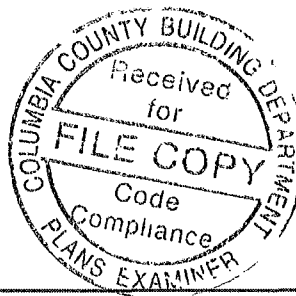


**JULIUS LEE PE.**



RE 484136 - THOMAS CONST - GOOTEE RES

**1109 COASTAL BAY BLVD,  
BOYNTON BEACH, FL 33435**

**Site Information:**

Project Customer Thomas Const Project Name 484136 Model Gootee Res  
Lot/Block Subdivision  
Address 630 SW Dyal Ave  
City Columbia Cty State FL

**Name Address and License # of Structural Engineer of Record, If there is one, for the building.**

Name Unknown at time of seal License # Unknown at time of seal  
Address Unknown at time of seal  
City Unknown at time of seal State Unknown at time of seal

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code FBC2010/TPI2007 Design Program MiTek 20/20 7 3  
Wind Code ASCE 7-10 Wind Speed: 130 mph Floor Load N/A psf  
Roof Load 32.0 psf

This package includes 28 individual, dated Truss Design Drawings and 0 Additional Drawings  
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet  
conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules  
This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

**In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany  
this coversheet. The latest approval dates supersede and replace the previous drawings.**

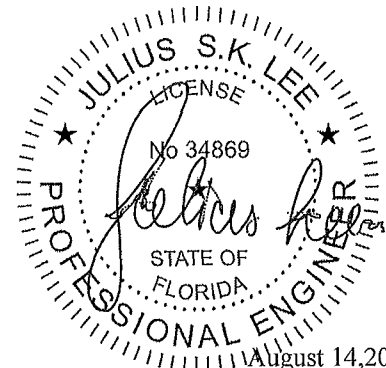
No	Seal#	Truss Name	Date	No	Seal#	Truss Name	Date
1	I7121956	CJ01	8/14/013	18	I7121973	T07	8/14/013
2	I7121957	CJ02	8/14/013	19	I7121974	T08	8/14/013
3	I7121958	CJ03	8/14/013	20	I7121975	T10	8/14/013
4	I7121959	CJ04	8/14/013	21	I7121976	T11	8/14/013
5	I7121960	CJ05	8/14/013	22	I7121977	T12	8/14/013
6	I7121961	EJ01	8/14/013	23	I7121978	T13	8/14/013
7	I7121962	EJ02	8/14/013	24	I7121979	T14	8/14/013
8	I7121963	EJ03	8/14/013	25	I7121980	T15	8/14/013
9	I7121964	EJ04	8/14/013	26	I7121981	T16	8/14/013
10	I7121965	HJ01	8/14/013	27	I7121982	T17	8/14/013
11	I7121966	HJ02	8/14/013	28	I7121983	T18	8/14/013
12	I7121967	T01	8/14/013				
13	I7121968	T01G	8/14/013				
14	I7121969	T02	8/14/013				
15	I7121970	T04	8/14/013				
16	I7121971	T05	8/14/013				
17	I7121972	T06	8/14/013				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name Julius Lee

My license renewal date for the state of Florida is February 28, 2015.

**NOTE:** The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2



August 14, 2013

August 14, 2013

August 14, 2013

Job 484136	Truss CJ05	Truss Type Jack-Open Truss	Qty 1	Ply 1	THOMAS CONST GOOTEE RES. Job Reference (optional)	17121960
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Builders FirstSource, Lake City FL 32055 7.350 s Sep 27 2012 MITek Industries Inc. Wed Aug 14 13:20:26 2013 Page 1  
 ID YkTbIFn1AcVXP6G5YcdV6IzUA0A-WUJlGif3REUD0rCXckxXGE7546Pez4VaknbNQCYmwL

Plate Offsets (X,Y): [2 0-1 12,0-0-9]								
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	In (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.17	Vert(LL) -0.00	10	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.02	Vert(TL) 0.00	10	>999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)					Weight: 10 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

SLIDER Left 2x4 SP No 3 1-6-0

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 1 7 13 oc purlins

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

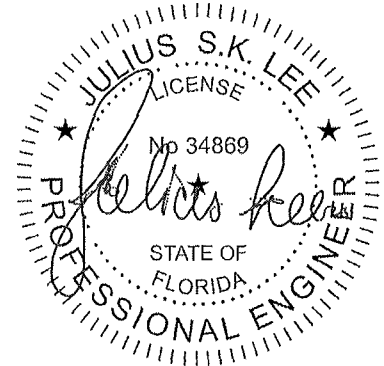
**REACTIONS** (lb/size) 2=140/0-3-8 (min 0-1-8) 4=12/Mechanical 7=3/Mechanical  
 Max Horz 2=73(LC 12) 4=32(LC 12) 7=11(LC 9)  
 Max Uplift 2=78(LC 12) 4=32(LC 12) 7=11(LC 9)  
 Max Grav 2=170(LC 2), 4=14(LC 2) 7=21(LC 3)

**FORCES** (lb) Max Comp/Max Ten All forces 250 (lb) or less except when shown

**NOTES** (7-9)

- 1) Wind: ASCE 7 10' Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3.0psf h=18ft, Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone end vertical left exposed porch left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2 0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 2 32 lb uplift at joint 4 and 11 lb uplift at joint 7
- 6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- 8) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB
- 9) Truss Design Engineer: Julius Lee PE Florida P E License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard



August 14,2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.  
 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB 89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719

Julius Lee PE  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	THOMAS CONST GOOTEE RES	17121962
484136	EJ02	Jack-Partial Truss	1	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055

