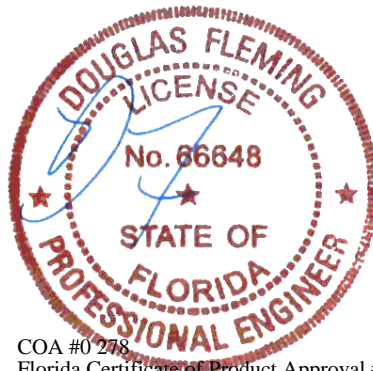
COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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SEQN: 34745	HIPS	Ply: 1	Job Number: 24-1285	Cust: R 215 JRef: 1Y0W2150008 T3
FROM:		Qty: 1	Hedrick	DrwNo: 176.24.1000.07233
Page 2 of 2			Truss Label: B01	NW / DF 06/24/2024

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

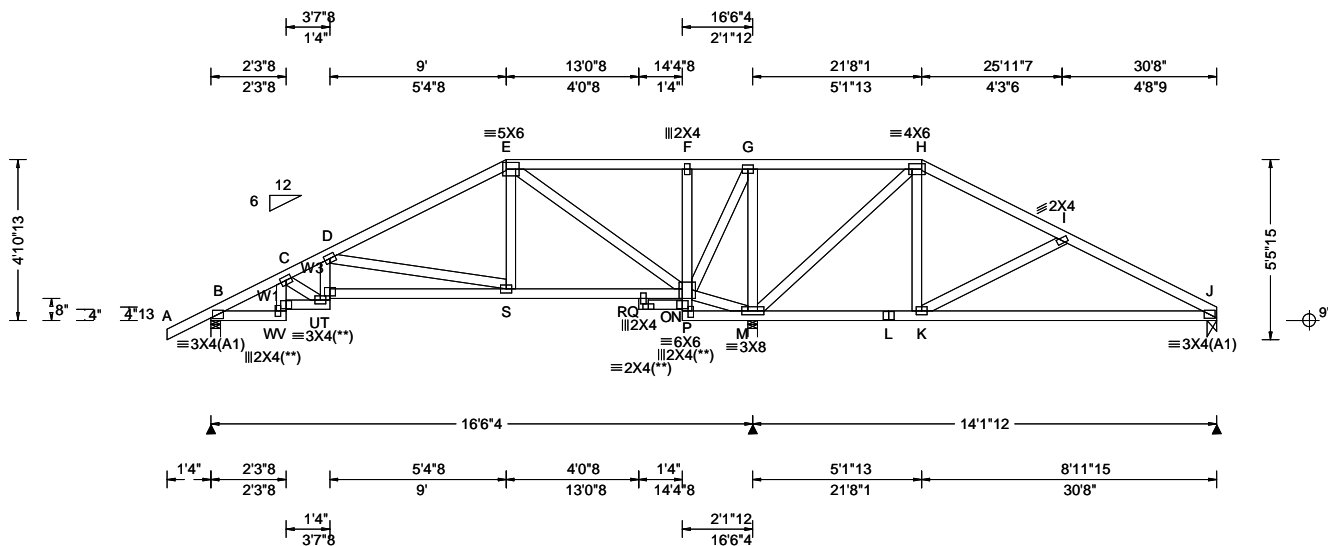


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SEQN: 34754 FROM:	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: B02	Cust: R 215 JRef: 1Y0W2150008 T26 DrwNo: 176.24.1000.12130 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.07 ft ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.043 D 999 240 VERT(CL): 0.093 D 999 180 HORZ(LL): 0.023 J - - HORZ(TL): 0.049 P - - Creep Factor: 2.0 Max TC CSI: 0.522 Max BC CSI: 0.555 Max Web CSI: 0.809 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 576 /- /- /365 /95 /137 M 1741 /- /- /921 /338 /- J 432 /- /- /283 /66 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) M Brg Wid = 3.5 Min Req = 1.7 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, M, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1,W3 2x4 SP M-31;

Plating Notes

All plates are 3X4 except as noted.

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

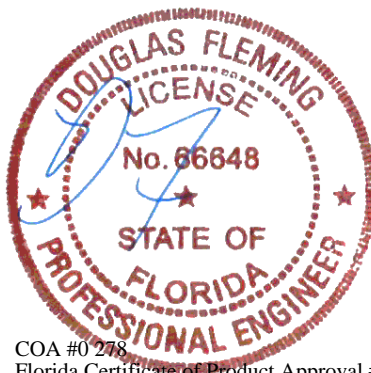
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

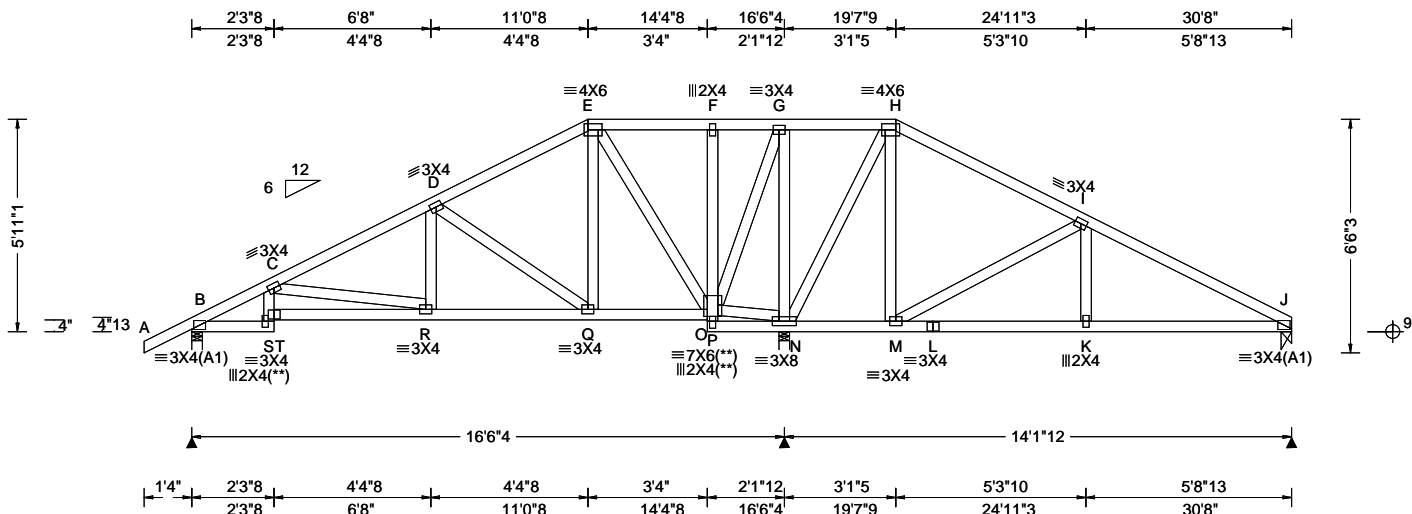


COA #0278
Florida Certificate of Product Approval #FL1999
06/24/2024

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Glenview, IL 60025

SEQN: 34756 FROM:	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: B03	Cust: R 215 JRRef: 1Y0W2150008 T20 DrwNo: 176.24.1000.14647 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.07 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.028 C 999 240 VERT(CL): 0.061 C 999 180 HORZ(LL): 0.015 J - - HORZ(TL): 0.031 N - - Creep Factor: 2.0 Max TC CSI: 0.446 Max BC CSI: 0.312 Max Web CSI: 0.759 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL B 571 - / - / - /357 /101 /164 N 1764 - / - / - /963 /315 - / - J 411 - / - / - /274 /80 - / - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) N Brg Wid = 3.5 Min Req = 1.7 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, N, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

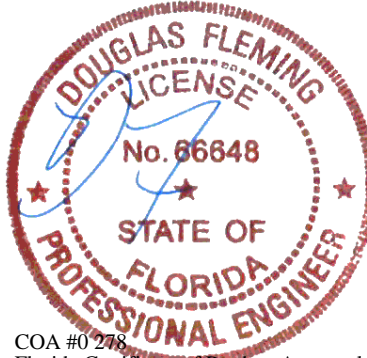
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - T	613 -150	M - L	399 -105
S - R	832 -204	L - K	399 -105
R - Q	444 -90	K - J	403 -104

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - R	125 -383	O - N	284 -680
D - Q	169 -492	G - N	402 -803
E - Q	387 -59	N - H	235 -804
E - O	239 -744	M - I	211 -579
O - G	661 -244		

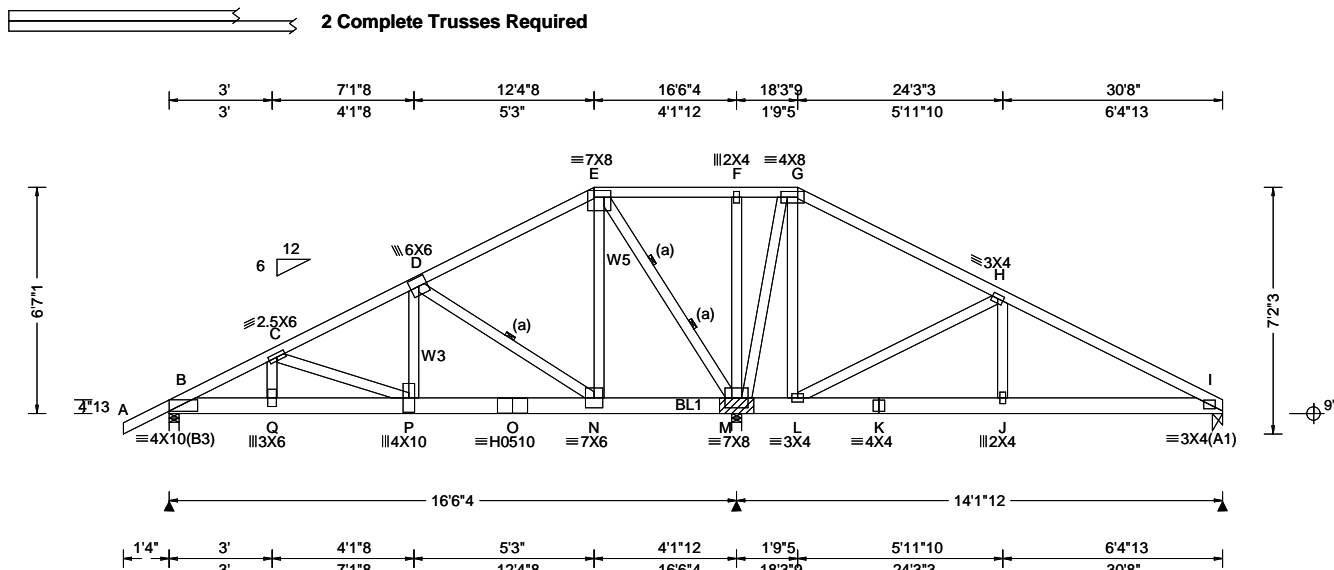


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Glenview, IL 60025

SEQN: 26505 FROM:	HIPS Qty: 1	Ply: 2	Job Number: 24-1285 Hedrick Truss Label: B04	Cust: R 215 JRRef: 1Y0W2150008 T41 DrwNo: 176.24.1000.53177 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.07 ft ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.094 P 999 240 VERT(CL): 0.188 P 999 180 HORZ(LL): 0.022 C - - HORZ(TL): 0.045 C - - Creep Factor: 2.0 Max TC CSI: 0.583 Max BC CSI: 0.525 Max Web CSI: 0.892 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 4715 -/- /- /- /912 -/ M 9687 -/- /- /- /1752 -/ I 438 -/-712 -/- /138 -/- /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 2.0 (Truss) M Brg Wid = 3.5 Min Req = - I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, M, & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber	Bearing Block(s)
Top chord: 2x4 SP #2; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3; W3,W5 2x4 SP #2;	Brg blocks:0.131"x3", min. nails brg x-loc #blocks length/blk #nails/blk wall plate 2 16.375' 1 12" 4 Rigid Surface Brg block to be same size and species as chord. Refer to drawing CNNAILSP1014 for more information.

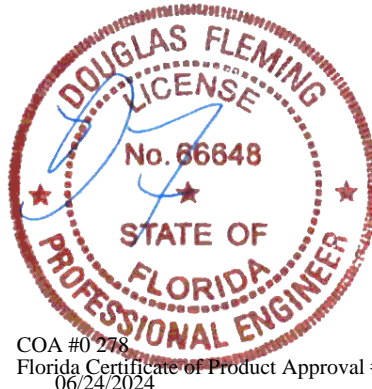
Bracing	Additional Notes
(a) Continuous lateral restraint equally spaced on member.	Negative reaction(s) of -712# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

Nailnote
Nail Schedule:0.131"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord: 1 Row @ 4.25" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 62 plf at -1.33 to 62 plf at 30.67 BC: From 4 plf at -1.33 to 4 plf at 0.00 BC: From 20 plf at 0.00 to 20 plf at 30.67 BC: 1775 lb Conc. Load at 3.12 BC: 1510 lb Conc. Load at 5.06, 7.06, 9.06,11.06 13.06 BC: 1751 lb Conc. Load at 15.06

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

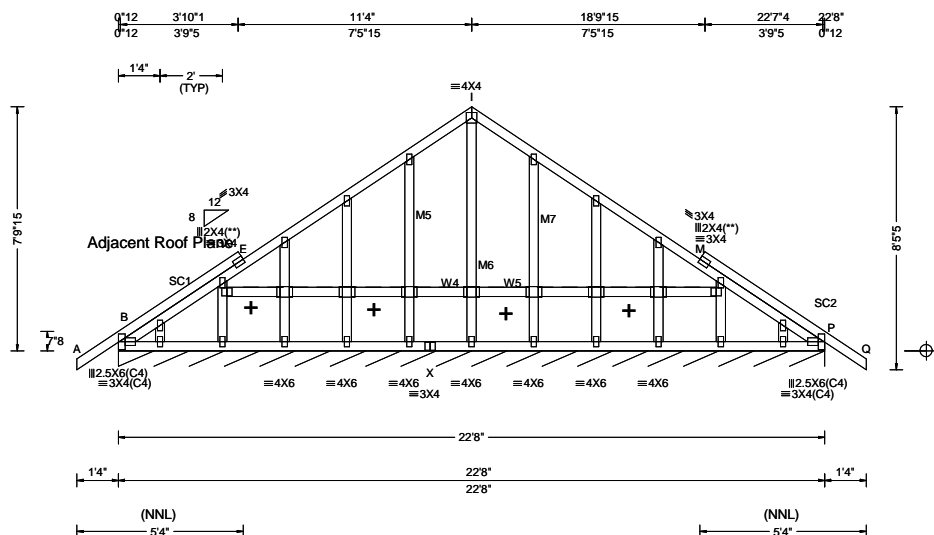
Wind
Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.



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SEQN: 32922 / FROM:	GABL Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: C01	Cust: R 215 JRef: 1Y0W2150008 T9 / DrwNo: 173.24.1518.48422 NW / FV 06/21/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 E 999 240 VERT(CL): 0.005 E 999 180 HORZ(LL): 0.003 M - - HORZ(TL): 0.004 M - - Creep Factor: 2.0 Max TC CSI: 0.238 Max BC CSI: 0.057 Max Web CSI: 0.526 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL P* 92 - / - / 49 / 15 / 11 Wind reactions based on MWFRS P Brg Wid = 272 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W4,W5,M5,M6,M7 2x4 SP M-31;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

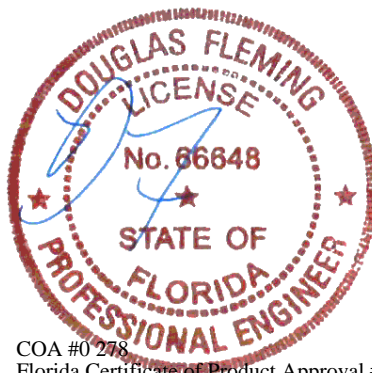
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

+ Member to be laterally braced for out of plane wind loads

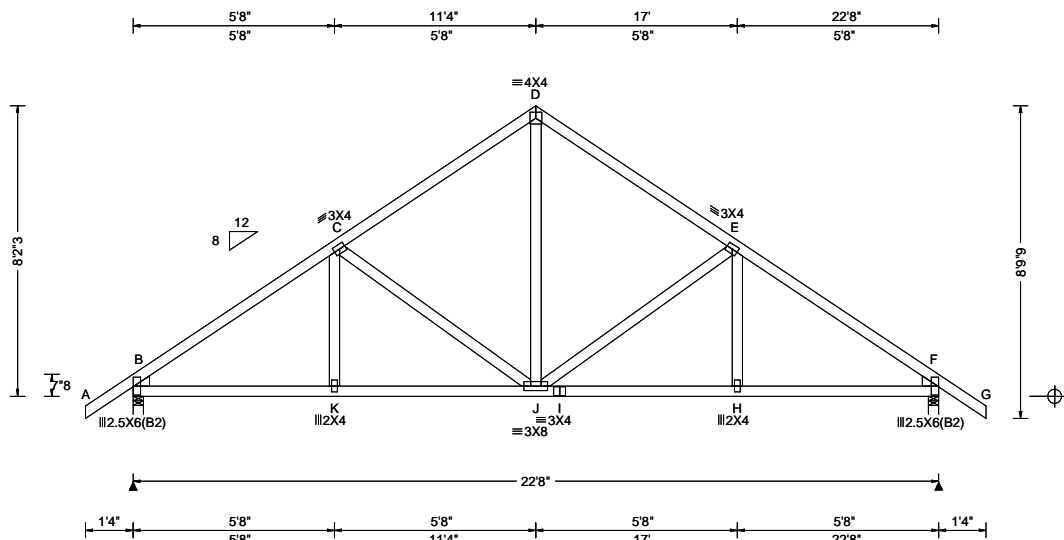


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06/24/2024

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Glenview, IL 60025

SEQN: 34758 FROM:	COMN Ply: 1 Qty: 7	Job Number: 24-1285 Hedrick Truss Label: C02	Cust: R 215 JRRef: 1Y0W2150008 T10 DrwNo: 176.24.1000.54683 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.041 J 999 240 VERT(CL): 0.084 J 999 180 HORZ(LL): 0.022 F - - HORZ(TL): 0.045 F - - Creep Factor: 2.0 Max TC CSI: 0.359 Max BC CSI: 0.401 Max Web CSI: 0.352 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1044 - / - / - / 642 / 176 / 262 F 1044 - / - / - / 642 / 176 / - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 253 - 1319 D - E 266 - 949 C - D 265 - 949 E - F 252 - 1318

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Wind

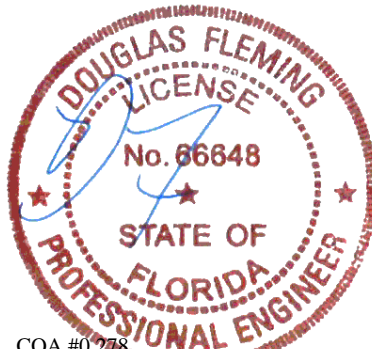
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - K	1014 -94	I - H	1013 -91
K - J	1013 -95	H - F	1014 -90
J - I	1013 -91		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - J	167 -379	J - E	166 -379
D - J	583 -137		

