ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1UW3487-Z0210062828

Truss Fabricator: Anderson Truss Company

Job Identification: 12-276A--Little & Williams Smith Res -- Lake City, FL

Truss Count: 19

Model Code: Florida Building Code 2010
Truss Criteria: FBC2010Res/TPI-2007(STD)

Engineering Software: Alpine Software, Version 12.03.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 37.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 120 MPH ASCE 7-10 -Closed

Notes:

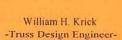
 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-

#	Ref Description	Drawing#	Date
1	17085-AV1 40' Common G	13130006	05/10/13
2	17086-AV2 35'5"8 Commo	13130004	05/10/13
3	17087-AV3 40' Common G	13130007	05/10/13
4	17088-FG1 3'10"14 Flat	13130005	05/10/13
5	17089A 30' Common	13130011	05/10/13
6	17090A1 30' Common	13130021	05/10/13
7	17091A2 40' Common	13130008	05/10/13
8	17092AGE 30' Gable	13130009	05/10/13
9	17093AGE1 30' Gable	13130010	05/10/13
10	17094A3 30' Common	13130022	05/10/13
11	17095AV 40' Common	13130012	05/10/13
12	17096AV4 40' Common	13130013	05/10/13
13	17097B 24' Common	13130014	05/10/13
14	17098B1 24' Common	13130015	05/10/13
15	17099-B2 23'8"8 Common	13130016	05/10/13
16	17100BGE 24' Gable	13130017	05/10/13
17	17101C 14' Common	13130018	05/10/13
18	17102C1 14' Common	13130019	05/10/13
19	17103CGE 14' Gable	13130020	05/10/13



MANUAL PROPERTY OF THE PARTY OF

1950 Marley Drive Haines City, FL 33844



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18 Top chord 2x4 SP_#1_12A :T5 2x4 SP M-30: Bot chord 2x4 SP M-30 :B2 2x4 SP #1 12A: :B3 2x6 SP #2_12A: :B4 2x6 SP_SS_12A: Webs 2x4 SP_#3_12A Negative reaction(s) of -573# MAX. (See below) from a non-wind load case requires uplift connection. Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC. Special at 19 at 25 at 30 at -1 at 10 at 25 at 25 33 to 33 to 33 to 34 to 35 77777777 =1.25) 8.26 19.00 25.00 30.37 40.00 10.33 25.00 30.48 40.00

Wind loads and reactions based on MWFRS

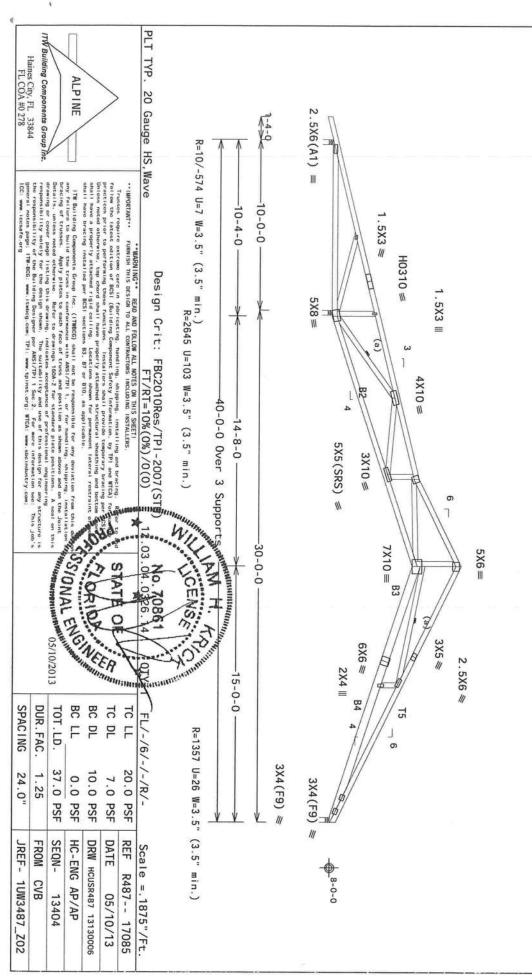
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (a) Continuous lateral bracing equally spaced on

Shim all supports to solid bearing

Calculated vertical deflection is 0.37" due to live load and 0.65" due to dead load at X = 31-10-13.

Calculated horizontal deflection is 0.23" due to live load and 0.39" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load



Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Calculated horizontal deflection is 0.09" due to live load and 0.18" due to dead load.

(a) Continuous lateral bracing equally spaced on member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Negative reaction(s) of -217# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not locate.

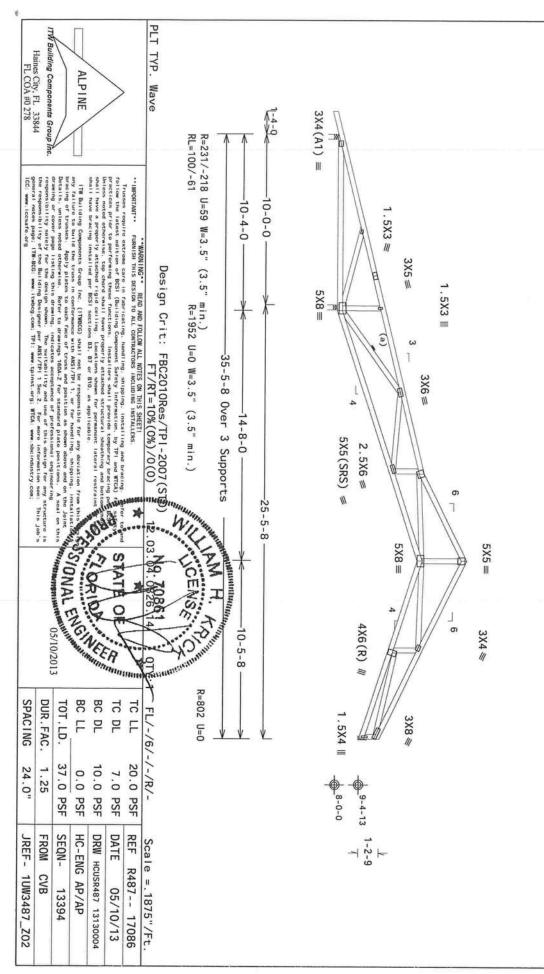
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load