7.350 s Sep 27 2012 MITek Industries Inc. Wed Aug 14 13:20:27 2013 Page 1

ID YkTbIFn1AcVXP6G5YcdV6IzUA0A-gh7I2ghCXc4e7nkARSmpRYEkWhUIWckzRLwyeynwl

Scale = 1/20

Plate Offsets (X,Y) [2 0-3-8,Edge], [4 0-6-0,0-2-8]	
LOADING (psf)	SPACING 2-0-0
TCLL 20.0	Plates Increase 1.25
TCDL 7.0	Lumber Increase 1.25
BCLL 0.0 *	Rep Stress Incr YES
BCDL 5.0	Code FBC2010/TPI2007
CSI	DEFL
TC 0.31	in (loc) I/defl L/d
BC 0.30	Vert(LL) 0.05 9-12 >999 240
WB 0.07	Vert(TL) -0.04 9-12 >999 180
(Matrix-M)	Horz(TL) -0.02 2 n/a n/a
	PLATES GRIP
	MT20 244/190
	Weight 39 lb FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-6-0

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS** (lb/size) 2~254/0-3-8 (min 0-1-8) 8=181/Mechanical

Max Horz 2=108(LC 12)

Max Uplift 2=72(LC 12) 8=67(LC 12)

Max Grav 2=305(LC 2) 8=213(LC 2)

**FORCES** (lb) Max Comp/Max Ten All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-352/179

BOT CHORD 2-9=-291/205

WEBS 4-8=-275/248

**NOTES** (9-11)

1) Unbalanced roof live loads have been considered for this design

2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 psf BCDL=3.0psf h=18ft, Cat. II Exp C, Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone end vertical left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip

3) Provide adequate drainage to prevent water ponding

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

5) \* This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members

6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 2 and 67 lb uplift at joint 8.

8) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code

10) Note: Visually graded lumber designation SPp represents new lumber design values as per SP1B

11) Truss Design Engineer Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard



August 14, 2013



**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute 583 D'Oro Drive, Madison, WI 53719

Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 484136	Truss EJ04	Truss Type Jack-Partial Truss	Qty 2	Ply 1	THOMAS CONST GOOTEE RES Job Reference (optional)	17121064
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Builders FirstSource Lake City FL 32055 7 350 s Sep 27 2012 MITek Industries, Inc. Wed Aug 14 13:20:32 2013 Page 1  
 ID YKtBtFn1AcVXP6G5YcdV6IzUA0A Le50xIjq14EMkmfhy\_2xWVF2uXNn9T6j2hasynwD

1-6-0 1-6-0 6-3-8 6-3-8

Scale = 1/16"

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.41	Vert(LL) 0 16 6-9 >463 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.36	Vert(TL) 0 13 6-9 >541 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(TL) -0.01 2 n/a n/a	
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)		Weight: 23 lb FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

**REACTIONS** (lb/size) 2=259/0-3-8 (min 0-1-8), 6=138/0-3-8 (min. 0-1-8)

Max Horz 2=100(LC 8)

Max Uplift 2=282(LC 8) 6=155(LC 8)

Max Grav 2=310(LC 2) 6=162(LC 2)

**FORCES** (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-601/1160

BOT CHORD 2-6=-1292/659

**NOTES** (7-9)

- 1) Wind: ASCE 7 10' Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=18ft, Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone\* porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 585 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 2 and 155 lb uplift at joint 6
- 6) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- 8) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
- 9) Truss Design Engineer Julius Lee, PE, Florida P E License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

August 14, 2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.  
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 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	THOMAS CONST GOOTEE RES.	I7121955
484136	HJ01	Diagonal Hip Girder	4	1	Job Reference (optional)	
Builders FirstSource		Lake City FL 32055	7 350 s Sep 27 2012 MiTek Industries Inc. Wed Aug 14 13:20 33 2013 Page 2			
ID YkTbIFn1AcVXP6G5YcdV6izUA0A-pqfO85kSoNMDMwEuWIZA3io9ZxgP8AGcLNoFAIynw1C						
LOAD CASE(S) Standard						
Concentrated Loads (lb)						
Vert. 3=49(F=24 B=24) 13=-5(F=-2 B=-2) 14=-90(F=-45 B=-45) 15=23(F=11 B=11) 16=-12(F=-6, B=-6) 17=-48(F=-24 B=-24)						