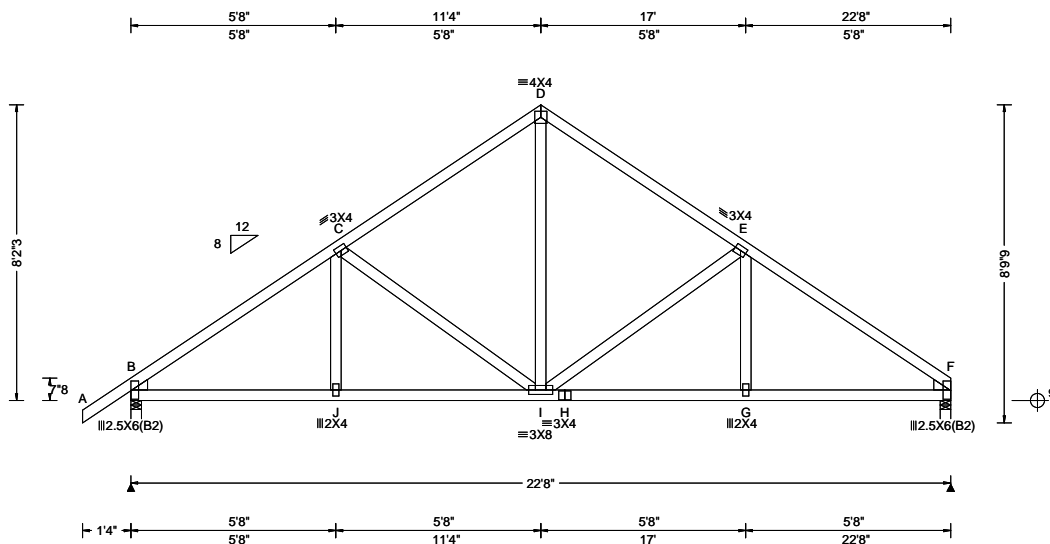


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Glenview, IL 60025

SEQN: 101321 FROM:	SPEC Ply: 1 Qty: 3	Job Number: 24-1285 Hedrick Truss Label: C03	Cust: R 215 JRRef: 1Y0W2150008 T18 DrwNo: 176.24.1000.56980 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.046 I 999 240 VERT(CL): 0.095 I 999 180 HORZ(LL): 0.025 F - - HORZ(TL): 0.051 F - - Creep Factor: 2.0 Max TC CSI: 0.459 Max BC CSI: 0.480 Max Web CSI: 0.363 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1047 - / - / - /642 /177 /243 F 950 - / - / - /557 /149 - Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 250 -1332 D - E 274 -954 C - D 267 -953 E - F 264 -1341

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3; Rt Wedge: 2x4 SP #3;

Wind

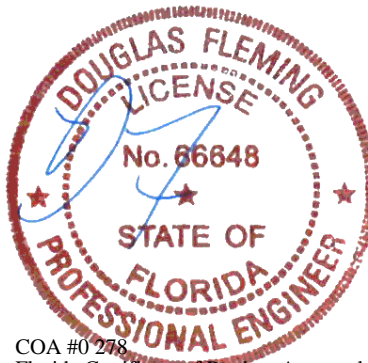
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	1017 -128	H - G	1027 -133
J - I	1016 -128	G - F	1028 -132
I - H	1027 -133		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - I	165 -378	I - E	172 -392
D - I	589 -145		



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[illegible]

Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31; W1,W2,W4 2x4 SP #3;
W3 2x6 SP 2400f-2.0E;
Stack Chord: SC1 2x4 SP M-31;
Stack Chord: SC2 2x4 SP M-31;

All plates are 2X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

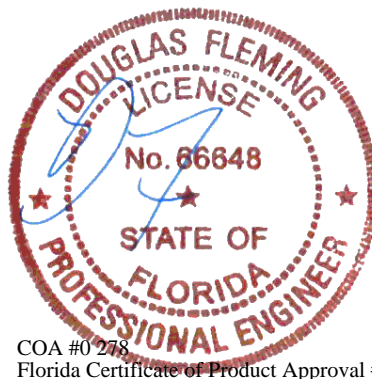
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/341.

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Bottom chord to be laterally braced for out of plane wind loads

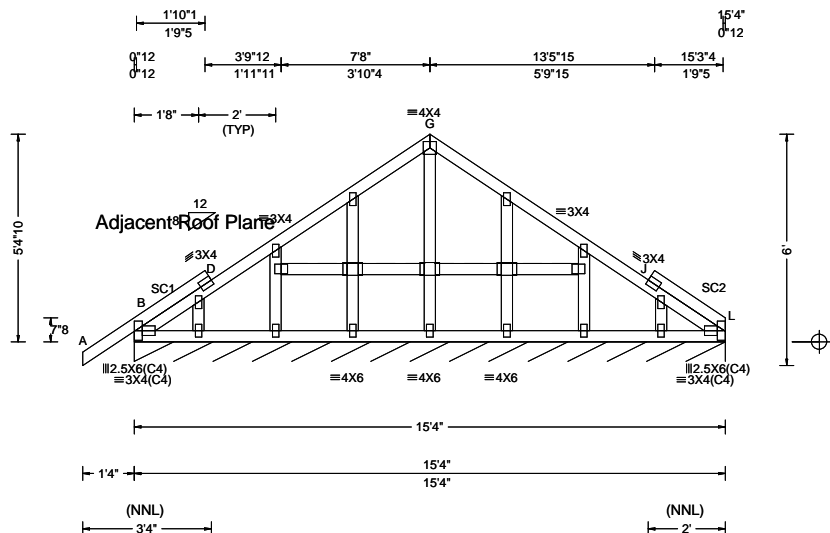


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SEQN: 32948 / FROM:	GABL Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: D01	Cust: R 215 JRef: 1Y0W2150008 T33 / DrwNo: 173.24.1518.48302 NW / FV 06/21/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 J 999 240 VERT(CL): 0.002 J 999 180 HORZ(LL): -0.002 D - - HORZ(TL): 0.002 D - - Creep Factor: 2.0 Max TC CSI: 0.157 Max BC CSI: 0.054 Max Web CSI: 0.487 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity L* 90 /- /- /50 /14 /11 Wind reactions based on MWFRS L Brg Wid = 184 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

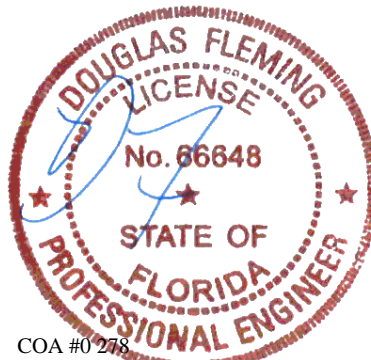
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

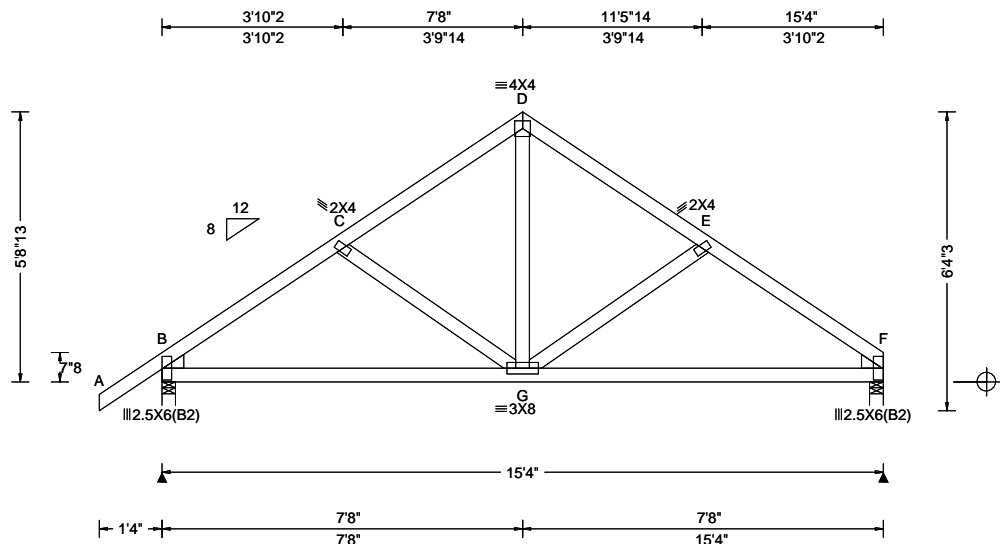


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SEQN: 26120 FROM:	COMN Ply: 1 Qty: 3	Job Number: 24-1285 Hedrick Truss Label: D02	Cust: R 215 JRef: 1Y0W2150008 T44 DrwNo: 176.24.1001.00080 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.021 G 999 240 VERT(CL): 0.043 G 999 180 HORZ(LL): 0.010 F - - HORZ(TL): 0.022 F - - Creep Factor: 2.0 Max TC CSI: 0.281 Max BC CSI: 0.541 Max Web CSI: 0.162 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 740 -/- /- /462 /128 /175 F 640 -/- /- /376 /99 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 382 -843 D - E 331 -645 C - D 328 -643 E - F 392 -850

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Wind

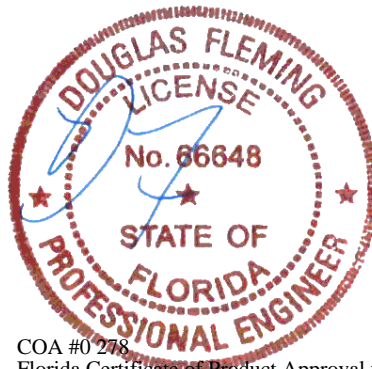
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - G	634 -226	G - F	647 -243

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
D - G	426 -181

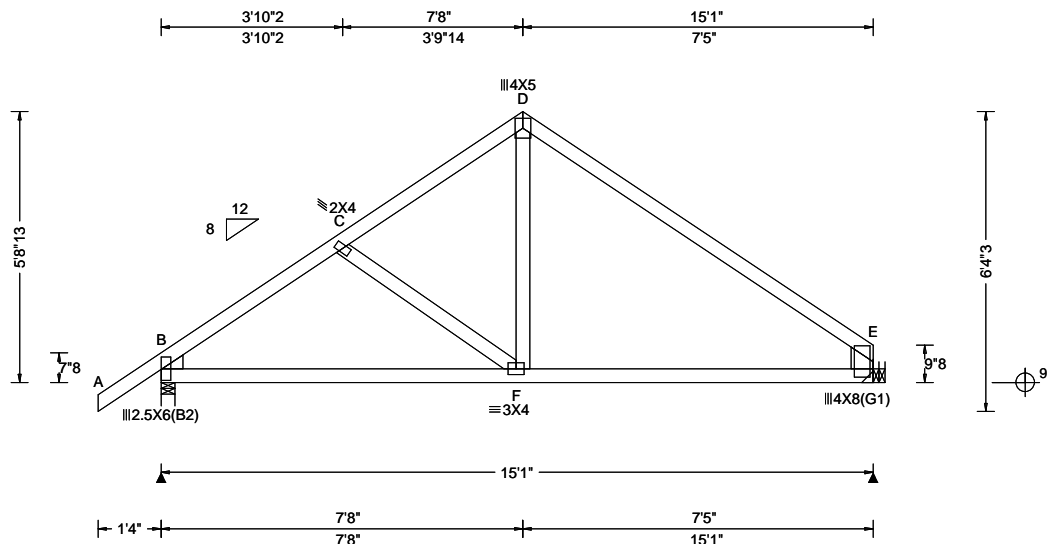


COA #0278
Florida Certificate of Product Approval #FL1999
06/24/2024

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26430 FROM:	COMN Ply: 1 Qty: 14	Job Number: 24-1285 Hedrick Truss Label: D03	Cust: R 215 JRRef: 1Y0W2150008 T47 DrwNo: 176.24.1001.01473 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.016 C 999 240 VERT(CL): 0.030 C 999 180 HORZ(LL): 0.011 E - - HORZ(TL): 0.019 E - - Creep Factor: 2.0 Max TC CSI: 0.635 Max BC CSI: 0.528 Max Web CSI: 0.139 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 766 /- /- /457 /126 /173 E 683 /- /- /367 /98 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 221 -889 D - E 192 -794 C - D 210 -712

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Stub Wedge: 2x6 SP 2400f-2.0E;
Lt Wedge: 2x4 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

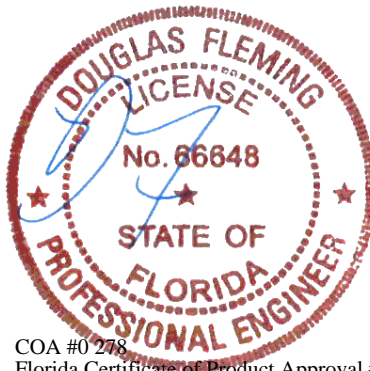
Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

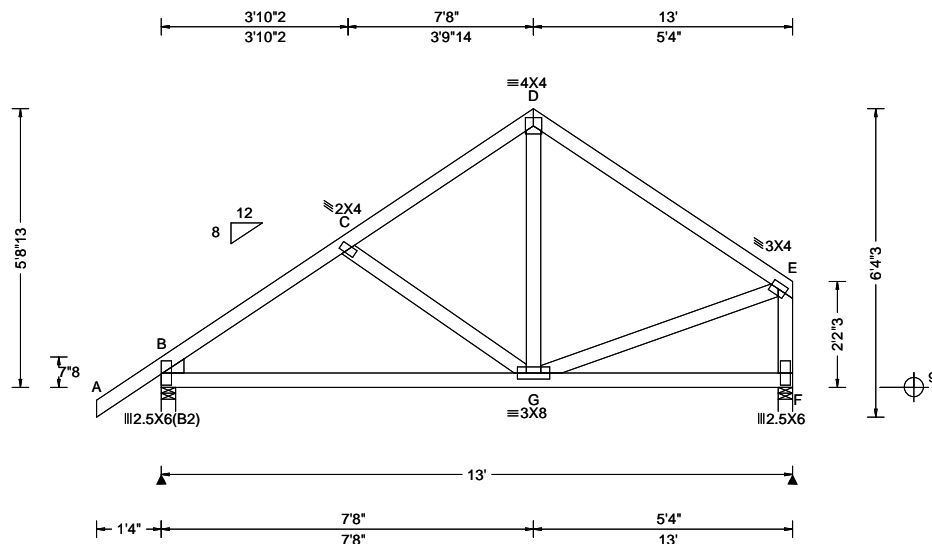


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06/24/2024

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26432 FROM:	COMN Ply: 1 Qty: 2	Job Number: 24-1285 Hedrick Truss Label: D04	Cust: R 215 JRef: 1Y0W2150008 T7 DrwNo: 176.24.1001.02673 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.011 C 999 240 VERT(CL): 0.023 C 999 180 HORZ(LL): 0.003 F - - HORZ(TL): 0.007 F - - Creep Factor: 2.0 Max TC CSI: 0.388 Max BC CSI: 0.471 Max Web CSI: 0.149 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL B 643 -/- /- /412 /12 /206 F 542 -/- /- /295 /1 /- Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 229 -677 D - E 180 -507 C - D 200 -480

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;

Wind

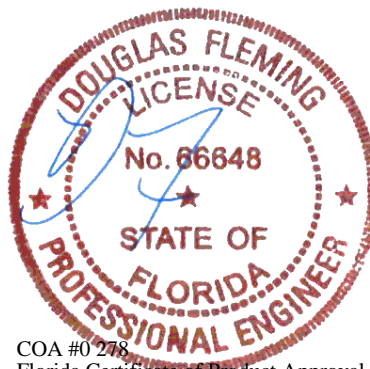
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.
B - G	499 -238

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
E - F	195 -502

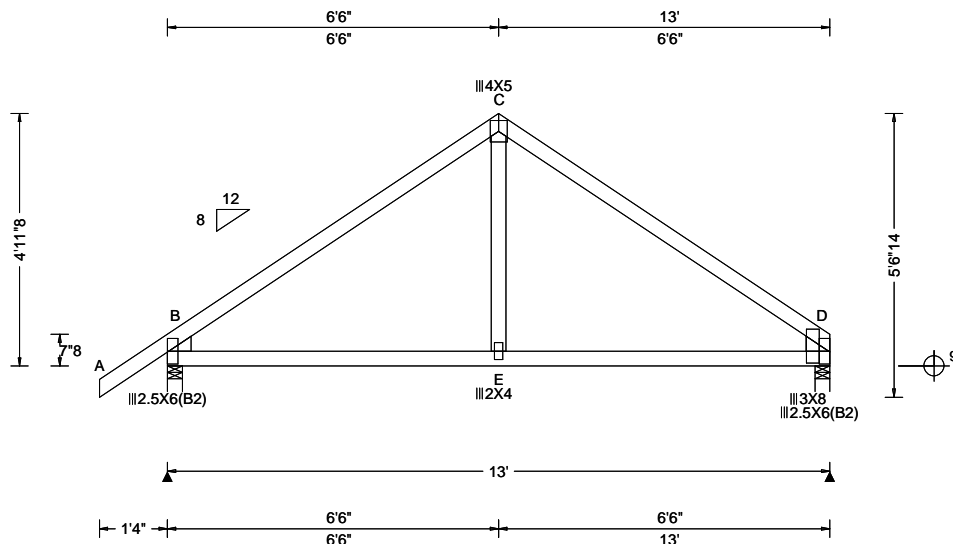


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06/24/2024

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26428 FROM:	COMN Ply: 1 Qty: 4	Job Number: 24-1285 Hedrick Truss Label: D05	Cust: R 215 JRef: 1Y0W2150008 T57 DrwNo: 176.24.1001.04190 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 D 999 240 VERT(CL): 0.019 D 999 180 HORZ(LL): -0.009 D - - HORZ(TL): 0.015 D - - Creep Factor: 2.0 Max TC CSI: 0.436 Max BC CSI: 0.398 Max Web CSI: 0.109 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 643 -/- /- /405 /112 /153 D 542 -/- /- /318 /83 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 205 -639 C - D 213 -637

Lumber

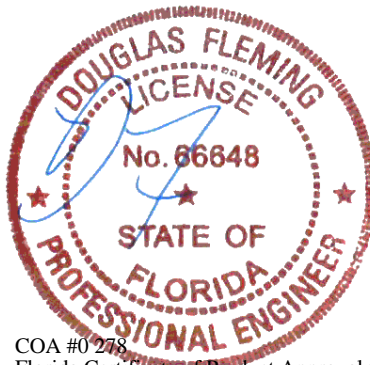
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - E	447 -58	E - D	447 -58

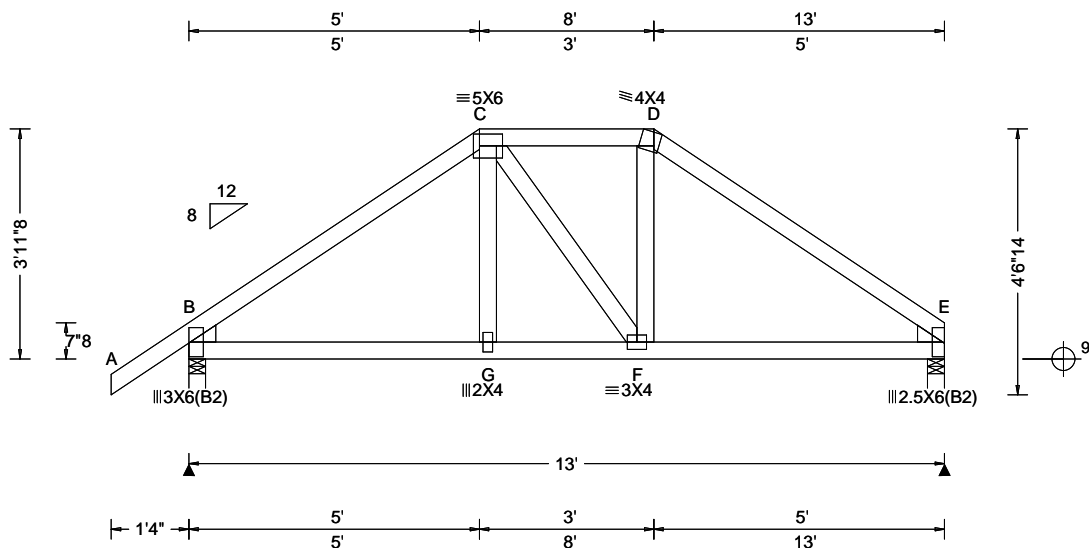


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06/24/2024

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26466 FROM:	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: D06	Cust: R 215 JRef: 1Y0W2150008 T30 DrwNo: 176.24.1001.35000 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.040 G 999 240 VERT(CL): 0.082 G 999 180 HORZ(LL): 0.025 E - - HORZ(TL): 0.050 E - - Creep Factor: 2.0 Max TC CSI: 0.607 Max BC CSI: 0.802 Max Web CSI: 0.093 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1239 -/- /- /- /269 -/ E 1137 -/- /- /- /235 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 369 -1640 D - E 372 -1644 C - D 272 -1297

