

# Columbia County Building Permit Application

ck# 3499 clc 3500  
(W) Revised 9-23-04

For Office Use Only Application # 0602-99 Date Received 2/28/06 By GT Permit # 1025/24296  
 Application Approved by - Zoning Official BLK Date 08.03.06 Plans Examiner OK JTH Date 3-23-06  
 Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3  
 Comments not on file

Applicants Name RONALD R & MARCIA A OLSZAK Phone 386 454-8450  
 Address P.O. BOX 2277 HIGH SPRINGS, FL 32655  
 Owners Name RONALD R & MARCIA A. OLSZAK Phone 386 454-8450  
 911 Address 200 SW BAY PL Ft. White FL 32038  
 Contractors Name OWNER/BUILDER Phone ABOVE  
 Address ABOVE  
 Fee Simple Owner Name & Address N/A  
 Bonding Co. Name & Address N/A  
 Architect/Engineer Name & Address RIVERA DESIGN GROUP, 1217 NW 16<sup>th</sup> AVE, GAINESVILLE FL 32611  
 Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy  
 Property ID Number 3D-75-17-10058-594 Estimated Cost of Construction 160K  
 Subdivision Name SANTE FE RIVER PLANTATION Lot 4 Block      Unit      Phase       
 Driving Directions FROM HIGH SPRINGS TAKE 27 N TO CR 138, TURN LEFT ON CR 138 TO 1ST ST. ON LEFT, HEFLIN AVE. GO TO 1ST ST ON RT. SW BAY PL TURN RT ON BAY TO 4<sup>th</sup> LOT ON LEFT.  
 Type of Construction SINGLE FAMILY Number of Existing Dwellings on Property 0  
 Total Acreage 2.24 Lot Size 180'9" x 541'47" Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive  
 Actual Distance of Structure from Property Lines - Front 185'8" Side 51' Side 74' Rear 232'  
 Total Building Height 22' Number of Stories 1 Heated Floor Area 2087 Roof Pitch 7/12  
Total 3273

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Marcia A. Olszak  
Ronald R. Olszak  
 Owner Builder or Agent (Including Contractor)


STATE OF FLORIDA  
 COUNTY OF COLUMBIA Alachua

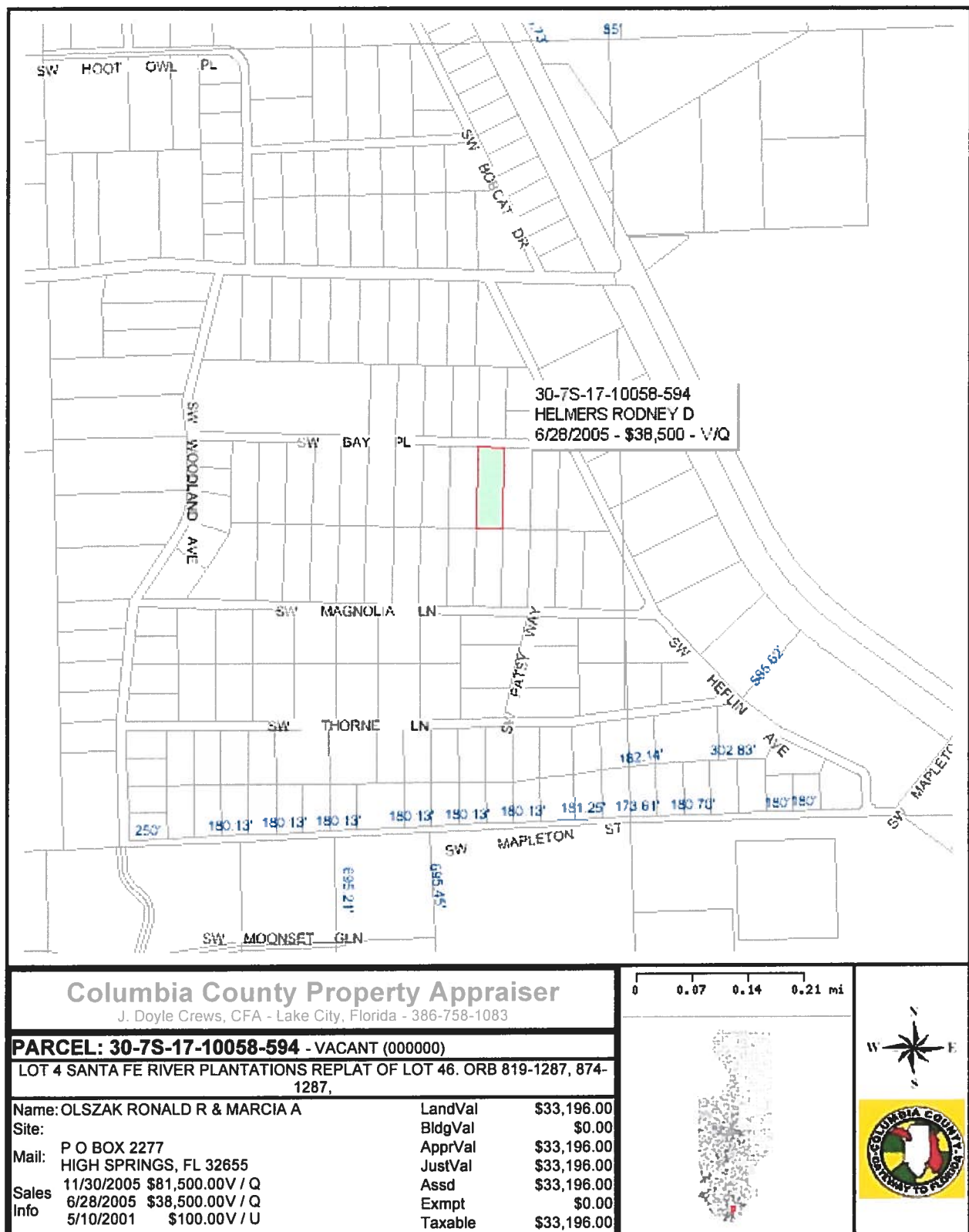
Sworn to (or affirmed) and subscribed before me  
 this 27<sup>th</sup> day of February 2006.  
 Personally known      or Produced Identification ✓

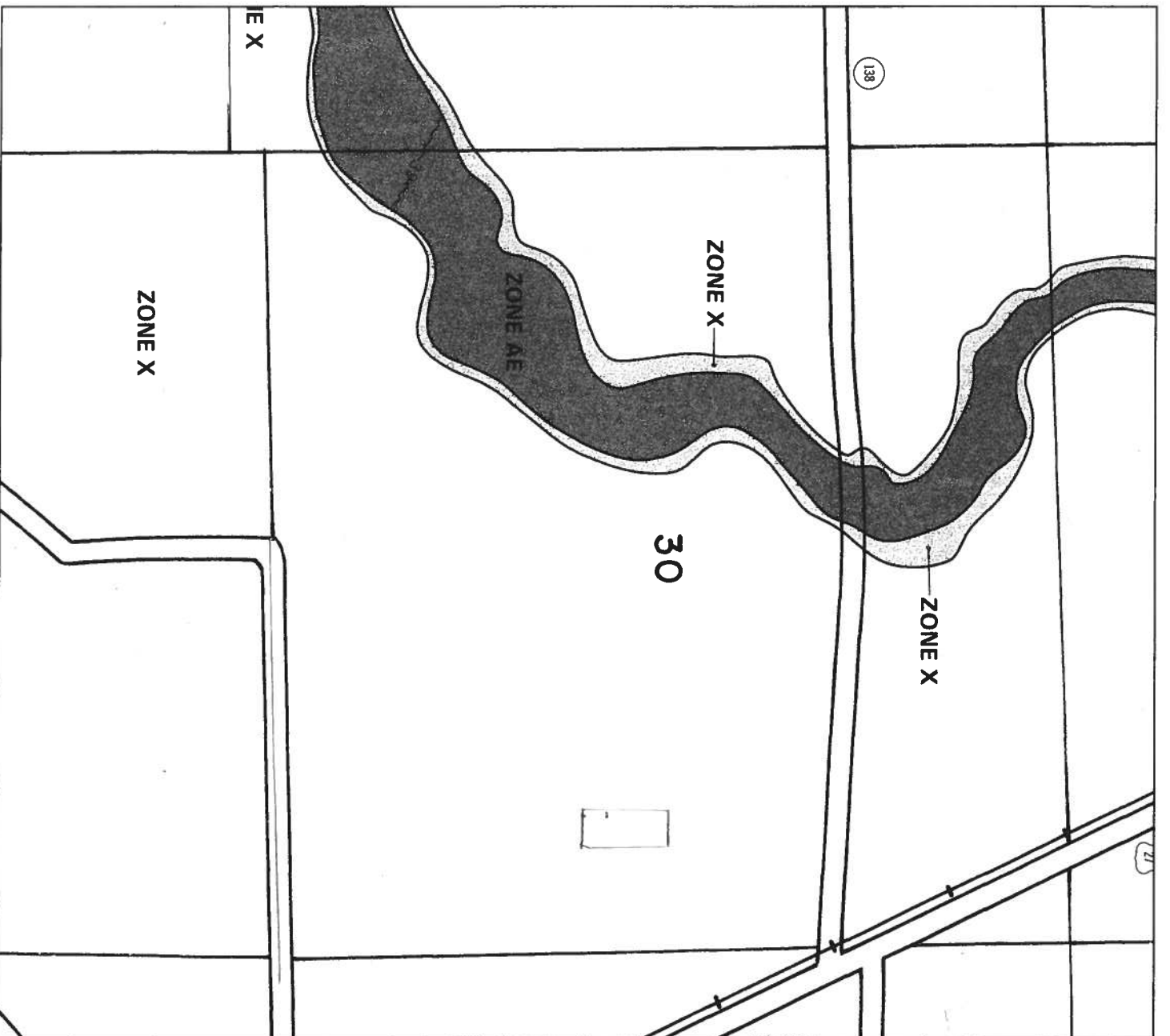
FL Driver's License

Contractor Signature       
 Contractors License Number       
 Competency Card Number       
 NOTARY STAMP/SEAL

Carol D. Short  
 Notary Signature

 **CAROL D. SHORT**  
 Notary Public, State of Florida  
 My comm. expires Nov. 19, 2007  
 No. DD 269014





APPROXIMATE SCALE IN FEET



NATIONAL FLOOD INSURANCE PROGRAM

# **FIRM** FLOOD INSURANCE RATE MAP

COLUMBIA  
COUNTY,  
FLORIDA  
(UNINCORPORATED AREAS)

PANEL 270 OF 290

PANEL LOCATION



COMMUNITY-PANEL NUMBER  
120070 0270 B  
EFFECTIVE DATE:  
JANUARY 6, 1988



Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Further information about National Flood Insurance Program flood hazard maps is available at [www.fema.gov/nfliscd](http://www.fema.gov/nfliscd).

Inst: 2005029922 Date: 12/02/2005 Time: 13:53  
Doc Stamp-Deed : 570.50  
ML DC, P. Dewitt Cason, Columbia County B: 1066 P: 2451

Prepared by  
Deborah Bissell, an employee of  
First American Title Insurance Company  
23335 NW County Road 236, Suite 10  
High Springs, Florida 32643  
(386)454-2727

Return to: Grantee

File No.: 1095-1022371

## **WARRANTY DEED**

This indenture made on **November 30, 2005 A.D.**, by

**Rodney D. Helmers**

whose address is: **P.O. Box 1173, High Springs, FL 32655**  
hereinafter called the "grantor", to

**Ronald R. Olszak and Marcia A. Olszak, husband and wife**

whose address is: **P.O. Box 2277, High Springs, FL 32655**  
hereinafter called the "grantee":

(Which terms "Grantor" and "Grantee" shall include singular or plural, corporation or individual, and either sex, and shall include heirs, legal representatives, successors and assigns of the same)

**Witnesseth**, that the grantor, for and in consideration of the sum of Ten Dollars, (\$10.00) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in **Columbia County, Florida**, to-wit:

Lot 4, of a Replat of Lots 38, 45, and 46, of SANTA FE RIVER PLANTATIONS, according to the Plat thereof as recorded in Plat Book 5, Page(s) 13, of the Public Records of Columbia County, Florida.

Parcel Identification Number: **R10058-594**

**The land** is not the homestead of the Grantor under the laws and constitution of the State of Florida and neither the Grantor nor any person(s) for whose support the Grantor is responsible reside on or adjacent to the land.

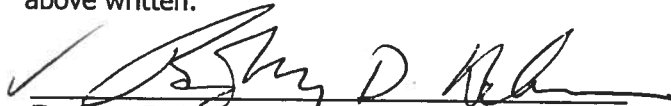
**Subject to** all reservations, covenants, conditions, restrictions and easements of record and to all applicable zoning ordinances and/or restrictions imposed by governmental authorities, if any.

**Together** with all the tenements, hereditaments and appurtenances thereto belonging or in any way appertaining.

**To Have and to Hold**, the same in fee simple forever.

**And** the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances except taxes accruing subsequent to December 31st of 2004.

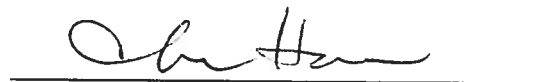
**In Witness Whereof**, the grantor has hereunto set their hand(s) and seal(s) the day and year first above written.

  
Rodney D. Helmers

*Signed, sealed and delivered in our presence:*

  
Witness Signature  
**DEBORAH BISSELL**

Print Name: \_\_\_\_\_

  
Witness Signature

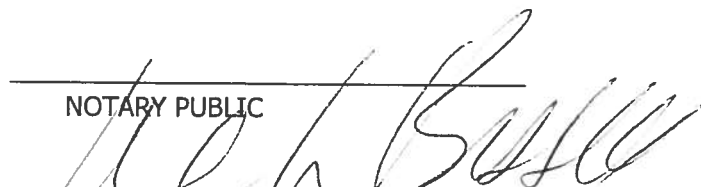
Print Name: Christopher Hauer

State of **FL**

County of **Alachua**

**The Foregoing Instrument Was Acknowledged** before me on **November 30, 2005**, by **Rodney D. Helmers** who is/are personally known to me or who has/have produced a valid driver's license as identification.



