

August 26th, 2019

LOG PACKAGE SUPPLIER

A NEW LOG HOME FOR:

GILMORE: 10089 US HWY 27, FT. WHITE, FLOR

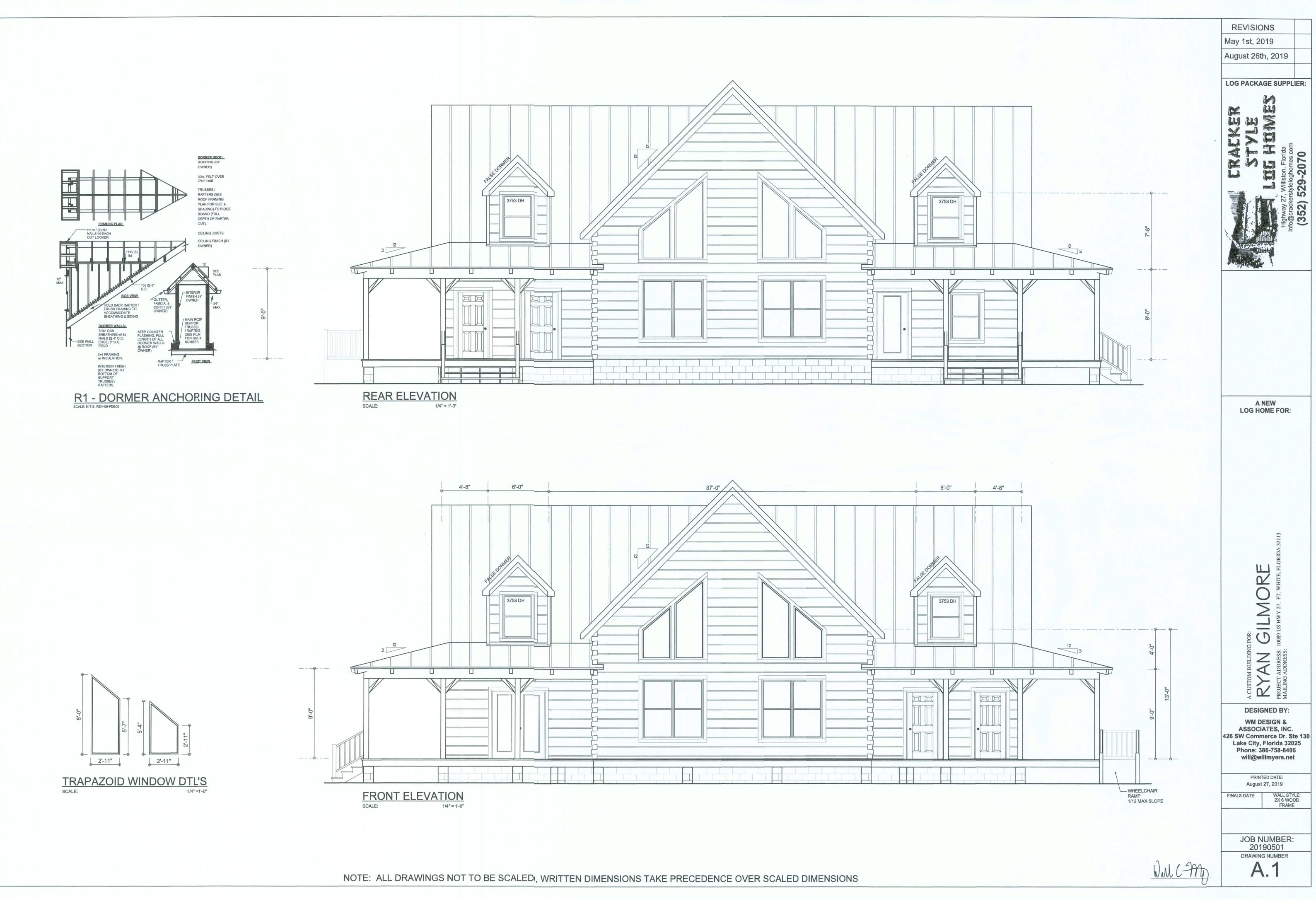
DESIGNED BY: WM DESIGN & ASSOCIATES, INC. 26 SW Commerce Dr. Ste 130 Lake City, Florida 32025

> PRINTED DATE: August 27, 2019

will@willmyers.net

WALL STYLE: 2X 6 WOOD FRAME

JOB NUMBER: 20190501 DRAWING NUMBER COVER



REVISIONS May 1st, 2019

August 26th, 2019

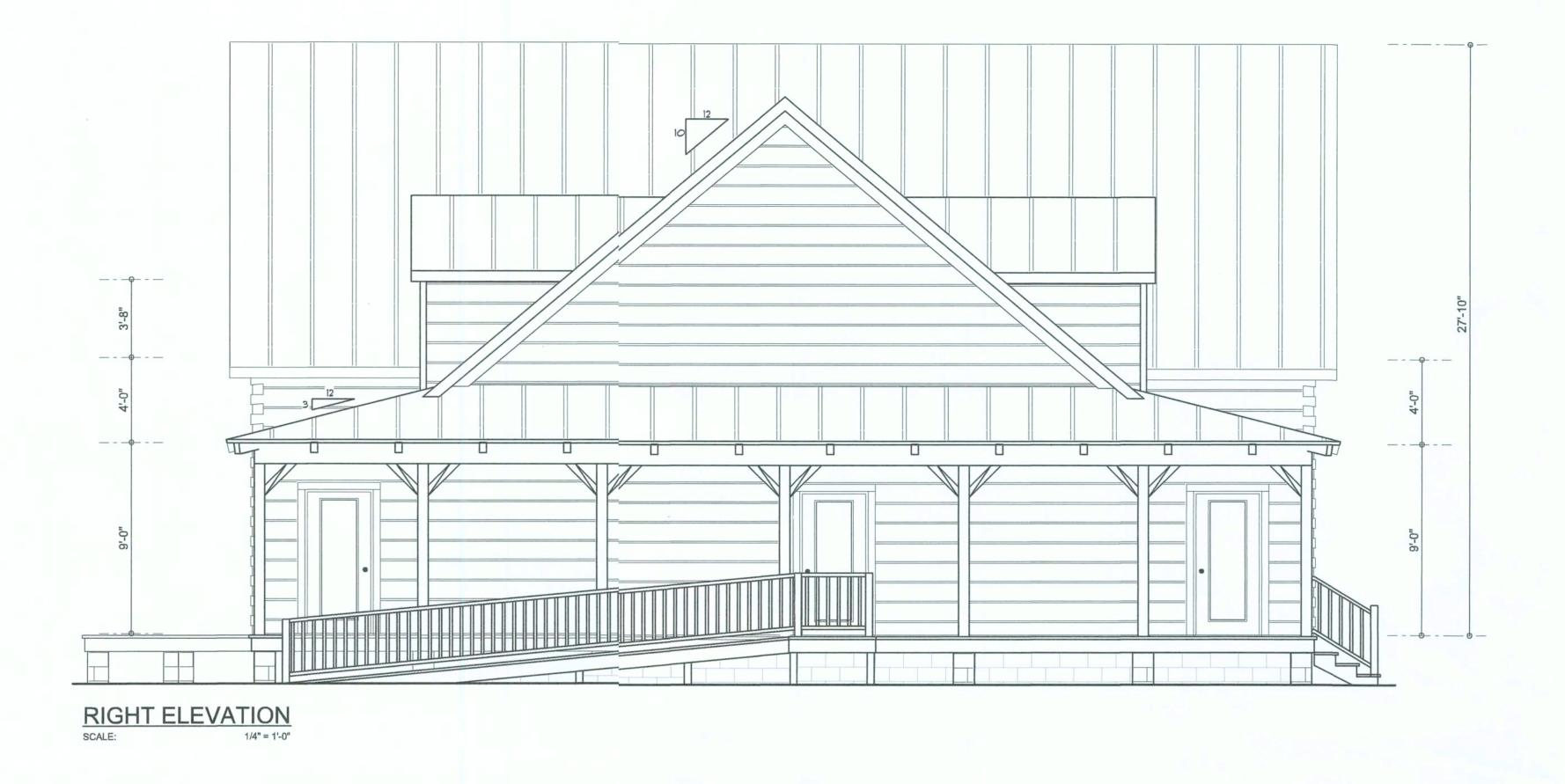
JOB NUMBER: 20190501 DRAWING NUMBER

PRINTED DATE:

WALL STYLE: 2X 6 WOOD FRAME

RYAN
PROJECT ADDRESS: 1

A.1





REVISIONS
May 1st, 2019
August 26th, 2019
LOG PACKAGE SUPPLIER:

CRACKER STYLE LIE HEIMES

Highway 27, Williston, Florida info@crackerstyleloghomes.co

A NEW LOG HOME FOR:

A CUSTOM BUILDING FOR:

RYAN GILMORE
PROJECT ADDRESS: 10089 US HWY 27, FT. WHITE, FLORIDA 32113
MAILING ADDRESS:

DESIGNED BY:

