

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI.

REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL.

(25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING

ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT.
AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED.

DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,



COEFFICIENT

ROOF ANGLE

INTERNAL PRESSURE

MEAN ROOF HEIGHT

FLOOR LOADING

ROOMS OTHER THAN

SLEEPING ROOM

FLAT OR < 4:12

4:12 TO < 12:12

12:12 & GREATER

FLOOD ZONE

WIND AREA (FT2)

7 GARAGE DOOR

16x7 GARAGE DOOR

SOIL BEARING CAPACITY 1500 PSF

SLEEPING ROOMS

**ROOF LOADING** 

DESIGN CRITERIA & LOA	DS:	not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.
BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)	CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable
CODE FOR DESIGN LOADS	ASCE 7-10	portions of the plan, relating to wind engineering comply with the 6th Edition Florida
WINDLOADS		Building Code Residential (2017)
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH	to the best of my knowledge.
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	С	LIMITATION: This design is valid for one building, at specified location.
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I	MARK DISOSWAY P.E. 53915
RISK CATEGORY	II .	THIS PDF HAS DIGITAL SIGNATURE
ENCLOSURE CLASSIFICATION	ENCLOSED	AND ELECTRONIC SEAL. PRINTED
Contract to the contract of th		

THIS BUILDING IS NOT IN THE FLOOD ZONE

**END 4' FROM ALL** 

**OUTSIDE CORNER** 

+42.6 -57

30 PSF LIVE LOAD

20 PSF LIVE LOAD

16 PSF LIVE LOAD 12 PSF LIVE LOAD

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C) (Vult)

+42.6 -46.2

+22.6 -25.5

+21.7 -24.1

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C) (ASD)

**COPIES ARE NOT CONSIDERED** SIGNED OR SEALED. YOU MUS 7-45 DEGREES VERIFY SIGNATURE ON THIS PDF. CLICK HERE TO VERIFY. C&C DESIGN PRESSURES SEE TABLE No 53915 40 PSF LIVE LOAD

> Tuesday, June 2, 2020 Mark Disosway P.E. 163 SW Midtown Place Suite 103

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution.

Mark Disosway, P.E. hereby expressly reserve

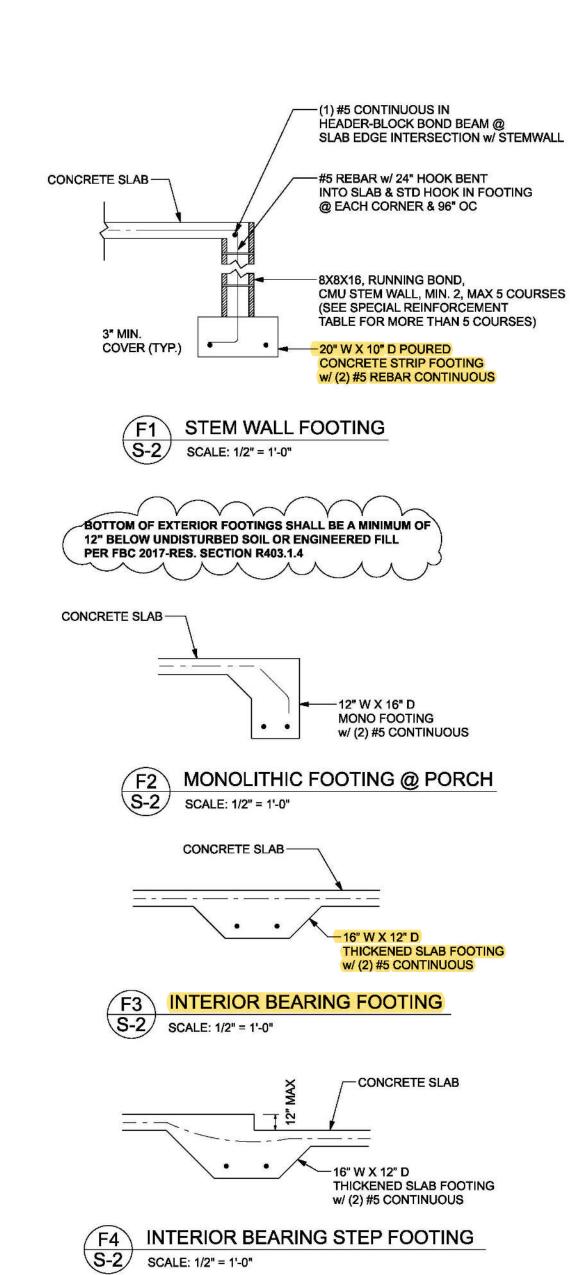
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STATE OF

Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com JOB NUMBER:

200405 **S-1** OF 6 SHEETS



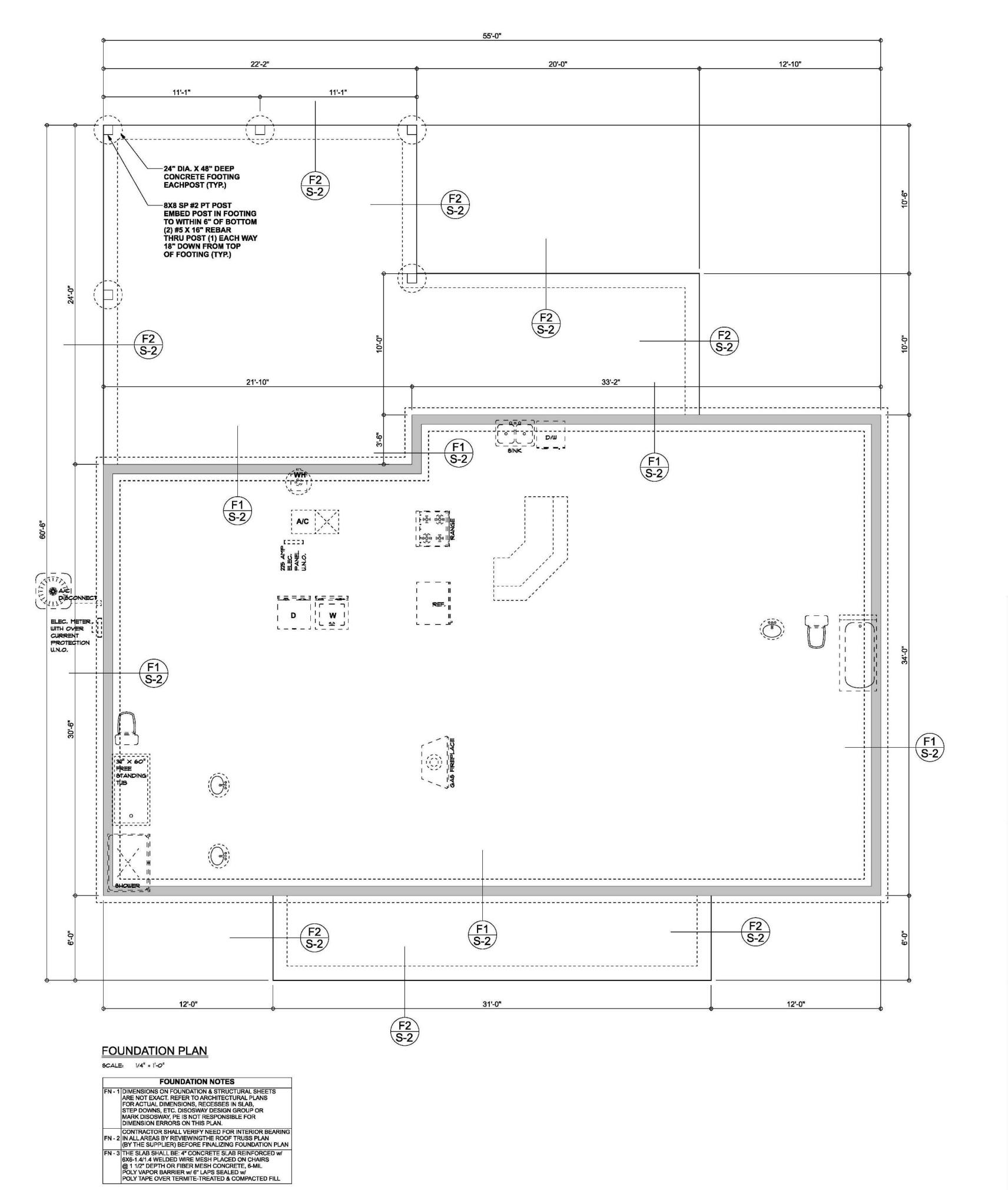


TALL STEM WALL TABLE:

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT
SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION
FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602).
THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE
PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS
BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.
ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY
THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ertically or	vall). If the wall is a horizontal bon CMU may be us	d beam w	ith 1#5 con	tinuous at r	nid height. I	For higher p	
STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



Austin & Lindsey Reiters Res.