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Special Loads

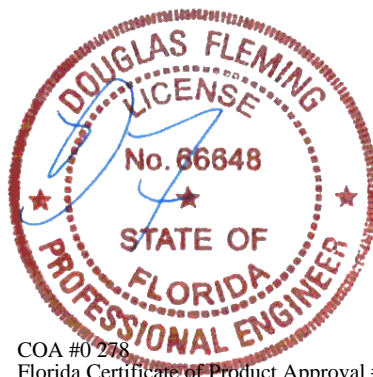
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 64 plf at -1.33 to 64 plf at 5.00
TC: From 32 plf at 5.00 to 32 plf at 8.00
TC: From 64 plf at 8.00 to 64 plf at 13.00
BC: From 5 plf at -1.33 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 5.03
BC: From 10 plf at 5.03 to 10 plf at 7.97
BC: From 20 plf at 7.97 to 20 plf at 13.00
TC: 362 lb Conc. Load at 5.03, 7.97
TC: 138 lb Conc. Load at 6.50
BC: 181 lb Conc. Load at 5.03, 7.97
BC: 94 lb Conc. Load at 6.50

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

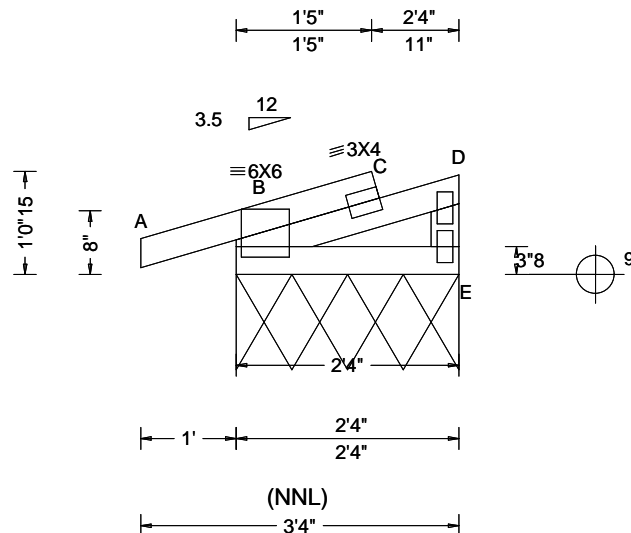


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 33367 / FROM:	GABL Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: E01	Cust: R 215 JRef: 1Y0W2150008 T61 / DrwNo: 173.24.1518.48224 NW / FV 06/21/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 C 999 180 HORZ(LL): 0.000 C - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.109 Max BC CSI: 0.040 Max Web CSI: 0.020 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 109 - / - /56 /21 /13 Wind reactions based on MWFRS E Brg Wid = 28.0 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Wind

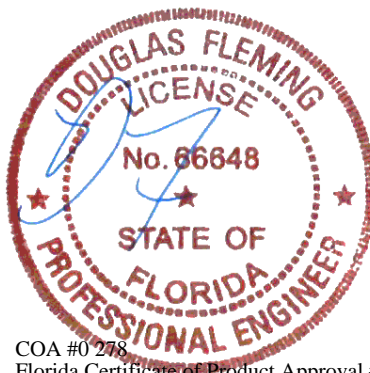
Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

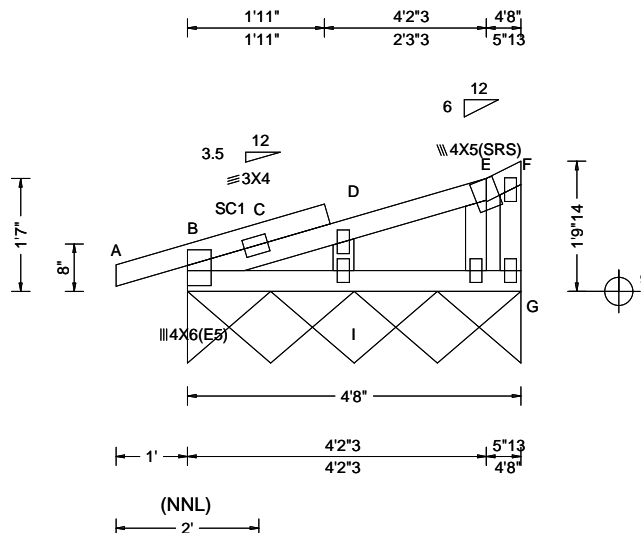


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 33373 / FROM:	GABL Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: E03	Cust: R 215 JRef: 1Y0W2150008 T53 / DrwNo: 173.24.1518.48162 NW / FV 06/21/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.002 C 999 180 HORZ(LL): -0.000 E - - HORZ(TL): 0.001 E - - Creep Factor: 2.0 Max TC CSI: 0.068 Max BC CSI: 0.037 Max Web CSI: 0.043 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G* 95 /- /- /53 /10 /10 Wind reactions based on MWFRS G Brg Wid = 56.0 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

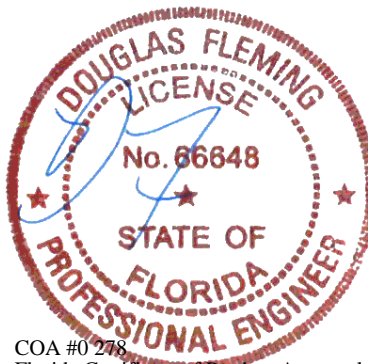
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

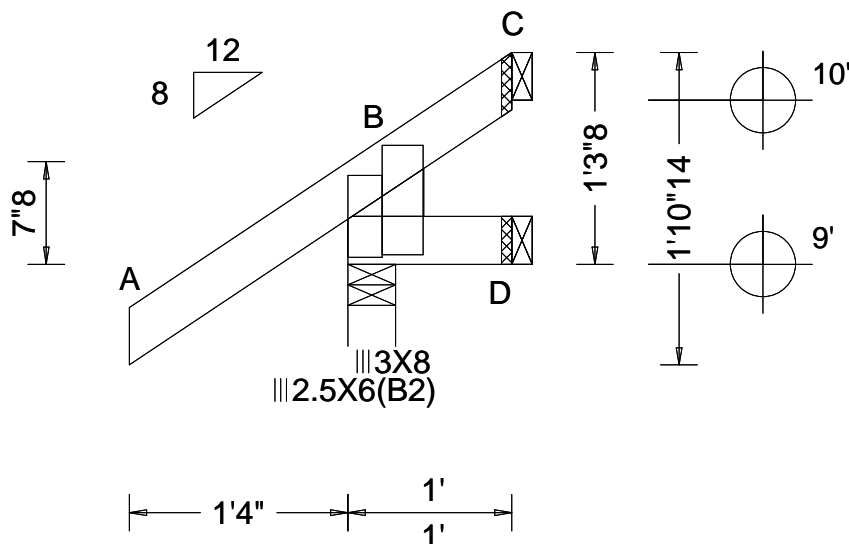


COA #0 278
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06/24/2024

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26098 FROM:	JACK Ply: 1 Qty: 4	Job Number: 24-1285 Hedrick Truss Label: J01	Cust: R 215 JRef: 1Y0W2150008 T32 DrwNo: 176.24.1001.39987 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.213 Max BC CSI: 0.024 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 195 /- /- /160 /41 /50 D 8 /-11 /- /14 /12 /- C - /-8 /- /17 /19 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Wedge: 2x4 SP #3;

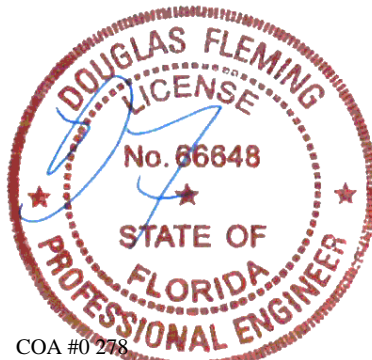
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.

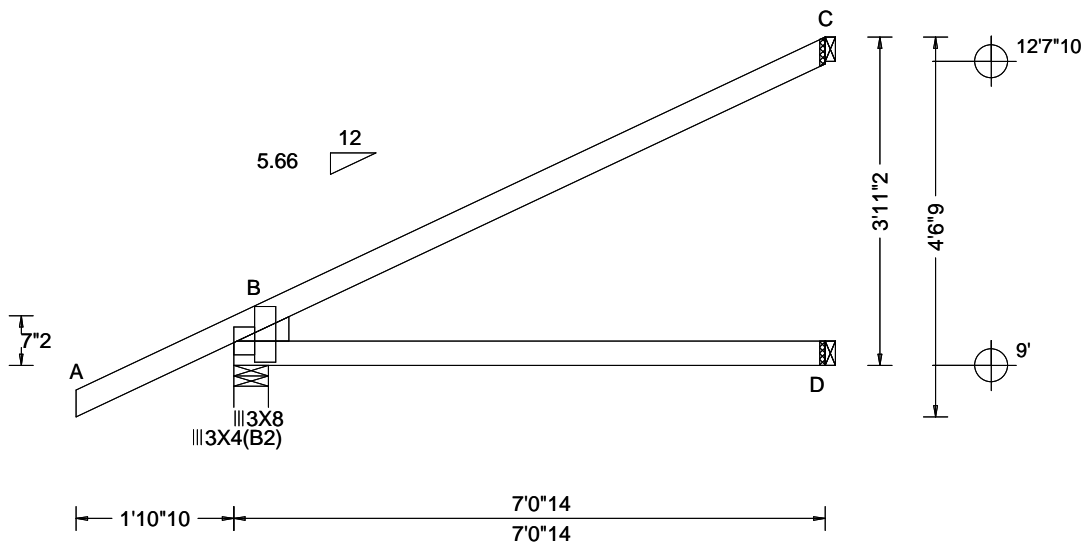


COA #0'278
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06/24/2024

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26102 FROM:	HIP_	Ply: 1 Qty: 2	Job Number: 24-1285 Hedrick Truss Label: J01HJ	Cust: R 215 JRef: 1Y0W2150008 T50 DrwNo: 176.24.1001.43977 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.017 B - - HORZ(TL): 0.035 B - - Creep Factor: 2.0 Max TC CSI: 0.775 Max BC CSI: 0.383 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 284 -/- /- /- /59 -/ D 87 -/- /- /29 -/- C 224 -/- /- /- /77 -/ Wind reactions based on MWFRS B Brg Wid = 4.9 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Wedge: 2x4 SP #3;

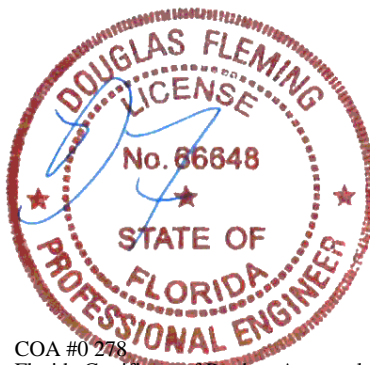
Loading

Hipjack supports 5-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe
nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe
nailed at Bot chord.

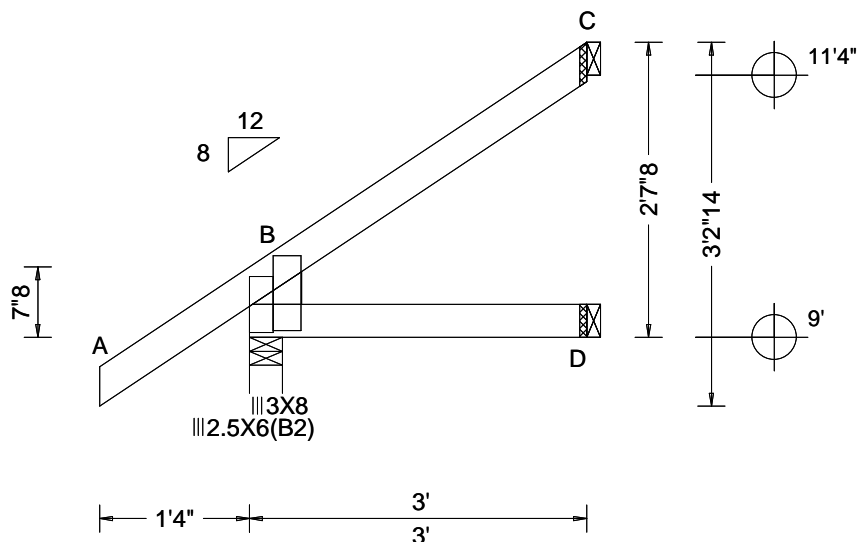


COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26096 FROM:	JACK Ply: 1 Qty: 4	Job Number: 24-1285 Hedrick Truss Label: J02	Cust: R 215 JRef: 1Y0W2150008 T28 DrwNo: 176.24.1001.47700 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.003 B - - Creep Factor: 2.0 Max TC CSI: 0.213 Max BC CSI: 0.076 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 238 - / - /176 /21 /97 D 54 - / - /33 - / - C 75 - / - /55 /47 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Wedge: 2x4 SP #3;

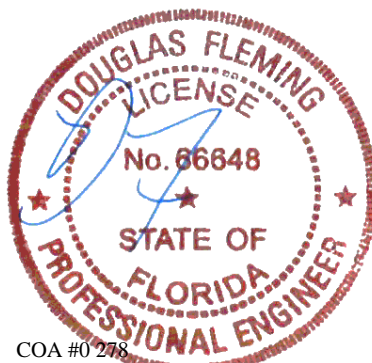
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.

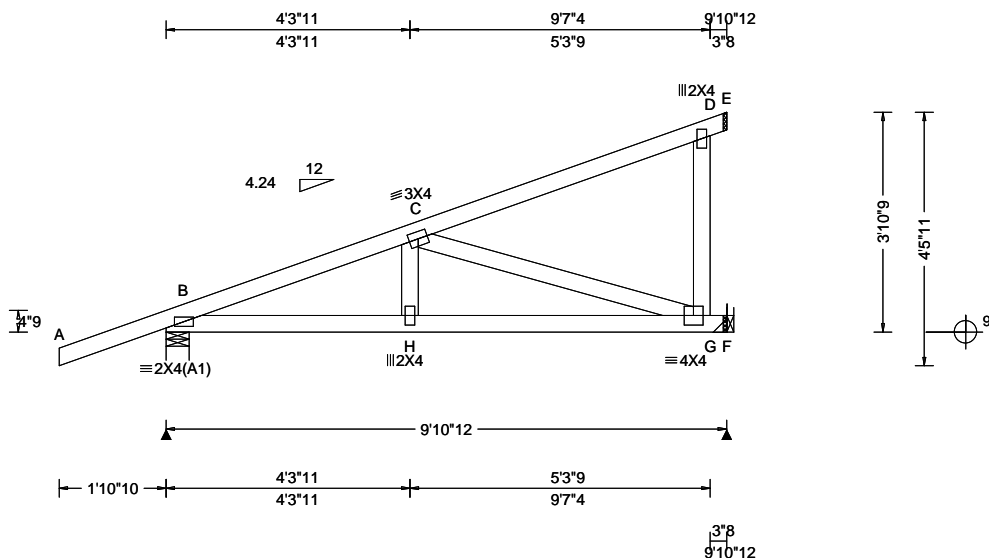


COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26360 FROM:	HIP_	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: J02HJ	Cust: R 215 JRef: 1Y0W2150008 T1 DrwNo: 176.24.1001.51767 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.030 C 999 240 VERT(CL): 0.060 C 999 180 HORZ(LL): 0.006 G - - HORZ(TL): 0.013 G - - Creep Factor: 2.0 Max TC CSI: 0.711 Max BC CSI: 0.688 Max Web CSI: 0.431 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL B 439 -/- /- /90 -/ F 625 -/- /- /107 -/ Non-Gravity Wind reactions based on MWFRS B Brg Wid = 4.9 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 132 -824

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

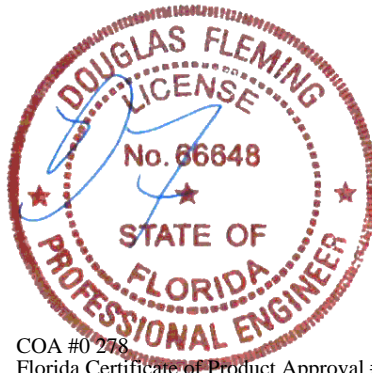
Loading

Hipjack supports 6-11-15 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.

