

DATE 04/02/2009

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT

000027724

APPLICANT RONALD L. JUSTICE PHONE 561.793.4463
ADDRESS 17560 66TH CT LOXAHATCHEE FL 33470
OWNER RONALD & CONNIE JUSTICE PHONE 561.793.4463
ADDRESS 529 SW MADISON CT LAKE CITY FL 32024
CONTRACTOR RONALD JUSTICE PHONE 561.793.4463
LOCATION OF PROPERTY 90-W TO PINEMOUNT ROAD, TL TO 4 MILES TO MADISON CT, TL GO
TO THE VERY END OF CUL-DE-SAC.
TYPE DEVELOPMENT METAL STORAGE BLDG. ESTIMATED COST OF CONSTRUCTION 30000.00
HEATED FLOOR AREA TOTAL AREA HEIGHT 16.00 STORIES 1
FOUNDATION CONC WALLS METAL ROOF PITCH 1'12 FLOOR CONC
LAND USE & ZONING A-3 MAX. HEIGHT 35
Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 07-4S-16-02791-108 SUBDIVISION WEST WIND ESTATES
LOT 8 BLOCK PHASE UNIT TOTAL ACRES 5.00

000001721

Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
18"X32'MITERED 09-0140 BLK RTJ N
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: ACCESSORY STRUCTURE. IMPACT FEE EXEMPT. NOC ON FILE.

Check # or Cash 8429

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by
Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by
Framing date/app. by Insulation date/app. by
Rough-in plumbing above slab and below wood floor date/app. by Electrical rough-in date/app. by
Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by Pool date/app. by
Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by
Pump pole date/app. by Utility Pole date/app. by M/H tie downs, blocking, electricity and plumbing date/app. by
Reconnection date/app. by RV date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 150.00 CERTIFICATION FEE \$ 0.00 SURCHARGE FEE \$ 0.00
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 250.00
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

File Number: 07-0215

General Warranty Deed

Columbia County Building Permit Application

For Office Use Only Application # 0903-11 Date Received 3/9 By JTW Permit # 27724 ¹⁷²¹
 Application Approved by - Zoning Official BLK Date 01.04.09 Plans Examiner My Date 3/31/09
 Flood Zone X plat Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
 Comments Impact Fee Exempt - Accessory Structure
☒ NOC ☒ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel # ☐ Development Per Fax

Name Authorized Person Signing Permit Ronald & Connie Justice Phone 561-793 4463
 Address 17560 66th W Loxahatchee, FL 33470
 Owners Name Ronald & Connie Justice Phone 561-793 4463
 911 Address 529 SW MADISON Ct LAKE CITY FL 32024
 Contractors Name HOME OWNER Phone SAME
 Address _____

Fee Simple Owner Name & Address _____
 Bonding Co. Name & Address NONE
 Architect/Engineer Name & Address _____
 Mortgage Lenders Name & Address NONE

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive En
 Property ID Number R02791-108 Estimated Cost of Construction 30,000
 Subdivision Name WEST WIND ESTATES Lot 8 Block _____ Unit _____ Phase _____
 Driving Directions US90 AND PINE MOUNT RD 252. GO WEST ON PINE MOUNT RD. ABOUT 4 MILE TO MADISON CT. TURN LEFT GO TO END OF STREET Lot 8

Type of Construction METAL STORAGE BLDG Number of Existing Dwellings on Property 0
 Total Acreage 5 AC Lot Size _____ Do you need a Culvert Permit or Culvert Waiver or Have an Existing I
 Actual Distance of Structure from Property Lines - Front 145' Side E 100 FT Side W 55 FT Rear 60 FT
 Total Building Height 16 FT Number of Stories 1 Heated Floor Area 0 Roof Pitch 1-12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Ronald R. Justice
 Owner Builder or Authorized Person by Notarized Letter

STATE OF FLORIDA
 COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me
 this _____ day of _____ 20____.

Personally known _____ or Produced Identification _____

JTW called Ronald 4/01/09.

Contractor Signature _____
 Contractors License Number _____
 Competency Card Number _____
 NOTARY STAMP/SEAL

CL# 8429

Notary Signature _____

(Revised Sept. 20

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: **YOU ARE HEREBY NOTIFIED** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.



Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Contractor's Signature (Permitee)

Contractor's License Number _____
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this ____ day of _____ 20____
Personally known _____ or Produced Identification _____

State of Florida Notary Signature (For the Contractor)

SEAL: _____

CALIFORNIA ALL-PURPOSE ACKNOWLEDGMENT

State of California

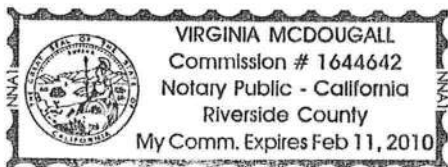
County of Riverside

On June 6, 2007 before me, Virginia McDougall Notary
Date Name and Title of Officer (e.g., "Jane Doe, Notary Public")

personally appeared Jacob Mathews
Name(s) of Signer(s)

☐ personally known to me

☒ (or proved to me on the basis of satisfactory evidence)



to be the person(s) whose name(s) is/are subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their authorized capacity(ies), and that by his/her/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument.

WITNESS my hand and official seal,

Place Notary Seal Above

Virginia McDougall
Signature of Notary Public

OPTIONAL

Though the information below is not required by law, it may prove valuable to persons relying on the document and could prevent fraudulent removal and reattachment of this form to another document.

Description of Attached Document

Title or Type of Document: General Warranty Deed

Document Date: June 6, 2007 Number of Pages: 2 including this Page

Signer(s) Other Than Named Above: _____

Capacity(ies) Claimed by Signer(s)

Signer's Name: Jacob Mathews

- ☒ Individual
☐ Corporate Officer — Title(s): _____
☐ Partner — ☐ Limited ☐ General
☐ Attorney in Fact
☐ Trustee
☐ Guardian or Conservator
☐ Other: _____

Signer Is Representing: _____

RIGHT THUMBPRINT
OF SIGNER
Top of thumb here

X

Signer's Name: _____

- ☐ Individual
☐ Corporate Officer — Title(s): _____
☐ Partner — ☐ Limited ☐ General
☐ Attorney in Fact
☐ Trustee
☐ Guardian or Conservator
☐ Other: _____

Signer Is Representing: _____

RIGHT THUMBPRINT
OF SIGNER
Top of thumb here

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY
CLERKS OFFICE BEFORE YOUR FIRST INSPECTION

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and
in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of
Commencement.

IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE
RECORDING YOUR NOTICE OF COMMENCEMENT.

Tax Parcel ID Number 07-45-16
R02791-108

Inst:200912003697 Date:3/9/2009 Time:12:09 PM
DC, P. DeWitt Cason, Columbia County Page 1 of 1 B:1168 P:2046

1. Description of property: (legal description of the property and street address or 911 address)

Lot #8 WESTWIND ESTATE. 529 SW MADISON CT
LAKE CITY FL 32024

2. General description of improvement: STEEL BUILDING STORAGE

3. Owner Name & Address RONALD & CONNIE JUSTICE
529 S.W. MADISON CT LAKE CITY Interest in Property 100%

4. Name & Address of Fee Simple Owner (if other than owner):

5. Contractor Name RONALD R. JUSTICE Phone Number 561-593-4461
Address

6. Surety Holders Name AAA Phone Number
Address

Amount of Bond

7. Lender Name NONE Phone Number
Address

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be
served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name Phone Number
Address

9. In addition to himself/herself the owner designates of
to receive a copy of the Lien Notice as provided in Section 713.13 (1) -

(a) 7. Phone Number of the designee

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of
recording, (Unless a different date is specified))

THE OWNER MUST SIGN THE NOTICE OF COMMENCEMENT AND NO ONE ELSE MAY BE PERMITTED TO SIGN
IN HIS/HER STEAD.

Ronald R. Justice
Signature of Owner

Sworn to (or affirmed) and subscribed before day of 3-9, 2009.

