

WIND ANALYSIS -- 120MPH Wind Velocity or as interpolated

2023 8th edition Florida Building Code

Calculations as per Section 1609ASCE 7-22

Prepared By
James Zaleski PE 51544

Prepared by (print legibly): James Zaleski
Design Professional FL Lic. #: 51544

Importance factor: 1.0 Building Category: ENCLOSED
Wind Exposure (s): B Risk Category II
Internal Pressure Coefficient +/- .18

Mean Roof Height 13.67 Zone Length 6.0 feet

MAX OVERHANG 1.5 FT MAX

SEE ATTACHED

Roof Slope =7/12

THE:

PLEASANTVIEW A

FOR:

FOYE AND KELLY JACKSON

FL

COLUMBIA - FL



TRUSS SPAN/LOCATION HURRICANE CLIPS

HC MODEL-1 Simpson H-10A IN ALL AREAS 2 SDWC15600 SCREWS EQUAL 1-H10A

ROOF SHEATHING MATERIAL 7/16" OSB

NAILING -8D RING SHANK

NAILING PATTERN

EDGES-

6" O.C. FIELD - 6" O.C.

Plan May Be Mirrored at Contractors Option

James Zaleski P.E. #51544 2305 haverhill rd tall fl 32312 ph 850-766-7778

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DESIGN CRITERIA:

2023 FLORIDA BUILDING CODE - BUILDING, 8TH EDITION

2018 WFCM (WOOD FRAME CONSTRUCTION MANUAL)

ASCE 7-22: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.

BUILDING OCCUPANCY CATEGORY= II

POSTS WILL UTILIZE SIMPSON MODEL ABU FOR THE POST BASES AND AN AC OR LCE4 AS APPLICABLE OR BETTER, UNLESS OTHERWISE NOTED.

Wall Exterior Panel – Sheath with 7/16 OSB

– 2 X 4 STUDS AT 16” O.C. UP TO 10 FEET

– 2 X 4 STUDS AT 12” O.C. UP TO 12 FEET

ALL WALLS OVER 10 FEET TO HAVE 2 ROWS OF BLOCKING

SIMPSON SP1 AT BASE OF STUDS AND SIMPSON SP2 AT TOP OF STUDS AT 32” O.C (SPH STRAPS MAY ALSO BE USED)

MIN NAIL PENETRATION – 1-1/2”

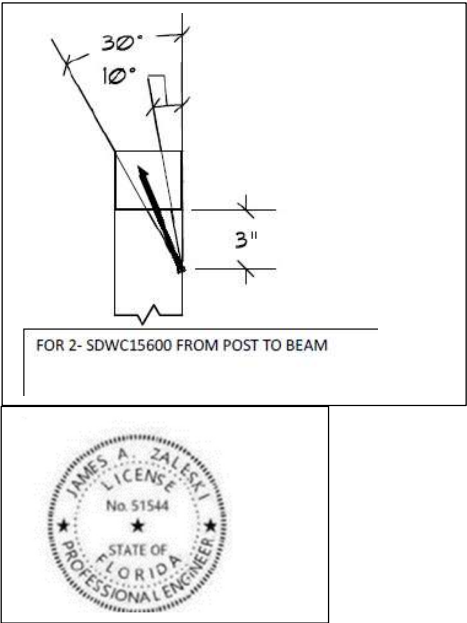
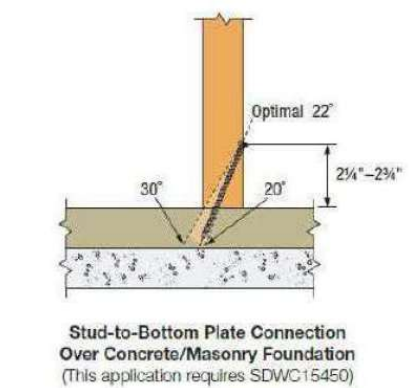
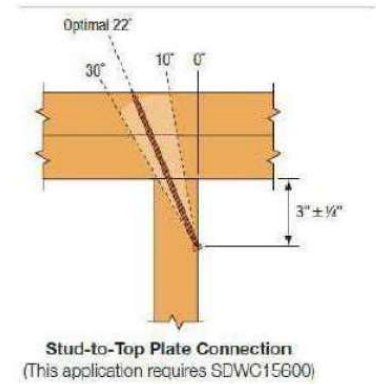
Nail Type8 D

Edge Nail Spacing 6” o.c

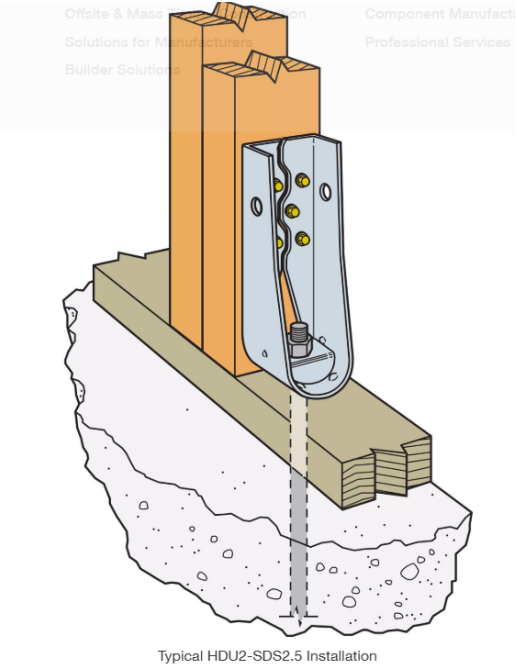
Intermediate Nail Spacing 6” o.c

BEAM TO WALL/CORNER CONNECTION – POCKET AND NAIL INTO WALL W/ (10) 16 PENNY NAILS, STRAP W/ SIMPSON H7Z.

10" 'ANCHOR' BOLT MINIMUM 32" O.C. w/ 3 GA. x 2" x 2" BEARING PLATE And 6" from each corner TO ATTACH SILL PLATE TO WALL



IN LIEU OF STRAPPING STUDS SDWC SCREWS MAY BE USED



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COMPONENTS AND CLADDING PRESSURES: (WORST CASE LOADS MAY BE USED)

COMPONENTS AND CLADDING

ZONE per

SEE ATTACHED

MAIN WIND FORCE RESISTING SYSTEMS (MWFRS) (WORST CASE LOADS MAY BE USED)

SEE ATTACHED

All Load Bearing and Shear Walls To be Framed as per FBC
Alternative Hurricane Clips are acceptable as long as they meet the requirements shown

See Attached header schedule

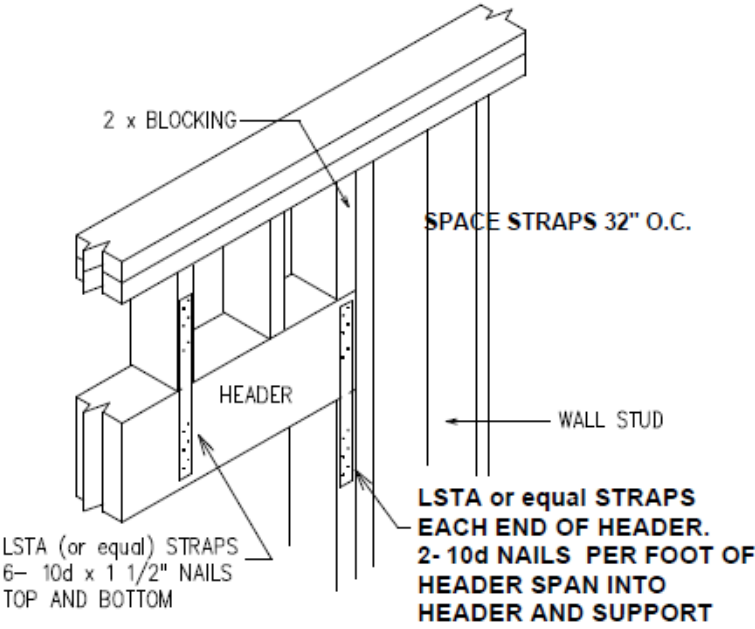
PROVIDE GABLE END BRACING DETAIL, all vaulted or high ceilings shall be balloon framed to the ceiling diaphragm.

NOTES: PLEASE READ & complete all blanks!!!!

- 1. See floor plan for wall bracing locations or circle 100% if structural sheathing is required on all exterior walls, with the nailing pattern indicated above.
- 2. There are , there are not x interior shear walls, locate interior shear walls on plan.
- 3. Gable ends required to be sheathed with same material as shear wall? Yes or No (circle one)
- 4. Wall sheathing used in lieu of vertical straps: Nailing @ N/A o.c. along top & bottom plates
- 5. Provide detail for 2 story bldgs showing continuous load path between 2nd floor stud & 1st floor studs.
- 6. Provide additional information for column base & column/beam connection if required for porches.
- 7. Provide calculations or documentation to substantiate method used as an attachment to this form(SEE PLANS)

Instructions:

- 1. The form should be completed & signed, sealed & dated by a Fla. licensed engineer or architect.
- 2. Since more than one methodology for determination of wind forces is permitted under Section 1609ASCE7-22, to comply with State Building Codes a space has been provided to indicate method used.
- 3. Wind Analysis Forms submitted & permitted to be used as Master Plans will be for identical plans only, minor deviations such as door swings. Any deviation from the exterior form, opening sizes or locations will not be permitted unless noted by the design professional.