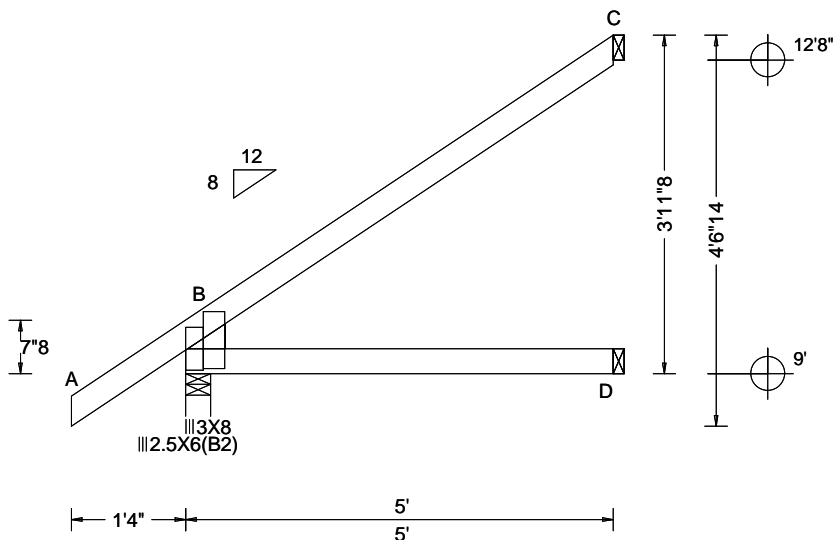


COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26100 FROM:	EJAC Ply: 1 Qty: 3	Job Number: 24-1285 Hedrick Truss Label: J03	Cust: R 215 JRef: 1Y0W2150008 T36 DrwNo: 176.24.1001.54670 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.009 B - - HORZ(TL): 0.017 B - - Creep Factor: 2.0 Max TC CSI: 0.446 Max BC CSI: 0.259 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 314 - / - /222 /17 /145 D 94 - / - /56 - / - C 138 - / - /101 /82 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Wedge: 2x4 SP #3;

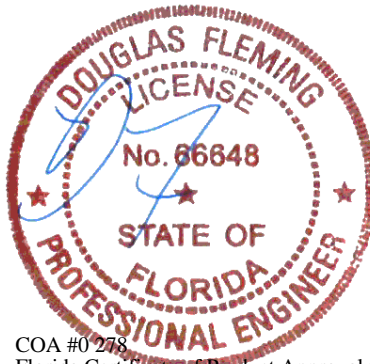
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0278

Florida Certificate of Product Approval #FL1999
06/24/2024

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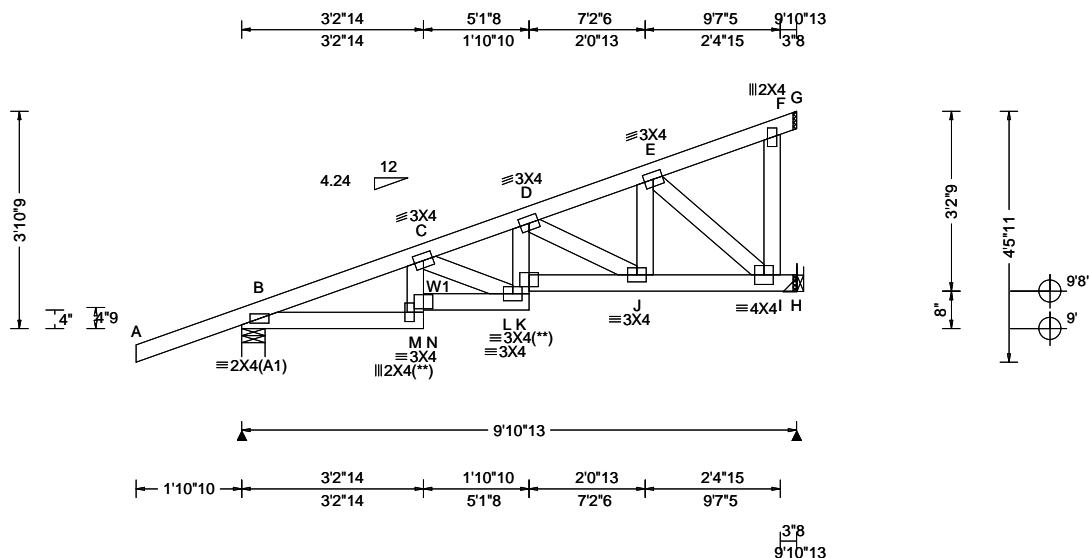
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26356 FROM:	HIP_	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: J03HJ	Cust: R 215 JRef: 1Y0W2150008 T25 DrwNo: 176.24.1001.57687 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.047 L 999 240 VERT(CL): 0.095 L 999 180 HORZ(LL): 0.022 I - - HORZ(TL): 0.044 I - - Creep Factor: 2.0 Max TC CSI: 0.308 Max BC CSI: 0.483 Max Web CSI: 0.925 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL B 439 -/- /- /- /90 -/ H 625 -/- /- /- /107 -/ Wind reactions based on MWFRS B Brg Wid = 4.9 Min Req = 1.5 (Truss) H Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 109 -671 D - E 113 -657 C - D 135 -787

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1 2x4 SP #2;

Plating Notes

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

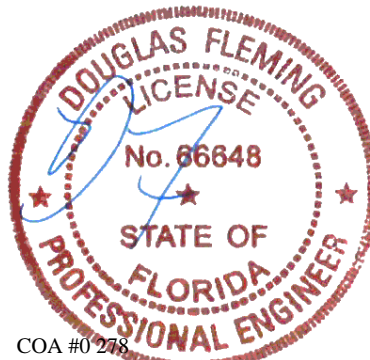
(J) Hanger Support Required, by others

Loading

Hipjack supports 7-0-0 setback jacks with no webs.

Wind

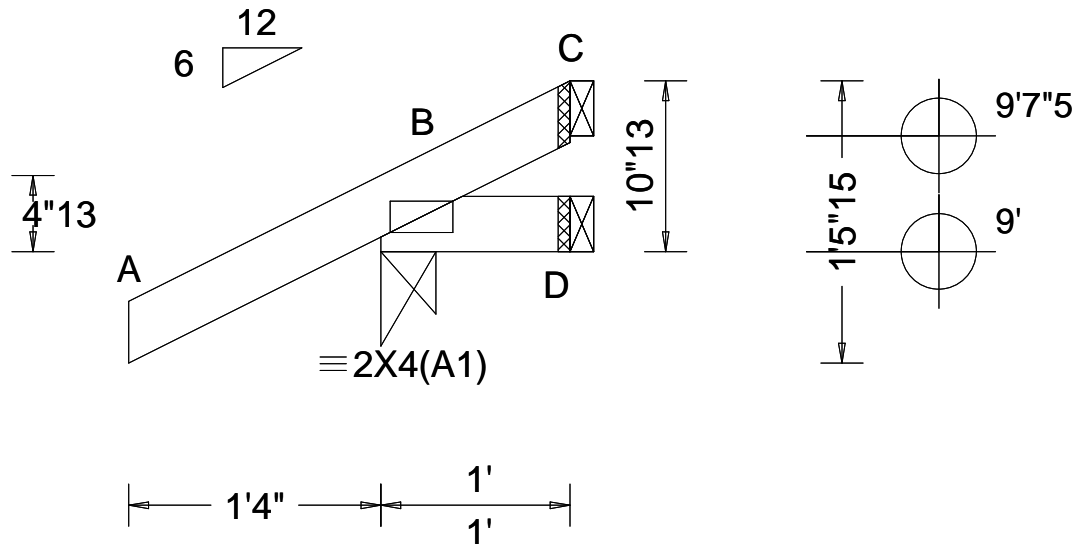
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	B 222 /- /- /174 /56 /36
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 B - -	D 8 /-10 /- /12 /10 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.001 B - -	C - /-41 /- /28 /40 /-
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.186	B Brg Wid = 3.5 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.028	D Brg Wid = 1.5 Min Req = -
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.000	C Brg Wid = 1.5 Min Req = -
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)		Bearing B is a rigid surface.
	Loc. from endwall: Any	Plate Type(s):		Members not listed have forces less than 375#
	GCpi: 0.18			
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.01A.1204.18	

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

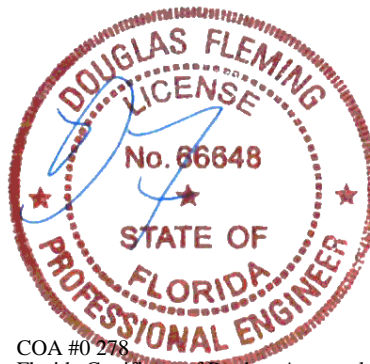
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.

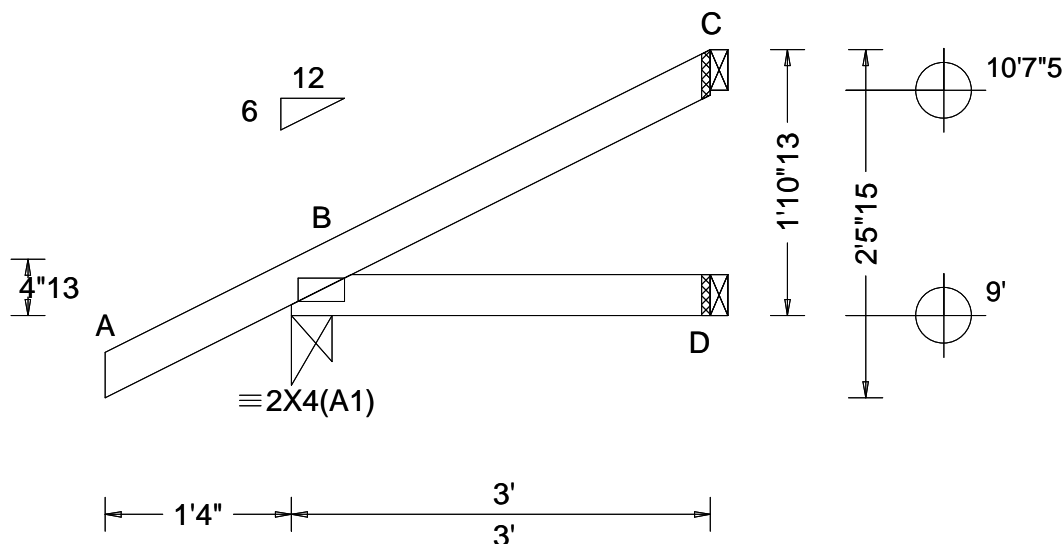


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SEQN: 26151 FROM:	JACK Qty: 2	Ply: 1 Qty: 2	Job Number: 24-1285 Hedrick Truss Label: J05	Cust: R 215 JRef: 1Y0W2150008 T23 DrwNo: 176.24.1002.04980 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.195 Max BC CSI: 0.069 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 244 - / - /175 /36 /71 D 51 - / - /27 /- /- C 66 - / - /39 /37 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

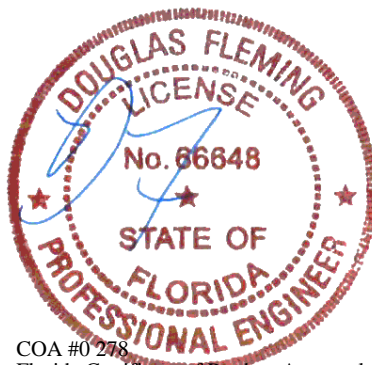
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.

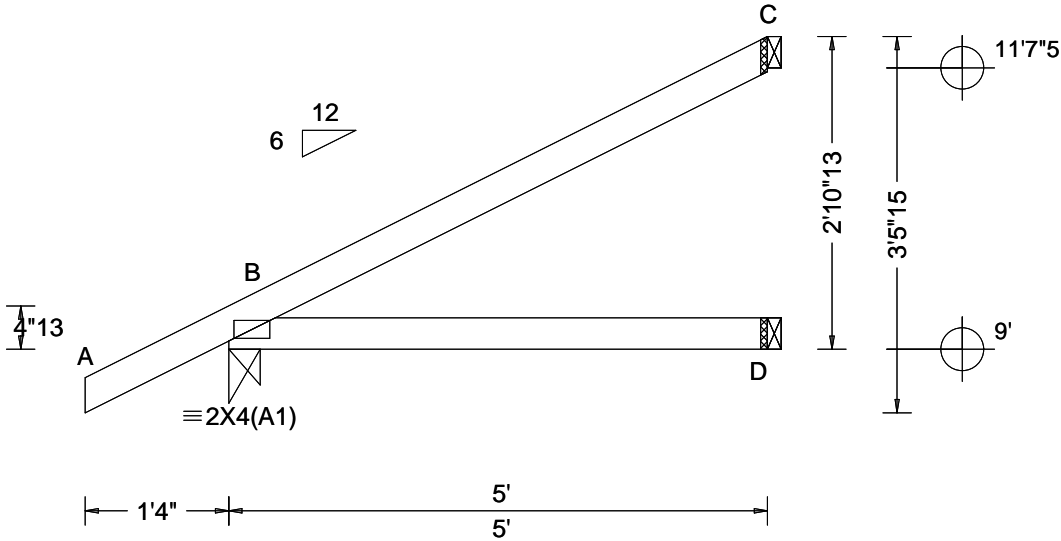


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SEQN: 26153 FROM:	JACK Ply: 1 Qty: 2	Job Number: 24-1285 Hedrick Truss Label: J06	Cust: R 215 JRef: 1Y0W2150008 T21 DrwNo: 176.24.1002.07323 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.008 B - - Creep Factor: 2.0 Max TC CSI: 0.336 Max BC CSI: 0.241 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 316 /- /- /218 /39 /107 D 91 /- /- /49 /- /- C 130 /- /- /81 /67 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

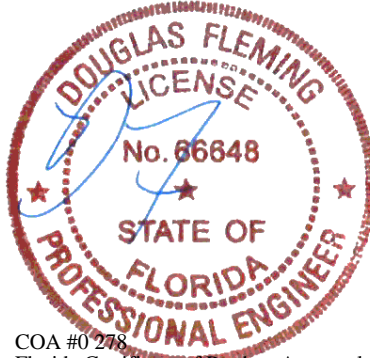
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.

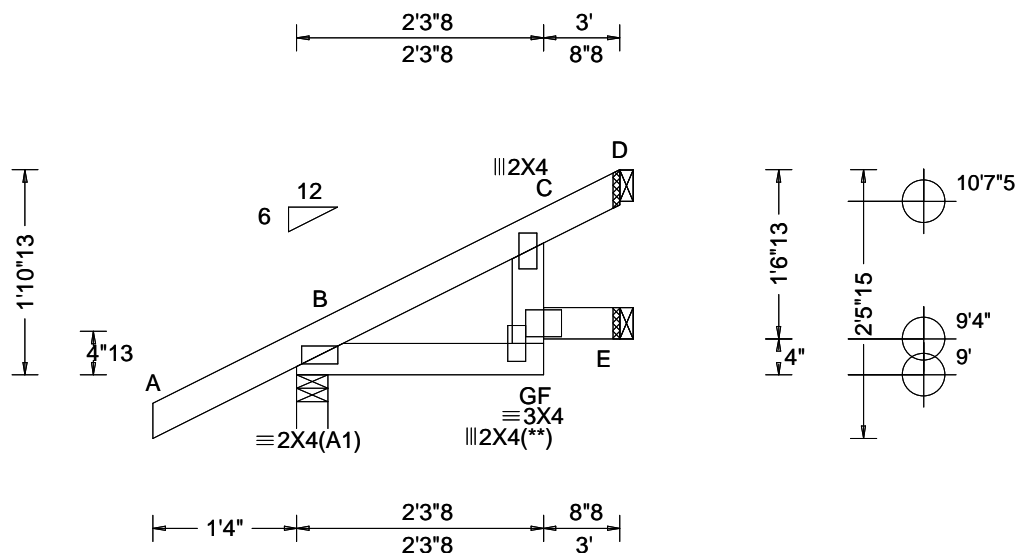


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Glenview, IL 60025

SEQN: 26165 FROM:	JACK Qty: 2	Ply: 1 Qty: 2	Job Number: 24-1285 Hedrick Truss Label: J07	Cust: R 215 JRef: 1Y0W2150008 T17 DrwNo: 176.24.1002.09547 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 F 999 240 VERT(CL): 0.007 F 999 180 HORZ(LL): 0.002 C - - HORZ(TL): 0.003 C - - Creep Factor: 2.0 Max TC CSI: 0.190 Max BC CSI: 0.038 Max Web CSI: 0.032 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 244 - / - /175 /36 /71 E 21 - / - /13 - / - D 76 - / - /51 /28 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

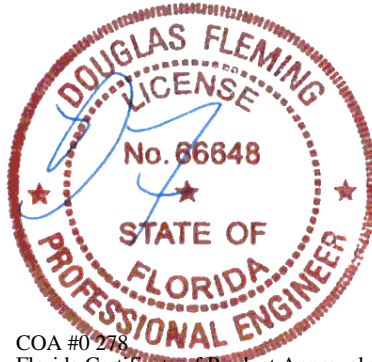
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.

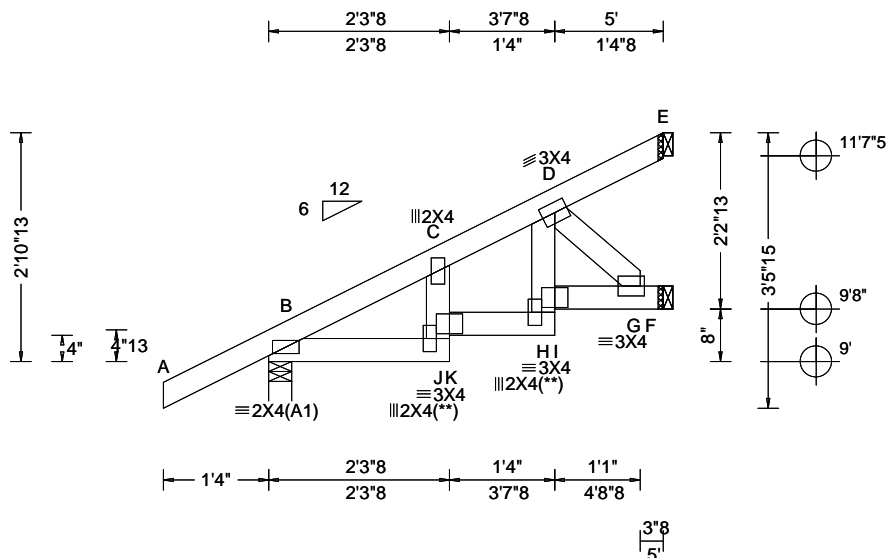


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Glenview, IL 60025

SEQN: 26161 FROM:	JACK Ply: 1 Qty: 2	Job Number: 24-1285 Hedrick Truss Label: J08	Cust: R 215 JRef: 1Y0W2150008 T45 DrwNo: 176.24.1002.12543 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.012 J 999 240 VERT(CL): 0.023 J 999 180 HORZ(LL): 0.006 C - - HORZ(TL): 0.012 C - - Creep Factor: 2.0 Max TC CSI: 0.160 Max BC CSI: 0.122 Max Web CSI: 0.172 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 316 /- /- /218 /39 /107 F 162 /- /- /117 /40 /- E 34 /-5 /- /14 /14 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 1.5 Min Req = - E Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

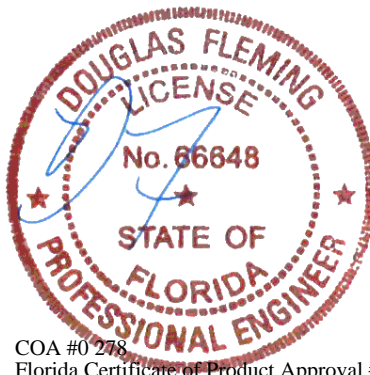
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.