  
NOTARY PUBLIC  
Notary Print Name  
My Commission Expires: \_\_\_\_\_

**Columbia County Building Department  
Culvert Waiver Permit / Application**

**Waiver No.**

APPLICANT RONALD R. & MARCIA A. OLSZEK PHONE 386-454-8450

ADDRESS P.O. BOX 2277 HIGH SPRINGS, FL 32655

OWNER RONALD R. & MARCIA A. OLSZEK PHONE 386-454-8450

ADDRESS P.O. BOX 2277 HIGH SPRINGS, FL 32655

CONTRACTOR OWNER/BUILDER PHONE ABOVE

LOCATION OF PROPERTY 200 SW BAY PL

PARCEL ID # 30-75-17-10058-594

SUBDIVISION (Lot/Block/Phase/Unit) SANTA FE RIVER PLANTATION, LOT 4

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNED: Ronald R. Olszek DATE: Jul 23, 2006

FEE: \$ 50.00 A SEPARATE CHECK IS REQUIRED.  
MAKE CHECKS PAYABLE TO BCC.

**Public Works Department Use Only**

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINE THAT THE CULVERT WAIVER IS:

\_\_\_\_\_ APPROVED \_\_\_\_\_ NOT APPROVED – NEEDS A CULVERT PERMIT

COMMENTS \_\_\_\_\_

SIGNED: \_\_\_\_\_ DATE: \_\_\_\_\_

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21, Lake City, FL 32055  
Phone: 386-758-1008 Fax: 386-758-2160

# COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787

PHONE: (386) 758-1125 \* FAX: (386) 758-1365 \* Email: ron\_croft@columbiacountyfla.com

## Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

**DATE REQUESTED:** 1/19/2006      **DATE ISSUED:** 1/23/2006

### ENHANCED 9-1-1 ADDRESS:

200      SW      BAY      PL

FORT WHITE      FL      32038

### PROPERTY APPRAISER PARCEL NUMBER:

30-7S-17-10058-594

### Remarks:

LOT 4 SANTA FE RIVER PLANTATIONS REPLAT OF LOT 46

Address Issued By: 

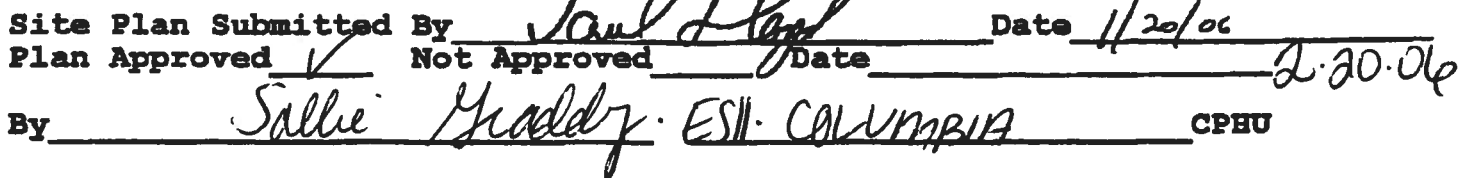
Columbia County 9-1-1 Addressing / GIS Department

**NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.**

016-0152N

OLSZAK/CR 05-3211

345' to well across road



**Notes:** \_\_\_\_\_



NOTICE OF COMMENCEMENT FORM  
COLUMBIA COUNTY, FLORIDA

\*\*\*THIS DOCUMENT MUST BE RECORDED AT THE COUNTY  
CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.\*\*\*

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 30-75-17-10058-594

1. Description of property: (legal description of the property and street address or 911 address)

LOT 4, OF A REPLAT OF LOTS 38, 45, AND 46, OF SANTA  
FE RIVER PLANTATIONS, ACCORDING TO THE PLAT THEREOF  
AS RECORDED IN PLAT BOOKS, PAGE(S) 13, OF PUBLIC  
RECORDS OF COLUMBIA COUNTY, FLORIDA

2. General description of improvement: NEW CONSTRUCTION SINGLE FAMILY

3. Owner Name & Address RONALD R. & MARCIA A. OLSZAK  
P.O. BOX 2277 HIGH SPRINGS, FL 32655 Interest in Property OWNERS

4. Name & Address of Fee Simple Owner (if other than owner):

5. Contractor Name OWNER/BUILDER Phone Number #3 ABOVE  
Address #3 ABOVE

6. Surety Holders Name N/A Phone Number  
Address  
Amount of Bond

7. Lender Name N/A Phone Number  
Address

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name Phone Number  
Address

9. In addition to himself/herself the owner designates  
to receive a copy of the Lienor's Notice as provided in Section 713.13 (1)(a) 7. Phone Number of the designee

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording (Unless a different date is specified)

**NOTICE AS PER CHAPTER 713, Florida Statutes:**

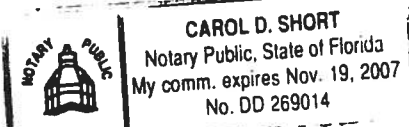
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Marcia A. Olszak  
Ronald R. Olszak  
Signature of Owner

Sworn to (or affirmed) and subscribed before me on day of 27th Feb, 2006

NOTARY STAMP/SEAL

Carol D. Short  
Signature of Notary



Inst: 2006004826 Date: 02/28/2006 Time: 14:36  
J.F. DC, P. Dewitt Cason, Columbia County B: 1075 P: 1290

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

## Florida Department of Community Affairs Residential Whole Building Performance Method A

Project Name: **Olszak Res.**  
Address:  
City, State: ,  
Owner:  
Climate Zone: **North**

Builder: owner  
Permitting Office: Columbia Co.  
Permit Number: 24296  
Jurisdiction Number: 221000

1. New construction or existing	New	___	12. Cooling systems		
2. Single family or multi-family	Single family	___	a. Central Unit	Cap: 42.0 kBtu/hr	___
3. Number of units, if multi-family	1	___		SEER: 14.00	___
4. Number of Bedrooms	3	___	b. N/A		___
5. Is this a worst case?	Yes	___	c. N/A		___
6. Conditioned floor area (ft²)	2087 ft²	___			___
7. Glass type <sup>1</sup> and area: (Label reqd. by 13-104.4.5 if not default)		___	13. Heating systems		
a. U-factor:	Description Area	___	a. Electric Heat Pump	Cap: 42.0 kBtu/hr	___
(or Single or Double DEFAULT)	7a. (Dble Default) 264.0 ft²	___		HSPF: 9.00	___
b. SHGC:		___	b. N/A		___
(or Clear or Tint DEFAULT)	7b. (Clear) 264.0 ft²	___	c. N/A		___
8. Floor types		___	14. Hot water systems		
a. Slab-On-Grade Edge Insulation	R=0.0, 198.0(p) ft	___	a. Electric Resistance	Cap: 40.0 gallons	___
b. N/A		___		EF: 0.90	___
c. N/A		___	b. N/A		___
9. Wall types		___	c. Conservation credits		___
a. Frame, Wood, Exterior	R=13.0, 1447.0 ft²	___	(HR-Heat recovery, Solar		___
b. Frame, Wood, Adjacent	R=13.0, 192.0 ft²	___	DHP-Dedicated heat pump)		___
c. N/A		___	15. HVAC credits		___
d. N/A		___	(CF-Ceiling fan, CV-Cross ventilation,		___
e. N/A		___	HF-Whole house fan,		___
10. Ceiling types		___	PT-Programmable Thermostat,		___
a. Under Attic	R=30.0, 1152.0 ft²	___	MZ-C-Multizone cooling,		___
b. Under Attic	R=30.0, 982.0 ft²	___	MZ-H-Multizone heating)		___
c. Under Attic	R=19.0, 272.0 ft²	___			___
11. Ducts		___			___
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 186.0 ft	___			___
b. N/A		___			___

Glass/Floor Area: 0.13

Total as-built points: 26113

Total base points: 30061

# PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: RD

DATE: 2-21-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: \_\_\_\_\_

DATE: \_\_\_\_\_

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: \_\_\_\_\_

DATE: \_\_\_\_\_



<sup>1</sup> Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
<b>GLASS TYPES</b>											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points				
.18	2087.0	20.04	7528.2	Double, Clear	W	0.0	0.0	82.0	38.52	1.00	3159.0
				Double, Clear	N	0.0	0.0	38.0	19.20	1.00	729.6
				Double, Clear	E	0.0	0.0	118.0	42.06	1.00	4963.5
				Double, Clear	S	0.0	0.0	6.0	35.87	1.00	215.2
				Double, Clear	NE	0.0	0.0	10.0	29.56	1.00	295.6
				Double, Clear	W	0.0	0.0	10.0	38.52	1.00	385.2
				<b>As-Built Total:</b>		<b>264.0</b>			<b>9748.1</b>		
<b>WALL TYPES</b> Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	192.0	0.70	134.4	Frame, Wood, Exterior	13.0		1447.0 1.50 2170.5				
Exterior	1447.0	1.70	2459.9	Frame, Wood, Adjacent	13.0		192.0 0.60 115.2				
<b>Base Total:</b> 1639.0 2594.3				<b>As-Built Total:</b>		<b>1639.0 2285.7</b>					
<b>DOOR TYPES</b> Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	18.0	2.40	43.2	Exterior Insulated			36.0 4.10 147.6				
Exterior	36.0	6.10	219.6	Adjacent Insulated			18.0 1.60 28.8				
<b>Base Total:</b> 54.0 262.8				<b>As-Built Total:</b>		<b>54.0 176.4</b>					
<b>CEILING TYPES</b> Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points				
Under Attic	2087.0	1.73	3610.5	Under Attic	30.0		1152.0 1.73 X 1.00 1993.0				
				Under Attic	30.0		982.0 1.73 X 1.00 1698.9				
				Under Attic	19.0		272.0 2.34 X 1.00 636.5				
<b>Base Total:</b> 2087.0 3610.5				<b>As-Built Total:</b>		<b>2406.0 4328.3</b>					
<b>FLOOR TYPES</b> Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Slab	198.0(p)	-37.0	-7326.0	Slab-On-Grade Edge Insulation	0.0		198.0(p) -41.20 -8157.6				
Raised	0.0	0.00	0.0								
<b>Base Total:</b> -7326.0				<b>As-Built Total:</b>		<b>198.0 -8157.6</b>					
<b>INFILTRATION</b> Area X BSPM = Points				Area X SPM = Points							
2087.0 10.21 21308.3				2087.0 10.21 21308.3							

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
<b>Summer Base Points: 27978.1</b>				<b>Summer As-Built Points: 29689.1</b>						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
27978.1	0.4266		11935.5	(sys 1: Central Unit 42000 btuh ,SEER/EFF(14.0) Ducts:Unc(S),Unc(R),Gar(AH),R6 0(INS) 29689 1.00 (1.09 x 1.147 x 1.00) 0.244 1.000 9048.9 <b>29689.1 1.00 1.250 0.244 1.000 9048.9</b>						

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
<b>GLASS TYPES</b>											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	2087.0	12.74	4785.9	Double, Clear	W	0.0	0.0	82.0	20.73	1.00	1699.7
				Double, Clear	N	0.0	0.0	38.0	24.58	1.00	934.0
				Double, Clear	E	0.0	0.0	118.0	18.79	1.00	2217.6
				Double, Clear	S	0.0	0.0	6.0	13.30	1.00	79.8
				Double, Clear	NE	0.0	0.0	10.0	23.57	1.00	235.7
				Double, Clear	W	0.0	0.0	10.0	20.73	1.00	207.3
				<b>As-Built Total:</b>				<b>264.0</b>	<b>5374.0</b>		
<b>WALL TYPES</b> Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	192.0	3.60	691.2	Frame, Wood, Exterior	13.0		1447.0	3.40	4919.8		
Exterior	1447.0	3.70	5353.9	Frame, Wood, Adjacent	13.0		192.0	3.30	633.6		
<b>Base Total:</b> 1639.0 6045.1				<b>As-Built Total:</b>		1639.0		5553.4			
<b>DOOR TYPES</b> Area X BWPM = Points				Type			Area X WPM = Points				
Adjacent	18.0	11.50	207.0	Exterior Insulated			36.0	8.40	302.4		
Exterior	36.0	12.30	442.8	Adjacent Insulated			18.0	8.00	144.0		
<b>Base Total:</b> 54.0 649.8				<b>As-Built Total:</b>		54.0		446.4			
<b>CEILING TYPES</b> Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	2087.0	2.05	4278.3	Under Attic	30.0		1152.0	2.05 X 1.00	2361.6		
				Under Attic	30.0		982.0	2.05 X 1.00	2013.1		
				Under Attic	19.0		272.0	2.70 X 1.00	734.4		
<b>Base Total:</b> 2087.0 4278.3				<b>As-Built Total:</b>		2406.0		5109.1			
<b>FLOOR TYPES</b> Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	198.0(p)	8.9	1762.2	Slab-On-Grade Edge Insulation	0.0		198.0(p)	18.80	3722.4		
Raised	0.0	0.00	0.0								
<b>Base Total:</b> 1762.2				<b>As-Built Total:</b>		198.0		3722.4			
<b>INFILTRATION</b> Area X BWPM = Points								Area X WPM = Points			
2087.0 -0.59 -1231.3						2087.0		-0.59 -1231.3			

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
<b>Winter Base Points:</b>		<b>16290.0</b>		<b>Winter As-Built Points:</b>				<b>18974.0</b>		
Total Winter Points	X System Multiplier	= Heating Points		Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier (1.069 x 1.169 x 1.00)	X System Multiplier 0.379	X Credit Multiplier 1.000	= Heating Points 8983.9	
<b>16290.0</b>	<b>0.6274</b>	<b>10220.4</b>		(sys 1: Electric Heat Pump 42000 btuh ,EFF(9.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0 18974.0	<b>1.000</b>	<b>1.250</b>	<b>0.379</b>	<b>1.000</b>	<b>8983.9</b>	
				<b>18974.0</b>	<b>1.00</b>					

**WATER HEATING & CODE COMPLIANCE STATUS**

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
WATER HEATING										
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X Credit	= Total
3		2635.00	7905.0	40.0	0.90	3		1.00	2693.56	8080.7
				As-Built Total:						8080.7

CODE COMPLIANCE STATUS													
BASE							AS-BUILT						
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
11935		10220		7905		30061	9049		8984		8081		26113