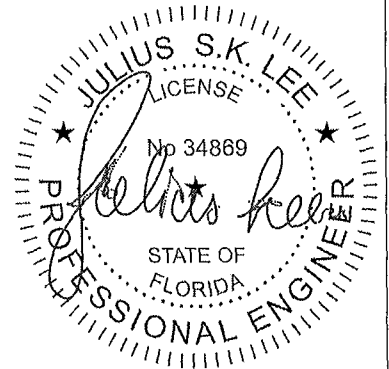


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Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 484136	Truss TD1	Truss Type Common Truss	Qty 8	Ply 1	THOMAS CONST GOOTEE RES	17121987																																				
Builders FirstSource Lake City FL 32055					7.350 s Sep 27 2012 MITek Industries, Inc. Wed Aug 14 13:20:34 2013 Page 1																																					
					ID YkTbIFn1AcVXP6G5YcdV6IzUA0A-H0CnLR14ZhU4_4p44P4PbwKHpKwErbRma1Xolkynw18																																					
Scale 1/4" = 1'-0"																																										
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:25%;">LOADING (psf)</td> <td style="width:25%;">SPACING</td> <td style="width:10%;">CSI</td> <td style="width:10%;">DEFL</td> <td style="width:10%;">PLATES</td> <td style="width:10%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>2-0-0</td> <td>TC 0.89</td> <td>in (loc) l/defl L/d</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Plates Increase 1.25</td> <td>BC 0.93</td> <td>Ver(LL) 0.41 10-12 &gt;645 240</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Lumber Increase 1.25</td> <td>WB 0.39</td> <td>Ver(TL) -0.64 10-12 &gt;412 180</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Rep Stress Incr NO</td> <td>(Matrix-M)</td> <td>Horz(TL) 0.07 8 n/a n/a</td> <td></td> <td></td> </tr> <tr> <td></td> <td>Code FBC2010/TPI2007</td> <td></td> <td></td> <td>Weight. 109 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP	TCLL 20.0	2-0-0	TC 0.89	in (loc) l/defl L/d	MT20	244/190	TCDL 7.0	Plates Increase 1.25	BC 0.93	Ver(LL) 0.41 10-12 >645 240			BCLL 0.0 *	Lumber Increase 1.25	WB 0.39	Ver(TL) -0.64 10-12 >412 180			BCDL 5.0	Rep Stress Incr NO	(Matrix-M)	Horz(TL) 0.07 8 n/a n/a				Code FBC2010/TPI2007			Weight. 109 lb	FT = 20%
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TCLL 20.0	2-0-0	TC 0.89	in (loc) l/defl L/d	MT20	244/190																																					
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	Code FBC2010/TPI2007			Weight. 109 lb	FT = 20%																																					
<p><b>LUMBER</b></p> <p>TOP CHORD 2x4 SP No.2</p> <p>BOT CHORD 2x4 SP No.2 *Except*</p> <p>WEBS 2x4 SP No.3</p> <p>SLIDER Left 2x4 SP No.3 1-6-0 Right 2x4 SP No.3 1-6-0</p> <p><b>BRACING</b></p> <p>TOP CHORD Structural wood sheathing directly applied or 2-8-10 oc purlins</p> <p>BOT CHORD Rigid ceiling directly applied or 8-5-12 oc bracing</p> <p style="border: 1px solid black; padding: 2px; margin-top: 5px;">MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> <p><b>REACTIONS</b> (lb/size) 2=898/0-3-8 (min 0-1-8) 8=898/0-3-8 (min 0-1-8)</p> <p>Max Horz 2=131(LC 12)</p> <p>Max Uplift 2=480(LC 12) 8=480(LC 13)</p> <p>Max Grav 2=1021(LC 2) 8=1021(LC 2)</p> <p><b>FORCES</b> (lb) Max. Comp./Max Ten All forces 250 (lb) or less except when shown</p> <p>TOP CHORD 2-3=-386/58 3-4=-1821/1005 4-5=-1734/1024 5-6=-1726/1021 6-7=-1814/1002 7-8=-380/48</p> <p>BOT CHORD 2-12=-753/1578 12-21=-431/1106 21-22=-431/1106 11-22=-431/1106 10-11=-431/1106</p> <p>WEBS 8-10=-758/1588</p> <p>5-10=-393/696, 5-12=-398/707</p> <p><b>NOTES</b> (9-11)</p> <p>1) Unbalanced roof live loads have been considered for this design</p> <p>2) Wind ASCE 7 10' Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=18ft, Cal. II Exp C, Encl GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone C-C for members and forces &amp; MWFRS for reactions shown: Lumber DOL=1.60 plate grip DOL=1.60</p> <p>3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</p> <p>4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf.</p> <p>5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi</p> <p>6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 480 lb uplift at joint 2 and 480 lb uplift at joint 8</p> <p>7) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss</p> <p>8) In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)</p> <p>9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code</p> <p>10) Note: Visually graded lumber designation SPP represents new lumber design values as per SPIB.</p> <p>11) Truss Design Engineer: Julius Lee PE, Florida P E License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435</p> <p><b>LOAD CASE(S)</b> Standard</p> <p>1) Regular: Lumber Increase=1.25 Plate Increase=1.25</p> <p>Uniform Loads (plf)</p> <p>Vert. 1-5=-44 5-9=-44 12-13=-10, 12-21=-61(F=-51) 21-22=-91(F=-51) 10-22=-61(F=-51) 10-17=-10</p>																																										



August 14, 2013

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Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435



Job 484136	Truss T02	Truss Type Common Truss	Qty 3	Ply 1	THOMAS CONST GOOTEE RES.	17121969																																																							
Builders FirstSource Lake City FL 32055		7.350 s Sep 27 2012 MiTek Industries Inc. Wed Aug 14 13:20:37 2013 Page 1 ID YKtBlFn1AcVXP6G5YcdV6lzUA0A-hbuV_TnzscsfXyflYe6DYyoCYyC2y5CG?mSJ3ynw8																																																											
<table border="1"> <tr> <td>LOADING (psf)</td> <td>SPACING</td> <td>2-0-0</td> <td>CSI</td> <td>DEFL</td> <td>in</td> <td>(loc)</td> <td>l/defl</td> <td>L/d</td> <td>PLATES</td> <td>GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase</td> <td>1.25</td> <td>TC 0.88</td> <td>Ver(LL)</td> <td>0.41</td> <td>9-11</td> <td>&gt;651</td> <td>240</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Lumber Increase</td> <td>1.25</td> <td>BC 0.91</td> <td>Ver(TL)</td> <td>-0.63</td> <td>9-11</td> <td>&gt;421</td> <td>180</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0</td> <td>Rep Stress Incr</td> <td>NO</td> <td>WB 0.39</td> <td>Horz(TL)</td> <td>0.07</td> <td>8</td> <td>n/a</td> <td>n/a</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Code FBC2010/TPI2007</td> <td></td> <td>(Matrix-M)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>Weight: 107 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	TCLL 20.0	Plates Increase	1.25	TC 0.88	Ver(LL)	0.41	9-11	>651	240	MT20	244/190	TCDL 7.0	Lumber Increase	1.25	BC 0.91	Ver(TL)	-0.63	9-11	>421	180			BCLL 0.0	Rep Stress Incr	NO	WB 0.39	Horz(TL)	0.07	8	n/a	n/a			BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)						Weight: 107 lb	FT = 20%
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<p>REACTIONS (lb/size) 8=825/0-3-8 (min. 0-1-8) 2=896/0-3-8 (min 0-1-8)</p> <p>Max Horz 2=89(LC 16)</p> <p>Max Uplift 8=241(LC 13) 2 =270(LC 12)</p> <p>Max Grav 8=837(LC 2) 2=1023(LC 2)</p>																																																													
<p>FORCES (lb) Max. Comp./Max Ten. All forces 250 (lb) or less except when shown</p> <p>TOP CHORD 2-3=-391/62 3-4=-1817/1012 4-5=-1730/1031 5-6=-1719/1036 6-7=-1806/1017 7-8=-404/115</p> <p>BOT CHORD 2-11=-803/1544 11-20=-473/1057 20-21=-473/1057 10-21=-473/1057 9-10=-473/1057</p> <p>8-9= 809/1532</p> <p>WEBS 5-9=-403/686 5-11=-395/701</p>																																																													
<p>NOTES (9-11)</p> <p>1) Unbalanced roof live loads have been considered for this design</p> <p>2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCCL=4 2psf BCDL=3.0psf h=18ft, Cat. II Exp C, Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</p> <p>3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads</p> <p>4) * This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf</p> <p>5) All bearings are assumed to be SP No 2 crushing capacity of 565 psi</p> <p>6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=241 2=270.</p> <p>7) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.</p> <p>8) In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)</p> <p>9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.</p> <p>10) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB</p> <p>11) Truss Design Engineer: Julius Lee PE, Florida P.E. License No 34869. Address: 1109 Coastal Bay Blvd. Boynton Beach FL 33435</p>																																																													
<p>LOAD CASE(S) Standard</p> <p>1) Regular: Lumber Increase=1.25 Plate Increase=1.25</p> <p>Uniform Loads (plf)</p> <p>Vert: 1-5=-44 5-8=-44 11 16=-10, 11-20=-61(F=-51) 20-21=-91(F=-51) 9-21=-61(F=-51) 9-12=-10</p>																																																													