Shim all supports to solid bearing.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi (+/-)=0.18 Top chord 2x4 SP_#1_12A :T5 2x4 SP M-30: Bot chord 2x4 SP M-30 :B2 2x4 SP_#1_12A: Webs 2x4 SP_#3__12A Negative reaction(s) of -631# MAX. (See below) from a non-wind load case requires uplift connection. (a) Continuous lateral bracing equally spaced on member Wind loads and reactions based on MWFRS Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC. Special loads 7577777777777777 5 556 556 566 271 271 at -1.33 to at 19.00 to at 25.00 to at -1.33 to at -1.33 to at -1.33 to at -1.33 to at 10.03 to at 40.00 to at 40.00 to at 40.00 to Dur. Fac. =1.25)
4 pif at 18.26
4 pif at 19.26
6 pif at 25.03
6 pif at 41.33
6 pif at (1.33)
6 pif at 41.33
7 pif at (1.33)
7 pif at (2.00)
7 pif at (2.33)
7 pif at (3.33) due to live load and 0.45"

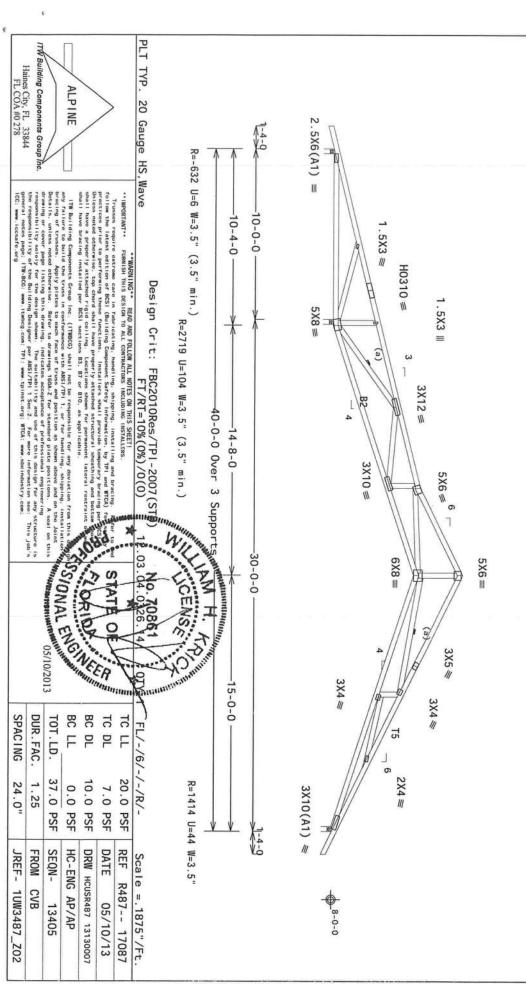
Calculated horizontal deflection is 0.27" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load

Calculated vertical deflection is 0.43" due to live load and 0.73" due to dead load at X = 32-0-14.

Shim all supports to solid bearing.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50.\,$



Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Special loads

-----(Lumber Dur.Fac.=1.25 / Piate Dur.Fac.=1.25)
TC- From 54 pif at 0.00 to 54 pif at 3.91
BC- From 20 pif at 0.00 to 20 pif at 3.91
BC- 802.26 ib Conc. Load at 1.97

Wind loads and reactions based on MWFRS

Truss must be installed as shown with top chord up

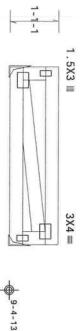
TRUSSES REQUIRED

2 COMPLETE TRUSSES R
Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row #1.50" o.c.
Bot Chord: 1 Row #7.50" o.c.
Webs 1 Row # 4" o.c.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

120 mph wind. 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18 Deflection meets $L/240\,$ live and $L/180\,$ total load. Creep increase factor for dead load is 1.50.

The TC of this truss shall be braced with attached spans at 24 $^{\circ}$ OC in lieu of structural sheathing.



3X4 ≡



R=542 U=0 < 3-10-14 Over 2 Supports R=549 U=0 V

Design Crit: FBC2010Res/TPI-2007(**S慢)**) FT/RT=10%(0%)/0(0)

PLT TYP. Wave

"MARNING" READ AND FOLLOW ALL NOTES ON THIS SHEET!

Trussos require extreme care in fabricating, handling, shipping, installing and brising, follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) represented by prior to performing those functions. Installers shall provide temporary bracing personal behaviors shall provide temporary bracing publishes noted otherwise, top chord shall have properly attached structural sheathing and bottom shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3. B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCC) shall not be responsible for any deviation from this any failure to build the truss in conformance with ARS/ITP 1, or for handling, shipping, installate bracing of trusses. Apply plates to each face of truss and position a shown above and on the Join Dotalls, unless noted otherwise. Refer to drawings 180A-Z for standard plate positions. A soal on A soal on this

Building Components Group Haines City, FL 33844 FL COA #0 278

ALPINE

0 SONAL ENGINEE FL/-/6/-/-/R/-TC DL DUR. FAC. BC DT TC LL SPACING TOT.LD. 37.0 PSF 20.0 PSF 24.0" 1.25 10.0 PSF 0.0 PSF 7.0 PSF

> DATE REF

Scale = .5"/Ft.

R487-- 17088 05/10/13

SEQN-

13403

FROM CVB

JREF - 1UW3487_Z02

HC-ENG AP/AP DRW HCUSR487 13130005

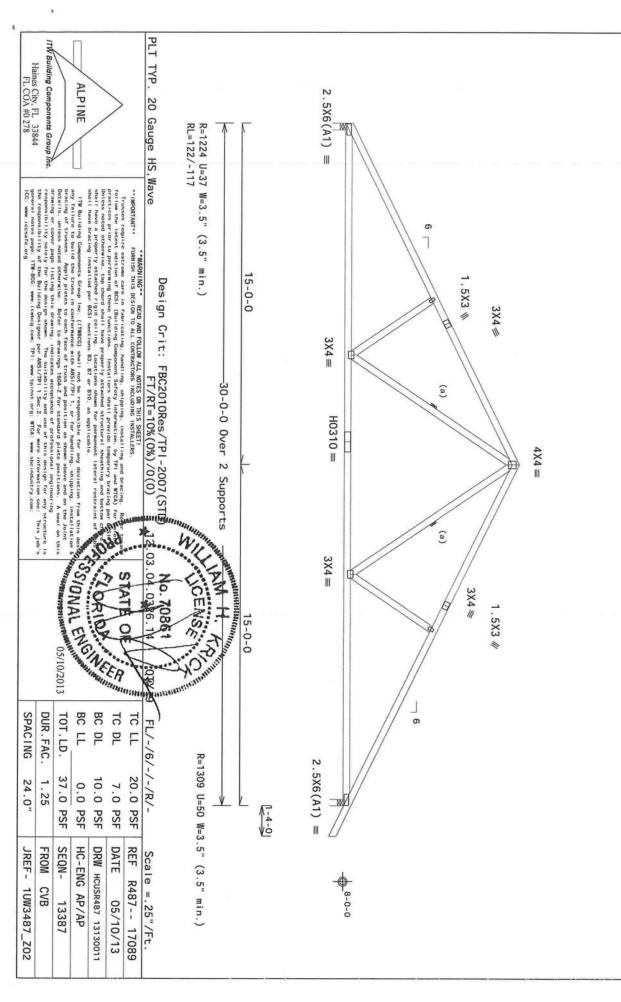
Top chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Webs 2x4 SP_#3__12A Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