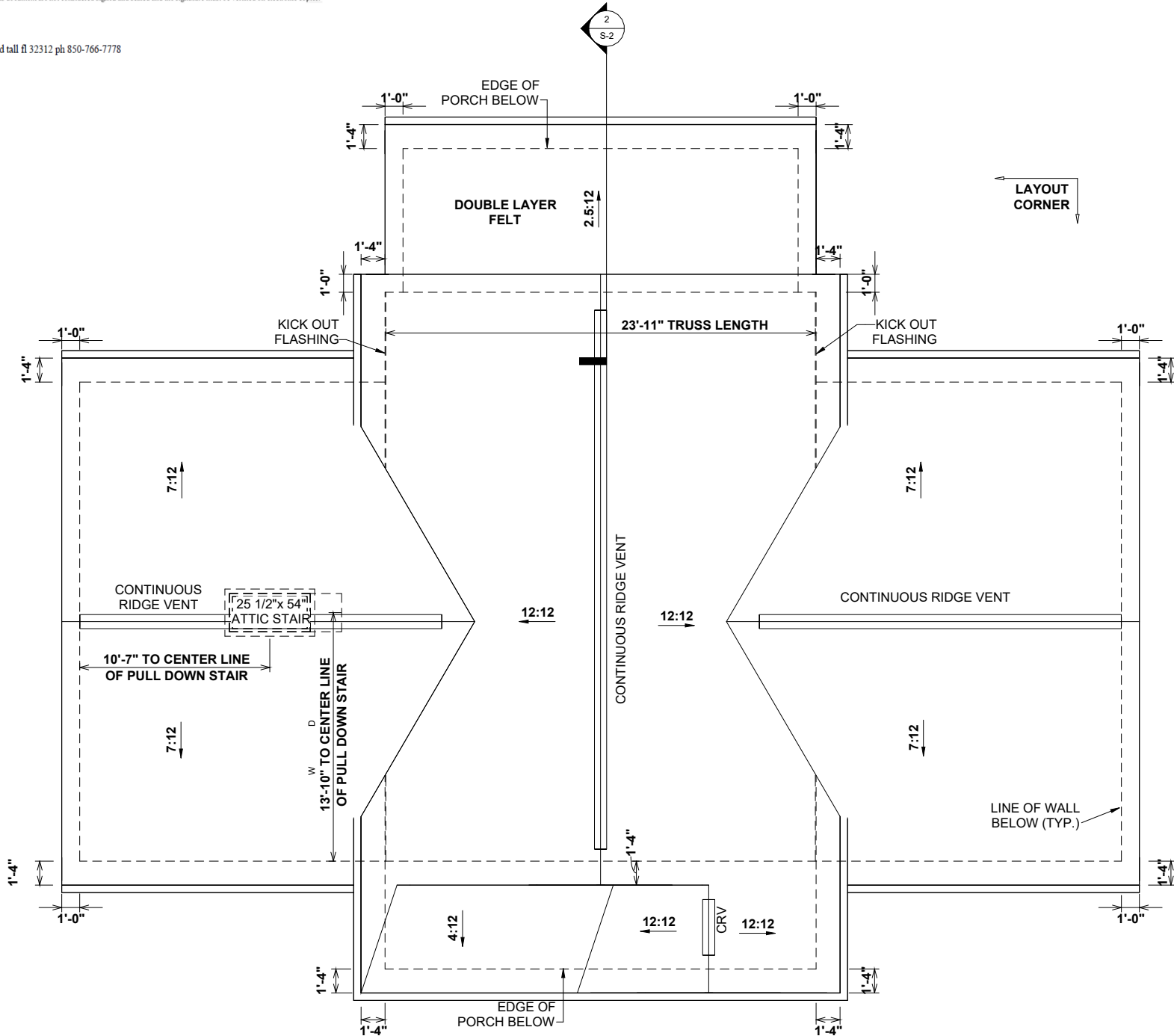


HEADER CONN.



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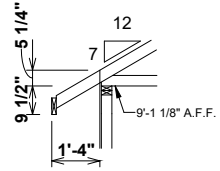
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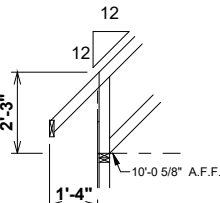
1 ROOF OVERVIEW
1/8" = 1'-0"

ROOF & FLOOR TRUSS NOTES

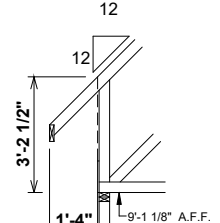
- ALL FLOOR AND ROOF TRUSS AND/OR ROOF FRAMING IS TO BE DETERMINED BY THE TRUSS MANUFACTURER AND TRUSS DRAWINGS SHALL BE SIGNED AND SEALED BY THE SAME. FRAMING LAYOUTS INCLUDED IN THE DRAWINGS HEREIN ARE TO BE CONSIDERED A PROPOSED SCHEMATIC REPRESENTATION ONLY AND FINAL MANUFACTURER DESIGN MAY VARY FROM THAT SHOWN.
- THE TRUSS MANUFACTURER'S CALCULATED SIZE AND SPACING OF PRE-ENGINEERED TRUSSES SHALL TAKE PRECEDENCE.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TRAY, CATHEDRAL, AND ANY OTHER DIMENSIONAL CEILING ASPECTS OF THE PROJECT THAT MAY NOT BE SHOWN ON THE PLANS PRIOR TO FABRICATION.
- THE CONTRACTOR SHALL UNDERSTAND ALL ASPECTS OF THE SPECIFIED TRUSS PACKAGE AND LAYOUT PRIOR TO COMMENCING INSTALLATION.
- DESIGN OF ROOF TRUSSES & WOOD FLOOR TRUSSES (IF APPLICABLE) SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, DESIGN LOAD DATA, AND SUPPORT REACTIONS SIGNED AND SEALED BY AN ENGINEER IN THE PROJECT STATE AND SUBMITTED TO THE BUILDING DEPARTMENT FOR PERMITTING. ANY REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS WITH REGARD TO TRUSS CONFIGURATION ONLY.
- ERECTION AND BRACING OF PREFABRICATED WOOD TRUSSES SHALL BE IN CONFORMANCE WITH THE TRUSS MANUFACTURER SPECIFICATIONS AND REQUIREMENTS AND THE TRUSS PLATE INSTITUTES "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS".
- SECURE EACH COMMON ROOF TRUSS/RAFTER TO THE TOP PLATE WITH A SIMPSON PRODUCT AS REQUIRED IN THE WIND ANALYSIS CALCULATIONS, OR OTHER NOTES CONTAINED THEREIN.
- USE SIMPSON H-7 OR BETTER AT GIRDER TRUSSES, OR AS OTHERWISE SPECIFIED BY THE TRUSS MANUFACTURER. PROVIDE A MINIMUM OF 2 STUDS (UNLESS OTHERWISE REQUIRED BY FBC) UNDER GIRDER TRUSS END BEARING.
- TRUSS TO TRUSS CONNECTIONS SHALL BE VERIFIED AND PROVIDED BY THE TRUSS DESIGNER/MANUFACTURER.
- CONTRACTOR TO PROVIDE ALL BLOCKING BETWEEN TRUSSES AND/OR RAFTER FRAMING.
- CONTRACTOR TO REFER TO FBC 2023 AND WFCM 2018 FOR FRAMING REQUIREMENTS OF WOOD FRAMED WALL SYSTEMS.
- TRUSS MANUFACTURER SHALL REFER TO ANY MEP DRAWINGS FOR OTHER ITEMS OR APPENDICES THAT MAY AFFECT THE TRUSS LOADING. ANY SUCH ITEMS SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGNER/ENGINEER OF RECORD.



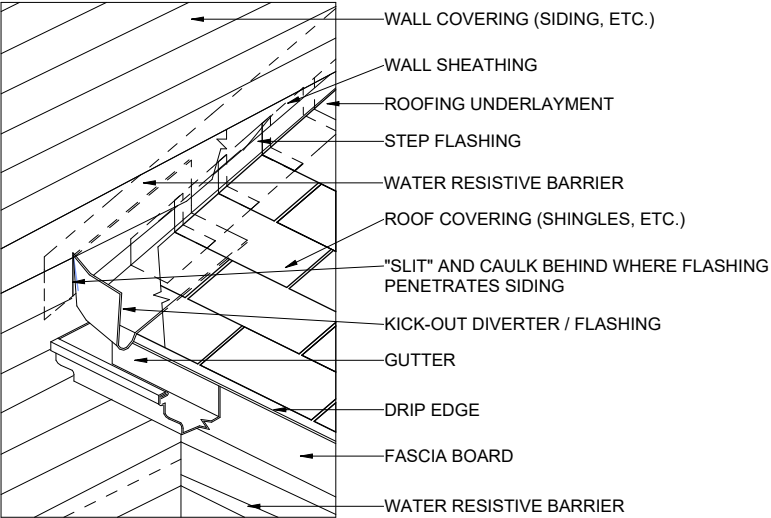
@ 7:12 ROOF



@ VAULTED AREA



@ DINING AREA

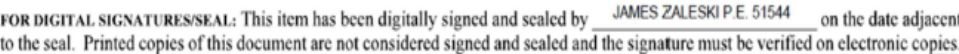


2 480.3 KICK OUT FLASHING DETAIL
6" = 1'-0"