Laurie Hodson
Signature of Notary NOTARY STAMP/SEAL



Columbia County Property Appraiser

DB Last Updated: 3/5/2009

2009 Preliminary Values

Tax Record

Property Card

Interactive GIS Map

Print

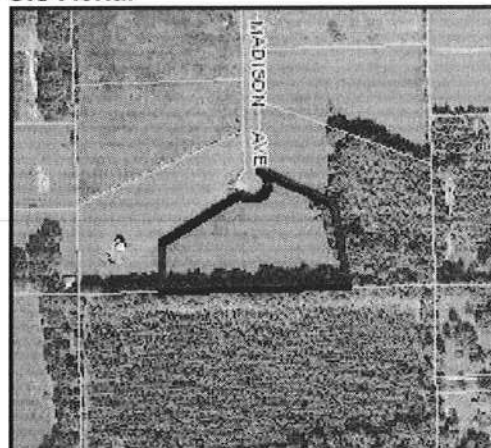
Parcel: 07-4S-16-02791-108

Search Result: 1 of 1

Owner & Property Info

Owner's Name	JUSTICE RONALD R & CONNIE S		
Site Address	MADISON		
Mailing Address	17560 66TH COURT N LOXAHATCHEE, FL 33470		
Use Desc. (code)	VACANT (000000)		
Neighborhood	7416.02	Tax District	3
UD Codes	MKTA01	Market Area	01
Total Land Area	5.210 ACRES		
Description	LOT 8 WESTWIND ESTATES. ORB 979-1744, WD 1012-47, WD 1055-2818, WD 1121-1108		

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (1)	\$61,132.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (0)	\$0.00
XFOB Value	cnt: (0)	\$0.00
Total Appraised Value		\$61,132.00

Just Value	\$61,132.00
Class Value	\$0.00
Assessed Value	\$61,132.00
Exempt Value	\$0.00
Total Taxable Value	\$61,132.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
6/6/2007	1121/1108	WD	V	Q		\$105,000.00
8/18/2005	1055/2818	WD	V	Q		\$72,000.00
4/7/2004	1012/47	WD	V	Q		\$45,800.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (5.210AC)	1.00/1.00/1.30/1.00	\$61,132.00	\$61,132.00