**PASS**

# Code Compliance Checklist

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

**6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST**

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

**6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)**

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	



# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

**ESTIMATED ENERGY PERFORMANCE SCORE\* = 85.4**

**The higher the score, the more efficient the home.**

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 42.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 14.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft <sup>2</sup> )	2087 ft <sup>2</sup>		
7. Glass type <sup>1</sup> and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 42.0 kBtu/hr
(or Single or Double DEFAULT)	7a.(Dble Default) 264.0 ft <sup>2</sup>		HSPF: 9.00
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 264.0 ft <sup>2</sup>	c. N/A	
8. Floor types		14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 198.0(p) ft	a. Electric Resistance	Cap: 40.0 gallons
b. N/A			EF: 0.90
c. N/A		b. N/A	
9. Wall types		c. Conservation credits	
a. Frame, Wood, Exterior	R=13.0, 1447.0 ft <sup>2</sup>	(HR-Heat recovery, Solar	
b. Frame, Wood, Adjacent	R=13.0, 192.0 ft <sup>2</sup>	DHP-Dedicated heat pump)	
c. N/A		15. HVAC credits	
d. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
e. N/A		HF-Whole house fan,	
10. Ceiling types		PT-Programmable Thermostat,	
a. Under Attic	R=30.0, 1152.0 ft <sup>2</sup>	MZ-C-Multizone cooling,	
b. Under Attic	R=30.0, 982.0 ft <sup>2</sup>	MZ-H-Multizone heating)	
c. Under Attic	R=19.0, 272.0 ft <sup>2</sup>		
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 186.0 ft		
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: \_\_\_\_\_ Date: \_\_\_\_\_

Address of New Home: \_\_\_\_\_ City/FL Zip: \_\_\_\_\_



*\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at [www.fsec.ucf.edu](http://www.fsec.ucf.edu) for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

<sup>1</sup> Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.  
EnergyGauge® (Version: FLRCSB v4.0)

# GEO-TECH, INC.

ENGINEERING CONSULTANTS IN GEOTECHNICAL • ENVIRONMENTAL • CONSTRUCTION MATERIALS TESTING

March 17, 2006  
Project No. 062726.01G

Mr. & Mrs. Olszak  
P.O. Box 2277  
High Springs, FL 32655

Attention: Mr. & Mrs. Olszak


Project: Proposed Residence and Shed, Santa Fe River Plantation, Columbia County, Florida  
**Soil Bearing Capacity**

Dear Mr. & Mrs. Olszak:

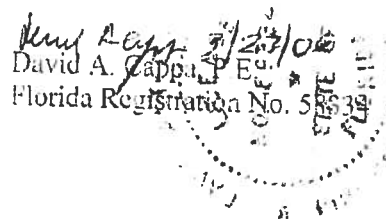
As requested, Geo-Technologies, Inc. (Geo-Tech) has visited the above referenced project site. The purpose of our visit was to perform penetrometer readings in the area that the proposed residence and shed will be placed based on Hand Penetrometer (HP) results in the native soils. Six (6) auger borings with penetrometer readings were performed to four (4) feet below site grade. Based on the results of the penetrometer readings, the maximum allowable soil bearing pressures found at these locations are approximately 2,000 pounds per square foot based

Geo-Technologies, Inc. (Geo-Tech) trust this report is sufficient to meet your immediate needs. Should you have any questions concerning this report or if we may be of further assistance, please do not hesitate to contact the undersigned.

Sincerely, w

  
Donald "Bubba" Youngblood  
Branch Manager

DY/DC: kw

  
David A. Cappa, P.E.  
Florida Registration No. 58339

*Hold For SOIL TEST*

From: The Columbia County Building Department  
Plans Review  
135 NE Hernando Av.  
P. O Box 1529  
Lake City Florida, 32056-1529

**0602-99**

Reference to: Build permit application Number:

Ronald & Marcia Olszak Owner/Builders of lot 4 Santa Fe Plantations

On the date of March 8, 2006 application 0602-99 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

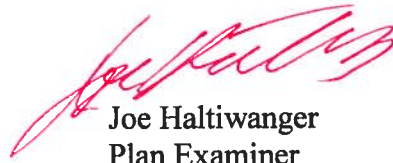
**Please include application number 0602-99 when making reference to this application.**

- ✓ 1. Please have Mr. Thomas Sputo the structural designer show on the plans the required load bearing capacities of the soils to provide adequate support for the foundations.
- ✓ 2. The elevation drawing submitted show that the roof pitch will be at a 6/12 pitch. The submitted engineered truss plans show the truss system to be a 7/12 pitch. Please confirm that the submitted engineered truss plans are of the correct roof pitch.
- ✓ 3. Please verify compliance with the FRC-2004 section R308.4 Hazardous locations: Glazing in doors and enclosures for hot tubs, whirlpools, saunas, steam rooms, bathtubs and showers. Glazing in any part of a building wall enclosing these compartments where the bottom exposed edge of the glazing is less than 60 inches (1524 mm) measured vertically above any standing or walking surface.

4. Please have Mr. Thomas Sputo show on the plans the load bearing supporting headers which will support all the opening (Garage doors, bay windows, doors and supporting beams for the porches.
5. On the electrical plan show the location of the electrical panel and include the total amperage rating of the electrical service panel also show the overcurrent protection device which shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.
6. Please show compliance with the FRC-2004 section R602.8 Fireblocking shall be provided to cut off all concealed draft openings (both vertical and horizontal) and to form an effective fire barrier between stories, and between a top story and the roof space. Fireblocking shall be provided in wood-frame construction in the following locations.
  1. In concealed spaces of stud walls and partitions, including furred spaces and parallel rows of studs or staggered studs; as follows:
    - 1.1. Vertically at the ceiling and floor levels.
    - 1.2. Horizontally at intervals not exceeding 10 feet (3048 mm).
  2. At all interconnections between concealed vertical and horizontal spaces such as occur at soffits, drop ceilings and cove ceilings.
  3. In concealed spaces between stairs stringers at the top and bottom of the run. Enclosed spaces under stairs shall comply with Section R311.2.2.

4. At openings around vents, pipes, and ducts at ceiling and floor level, with an approved material to resist the free passage of flame and products of combustion.
7. The air handler unit location as shown on the plans will be suspended from the bottom cords of roof truss system. Please provide an approval letter from the truss manufacture that the truss design is of adequate strength to support the suspension of the air handler unit.

Thank you,



Joe Haltiwanger  
Plan Examiner  
Columbia County Building Department

phone # 386-454-8450

The Columbia County Building Department  
Plans Review  
136 NE Hernando Av.  
P. O. Box 1529  
Lake City Florida, 32056-1529

0602-99

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Thank you,

Joe Haltiwanger  
Plan Examiner  
Columbia County Building Department



**WIND RESISTANCE ENGINEERING  
CALCULATIONS FOR  
OLSZAK RESIDENCE  
200 SW Bay PI  
Ft. White, FL**

HOUSE

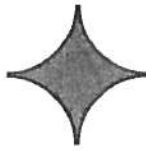
Revised - 9 Mar 06  
per Bldg Dept Comments

DESIGNED IN ACCORDANCE WITH  
REQUIREMENTS OF 2004 FLORIDA BUILDING CODE,  
SECTION 1609 FOR 110 MPH WIND SPEED



THOMAS SPUTO, PH.D., P.E.  
PE 39142

**SPUTO AND LAMMERT ENGINEERING, LLC  
STRUCTURAL ENGINEERS  
10 SW 1st AVENUE, GAINESVILLE, FLORIDA 32601  
(352) 378-0448  
CA 6855**



# SPUTO AND LAMMERT ENGINEERING, LLC

## STRUCTURAL ENGINEERS

10 SW 1<sup>ST</sup> AVENUE, GAINESVILLE, FL 32601

PHONE: 352-378-0448 FAX: 352-373-1331

E-MAIL: sputoandlammert@mindspring.com

## HOUSE

Wind resistance of the referenced building has been designed using a wind speed of 110 mph as required by Section 1609, 2004 Florida Building Code.

ROOF SHEATHING: 1/2" Plywood or 7/16" OSB, installed without blocking. Use 8d common or 10-1/4 gage x 2" minimum length power nails at 6" o.c. at sheet edges and 12" o.c. in the sheet field. The roof acts as a structural diaphragm.

WALL SHEATHING: 1/2" Plywood or 7/16" OSB, installed with blocking at all horizontal sheet edges. Sheathing is installed from bottom to top plate to provide a continuous load path. Use 8d common or 10-1/4 gage x 2" minimum length power nails at 4" o.c. at vertical sheet edges, 4" o.c. at horizontal sheet edges, and 12" o.c. in the sheet field.

SHEARWALLS: See plan sheet for locations.

WALL STUDS: #2 Spruce or better 2x6 at 16" o.c.

ANCHOR BOLTS: 1/2" with 3" SQUARE washer at maximum spacing of 48" o.c. Install one bolt within 6" of all corners, and within 6" of the ends of all windows and doors. (Anchor bolt alternate - 5/8" wedge anchor with 4" embed into concrete.) USE 3" SQUARE WASHERS AT EACH END OF ALL SHEARWALLS.

HURRICANE CLIPS: Sized as follows.

TRUSS	BEARING	HARDWARE
A1	2, 10	(1) Simpson H10 and (1) Simpson LSTA21
B1	2, 10	(1) Simpson H10 and (1) Simpson LSTA21
C1	2, 8	(2) Simpson H10

One Ply Truss (except trusses above): (1) Simpson H10  
Truss to Truss: Specified by truss manufacturer, IAW Wood Truss  
Council of America Standard WTCA 1-1995.

CONCRETE: All concrete shall have a 28 day compressive strength of 3000 psi.

REINFORCING STEEL: Grade 40 - #5 bars. All lap splices to be a minimum of 25 inches.

POSTS: Shall be 6x6 PT #2 Southern Pine. Post cap shall be Simpson CCQ46SDS2.5-HDG with Simpson ECCQ46SDS2.5-HDG for end conditions. Post base shall be Simpson ABU66Z. Alternate products with uplift capacity of 1225# may be substituted.

NOTE: 10-1/4 GAGE NAILS HAVE A DIAMETER OF 0.131 INCHES.

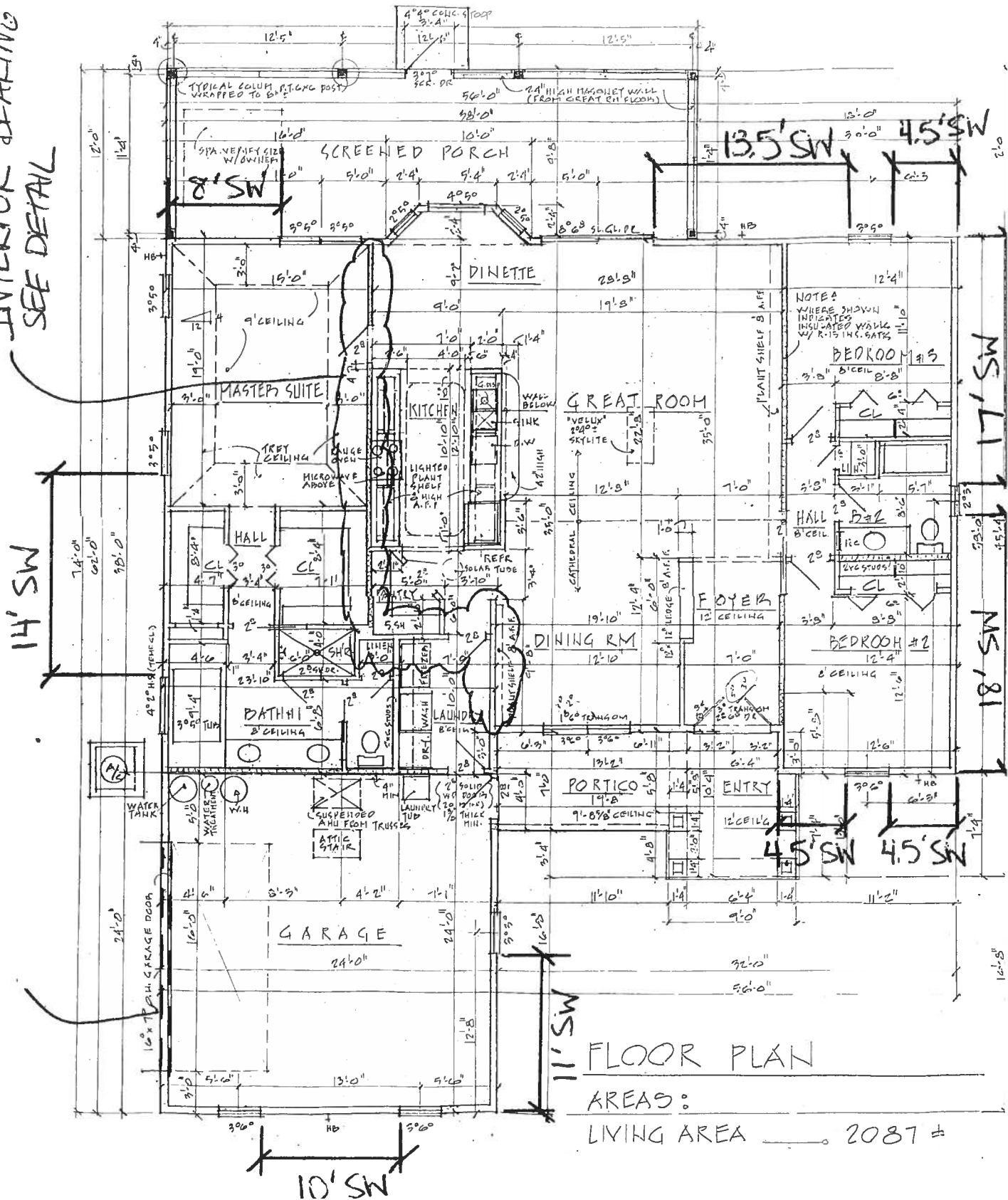
#### 2004 Florida Building Code Section 1603.1.4 Information

Basic Wind Speed	=	110 mph
Importance Factor	=	1.00
Building Category	=	II
Wind Exposure	=	B
Internal Pressure Coefficient	=	+ - 0.18
C & C Pressures	=	Zone 4 = 22.6 psf Zone 5 = 27.2 psf

