



Alpine, an ITW Company 6750 Forum Drive, Suite 305 Orlando, FL 32821 Phone: (800)755-6001 www.alpineitw.com

Site Information:

Customer: W. B. Howland Company, Inc.

Job Number: 21-5051

Job Description: 50x30 pavilion

Address:

| Job Engineering Criteria:                      |                                |  |  |  |  |  |  |
|--|--------------------------------|--|--|--|--|--|--|
| Design Code: FBC 7th Ed. 2020 Res              | IntelliVIEW Version: 20.01.01A |  |  |  |  |  |  |
|  | JRef #: 1X222150007            |  |  |  |  |  |  |
| Wind Standard: ASCE 7-16 Wind Speed (mph): 130 | Design Loading (psf): 40.00    |  |  |  |  |  |  |
| Building Type: Open Obstructed                 |                                |  |  |  |  |  |  |

This package contains general notes pages, 2 truss drawing(s) and 2 detail(s).

| Item | Drawing Number    | Truss |
|------|-------------------|-------|
| 1    | 014.21.1338.04430 | A01   |
| 3    | A16015ENC160118   |       |

| Item | Drawing Number    | Truss |
|------|-------------------|-------|
| 2    | 014.21.1338.11383 | A02   |
| 4    | GBLLETIN0118      |       |



# **General Notes**

# Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

## **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

## Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

## **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

## Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

# **General Notes** (continued)

# **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

## References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; <a href="https://www.alpineitw.com">www.alpineitw.com</a>.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 397829 COMN Ply: 1 Job Number: 21-5051 Cust: R 215 JRef: 1X222150007 T1 FROM: CDM 50x30 pavilion Qty: 24 DrwNo: 014.21.1338.04430 Truss Label: A01 / YK 01/14/2021 5'4"5 10'2"3 19'9"13 24'7"11 30' 5'4"5 4'9"13 4'9"13 4'9"13 4'9"13 5'4"5 6\*6 L ≡3X4 =7X8 J ≡3X4 =4X4(A1) =4X4(A1) 30' 7'9"4 7'2"12 7'2"12 7'9"4 7'9"4 15' 22'2"12 30' ▲ Maximum Reactions (lbs)

| Loading Criteria (psf)            | Wind Criteria  | Snow Criteria (Pg,Pf in PSF)  | Defl/CSI Criteria           |   |
|-----------------------------------|--|---|-----------------------------|---|
| Loading Criteria (psf)            | Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Open Obst. Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) | Defl/CSI Criteria           |   |
| GCpi: 0.00<br>Wind Duration: 1.60 |  | Plate Type(s):<br>WAVE  | VIEW Ver: 20.01.01A.0724.11 | ] |
| Lumber                            |  |   |                             |   |

### Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 1367 /-/949 /306 /242 1367 /-/-/949 /306 /-Wind reactions based on MWFRS Brg Width = 3.5Min Reg = 1.5Brg Width = 3.5 Min Req = 1.5 Bearings M & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 542 - 2157 450 - 1421

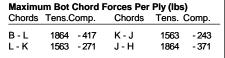
Top chord: 2x6 SP 2400f-2.0E(PT-Borate); Bot chord: 2x6 SP 2400f-2.0E(PT-Borate); Webs: 2x4 SP #3(PT-Borate);

Wind loads based on MWFRS with additional C&C member design.

## **Additional Notes**

The overall height of this truss excluding overhang is

NOTE: BUILDING ENCLOSURE IS OPEN PER REQUEST



F-G

G-H

520 - 1963

542 - 2157

# Maximum Web Forces Per Ply (lbs)

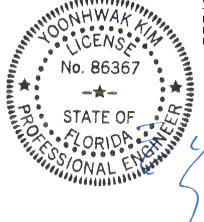
520 - 1963

450 - 1421

C - D

D-E

| AA GD2 | rens.comp. | MEDS | rens. Comp. |  |  |  |  |
|--------|------------|------|-------------|--|--|--|--|
| L-D    | 391 - 55   | K-F  | 250 - 503   |  |  |  |  |
| D - K  | 248 - 503  | F-J  | 391 - 58    |  |  |  |  |
| E - K  | 887 - 241  |      |             |  |  |  |  |