August 14,2013

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Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.  
Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB 89 and BC511 Building Component Safety Information available from Truss Plate Institute 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	THOMAS CONST GOOTEE RES	17121970
484136	T04	Hip Truss	1	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Aug 14 13:20:39 2013 Page 2  
ID`YkTbIFn1AcVXP6G5YcdV6izUA0A-d\_0fP8pDND7N4r1tygalz29ZLd6VWkqVJlFZNyywnw6

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code

12) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB

13) Truss Design Engineer: Julius Lee PE, Florida P E License No. 34869, Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-44 3-6=-44, 6-7=-44 7-9=-44 8-15=-10

Concentrated Loads (lb)

Vert: 3=-86(F) 6=-86(F) 14=-252(F) 11=-40(F) 20=-86(F) 21=-86(F) 22=-86(F) 23=-86(F) 24=-86(F) 25=-86(F) 26=-40(F) 27=-40(F) 28=-40(F) 29=-40(F) 30=-40(F) 31=-40(F) 32=-171(F) 33=-171(F)



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Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 484136	Truss T06	Truss Type Hip Truss	Qty 1	Ply 1	THOMAS CONST GOOTEE RES.	17121972
Builders FirstSource Lake City FL 32055		7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Aug 14 13:20:41 2013 Page 1				
		ID YkTbIFn1AcVXP6G5YcdV6IzUA0A-aM7QpqqTvrN5J9sQ_Ni2NO7aH9QB_j0oBckgSrynwl				
<div style="display: flex; justify-content: space-between;"> <div> 1-6-0   5-11-3   11-0-0   17-3-0   22-0-6   26-0-0   32-0-0   33-6-0  1-6-0   5-11-3   5-0-13   6-3-0   4-9-6   3-11-11   6-0-0   1-6-0 </div> <div style="text-align: right;">Scale 1/8" = 1'-0"</div> </div>						
<div style="display: flex; justify-content: space-between;"> <div> 5-11-3   11-0-0   17-3-0   25-10-4   26-0-0   32-0-0  5-11-3   5-0-13   6-3-0   8-7-4   0-1'-12"   6-0-0 </div> <div></div> </div>						
Plate Offsets (X,Y) [2 0-4-9,Edge], [6 0-6-0,0-2-8], [9 0-3-7,Edge]						
<b>LOADING (psf)</b> TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 5.0		<b>SPACING</b> 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007		<b>CSI</b> TC 0.49 BC 0.46 WB 0.59 (Matrix-M)		<b>DEFL</b> in (loc) l/def l/d Vert(LL) 0.09 11 22 >798 240 Vert(TL) -0.22 11 12 >999 180 Horz(TL) 0.04 11 n/a n/a
				<b>PLATES</b> MT20 <b>GRIP</b> 244/190 Weight: 166 lb FT = 20%		
<b>LUMBER</b> TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.3 1-6-0						
<b>BRACING</b> TOP CHORD Structural wood sheathing directly applied or 4-4-5 oc purlins BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing WEBS 1 Row at midpt 6-14 <div style="border: 1px solid black; padding: 2px; margin-top: 5px;">MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</div>						
<b>REACTIONS</b> (lb/size) 2=753/0-3-8 (min 0-1-8) 11=908/0-3-8 (min 0-1-8) 9=199/0-3-8 (min. 0-1-8) Max Horz 2=-81(LC 10) Max Uplift 2=-217(LC 12) 11=-226(LC 13) 9=-214(LC 9) Max Grav 2=895(LC 2) 11=1073(LC 2) 9=255(LC 28)						
<b>FORCES</b> (lb) Max Comp./Max. Ten All forces 250 (lb) or less except when shown TOP CHORD 2-3=-438/90 3-4=-1365/790 4-5=-1092/690 5-6=-953/674 6-7=-987/622 8-9=-354/715 BOT CHORD 2-15=-555/1141 14-15=-555/1141 13-14=-273/778 12 13=-273/778 11 12=-334/693 9-11=-750/385 WEBS 4-14=-330/274 7 11=-1193/634						
<b>NOTES</b> (9-11) 1) Unbalanced roof live loads have been considered for this design 2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone end vertical left exposed porch right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=lb) 2=217 11=226 9=214 8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss. 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 10) Note Visually graded lumber designation SPP represents new lumber design values as per SPIB. 11) Truss Design Engineer Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435						
LOAD CASE(S) Standard						