2.5:12 O.W.H. = 3 3/4"
7:12 O.W.H. = 5 1/4"
12:12 O.W.H. = 3'-2 1/2"
12:12 O.W.H. = 2'-3" @VAULT

UNLESS OTHERWISE NOTED ON ROOF OVERVIEW

SHEET #		PROPERTY OF:	DRAWN BY:	JOB #	THE:	REVISION SCHEDULE					
E-3		 © COPYRIGHT - 2025	LUCAS N.	66-25-031	PLEASANTVIEW A	FIRST FLOOR HEATED	1,667 SF	REV #	DESCRIPTION	DATE	
			CHECKED BY:		2X6 EXTERIOR WALLS	FOR:	FRONT PORCH	144 SF			
			K.S.				SCREEN PORCH	176 SF			
			PRINTED:	12/3/2025 9:56:29 AM	FOUNDATION TYPE:		TOTAL UNDER ROOF	320 SF			
								1,987 SF			
SCALE: As indicated		ROOF OVERVIEW									
					OFFICE:	SOLD BY:					
					VALDOSTA SALES	B. DIMICK					



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A" = MAX. HORIZONTAL SPAN FOR OVERFRAME RAFTER*		
2X4	16" O.C.	9'-5"
	24" O.C.	8'-0"
2X6	16" O.C.	14'-4"
	24" O.C.	11'-9"
2X8	16" O.C.	18'-2"
	24" O.C.	14'-10"
2X10	16" O.C.	22'-3"
	24" O.C.	18'-2"

* 2018 IRC TABLE R802.4.1(1)

OPT. PAPER SIZE: for 1/4" = 1'-0" on 24" x 36" PAPER SIZE ENLARGE PRINTS TO 200%

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PAPER SIZE: 11" x 17"

SCALE: 1/4" = 1'-0"

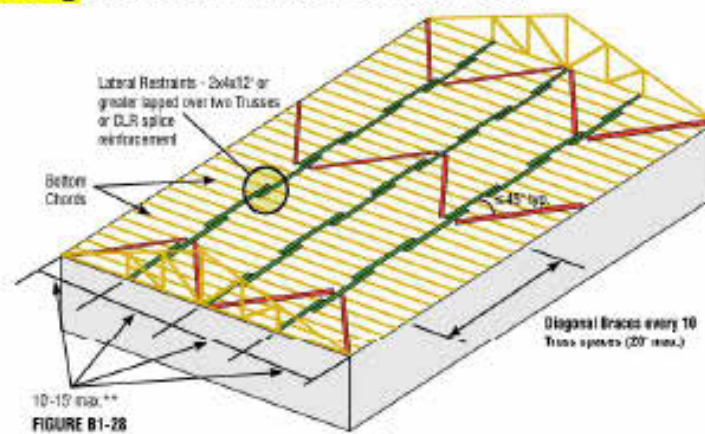
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				CHECKED BY:		2x6 EXTERIOR WALLS		FOR:		FRONT PORCH		144 SF	
				K.S.		FOUNDATION TYPE:		FOYE AND KELLY JACKSON		SCREEN PORCH		176 SF	
				PRINTED:		STEMWALL SLAB		FL		TOTAL UNDER ROOF		320 SF	
				12/3/2025 9:56:33 AM				COLUMBIA - FL				1,987 SF	
DETAILS & SECTIONS								OFFICE:					
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TRUSS BRACING SUMMARY

Five to Thrive

#1 CLR - Continuous Lateral Bracing on bottom chord 10' on center

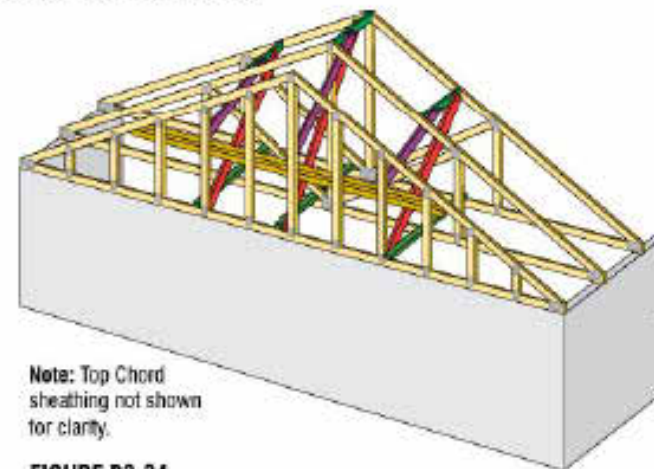
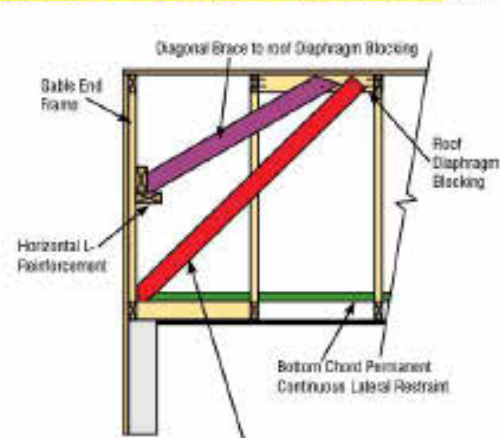
#2 DLR - Diagonal Lateral Bracing on bottom chord - No more than 20' in between



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#3 GEB - Gable End Bracing up to ridge or top chord

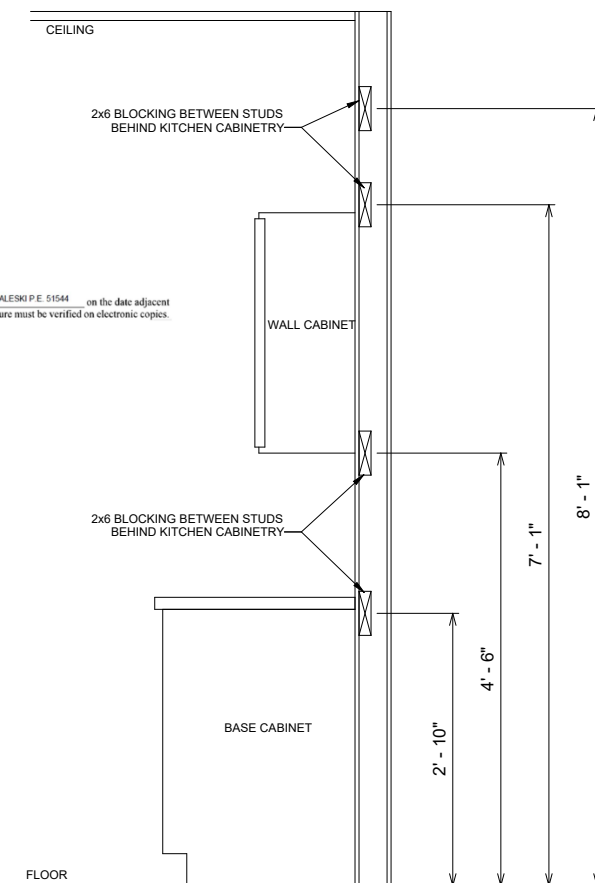
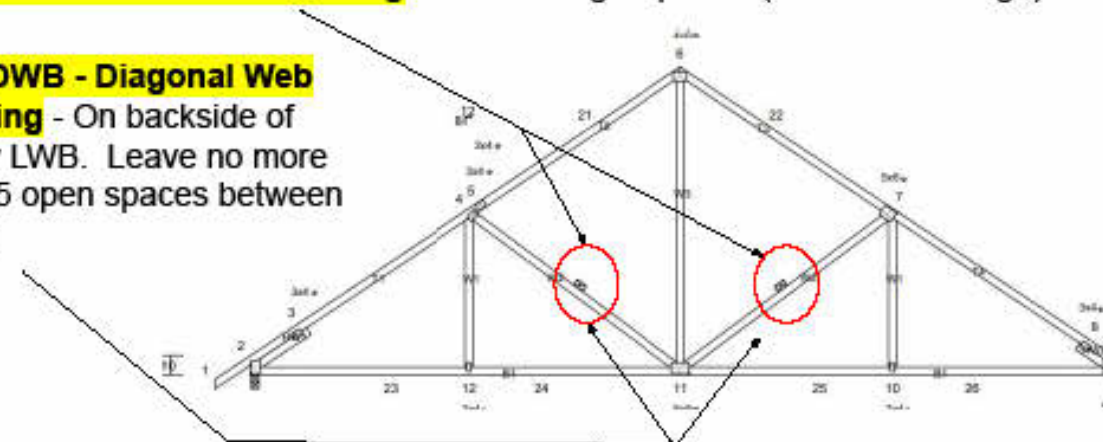


Note: Top Chord sheathing not shown for clarity.