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/14/2021

\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

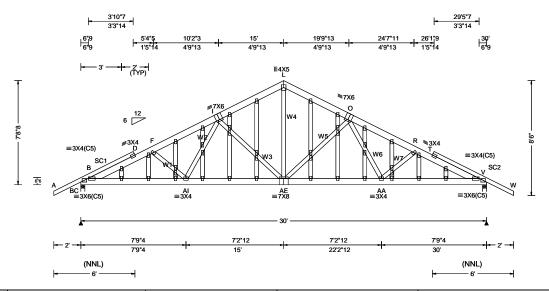
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 397832 GABL Ply: 1 Job Number: 21-5051 Cust: R 215 JRef: 1X222150007 T2 DrwNo: 014.21.1338.11383 FROM: CDM Qty: 2 50x30 pavilion Truss Label: A02 / YK 01/14/2021



| Loading Criteria (psf) | Wind Criteria                           | Snow Criteria (Pg,Pf in PSF)        | DefI/CSI Criteria               | Ī |
|------------------------|---|-------------------------------------|---------------------------------|---|
| TCLL: 20.00            | Wind Std: ASCE 7-16                     | Pg: NA Ct: NA CAT: NA               | PP Deflection in loc L/defl L/# |   |
| TCDL: 10.00            | Speed: 130 mph                          | Pf: NA Ce: NA                       | VERT(LL): 0.056 AG 999 240      | þ |
| BCLL: 0.00             | Enclosure: Open Obst.                   | Lu: NA Cs: NA                       | VERT(CL): 0.113 AG 999 180      | þ |
| BCDL: 10.00            | Risk Category: II                       | Snow Duration: NA                   | HORZ(LL): 0.016 X               |   |
| Des Ld: 40.00          | EXP: C Kzt: NA<br>Mean Height: 15.00 ft |                                     | HORZ(TL): 0.032 X               |   |
| NCBCLL: 10.00          | TCDL: 5.0 psf                           | Building Code:                      | Creep Factor: 2.0               |   |
| Soffit: 2.00           | BCDL: 5.0 psf                           | FBC 7th Ed. 2020 Res.               | Max TC CSI: 0.407               |   |
| Load Duration: 1.25    | MWFRS Parallel Dist: 0 to h/2           | TPI Std: 2014                       | Max BC CSI: 0.200               |   |
| Spacing: 24.0 "        | C&C Dist a: 3.00 ft                     | Rep Fac: Yes                        | Max Web CSI: 0.332              |   |
|                        | Loc. from endwall: Any                  | FT/RT:20(0)/10(0)                   |                                 |   |
|                        | GCpi: 0.00                              | Plate Type(s):                      |                                 | 4 |
| Wind Duration: 1.60    |   | WAVE                                | VIEW Ver: 20.01.01A.0724.11     |   |
| Spacing: 24.0 "        | Loc. from endwall: Any<br>GCpi: 0.00    | FT/RT:20(0)/10(0)<br>Plate Type(s): |                                 |   |

### Loc R+ /Rh /Rw /U /RL BC 1369 /-/712 /29 1369 /-/712 /-Wind reactions based on MWFRS BC Brg Width = 3.5 Min Rea = 1.5Brg Width = 3.5 Min Rea = 1.5Bearings BC & V are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 0 - 971 40 - 1454 B - D 0 - 1365 0 - R 151 - 2069 D-F 0 - 2241 R - T 141 - 2241 F - I 0 - 2069 T - V 74 - 971

Non-Gravity

▲ Maximum Reactions (lbs)

Gravity

Top chord: 2x6 SP 2400f-2.0E(PT-Borate); Bot chord: 2x6 SP 2400f-2.0E(PT-Borate); Webs: 2x4 SP #3; W1,W2,W3,W4,W5,W6, W7 2x4 SP #3(PT-Borate); Stack Chord: SC1 2x4 SP #2(PT-Borate); Stack Chord: SC2 2x4 SP #2(PT-Borate);

## **Plating Notes**

All plates are 2X4 except as noted.