ASSUMED SOIL BEARING PRESSURE = 2000 PSF

1. TRANSVERSE  $\leftrightarrow$  LONGITUDINAL

INTERIOR BEARING  
SEE DETAIL



(2)  $1\frac{3}{4}'' \times 14''$  LVL

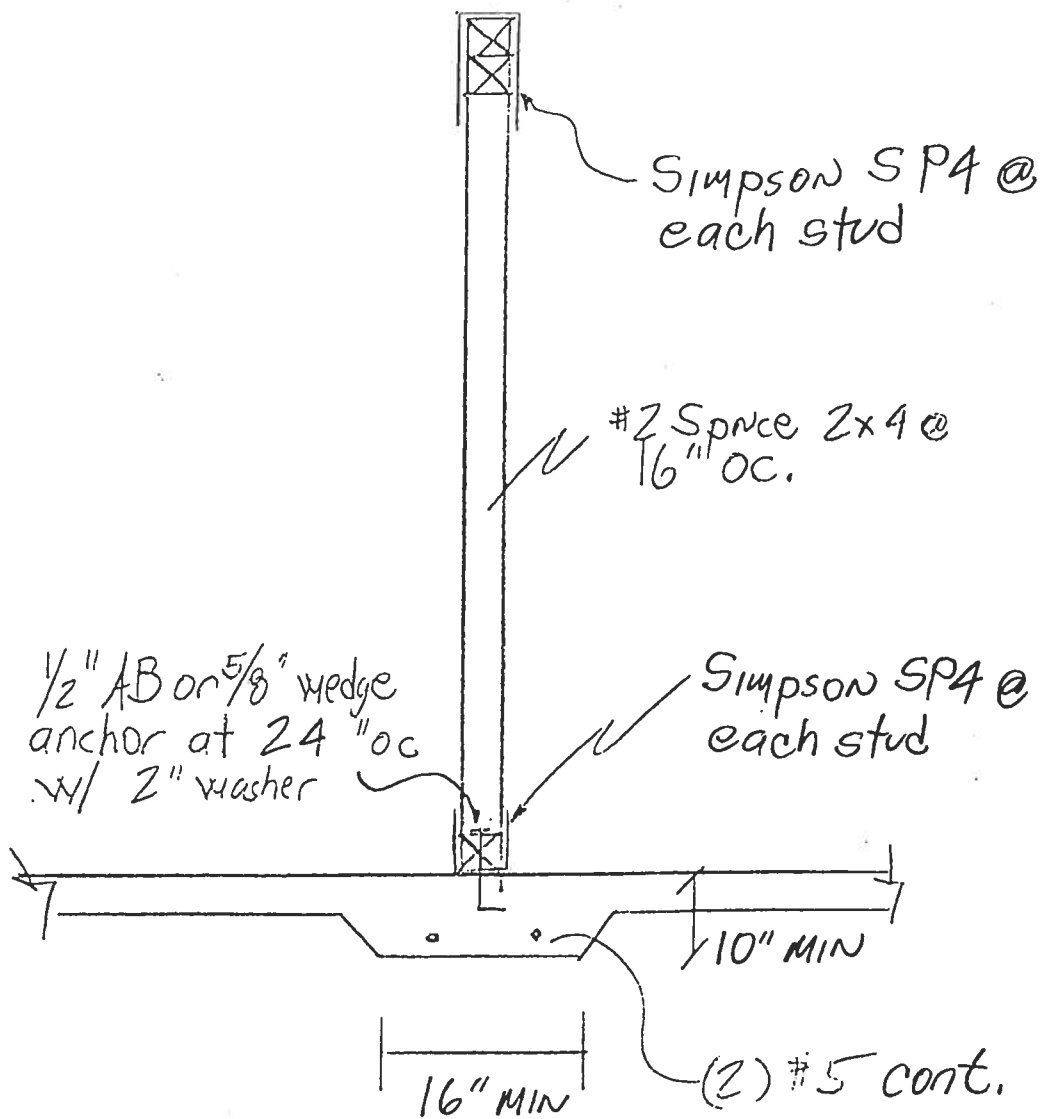
(3)  $2 \times 12$

(3)  $2x/2$

✓ (3)  $2 \times 12$

AREAS:

LIVING AREA \_\_\_\_ 2087 #

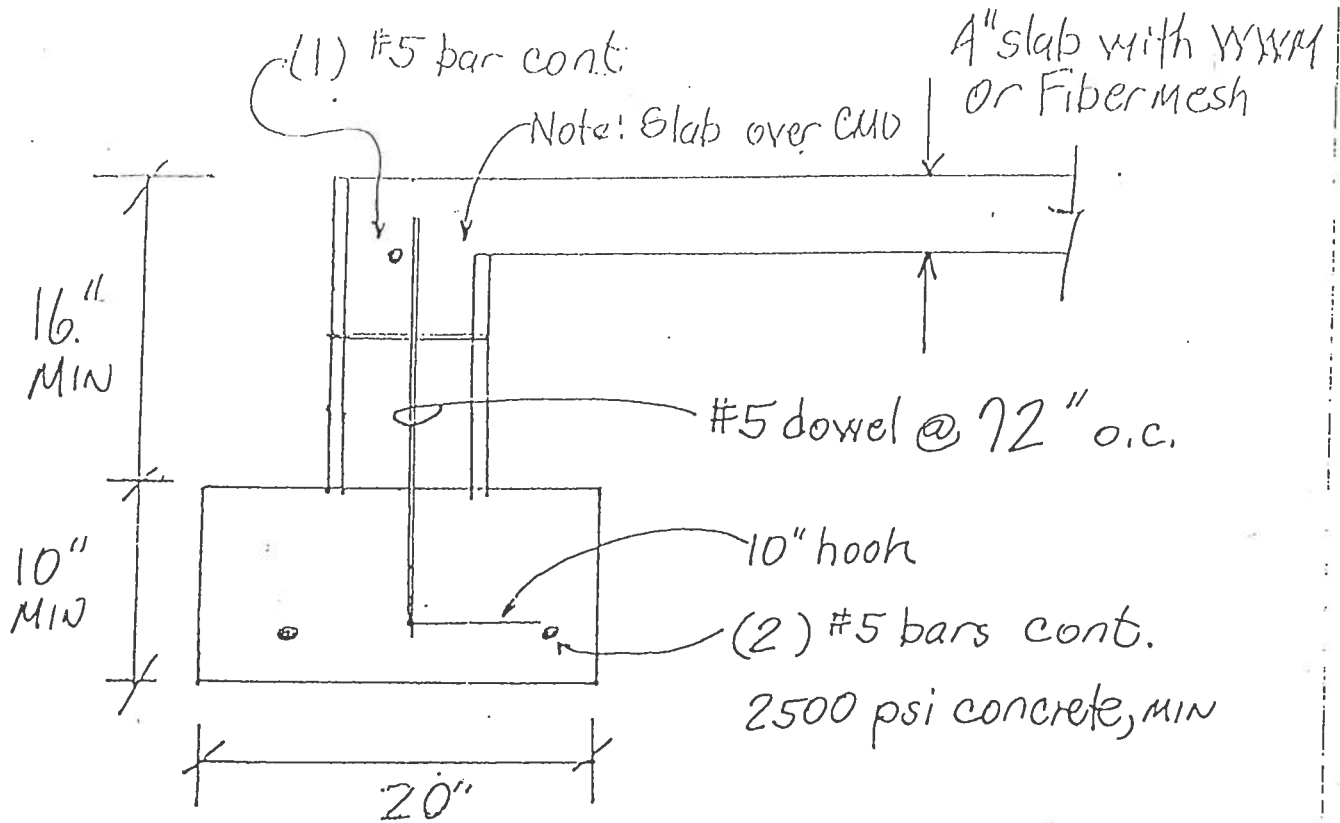


## Interior Bearing Wall

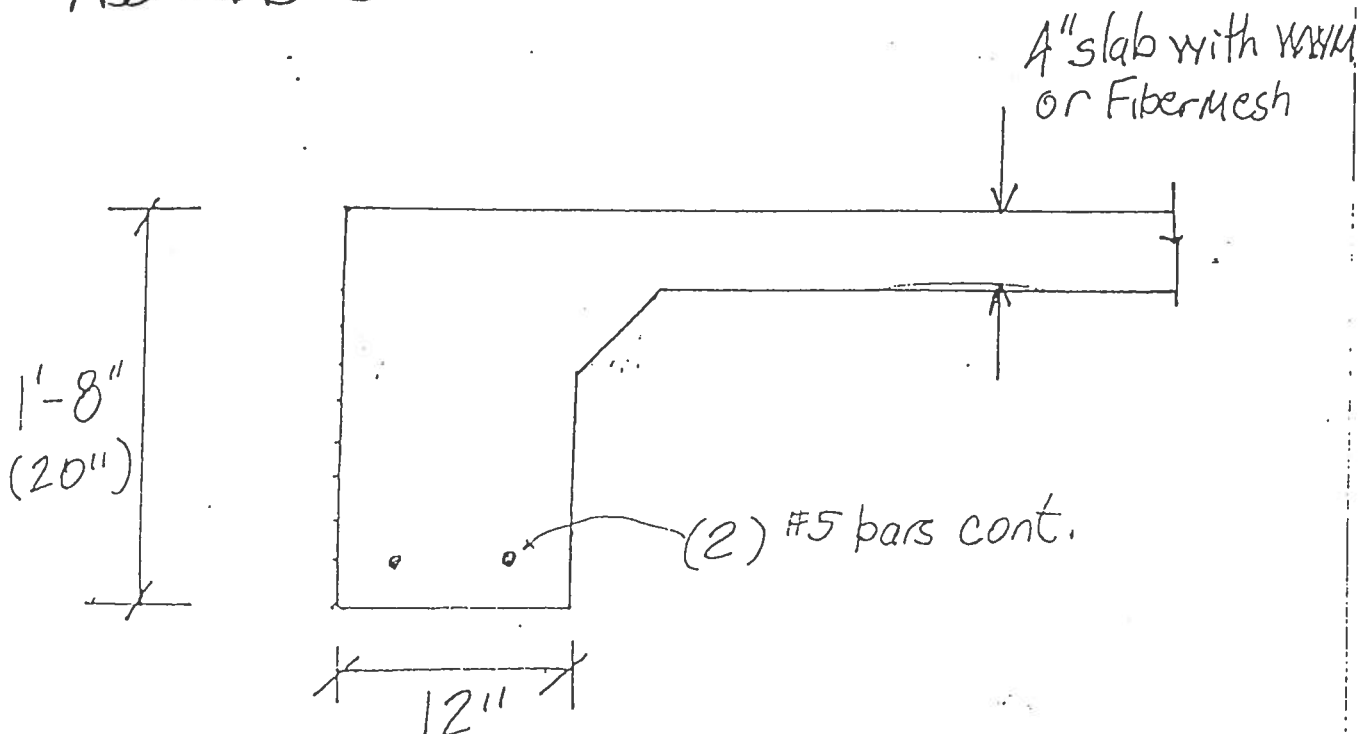
ASSUMED BEARING PRESSURE = 2000 PSF

# Foundation Alternatives

All rebar - Grade 40  
All concrete - 3000 psi min



ASSUMED SOIL BEARING PRESSURE = 2000 PSF



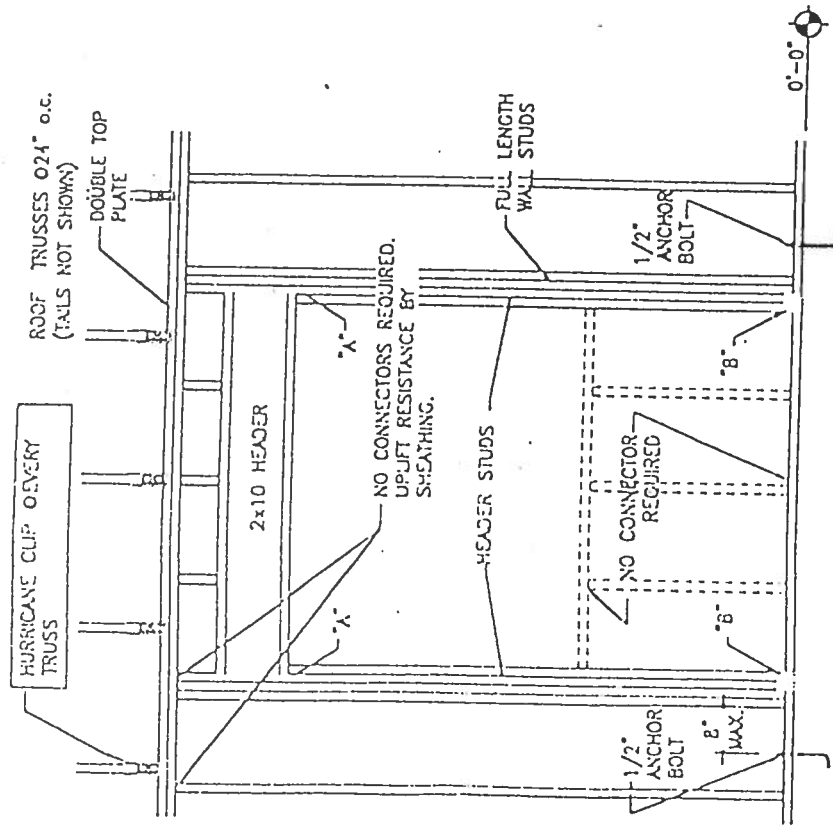
## CONNECTOR SELECTION

SPAN	'A'	'B'	ANCHOR BOLTS
9'-0" AND UNDER	(1) LSTA24	(1) SP4	(1) EACH END
OVER 9'-0"	(2) LSTA24	(2) SP4	(1) EACH END
• USE SP6 ON 2X6 WALLS			
NOTES:			

1. CONNECTORS INDICATED ARE BY SIMPSON STRONG TIE CO., INC. PRE-APPROVED EQUAL MAY BE USED.
2. STANDARD WALL HEIGHT SHOWN. WALL HEIGHT MAY VARY. ADJUST HEAD AND SILL HEIGHT WITH CRIPPLES AS REQUIRED.
3. REFER TO HEADER HOLD DOWN CHART FOR NUMBER OF FULL-LENGTH AND HEADER STUDS REQUIRED FOR DIFFERENT OPENING WIDTHS.
4. SHEAR AND UPLIFT RESISTANCE PROVIDED BY SHEATHING. REFER TO STRUCTURAL ENGINEER'S NOTES ON THIS SHEET.