WM DESIGN &
ASSOCIATES, INC.
426 SW Commerce Dr. Ste 130
Lake City, Florida 32025
Phone: 386-758-8406
will@willmyers.net

PRINTED DATE:
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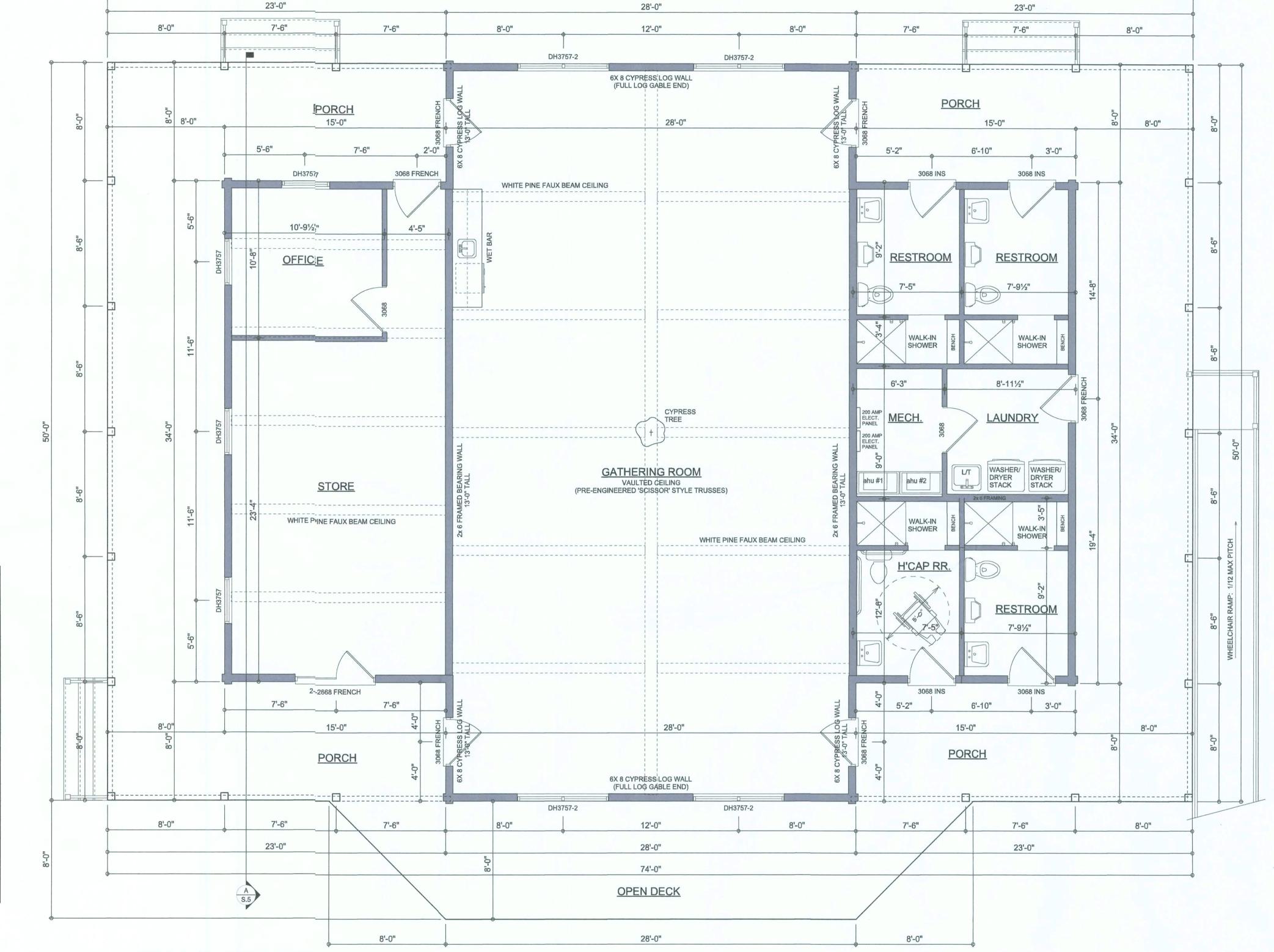
FINALS DATE: WALL S
2X 6 W

INALS DATE: WALL STYLE: 2X 6 WOOD FRAME

JOB NUMBER: 20190501
DRAWING NUMBER

A.2

Jul C-Am



74'-0"

D	OOR					REMARKS	SWING	STYLE
	SIZE			LOG WALL	OPENING			
DOOR NUMBER	PAIR	HTQIM	HEIGHT	WIDTH	HEIGHT			
1	PAIR	5'-0"	6'-8"	5'-3"	6'-9 1/2"	FRENCH DOOR EXTERIOR FIBERGLASSTHERMOTRU OR EQUAL		
2		3'-0"	6'-8"	3'-3"	6'-9 1/2"	FRENCH DOOR EXTERIOR FIBERGLASSTHERMOTRU OR EQUAL		
3		3'-0"	6'-8"	3'-3"	6'-9 1/2"	EXTERIOR FIBERGLASSTHERMOTRU OR EQUAL		
4		2'-8"	6'-8"	N/A	N/A	INTERIOR DOOR		
5		2'-0"	6'-8"	N/A	N/A	INTERIOR DOOR		
6		6'-0"	6'-8"	N/A	N/A	FOLDING CLOSET DOOF		
٦		4'-6"	6'-8"	N/A	N/A	FOLDING CLOSET DOOF		
8		3'-0"	6'-8"	N/A	N/A	FOLDING CLOSET DOOF		
9		1'-6"	6'-8"	N/A	N/A	FOLDING CLOSET DOOF		
10		2'-6"	6'-8"	N/A	N/A	INTERIOR POCKET DOCR		
11		2'-0"	6'-8"	N/A	N/A	INTERIOR POCKET DOCR		

LOG WALL OPENINGS ARE TO BE CUT 3" LARGER THAN DOOR ROUGH OPENING (AM)
LOG WALL OPENINGS ARE TO BE CUT 1 1/2" LARGER THEN DOOR ROUGH OPENING (HEAD)

QTY	MARK	MODEL NO.	FRAME SIZE		ROUGH OPENING		LOG WALL		MATERIAL	MANUFACTURER
			WIDTH	HEIGHT	WIDTH	HEIGHT	WIDTH	HEIGHT		
	A	DH3757	3'-1"	4'-9"	3'-1 3/4"	4'3 3/4"	3'-4 3/4"	5'-0 3/4"	WOOD INTERIOR VINYL EXTERIOR	PELLA (EGRESS WINDOW.
	В	DH3753	3'-1"	4'-5"	3'-1 3/4"	4'-> 3/4"	3'-4 3/4"	4'-8 3/4"	WOOD INTERIOR VINYL EXTERIOR	PELLA (FALSE DORMERS
	С	DH3T57-2	6'-2"	4'-9"	6'-2 3/4"	4'9 3/4"	6'- 5 3/4"	5'-0 3/4"	WOOD INTERIOR VINYL EXTERIOR	PELLA (EGRESS WINDOW.

DIMENSIONED FLOOR PLAN SCALE: NOTE: ALL CEILING HEIGHTS SHALL BE 9'-0" UNLESS OTHERWISE: NOTED

AREA SUMMARY

CONDITIONED FLOOR AREA 2,420 S.F. COVERED PORCH AREA'S

TOTAL AREA

1,280 S.F. 3,700 S.F.

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

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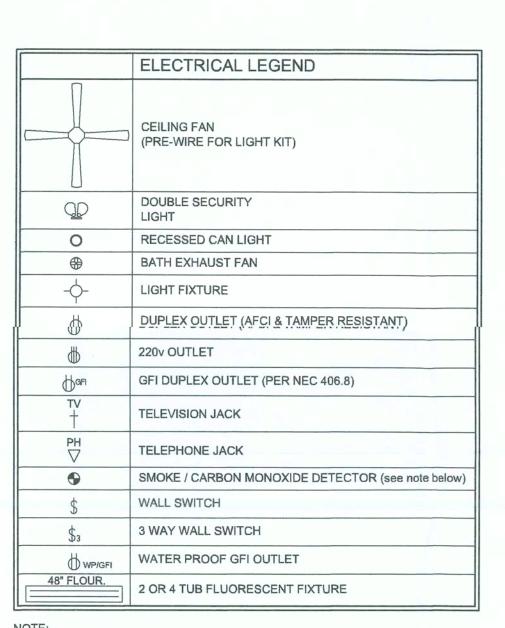
RYAN GILMORE
PROJECT ADDRESS: 10089 US HWY 27, FT. WHITE, FLOMALING ADDRESS:

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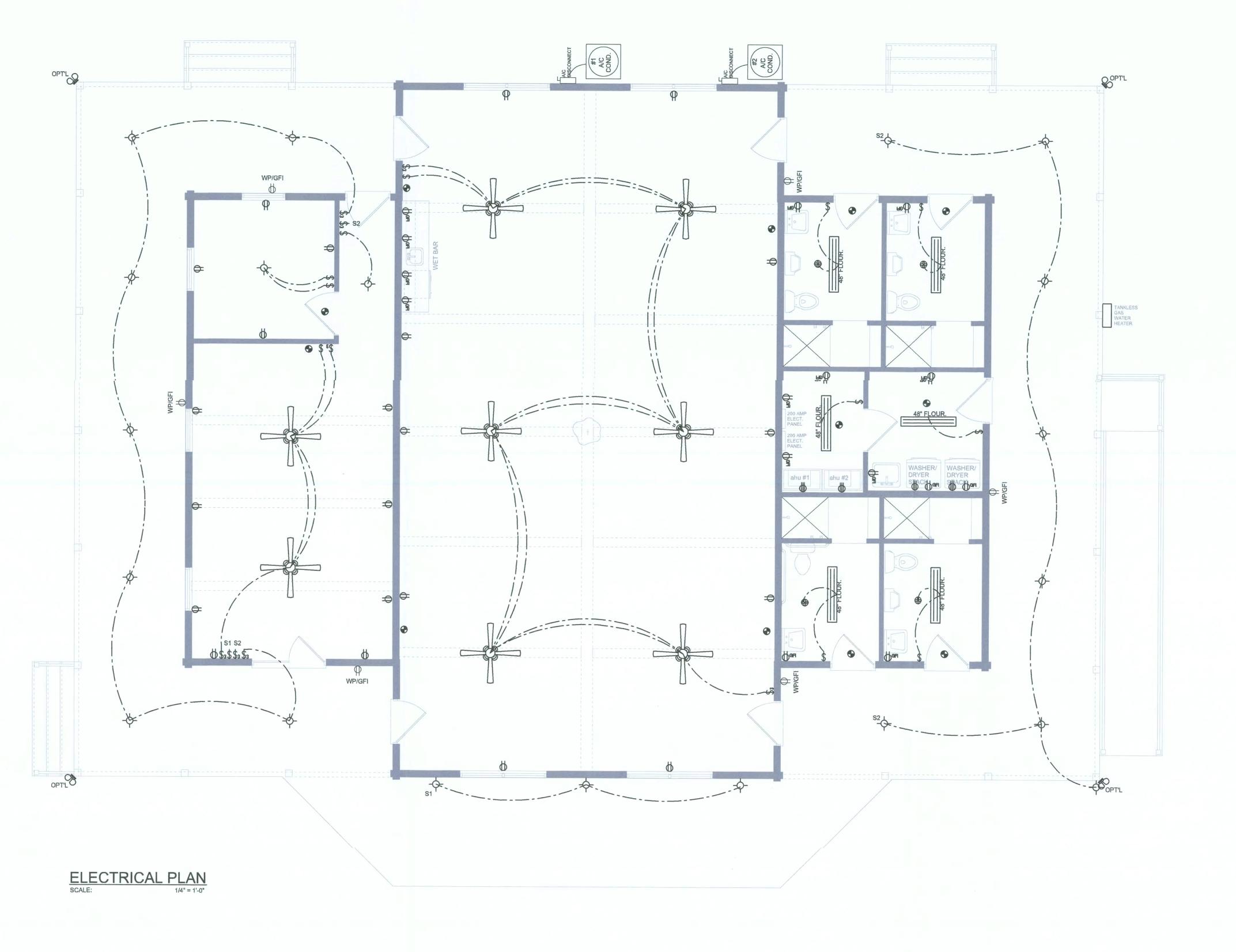


NOTE: ALL INTERIOR RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR AND SHALL HAVE BATTERY BACKUP POWER AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE NFPA70 2014 NATIONAL ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



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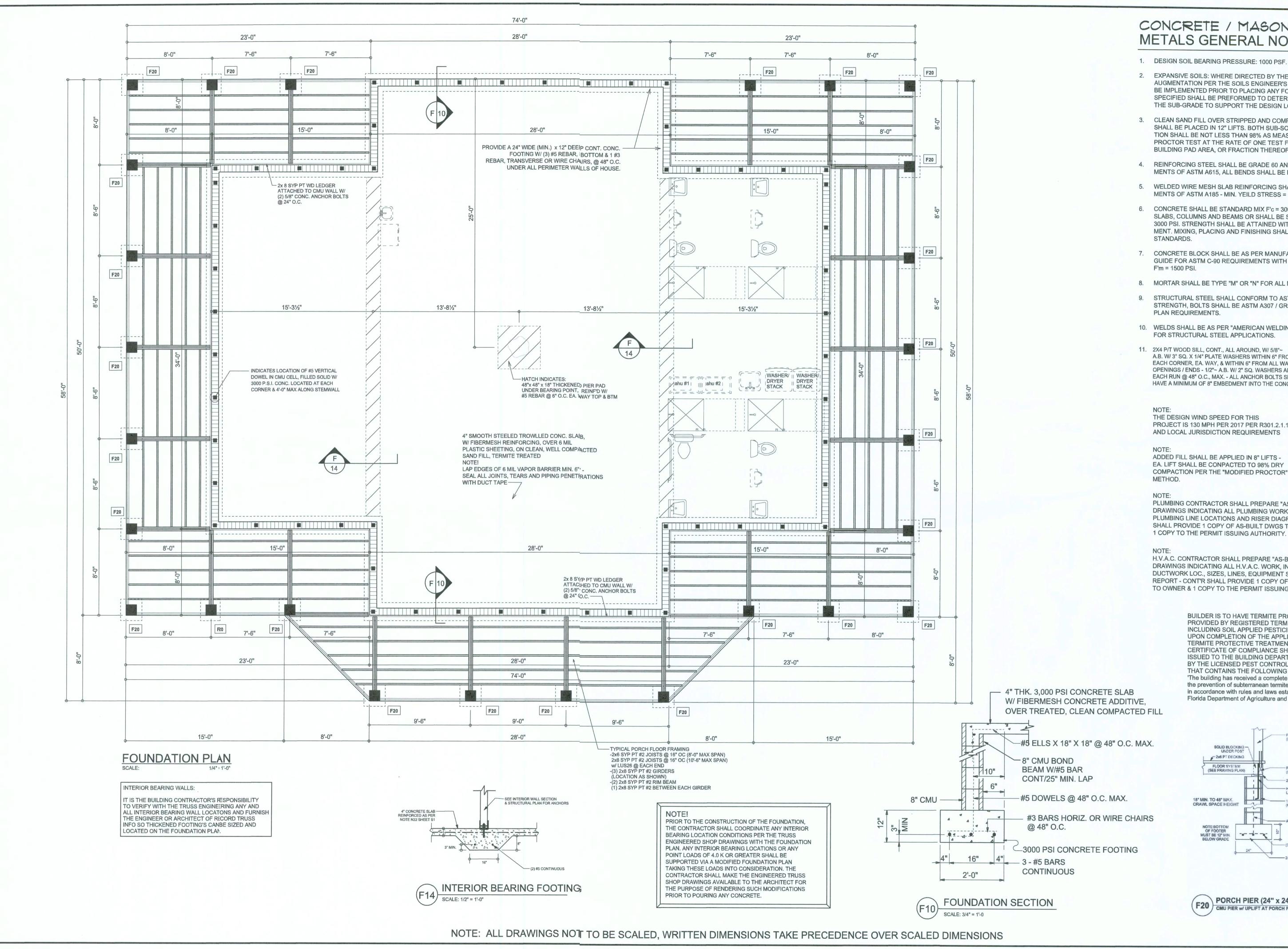
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CONCRETE / MASONRY / **METALS GENERAL NOTES:**

- DESIGN SOIL BEARING PRESSURE: 1000 PSF.
- 2. EXPANSIVE SOILS: WHERE DIRECTED BY THE SOILS ENGINEER, SOIL AUGMENTATION PER THE SOILS ENGINEER'S SPECIFICATIONS SHALL BE IMPLEMENTED PRIOR TO PLACING ANY FOUNDATIONS - TESTS AS SPECIFIED SHALL BE PREFORMED TO DETERMINE THE SUITABILITY OF THE SUB-GRADE TO SUPPORT THE DESIGN LOADS.
- CLEAN SAND FILL OVER STRIPPED AND COMPACTED EXISTING GD. SHALL BE PLACED IN 12" LIFTS. BOTH SUB-SOIL AND FILL COMPAC-TION SHALL BE NOT LESS THAN 98% AS MEASURED BY A MODIFIED PROCTOR TEST AT THE RATE OF ONE TEST FOR EACH 1500 SF OF BUILDING PAD AREA, OR FRACTION THEREOF, FOR EACH 12" LIFT.
- 4. REINFORCING STEEL SHALL BE GRADE 60 AND MEET THE REQUIRE-MENTS OF ASTM A615, ALL BENDS SHALL BE MADE COLD.
- 5. WELDED WIRE MESH SLAB REINFORCING SHALL MEET THE REQUIRE-MENTS OF ASTM A185 - MIN. YEILD STRESS = 85 KSI.
- 6. CONCRETE SHALL BE STANDARD MIX F'c = 3000 PSI FOR ALL FTGS, SLABS, COLUMNS AND BEAMS OR SHALL BE STANDARD PUMP MIX F'c = 3000 PSI. STRENGTH SHALL BE ATTAINED WITHIN 28 DAYS OF PLACE-MENT. MIXING, PLACING AND FINISHING SHALL BE AS PER ACI
- 7. CONCRETE BLOCK SHALL BE AS PER MANUFACTURER'S PRODUCT GUIDE FOR ASTM C-90 REQUIREMENTS WITH MEDIUM SURFACE FINISH -
- 8. MORTAR SHALL BE TYPE "M" OR "N" FOR ALL MASONRY UNITS.
- 9. STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 STANDARDS FOR STRENGTH, BOLTS SHALL BE ASTM A307 / GRADE 1 OR A325, AS PER PLAN REQUIREMENTS.
- 10. WELDS SHALL BE AS PER "AMERICAN WELDING SOCIETY" STANDARDS FOR STRUCTURAL STEEL APPLICATIONS.
- 11. 2X4 P/T WOOD SILL, CONT., ALL AROUND, W/ 5/8"~ A.B. W/ 3" SQ. X 1/4" PLATE WASHERS WITHIN 6" FROM EACH CORNER, EA. WAY, & WITHIN 6" FROM ALL WALL OPENINGS / ENDS - 1/2"~ A.B. W/ 2" SQ. WASHERS ALONG EACH RUN @ 48" O.C., MAX. - ALL ANCHOR BOLTS SHALL HAVE A MINIMUM OF 8" EMBEDMENT INTO THE CONCRETE.