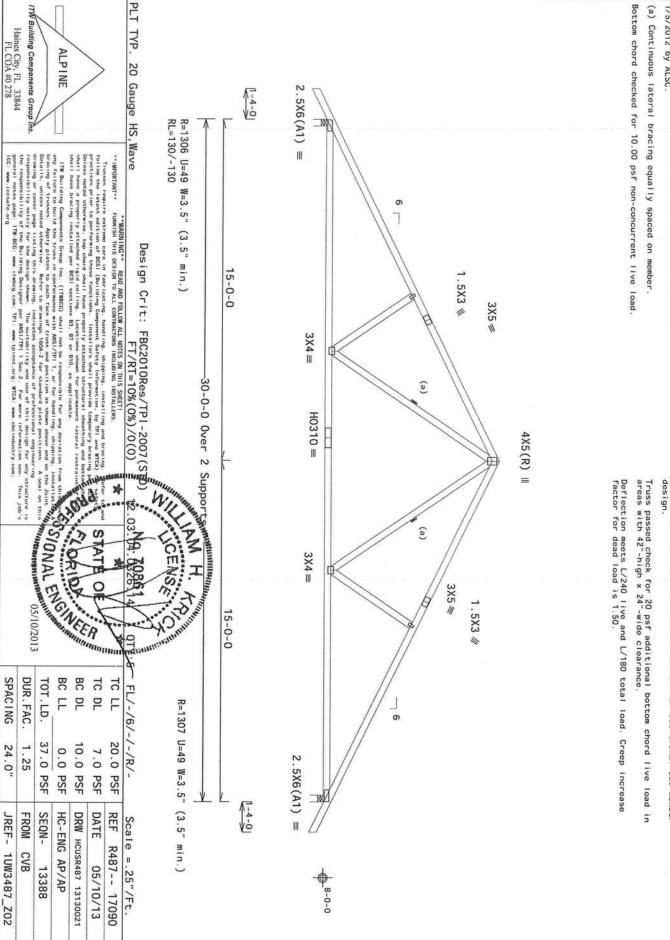
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Truss passed check for 20 psf additional bottom chord live load in areas with 42° -high x 24° -wide clearance. Wind loads and reactions based on MWFRS with additional C&C member



Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member



Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Gable end supports 8" max rake overhang.

(a) 1x4 #3SRB SPF-S or better "L" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)naiis @ 6" OC.

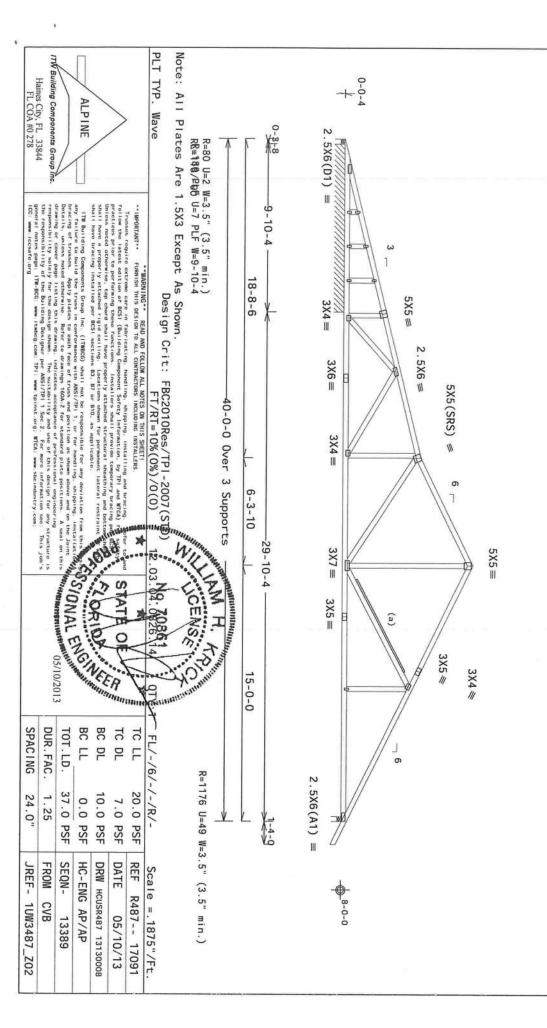
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.



Top chord 2x4 SP #1_12A Bot chord 2x4 SP #1_12A Bot SP 2x4 SP #3 12A Stack Chord SC1_2x4 SP #1_12A:

Lumber grades designated with "12A" use 1/5/2012 by ALSC. design values approved

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load

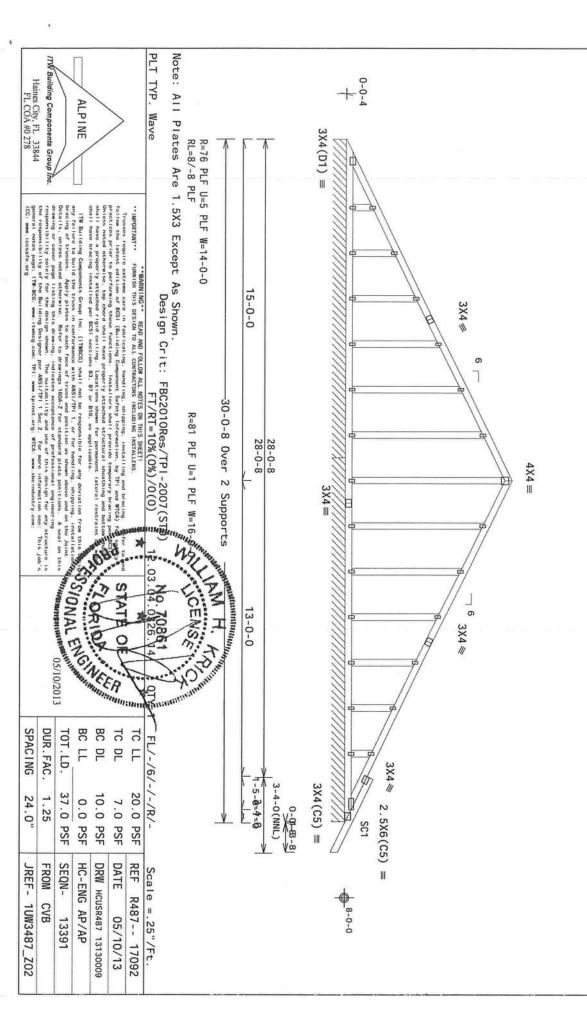
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member