## HEADER HOLD DOWNS

UNSUPPORTED WALL HEIGHT	STUD SPACING	MAXIMUM HEADER SPAN (1L)						
		3	6	9	12	15	18	
		NUMBER OF HEADER STUDS SUPPORTING END OF HEADER						
10'-0" OR LESS	12 in.	1	1	2	2	2	2	
	16 in.	2	2	3	3	3	3	
	24 in.	1	2	2	2	2	2	
	12 in.	2	2	3	4	5	5	
GREATER THAN 10'-0"	16 in.	2	2	3	3	4	4	
	24 in.	1	2	2	2	2	3	



## OPENING FRAMING DETAIL

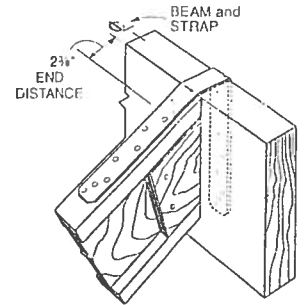
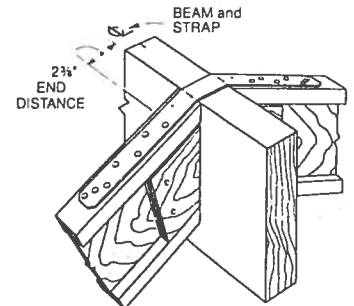


**HRS/ST/FHA/PS/HST/LSTA/LSTI/MST/MSTA/MSTC/MSTI** - Strap Ties

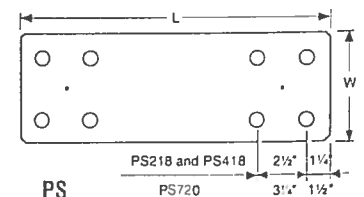
CODES: See page 12 for Code Listing Key Chart.

Available with additional corrosion protection. Check with factory.

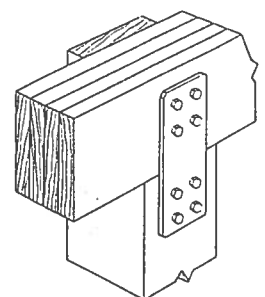
Model No.	Ga	Dimensions		Fasteners (Total)	Allowable Tension Loads (DF/SP)		Allowable Tension Loads (SPF/HF)		Code Ref.
		W	L		(133)	(160)	(133)	(160)	
LSTA9		1 1/4	9	8-10d	645	775	555	665	7, 62, 90, 128
LSTA12		1 1/4	12	10-10d	805	970	695	830	
LSTA15		1 1/4	15	12-10d	970	1160	830	1000	
LSTA18		1 1/4	18	14-10d	1130	1235	970	1165	
LSTA21		1 1/4	21	16-10d	1235	1235	1110	1235	
LSTA24	20	1 1/4	24	18-10d	1235	1235	1235	1235	3, 39, 88, 104, 121, 128
ST292		2 1/16	9 1/16	12-16d	1120	1265	970	1160	
ST2122		2 1/16	12 13/16	16-16d	1505	1535	1290	1535	
ST2115		3/4	16 5/16	8-16d	665	665	665	665	
ST2215		2 1/16	16 5/16	20-16d	1880	1880	1625	1880	
LSTA30		1 1/4	30	22-10d	1640	1640	1555	1640	7, 62, 90, 128
LSTA36		1 1/4	36	24-10d	1640	1640	1640	1640	
LSTI49		3 3/4	49	32-10dx1 1/2	2580	3100	2220	2660	9, 121, 128
LSTI73		3 3/4	73	48-10dx1 1/2	3870	4215	3330	3995	
MSTA9	18	1 1/4	9	8-10d	650	780	565	680	7, 62, 90, 123, 128
MSTA12		1 1/4	12	10-10d	815	975	705	850	
MSTA15		1 1/4	15	12-10d	975	1170	850	1020	
MSTA18		1 1/4	18	14-10d	1140	1365	990	1185	
MSTA21		1 1/4	21	16-10d	1300	1560	1130	1355	
MSTA24		1 1/4	24	18-10d	1465	1640	1270	1525	7, 62, 90, 128
MSTA30		1 1/4	30	22-10d	1835	2050	1585	1900	
MSTA36		1 1/4	36	26-10d	2050	2050	1870	2050	
ST6215		2 1/16	16 5/16	20-16d	1895	2095	1640	1970	
ST6224		2 1/16	23 5/16	28-16d	2540	2540	2315	2540	
ST9		1 1/4	9	8-16d	755	910	655	785	3, 39, 88, 104, 121, 128
ST12	16	1 1/4	11 5/8	10-16d	945	1135	820	985	
ST18		1 1/4	17 3/4	14-16d	1325	1420	1150	1380	3, 39, 88, 121, 128
ST22		1 1/4	21 5/8	18-16d	1420	1420	1420	1420	
MSTC28		3	28 1/4	36-16d sinkers	3000	3600	2590	3110	9, 23, 121, 128
MSTC40		3	40 1/4	52-16d sinkers	4335	4585	3745	4495	
MSTC52		3	52 1/4	62-16d sinkers	4585	4585	4465	4585	9, 23, 128
MSTC66		3	65 3/4	76-16d sinkers	5660	5660	5660	5660	
MSTC78	14	3	77 3/4	76-16d sinkers	5660	5660	5660	5660	3, 39, 88, 104, 121, 128
ST6236		2 1/16	33 13/16	40-16d	3845	3845	3465	3845	
HRS6		1 3/8	6	6-10d	525	630	455	545	128
HRS8		1 3/8	8	10-10d	875	1050	760	910	
HRS12		1 3/8	12	14-10d	1225	1465	1065	1275	
FHA6		1 7/16	6 3/8	8-16d	810	975	705	845	
FHA9		1 7/16	9	8-16d	810	975	705	845	3, 39, 88, 121, 128
FHA12		1 7/16	11 5/8	8-16d	810	975	705	845	
FHA18	12	1 7/16	17 3/4	8-16d	810	975	705	845	
FHA24		1 7/16	23 3/8	8-16d	810	975	705	845	
FHA30		1 7/16	30	8-16d	810	975	705	845	
MSTI26		2 1/16	26	26-10dx1 1/2	2355	2830	2045	2455	
MSTI36		2 1/16	36	36-10dx1 1/2	3265	3915	2830	3400	
MSTI48		2 1/16	48	48-10dx1 1/2	4350	5080	3775	4530	
MSTI60		2 1/16	60	60-10dx1 1/2	5080	5080	4720	5080	
MSTI72		2 1/16	72	64-10dx1 1/2	5080	5080	5080	5080	

Typical LSTA Installation  
(hanger not shown)Typical LSTA Installation  
(hanger not shown)

Model No.	Material Thickness mil (ga)	Dim.		Bolts		Code Ref.
		W	L	Qty	Dia	
PS218 <sup>5</sup>	171 mil	2	18	4		180
PS418 <sup>5</sup>	(7 ga)	4	18	4		
PS720 <sup>5</sup>		6 1/4	20	8	1/2	



PS



Typical PS720 Installation

1. Loads include a 33% or 60% load duration increase on the fasteners for earthquake or wind loading, but DO NOT include a 33% stress increase on the steel capacity. Refer to page 13 for further explanation.
2. 10dx1 1/2" nails may be substituted where 16d sinkers are specified at 100% of the table loads.
3. 10d commons may be substituted where 16d sinkers are specified at 100% of table loads.
4. 16d sinkers or 10d commons may be substituted where 16d commons are specified at 0.85 of the table loads.
5. Use half of the nails in each member being connected to achieve the listed loads.
6. PS strap design loads must be determined by the designer for each installation. Bolts are installed both perpendicular and parallel-to-grain. Hole diameter in the part may be oversized to accommodate the HDG. Designer must determine if the oversize creates an unacceptable installation.
7. For overlap splice details, refer to T-CMST.

# DSP/SSP/SP/SPH/RSP4 Stud Plate Ties



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

DSP and SSP provide flexibility in the field — can be used as a plate to stud connection AND top plate to stud connection.

The RSP4 is a reversible stud plate tie with locating tabs, which aid placement on double top plates or a single bottom plate.

**MATERIAL:** DSP/SSP/SPH—18 gauge, all others—20 gauge

**FINISH:** Galvanized. Some products available in Z-MAX;

see Corrosion-Resistance, page 6-7.

**INSTALLATION:** • Use all specified fasteners; see General Notes.

- DSP/SSP—sill plate installation—fill all round holes.
- DSP/SSP—top plate installation—fill all round and triangle holes
- SP—one of the 10d common stud nails is driven at a 45° angle through the stud into the plate.

**CODES:** See page 12 for Code Listing Key Chart.

Available with additional corrosion protection. Check with factory.

Model No.	Dim.		Fasteners		Allowable Uplift Loads (133/160)			Code Ref.
	W	L	Studs	Double Top Plate	Double Top Plate	Single Sill Plate	Single Sill Plate	
SSP	1 3/8"	6 1/16"	4-10dx1 1/2"	3-10dx1 1/2"	350	—	—	62, 125
			—	1-10dx1 1/2"	—	420	325	
			4-10d	3-10d	435	—	—	
			—	1-10d	—	455	420	
DSP	2 3/4"	6 1/16"	8-10dx1 1/2"	6-10dx1 1/2"	775	—	—	62, 125
			—	2-10dx1 1/2"	—	660	545	
			8-10d	6-10d	825	—	—	
			—	2-10d	—	825	600	

1. Allowable loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed.

2. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

3. Allowable loads for DSP installed to a rim joist are 660 lbs (DF SP), 545 lbs (SPF HF).

4. NAILS 10d = 0.143" dia x 3" long; 10dx1 1/2" = 0.148" dia x 1 1/2" long

See page 16-17 for other nail sizes and information.

Model No.	Dim.		Stud	Plate Width	Fasteners		Allowable Uplift Loads				Code Ref.
	W	L			Stud	Plate	DF/SP	SPF	(133) <sup>2</sup>	(160) <sup>2</sup>	
SP1	3 1/2"	5 1/16"	2x	—	6-10d	4-10d	585	585	535	535	6, 121
SP2	3 1/2"	6 3/8"	2x	—	6-10d	6-10d	890	1065	605	605	
SP3	4 1/2"	6 3/8"	3x	—	6-10d	6-10d	890	1065	605	605	160
SP4	3 3/16"	7 1/4"	2x	4x	6-10dx1 1/2"	—	735	885	630	760	7, 121
SP5	4 1/2"	5 1/16"	3x	—	6-10d	4-10d	585	585	535	535	
SP6	5 9/16"	7 3/4"	2x	6x	6-10dx1 1/2"	—	735	885	630	760	7, 121
SP8	7 9/16"	8 3/16"	2x	8x	6-10dx1 1/2"	—	735	885	630	760	
SPH4	3 3/16"	8 3/4"	2x	4x	10-10dx1 1/2"	—	1240	1240	1065	1065	62, 123
					12-10dx1 1/2"	—	1360	1360	1170	1170	
SPH6	5 9/16"	9 1/4"	2x	6x	10-10dx1 1/2"	—	1240	1240	1065	1065	
					12-10dx1 1/2"	—	1360	1360	1170	1170	
SPH8	7 9/16"	8 3/8"	2x	8x	10-10dx1 1/2"	—	1240	1240	1065	1065	6, 30, 99, 121
					12-10dx1 1/2"	—	1360	1360	1170	1170	
RSP4(1)	2 1/8"	4 1/2"	2x	—	4-8dx1 1/2"	4-8dx1 1/2"	315	315	285	285	6, 30, 99, 121
RSP4(2)	2 1/8"	4 1/2"	2x	—	4-8dx1 1/2"	4-8dx1 1/2"	450	450	370	370	

1. SP1, 2, 3 and SP5: drive one stud nail at an angle through the stud into the plate to achieve the table load (see illustration).

2. Allowable loads have been increased for earthquake or wind loading; no further increase allowed. Reduce where other loads govern.

3. RSP4—see Installation details (1) and (2) for reference.

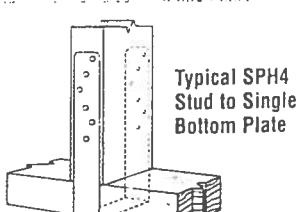
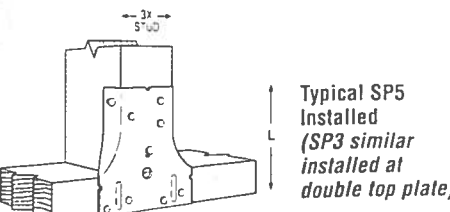
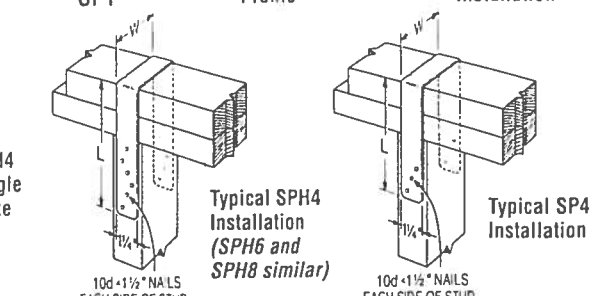
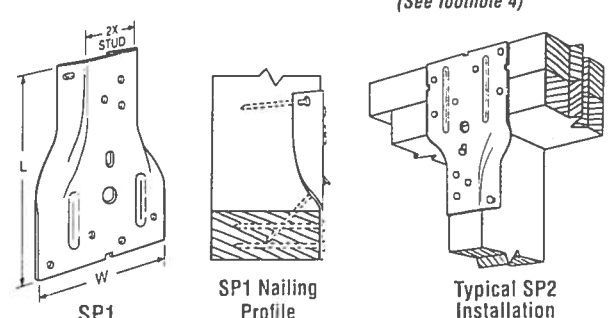
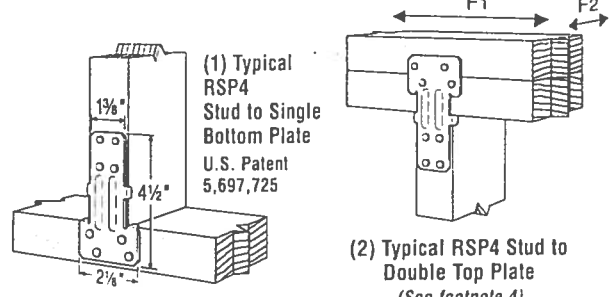
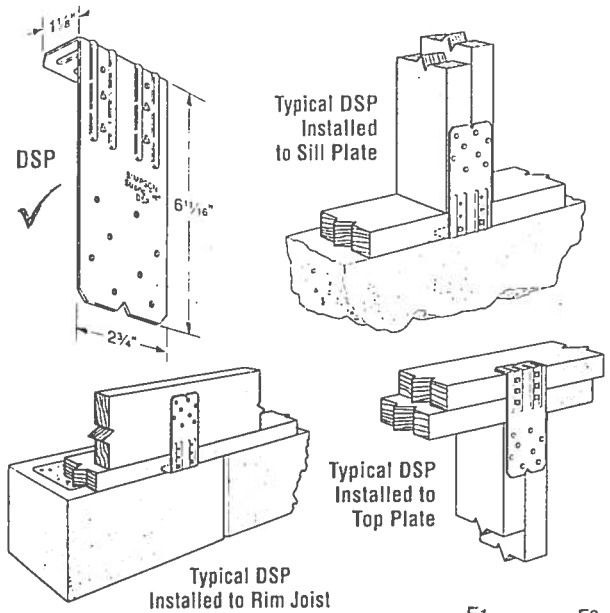
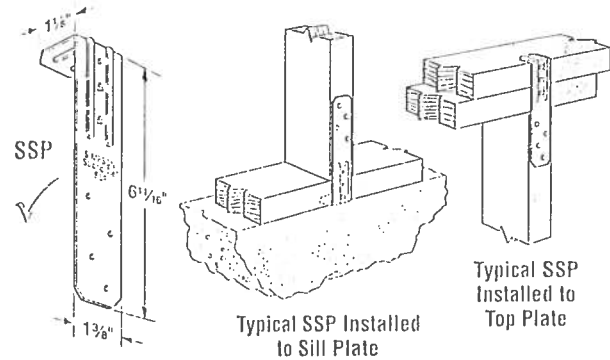
4. RSP4 F2 is 280 lbs (installation 1) and 305 lbs (installation 2). F1 load is 210 lbs for both installations.