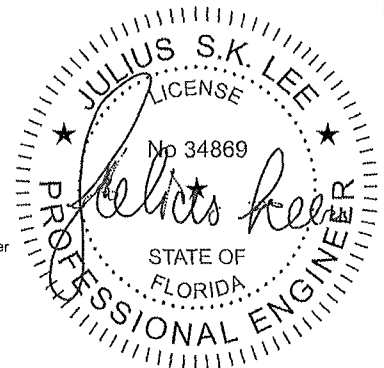
August 14,2013

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1109 Coastal Bay  
Boynton Beach, FL 33435



Job 484136	Truss T08	Truss Type Special Truss	Qty 1	Ply 1	THOMAS CONST GOOTEE RES	17121974																											
Builders FirstSource Lake City FL 32055		7 350 s Sep 27 2012 MITek Industries Inc. Wed Aug 14 13 20:43 2013 Page 1 ID: YKtBlFn1AcVXP6G5YcdV6IzUA0A-WiFAEWskRSdpZS7p6oIWSpCrZz4iSfw5ewDmVjynwI2																															
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Scale 1:500																																	
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<b>REACTIONS</b> (lb/size) 2=815/0-3-8 (min 0-1-8) 11=938/0-3-8 (min 0-1-8) 9=226/0-3-8 (min 0-1-8) Max Horz 2=-102(LC 10) Max Uplift 2=-232(LC 12) 11=-251(LC 13) 9=-213(LC 9) Max Grav 2=903(LC 2) 11=1037(LC 2) 9=285(LC 28)																																	
<b>FORCES</b> (lb) Max. Comp./Max Ten. All forces 250 (lb) or less except when shown TOP CHORD 2-3=-599/79 3-4=-1538/648 4-5= 1377/792 5-6= 1297/818 6-7= 1143/693 8-9=-392/694 BOT CHORD 2-14=-610/1299 14-22=-219/780 22-23=-219/780 13-23=-219/780 12 13=-219/780 11 12=-383/934 9-11=-726/420 WEBS 4-14=-380/384 6-14=-311/531 6-12=-125/265 7 11=-1290/631 8-11 293/284																																	
<b>NOTES</b> (8-10) 1) Unbalanced roof live loads have been considered for this design 2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=18ft; Cat. II Exp C, Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone end vertical left exposed porch right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads. 4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5 0psf 5) All bearings are assumed to be SP No 2 crushing capacity of 565 psi 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j)=2 232 11=251 9=213. 7) Semi-rigid pitchbreaks including heels Member and fixity model was used in the analysis and design of this truss. 8) This manufactured product is designed as an individual building component The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 9) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB. 10) Truss Design Engineer Julius Lee PE Florida P.E. License No 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435																																	
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Job 484138	Truss T11	Truss Type Special Truss	Qty 2	Ply 1	THOMAS CONST GOOTEE RES	17121976																																				
Builders FirstSource Lake City FL 32055		7.350 s Sep 27 2012 MITek Industries, Inc. Wed Aug 14 13 20:46 2013 Page 1																																								
<div style="display: flex; justify-content: space-between;"> <span>ID:YKtBlFn1AcVXP6G5YcdV8IzUA0A-wKxJiYuckN?NQwkNnxID4SqMwa5Tl7SXKuRR72ynw?</span> <span>Job Reference (optional)</span> </div>																																										
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<b>REACTIONS</b> (lb/size) 1=747/0-3-8 (min 0-1-8) 9=937/0-3-8 (min 0-1-8) 7=228/0-3-8 (min 0-1-8) Max Horz 1=-110(LC 13) Max Uplift 1 =-203(LC 12) 9=-251(LC 13) 7=-213(LC 9) Max Grav 1=820(LC 2) 9=1036(LC 2) 7=286(LC 28)																																										
<b>FORCES</b> (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown TOP CHORD 1 2=-617/144 2-3=-1548/858, 3-4=-1385/827 4-5=-1146/697 6-7=-394/698 BOT CHORD 1 12=-632/1315 12-20=-225/785, 20-21=-225/785 11 21=-225/785 10-11=-225/785 WEBS 9-10=-390/940 7-9=-729/422 3-12=-385/389 4-12=-319/538 4-10=-125/265, 5-9=-1285/625 6-9=-295/287																																										
<b>NOTES</b> (8-10) 1) Unbalanced roof live loads have been considered for this design. 2) Wind, ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone porch right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) * This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0 psf 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=203 9=251 7=213. 7) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss. 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 9) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB. 10) Truss Design Engineer Julius Lee PE Florida P.E. License No. 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435																																										
<b>LOAD CASE(S)</b> Standard																																										



August 14,2013

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Julius Lee PE  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	THOMAS CONST GOOTEE RES.	17121977
484136	T12	Half Hip Truss	1	1	Job Reference (optional)	
Builders FirstSource		Lake City FL 32055	7 350 s Sep 27 2012 MiTek Industries Inc. Wed Aug 14 13:20:47 2013 Page 2			
			ID YkTbIFn1AcVXP6GSYcdV6IzUA0A-QWVh4tvEVh7E24JaLepSdfNTMaMhOOlgZYB_gUynwI			
<b>LOAD CASE(S)</b> Standard						
1) Regular: Lumber Increase=1.25, Plate Increase=1.25						
Uniform Loads (plf)						
Vert: 1-4=-44 4-8=-44 9-13=-10						
Concentrated Loads (lb)						
Vert. 4=-86(B) 6=-86(B) 8=-130(B) 12=-252(B) 5=-86(B) 16= 86(B) 17=-86(B) 18=-86(B) 19=-86(B) 20=-86(B) 21=-86(B) 22=-86(B) 23=-40(B) 24=-40(B) 25=-40(B) 26=-40(B) 27=-40(B)						
28=-40(B) 29=-40(B) 30=-40(B) 31=-40(B)						



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Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435