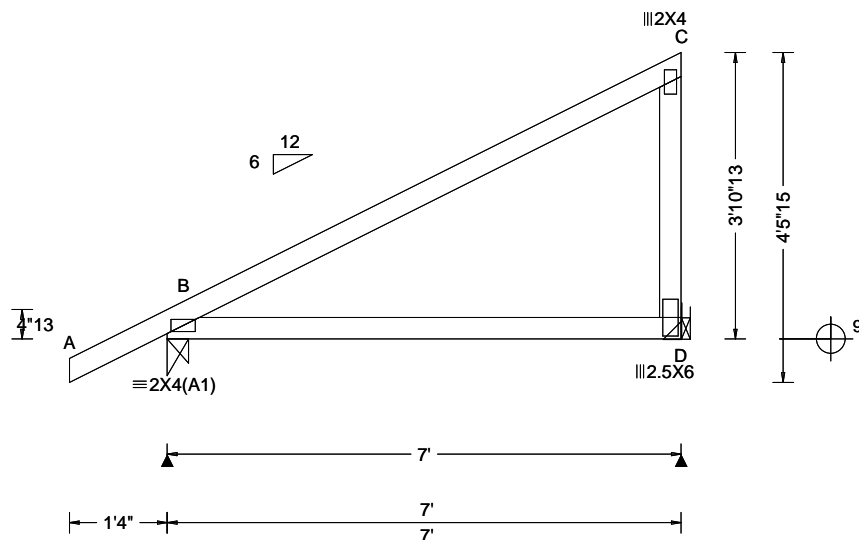


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Glenview, IL 60025

SEQN: 26334 / FROM:	EJAC	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: J09	Cust: R 215 JRef: 1Y0W2150008 T6 DrwNo: 173.24.1518.48427 NW / FV 06/21/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.012 B - - HORZ(TL): 0.023 B - - Creep Factor: 2.0 Max TC CSI: 0.659 Max BC CSI: 0.472 Max Web CSI: 0.266 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 394 - / - / 267 / 43 / 142 D 272 - / - / 193 / 78 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

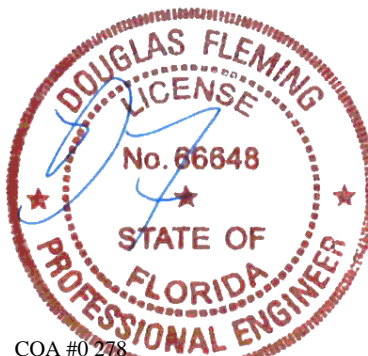
(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

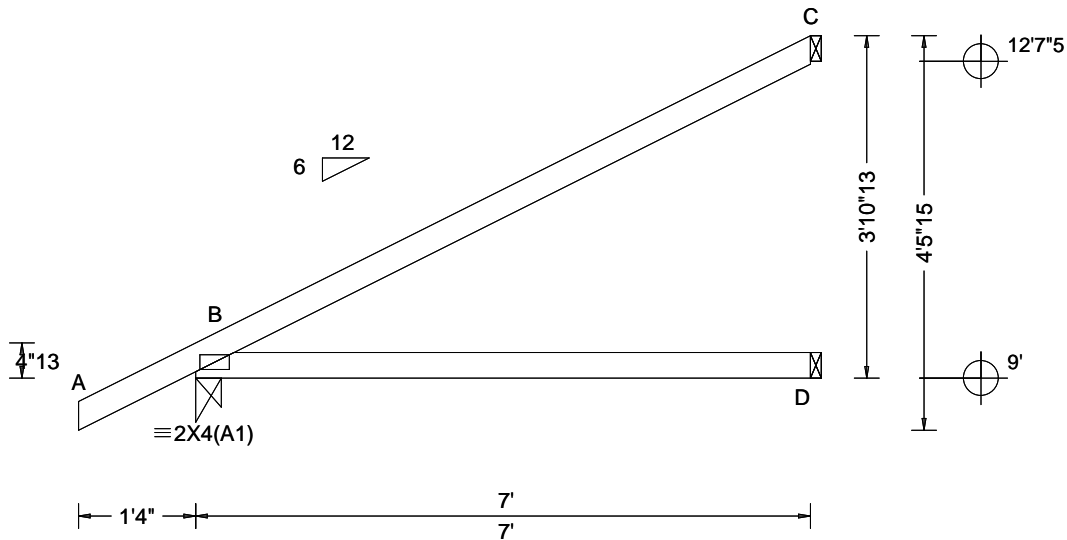
Wind loading based on both gable and hip roof types.



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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	B 394 /- /- /267 /43 /142
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.013 B - -	D 130 /- /- /73 /- /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.026 B - -	C 190 /- /- /120 /96 /-
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.735	B Brg Wid = 3.5 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.520	D Brg Wid = 1.5 Min Req = -
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.000	C Brg Wid = 1.5 Min Req = -
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)		Bearing B is a rigid surface.
	Loc. from endwall: not in 4.50 ft	Plate Type(s):		Members not listed have forces less than 375#
	GCpi: 0.18			
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.01A.1204.18	

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

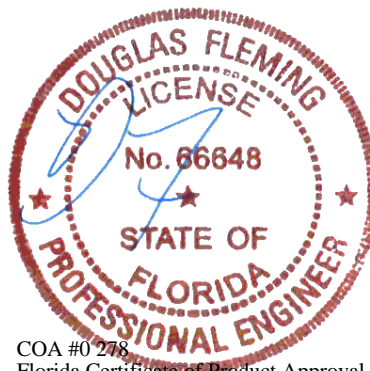
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.

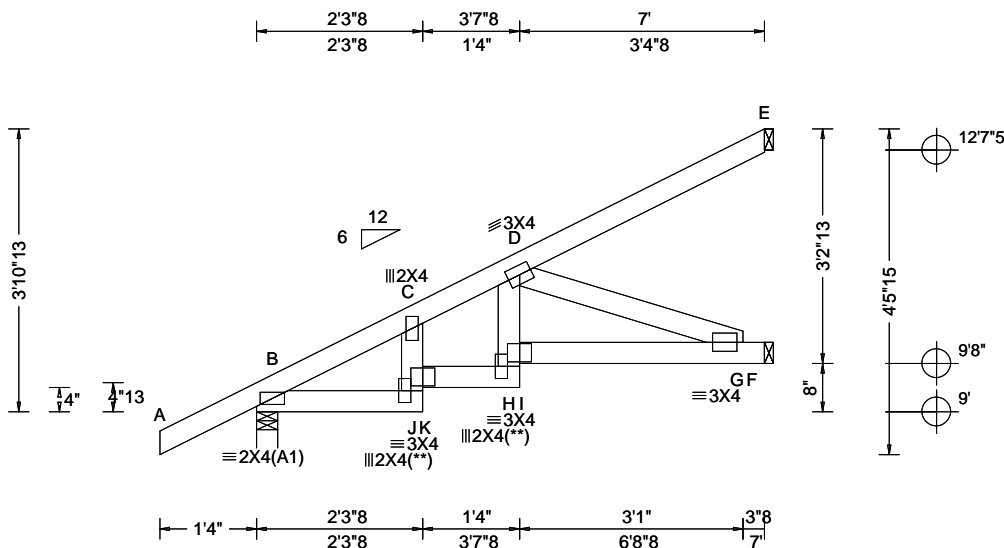


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SEQN: 26338 FROM:	EJAC Ply: 1 Qty: 3	Job Number: 24-1285 Hedrick Truss Label: J11	Cust: R 215 JRef: 1Y0W2150008 T22 DrwNo: 176.24.1002.16027 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.017 J 999 240 VERT(CL): 0.032 J 999 180 HORZ(LL): 0.010 G - - HORZ(TL): 0.020 G - - Creep Factor: 2.0 Max TC CSI: 0.161 Max BC CSI: 0.246 Max Web CSI: 0.397 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 394 - / - / - /267 /43 /142 F 181 - / - / - /137 /31 - E 94 - / - / - /57 /47 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 1.5 Min Req = - E Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. H - G 412 -311

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

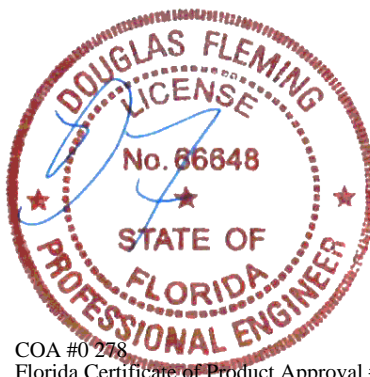
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

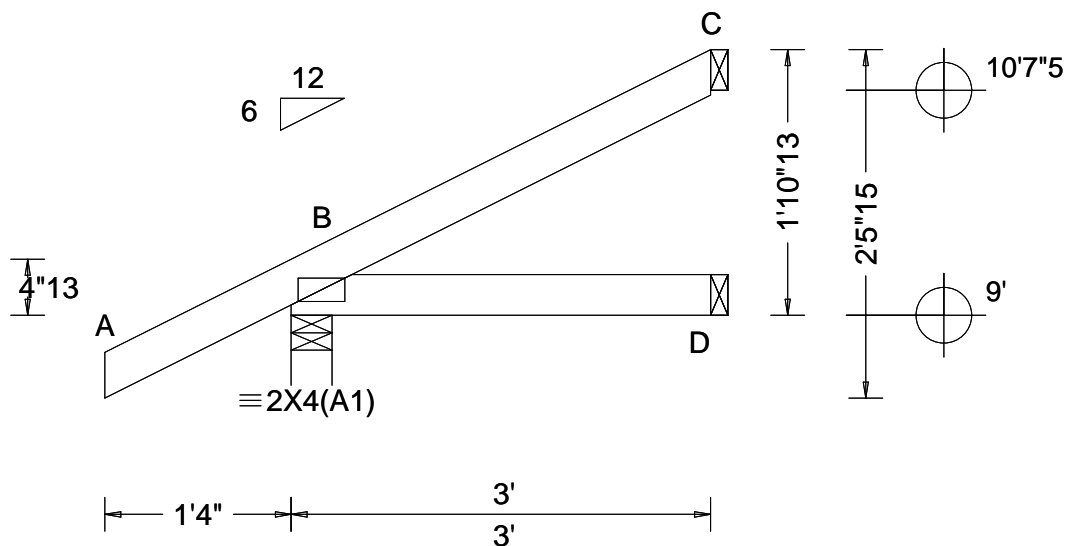


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SEQN: 26495 / FROM:	EJAC Ply: 1 Qty: 18	Job Number: 24-1285 Hedrick Truss Label: J13	Cust: R 215 JRef: 1Y0W2150008 T4 / DrwNo: 173.24.1518.48444 NW / FV 06/21/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.195 Max BC CSI: 0.068 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 244 - / - /175 /37 /71 D 51 - / - /27 - / - C 66 - / - /39 /37 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

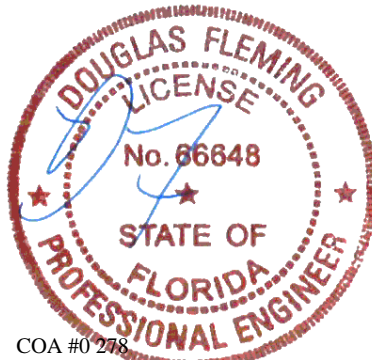
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.

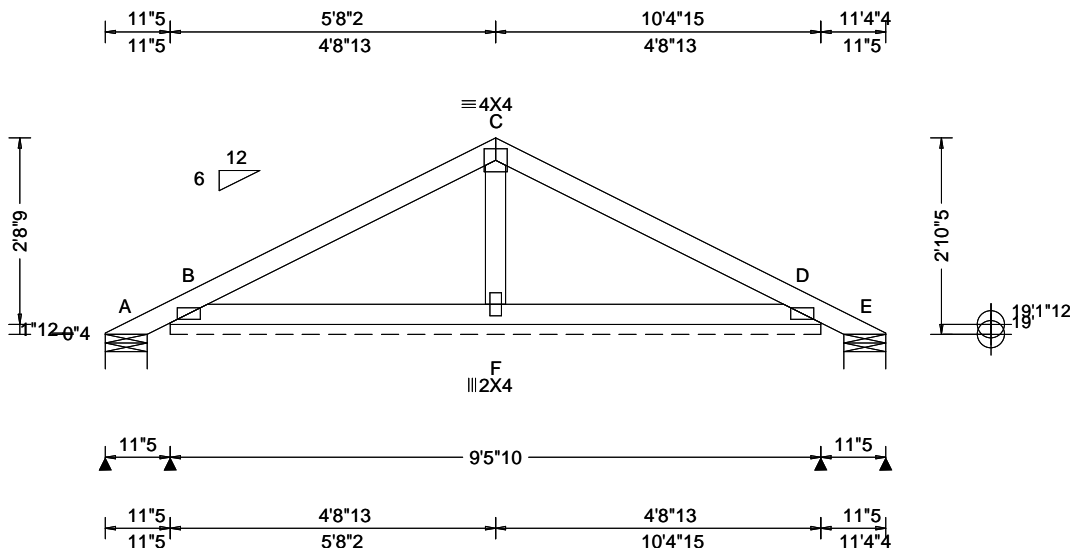


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06/24/2024

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North Building, 4th Floor
Glenview, IL 60025

SEQN: 26268 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: PB01	Cust: R 215 JRef: 1Y0W2150008 T12 DrwNo: 176.24.0951.01433 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.30 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.06 ft ft Loc. from endwall: not in 6.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 D 999 240 VERT(CL): 0.005 D 999 180 HORZ(LL): -0.002 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.237 Max BC CSI: 0.104 Max Web CSI: 0.030 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-91 /- /66 /107 /74 B* 95 /- /- /63 /33 /- E - /-91 /- /48 /75 /- Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 (Truss) B Brg Wid = 113 Min Req = - E Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

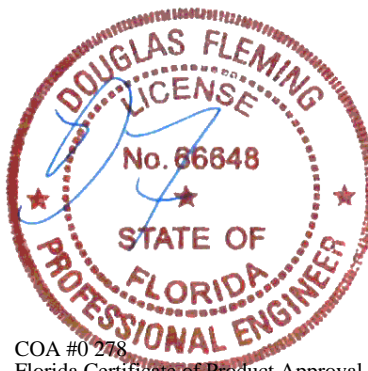
Wind

Wind loads based on MWFRS.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160220723 for piggyback details.

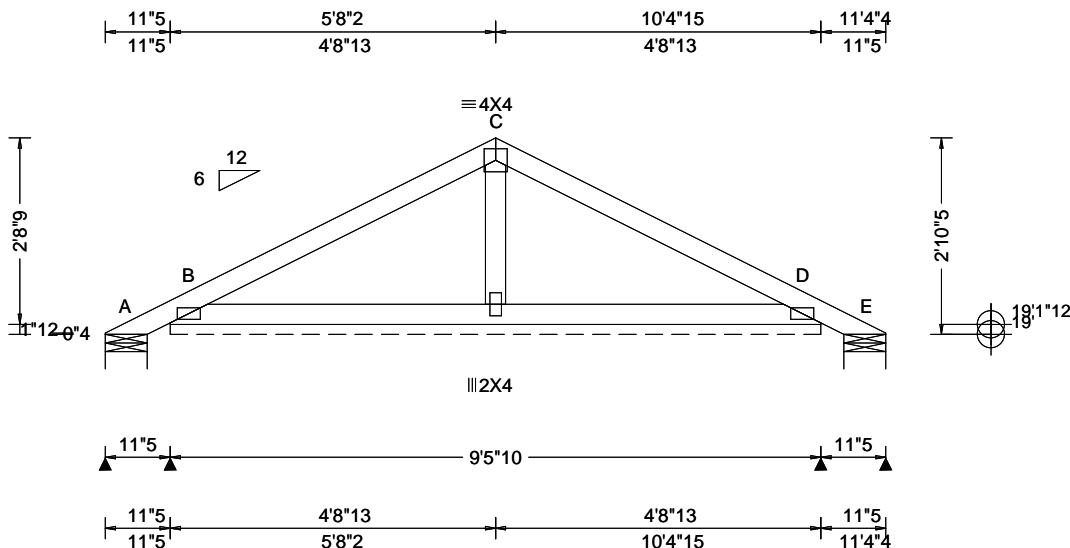


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North Building, 4th Floor
Glenview, IL 60025

SEQN: 34760 FROM:	COMN Qty: 12	Ply: 1 Hedrick Truss Label: PB02	Cust: R 215 JRef: 1Y0W2150008 T31 DrwNo: 176.24.0951.02617 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.30 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 D 999 240 VERT(CL): 0.007 D 999 180 HORZ(LL): -0.002 D - - HORZ(TL): 0.004 D - - Creep Factor: 2.0 Max TC CSI: 0.243 Max BC CSI: 0.178 Max Web CSI: 0.027 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-101 /- /64 /110 /76 B* 116 /- /- /65 /6 /- E - /-101 /- /30 /77 /- Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 (Truss) B Brg Wid = 113 Min Req = - E Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

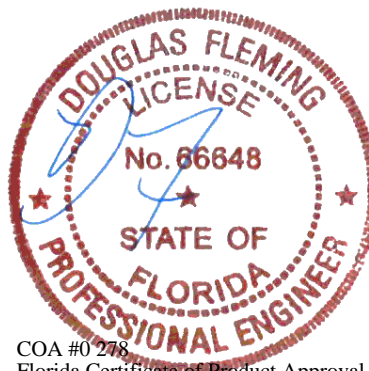
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160220723 for piggyback details.



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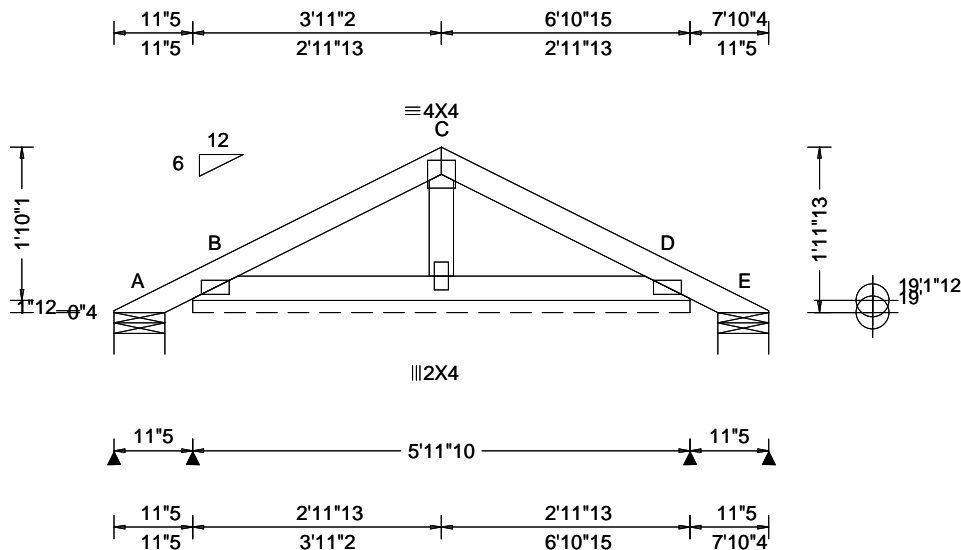
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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 D 999 240	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.003 D 999 180	A 12 /-30 /- /34 /35 /67
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.000 D - -	L* 92 /- /- /49 /7 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.001 D - -	H 14 /- /- /7 /5 /-
NCBCLL: 10.00	Mean Height: 20.28 ft	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.043	A Brg Wid = 7.3 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.013	L Brg Wid = 105 Min Req = -
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.053	H Brg Wid = 7.3 Min Req = 1.5 (Truss)
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)		Bearings A, L, & H are a rigid surface.
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Members not listed have forces less than 375#
	GCpi: 0.18	WAVE		
	Wind Duration: 1.60		VIEW Ver: 23.02.01A.1204.18	

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SEQN: 26254 FROM:	COMN Ply: 1 Qty: 3	Job Number: 24-1285 Hedrick Truss Label: PB04	Cust: R 215 JRef: 1Y0W2150008 T55 DrwNo: 176.24.0951.23930 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.002 B 999 180 HORZ(LL): -0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.086 Max BC CSI: 0.074 Max Web CSI: 0.017 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-8 /- /29 /29 /50 B* 105 /- /- /59 /3 /- E - /-8 /- /7 /7 /- Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 (Truss) B Brg Wid = 71.6 Min Req = - E Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

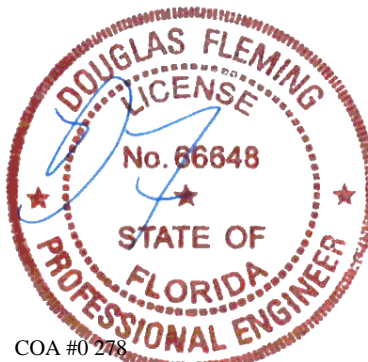
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160220723 for piggyback details.