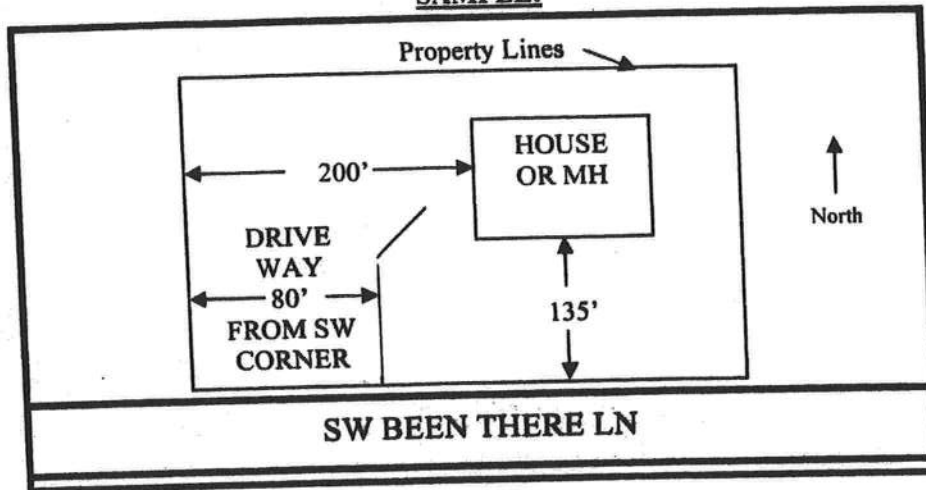
Columbia County Property Appraiser

DB Last Updated: 3/5/2009

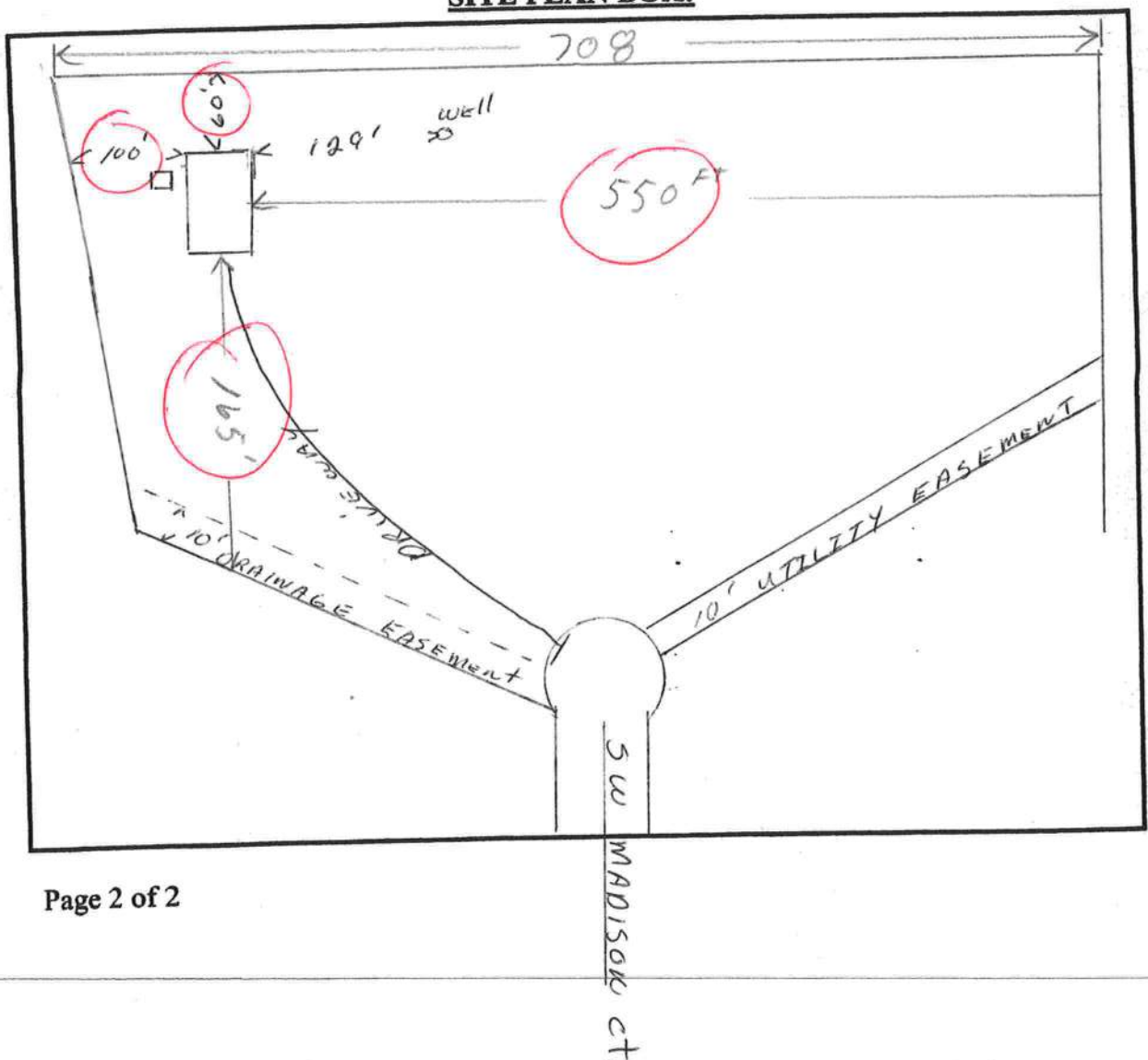