5. Maximum load for SPH in Southern Yellow Pine is 1490 lbs.

6. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

7. For retrofit application see T-STRAP.

10d = 0.143" dia x 3" long  
10dx1 1/2" = 0.148" dia x 1 1/2" long  
6-10d = 0.125" dia x 1" long  
See page 16-17 for other nail sizes and information.

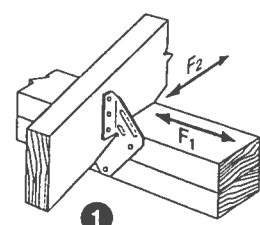


Available with additional corrosion protection. Check with factory.

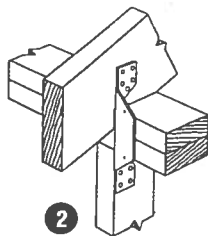
Model No.	Ga	Fasteners			DF/SP Allowable Loads				Uplift Load with 8dx1½" Nails (133 & 160)	SPF/HF Allowable Loads				Uplift Load with 8dx1½" Nails (133 & 160)	Code Ref.
		To Rafter/Truss	To Plates	To Studs	Uplift		Lateral (133/160)			Uplift		Lateral (133/160)			
					(133)	(160)	F <sub>1</sub>	F <sub>2</sub>		(133)	(160)	F <sub>1</sub>	F <sub>2</sub>		
H1	18	6-8dx1½	4-8d	—	490	585	485	165	455	400	400	415	140	370	2, 40, 62, 121, 140
H2	18	5-8d	—	5-8d	335	335	—	—	335	230	230	—	—	230	
H2.5	18	5-8d	5-8d	—	415	415	150	150	415	365	365	130	130	365	
H2.5A	18	5-8d	5-8d	—	600	600	110	110	480	520	535	110	110	480	
H2.5T	18	5-8d	5-8d	—	545	545	135	145	425	510	515	135	145	425	122
H3	18	4-8d	4-8d	—	455	455	125	160	415	320	320	105	140	290	2, 40, 82, 121, 140
H4	20	4-8d	4-8d	—	360	360	165	160	360	235	235	140	135	235	2, 40, 121, 140
H5	18	4-8d	4-8d	—	455	465	115	200	455	265	265	100	170	265	2, 40, 82, 121, 140
H5A	18	3-8d	3-8d	—	350	420	115	180	290	245	245	100	120	170	10
H6	16	—	8-8d	8-8d	915	950	650	—	—	785	820	560	—	—	5, 41, 121, 140
H7Z	16	4-8d	2-8d	8-8d	930	985	400	—	—	800	845	345	—	—	125
H8	18	5-10dx1½	5-10dx1½	—	620	745	75	—	—	530	565	75	—	—	170
H9KT	18	4-SDS¼x1½	5-SDS¼x1½	—	875	875	680	125	—	755	755	680	125	—	9, 121
H10	18	8-8dx1½	8-8dx1½	—	905	990	585	525	—	780	850	505	450	—	6, 121
H10R	18	8-8dx1½	8-8dx1½	—	905	990	585	525	—	780	850	505	450	—	170
H10-2	18	6-10d	6-10d	—	760	760	455	395	—	655	655	390	340	—	125
H11Z	18	6-16dx2½	6-16dx2½	—	830	830	525	760	—	715	715	450	655	—	125
H14	18	1 12-8dx1½	13-8d	—	1350	1350	515	265	—	1050	1050	480	245	—	125
		2 12-8dx1½	15-8d	—	1350	1350	515	265	—	1050	1050	480	245	—	

1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed, reduce where other loads govern.
2. Allowable loads are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist and on the same side of the plate (except for H2.5).
3. Allowable uplift load for stud to bottom plate installation is 400 lbs (H2.5); 390 lbs (H2.5A); 360 lbs (H4) and 310 lbs (H8).
4. Allowable loads in the F<sub>1</sub> direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements

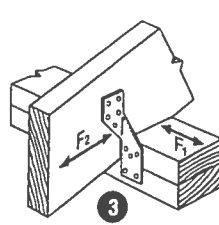
5. Hurricane Ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.
6. Southern Pine allowable loads for H14: 1465 lbs (133/160), 560 lbs (F<sub>1</sub> Lateral 133/160) and 285 lbs (F<sub>2</sub> Lateral 133/160).
7. Refer to TIE-BEARING for selected hurricane tie allowable bearing enhancement loads.
8. H2.5: 10 1/2" x 0.162" dia x 2 1/2" long. 10d = 0.143" dia x 3" long. 10dx1 1/2" = 0.143" dia x 1 1/2" long. 3/4" = 0.131" dia x 2 1/2" long. 8dx1 1/2" = 0.131" dia x 1 1/2" long. See page 16-17 for other nail sizes and information.



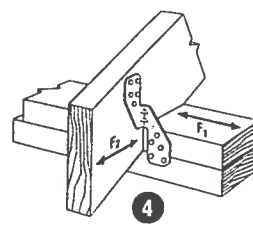
H1 Installation



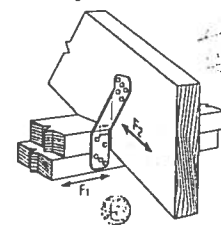
H2 Installation



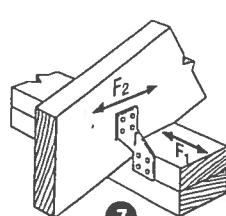
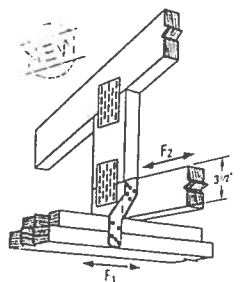
H2.5 Installation  
(Nails into both top plates)



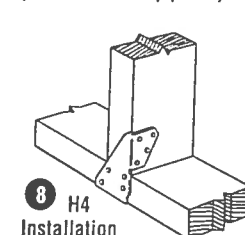
H2.5A Installation  
(Nails into both top plates)



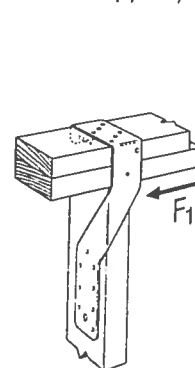
H2.5T Installation  
(Nails into both top plates)



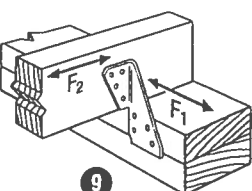
H3 Installation  
(Nails into upper top plate)



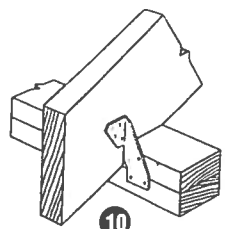
H4 Installation  
(H2.5 similar)  
(see footnote 3, page 142)



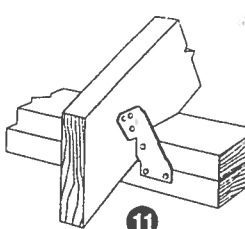
H5 Stud to Top Plate Installation



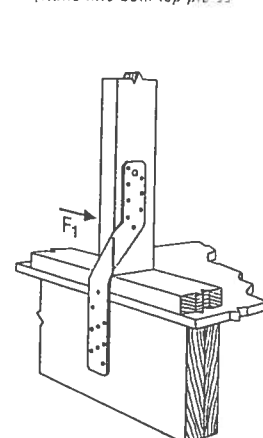
H4 Installation  
(Nails into upper top plate)



H5 Installation  
(Nails into both top plates)



H5A Installation  
(Nails into both top plates)



H6 Stud to Band Joist Installation

**AB/ABA/ABE/ABU** Adjustable and Standoff Post Bases**SIMPSON****Strong-Tie**

Additional standoff bases are on page 187.

The AB is an adjustable post base which offers moisture protection and finished hardware appearance.

Post Bases provide tested capacity. They feature 1" standoff height above concrete floors, code-required when supporting permanent structures that are exposed to the weather or water splash, or in basements. They reduce the potential for decay at post and column ends.

**MATERIAL:** AB—12 ga plates; 16 ga base cover; all others—see table

**FINISH:** Galvanized. Some products available in Z-MAX; see Corrosion-Resistance, page 6-7.

**INSTALLATION:** • Use all specified fasteners. See General Notes.

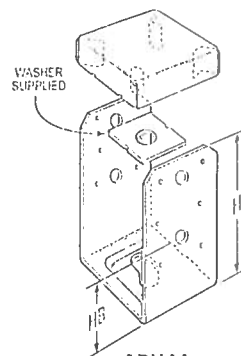
- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).
- AB supplied as shown; position the post, secure the easy-access nut, then bend up the fourth side.
- AB, ABA, ABE and ABU—for pre-pour installed anchors. For epoxy or wedge anchors, select and install according to anchor manufacturer's recommendations; anchor diameter shown in table. Install required washer, which is not included for ABAs.
- See Simpson Anchor Systems for tested, load-rated anchors and request T-ANCHORSPEC for more information.

**CODES:** See page 12 for Code Listing Key Chart.

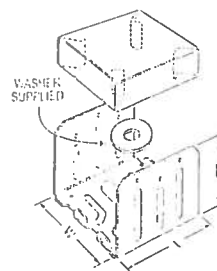
Available with additional corrosion protection. Check with factory.

Model No.	Dimensions			Anchor Dia.	Fasteners	Allowable Download (100)	Code Ref.
	W	L	H				
AB44	3 <sup>9</sup> / <sub>16</sub>	3 <sup>9</sup> / <sub>16</sub>	2 <sup>1</sup> / <sub>4</sub>	1/2	8-10d	4065	2, 40, 82, 121
AB44R	4	4 <sup>1</sup> / <sub>16</sub>	2 <sup>9</sup> / <sub>16</sub>	1/2	8-10d	4065	
AB46	3 <sup>9</sup> / <sub>16</sub>	5 <sup>3</sup> / <sub>8</sub>	3	1/2	8-10d	4165	
AB46R	4	6	2 <sup>13</sup> / <sub>16</sub>	1/2	8-10d	4165	
AB66	5 <sup>1</sup> / <sub>2</sub>	5 <sup>9</sup> / <sub>16</sub>	3	1/2	8-10d	5335	
AB66R	6	6	2 <sup>13</sup> / <sub>16</sub>	1/2	8-10d	5335	

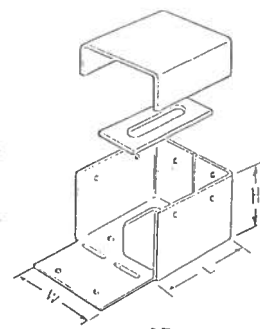
1. Loads may not be increased for short-term loading



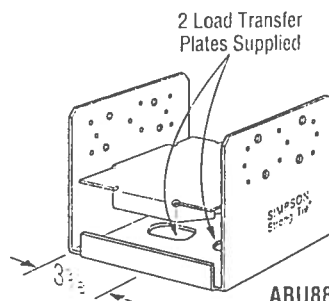
**ABU44**  
(other sizes similar)



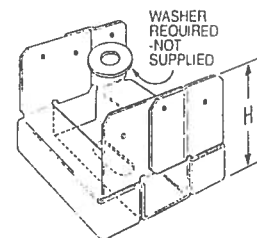
**ABE44**  
ABE46, 46R, 66 and 66R  
supplied with washer.