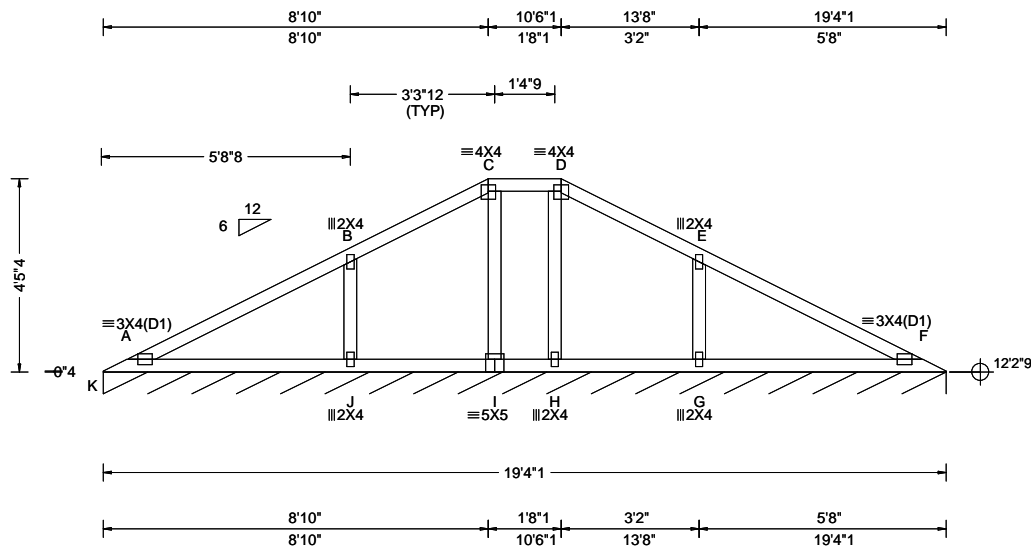


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Glenview, IL 60025

SEQN: 26204 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V01	Cust: R 215 JRRef: 1Y0W2150008 T60 DrwNo: 176.24.1002.20990 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.019 F 999 240 VERT(CL): 0.039 F 999 180 HORZ(LL): -0.007 F - - HORZ(TL): 0.014 F - - Creep Factor: 2.0 Max TC CSI: 0.392 Max BC CSI: 0.246 Max Web CSI: 0.080 VIEW Ver: 23.02.01A.1204.18	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity K* 82 - / - / 42 / 1 / 5 Wind reactions based on MWFRS K Brg Wid = 232 Min Req = - Bearing K is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

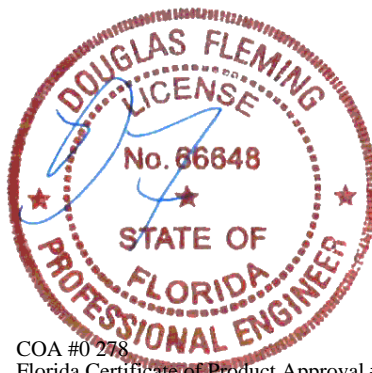
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

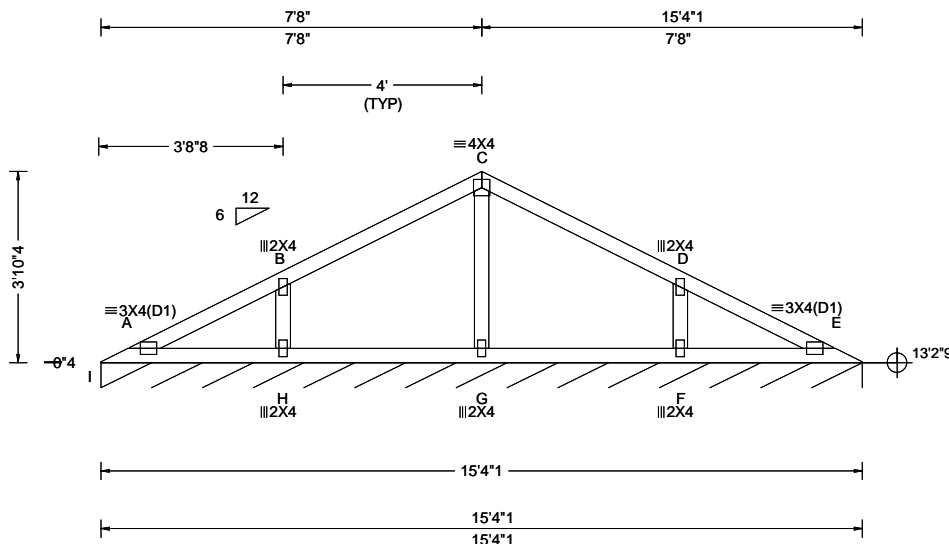


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SEQN: 26195 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V02	Cust: R 215 JRef: 1Y0W2150008 T62 DrwNo: 176.24.1002.22150 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.30 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 E 999 240 VERT(CL): 0.008 E 999 180 HORZ(LL): -0.001 E - - HORZ(TL): 0.003 E - - Creep Factor: 2.0 Max TC CSI: 0.275 Max BC CSI: 0.126 Max Web CSI: 0.067 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 82 /- /- /42 /1 /6 Wind reactions based on MWFRS I Brg Wid = 184 Min Req = - Bearing I is a rigid surface. Members not listed have forces less than 375#

Lumber

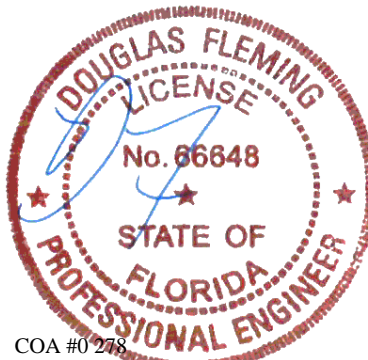
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

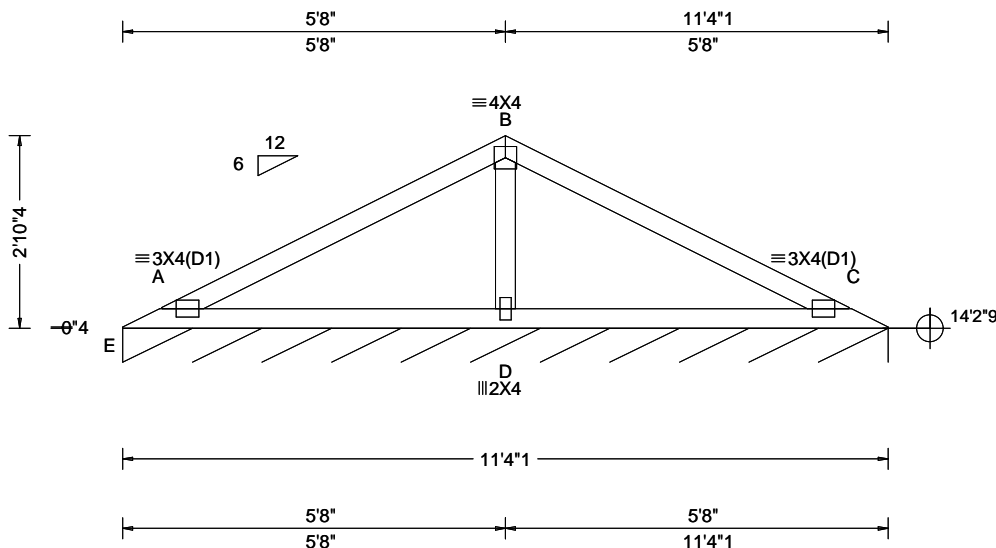


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Glenview, IL 60025

SEQN: 26197 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V03	Cust: R 215 JRef: 1Y0W2150008 T63 DrwNo: 176.24.1002.23547 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.80 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.021 C 999 240 VERT(CL): 0.044 C 999 180 HORZ(LL): -0.009 C - - HORZ(TL): 0.018 C - - Creep Factor: 2.0 Max TC CSI: 0.434 Max BC CSI: 0.369 Max Web CSI: 0.133 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 82 /- /- /41 /0 /6 Wind reactions based on MWFRS E Brg Wid = 136 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 455 -213 B - C 455 -227 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - D 382 -626

Lumber

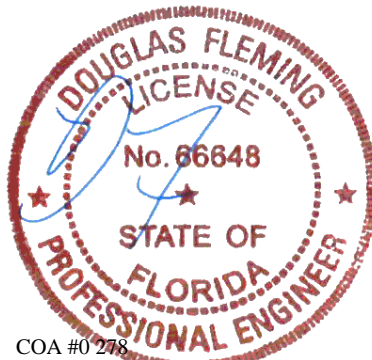
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

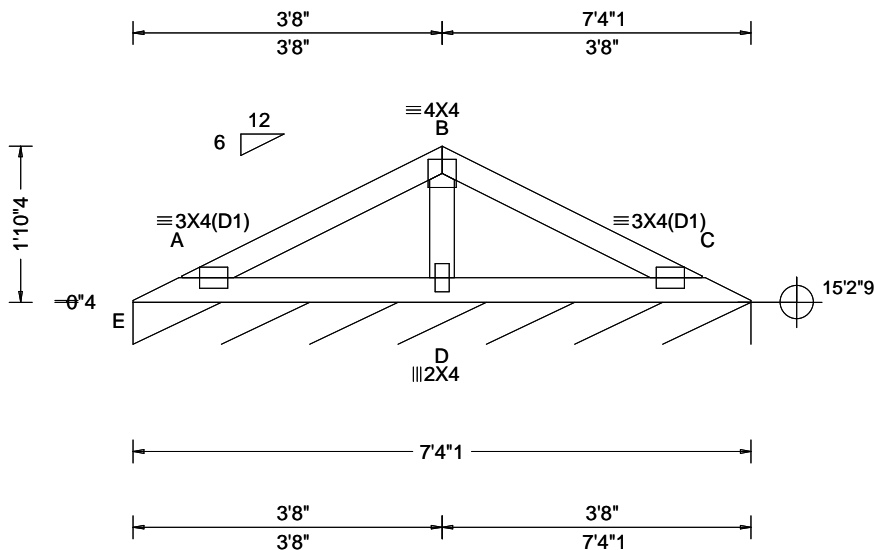


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Glenview, IL 60025

SEQN: 26199 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V04	Cust: R 215 JRef: 1Y0W2150008 T46 DrwNo: 176.24.1003.49680 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.30 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.011 C 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.152 Max BC CSI: 0.139 Max Web CSI: 0.074 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 82 /- /- /40 /- /5 Wind reactions based on MWFRS E Brg Wid = 88.0 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375#

Lumber

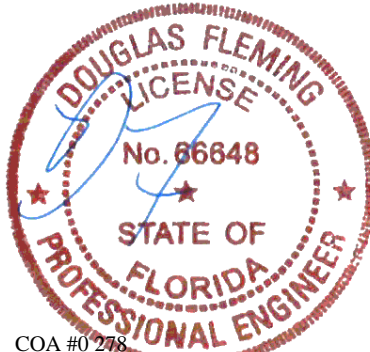
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

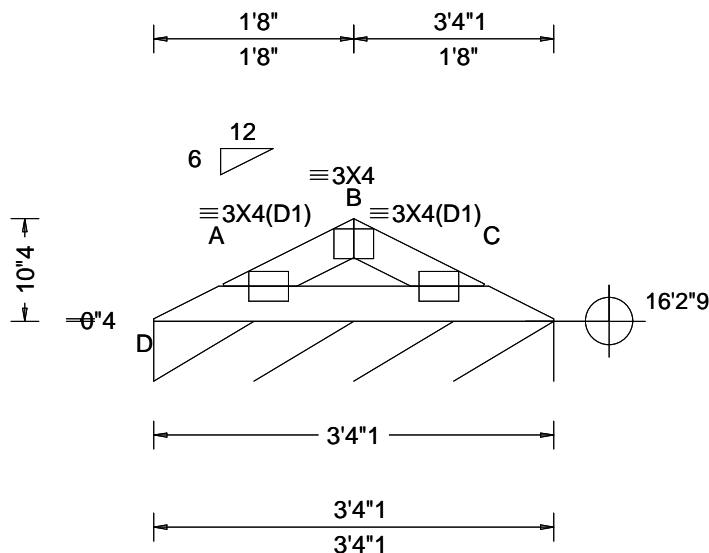


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06/24/2024

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Glenview, IL 60025

SEQN: 26201 FROM:	VAL Qty: 1	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V05	Cust: R 215 JRef: 1Y0W2150008 T42 DrwNo: 176.24.1003.51183 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.80 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 C 999 240 VERT(CL): 0.005 C 999 180 HORZ(LL): -0.001 A - - HORZ(TL): 0.002 A - - Creep Factor: 2.0 Max TC CSI: 0.048 Max BC CSI: 0.074 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 82 /- /- /35 /- /4 Wind reactions based on MWFRS D Brg Wid = 40.0 Min Req = - Bearing D is a rigid surface. Members not listed have forces less than 375#

Lumber

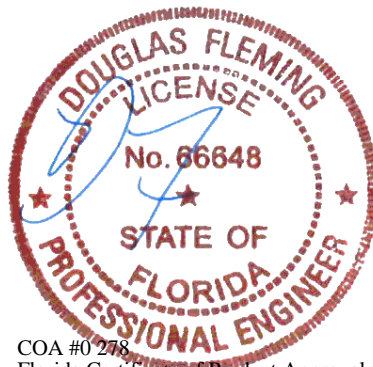
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

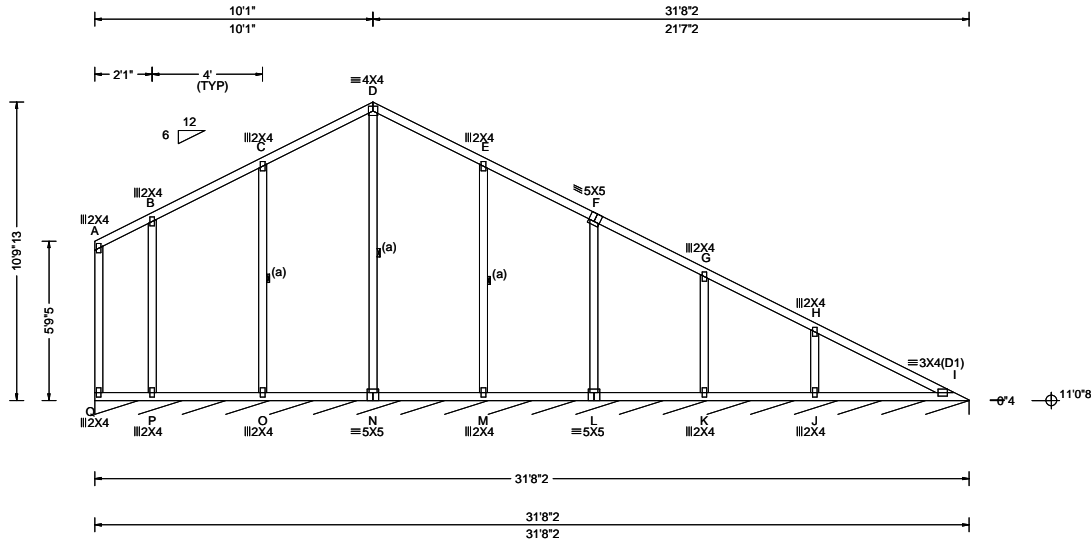


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Glenview, IL 60025

SEQN: 26129 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V06	Cust: R 215 JRef: 1Y0W2150008 T65 DrwNo: 176.24.1003.54010 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.61 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.17 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.021 I 999 240 VERT(CL): 0.043 I 999 180 HORZ(LL): 0.008 C - - HORZ(TL): 0.013 C - - Creep Factor: 2.0 Max TC CSI: 0.336 Max BC CSI: 0.251 Max Web CSI: 0.203 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL Q* 82 /- /- /47 /14 /9 Wind reactions based on MWFRS Q Brg Wid = 380 Min Req = - Bearing Q is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

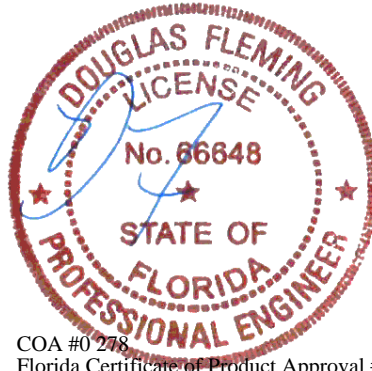
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

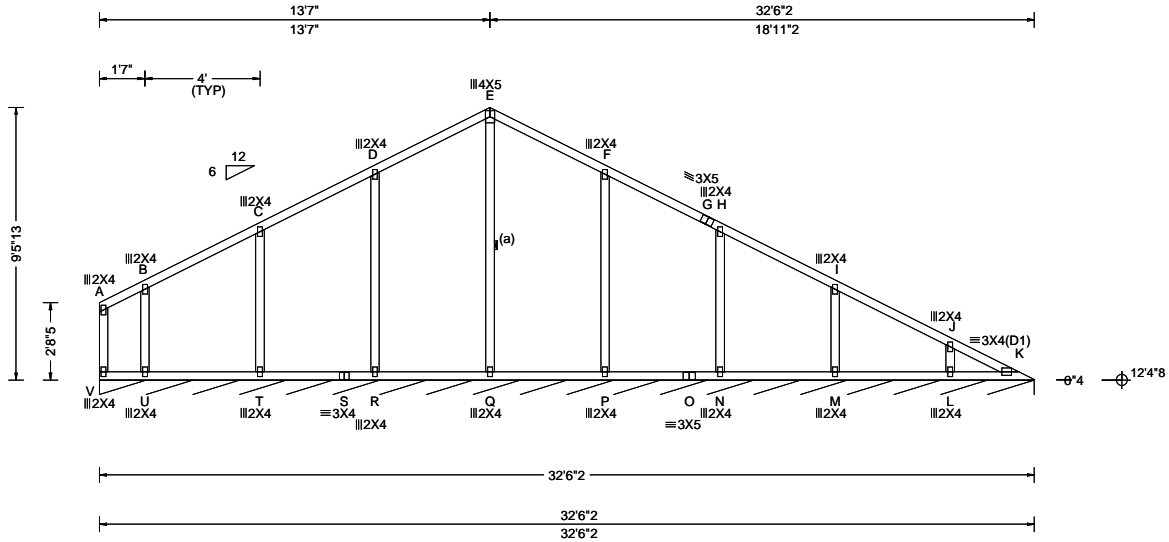


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North Building, 4th Floor
Glenview, IL 60025

SEQN: 26440 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V07	Cust: R 215 JRRef: 1Y0W2150008 T69 DrwNo: 176.24.1003.56050 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.27 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.26 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 F 999 240 VERT(CL): 0.004 F 999 180 HORZ(LL): -0.007 B - - HORZ(TL): 0.013 B - - Creep Factor: 2.0 Max TC CSI: 0.202 Max BC CSI: 0.113 Max Web CSI: 0.288 VIEW Ver: 23.02.01A.1204.18	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
				V* 82	/-	/-	/44	/14	/7	
				Wind reactions based on MWFRS						
				V Brg Wid = 390 Min Req = -						
				Bearing V is a rigid surface.						
				Members not listed have forces less than 375#						

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

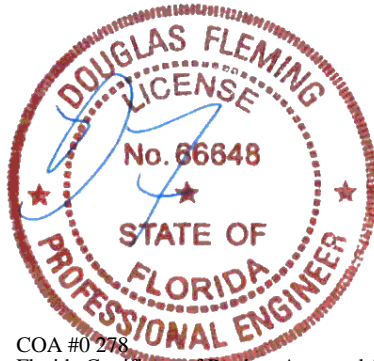
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