Job 484136	Truss T16	Truss Type Common Truss	Qty 4	Ply 1	THOMAS CONST GOOTEE RES.	(7121981)																																																																														
Builders FirstSource Lake City FL 32055		Job Reference (optional)																																																																																		
7 350 s Sep 27 2012 MiTek Industries Inc. Wed Aug 14 13:20:51 2013 Page 1																																																																																				
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<p><b>REACTIONS</b> (lb/size) 1=754/0-3-8 (min. 0-1-8) 7=760/0-3-8 (min. 0-1-8)</p> <p>Max Horz 1=114(LC 12)</p> <p>Max Uplift 1=204(LC 12) 7=192(LC 13)</p> <p>Max Grav 1=827(LC 2) 7=827(LC 2)</p>																																																																																				
<p><b>FORCES</b> (lb) Max Comp./Max. Ten. All forces 250 (lb) or less except when shown</p> <p>TOP CHORD 1-2=-626/147 2-3=-1563/864 3-4=-1401/835 4-5=-1174/713</p> <p>BOT CHORD 1 10=-740/1346 10-15=-336/819 9-15=-336/819 9-16=-336/819 8-16=-336/819 7-8=-511/994</p> <p>WEBS 3-10=-384/389 4-10=-316/536, 4-8= 140/288 5-7=-1186/606</p>																																																																																				
<p><b>NOTES</b> (8-11)</p> <p>1) Unbalanced roof live loads have been considered for this design</p> <p>2) Wind ASCE 7 10- Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=18ft, Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces &amp; MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60</p> <p>3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads</p> <p>4) * This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0 psf</p> <p>5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi</p> <p>6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=204 7=192.</p> <p>7) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.</p> <p>8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code</p> <p>9) Note: Visually graded lumber designation SPP represents new lumber design values as per SPIB</p> <p>10) Truss Design Engineer: Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435</p> <p>11) Use Simpson HTU26 to attach Truss to Carrying member</p>																																																																																				
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August 14, 2013

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Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435



Job 484136	Truss T18	Truss Type Common Truss	Qty 2	Ply 1	THOMAS CONST GOOTEE RES.	17121983																																																																															
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<b>FORCES</b> (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3= 297/350, 3-4=656/1012 4-5=656/1012 5-6=297/350 BOT CHORD 2-8=-741/515 6-8=-741/515 WEBS 4-8=-634/259																																																																																					
<b>NOTES</b> (8-10) 1) Unbalanced roof live loads have been considered for this design 2) Wind: ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=18ft; Cat II Exp C Encl GCp=0.18, MWFRS (envelope) and C-C Exterior(2) zone; end vertical left and right exposed porch left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=243 6=243 7) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 9) Note: Visually graded lumber designation SPP represents new lumber design values as per SPIB 10) Truss Design Engineer Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435																																																																																					
<b>LOAD CASE(S)</b> Standard																																																																																					



August 14, 2013

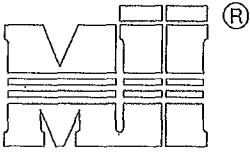
**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473 BEFORE USE.  
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2



MiTek Industries, Inc

MiTek Industries, Chesterfield, MO Page 1 of 1

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

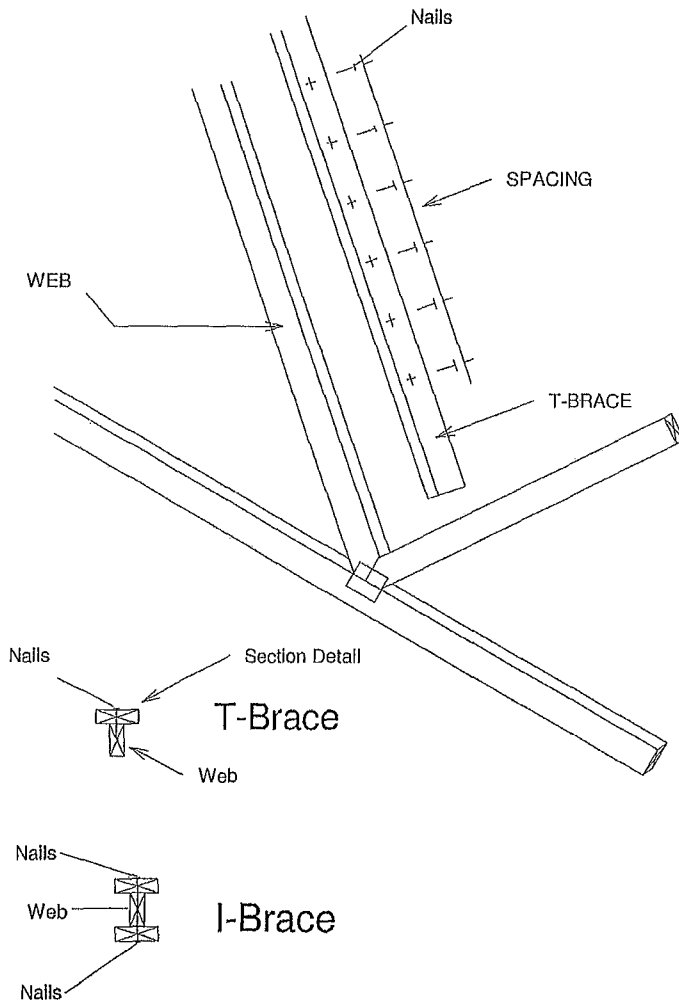
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

Brace Size for One-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I Brace must be same species and grade (or better) as web member.