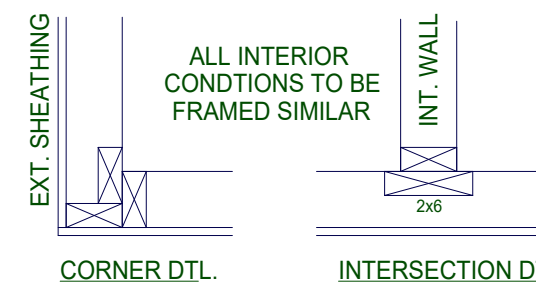
FIGURE B3-34

#4 - LWB - Lateral Web Bracing - Truss Design Specific (See Truss Design)

#5 - DWB - Diagonal Web Bracing - On backside of every LWB. Leave no more than 5 open spaces between DWB



1 KITCHEN CABINET WALL BLOCKING



TYPICAL FRAMING DETAILS

2 43.2 CORNER T FRAMING DETAIL

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TRUSS BRACING SUMMARY

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				CHECKED BY:		2x6 EXTERIOR WALLS		FOYE AND KELLY JACKSON		FRONT PORCH		144 SF									
				K.S.				FL COLUMBIA - FL		SCREEN PORCH		176 SF									
				PRINTED:		FOUNDATION TYPE:				TOTAL UNDER ROOF		320 SF									
				12/13/2025 9:56:33 AM		STEMWALL SLAB						1,987 SF									
TRUSS BRACING SUMMARY				OFFICE:				SOLD BY:													
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FOUNDATION PLAN

STEM WALL CHART

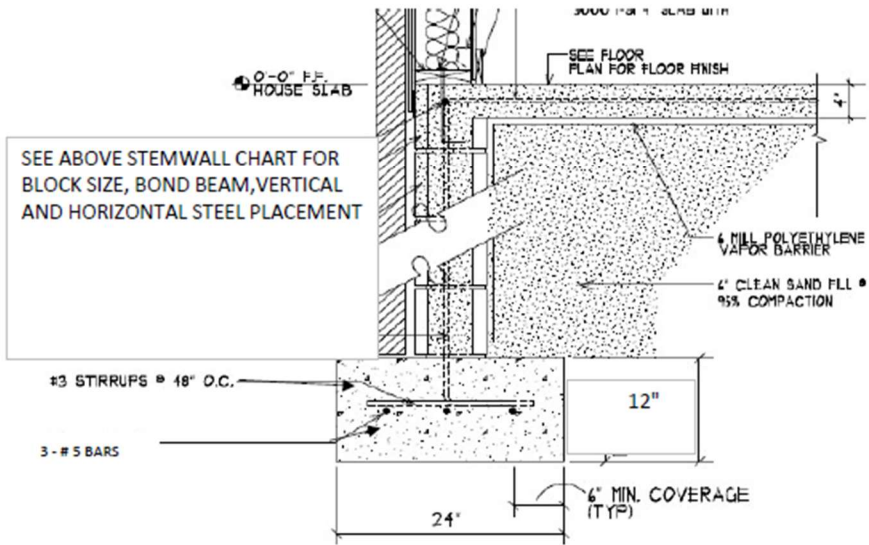
WALL HEIGHT	BOND BEAM	VERTICAL STEEL PLACEMENT	BLOCK SIZE	OTHER
0"-56"	NONE	# 5 Bar 36" o.c. AND All corners	12"	1 # 5 Bar Continous in Top Course IF WALL DOES NOT EXCEED 56 - "8" BLOCK MAY BE USED
56"-120"	BOND BEAM CONTINUOUS WITH 1- # 5 BAR AT 56" AND TOP COURSE	# 5 Bar 24" o.c.	12"	1 # 5 Bar Continous in Top Course

- 1 IN ALL CONFIGURATIONS, VERTICAL REBAR SHALL BE PLACED AT EITHER SIDE OF OPENINGS IN FOUNDATION WALL AND AT EACH CORNER OF THE FOUNDATION AND THE CELLS POURED WITH 3,000 PSI CONCRETE. ADDITIONALLY, ONE #5 REINFORCING BAR WITH THE CELL POURED FULL WITH 3,000 PSI CONCRETE SHALL BE PLACED UNDER ANY POINT LOADS THAT ARE LOCATED ON THE MASONRY WALL.
- 2 BACKFILL PLACEMENT: BACKFILL SHALL NOT BE PLACED AGAINST THE WALL UNTIL THE WALL HAS SUFFICIENT STRENGTH AND HAS BEEN ANCHORED TO THE FLOOR ABOVE, OR HAS BEEN SUFFICIENTLY BRACED TO PREVENT DAMAGE BY BACKFILLING
- 3 MORTAR: MORTAR SHALL BE TYPE M OR TYPE S; (28 DAY STRENGTH OF 2000 PSI). MASONRY SHALL BE LAID IN A RUNNING BOND.
- 4 THE MINIMUM COMPRESSIVE STRENGTH OF THE MASONRY SHALL BE F'M = 1500 PSI

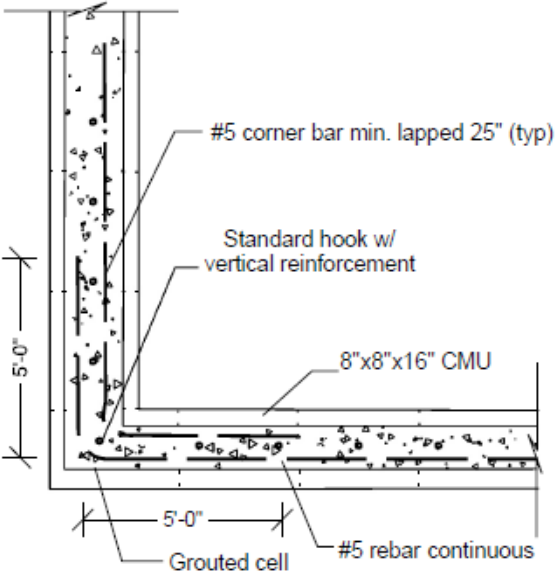
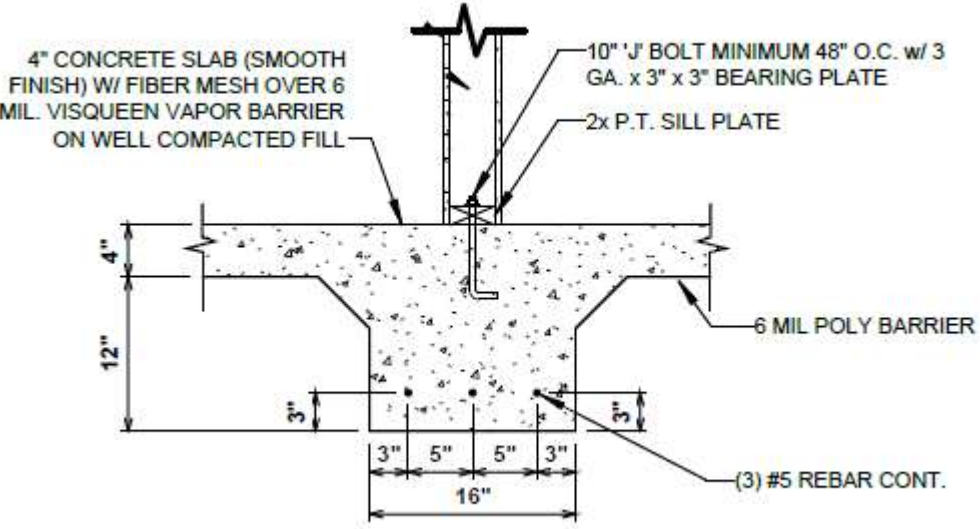
SATISFACTORY FILL MATERIAL SHALL BE FREE OF VEGETATION AND ORGANIC MATTER, WITH NOT MORE THAN 20 PERCENT BY WEIGHT PASSING THE 200 SIEVE. FILL LIFTS SHALL BE 12 INCHES MAXIMUM. ALL TOP SOIL CONTAINING UNSUITABLE MATERIAL SHALL BE REMOVED PRIOR TO THE PLACEMENT OF BACKFILL

EACH LAYER OF CLEAN FILL SHALL BE ADEQUATELY COMPACTED TO AT LEAST 95% IN THE TOP 3 FEET OF OPTIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR TEST.

SOIL BENEATH SLAB SHALL BE CHEMICALLY TREATED FOR TERMITES.