Gable end supports 8" max rake overhang

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



Top chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Webs 2x4 SP_#3__12A :Stack Chord SC1 2x4 SP_#1 See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements. Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC. _12A::Stack Chord SC2 2x4 SP_#1_12A:

Bottom chord checked for 10.00 psf non-concurrent live load

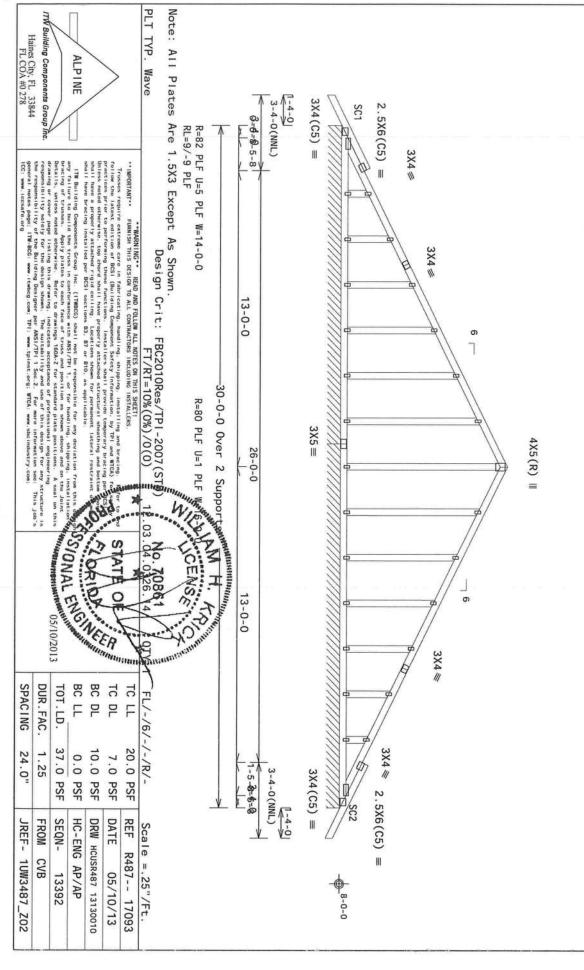
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member

Gable end supports 8" max rake overhang.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tic-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



Top chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Webs 2x4 SP_#3__12A :W3 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Calculated horizontal deflection is 0.52" due to live load and 0.71" due to dead load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

120 mph wind. 15.00 ft mean hgt. ASCE 7-10. CLOSED bidg. not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf. GCpi(+/-)=0.18

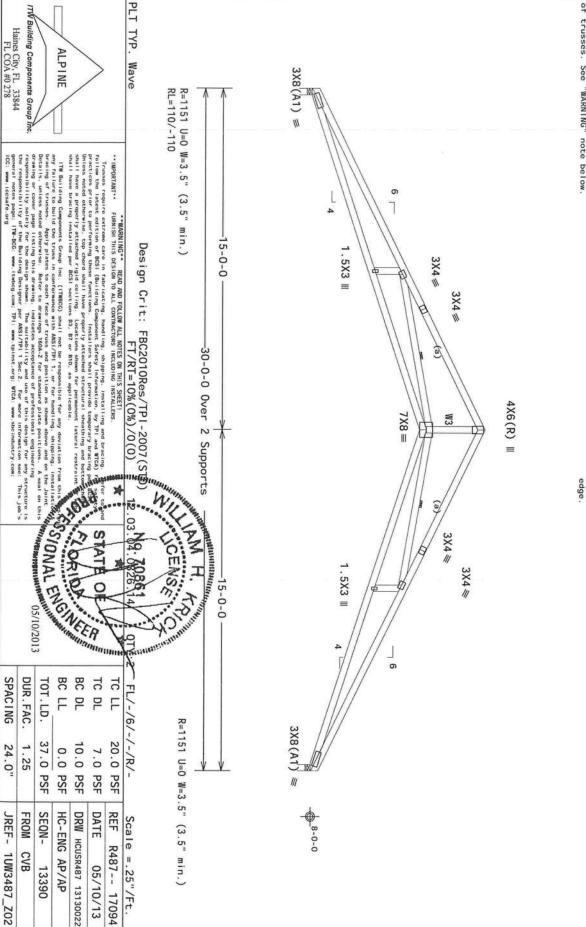
Wind loads and reactions based on MWFRS with additional C&C member

(a) Continuous lateral bracing equally spaced on member

Bottom chord checked for 10.00 psf non-concurrent live load

Calculated vertical deflection is 0.62° due to live load and 0.86° to dead load at X = 15-0-0. due

MWFRS loads based on trusses located at least 15.00 ft. from roof



Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Calculated horizontal deflection is 0.20" due to live load and 0.37" due to dead load.

(a) Continuous lateral bracing equally spaced on member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Negative reaction(s) of -493# MAX. (See below) from a non-wind load case requires uplift connection.

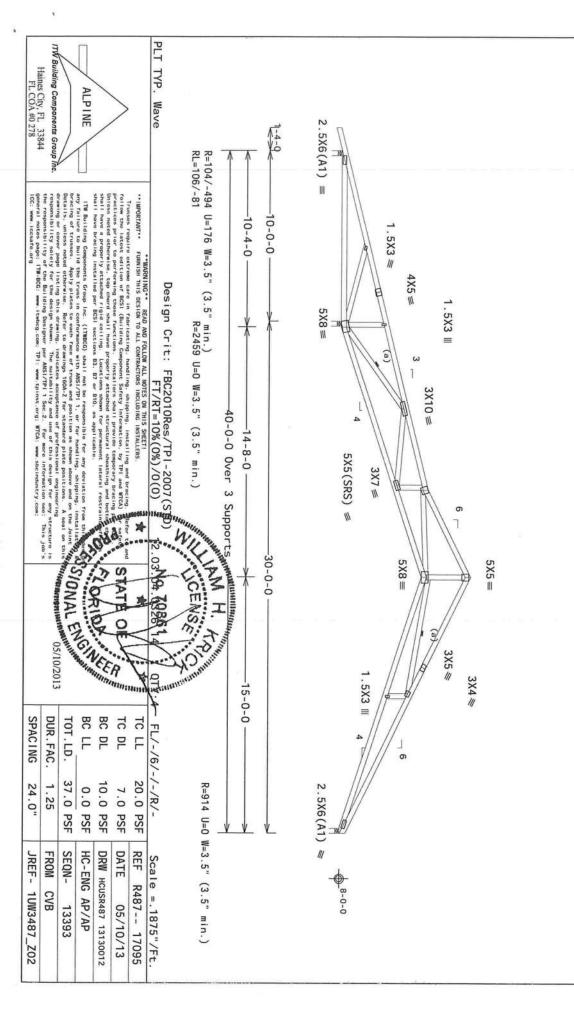
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Shim all supports to solid bearing.