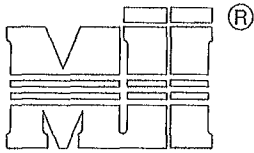


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JANUARY 1, 2009

## LATERAL TOE-NAIL DETAIL

ST-TOENAIL\_SP



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## NOTES

- 1 TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN)
- 2 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD
- 3 ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	131	88.0	80.6	69.9	68.4	59.7
	135	93.5	85.6	74.2	72.6	63.4
	162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	128	74.2	67.9	58.9	57.6	50.3
	131	75.9	69.5	60.3	59.0	51.1
	148	81.4	74.5	64.6	63.2	52.5

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.  
 APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

## EXAMPLE

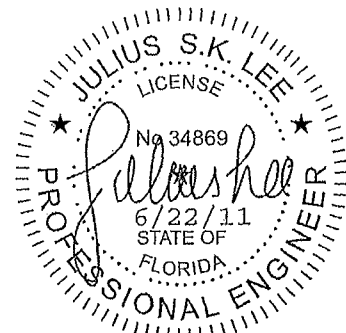
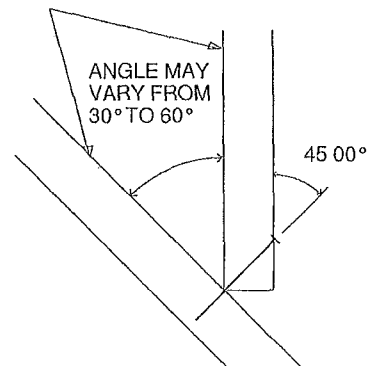
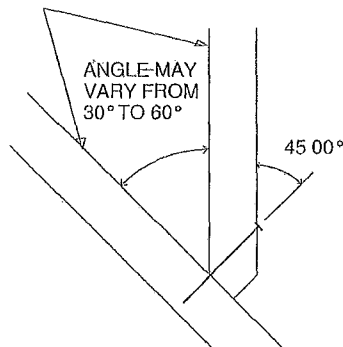
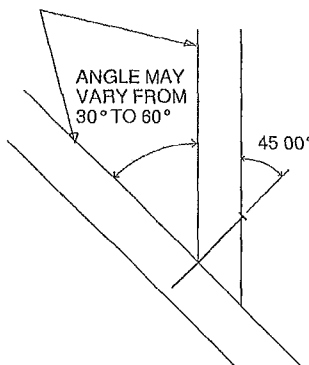
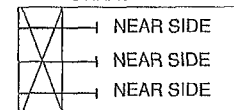
(3) - 16d NAILS (162" diam x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

SIDE VIEW

3 NAILS



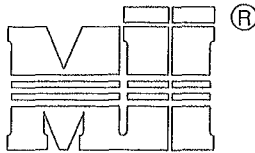
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FEBRUARY 14, 2012

# STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

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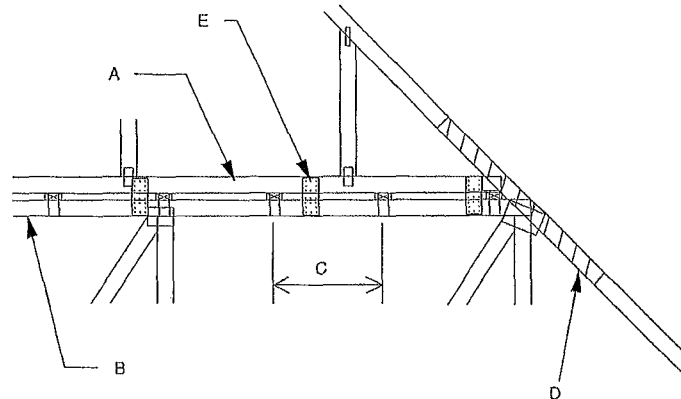


MiTek Industries, Inc

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E  
MAX MEAN ROOF HEIGHT = 30 FEET  
MAX TRUSS SPACING = 24" O.C  
CATEGORY II BUILDING  
EXPOSURE B or C  
ASCE 7 10  
DURATION OF LOAD INCREASE 1 60

DETAIL IS NOT APPLICABLE FOR TRUSSES  
TRANSFERING DRAG LOADS (SHEAR TRUSSES)  
ADDITIONAL CONSIDERATIONS BY BUILDING  
ENGINEER/DESIGNER ARE REQUIRED

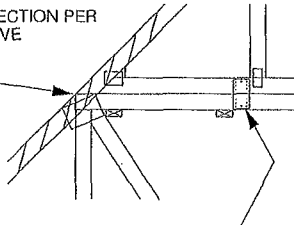
- A PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED
- B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING
- C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
- D 2 X 4" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT IN BOTH DIRECTIONS AND
- 1 WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
- 2 WIND SPEED OF 115 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E FOR WIND SPEEDS BETWEEN 128 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C W/ (4) 0.131" X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE. (MIN 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



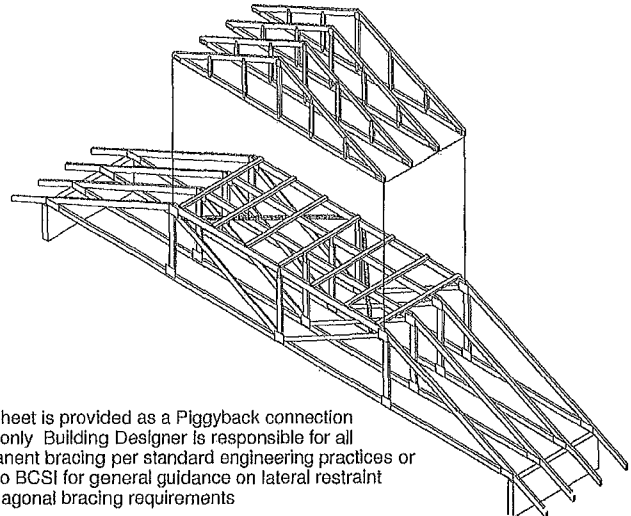
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER NOTE D ABOVE

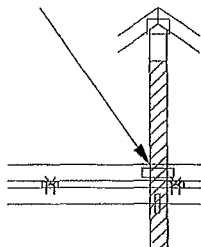


FOR ALL WIND SPEEDS, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C W/ (4) 0.131" X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE