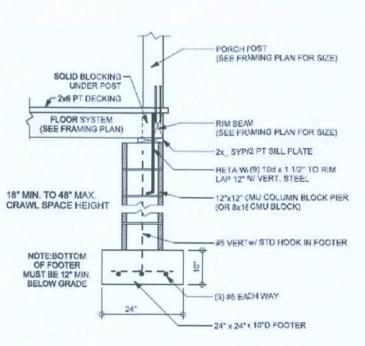
THE DESIGN WIND SPEED FOR THIS PROJECT IS 130 MPH PER 2017 PER R301,2,1,1 AND LOCAL JURISDICTION REQUIREMENTS

ADDED FILL SHALL BE APPLIED IN 8" LIFTS -EA. LIFT SHALL BE CONPACTED TO 98% DRY

PLUMBING CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL PLUMBING WORK, INCLUDING ALL PLUMBING LINE LOCATIONS AND RISER DIAGRAM - CONT'R SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER AND 1 COPY TO THE PERMIT ISSUING AUTHORITY.

H.V.A.C. CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL H.V.A.C. WORK, INCLUDING ALL DUCTWORK LOC., SIZES, LINES, EQUIPMENT SCH. & BALANCING REPORT - CONT'R SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

> BUILDER IS TO HAVE TERMITE PROCTECTION PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES UPON COMPLETION OF THE APPLICATION OF TERMITE PROTECTIVE TREATMENT. A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT. 'The building has received a complete treatment for the prevention of subterranean termites. Treatment is in accordance with rules and laws established by the Florida Department of Agriculture and Consumer Services."

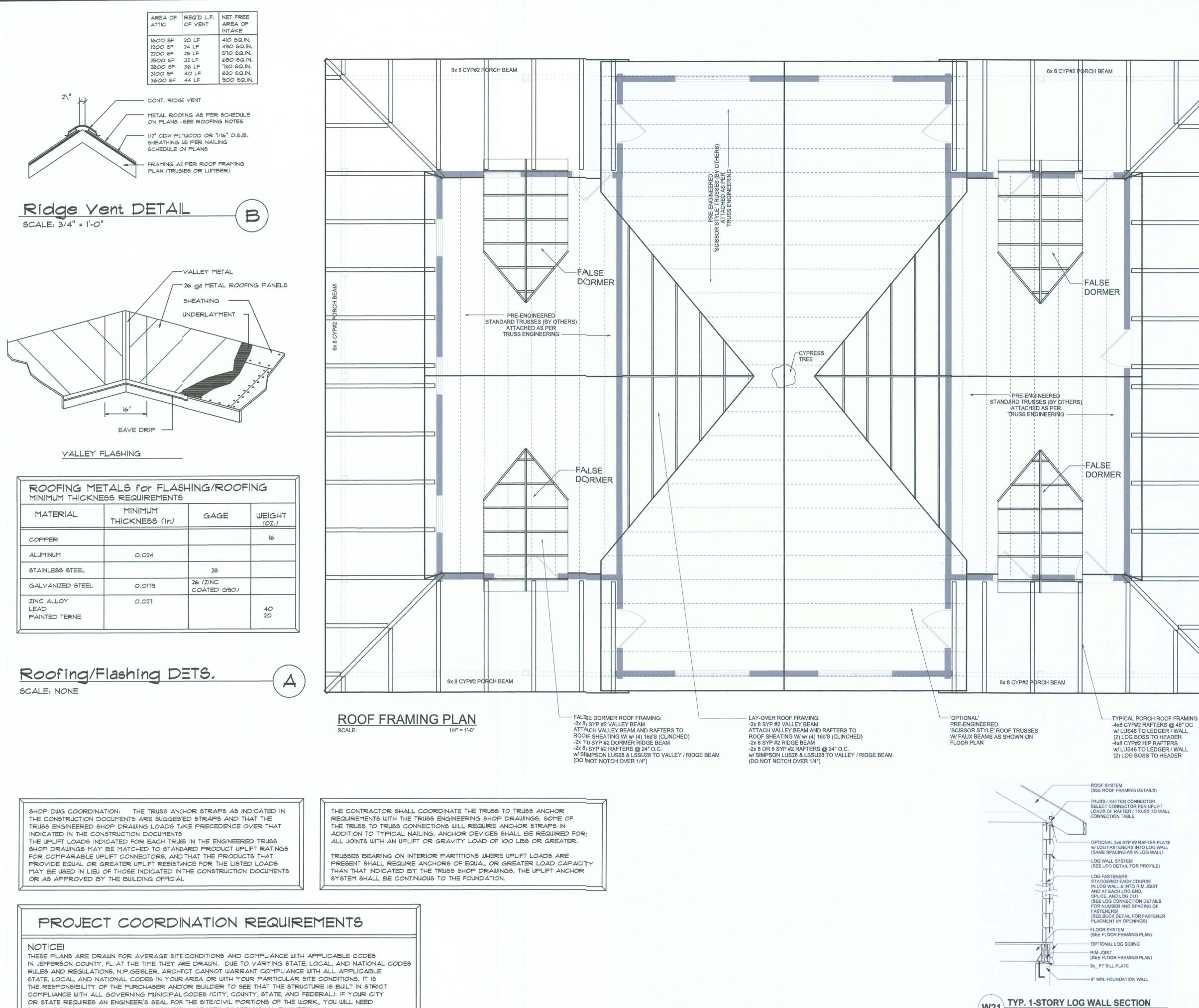


PORCH PIER (24" x 24" x 10") CMU PIER w/ UPLIFT AT PORCH POST SCALE: 1/2" = 1'-0"

ANN

JOB NUMBER 20190501

SHEET NUMBER OF 4 SHEETS



GENERAL NOTES:

1. BUILDER / OWNER IS TO VERIFY ALL DIMENSIONS BEFOR BEGINNING CONSTRUCTION.

2. ALL FRAMING MEMBERS ARE TO BE SYP#2 U.N.O. 3. ALTHOUGH TRUSSES ARE SHOWN AT 24" OC IT IS THE RESPONSIBLITY OF THE TRUSS MANUFACTURER TO DETERMIN TRUSS SPACING

4. = SEE DETAIL # SHOWN IN BOX

AND PLACEMENT.

7/16" OSB OR 5/8" CDX ROOF SHEATHING

-PRE-ENGINEERED ROOF TRUSSES AT 24" O.C. MAX. (DESIGNED BY OTHERS)

-2x6 SYP#2 LEDGER

ATTACHED PER EAVE CONNECTION DETAIL

W/ (6) .131" x 3.25" NAILS @ 24" OC TO SCAB / TRUSS FRAMING

LEDGER INTO RAFTER

- ROOFING AS PER OWNER

w/ (2) 16d EACH RAFTER

- HEAVY TIMBER RAFTERS

TRUSS HEEL W/ 7/16" OSB OR 5/6" CDX SHEATHING FULLEY BLOCKED

w/ 8d @ 6" OC EDGE AND 4" OC FIELD

TRUSS TO TOP PLATE

(FOR SHEAR LOADS)

HEAVY TIMBER RAFTERS 8' MAX SPAN

(4) .131X3.25" TOE NAILS

PER UPLIFT LOADS FROM

-SELECT TRUSS CONNECTORS

FROM THE CONNECTOR TABLE

ENGINEERED TRUSSES DRAWINGS

TYP. PORCH @ TRUSS HEEL DETAIL

- 15/16" T&G DECKING

VALLEY FLASHING

PER DETAIL, 'A'

2x4/6 SCAB w/ (2) .131" x 3.25" NAILS @ 6" OC

WALL OR -BEAM

(SEE WALL SECTIONS)

THIS SHEET-

ATTACH RAFTERS TO PORCH HEADER W/ ---

(2) LOG BOSS THRU RAFTER TO HEADER OR

TOE-SCREWED FROM HEADER INTO RAFTER

- ATTACH RAFTERS TO LEDGER w/ (1) LOG

FASTERN AND (8) .131" x 3.25" NAILS

@ 48" OC MAX

POST TO

HEADER

w/ (6) LOG

FROM BACK SIDE OF WALL THROUGH

SOFTPLAN

AMING DETAIL

N.C.A.