MWFRS loads based on trusses located at least $15.00\ \text{ft.}$ from roof edge.



Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Calculated horizontal deflection is 0.20" due to live load and 0.37" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Negative reaction(s) of -520# MAX. (See below) from a non-wind load case requires uplift connection.

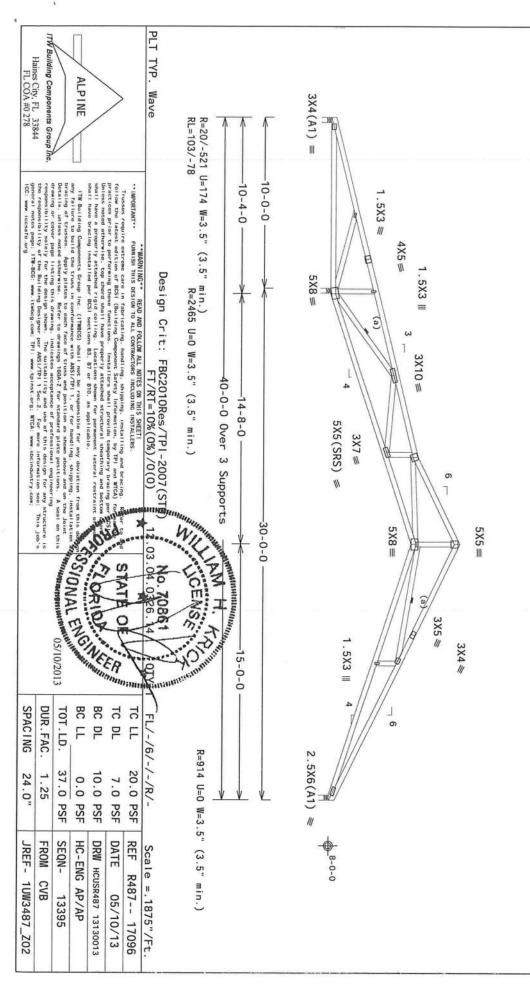
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load

Shim all supports to solid bearing.

MWFRS loads based on trusses located at least 15.00 ft, from roof



Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

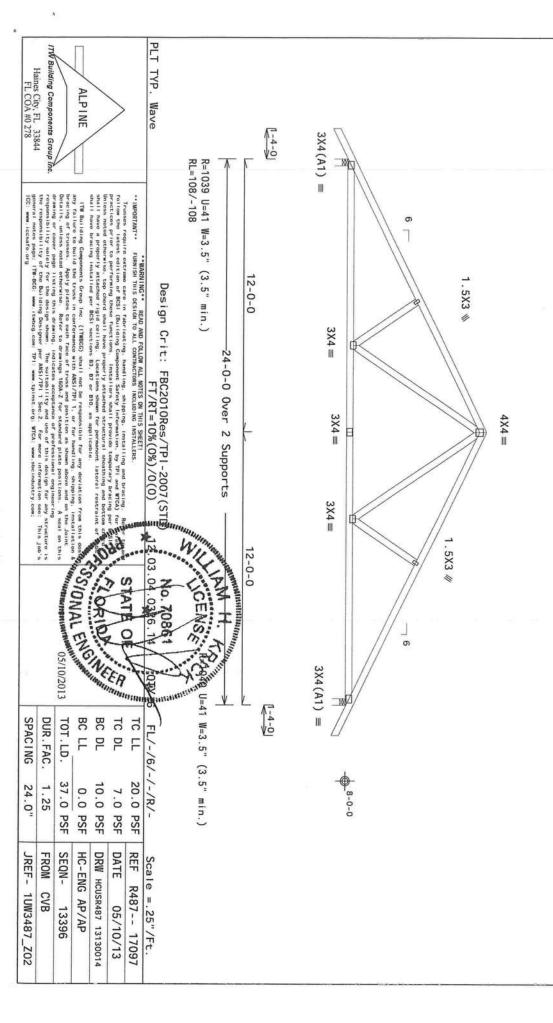
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

120 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, RISK CAT II, EXP B, DL=5.0 psf. GCpi(+/-)=0.18 7-10, CLOSED bldg, Located wind TC DL=3.5 psf, wind BC

Wind loads and reactions based on MWFRS with additional C&C member design.

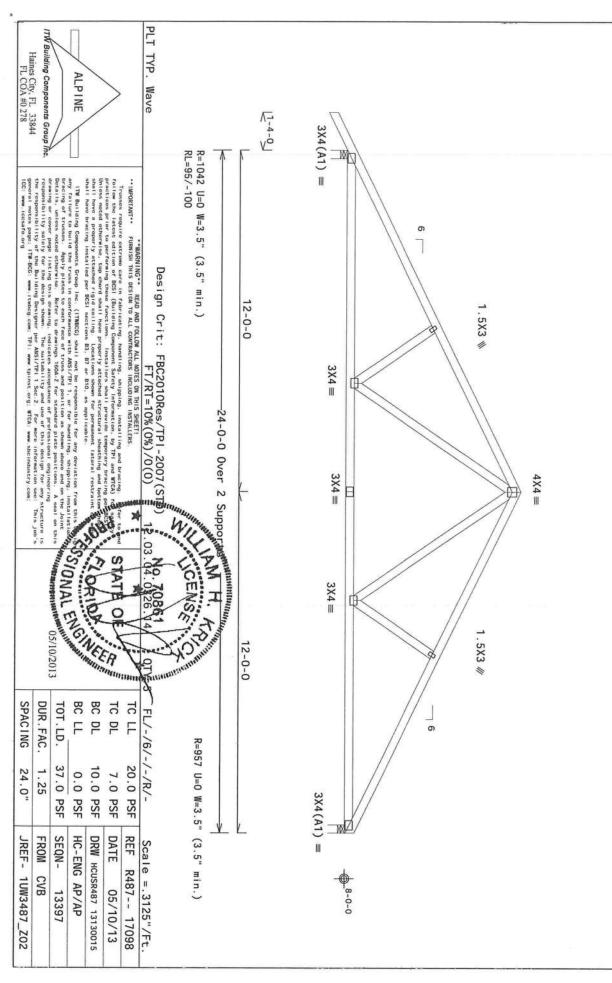
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets $L/240\,$ live and $L/180\,$ total load. Creep increase factor for dead load is 1.50.