FOOTER SECTION USE MIN 3000 PSI CONCRETE
OPTIONAL STEMWALL SECTION



CORNER CONTINUITY OF BOND BEAM

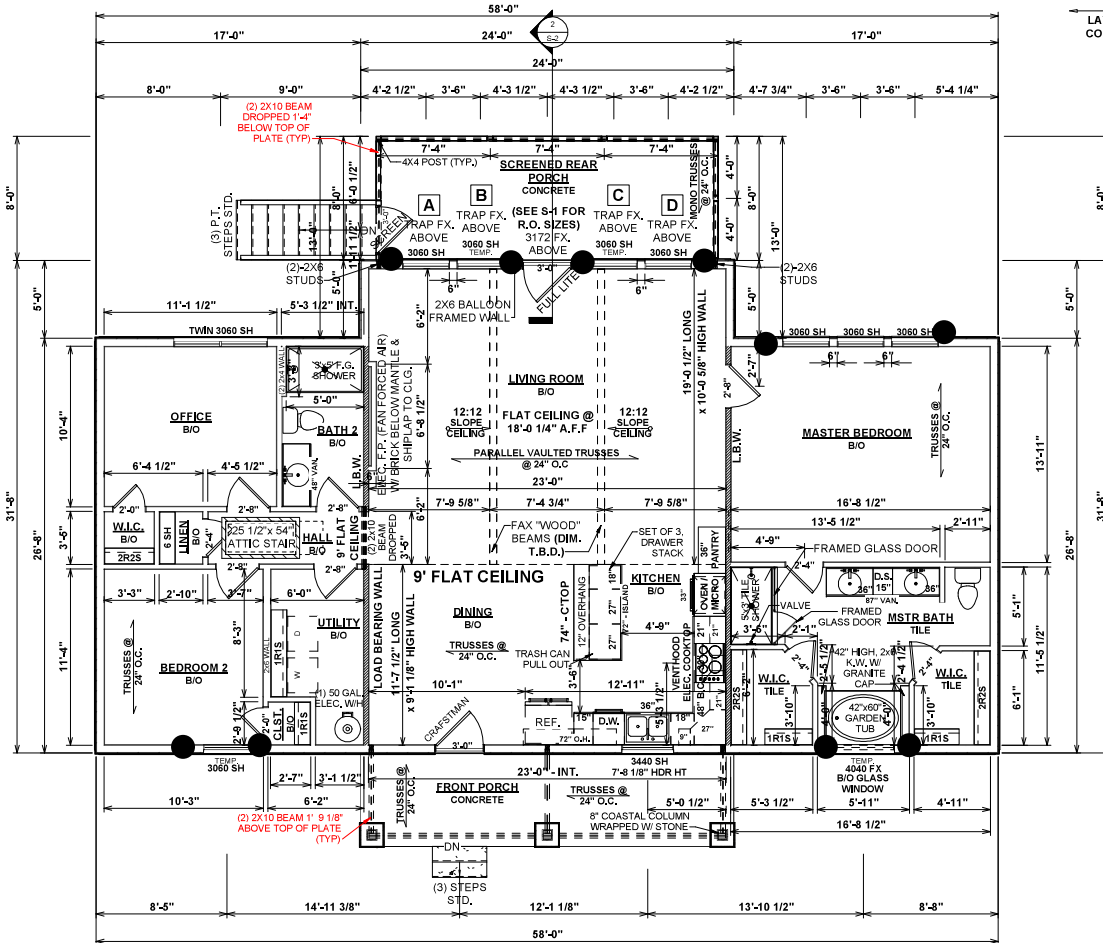
NOT TO SCALE

THE:	PLEASANTVIEW A
FOR:	FOYE AND KELLY JACKSON
	FL COLUMBIA - FL



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HDU2 HOLD DOWN



1 FIRST FLOOR PLAN
1/8" = 1'-0"



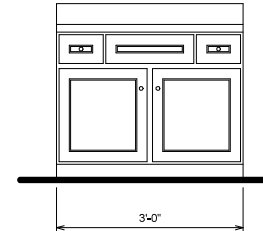
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GENERAL NOTES:

- MIN 9'-1 1/8" CEILING HEIGHT ON FIRST FLOOR
- ALL LOAD BEARING WALLS & EXT. OPENINGS TO HAVE (2) 2X10 HEADERS UNLESS OTHERWISE NOTED
- STANDARD FIRST FLOOR WINDOW HEADERS TO BE FRAMED DOWN 2'-2" FROM T.O.P, EXCEPT AS NOTED
- STANDARD SECOND FLOOR WINDOW HEADERS TO BE FRAMED DOWN 1'-2" FROM T.O.P, EXCEPT AS NOTED
- 7/16" O.S.B. AND HOUSEWRAP REQUIRED
- DIMENSIONS ARE TO SHEATHING EXTERIOR; SUBTRACT 1/2" FROM DIMENSIONS FOR EXTERIOR WINDOW AND DOOR FRAMING LOCATION IF OPENINGS ARE FRAMED BEFORE SHEATHING INSTALLATION
- ALL INTERIOR DOORS ARE EITHER CENTERED ON WALLS OR R.O. STARTED MIN OF 4" FROM ADJOINING WALL UNLESS OTHERWISE DIMENSIONED
- NUMBER OF STAIR TREADS & RISERS MAY VARY AS A RESULT OF LOCAL BUILDING CODES, STANDARDS AND FINAL GRADE
- CLOSET SHELF HEIGHT OFF FLOOR:
SINGLE-98"
DOUBLE 42" & 84"
- ALL PLUMBING FIXTURES SHOWN ARE A REPRESENTATION OF SIZE AND LOCATION ONLY. ACTUAL STYLE AND BRAND OF FIXTURES MAY VARY PER OFFICE LOCATION
- ALL TUBS/SHOWERS ARE TO HAVE NAILERS AT FLANGE
- INSTALL A 24" WIDE WALKWAY FROM ATTIC ACCESS TO FURNACE PLATFORM
- PORCH, STOOP, & DECK HANDRAILS NOT INCLUDED W/ SLAB FOUNDATION
- RAILINGS ARE A FORCED OPTION WHEN PORCH IS OVER 30" HIGH
- SCHLUTER DITRA UNCOUPLING AND WATERPROOFING MEMBRANE TO BE USED ON ALL TILE SHOWER INSTALLATIONS

SPECIAL NOTES:

- TILE BACK-SPLASH LEVEL 2 @ KITCHEN
- CERAMIC TILE LEVEL 2 ILO CARPET @ MSTR BATH
- T.B.D. FLUSH MOUNTS ON REAR PORCH
- B/O FLOORING THROUGHOUT EXCEPT MASTER BATH
- TINTED UPPER WINDOWS ON REAR OR HOME
- GENERATOR DISCONNECT/TRANSFER SWITCH T.B.D.
- ESTIMATE ONLY-RADIANT BARRIER OSB IN ROOF
- LEVEL 2 COUNTERTOPS THROUGH OUT HOME-BLACK PEARL AND OYSTER WHITE
- TILE SHOWER IN BATH 2



2 891.1 36" VANITY ELEVATION
1/2" = 1'-0"

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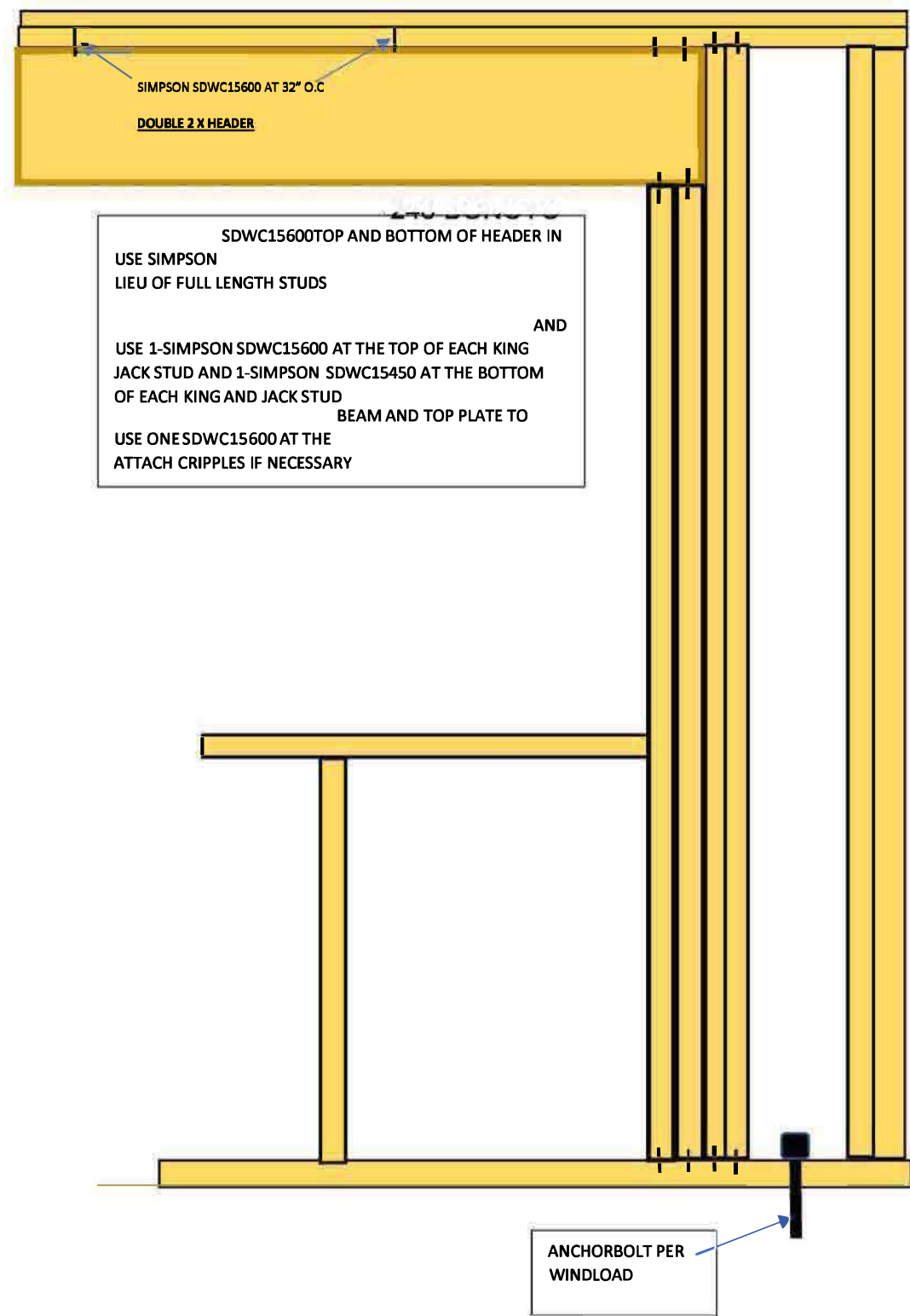
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F-2		LUCAS N. CHECKED BY: K.S.	66-25-031	PLEASANTVIEW A FOYE AND KELLY JACKSON FL COLUMBIA - FL	AREAS:		
					FIRST FLOOR HEATED	1,667 SF	
					FRONT PORCH	144 SF	
					SCREEN PORCH	176 SF	
					TOTAL UNDER ROOF	1,967 SF	
© COPY RIGHT - 2025			OFFICE:	SOLD BY:			
FIRST FLOOR PLAN			VALDOSTA SALES	B. DIMICK			