JOB NUMBER 20190501

- KICK-OUT FLASHING PER

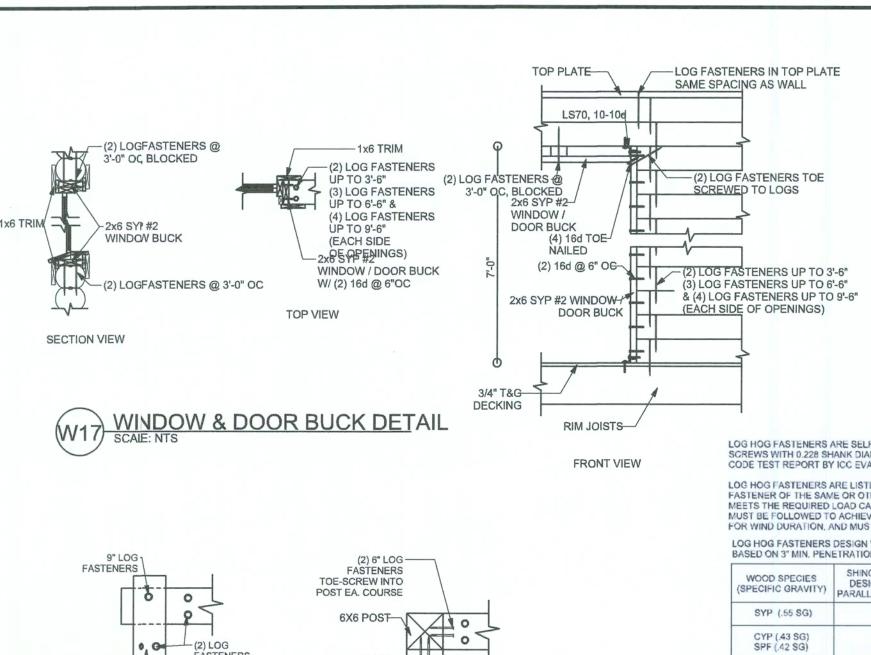
SHEET NUMBER

OF 4 SHEETS

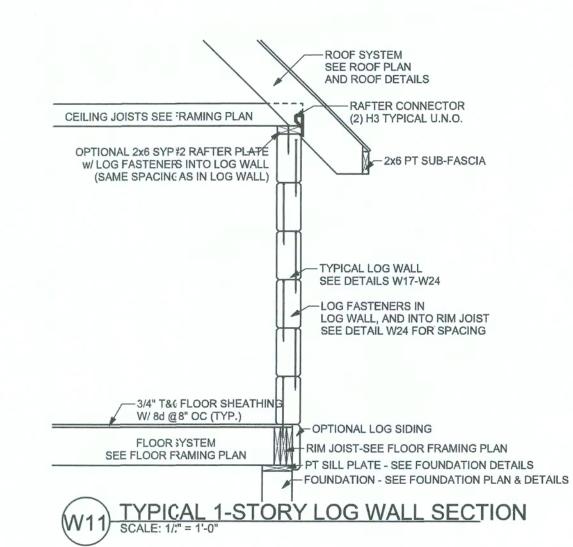
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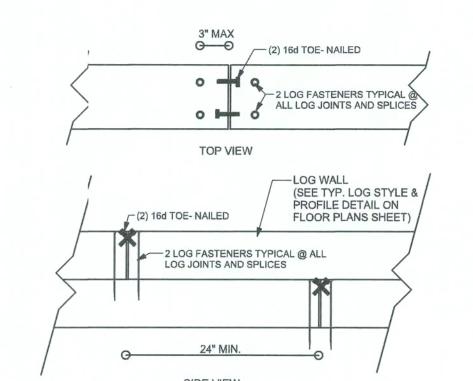
COMPLIANCE WITH ALL GOVERNING MUNICIPAL CODES (CITY, COUNTY, STATE, AND FEDERAL). IF YOUR CITY OR STATE REQUIRES AN ENGINEER'S SEAL FOR THE SITE/CIVIL PORTIONS OF THE WORK,, YOU WILL NEED TO HAVE THAT DONE LOCALLY BY A QUALIFIED, LICENCED PROFESSIONAL ENGINEER.

ON RIM @ WOOD FLOOR SYSTEM



OUTSIDE CORNER TOPVIEW INSIDE CORNER TOP VIEW **FASTENERS** (SEE TYP, LOG STYLE & PROFILE DETAIL ON SIDE VIEW





LOG HOG FASTENERS ARE SELF DRILLING, HIGH STRENGTH, Ft = 111ksi, STEEL WOOD SCREWS WITH 0.228 SHANK DIAMETER. THE FOLLOWING DESIGN VALUES ARE BASED ON CODE TEST REPORT BY ICC EVALUATION SERVICE, INC. AND NDS 2001:

LOG HOG FASTENERS ARE LISTED FOR EXAMPLE INOT ENDORSEMENT. AN EQUIVALENT FASTENER OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. ILISTED LOADS HAVE BEEN INCREASED FOR WIND DURATION, AND MUST BE ADJESTED FOR OTHER DURATIONS OR SPECIES. LOG HOG FASTENERS DESIGN VALUE FOR 1.6 WIND DURATION (LB)
BASED ON 3" MIN. PENETRATION INTO MAIN MEMBER & 2.5" MIN. SIDE MEMBER

RAFTERS SHALL BE IN ACCORDANCE WITH RAFTER SPAN TABLES.

IS 1 1/2" ON WOOD OR METAL AND 3" ON MASONRY OR CONCRETE.

REQUIRED WHEN RIDGE BEAM IS PROVIDED.

RAFTER OVERHANGS SHALL NOT EXCEED THE LESSER OF 1/3 RAFTER SPAN OR 24" UNO.

RAKE OVERHANG OUTLOOKERS ARE 2X4 PURLINS CONNECTED PER GABLE DETAIL UNO. RAKE OVERHANGS SHALL NOT EXCEED THE LESSER OF 1/2 PURLIN LENGTH OR 24".

RAKE OVERHANGS USING 2X4/6 OUTLOOK BLOCKING @ 24" OC SHALL NOT EXCEED 12"

RAFTERS AND CEILING JOISTS HAVING A DEPTH-TO-THICKNESS RATIO EXCEEDING 5:1

RAFTER BEARING ON BEAM OR HANGERS. CEILING JOISTS OR RAFTER TIES ARE NOT

EACH OTHER, AND WALLS ARE TIED BY, RAFTER TIES, CEILING OR FLOOR SYSTEM.

WHERE THE ROOF PITCH IS LESS THAN 3:12. STRUCTURAL MEMBERS THAT SUPPORT

A CONTINUOUS LOAD PATH SHALL BE PROVIDED TO TRANSMIT THE UPLIFT FORCES

SHALL EXTEND SO THE TOP NAIL IS WITHIN 1" OF THE TOP OF THE RAFTER, OR

WHEN CEILING JOISTS ARE USED TO PROVIDE RESISTANCE TO RAFTER THRUST

JOISTS, FOR LESS THAN 4 FT, WIDE USE A SINGLE HEADER AND TRIMMER JOIST

FOR OPENINGS MORE THAN 4 FT. WIDE DOUBLE THE TRIMMER AND HEADER JOISTS

PROVIDE HANGER OR LEDGER AT HEADER FOR RAFTERS AND JOISTS OVER 12' SPAN.

MINIMUM THICKNESS OF STRUCTURAL PANEL ROOF SHEATHING: 16" OC RAFTERS USE 7/16" FOR UPTO 130 MPH; 24" OC RAFTER USE 7/16" UPTO 110 MPH, 5/8" UPTO 140 MPH

PROVIDE EDGE SUPPORT BLOCKING OR EDGE CLIPS FOR 7/16" SHEATHING w/ 24" SPANS

ATTACH STRUCTURAL SHEATHING w/ 8d RING-SHANK @ 6"OC EDGE & FIELD, 4"OC GABLE

0.113 INCH NOMINAL SHANK DIAMETER / RING DIAMETER OF 0.012" OVER SHANK DIA.

16 TO 20 RINGS PER INCH / 0.280" FULL ROUND HEAD DIAMETER / 2" NAIL LENGTH

ROOF SYSTEM DESIGN NOTE

WIND LOADS PER 5th EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017) R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;

WOOD RAFTER FRAMING

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

2.) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY

1.) BASIC WIND SPEED = 140 MPH, (3 SEC GUST, 33 FT, EXP. C)

.) IOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIFY

) INTERNAL PRESSURE COEFFICIENT = +/- .18 (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone Effective Wind Area (ft2)

46 -79

5 50 -67

Garage Door

2014 FBCR,

Table R301.2.(4)

9x7 Garage Door 44

16x7 Garage Door 42 -47

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

4.) BUILDING CATEGORY = II, (MRI = 700 YR)

ROOF ANGLE = 7-45 DEGREES

MEAN ROOF HEIGHT = <30 FT

DESIGN DATA

MEAN ROOF HEIGHT

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (ATTICS WITH STORAGE

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

30 PSF (SLEEPING ROOMS)

16 PSF (4:12 TO <12:12)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

ROOF 20 PSF (FLAT OR <4:12)

PROVIDE DIAPHRAGM BLOCKING AT PANEL EDGES IN THE FIRST 2 BAYS, 48" OC MAX.