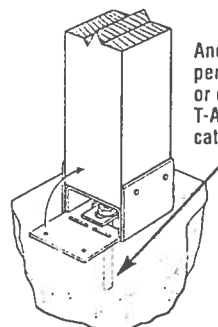
**AB**



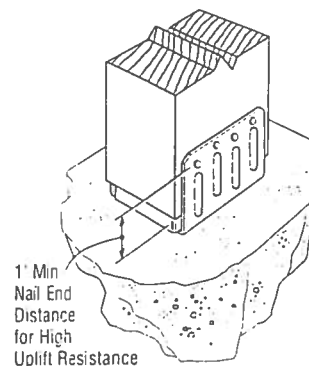
**ABU88**



**ABA44**  
(other sizes similar)  
U.S. Patent  
5,333,435



Typical AB Installation



Typical ABE46R Installation for rough lumber (ABE similar)

	Model No.	Nominal Post Size	Material		Dimensions				Fasteners				Allowable Loads (DF/SP)					Code Ref.
			Base (Ga)	Strap (Ga)	W	L	H	HB <sup>5</sup>	Anch. Dia	Post		Uplift (133)		Uplift (160)		Down (100)		
										Nails	Machine Bolts	Nails	Bolts	Nails	Bolts			
											Qty	Dia						
ABA44	4x4	16	16	3 <sup>9</sup> / <sub>16</sub>	3 <sup>1</sup> / <sub>8</sub>	3 <sup>1</sup> / <sub>8</sub>	—	1/2	6-10d	—	—	—	555	—	555	—	6000	5, 41, 121
ABE44	4x4	16	16	3 <sup>9</sup> / <sub>16</sub>	3 <sup>1</sup> / <sub>2</sub>	2 <sup>3</sup> / <sub>4</sub>	—	1/2	6-10d	—	—	—	520	—	520	—	6665	6, 86, 121
ABU44	4x4	16	12	3 <sup>9</sup> / <sub>16</sub>	3	5 <sup>1</sup> / <sub>2</sub>	1 <sup>3</sup> / <sub>4</sub>	5/8	12-16d	2	1/2	—	2200	1800	2200	2160	6665	8, 36, 91, 121
ABA44R	RGH 4x4	16	16	4 <sup>1</sup> / <sub>16</sub>	3 <sup>1</sup> / <sub>8</sub>	2 <sup>13</sup> / <sub>16</sub>	—	1/2	6-10d	—	—	—	555	—	555	—	8000	5, 41, 121
ABE44R	RGH 4x4	16	16	4	3 <sup>1</sup> / <sub>2</sub>	2 <sup>9</sup> / <sub>16</sub>	—	1/2	6-10d	—	—	—	400	—	400	—	6665	6, 86
ABE46	4x6	12	16	3 <sup>9</sup> / <sub>16</sub>	5 <sup>7</sup> / <sub>16</sub>	4 <sup>1</sup> / <sub>16</sub>	—	5/8	8-16d	—	—	—	810	—	810	—	7335	9, 121
ABA46	4x6	14	14	3 <sup>9</sup> / <sub>16</sub>	5 <sup>3</sup> / <sub>16</sub>	3 <sup>1</sup> / <sub>8</sub>	—	5/8	8-16d	—	—	—	700	—	700	—	9435	5, 41, 121
ABU46	4x6	12	12	3 <sup>9</sup> / <sub>16</sub>	5	7	2 <sup>5</sup> / <sub>8</sub>	5/8	12-16d	2	1/2	—	2255	2300	2300	2300	10335	8, 36, 91, 121
ABE46R	RGH 4x6	12	16	4 <sup>1</sup> / <sub>16</sub>	5 <sup>7</sup> / <sub>16</sub>	3 <sup>9</sup> / <sub>16</sub>	—	5/8	8-16d	—	—	—	810	—	810	—	7335	9
ABA46R	RGH 4x6	14	14	4 <sup>1</sup> / <sub>16</sub>	5 <sup>3</sup> / <sub>16</sub>	2 <sup>7</sup> / <sub>8</sub>	—	5/8	8-16d	—	—	—	700	—	700	—	12000	5, 41, 121
ABA66	6x6	14	14	5 <sup>1</sup> / <sub>2</sub>	5 <sup>1</sup> / <sub>2</sub>	3 <sup>1</sup> / <sub>8</sub>	—	5/8	8-16d	—	—	—	720	—	720	—	10665	—
ABE66	6x6	12	14	5 <sup>1</sup> / <sub>2</sub>	5 <sup>1</sup> / <sub>2</sub>	3 <sup>1</sup> / <sub>8</sub>	—	5/8	8-16d	—	—	—	900	—	900	—	12000	9, 121
ABU66	6x6	12	10	5 <sup>1</sup> / <sub>2</sub>	5	6 <sup>1</sup> / <sub>16</sub>	1 <sup>3</sup> / <sub>4</sub>	5/8	12-16d	2	1/2	—	2300	2300	2300	2300	12000	8, 36, 91, 127
ABA66R	RGH 6x6	14	14	6	5 <sup>3</sup> / <sub>16</sub>	2 <sup>7</sup> / <sub>8</sub>	—	5/8	8-16d	—	—	—	720	—	720	—	12665	5, 41, 121
ABE66R	RGH 6x6	12	14	6 <sup>1</sup> / <sub>16</sub>	5 <sup>7</sup> / <sub>16</sub>	2 <sup>7</sup> / <sub>8</sub>	—	5/8	8-16d	—	—	—	900	—	900	—	12000	9
ABU88	8x8	14	12	7 <sup>1</sup> / <sub>2</sub>	7	7	—	2 <sup>5</sup> / <sub>8</sub>	18-16d	—	—	—	2320	—	2320	—	24335	8, 36, 127
ABU88R	RGH 8x8	14	12	8	7	7	—	2 <sup>5</sup> / <sub>8</sub>	18-16d	—	—	—	2320	—	2320	—	24335	8, 36

1. Uplift loads have been increased for earthquake or wind load durations with no further increase allowed; reduce where other load durations govern.

2. Downloads may not be increased for short-term loading

3. Specifier to design concrete for shear capacity

4. ABU88 and ABU88R may be installed with 8-SDS<sup>1</sup>/<sub>2</sub>x3 wood screws for the same table load

5. For higher downloads, solidly pack grout under 1" standoff plate before installing into concrete. Base download on column or concrete, according to the code

6. HB dimension is the distance from the bottom of the post up to the first bolt hole

7. NAILS 15d = 0.162" dia. x 3<sup>1</sup>/<sub>2</sub>" long.  
10d = 0.143" dia. x 3" long  
See page 16-17 for other nail sizes and information.



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

This design uses SDS screws to provide faster installation and The SDS screws provide for a lower profile compared to standard through bolts.

**MATERIAL:** CCQ3, ECCQ3, CCQ4, ECCQ4, CCQ6, ECCQ6—7 gauge; all others—3 gauge.

**FINISH:** Simpson gray paint, available in HDG with HDG screws.

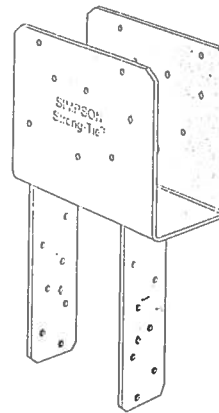
**INSTALLATION:** \* Fasteners provided. See General Notes.

- Install Simpson's SDS  $\frac{1}{4}$ "x2  $\frac{1}{2}$ " wood screws, which are provided with the column cap. (Lag screws will not achieve the same load.)

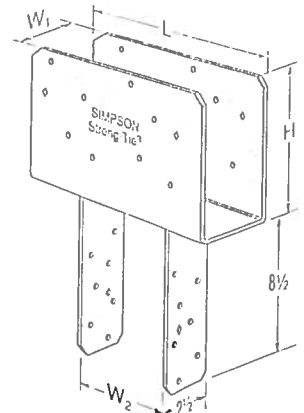
#### OPTIONS:

- Straps may be rotated 90° where  $W_1 \geq W_2$ . For end conditions, specify ECCQ. Loads do not apply to CCQ and ECCQ.

**CODES:** See page 12 for Code Listing Key Chart.



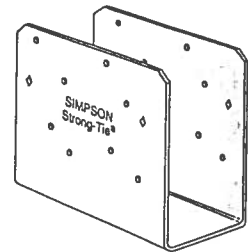
ECCQ46SDS2.5



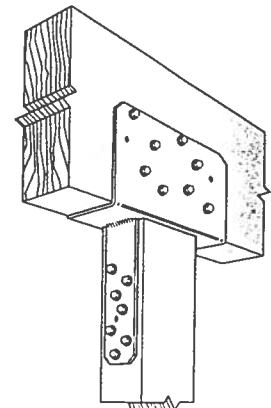
CCQ46SDS2.5

Available with additional corrosion protection. Check with factory.

Model No.	Beam Width	Dimensions					No. of <sup>9</sup> SDS ¼"x2 ½" Screws		Allowable Loads					Code Ref.
		W <sub>1</sub>	W <sub>2</sub>	L		H			CCQ			ECCQ		
				CCQ	ECCQ				Beam	Post	(133)	(160)	(100)	
CCQ3-4SDS2.5	3⅝	3¼	3⅝	11	8½	7	16	14	5680	5680	19250	3695	6125	46, 107, 124
CCQ3-6SDS2.5	3⅝	3¼	5½	11	8½	7	16	14	5680	5680	19250	3695	9625	
CCQ44SDS2.5	4x	3⅝	3⅝	11	8½	7	16	14	5680	5680	24065	4040	7655	
CCQ46SDS2.5	4x	3⅝	5½	11	8½	7	16	14	5955	7145	24065	4040	12030	
CCQ48SDS2.5	4x	3⅝	7½	11	8½	7	16	14	5955	7145	24065	4040	16405	
CCQ5-4SDS2.5	5⅝	5¼	3⅝	11	8½	7	16	14	5680	5680	31570	4040	10045	
CCQ5-6SDS2.5	5⅝	5¼	5½	11	8½	7	16	14	6270	7245	31570	5535	15785	
CCQ5-8SDS2.5	5⅝	5¼	7½	11	8½	7	16	14	6270	7245	31570	5535	21525	
CCQ64SDS2.5	6x	5½	3⅝	11	8½	7	16	14	5680	5680	37815	4040	12030	
CCQ66SDS2.5	6x	5½	5½	11	8½	7	16	14	5955	7145	37815	4040	18905	
CCQ68SDS2.5	6x	5½	7½	11	8½	7	16	14	5955	7145	37815	4040	25780	160
CCQ6-7.13SDS2.5	6x	5½	7⅞	11	8½	7	16	14	5955	7145	37815	4040	24490	
CCQ74SDS2.5	6¾	6⅞	3⅝	11	8½	7	16	14	5680	5680	41580	4040	13230	46, 107, 124
CCQ76SDS2.5	6¾	6⅞	5½	11	8½	7	16	14	6270	7245	41580	5535	20790	
CCQ77SDS2.5	6¾	6⅞	6⅞	11	8½	7	16	14	6270	7245	41580	5535	25515	
CCQ78SDS2.5	6¾	6⅞	7½	11	8½	7	16	14	6270	7245	41580	5535	28350	
CCQ7-1-4SDS2.5	7	7⅞	3⅝	11	8½	7	16	14	5680	5680	57750	4040	19030	160
CCQ7-1-6SDS2.5	7	7⅞	5½	11	8½	7	16	14	6270	7245	57750	5535	28875	
CCQ7-1-7.1SDS2.5	7	7⅞	7⅞	11	8½	7	16	14	6270	7245	57750	5535	37405	
CCQ7-1-8SDS2.5	7	7⅞	7½	11	8½	7	16	14	6270	7245	57750	5535	39375	
CCQ86SDS2.5	8x	7½	5½	11	8½	7	16	14	6270	7245	51565	5535	25780	
CCQ88SDS2.5	8x	7½	7½	11	8½	7	16	14	6270	7245	51565	5535	35155	
CCQ96SDS2.5	8¾	8⅞	5½	11	8½	7	16	14	6270	7245	53900	5535	26950	
CCQ98SDS2.5	8¾	8⅞	7½	11	8½	7	16	14	6270	7245	53900	5535	36750	
CCQ106SDS2.5	10x	9½	5½	11	8½	7	16	14	6270	7245	65315	5535	32655	



CCQ4-SDS2.5



Typical CCQ46SDS2.5 Installation

- 1 Downloads are determined using  $F_c L$  equal to: 560 psi for glulam sizes and 625 psi for all others; reduce where end bearing or other criteria are limiting
- 2 Spliced conditions must be detailed by the members by means other than the column cap. to transfer tension loads between spliced
- 3 Uplift loads do not apply to splice conditions.
- 4 Post sides are assumed to lie in the same vertical plane as the beam sides
- 5 Loads may not be increased for short-term loading
- 6 Uplift loads have been increased for earthquake or wind reduce where other govern no further increase allowed
- 7 ECCQ downloads assume a post of  $W_1 \times W_2$
- 8 When using structural composite lumber columns screws must be applied to the wide face of the column
- 9 ECCQ uses 14-SDS screws into the beam and 14-SDS screws into the post
- 10 Beam depth must be greater than 7".

## WIND LOAD DESIGN PER 2004 FBC (1609.6)

### SINGLE STORY BUILDING

#### BUILDING DIMENSIONS:

L = 56 FEET  
W = 74 FEET  
EAVE = 8.08 FEET  
PITCH = 7 /12 = 30.3 DEG.  
O'HANG = 1.5 FEET  
RIDGE = 29.66333 FEET  
MEAN RF = 18.87167 FEET

#### WIND EXPOSURE:

VELOCITY = 110 MPH  
I = 1.00 (IMPORTANCE FACTOR)  
EXPOSURE = B  
ADJUSTMENT 1.00 (PER TABLE 1609.6D)

MWFRS PRESSURE PER TABLE 1609.6A (BASE PRESSURE W/O ADJUSTMENT)  
(PRESSURES IN PSF)

#### TRANSVERSE WIND DIRECTION

	END ZONE		INTERIOR ZONE	
HORIZONTAL LOADS	WALL	ROOF	WALL	ROOF
	21.6	14.8	17.2	11.8
VERTICAL LOADS	WINDWD	LEEWD	WINDWD	LEEWD
	8.3	-13.1	7.2	-11.3
O'HANG	-7.6		-8.7	

#### LONGITUDINAL WIND DIRECTION

	END ZONE		INTERIOR ZONE	
HORIZONTAL LOADS	WALL	ROOF	WALL	ROOF
	19.2	-10.0	12.7	-5.9
VERTICAL LOADS	WINDWD	LEEWD	WINDWD	LEEWD
	-23.1	-13.1	-16.0	-10.1
O'HANG	-32.3		-25.3	

#### CALCULATE EDGE STRIPS:

7.4 FEET	(10% OF LEAST DIM)	
3.232 FEET	(40% OF EAVE)	
	Least =	3.232 FEET
2.96 FEET	(4% OF LEAST DIM)	
3 FEET	(3 FEET)	
	Max =	3 FEET

A = 3.232 FEET  
2A = 6.464 FEET

HORIZONTAL TRANSVERSE LOAD  
ROOF 15099 LBS.

WALL 8242 LBS.

LONGITUDINAL TRANSVERSE LOAD

ROOF 10300 LBS.  
WALL 8273 LBS.