HEADER SIZE AND STRAPPING CHART

SPAN	HEADER SIZE	QUANTITY OF JACK STUDS AT EACH END	QUANTITY OF KING STUDS AT EACH END	STRAPPING TO JACK STUDS AT EACH END TOP AND BOTTOM	STRAPPING TO KING STUDS AT EACH END TOP AND BOTTOM
0'-0" TO 7'-6"	2 - 2X10" WITH ½" PLATE	1	1	1 SIMPSON MSTA24	1 SIMPSON SPH4
7'-6" – 11'-3"	2 - 2X12" WITH ½" PLATE OR 4-2 X 10" WITH ½" PLATE	3	2	2 SIMPSON MSTA24	2 SIMPSON SPH4
11'-3" – 14'-0"	2- 1 ¾" X 9 ¼" LVL	3	2	2 SIMPSON MSTA24	2 SIMPSON SPH4

IN LIEU OF STRAPPING USE A SDWC15600 AT THE TOP OF EACH JACK AND KING STUD AND ONE SDWC15450 AT THE BASE OF EACH JACK AND KING STUD

AT ALL CORNERS USE 3- SIMPSON SDWC1540 SCREWS AT BASE AND 3- SIMPSON SDWC15600 AT TOP – IF NECESSARY, USE 2-SCREWS IN ONE STUD AND ADD ONE MORE STUD MIDSPAN



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MecaWind v2545

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Calculations Prepared by:

Date: Dec 04, 2025

File Location: Current Project Not Saved



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General:

Reference Abbreviations: T: Table, F: Figure, E: Equation, S: Section, ¶: Paragraph

Wind Load Standard	=	ASCE 7-22	Basic Wind Speed	=	120.0 mph
Exposure Classification	=	B	Risk Category	=	II
Structure Type	=	Building	Basis for Wind Pressures	=	ASD
MWFRS Analysis Method	=	Ch 27	C&C Analysis Method	=	Ch 30 Pt 1
Dynamic Type of Structure	=	Rigid	Advanced Options	=	False

Building:

Roof = Roof Type	=	Gabled	Encl = Enclosure Classification	=	Enclosed
IsCust = Custom Roof	=	False	W = Building Width	=	32.0000 ft
L = Building Length	=	58.0000 ft	R _{ht} = Ridge Height	=	18.333 ft
E _{ht} = Eave Height	=	9.000 ft	Pitch = Pitch of Roof	=	6.4 :12
θ = Slope of Roof	=	28.07 °	OH = Overhang Configuration	=	All Soffit
Par = Parapet	=	None	z _i = Highest Opening Elevation	=	6.0000 ft
HT _{over} = Override Mean Roof Height	=	False	Ht _{man} = Mean Roof Height	=	13.667 ft
RA _{over} = Override Roof Area	=	False	GC _{pi_o} = Override GC _{pi} value	=	False
IsElev = Building is Elevated	=	False			

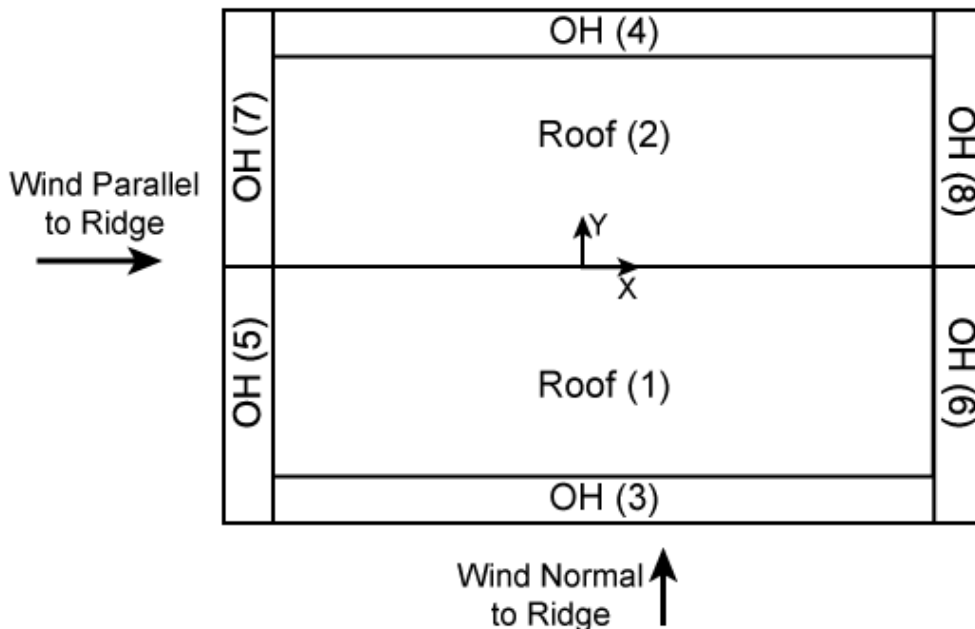
Exposure Constants T:26.11-1:

α = 3-s Gust-speed exponent	=	7.500	Z _g = Nominal Ht of Boundary Layer	=	3280.000 ft
â = Reciprocal of α	=	0.133	b = 3 sec gust speed factor	=	0.840
α _m = Mean hourly Wind-Speed Exponent	=	0.222	b _m = Mean hourly Windspeed Exponent	=	0.470
c = Turbulence Intensity Factor	=	0.300	ε = Integral Length Scale Exponent	=	0.3333

Overhang Inputs:

Std	=	Overhangs on all sides are the same	=	True
OHType	=	Type of Roof Wall Intersections	=	Soffit
OH	=	Overhang of Roof Beyond Wall	=	1.500 ft

Main Wind Force Resisting System (MWFRS) Wind Calculations per Ch 27



h = Mean structure height	=	13.667 ft	K _h = 2.41 • (15/Z _g) ^{2/α} _{T:26.10-1}	=	0.573
K _{zt} = No Topographic Feature	=	1.000	K _d = Directionality Factor _{T:26.6-1}	=	0.85
GC _{pi} = ± Internal Press Coef _{T:26.13-1}	=	±0.18	LF = ASD Load Factor	=	0.60
K _e = Ground Elev Factor _{T:26.10-1}	=	1.000	q _h = .00256 • K _h • K _{zt} • K _e • V ² • LF _{E:26.10-1}	=	12.67 psf
q _{in} = - Internal Press: q _h	=	12.67 psf	q _{ip} = For +GC _{pi} use q _h	=	12.67 psf

A_{Roof} = Roof Area = 2,419.67 ft²

MWFRS Wind Loads [Normal to Ridge]

h = Mean Roof Height of Building	= 13.6667 ft	R_{Ht} = Ridge Height Of Roof	= 18.3333 ft
B = Building Width Normal To Wind	= 58.0000 ft	L = Building Width Parallel To Wind	= 32.0000 ft
L/B = Ratio: L/B	= 0.552	h/L = Ratio: h/L	= 0.427
θ = Slope of Roof	= 28.07 °	$C_{p\text{W}}$ = Windward Wall Coefficient	= 0.800
$C_{p\text{LW}}$ = Leeward Wall Coefficient	= -0.500	$C_{p\text{SW}}$ = Side Wall Coefficient	= -0.700

Gust Factor Calculation for Wind: [Normal to Ridge]

Gust Factor Category I Rigid Structures - Simplified Method

G_1 = Simplified: For Rigid Structures can use 0.85 = 0.85

Gust Factor Category II Rigid Structures - Complete Analysis

Z_m = Equiv Struc Height: $\text{Max}(0.6 \cdot h, Z_{\text{min}})$	= 30.000 ft
I_{zm} = Turbulence Intensity: $c \cdot (33/Z_m)^{1/6}$ E:26.11-7	= 0.305
L_{zm} = Turbulence Integral Length Scale: $l \cdot (Z_m/33)^E$ E:26.11-9	= 309.993 ft
B = Building Width Width Normal to Wind Direction	= 58.000 ft
Q = $[1/(1+0.63 \cdot [(B+h)/L_{zm}]^{0.63})]^{0.5}$ E:26.11-8	= 0.894
G_2 = Detailed: $0.925 \cdot [(1+1.7 \cdot g_q \cdot I_{zm} \cdot Q)/(1+1.7 \cdot g_v \cdot I_{zm})]$ E:26.11-6	= 0.863

Gust Factor Used in Analysis

G = Gust Factor: $\text{Min}(G_1, G_2)$ = 0.850

Wall Wind Pressures [Normal to Ridge]

All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC_{pi}	q_i psf	K_z	K_{zt}	q_z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
9.000	+0.18	12.67	0.573	1.000	12.67	5.39	-6.52	-8.35	11.90	9.60
9.000	-0.18	12.67	0.573	1.000	12.67	9.26	-2.64	-4.47	11.90	9.60

K_z = $2.41 \cdot (15/Z_q)^{2/\alpha}$	K_{zt} = No Topographic Feature
GC_{pi} = +Internal Coef T:26.13-1	q_z = $.00256 \cdot K_z \cdot K_{zt} \cdot K_d \cdot V^2 \cdot LF$ E:26.10-1
q_{ip} = For + GC_{pi} use q_h	q_{in} = - Internal Press: q_h
Side = $q_h \cdot K_d \cdot G \cdot C_{p\text{SW}} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ E:27.3-1	Leeward = $q_h \cdot K_d \cdot G \cdot C_{p\text{LW}} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ E:27.3-1
Windward = $q_z \cdot K_d \cdot G \cdot C_{p\text{W}} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ E:27.3-1	Total = Windward - Leeward
+Press = Pressure Acting Toward Surface	-Press = Pressure Acting Away from Surface
§27.1.5	

Roof Wind Pressures [Normal to Ridge]