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

MWFRS loads based on trusses located at least 15.00 ft. from roof edge. Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high \times 24"-wide clearance. Deflection meets L/240 live and L/180 total load. Greep increase factor for dead load is 1.50. Wind loads and reactions based on MWFRS with additional C&C member design. Bottom chord checked for 10.00 psf non-concurrent live load



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC. See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more

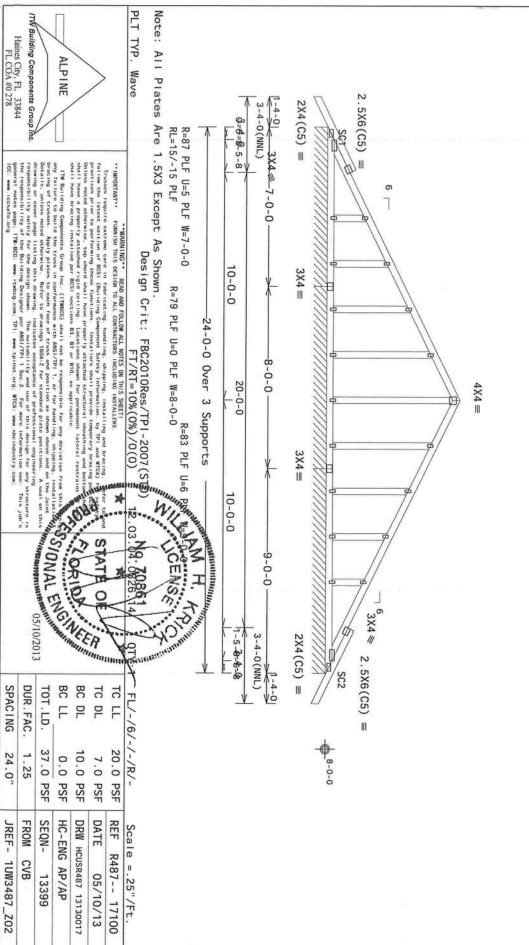
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Bottom chord checked for 10.00 psf non-concurrent live load

Wind loads and reactions based on MWFRS with additional C&C member

Gable end supports 8" max rake overhang.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6. Top chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Webs 2x4 SP_#3_12A :Stack Chord SC1 2x4 SP_#1_12A::Stack Chord SC2 2x4 SP_#1_12A: Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC. (12-276A--Little & Williams R=87 PLF U=4 PLF W=14-0-0 C1-4-0 2X4 (C5) = 3-4-0(NNL) 2.5X6(C5) =0-0-0 1-5-8 Smith Res -- Lake City, FL 3X4 // 14-0-0 Over Continuous Support 6 5-0-0 32055 - CGE 14' Gable) 10-0-0 5-0-0 6 design. 120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Bottom chord checked for 10.00 psf non-concurrent live load See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more Wind loads and reactions based on MWFRS with additional C&C member DA H. A. P. Y OENSON 3X4 ₩ William S County William 1-5-8 0-6-8 THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR 3-4-0(NNL) $2X4(C5) \equiv$ 2.5X6(C5) =K1-4-0

PLT TYP.

Wave

· IMPORTANT · ·

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Design Crit: FBC2010Res/TPI-2007(STE) FT/RT=10%(0%)/0(0)

10.7086 .0386.

TC LL

20.0 PSF

REF

Scale = .375"/Ft. R487-- 17103

FL/-/6/-/-/R/-

TC DL

7.0 PSF

DATE

05/10/13

BC

10.0

0.0

PSF PSF

HC-ENG AP/AP DRW HCUSR487 13130020

Building Components Group Haines City, FL 33844 FL COA #0 278

drawing or cover page listing this drawing, indicates responsibility solely for the design shown. The suit; the responsibility of the Building Designer per ANSI/T general notes page: ITW-BCG: www.ltwbcg.com; TPI: www.

r any structure is n see: This Job's y.com; A seal on this

I'll Building Components Group Inc. (I'll BEC) shall not be responsible for any deviation from this d any failure to avail the trust in conformance with ABS/TPI 1, or for handling, shipping, installant breaking of trustees. Apply places to each free of trust and position as shown above and on the Joint Details, unless motion exceeded the factor to drawing 1604-2 for standard place positions. A seal on drawing or cover page listing this drawing, telestas acceptance of professional regional ring desired.

Trossos require extreme care in febricating, handling, shipping, installing and bracing. Refer to an Trossos require extreme care in febrication, but the states, and NTCA) for a february practices prior to performing these functions. Installers shall provide temporary bracing per 65:10 biless noted otherwise, top chord shall have properly attached structural sheathing and bottom of the shall have a properly attached rigid colling. Locations shown for permanent tatoral restraint or sessions thave a properly attached rigid colling. Locations shown for permanent tatoral restraint or sessions and have a properly attached rigid colling.

OSIONAL ENGINEE

TOT.LD.

SEQN-

13402

FROM

CVB

DUR. FAC

SPACING

24.0" 1.25 37.0 PSF

JREF-

1UW3487_Z02

ALPINE

Note: All Plates Are 1.5X3 Except As Shown.

RL=5/-5 PLF

BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES

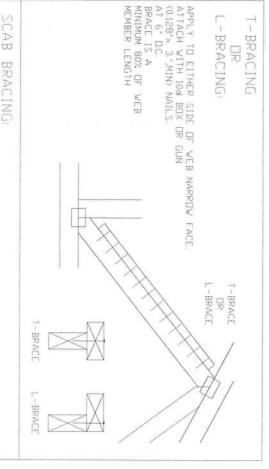
THIS DETAIL IS DNLY APPLICABLE FOR CHANGING THE SPECIFIED OR SCAB BRACING

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING

SADA 2 8X2	SMB4 2 5X2	2X3 DR 2X4 L RDW S	SIZE BRACING
5X8	2×4 2×6	2×5 6	T DR L-BRACE
5-5xe-3 8x2-1	1-2×6 2-2×4(*)	1-2×4 2-2×4	IVE BRACING

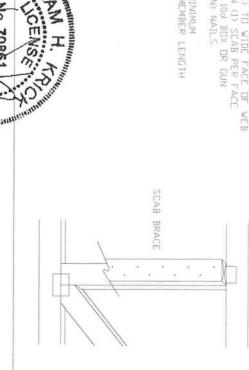
T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE DR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

CENTER SCAB ON WIDE FACE OF FACE OF WEB. WEB. APPLY (I) SCAB TO EACH



BRACING:

APPLY SCABKS) TO WIDE FACE OF NO MORE THAN (1) SCAB PER FACE ATTACH WITH 100 BOX OR GUN (0.128"x 3.",MIN) NAILS. AT 6" D.C. AT 6° D.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH WEB.