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind loads based on MWFRS with additional C&C member design.

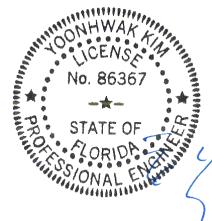
### **Additional Notes**

See DWGS A16015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is

NOTE: BUILDING ENCLOSURE IS OPEN PER REQUEST



I-L 73 - 1454 61 - 1365 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 1634 R-AI 2049 AF-AA n AI-AE 1634 0 AA-V 2049 - 15 Maximum Web Forces Per Ply (lbs)

| Webs  | Tens.C | Comp. | Webs  | Tens. ( | Comp. |
|-------|--------|-------|-------|---------|-------|
| Al- I | 408    | 0     | AE- O | 216     | - 553 |
| I -AE | 0      | - 553 | O -AA | 408     | - 116 |
| L -AE | 870    | - 19  |       |         |       |

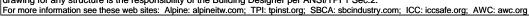
FL REG# 278, Yoonhwak Kim, FL PE #86367 01/14/2021

\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.





# Gable Stud Reinforcement Detail

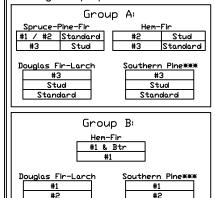
ASCE 7-16: 160 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