All wind pressures include a Load Factor (LF) of 0.6

Component	Description	Location	Start ft	End ft	θ °	Basis	GC_{pi}	$C_{p\text{Min}}$	$C_{p\text{Max}}$	P_{min} psf	P_{max} psf	P_{min} psf
Overhang	Leeward	7,8	All	All	28.07	N	0	-0.6	-0.6	-5.49	-5.49	4.80
Overhang	Windward	5,6	All	All	28.07	N	0	0.229	-0.227	2.10	-2.08	4.80
Overhang	Leeward	4	All	All	28.07	N	+0.1800	-0.6	-0.6	-7.43	-7.43	4.80
Overhang	Windward	3	All	All	28.07	N	+0.1800	0.229	-0.227	0.16	-4.02	4.80
Roof	Leeward	2	All	All	28.07	N	+0.1800	-0.6	-0.6	-7.43	-7.43	4.80
Roof	Windward	1	All	All	28.07	N	+0.1800	0.229	-0.227	0.16	-4.02	4.80
Soffit	Bottom	3	All	All	0.0	N/A	+0.1800	0.8	0.8	5.39	5.39	4.80
Overhang	Leeward	4	All	All	28.07	N	-0.1800	-0.6	-0.6	-3.55	-3.55	4.80
Overhang	Windward	3	All	All	28.07	N	-0.1800	0.229	-0.227	4.04	-0.14	4.80
Roof	Leeward	2	All	All	28.07	N	-0.1800	-0.6	-0.6	-3.55	-3.55	4.80
Roof	Windward	1	All	All	28.07	N	-0.1800	0.229	-0.227	4.04	-0.14	4.80
Soffit	Bottom	3	All	All	0.0	N/A	-0.1800	0.8	0.8	9.26	9.26	4.80

Roof Pressures based upon ch 27:

Component = The building component for pressures

Start = Start Dist from Windward Edge

$C_{p\text{Min}}$ = Smallest Coefficient Magnitude

P_{min} = $q_h \cdot K_d \cdot G \cdot C_{p\text{Min}} - q_{ip} \cdot K_d \cdot GC_{pi}$ E:27.3-1

GC_{pi} = +Internal Coef T:26.13-1

P_{min} = Min Press projected on vertical plane §27.1.5

§27.1.5 = MWFRS Min Wall Pressure = 9.60 psf

-Press = Pressure Acting Away from Surface

Location = Reference Graphic in Output for Values

End = End Dist from Windward Edge

$C_{p\text{Max}}$ = Largest Coefficient Magnitude

P_{max} = $q_h \cdot K_d \cdot G \cdot C_{p\text{Max}} - q_{in} \cdot K_d \cdot GC_{pi}$ E:27.3-1

Basis = P=Parallel to Ridge: N=Normal to Ridge

θ = Roof Slope Relative to Wind

+Press = Pressure Acting Toward Surface

• The smaller uplift pressures due to $C_{p\text{Min}}$ can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7



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MWFRS Wind Loads [Parallel to Ridge]

h = Mean Roof Height of Building	= 13.6667 ft	R _{ht} = Ridge Height Of Roof	= 18.3333 ft
B = Building Width Normal To Wind	= 32.0000 ft	L = Building Width Parallel To Wind	= 58.0000 ft
L/B = Ratio: L/B	= 1.813	h/L = Ratio: h/L	= 0.236
θ = Slope of Roof	= 28.07 °	C _{pww} = Windward Wall Coefficient	= 0.800
C _{pLW} = Leeward Wall Coefficient	= -0.338	C _{pSW} = Side Wall Coefficient	= -0.700

Gust Factor Calculation for Wind: [Parallel to Ridge]

Gust Factor Category I Rigid Structures - Simplified Method

G₁ = Simplified: For Rigid Structures can use 0.85 = 0.85

Gust Factor Category II Rigid Structures - Complete Analysis

Z _m	=	Equiv Struc Height: Max(0.6•h, Z _{min})	= 30.000 ft
I _{zm}	=	Turbulence Intensity: c•(33/Z _m) ^{1/6} E:26.11-7	= 0.305
L _{zm}	=	Turbulence Integral Length Scale: l•(Z _m /33) ^{E:26.11-9}	= 309.993 ft
B	=	Building Width Normal To Wind Direction	= 32.000 ft
Q	=	[1/(1+0.63•[(B+h)/L _{zm}] ^{0.63})] ^{0.5} E:26.11-8	= 0.917
G ₂	=	Detailed: 0.925•[(1+1.7•g _q •I _{zm} •Q)/(1+1.7•g _v •I _{zm})] E:26.11-6	= 0.876

Gust Factor Used in Analysis

G = Gust Factor: Min(G₁, G₂) = 0.850

Wall Wind Pressures [Parallel to Ridge]

All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC _{pi}	q _i psf	K _z	K _{zt}	q _z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
18.333	+0.18	12.67	0.604	1.000	13.37	5.79	-5.03	-8.35	10.82	9.60
13.667	+0.18	12.67	0.573	1.000	12.67	5.39	-5.03	-8.35	10.41	9.60
9.000	+0.18	12.67	0.573	1.000	12.67	5.39	-5.03	-8.35	10.41	9.60
18.333	-0.18	12.67	0.604	1.000	13.37	9.67	-1.15	-4.47	10.82	9.60
13.667	-0.18	12.67	0.573	1.000	12.67	9.26	-1.15	-4.47	10.41	9.60
9.000	-0.18	12.67	0.573	1.000	12.67	9.26	-1.15	-4.47	10.41	9.60

K_z = 2.41•(15/Z_g)^{2/a}
GC_{pi} = +Internal Coef T:26.13-1
q_{ip} = For +GC_{pi} use q_h
Side = q_h•K_d•G•C_{pSW}-q_{ip}•K_d•(GC_{pi}+) E:27.3-1
Windward = q_z•K_d•G•C_{pWW}-q_{ip}•K_d•(GC_{pi}+) E:27.3-1
+Press = Pressure Acting Toward Surface
§27.1.5 = MWFRS Min Wall Pressure = 9.60 psf

K_{zt} = No Topographic Feature
q_z = .00256•K_z•K_{zt}•K_e•V²•LF E:26.10-1
q_{in} = - Internal Press: q_h
Leeward = q_h•K_d•G•C_{pLW}-q_{ip}•K_d•(GC_{pi}+) E:27.3-1
Total = Windward - Leeward
-Press = Pressure Acting Away from Surface

Roof Wind Pressures [Parallel to Ridge]

All wind pressures include a Load Factor (LF) of 0.6

Component	Description	Location	Start ft	End ft	θ °	Basis	GC _{pi}	C _{pMin}	C _{pMax}	P _{min} psf	P _{max} psf	P _{min} psf
Overhang	Overhang 0 to h/2	5,7	0.000	1.500	0.0	P	0	-0.9	-0.18	-8.24	-1.65	4.80
Overhang	Overhang ≥ 2•h	6,8	59.500	61.000	0.0	P	0	-0.3	-0.18	-2.75	-1.65	4.80
Overhang Bottom	Bottom	5,7	All	All	0.0	N/A	0	0.8	0.8	7.32	7.32	4.80
Overhang	Overhang 0 to h	3,4	1.500	13.667	0.0	P	+0.1800	-0.9	-0.18	-10.18	-3.59	4.80
Overhang	Overhang h to 2•h	3,4	13.667	27.333	0.0	P	+0.1800	-0.5	-0.18	-6.52	-3.59	4.80
Overhang	Overhang ≥ 2•h	3,4	27.333	59.500	0.0	P	+0.1800	-0.3	-0.18	-4.69	-3.59	4.80
Roof	Roof 0 to h	1,2	1.500	13.667	0.0	P	+0.1800	-0.9	-0.18	-10.18	-3.59	4.80
Roof	Roof h to 2•h	1,2	13.667	27.333	0.0	P	+0.1800	-0.5	-0.18	-6.52	-3.59	4.80
Roof	Roof ≥ 2•h	1,2	27.333	59.500	0.0	P	+0.1800	-0.3	-0.18	-4.69	-3.59	4.80
Overhang	Overhang 0 to h	3,4	1.500	13.667	0.0	P	-0.1800	-0.9	-0.18	-6.30	0.29	4.80
Overhang	Overhang h to 2•h	3,4	13.667	27.333	0.0	P	-0.1800	-0.5	-0.18	-2.64	0.29	4.80
Overhang	Overhang ≥ 2•h	3,4	27.333	59.500	0.0	P	-0.1800	-0.3	-0.18	-0.81	0.29	4.80
Roof	Roof 0 to h	1,2	1.500	13.667	0.0	P	-0.1800	-0.9	-0.18	-6.30	0.29	4.80
Roof	Roof h to 2•h	1,2	13.667	27.333	0.0	P	-0.1800	-0.5	-0.18	-2.64	0.29	4.80
Roof	Roof ≥ 2•h	1,2	27.333	59.500	0.0	P	-0.1800	-0.3	-0.18	-0.81	0.29	4.80

Roof Pressures based upon ch 27:

Component = The building component for pressures
Start = Start Dist from Windward Edge
C_{pMin} = Smallest Coefficient Magnitude
P_{min} = q_h•K_d•G•C_{pMin}-q_{ip}•K_d•GC_{pi}E:27.3-1
GC_{pi} = +Internal Coef T:26.13-1
P_{min} = Min Press projected on vertical plane §27.1.5