AVABRIDIA CEAN AND FOLLIDY ALL MITES DN 1HIS SEET!
Trusses require extreme core on fobricating broading, plagping, installing and broking. Refer
folials REST Building Component Servey, Information, by 19 and VITAN For as 16'ty, projectives
per forming these functions. Installing should provide temporary brokens as 80'tts. Challes of the make top properly attacked should be only the properly attacked should be only the properly attacked provided right ceiling. Locations shown for promoters that terrol installing the short should be only the properly attacked provided right ceiling. Locations shown for promoters that terrol installing the short should be s

IMPORTANT FURNISH CIPMY OF THIS USSIGN TO INSTALLATION CONTRACTION
IT BUILDING Components Group the LITVERIOR shall not be responsible for any deviation from this
design, any failure to build the truss in conformance with TPI, or fabrication, beinging, installing & broading of trusses. ITVERC connector plates are made of 2018/16GA (V,VVVXVA ASTN AGS)
groups 37/40/60 (X/VVXVA) galv. steel. Apply plates to each face of truss, positioned as shown above
and on Jaint Derivation.

on this drawing or cover page indicates acceptance and professional engineering responsibility for the trues component design about. The suitability and use of this component for any gis the responsibility of the Bulding Designer per AMS/TPF1 Sec 2.