LUMBER ROOF SHEATHING SHALL BE A MIN. OF 5/8" THICK FOR 24" OC RAFTER SPACING

CEILING JOISTS SHALL BE IN ACCORDANCE WITH CEILING JOIST SPAN TABLE.

ATTACHED AT EACH END, TO BEARING WALLS, HEADERS, OR RIDGE BEAMS.

1/2" THICK T&G FOR RAFTERS @ 48" OC. ATTACH WITH (2) 16d EACH JOIST.

HANGERS ARE REQUIRED FOR ALL HEADER JOISTS OVER 6 FT. WIDE.

RAFTERS & CEILING JOISTS (RIDGES, HIPS, & VALLEYS) SHALL BE DESIGNED AS BEAMS

COLLAR TIES OR RIDGE TENSION STRAPS TO RESIST WIND UPLIFT SHALL BE PROVIDED

FROM RAFTER/TRUSS TIES TO THE FOUNDATION. FOR RAFTER CONSTRUCTION, STRAPS

WRAPPED AROUND TOP OF THE RAFTER WITH ONE NAIL MIN. ON THE OPPOSITE SIDE.

LAPPED JOISTS MUST BE NAILED TOGETHER OR STRAPPED TOGETHER TO RESIST LOAD.

CATHEDRAL CEILINGS WITHOUT CEILING JOISTS OR RAFTER TIES SHALL HAVE RAFTERS

OPENINGS IN ROOF AND CEILING FRAMING SHALL BE FRAMED WITH HEADER & TRIMMER

RIDGE BOARDS ARE BE PERMITTED PROVIDED: MIN. THICKNESS IS 1" NOMINAL DEPTH IS

NOT LESS THAN CUT END OF THE RAFTERS, RAFTERS ARE PLACED DIRECTLY OPPOSITE

SHALL HAVE LATERAL SUPPORT AT POINTS OF BEARING TO PREVENT ROTATION.

RIDGE, HIP, AND VALLEY BEAMS SHALL BE INSTALLED PER FRAMING PLANS, WITH

RAFTERS AND CEILING JOISTS SHALL BEARING DIRECTLY ON BEAMS, GIRDERS, LEDGERS

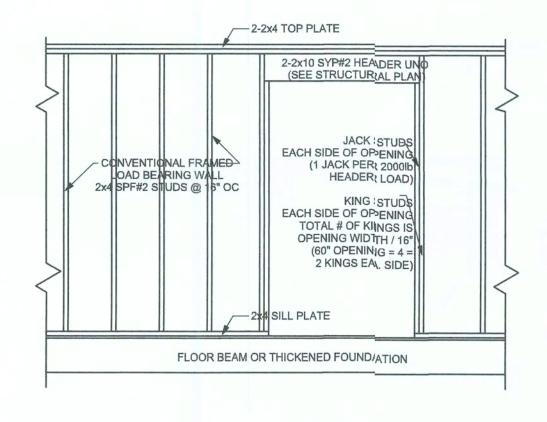
OR BEARING WALLS OR BE SUPPORTED BY JOIST HANGERS. MINIMUM BEARING LENGTH

WOOD SPECIES (SPECIFIC GRAVITY)	SHINGLE SHEAR DESIGN VALUE PARALLEL TO GRAIN	SHIINGLE SHEAR DESIGN VALUE PERPENDICULAR TO GRAIN	WITHDRAWAL DESIGN VALUE
SYP (.55 SG)	502	477	1060
CYP (.43 SG) SPF (.42 SG)	438	387	710

WHEN PENETRATION (p) IS LESS THAN 3" VALUE ! SHALL BE MUTIPLIED BY: p/3" SHEAR VALUES ARE BASED ON 10D MIN. PENETRATION INTO MAIN MEMBER WHEN PENETRATION (p) IS 6D<p<10D SHEAR VALUE SHALL BE MUTIPLIED BY: p/10D
-TABLE VALUES ARE FOR 1.5 WIND LOAD-DURATIONS, ADJUST FOR OTHER DURATIONS.
- END GRAIN FACTOR (NDS 11.5.2) ITHDRAWAL LOADS MULTIPLY BY: = .75, LATER/AL LOADS MULTIPLY BY: .67 TOE-SCREW FACTOR (NDS 11.5.4) WITHDRAWAL LOADS MULTIPLY BY: = .67, LATER/AL LOADS MULTIPLY BY: .83 CONNECTION GEOMETRY MINIMUM CRITERIA (PER REPORT): MIN. EDGE DISTANCE (LOADED EDGE)= 8d = 1 3/4" - MIN. EDGE DISTANCE (LOADED EDGE) - 03 - 1 GG - MIN. END DISTANCE, TENSION LOAD PARALLEL TO GRAIN = 18d = 3 3/4" - COMPRESSION LOAD PARALLEL TO GRAIN = 10d = 2 3/8" SPACING BETWEEN FASTENERS IN A ROW, PARALLEL TO GRAIN = 15d = 3 1/2" - PERPENDICULAR TO GRAIN = 10d = 2 3/8" - SPACING BETWEEN ROW OF FASTENERS, IN LINE = 5d = 1 1/4" - SPACING BETWEEN ROWS OF FASTENERS, STAG;GERED = 2.5d = 5/8"

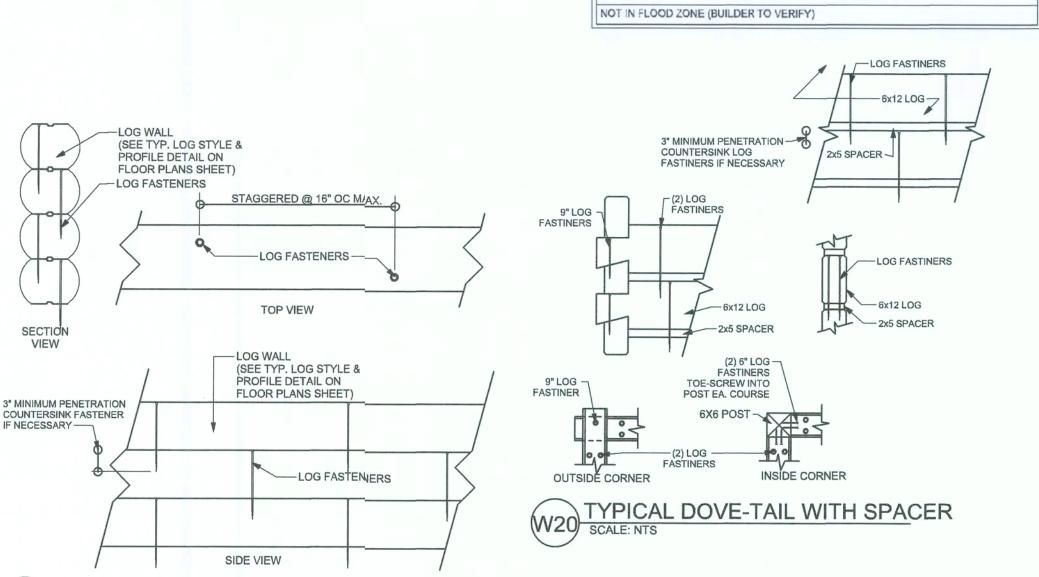
- WITHDRAWAL VALUES ARE BASED ON 3" PENETRATION INTO MAIN MEMBER.

LOG HOG FASTENER DATA TYPICAL DESIGN VALUES



(W5) CONVENTIONAL FRAMED LOAD BEARING WALL

W24 TYPICAL LOG WALL / FASTENIER PLACEMENT



AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. ALL CONNECTIONS EXPOSED DIRECTLY TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION.

REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

ATTACHED TO THE COURSE BELOW WITH LOG FASTENERS. FASTENER SPACING IS BASED ON REQUIRED

INTERIOR STUD WALLS: ALL INTERIOR STUD WALLS ARE NON-LOAD BEARING; UNO. ROOF LOADS TO BE CARRIED ON LOG WALLS OR ROOF BEAMS WITH INTERIOR SUPPORT COLUMNS; UNO. BEARING WALL STUDS TO BE SPF#2; UNO. NON-LOAD BEARING WALL STUDS MAY BE SPF STUD GRADE.