ROOF DIAPHRAM

TRANSVERSE

TOTAL DRAGSTRUT LENGTH = 124 FEET

LOAD RESISTED = 15099 ROOF  
4121 WALL  
19220 TOTAL  
155.0 PLF

7/16" OSB

8d COMMON OR 0.131" DIA. P-NAIL

6"/12"	4"/12"	3"/12"	
357	476	707	PINE
OK ✓	OK	OK	

LONGITUDINAL

TOTAL DRAGSTRUT LENGTH = 112 FEET

LOAD RESISTED = 10300 ROOF  
4136 WALL  
14437 TOTAL  
128.9 PLF

6"/12"	4"/12"	3"/12"	
357	476	707	PINE
OK ✓	OK	OK	

SHEARWALLS

TRANSVERSE

TOTAL SHEARWALL LENGTH = 60 FEET

LOAD RESISTED = 15099 ROOF  
4121 WALL  
19220 TOTAL  
320.3 PLF

7/16" OSB

8d COMMON OR 0.131" DIA. P-NAIL

6"/12"	4"/12"	3"/12"	
364	532	686	PINE
OK ✓	OK	OK	
298	436	563	SPRUCE
N.G.	OK ✓	OK	

LONGITUDINAL

TOTAL SHEARWALL LENGTH = 45 FEET

LOAD RESISTED = 10300 ROOF  
4136 WALL  
14437 TOTAL  
320.8 PLF

6"/12"	4"/12"	3"/12"	
364	532	686	PINE
OK	OK	OK	
298	436	563	SPRUCE
N.G.	OK ✓	OK	

# WALL TENSION TIE USING SHEATHING

WALL TO  
WALL O'HANG  
35 1.5

UPLIFT  
LOAD  
213 PLF

3/8" MINIMUM SHEATHING  
8d COMMON NAIL SPACING  
SPRUCE PINE  
5.3 6.6 INCHES

## ANCHOR BOLT SPACING

WALL TO  
WALL  
35

UPLIFT  
LOAD  
213

1634  
2" ROUND  
WASHER  
92.1 IN.  
MAX

3173  
3" SQ  
WASHER  
178.8 IN.  
MAX

## SHEARWALL ANCHORAGE

### TRANSVERSE

CHORD FORCE = 2588 LBS  
ANCHOR BOLT = 48 INCHES O.C.  
REQUIRED FORCE = 3014 LBS

1634  
2" ROUND  
WASHER

3173  
3" SQ  
WASHER

NO GOOD

OK

### LONGITUDINAL

CHORD FORCE = 2592 LBS  
REQUIRED FORCE = 2592 LBS

NO GOOD

OK

## WALL STUD DESIGN

DESIGN PRESSURES: 22.6 PSF  
27.2 PSF

INTERIOR ZONE  
END ZONE

### INTERIOR ZONE STUDS

STUD LENGTH FEET	SPACING INCHES	INTERIOR MOMENT IN-#	Sx Fb allow	#2 SPF 2X4 3.06 2415	#2 PINE 2X4 3.06 2760	#2 SPF 2X6 7.56 2093	#2 PINE 2X6 7.56 2300	#2 PINE 3X4 5.11 2760
8	16	2893		945 OK	945 OK	383 OK	383 OK	566 OK
0	0	0		0 OK	0 OK	0 OK	0 OK	0 OK
0	0	0		0 OK	0 OK	0 OK	0 OK	0 OK
0	0	0		0 OK	0 OK	0 OK	0 OK	0 OK



END ZONE STUDS WITHIN 3.232 FEET OF CORNERS			END ZONE MOMENT IN-#					
8	16		3482	1138 OK	1138 OK	461 OK	461 OK	681 OK
0	0		0	0 OK	0 OK	0 OK	0 OK	0 OK
0	0		0	0 OK	0 OK	0 OK	0 OK	0 OK
0	0		0	0 OK	0 OK	0 OK	0 OK	0 OK

#### SHEARWALL CAPACITIES PER 2004 FBC

NAIL = 8d COMMON OR 0.131" POWER NAIL  
SHEATHING = 7/16" OSB

6"/12"	4"/12"	3"/12"
260	380	490
0.82	0.82	0.82
1.4	1.4	1.4
298	436	563

Per Table 2306.4.1, using 15/32" sheathing  
as allowed by para 2306.4.1

1.00 = Pine, 0.82 = SPF

Increase per para 2306.4.1

#### SHEARWALL CAPACITIES PER 2001 SBC

NAIL = 8d COMMON OR 0.131" POWER NAIL  
SHEATHING = 7/16" OSB

6"/12"	4"/12"	3"/12"
260	380	490
1	1	1

Per Table 2310.2B, using 15/32" sheathing  
as allowed by para 2310.4.6

1.00 = Pine, 0.82 = SPF

1.4	1.4	1.4	Increase per para 2313.2.5
364	532	686	

Post Uplift Design									
Truss Uplift		193	Pounds per truss at 24" on center						
Post Spacing		12.5	Feet						
Uplift per post		1206.25	Pounds						
Post Base	Allowable			Select ?	Fasteners				
Simpson ABE44	520	No Good			1/2" anchor bolt and (6) 10d common nails				
Simpson ABU44	2200	OK			5/8" anchor bolt and (12) 16d common nails				
Simpson ABE66	900	No Good			5/8" anchor bolt and (8) 16d common nails				
Simpson ABU66	2300	OK		X	5/8" anchor bolt and (12) 16d common nails				
Simpson CBSQ44-SDS2	5335	OK			Imbeded into concrete and (14) Simpson SDS 1/4x2" screws				
Simpson CBSQ66-SDS2	6855	OK			Imbeded into concrete and (14) Simpson SDS 1/4x2" screws				
Post Cap	Allowable			Select ?	Fasteners				
Simpson BC4 (4x4 post, 4" beam)	980	No Good			(6) 16d common nails				
Simpson BC6 (6x6 post, 6" beam)	1050	No Good			(12) 16d common nails				
Simpson CCQ44SDS2.5 (4x4 post, 4" beam)	5680	OK			Simpson SDS 1/4x2" screws - (16) to beam - (14) to post				
Simpson ECCQ44SDS2.5 END CONDITION (4x4 post, 4" beam)	4040	OK			Simpson SDS 1/4x2" screws - (16) to beam - (14) to post				
Simpson CCQ66SDS2.5 (6x6 post, 6" beam)	7145	OK			Simpson SDS 1/4x2" screws - (16) to beam - (14) to post				
Simpson ECCQ66SDS2.5 END CONDITION (6x6 post, 6" beam)	4040	OK			Simpson SDS 1/4x2" screws - (16) to beam - (14) to post				
Simpson CCQ46SDS2.5 (6x6 post, 4" beam)	7145	OK		X	Simpson SDS 1/4x2" screws - (16) to beam - (14) to post				
Simpson ECCQ46SDS2.5 END CONDITION (6x6 post, 4" beam)	4040	OK		X	Simpson SDS 1/4x2" screws - (16) to beam - (14) to post				

Sputo and Lammert Engineering, LLC  
 10 SW 1st Avenue  
 Gainesville, FL 32601  
 Phone 352-378-0448  
 CA 6855

Title :  
 Dsgnr:  
 Description :

Job #  
 Date: 1:14PM, 6 FEB 06

Scope :

Rev. 580004  
 User: KW-0602180, Ver 5.8.0, 1-Dec-2003  
 (c)1983-2003 ENERCALC Engineering Software

## General Timber Beam

Page 1

Description Garage Beam

### General Information

Code Ref: 1997/2001 NDS, 2000/2003 IBC, 2003 NFPA 5000. Base allowables are user defined

Section Name	3-2x12	Center Span	16.33 ft	Lu	0.00 ft
Beam Width	4.500 in	Left Cantilever	ft	Lu	0.00 ft
Beam Depth	11.250 in	Right Cantilever	ft	Lu	0.00 ft
Member Type	Sawn	Southern Pine, No.2 2 -4 Thick, 12 Wid			
		Fb Base Allow	975.0 psi		
Load Dur. Factor	1.250	Fv Allow	175.0 psi		
Beam End Fixity	Pin-Pin	Fc Allow	565.0 psi		
		E	1,600.0 ksi		

### Full Length Uniform Loads

Center	DL	234.00 #/ft	LL	#/ft
Left Cantilever	DL	#/ft	LL	#/ft
Right Cantilever	DL	#/ft	LL	#/ft

### Summary

Beam Design OK

Span= 16.33ft, Beam Width = 4.500in x Depth = 11.25in, Ends are Pin-Pin

Max Stress Ratio 0.809 : 1

Maximum Moment Allowable 7.8 k-ft 9.6 k-ft Maximum Shear \* 1.5 Allowable 2.5 k 11.1 k

Max. Positive Moment	7.80 k-ft	at	8.165 ft	Shear:	@ Left	1.91 k
Max. Negative Moment	0.00 k-ft	at	0.000 ft		@ Right	1.91 k
Max @ Left Support	0.00 k-ft			Camber:	@ Left	0.000 in
Max @ Right Support	0.00 k-ft				@ Center	0.657 in
Max. M allow	9.64				@ Right	0.000 in
fb	986.08 psi	fv	50.27 psi	Reactions...		
Fb	1,218.75 psi	Fv	218.75 psi	Left DL	1.91 k	Max 1.91 k
				Right DL	1.91 k	Max 1.91 k

### Deflections

Center Span...	Dead Load	Total Load	Left Cantilever...	Dead Load	Total Load
Deflection	-0.438 in	-0.438 in	Deflection	0.000 in	0.000 in
...Location	8.165 ft	8.165 ft	...Length/Defl	0.0	0.0
...Length/Defl	447.1	447.14	Right Cantilever...		
Camber ( using 1.5 * D.L. Defl ) ...			Deflection	0.000 in	0.000 in
@ Center	0.657 in		...Length/Defl	0.0	0.0
@ Left	0.000 in				
@ Right	0.000 in				

### Stress Calcs

Bending Analysis					
Ck	29.385	Le	0.000 ft	Sxx	94.922 in3
Cf	1.000	Rb	0.000	Cl	0.000
				Area	50.625 in2
		Max Moment		Sxx Req'd	Allowable fb
@ Center		7.80 k-ft		76.80 in3	1,218.75 psi
@ Left Support		0.00 k-ft		0.00 in3	1,218.75 psi
@ Right Support		0.00 k-ft		0.00 in3	1,218.75 psi
Shear Analysis		@ Left Support		@ Right Support	
Design Shear		2.54 k		2.54 k	
Area Required		11.634 in2		11.634 in2	
Fv: Allowable		218.75 psi		218.75 psi	
Bearing @ Supports					
Max. Left Reaction		1.91 k		Bearing Length Req'd	0.751 in
Max. Right Reaction		1.91 k		Bearing Length Req'd	0.751 in

Sputo and Lammert Engineering, LLC  
10 SW 1st Avenue  
Gainesville, FL 32601  
Phone 352-378-0448  
CA 6855

Title :  
Dsgnr:  
Description :

Job #  
Date: 1:14PM, 6 FEB 06

Scope :

Rev: 580004  
User: KW-0602180, Ver 5.8.0, 1-Dec-2003  
(c)1983-2003 ENERCALC Engineering Software

## General Timber Beam

Page 2

Description      Garage Beam

### Query Values

M, V, & D @ Specified Locations		Moment	Shear	Deflection
@ Center Span Location =	0.00 ft	0.00 k-ft	1.91 k	0.0000 in
@ Right Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in
@ Left Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in

- 2. All elevation drawings shown on plans (pages 3 & 4) show a correct roof pitch of 7/12.**



**#3. Glazing windows. This new house is the same as our last house. Please notice in the supplied picture that the hot tub/spa is a stand alone or portable and can be moved. The hot tub is on our screened in porch and entrance is more than 6 feet from any windows or glass.**

THIS ITEM SHOULD BE ADDRESSED IN THE SPUTO REVISED WIND LOAD  
ANALYSIS SHOWN IN ITEM NUMBER 1.



## NOVEMBER 2003

DWG# 1994  
NO.


**SCHLAPPE D**  
**Schneider Electric**

[illegible]



**Gale Insulation**  
13450 N W 104th Terrace, Alachua, FL,  
Tel: (352) 332-0401, Fax: (352) 332-0402  
**WORK AGREEMENT**



<b>TO: RON OLSZAK</b>		<b>RE: OLSR-IN-OLSZAK RES.</b>	
<b>Address: 200 SW BAY PLACE, ***** COD ***** FORT WHITE, FL, 32038</b>		<b>Address: 200 SW BAY PLACE FORT WHITE, FL, 32038</b>	
<b>Contact:</b>	<b>Date: 02/15/2006</b>	<b>Expiration Date: 03/17/2006</b>	
<b>Tel:</b>	<b>Estimator: Mark Houghtaling</b>	 <b>P 34A3987-1</b>	
<b>Fax:</b>			

**Scope of Work (the "Work") to be performed:**

Draft stop, fire block, fire stop (UBC 708.2.1 et seq., formerly 2516(f), or locally adopted equivalent), fire rated caulking are not included within Contractor's scope of work unless specifically listed below.

**WARNING:** If cellulose is to be applied with a wet application you must allow adequate time for it to cure and dry before installing drywall or other materials. The adequate time required varies depending upon climate and weather. Be sure to schedule your trades accordingly.

**Contractor is willing to furnish to you all material and labor required for the application of:**

Model Name:	Permit #:	Plan #:
<b>Work Area</b> Ceiling Area Porch  Knee Wall Exterior Walls Sound Walls Garage Common Wall	<b>Product</b> Kraft Faced, R-30, 24" Kraft Faced, R-30, 24" Foam <u>High Heat Caulk</u> Kraft Faced, R-19, 23" Unfaced, R-19, 15" Kraft Faced, R-13, 15" Kraft Faced, R-19, 15" Baffles - 33" Cardboard	<b>Notes</b>  Back Porch  And In Skylight

**Base Price:** [REDACTED]

**Notes:**

IF COCOON INSULATION IS USED IN THE WALLS & CEILING TAKE \$2074.00 OFF OF THE PRICE.

for the total of [REDACTED]

X R.O.

**READ THE LAST PAGE OF THIS AGREEMENT. IT CONTAINS IMPORTANT PROVISIONS AND IS PART OF THIS AGREEMENT.**

**TERMS OF PAYMENT:** Payment in full due as stated on invoice regardless of any payment arrangements you have with third parties.

**ACCEPTANCE:** Contractor may change and/or withdraw this agreement if Contractor does not receive your signed acceptance within 10 business days after the date listed above.

**PRICING:** Any additional work performed is subject to Contractor's then current pricing (unless Contractor otherwise agrees in writing) and to these terms and conditions. The prices above shall remain firm for 90 days after the date You sign this agreement. If performance of this agreement extends beyond those 90 days, You agree to pay Contractor's then current pricing for the Work performed after those 90 days.

**CUSTOMER:**  
Date 2/21/06  
Company Name \_\_\_\_\_  
By: [Signature]  
SIGNATURE TITLE

**CONTRACTOR:**  
By: [Signature]  
SIGNATURE TITLE  
**Mark Houghtaling**

**THE INFORMATION CONTAINED IN THIS AGREEMENT IS CONFIDENTIAL. NEITHER THIS AGREEMENT NOR ITS TERMS MAY BE DISCLOSED TO THIRD PARTIES.**