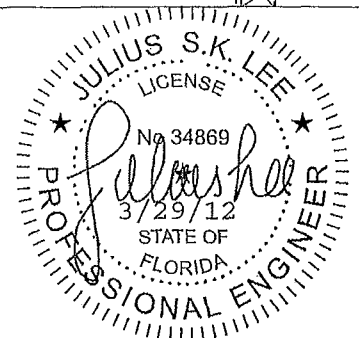
This sheet is provided as a Piggyback connection detail only Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK

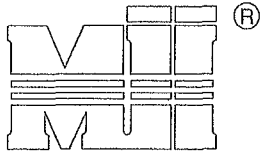


FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x 4" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C FROM EACH FACE (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@15') REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN



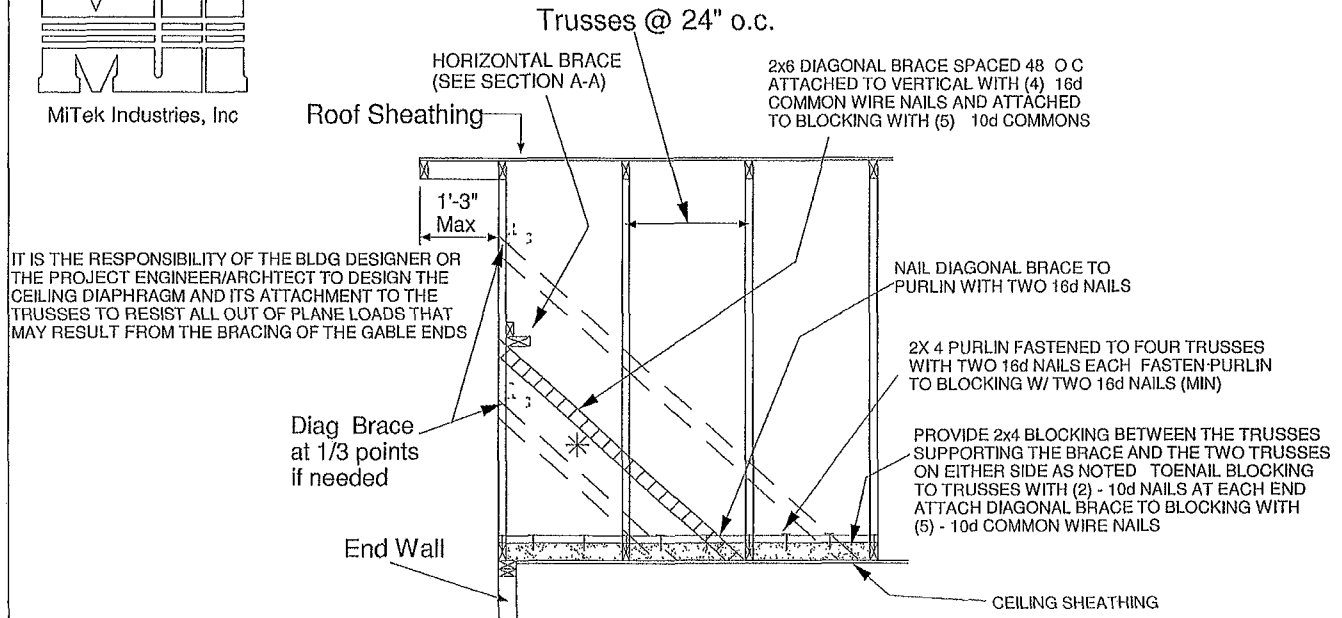
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## ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



## BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED

METHOD 1 ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE

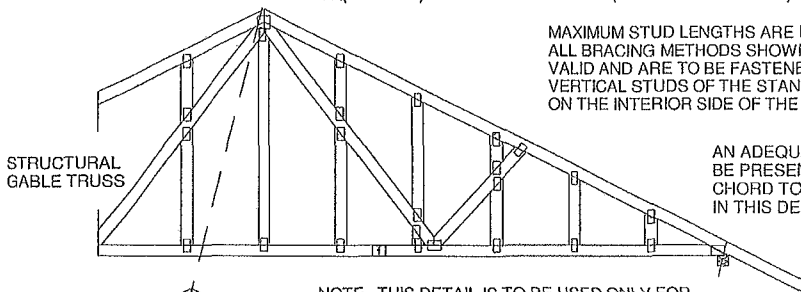
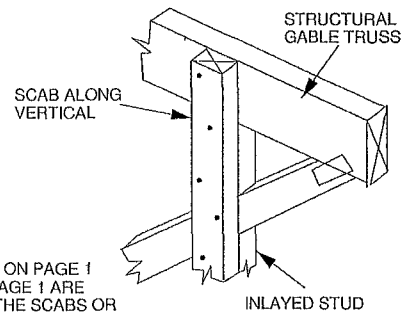
METHOD 2 ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

## NAILING SCHEDULE

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL

MEMBERS WITH ONE ROW OF 10d (131" X 3") NAILS SPACED 6" O.C.

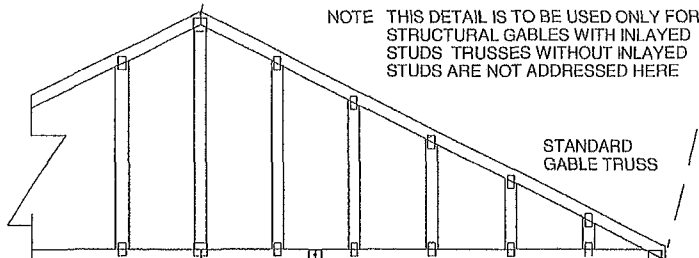
FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)



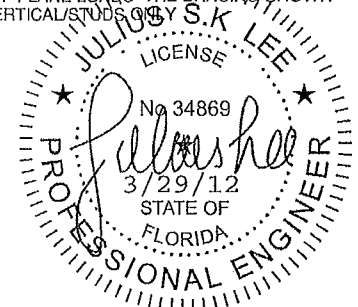
MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1

ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.

AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.



NOTE THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAYS STUDS. TRUSSES WITHOUT INLAYS STUDS ARE NOT ADDRESSED HERE



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