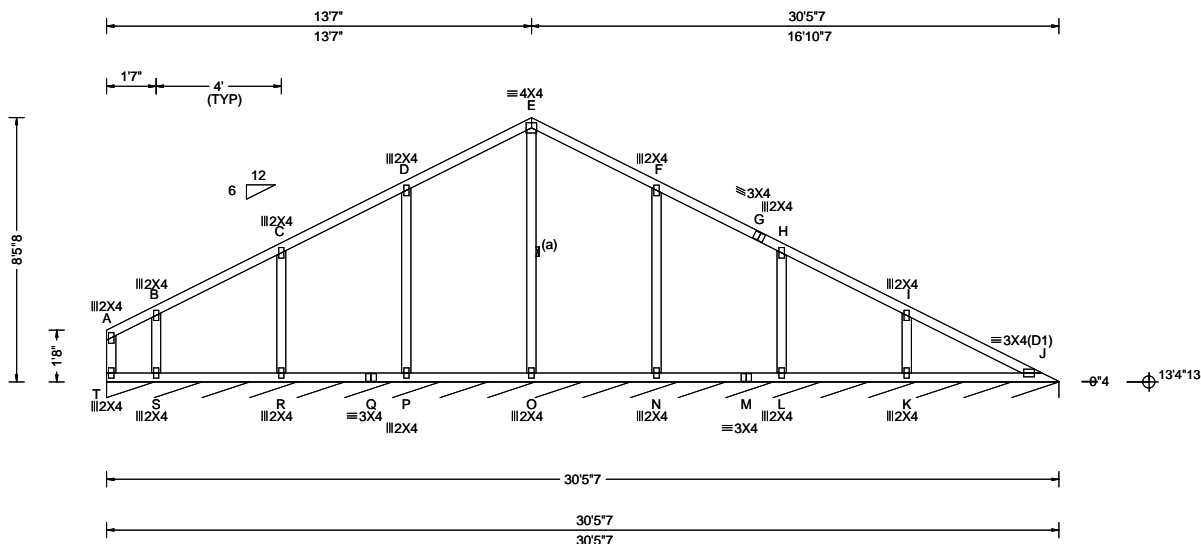
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Glenview, IL 60025

SEQN: 26454 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V08	Cust: R 215 JRef: 1Y0W2150008 T54 DrwNo: 176.24.1003.57320 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.79 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.05 ft ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.013 J 999 240 VERT(CL): 0.026 J 999 180 HORZ(LL): -0.006 A - - HORZ(TL): 0.011 A - - Creep Factor: 2.0 Max TC CSI: 0.277 Max BC CSI: 0.206 Max Web CSI: 0.214 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL T* 82 /- /- /43 /14 /7 Wind reactions based on MWFRS T Brg Wid = 365 Min Req = - Bearing T is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

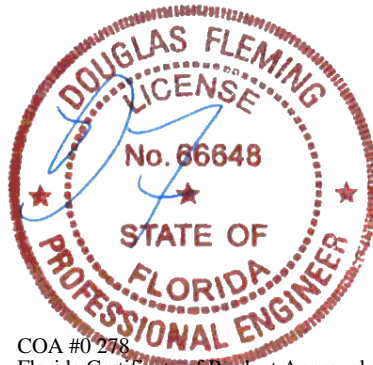
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

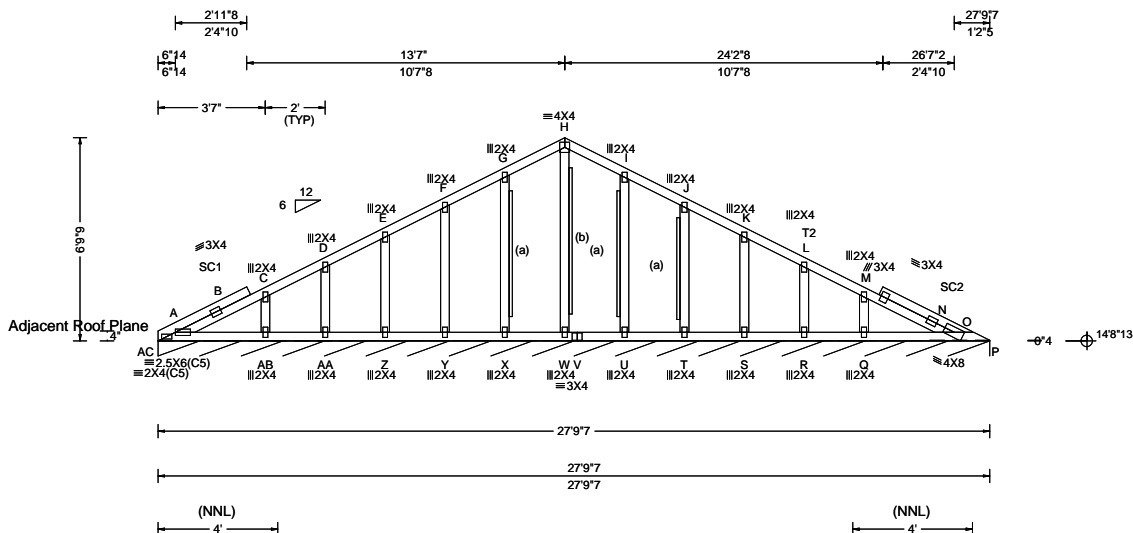


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SEQN: 9969	VAL	Ply: 1	Job Number: 24-1285	Cust: R 215 JRef: 1Y0W2150008 T15
FROM:		Qty: 1	Hedrick	DrwNo: 176.24.1003.59853
			Truss Label: V09	NW / DF 06/24/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.004 B 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.009 B 999 180	AC*82 -/- /- /42 /13 /6
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 G - -	Wind reactions based on MWFRS
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.003 B - -	AC Brg Wid = 333 Min Req = -
NCBCLL: 10.00	Mean Height: 18.28 ft	Building Code:	Creep Factor: 2.0	Bearing AC is a rigid surface.
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.070	Members not listed have forces less than 375#
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.080	
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.932	
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)		
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		
	GCpi: 0.18	WAVE	VIEW Ver: 24.01.02.0529.20	
	Wind Duration: 1.60			

Lumber

Top chord: 2x4 SP M-31; T2 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP M-31;
Stack Chord: SC2 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/287.

Gable Reinforcement

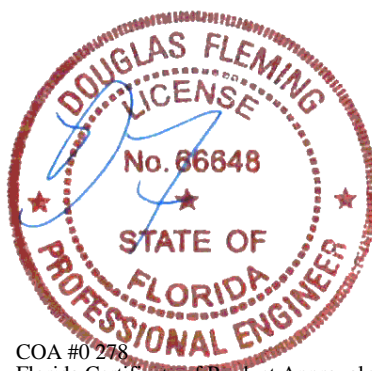
(a) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

(b) 1x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6. See DWGS VALTN220723 and VAL180220723 for valley details.



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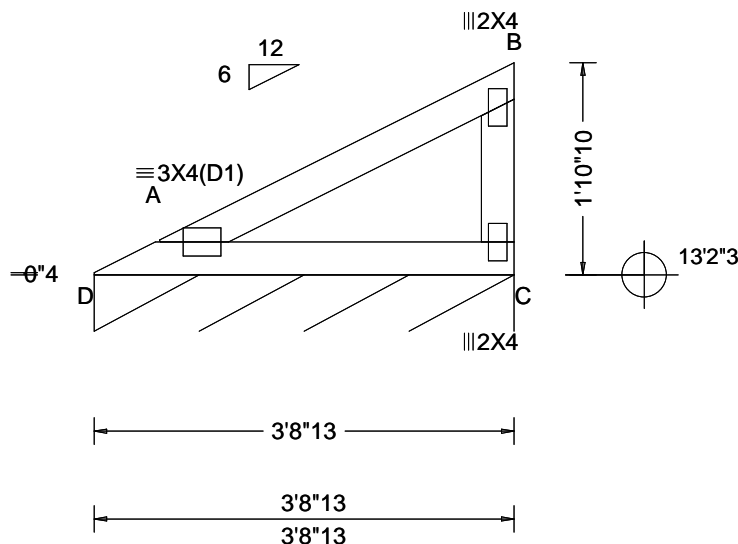
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;	
Wind Wind loads based on MWFRS with additional C&C member design. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.	
Additional Notes See DWGS VALTN220723 and VAL180220723 for valley details.	



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SEQN: 26456 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V11	Cust: R 215 JRef: 1Y0W2150008 T37 DrwNo: 176.24.1004.12467 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.002 A - - HORZ(TL): 0.005 A - - Creep Factor: 2.0 Max TC CSI: 0.166 Max BC CSI: 0.130 Max Web CSI: 0.079 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 82 /- /- /50 /10 /20 Wind reactions based on MWFRS D Brg Wid = 44.8 Min Req = - Bearing D is a rigid surface. Members not listed have forces less than 375#

Lumber

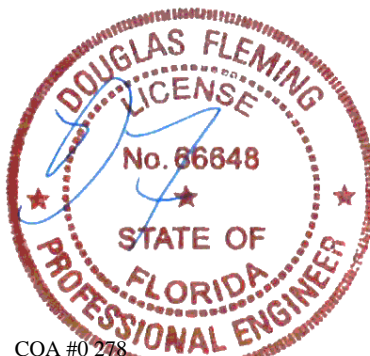
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

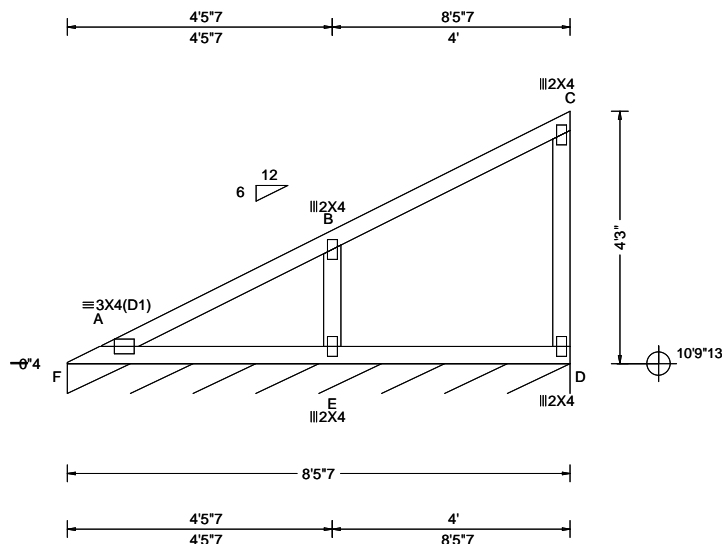


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SEQN: 26458 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V13	Cust: R 215 JRef: 1Y0W2150008 T66 DrwNo: 176.24.1004.14680 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 A 999 240 VERT(CL): 0.018 A 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.005 A - - Creep Factor: 2.0 Max TC CSI: 0.295 Max BC CSI: 0.198 Max Web CSI: 0.319 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F* 82 /- /- /53 /12 /21 Wind reactions based on MWFRS F Brg Wid = 101 Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375#

Lumber

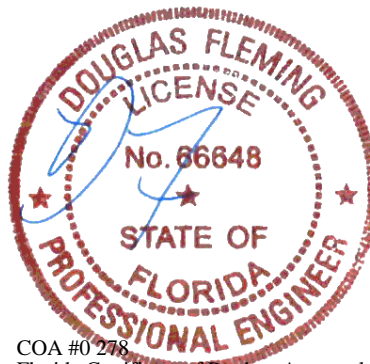
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

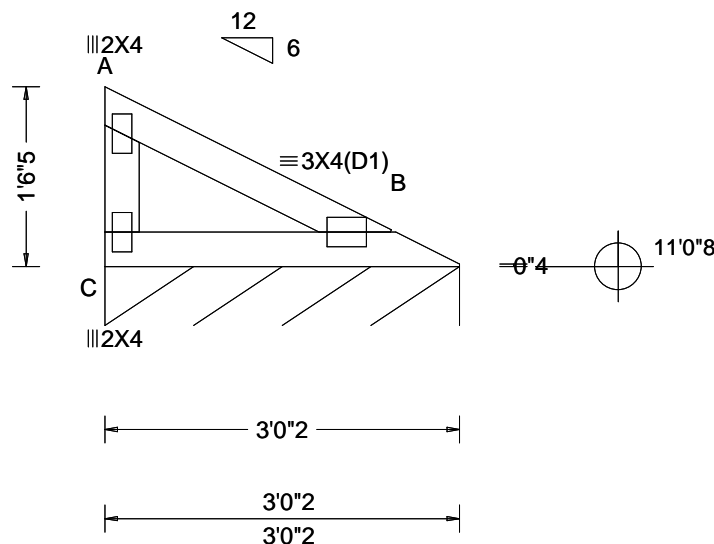


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SEQN: 26489 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V14	Cust: R 215 JRef: 1Y0W2150008 T8 DrwNo: 176.24.1004.16837 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 B 999 240 VERT(CL): 0.009 B 999 180 HORZ(LL): -0.002 B - - HORZ(TL): 0.003 B - - Creep Factor: 2.0 Max TC CSI: 0.099 Max BC CSI: 0.087 Max Web CSI: 0.051 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 82 /- /- /48 /1 /10 Wind reactions based on MWFRS C Brg Wid = 36.1 Min Req = - Bearing C is a rigid surface. Members not listed have forces less than 375#

Lumber

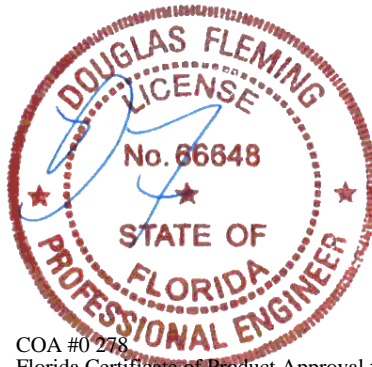
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

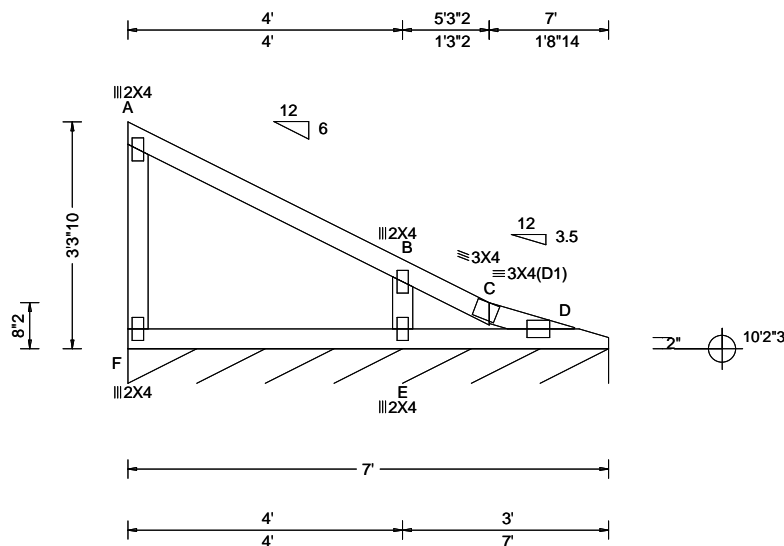


COA #0'248
Florida Certificate of Product Approval #FL1999
06/24/2024

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Glenview, IL 60025

SEQN: 34716 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 24-1285 Hedrick Truss Label: V15	Cust: R 215 JRef: 1Y0W2150008 T14 DrwNo: 176.24.1004.18633 NW / DF 06/24/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.002 D 999 180 HORZ(LL): 0.001 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.198 Max BC CSI: 0.117 Max Web CSI: 0.091 VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 82 /- /- /51 /3 /11 Wind reactions based on MWFRS D Brg Wid = 84.0 Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375#

Lumber

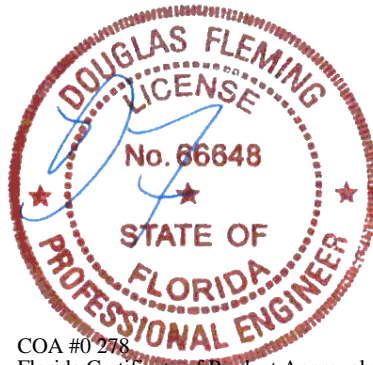
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



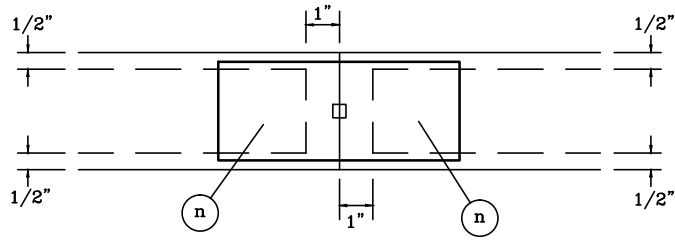
COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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TRULOX INFORMATION DETAIL

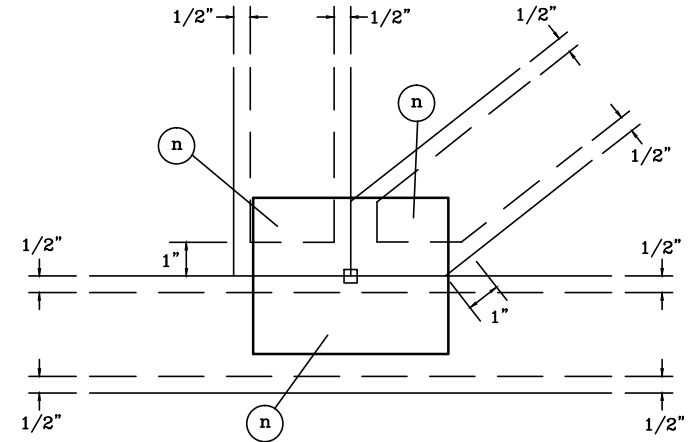
TYPICAL OFF PANEL SPLICE



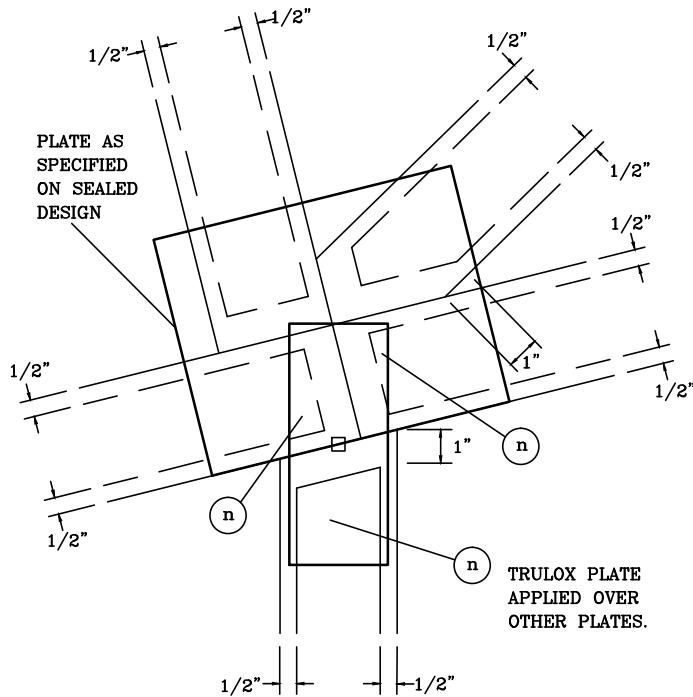
DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LUMBER.