PROJECT ADDRESS:
SW Fritz Glen
Lake City, FL

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have

examined this plan, and that the applicable

portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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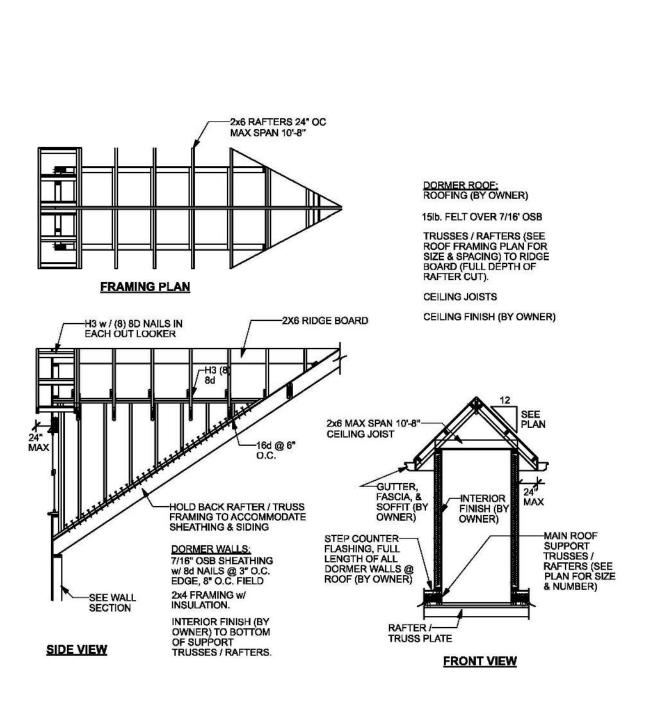
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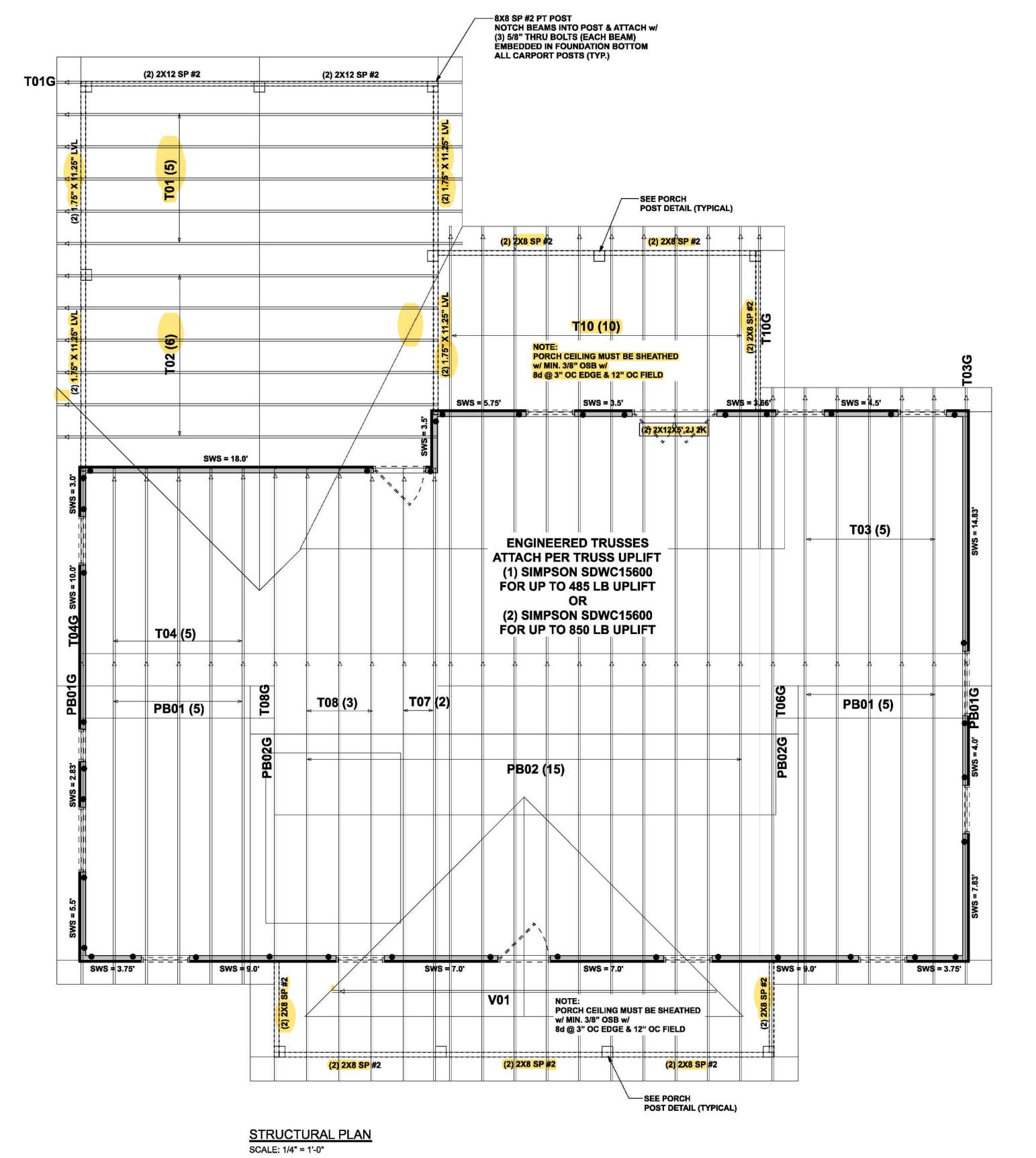
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JOB NUMBER: 200405 **S-2** 

OF 6 SHEETS



DORMER ANCHORING DETAIL (ON ROOF) SCALE: N.T.S.



## STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD

EACH SIDE (U.N.O.)

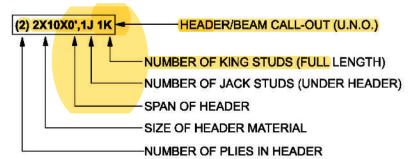
SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD

SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## **HEADER LEGEND**



## THREADED ROD LEGEND



## ACTUAL vs REQUIRED SHEARWALL

TOIOAL	13 ILEGOIILE	DOILLAND
	TRANSVERSE	LONGITUDUNAL
ACTUAL	20596 LBF	17978 LBF
REQUIRED	17451 LBF	12877 LBF
		_

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> JOB NUMBER: 200405

**S-3** OF 6 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2344397