EXTERIOR STUD WALLS: ALL EXTERIOR STUD WALLS ARE LOAD BEARING SHEAR WALLS WITH SPF#2 STUDS, SYP#2 PT BOTTOM PLATE, SPF#2 DOUBLE TOP PLATE WITH 10-16d NAILS PER LAP SPLICE: SP4, 6-10d "U" STRAP TOP AND BOTTOM AT 48" OC UNO; 7/16" OSB OR 5/8" CDX SHEATHING, WITH PANEL EDGES FULLY BLOCKED, FASTENED WITH 8d COMMOM NAILS, SPACING 6" OC PANEL EDGES, 12" OC

ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING

UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES, STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES 6"OC INTERMEDIATE MEMBERS, 4" OC GABLE ENDS AND DIAPHRAGM BOUNDARY; UNO. TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE

WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE.

IN A WORKMANLIKE MANNER TO PREVENT ANY POSSIBILITY OF MOISTURE DAMAGE, TOXIC MOLD, OR ANY OTHER DETRIMENTAL EFFECT. ALSO, FOLLOW FLASHING MANUFACTURER'S DATA SHEET AND SMACNA LITERATURE AND STANDARDS.

N22 SITE / FOUNDATION NOTES

SITE PREPERATION: SITE ANALYSIS AND PREPERATION INFORMATION IS NOT PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE OWNER. ALL FOUNDATIONS AND FOOTINGS ARE DESIGNED FOR STABLE SOIL CONDITIONS WITH 2000 PSF BEARING CAPACITY. SITE INSPECTION OF SOIL CONDITIONS SHALL DETERMINE IF THERE IS ANY EVIDENCE OF UNSUITABLE BEARING MATERIALS. QUESTIONABLE MATERIALS PRESENT SHOULD CALL FOR SOILS TEST AND ANALYSIS BY GEOTECHNICAL ENGINEER TO ASSURE THAT EXPANDING CLAYS AND OTHER PROBLEMATIC SOILS CONDITIONS DO NOT EXIST, OR TO ALLOW MITIGATION SHOULD THEY EXIST. ALL FILL UNDER STRUCTURAL ELEMENTS SHALL BE CLEAN SAND/SOIL FILL, FREE FROM DEBRIS AND ORGANIC MATERIALS COMPACTED IN LIFTS OF NOT MORE THAN 6 IN., LOOSE MEASURE. IT IS THE OWNER'S / BUILDER'S RESPONSIBILITY TO VERIFY EXISTING SOIL AND CLEAN FILL ARE COMPACTED TO 95% OF MAX. DRY DENSITY PER THE MODIFIED PROCTOR FEST AND PROVIDE 2000 PSF MIN. BEARING CAPACITY OR REQUEST FOUNDATION DESIGN BASED ON ACTUAL SITE CONDITIONS.

FOUNDATION: THE OWNER HAS NOT YET PROVIDED A GEOTECHNICAL REPORT TO THE ENGINEER. ASSUMED SAFE BEARING CAPACITY OF 2000 PSF SHALL BE CONFIRMED IN THE FIELD BY REGISTERED GEOTECHNICAL ENGINEER OR SHALL BE APPROVED BY THE OWNER. FOOTING AND SLABS ARE TO BEAR ON FIRM UNDISTURBED EARTH OR CLEAN SAND / SOIL FILL, FREE FROM DEBRIS, AND ORGANIC MATERIALS COMPACTED IN LIFTS OF NOT MORE THAN 6 IN., LOOSE MEASURE. WHERE UNACCEPTABLE MATERIAL OCCURS, EXCAVATE AND REPLACE WITH ENGINEERED FILL. NO FOUNDATION CONCRETE SHALL BE INSTALLED UNTIL ALL FOUNDATION WORK HAS BEEN COORDINATED WITH UNDERGROUND UTILITIES. FOOTING SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES. TO MINIMZE WEATHERING, THE LAST 6" OF EXCAVATION FOR ALL FOOTINGS SHALL BE MADE IMMEDIATELY PRIOR TO PLACEMENT OF FOOTINGS.

EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED. THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH NS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OF SURFACE. ALL CONCRETE SHALL BE VIBRATER. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF ENGINEER, OWNER OR HIS

FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

V21	ST	RUCT	Ū	RAL	DESI	GN	NO.	TES
TRUCT	URAL	CONNECTO	RS:	MANUE	ACTURER	SAND	PRODU	CT NUME

MBER FOR CONNECTORS, ANCHORS,

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST

LOG WALLS: ALL LOG WALLS ARE MILLED LOGS WITH FLAT STACKING SURFACES, EACH COURSE IS PULLOUT STRENGTH FOR WIND UPLIFT AND REQUIRED SHEAR STRENGHT FOR LATERAL WIND LOADS.

ALL PLATES NOT PROTECTED FROM MOSTURE TO BE SYP#2 PT.

INTERMEDIATE FRAMING MEMBERS; UNO.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ROOF SEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB OR 5/8" CDX SHEATHING.

FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED AND SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE. REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO

ROOF VENTILATION: ROOF VENTILATION IS TO MEET OR EXCEED FLORIDA BUILDING CODE RES.

FLASHING: BUILDER IS TO PROVIDE FLASHING TO MEET LOCAL CODE REQUIRMENTS AND INSTALLED

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WHERE

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT

FIBER CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

(25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 23".

CONCRETE BLOCK: ASTM C-90 WITH MEDIUM SURFACE FINISH, F'm = 1500 PSI. MORTAR: TYPE M OR N FOR ALL MASONRY UNITS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

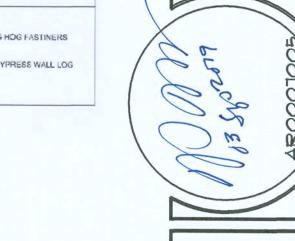
MODEL NO. ALL		WABLE	OAD	S LB (DKIW			
STRAPS & TIES	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER	TO PLAT
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1
НЗ	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1
H6	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1
H10-2	760	655	455	395	390	340	6-10d	6-10d
H16	1470	1265		1	-		2-10d x 1 1/2"	10-10d x 1
H16-2	1470	1265			-		2-10d x 1 1/2"	10-10d x 1
LTS12 - LTS20	1000	620			-		6-10d x 1 1/2"	6-10d x 1
MTS12 - MTS30	1000	860	\vdash	-	-		7-10d x 1 1/2"	7-10d x 1
HTS16 - HTS30	1450	1245			-		12-10d x 1 1/2"	12-10d x 1
STUD STRAP	UPLIFT	UPLIFT		_	_	_		
CONNECTOR	SYP	SPF					TO STUDS	TO PLAT
SSP (2-PLATES)	435	435					4 -10d	3-10d
SSP (1-PLATE)	455	420					4 -10d	1-10d
DSP (2-PLATES)	825	825					8 -10d	6 -10d
DSP (1-PLATE)	825	600			_		8 -10d	2 -10d
SP1	585	535			-		6 -10d	4 -10d
SP2	1065	605			-		6 -10d	6 -10d
SP4	885	760			-	-	6-10d x 1 1/2"	0 100
SPH4	1240	1065			-	-	10-10d x 1 1/2"	
SP6	885	760		_	-	-	6-10d x 1 1/2"	
SPH6		1065			_		10-10d x 1 1/2"	
	1240				_		10-10d x 1 1/2	
LSTA18	1235	1110			-			
LSTA21	1235	1235			_		16-10d	
CS20	1030	1030			_		14-10d	
CS16	1705	1705					22-10d	
HEAVY GIRDER TIEDOWNS	SYP	UPLIFT SPF	F1 SYP	F2 SYP	SPF	F2 SPF	TO GIRDER	TO BEAM FOUNDAT
LGT2	2050	1785	700	170	700	170	14-16d	14-160
LGT3-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16d
MGT	3965	3330					22 -10d	5/8" ANCH
HGT-2	10980	6485					16 -10d	2-5/8" ANC
HGT-3	10530	9035			_		16 -10d	2-5/8" ANC
EMBEDDED ANCHORS	UPLIFT SYP 1-PLY	UPLIFT SYP 2-PLY	F1 SYP	F2 SYP			TO BEAM OR POST	TO FOUNDAT
META12 - META40	1450	1450	340	725			7-10d x 1 1/2" or 6-16d	EMBEDD
(2) META (CMU)	1985	1900	1210	1160	_		10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY)	EMBEDD
(2) META (CONCRETE)	1985	2565	1210	1160			10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY)	EMBEDD
HETA16 - HETA40	1810	1810	340	725			9-10d x 1 1/2" or 8-16d	EMBEDD
(2) HETA (CMU)	2035	2500	1225	1520			10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY)	EMBEDD
(2) HETA (CONCRETE)	2035	2700	1225	1520			10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY)	EMBEDD
HHETA16 - HHETA40	2235	2235	340	815			10-10d x 1 1/2" or 9-16d	EMBEDD
(2) HHETA (CMU)	2035	2500	1225	1520			10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY)	EMBEDD
(2) HHETA (CONCRETE) HOLDOWNS /	2035	3350 UPLIFT	1225	1520			10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) TO BEAM	EMBEDDI
POST BASE	SYP	SPF					OR POST	FOUNDAT
LTT19	1350	1305				_	8-16d	1/2" ANCH
LTTI31	2310	2310			-	-	18-10d x 1 1/2"	5/8" ANCH
HTT16	4175	ministration	_	_	-	-		and the second second second
HTT22		3695				_	18-16d	5/8" ANCH
1411122	5260	5250					32-16d	5/8" ANCI
	3075	2215					6-SDS 1/4"x2 1/2"	5/8" ANCH
HDU2-SDS2.5							10-SDS 1/4"x2 1/2"	5/8" ANCH
HDU2-SDS2.5 HDU4-SDS2.5	4565	3285						WHEN ARICH
HDU2-SDS2.5		3285 4065					14-SDS 1/4"x2 1/2"	5/8" ANCH
HDU2-SDS2.5 HDU4-SDS2.5 HDU5-SDS2.5 ABU44	4565						14-SDS 1/4"x2 1/2" 12-16d	5/8" ANCH
HDU2-SDS2.5 HDU4-SDS2.5 HDU5-SDS2.5	4565 5645	4065						



GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

CYPRESS LOG
LOG HOG FASTINERS CYPRESS WALL LOG





231 830 ALLOWABLE WITHDRAWAL STRENGTH IS BASED ON NDS2001. TABLE 11.2A 1/4" LAG SCREW VALUE x 3" THREAD x 1.6 WIND LOAD DURATION FACTOR. EXAMPLE FOR SYP .555G (280LB x)" x 1.8 = 1248 WITHDRAWAL DESIGN VALUE) -ALLOWABLE SINGLE SHEAR RESIS'ANCE IS AS LISTED IN PERMA-CHINKS PUBLICATION BASED ON NDS DESIGN VALUE FOR 3.5" SIDE MEMBER THICKNES

SHINGLE SHEAR DESIGN VALUE

WITHDRAWAL DESIGN VALUE

OG BOSS FASTENERS ARE SELF DRILLING, HIGH STRENGTH, STEEL WOOD SCREWS WITH

LOG BOSS FASTENERS ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT FASTENER OF THE SAME OR OTHERMANUFACTURER CAN BE SUBSTITUTED AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST DE FOLLOWED TO ACHIEVE RIVED LOADS. LISTED LOADS I LAVE BEEN INCREASED FOR WIND DURATION, AND MUST BEADJESTED FOR OTHER DURATIONS OR SPECIES.

DJUSTMENT FACTORS: WITHDRAWAL VALUES ARE BASEDON 3" PENETRATION INTO MAIN MEMBER. - WITHDRAWAL VALUES ARE BASEDON 3" PENETRATION INTO MAIN MEMBER.
WHEN PENETRATION IS LESS THAN 3" WITHDRAWL VALUE SHALL BE MUTIPLIED BY: p/3"
- SHEAR VALUES ARE BASED ON 10L MIN. PENETRATION INTO MAIN MEMBER.
WHEN PENETRATION IS 60-px-10D SHEAR VALUE SHALL BE MUTIPLIED BY: p/10D
- FACTOR VALUES FOR LOAD-DURAIIONS OTHER THAN 1.6 WIND DURATIONS (SHOWN)
- NDS LAG SCREW ADJUSTMENT FACTORS:
- FOR WITHDRAWAL LOADS IN END GRAIN MUTIPLY BY: .75
- FOR LATERAL LOADS IN END GRAIN MUTIPLY BY: .67
- FOR TOE-SCREWED WITHDRAWAL LOADS MUTIPLY BY: .83

N22 LOG BOSS FASTENER DATA
TYPICAL DESIGN VALUES

LOG BOSS FASTENERS DESIGN VALUE (LB)

OUTHER YELLOW PINE .55 SG

JOB NUMBER

20190501 SHEET NUMBER

OF 4 SHEETS

HANDRAIL PROFILE:
- 'TYPE I: HANDRAILS WITH A CIRCULAR CROSS SECTION SHALL HAVE AN OUTSIDE DIA. OF
- 114" - 2". IF HANDRAIL IS NOT CIRCULAR IT SHALL HAVE A PERIMETER DIMENSION OF
- 6" 14" AND MAX. CROSS SECTION OF 2 114".
- TYPE II: HANDRAILS WITH A PERIMETER GREATER THAN 6 114" SHALL PROVIDE A GRASPABLE
FINGER RECESS AREA ON BOTH SIDES OF THE PROFILE. THE FINGER RECESS SHALL BEGIN
WITHIN A DISTANCE OF 314" VERTICALLY FROM THE TALLEST PORTION OF THE PROFILE AND
A DEPTH OF AT LEAST 518" WITHIN 718" BELOW THE WIDDEST PORTION OF THE PROFILE. THIS
REQUIRED DEPTH SHALL CONTINUE FOR AT LEAST 318" TO A LEVEL THAT IS NOT LESS THAN
- 1344" BELOW THE TALLEST PORTION OF THE PROFILE. THE WIDTH OF THE HANDRAIL ABOVE
THE RECESS SHALL BE 1 314" - 2 314". EDGES SHALL HAVE A MIN. RADIUS OF 0.01". **GUARDS ON SIDES OF STAIR** NOTE: THE SUM OF TWO RISERS AND A TREAD, EXCLUDING HOSING, SHALL BE BETWEEN 24" AND 25" ABOVE HANDRAIL (2 RISERS + 1 TREAD = 24" - 25") → 4 1/2" MAX, HANDRAIL → PROJECTION HANDRAIL ON BOTH SIDES A 6" SPHERE CANNOT PASS THROUGH THE TRIANGULAR AREA FORMED BY RISER, TREAD AND GUARD RAIL LEADING EDGE OF TREAD SHALL BE NO GRATER THAT 9/16" BEVELING OF NOSING SHALL NOT EXCEED 1/2" RISERS SHALL BE VERTICAL OR SLOPED AT AN ANGLE NOT MORE THAN 30 DEGREES FROM VERTION OPEN RISERS ARE PERMITTED, PROVIDED THAT TO THE RESULT OF THE PROVIDED THAT TO THE RESULT OF THE PROVIDED THAT TO THE RESULT OF HANDRAIL ON ONE SIDES A LANDING SHALL BE PROVIDED AT THE TOP AND BOTTOM, LANDINGS SHALL HAVE A MIN WIDTH OF NOT LESS THAT STAIR WIDTH AND HAVE A MIN 36" MEASURED IN DIRECTION PASSAGE OF A 4" DIA. SPHERE. OPENING BETWEEN TREADS IS NOT LIMITED IF TOTAL RISE IS 30" OR LESS OF TRAVEL, A DOOR AT THE TOP OF STAIR IS PERMITTED, PROVIDED IT DOES NOT SWING OVER STAIR. 4 OR MORE RISERS EXCEED THE SMALLEST BY MORE THAN 3/8" STAIR DESIGN LOAD REQUIRMENTS: GUARDRAILS AND HANDRAILS: 200 LB SINGLE CONCENTRATED LIVE GUARDRAILS AND HANDRAILS: 200 LB SINGLE CONCENTRATED LIVE LOAD APPLIED IN ANY DIRECTION AT ANY POINT AL LONG THE TOP GUARDRAILS IN-FILL COMPONENTS: SO LB LIVE LOAD APPLIED HORIZ. ON AN AREA EQUAL TO 1 FT? (THIS LOAD NEED NOT BE ASSUMED TO ACT CONCURRENTLY WITH ANY OTHER LIVE LOAD REQUIRMENT.) GLAZING USED IN HANDHAIL ASSEMBLIES AND GUARDS SMALL BE DESIGNED WITH A SAFETY FACTOR OF 4. THE SAFETY FACTOR SHALL BE APPLIED TO TO EACH OF THE CONCENTRATED LOADS APPLIED TO THE TOP OF THE CONCENTRATED LOADS APPLIED TO THE TOP OF THE RAIL, AND TO THE LOAD ON THE IN-FILL COMPONENTS. THIES LOADS SHALL BE DETERMINED INDEPENDENT OF ONE ANOTHER, AND LOADS ARE ASSUMED NOT TO OCCUR WITH ANY OTHER LIVE LOAD. STAIRS: 40 PSF LIVE LOAD, OR 200 LB CONCENTRATED LOAD QVER AN AREA OF 4 INZ (WHICHEVER PRODUCES THE GREATER STRESSES) THE SMALLEST BY MORE THAN 3/8" WINDER TREADS SHALL HAVE A MIN. DEPTH OF 6" AT ANY
POINT. WITHIN A FLIGHT OF STAIRS THE GREATEST DEPTH
AT 12" SHALL NOT EXCEED THE SMALLEST BY NORE THAN 3/8"

(1" VERT UNIT IN 48" HORZ. UNITS) MAX. TREAD SLOPE OF 2 PERCENT STAIR & GUARDRAIL REQUIRMENTS TYPICAL REQUIRMENTS PER FBCR