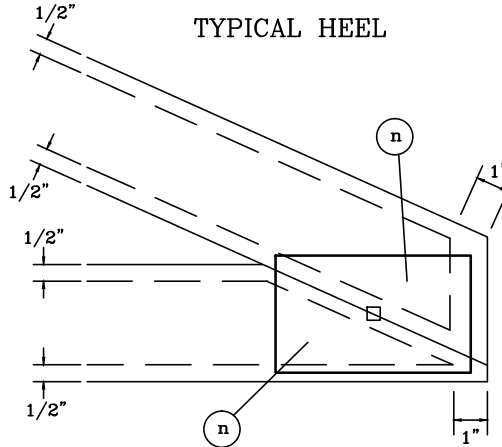
TYPICAL PANEL POINT WITHOUT SPLICE



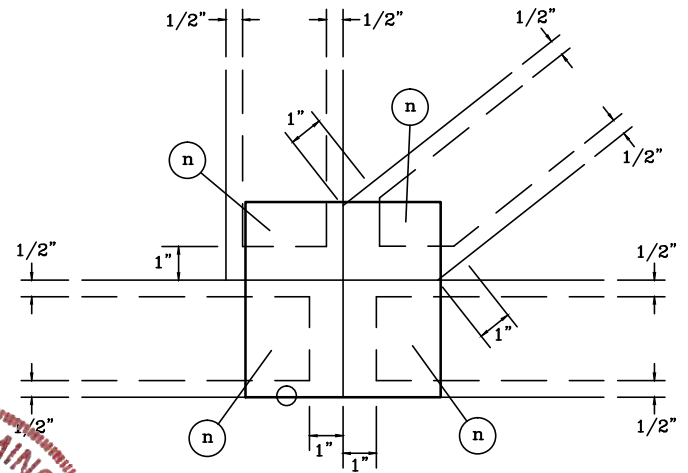
TYPICAL FILLER



TYPICAL HEEL



TYPICAL PANEL POINT SPLICE

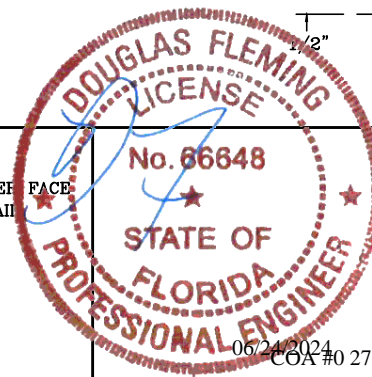


NOTES:

- (n) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.
- LOCATES PLATE CORNER OR FLUSH EDGE.
- LOCATES PLATE CENTER.



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TRULOX PLATING

160
TL

PAGE 1 OF 1

DATE 10/01/14

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

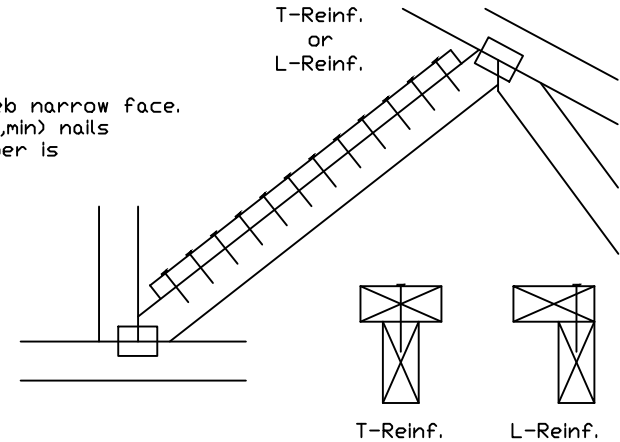
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x6(✕)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(✕)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(✕) Center scab on wide face of web. Apply (1) scab to each face of web.

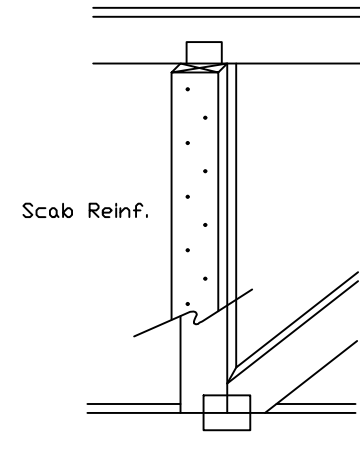
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



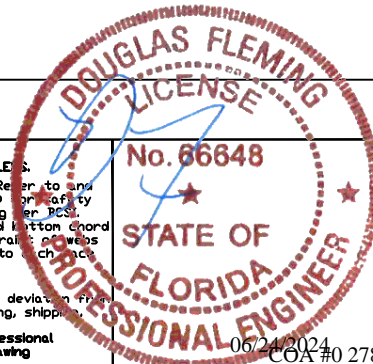
Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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TC LL	PSF	REF	CLR Subst.
TC DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

Commentary: Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

L = Span of Truss (inches)
D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

Truss Type	L/D	Deflection Limits	
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0.75" (horizontal)	1.25" (horizontal)

Truss Type	Recommended Camber
Pitched Trusses	1.00 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	1.5 x Vertical Deflection from Actual Dead Load
Floor Trusses	(0.25 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)

Note: The actual dead load may be considerably less than the design dead load.

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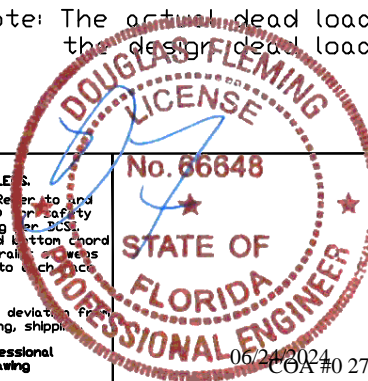
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



REF	DEFLEC/CAMB
DATE	10/01/14
DRWG	DEFLCAMB1014

Piggyback Detail - ASCE 7-22: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

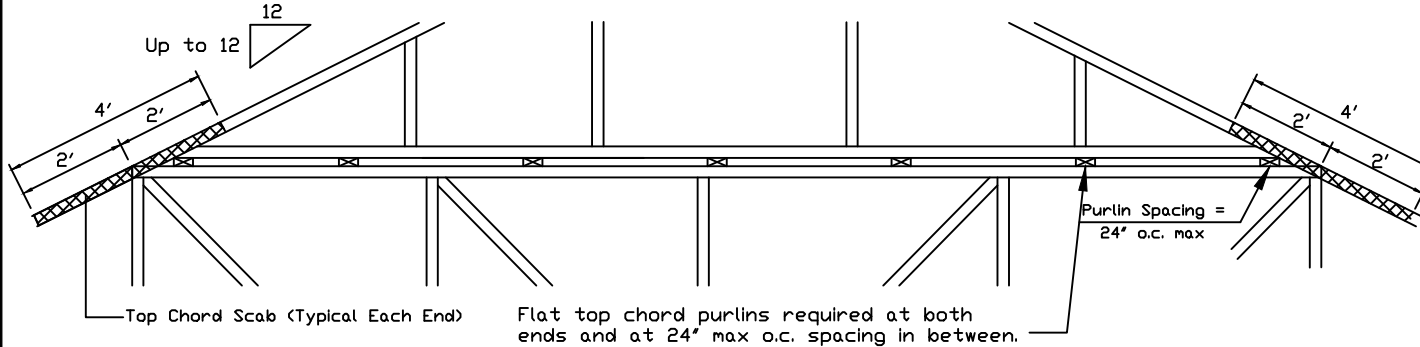
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building designer shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. Detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

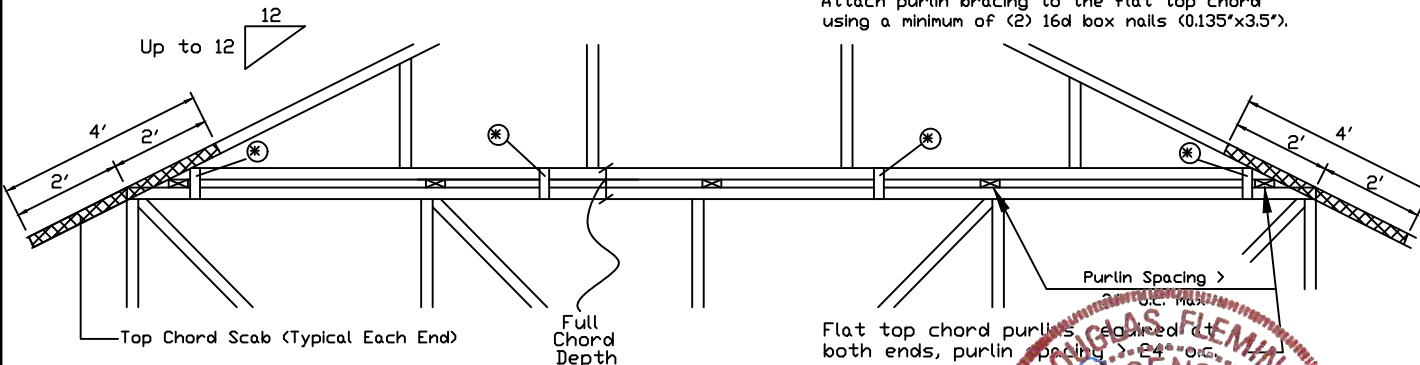


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

* In addition, provide connection with one of the following methods:

Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.
2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.
28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



REF PIGGYBACK
DATE 07/03/2023
DRWG PB160220723

SPACING 24.0"

Valley Detail - ASCE 7-22: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

**** Attach each valley to every supporting truss with:**
 535# connection or with (1) Simpson H2.5A or
 equivalent connector for
 ASCE 7-22 180 mph. 30' Mean Height, Part. Enc.
 Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-22 160 mph. 30' Mean Height, Part. Enc.
 Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut
 as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

Or

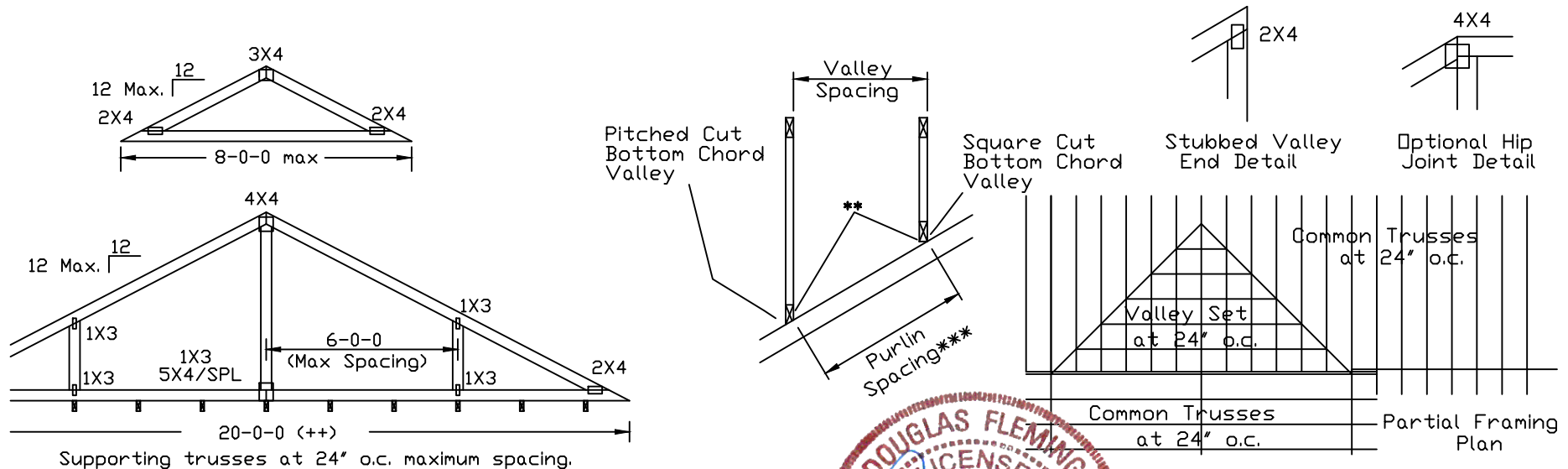
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

Or

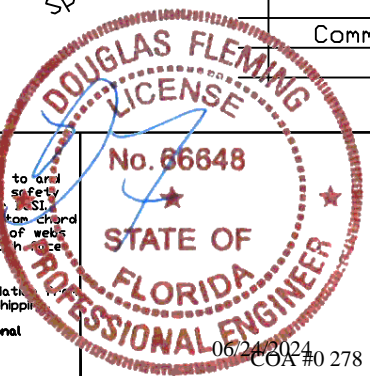
By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

***** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.**

**++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".**



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 A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
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 ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	07/03/2023
BC DL	10	10	10 PSF	DRWG	VAL180220723
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.1.25/1.33	1.15	1.15			
SPACING	24.0"				

Valley Detail - ASCE 7-22: 30' Mean Height, Enclosed, Exp. C, $K_{zt}=1.00$

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

*** Attach each valley to every supporting truss with:
(2) 16d box (0.135" x 3.5") nails toe-nailed for
ASCE 7-22, 30' Mean Height, Enclosed Building, Exp. C,
Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
supporting truss material at connection location:
140 mph for SP (G = 0.55, min.),
125 mph for DF-L (G = 0.50, min.), or
105 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

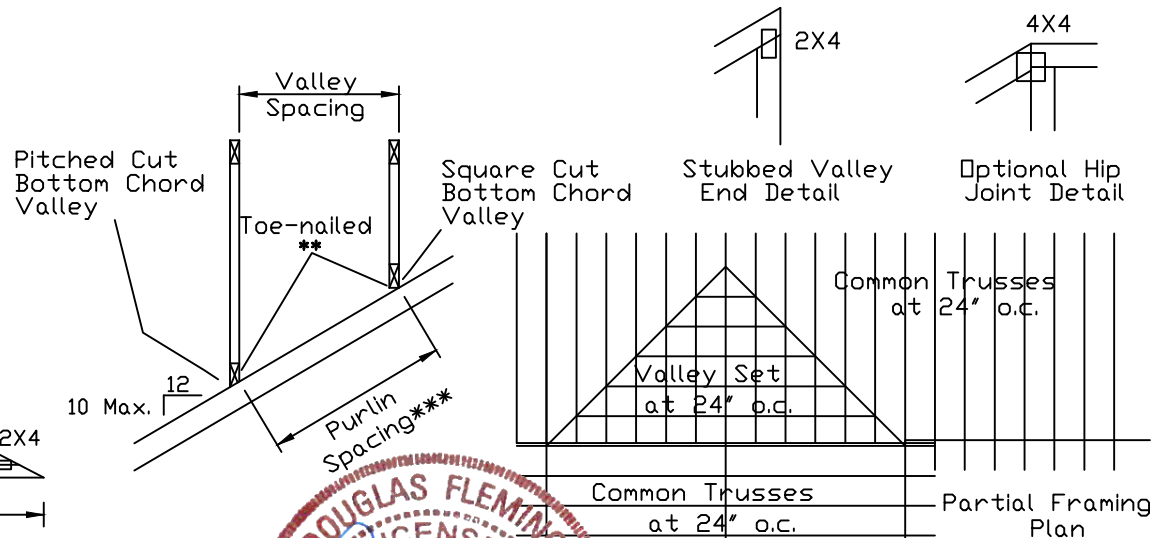
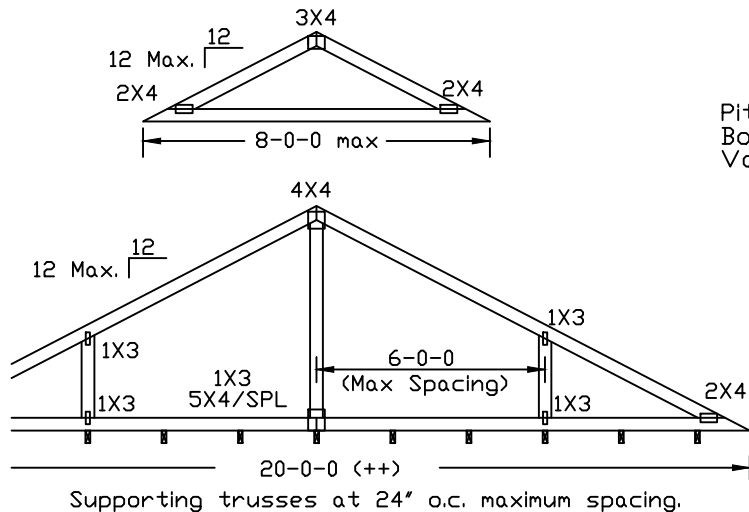
Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.

Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	07/03/2023
BC DL	10	10	10 PSF	DRWG	VALTN220723
BC LL	0	0	0 PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING		24.0"			

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

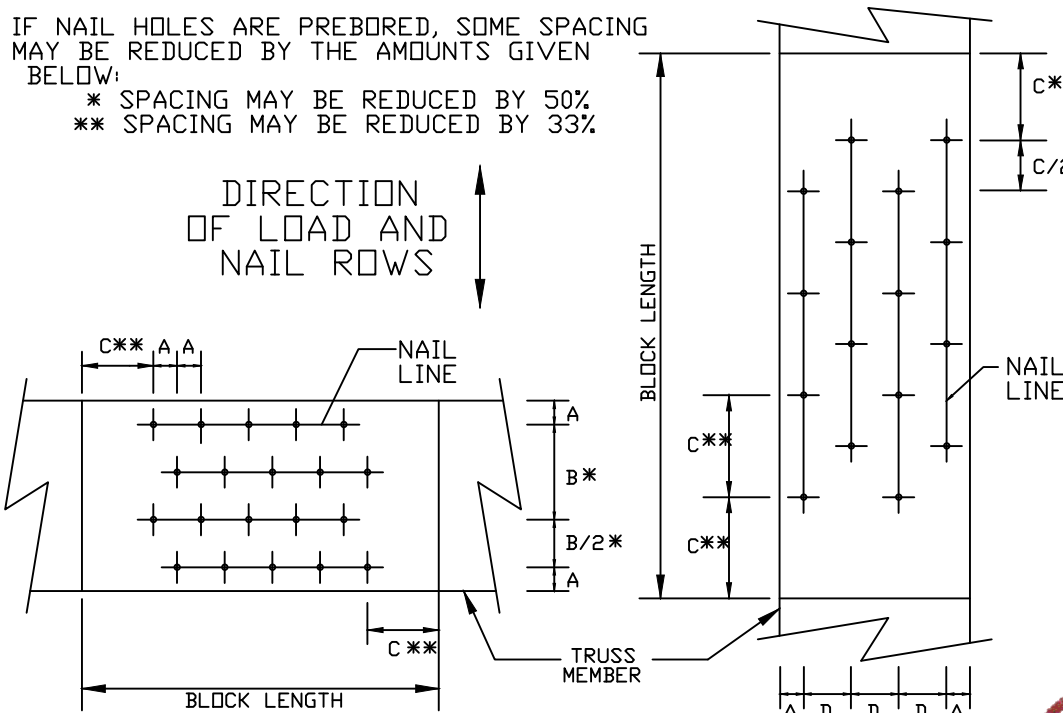
IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

* SPACING MAY BE REDUCED BY 50%

** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN

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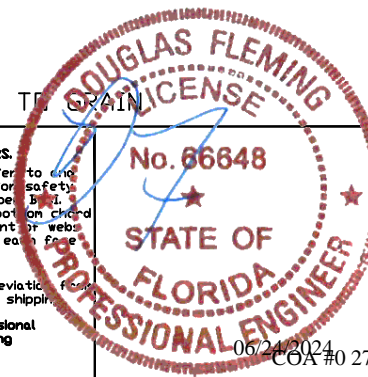
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



REF NAIL SPACE
 DATE 10/01/14
 DRWG CNNAILSP1014