|                     |          | 2x4<br>Vertica | Brace    | No             | (1) 1×4 "L    | Brace *        | (1) 2×4 *L     | " Brace *     | (2) 2x4 <b>"</b> L | Brace ** | (1) 2×6 *L | * Brace * | (2) 2×6 L | Brace ** | 1 |
|---------------------|----------|----------------|----------|----------------|---------------|----------------|----------------|---------------|--------------------|----------|------------|-----------|-----------|----------|---|
|                     | Spacing  | Species        | Grade    | _              | Group A       | Group B        | Group A        | Group B       | Group A            | Group B  | Group A    | Group B   | Group A   | Group B  |   |
| 수                   |          | CDE            | #1 / #2  | 3′ 10 <b>″</b> | 6′ 7″         | 6′ 10 <b>″</b> | 7′ 9″          | 8′ 1″         | 9′ 3″              | 9′ 7″    | 12′ 2″     | 12′ 8″    | 14′ 0″    | 14′ 0″   | ĺ |
|                     |          | SPF            | #3       | 3′ 8″          | 5′ 9″         | 6′ 2″          | 7′ 8 <b>″</b>  | 7′ 11″        | 9′ 1″              | 9′ 6″    | 12′ 0″     | 12′ 6″    | 14′ 0″    | 14′ 0″   |   |
| ഉ                   | Ų        | HF             | Stud     | 3′ 8″          | 5′ 9″         | 6′ 1″          | 7′ 8 <b>″</b>  | 7′ 11″        | 9′ 1″              | 9′ 6″    | 12′ 0″     | 12′ 6″    | 14′ 0″    | 14′ 0″   |   |
| Č                   | Ō        | 1 11           | Standard | 3′ 8″          | 4′ 11″        | 5′ 3″          | 6′ 7 <b>″</b>  | 7′ 1″         | 8′ 11 <b>″</b>     | 9′ 6″    | 10′ 4″     | 11′ 1″    | 14′ 0″    | 14′ 0″   |   |
| o.                  |          |                | #1       | 4′ 0″          | 6′ 8″         | 6′ 11″         | 7′ 10″         | 8′ 2″         | 9′ 4″              | 9′ 8″    | 12′ 4″     | 12′ 9″    | 14′ 0″    | 14′ 0″   |   |
|                     | *        | SP             | #2       | 3′ 10 <b>″</b> | 6′ 7″         | 6′ 10 <b>″</b> | 7′ 9″          | 8′ 1″         | 9′ 3″              | 9′ 7″    | 12′ 2″     | 12′ 8″    | 14′ 0″    | 14′ 0″   |   |
|                     | 4        | ا ہے۔ ا        | #3       | 3′ 9″          | 5′ 3 <b>″</b> | 5′ 7 <b>″</b>  | 6′ 11″         | 7′ 5″         | 9′ 2″              | 9′ 7″    | 10′ 11″    | 11′ 8″    | 14′ 0″    | 14′ 0″   | 1 |
| g                   | $\Omega$ | IDFLI          | Stud     | 3′ 9″          | 5′ 3″         | 5′ 7 <b>″</b>  | 6′ 11″         | 7′ 5″         | 9′ 2″              | 9′ 7″    | 10′ 11″    | 11′ 8″    | 14′ 0″    | 14′ 0″   |   |
|                     |          |                | Standard | 3′ 6″          | 4′ 7″         | 4′ 11″         | 6′ 2 <b>′</b>  | 6′ 7″         | 8′ 4″              | 8′ 11″   | 9′ 8″      | 10′ 4″    | 13′ 1″    | 14′ 0″   |   |
| <u>U.</u>           |          | CDE            | #1 / #2  | 4′ 5″          | 7′ 6″         | 7′ 9″          | 8′ 10 <b>″</b> | 9′ 3″         | 10′ 7″             | 11′ 0″   | 13′ 11″    | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
|                     |          | SPF<br>HF      | #3       | 4′ 2″          | 7′ 1″         | 7′ 9″          | 8′ 9 <b>″</b>  | 9′ 1″         | 10′ 5″             | 10′ 10″  | 13′ 9″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
| `_                  | U        |                | Stud     | 4′ 2″          | 7′ 1″         | 7′ 6″          | 8′ 9 <b>″</b>  | 9′ 1″         | 10′ 5″             | 10′ 10″  | 13′ 9″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
| امَا                | U O ,    |                | Standard | 4′ 2″          | 6′ 1″         | 6′ 5 <b>″</b>  | 8′ 1″          | 8′ 8 <b>″</b> | 10′ 5″             | 10′ 10″  | 12′ 8″     | 13′ 7″    | 14′ 0″    | 14′ 0″   |   |
| 🖑                   |          | SP             | #1       | 4′ 7″          | 7′ 7″         | 7′ 11″         | 9′ 0″          | 9′ 4″         | 10′ 8″             | 11′ 1″   | 14′ 0″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
| /                   |          |                | #2       | 4′ 5″          | 7′ 6″         | 7′ 9″          | 8′ 10″         | 9′ 3″         | 10′ 7″             | 11′ 0″   | 13′ 11″    | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
|                     | è        | l              | #3       | 4′ 4″          | 6′ 5 <b>″</b> | 6′ 10″         | 8′ 6 <b>″</b>  | 9′ 1″         | 10′ 6″             | 10′ 11″  | 13′ 4″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
| ll orl              | 16       | IDFLI          | Stud     | 4′ 4″          | 6′ 5 <b>″</b> | 6′ 10″         | 8′ 6 <b>″</b>  | 9′ 1″         | 10′ 6″             | 10′ 11″  | 13′ 4″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
|                     |          |                | Standard | 4′ 2″          | 5′ 8″         | 6′ 0 <b>″</b>  | 7′ 6 <b>″</b>  | 8′ 0 <b>″</b> | 10′ 2″             | 10′ 10″  | 11′ 10″    | 12′ 7″    | 14′ 0″    | 14′ 0″   |   |
| Jap                 |          | SPF            | #1 / #2  | 4′ 10″         | 8′ 3″         | 8′ 7 <b>″</b>  | 9′ 9″          | 10′ 2″        | 10′ 7″             | 12′ 1″   | 14′ 0″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
| d                   |          | 12LL           | #3       | 4′ 7″          | 8′ 2 <b>″</b> | 8′ 5 <b>″</b>  | 9′ 8″          | 10′ 0″        | 11′ 6″             | 12′ 0″   | 14′ 0″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
| اقاا                | Ų        | HF             | Stud     | 4′ 7″          | 8′ 2 <b>″</b> | 8′ 5 <b>″</b>  | 9′ 8 <b>″</b>  | 10′ 0″        | 11′ 6″             | 12′ 0″   | 14′ 0″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
|                     | Ō        | 1 11           | Standard | 4′ 7″          | 7′ 0″         | 7′ 5″          | 9′ 4″          | 10′ 0″        | 11′ 6″             | 12′ 0″   | 14′ 0″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
| ×                   |          |                | #1       | 5′ 1″          | 8′ 5 <b>″</b> | 8′ 8 <b>″</b>  | 9′ 11″         | 10′ 3″        | 11′ 9″             | 12′ 3″   | 14′ 0″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
| 3                   | *        | SP             | #2       | 4′ 10″         | 8′ 3″         | 8′ 7 <b>″</b>  | 9′ 9″          | 10′ 2″        | 11′ 7″             | 12′ 1″   | 14′ 0″     | 14′ 0″    | 14′ 0″    | 14′ 0″   | 1 |
| $  \breve{\Sigma} $ | ù        | ا ہے۔ ا        | #3       | 4′ 9″          | 7′ 4″         | 7′ 10″         | 9′ 8″          | 10′ 1″        | 11′ 7″             | 12′ 0″   | 14′ 0″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
|                     | 10       | IDFLI          | Stud     | 4′ 9″          | 7′ 4″         | 7′ 10″         | 9′ 8″          | 10′ 1″        | 11′ 7″             | 12′ 0″   | 14′ 0″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |
|                     | •        | _              | Standard | 4′ 7″          | 6′ 6″         | 6′ 11″         | 8′ 8 <b>″</b>  | 9′ 3″         | 11′ 6″             | 12′ 0″   | 13′ 7″     | 14′ 0″    | 14′ 0″    | 14′ 0″   |   |