Location = Reference Graphic in Output for Values
End = End Dist from Windward Edge
C_{pMax} = Largest Coefficient Magnitude
P_{max} = q_h•K_d•G•C_{pMax}-q_{in}•K_d•GC_{pi}E:27.3-1
Basis = P=Parallel to Ridge: N=Normal to Ridge
θ = Roof Slope Relative to Wind

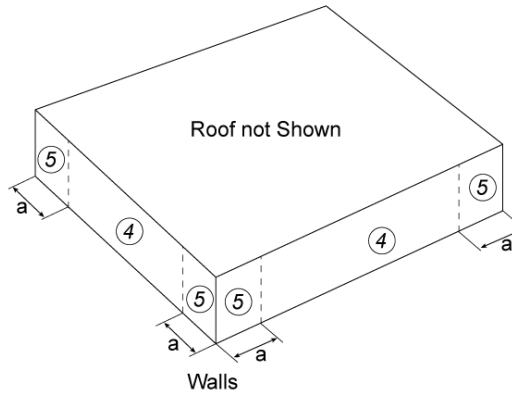
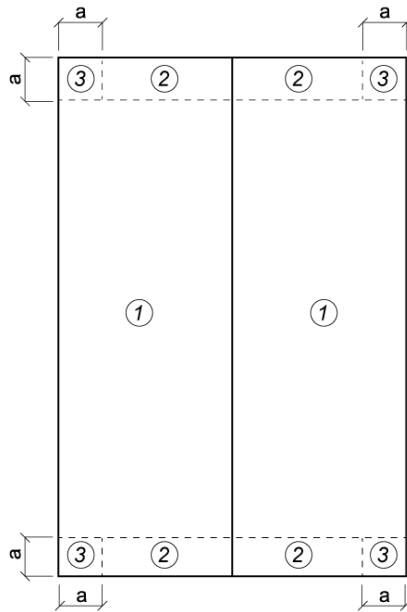


§27.1.5 = MWFRS Min Wall Pressure = 9.60 psf
-Press = Pressure Acting Away from Surface

+Press = Pressure Acting Toward Surface

• The smaller uplift pressures due to C_{pmin} can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7

Components and Cladding (C&C) Wind Loads per Ch 30 Pt 1 Roof & Wall



h = Mean structure height = 13.667 ft
 K_{zt} = No Topographic Feature = 1.000
 GC_{pi} = $\pm$ Internal Press Coef $T:26.13-1$ = ± 0.18
 K_e = Ground Elev Factor $T:26.10-1$ = 1.000
 θ = Slope of Roof = 28.07 °
 a = Max(a_1 , 0.04•B, 3 ft [0.9 m]) = 3.200 ft

K_h = $2.41 \cdot (15/Z_g)^{2/\alpha}$ = 0.573
 K_d = Directionality Factor $T:26.6-1$ = 0.85
LF = ASD Load Factor = 0.60
 q_h = $.00256 \cdot K_h \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF$ $E:26.10-1$ = 12.67 psf
 a_1 = Min($0.1 \cdot B$, $0.4 \cdot h$) = 3.200 ft

C&C Wind Roof & Wall Detailed per Ch 30 Pt 1 All wind pressures include a Load Factor (LF) of 0.6

Description	Zone	Width ft	Span ft	Area ft ²	1/3 Rule	Reference	a ft	GC_{pi}	GC_{pd}	GC_{pu}	P_{max} psf	P_{min} psf
Zone 1	1	1.0000	1.0000	1.00	No	F:30.3-2D	3.2	± 0.18	0.90	-1.80	11.63	-21.33
Zone 2	2	1.0000	1.0000	1.00	No	F:30.3-2D	3.2	± 0.18	0.90	-2.00	11.63	-23.48
Zone 3	3	1.0000	1.0000	1.00	No	F:30.3-2D	3.2	± 0.18	0.90	-2.50	11.63	-28.87
Zone 4	4	1.0000	1.0000	1.00	No	F:30.3-1	3.2	± 0.18	1.00	-1.10	12.71	-13.79
Zone 5	5	1.0000	1.0000	1.00	No	F:30.3-1	3.2	± 0.18	1.00	-1.40	12.71	-17.02

GC_{pd} = Down (+) External Coefficient
 P_{max} = $q_h \cdot K_d \cdot [GC_{pd} - GC_{pi}]$ $E:30.3-1$
+Press = Pressure Acting Toward Surface
§30.2.2 = C&C Min Pressure = 9.60 psf
Width = Width of Component
Area = Span • Width
 a = Max[Min($.1 \cdot B$, $.4 \cdot h$), $.04 \cdot B$, 3] $F:30.3-2D$
 GC_{pi} = Internal Coef $T:26.13-1$

GC_{pu} = Uplift (-) External Coefficient
 P_{min} = $q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ $E:30.3-1$
-Press = Pressure Acting Away from Surface
Zone = Applicable Zone per Figure
Span = Span of Component
1/3 Rule = Width limited to Span/3
 a = Max[Min($.1 \cdot B$, $.4 \cdot h$), $.04 \cdot B$, 3] $F:30.3-1$
Reference = Applicable Reference from Standard

C&C Wind Roof & Wall Overhangs Detailed per Ch 30 Pt 4 All wind pressures include a Load Factor (LF) of 0.6

Description	Zone	Width ft	Span ft	Area ft ²	1/3 Rule	Reference	a ft	GC_{pi}	GC_{pd}	GC_{pu}	P_{max} psf	P_{min} psf
Zone 1_OHS	1_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	3.2	± 0.18	0.00	-2.80	9.60	-32.10
Zone 2_OHS	2_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	3.2	± 0.18	0.00	-3.00	9.60	-34.25
Zone 3_OHS	3_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	3.2	± 0.18	0.00	-3.50	9.60	-39.64

GC_{pd} = Down (+) External Coefficient
 P_{max} = $q_h \cdot K_d \cdot [GC_{pd} - GC_{pi}]$ $E:30.7-1$
+Press = Pressure Acting Toward Surface
§30.2.2 = C&C Min Pressure = 9.60 psf
Width = Width of Component

GC_{pu} = Uplift (-) External Coefficient
 P_{min} = $q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ $E:30.7-1$
-Press = Pressure Acting Away from Surface
Zone = Applicable Zone per Figure
Span = Span of Component

Area = Span • Width
 a = Max[Min(.1•B,.4•h),.04•B,3]_{F:30.3-2D}
 Reference = Applicable Reference from Standard
 Soffit = Soffit present so use building GC_{pi}

1/3 Rule = Width limited to Span/3
 GC_{pi} = Internal Coef _{T:26.13-1}
 #_OHS = Roof Zone # on Overhang Soffit

C&C Wind Roof & Wall Summary per Ch 30 Pt 1 (Table 1 of 2)
 All wind pressures include a Load Factor (LF) of 0.6

Zone	Reference	P_{max} A ≤ 10 ft ² psf	P_{min} A ≤ 10 ft ² psf	P_{max} A = 20 ft ² psf	P_{min} A = 20 ft ² psf	P_{max} A = 50 ft ² psf	P_{min} A = 50 ft ² psf
1	F:30.3-2D	11.63	-21.33	10.64	-18.08	9.60	-13.80
2	F:30.3-2D	11.63	-23.48	10.64	-20.99	9.60	-17.69
3	F:30.3-2D	11.63	-28.87	10.64	-25.13	9.60	-20.19
4	F:30.3-1	12.71	-13.79	12.14	-13.21	11.38	-12.46
5	F:30.3-1	12.71	-17.02	12.14	-15.87	11.38	-14.36
1_OHS	F:30.3-2D/F:30.3-1	9.60	-32.10	9.60	-28.28	9.60	-23.24
2_OHS	F:30.3-2D/F:30.3-1	9.60	-34.25	9.60	-31.19	9.60	-27.14
3_OHS	F:30.3-2D/F:30.3-1	9.60	-39.64	9.60	-35.33	9.60	-29.63

C&C Wind Roof & Wall Summary per Ch 30 Pt 1 (Table 2 of 2)
 All wind pressures include a Load Factor (LF) of 0.6

Zone	Reference	P_{max} A = 100 ft ² psf	P_{min} A = 100 ft ² psf	P_{max} A = 200 ft ² psf	P_{min} A = 200 ft ² psf	P_{max} A > 500 ft ² psf	P_{min} A > 500 ft ² psf
1	F:30.3-2D	9.60	-10.56	9.60	-10.56	9.60	-10.56
2	F:30.3-2D	9.60	-15.20	9.60	-12.71	9.60	-12.71
3	F:30.3-2D	9.60	-16.45	9.60	-12.71	9.60	-12.71
4	F:30.3-1	10.81	-11.88	10.24	-11.31	9.60	-10.56
5	F:30.3-1	10.81	-13.21	10.24	-12.07	9.60	-10.56
1_OHS	F:30.3-2D/F:30.3-1	9.60	-19.42	9.60	-18.85	9.60	-18.09
2_OHS	F:30.3-2D/F:30.3-1	9.60	-24.07	9.60	-21.01	9.60	-20.25
3_OHS	F:30.3-2D/F:30.3-1	9.60	-25.32	9.60	-21.01	9.60	-20.25

P_{max} = Maximum Pressure
 Area = Span • Width
 Span = Span of Component

_{§30.2.2} = C&C Min Pressure = 9.60 psf

P_{min} = Minimum Pressure
 Width = Width of Component
 Reference = Applicable Reference from Standard
 Interpolate = Interpolate for Areas between columns



FOR DIGITAL SIGNATURES/SEAL: This item has been digitally signed and sealed by JAMES ZALESKI P.E. 51544 on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on electronic copies.

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