Earth City, MO 63045

May 10 '13

SSIONAL ENGINEER BC PO SPACING DUR. FAC

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PSF PSF PSF PSF PSF

REF

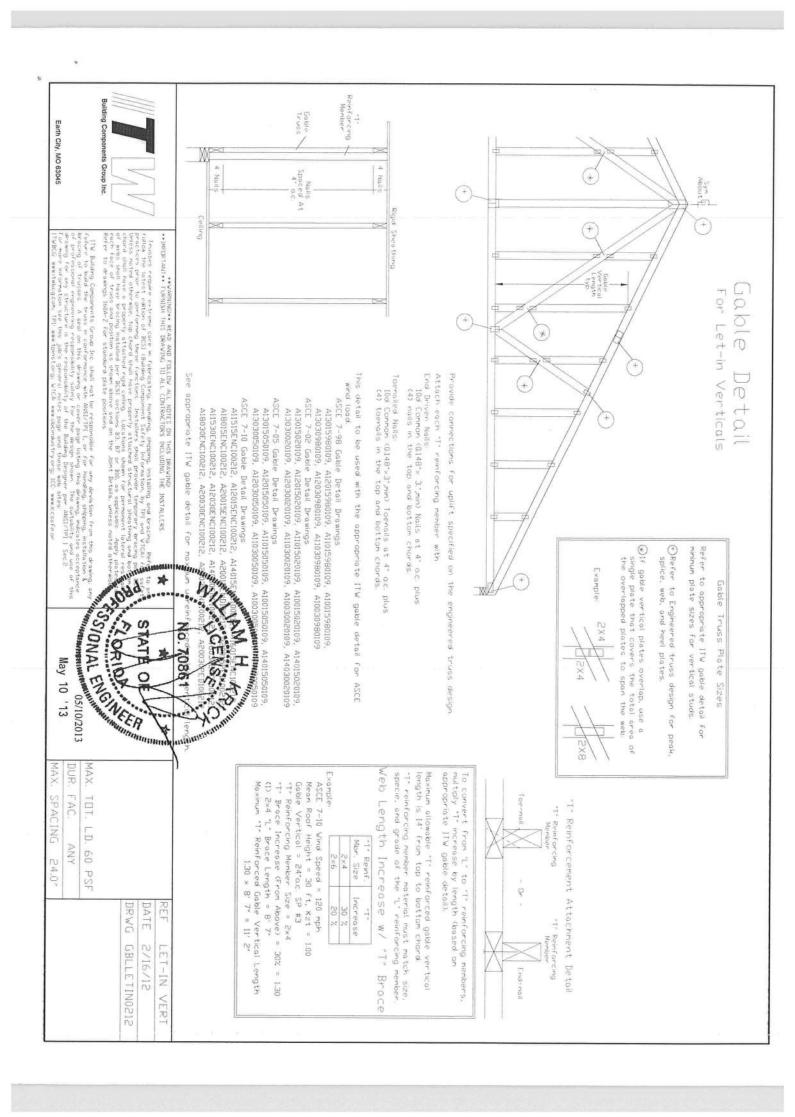
CLB 60/1/1

SUBST

DRWG DATE

BRCLBSUB0109

Disgonal brace options vertical length may be doubled when disgonal brace is used Connect disgonal brace for 33% at each read Max web total length is 14". Max Vertical Length Earth City, MO 63045 Connect diagonal at midpoint of vertical web pacing Species Gable Va 16" 12" 24" ents Group Inc SPF SPF H Vertical Grade Standard Standard Standard #1 / #2 tandard tandara Stud ## Trusses require extrem care in fabricating, kinding, shipping, installing and bracing. Re folios the latest existen of BCN (Building Component Safety Information, by IPI and VITA) practices prior to performing these fonctions, facultiers shall provide temporary because unless noted attended structured shall have properly attached structured shall not properly attached structured shall not be shall not properly attached structured shall not be shall not properly attached structured shall not properly attached structured shall not properly attached structured. Lacations, shall not properly attached structured shall not properly attached sh Brace ** IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS. ITV Studing Components Group Inc. shall not be responsible for any deviation from this drawing, any salary to build the thouse in conformance with AMSI/TPI I, or for Fronding, shipping, installation & except of trustees. A soil on this drawing or cover page listing this drawing indicates a complance if professional engineering responsibility soilety for the design shown. The statistity and use of this browing for any structure is the responsibility to the design shown. AMSI/TPI I sec or more information see this job's general nates page and these was sites. 7-10: Jable Truss Broces 4 4 400 1 N O better diagonal brace; single or double cut (as shown) at Group (1) 1x4 "L" Brace * (1) 2x4 "L" Brace * (2) 2x4 "L" Brace ** (1) 2x6 "L" Brace * (2) 2x6 "L" Brace 8×4 DF-L #2 or D. æ φ 300 က္ ယ္ Þ 77 Group Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 100 o, Gable w Group A 元元 元 元 元 元 元 元 4 日 三 三 三 0 0 0 Stud Group B 9' 10' 8' 6' 11' 6' 11' 4' 9 10 8 Reinforcement Group A 13 0 0 15 5 6 11 15 6 6 11 15 6 6 11 15 6 6 11 15 6 6 11 15 6 0000000000000000 Group B 딦딦딦딦 222222222 777970 0 SSIONAL ENGINEER MANAGEMENT Group A 14 4 0 0 4 4 Detail May 10 '13 Group B 4 4 4 4 4 4 4 4 4 4 4 13.5 14' 0" Group A Exposure Group B 14' 0' 14' 0' 14' 0' 14' 0' 14' 0' 4 4 4 4 0 TOT. SPACING Refer to the Building Designer for conditions not addressed by this detail. Goble and supports load from 4°0° outlookers with 2°0° averhang, or 12° plywood overhang "L" bracing must be a minimum of 80% of web member length. # For it? "L" brace Attach "L" braces with 10d (0.128'x 3.0" min) halls Wind Load deflection criterion is L/240 industrial 45 Stress-Rated Boards. Group volues may be used with these grades. ix4 Braces shall be SRB (Stress-Rated Board) Spruce-Pine-Fire
#11 / #2 Standard
#3 Stud continuous bearing (5 psf TC Bead Load). the ALSE January, 2012 rulin Douglas Fir-Larch #3 Stud Standard Bracing Group Species and Grades in 18° end zones and 6° ac between zones. For (1) "L" brace space nails at 2" a.c. in 18" end zones and 4" a.c. between zanes Pine lumber design values based on Vertical Length
Less than 4' 0'
Greater than 4' 0', but
less than 11' 6' Gable Truss Detail Notes: 50 大 N 大 Greather than II' 6" Refer to common truss design for peak, splice, and heel plates uglas fir-Larch 24.0" Gable Vertical Plate Sizes PSF () DRWG DATE #1 & Btr Group group A12015ENC100212 2/14/12 ASCE 7-10-GABI2015 Southern Pine*** w Þ Southern Pingxxx No Splice 1X4 or 2X3 Stondord 2.5X4 ∂X4 3 Standard Stud



Common Residential 7-10:Gable 120 mph, End 30, Wind Bracing Requirements Mean Height, Closed, Exposure Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or 100 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or 100 mph, 30ft Mean Hgt, ASCE 7-10, Part, Enclosed, Exp C, Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

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Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphrogm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

Nails: 10d box or gun (0.128"×3",min) nails

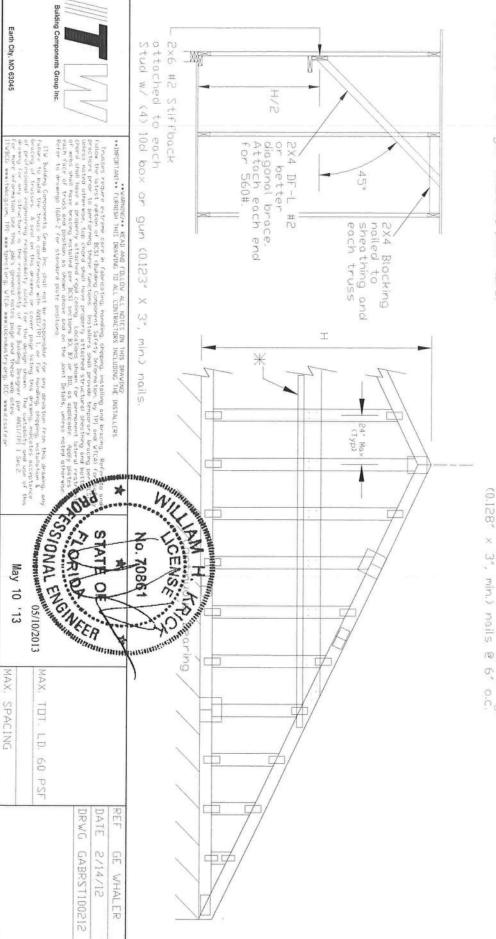
H Less than 4'6" - no stud bracing required

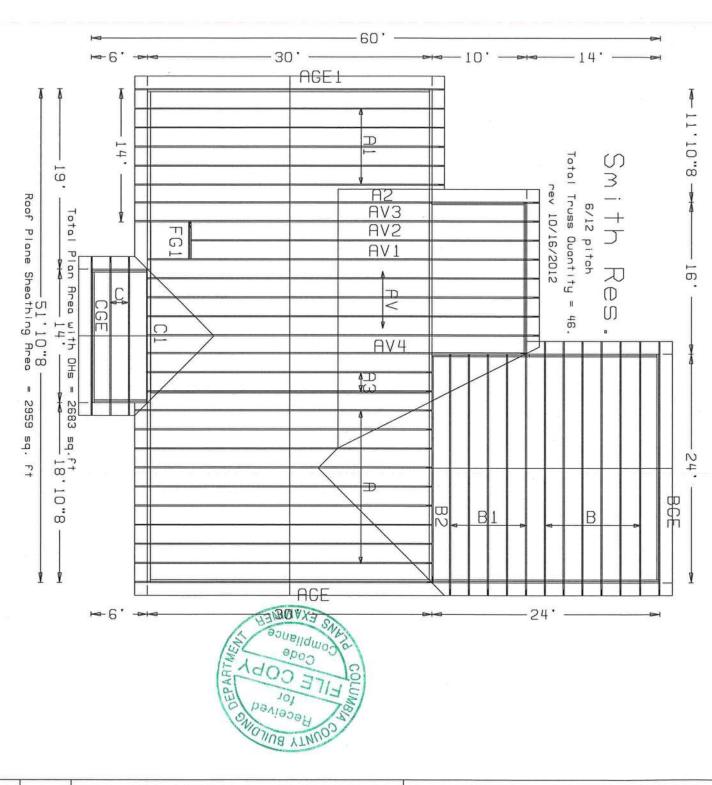
- Greater than 4'6" to 7'6" in length provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRVG A12030ENC10).
- Greater than 7'6" to 12'0" max:
 provide a 2x6 stiffback at mid-height and brace
 to roof diaphragm every 4'0" (see detail below or
 refer to DRWG A12030ENCIO).

 Optional 2x L-reinforcement attached

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* Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.





JOB NO: 12-276A PAGE NO: 1 OF 1 Location: JOB DESCRIPTION:: Little & Williams /: Smith Res.