Bracing Group Species and Grades:

1x4 Braces shall be SRB (Stress-Rated Board) \*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 75 plf over continuous bearing (5 psf TC Dead Load).

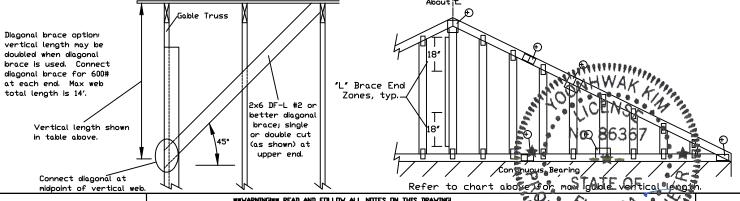
Gable end supports load from 4' 0" outlookers with 2' 0' overhang, or 12' plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nalls at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Gable Vertical Plate Sizes  |           |  |  |  |
|---|-----------|--|--|--|
| Vertical Length   | No Splice |  |  |  |
| Less than 4' 0"   | 2X3       |  |  |  |
| Greater than 4' 0", but<br>less than 11' 6"                       | 3X4       |  |  |  |
| Greater than 11' 6" 4X4   |           |  |  |  |
| + Refer to common truss design for peak, splice, and heel plates. |           |  |  |  |

Refer to the Building Designer for conditions not addressed by this detail.



\*\*\*VARNINGI\*\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING \*\*\*IMPORTANT\*\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, shaping, installing and installing and follow the latest edition of BCSI (Building Component Safety Information, brill and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design his drawing, indicates acceptance of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sited 14/2021 ALPINE: www.alpineitw.com, TPI: www.tpinstorg, SBCA: www.sbcindustry.org, ICC: www.lcestere.org, 278 Yoonhwak Kim, FL PE #86367

ASCE7-16-GAB16015 |DATE 01/26/2018 

MAX, TOT, LD, 60 PSF

MAX. SPACING 24.0"

514 Earth City Expressway Suite 242

Earth City, MO 63045

# Gable Detail For Let-in Verticals Gable Truss Plate Sizes Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs. (+) Refer to Engineered truss design for peak, splice, web, and heel plates. \*If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web. Gable Example: Length typ.

Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

10d Common (0.148"x 3.", min) Nails at 4" o.c. plus

(4) nails in the top and bottom chords.

10d Common (0.148"x3".min) Toenails at 4" o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A14013ENC100118,

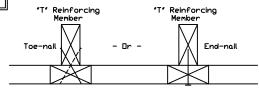
A18015ENC100118, A12015ENC100118, A12015ENC100118, A12015ENC100118, A120015ENC100118, A120015ENC100118, A120015ENC100118, A120015ENC100118, A12003ENC100118, A12003ENC100118, A120030ENC100118, A120030ENC100118,

\$18015ENC100118, \$20015ENC100118, \$20015END100118, \$20015PED100118 \$11530ENC100118, \$12030ENC100118, \$14030ENC100118, \$12030ENC100118)

\$18030ENC100118, \$20030ENC100118, \$20030END100118, \$20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical

### "T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

## Web Length Increase w/ "T" Brace

| "T" Reinf. | "T"      |
|------------|----------|
| Mbr. Size  | Increase |
| 2×4        | 30 %     |
| 2x6        | 20 %     |

### Example:

ASCE 7-10 Wind Speed = 120 mph Mean Roof Height = 30 ft, Kzt = 1.00 Gable Vertical = 24°o.c. SP #3 "T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30

(1) 2x4 "L" Brace Length = 8' 7" Maximum "T" Reinforced Gable Vertical Length

 $1.30 \times 8' \ 7'' = 11' \ 2''$ 

\*\*\*VARNINGI\*\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING \*\*\*IMPORTANT\*\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS. Trusses require extreme care in fabricating, shaping, shipping, installing and pracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, br PI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, nstallation 8 bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional

engineering responsibility solely for the design shown. The suitability and use of this for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: 1/2021 ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.lefs.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.lefs.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.sbcindu

IREF LET-IN VERT DATE 01/02/2018 DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF DUR. FAC. ANY

MAX. SPACING 24.0"



Rigid Sheathing

Ceiling

4 Nails

Nails

Spaced At

4 Nails

Reinforcing Member

Gable

Truss

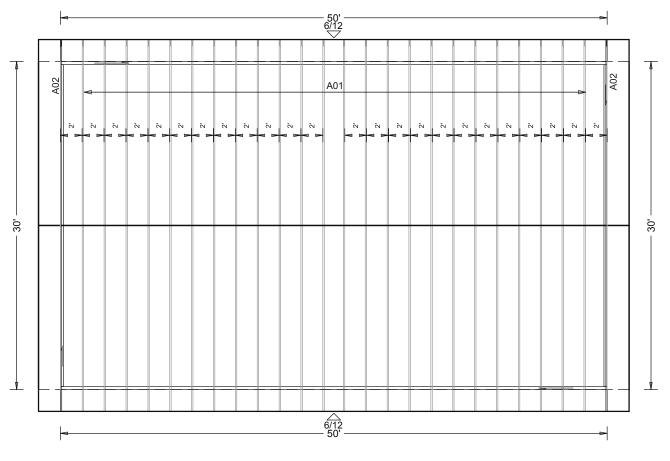
514 Earth City Expressway Suite 242 Earth City, MO 63045

JOB NO: 21-5051

PAGE NO:

1 OF 1

\*\*\*2X6 top and bottom chords
\*\*\*All pressure treated lumber used



W.B. Howland Truss Co. 610 11th St. SW Live Oak, FL 32064 (386) 362-1235 (386) 362-7124 (Fax) howlandtruss@gmail.com

DATE: 1/14/21