1 of 1

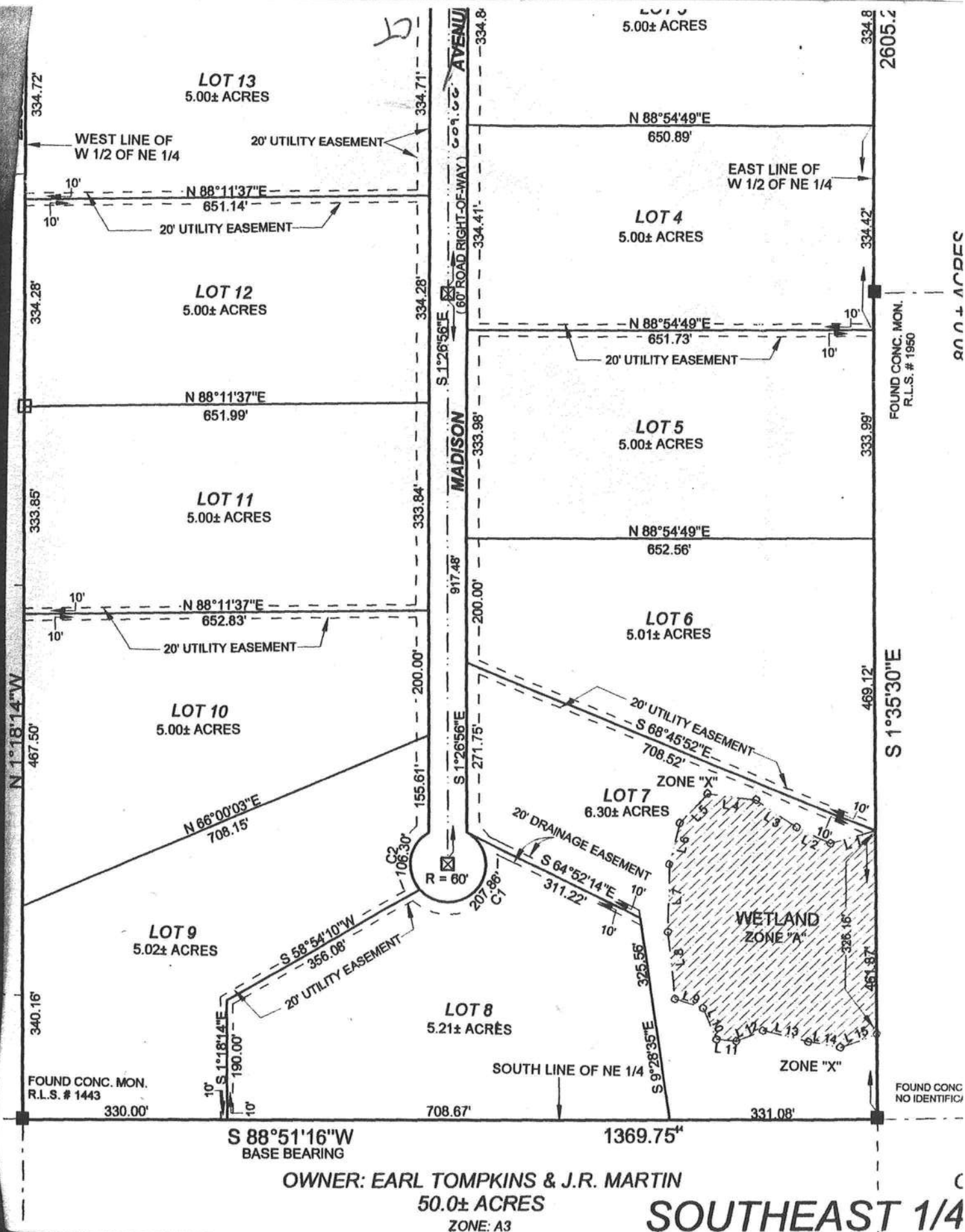
1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



SITE PLAN BOX:







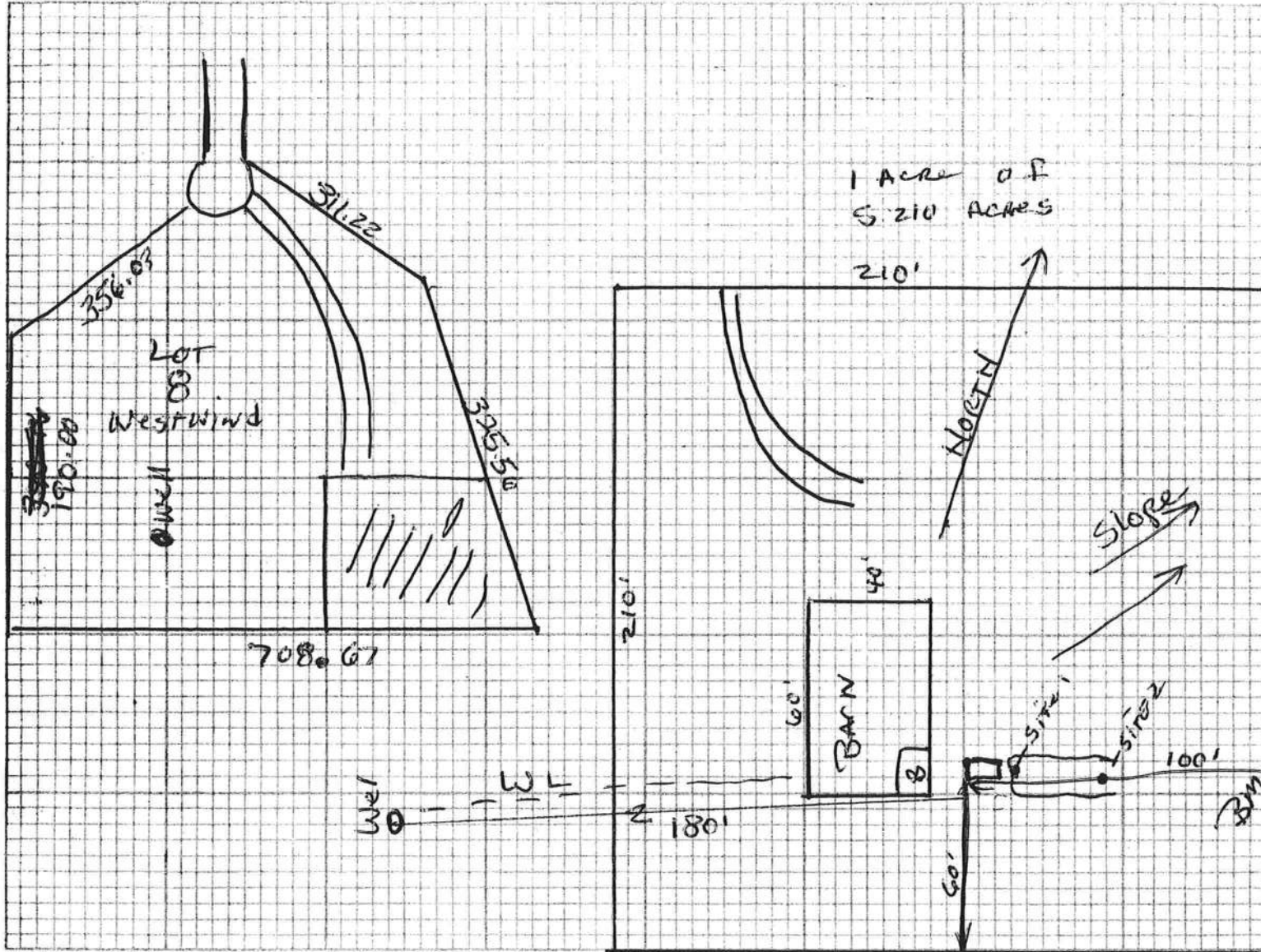
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 09-0141

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes:

Ronald & Connie Justice
LOT 8 WESTWIND ESTATES
07-45-16-02-791-108

Site Plan submitted by: Ronald W. Justice
Signature

Plan Approved APPROVED Not Approved
By [Signature] **Columbia CHD**

Agust
Title

Date 3/19/9

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT
Authority: Chapter 381, FS & Chapter 10D-6, FAC

PERMIT # 914407
DATE PAID 3/10/09
FEE PAID \$ 310.00
RECEIPT # 1103815

APPLICATION FOR:

☒ New System ☐ Existing System ☐ Holding Tank ☐ Temporary/Experimental
☐ Repair ☐ Abandonment ☐ Other (Specify) _____

APPLICANT: RONALD & CONNIE JUSTICE

TELEPHONE: 755-6372

AGENT: Robert Ford NFST inc

MAILING ADDRESS: 580 NW Guerdon Rd Lake City FL 32055

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. ATTACH BUILDING PLAN AND TO-SCA SITE PLAN SHOWING PERTINENT FEATURES REQUIRED BY CHAPTER 10D-6, FLORIDA ADMINISTRATIVE CODE.

PROPERTY INFORMATION [IF LOT IS NOT IN A RECORDED SUBDIVISION, ATTACH LEGAL DESCRIPTION OR DEE

LOT: 8 BLOCK: ✓ SUBDIVISION: WESTWIND ESTATES DATE OF SUBDIVISION: _____

PROPERTY ID #: 07-45-16-02791-108 [Section/Township/Range/Parcel No.] ZONING: _____

PROPERTY SIZE: 5.210 ACRES [Sqft/43560] PROPERTY WATER SUPPLY: ☒ PRIVATE ☐ PUBL

PROPERTY STREET ADDRESS: SW MADISON AVE

DIRECTIONS TO PROPERTY: HWY 90 WEST TO PINE MOUNT TL

GO TO SW MADISON AVE LOT AT DEADEND

BUILDING INFORMATION ☒ RESIDENTIAL ☐ COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	# Persons Served	Business Activity For Commercial Only
1	<u>BARN</u>	<u>2</u>	<u>40x60</u> <u>2400</u>	<u>2</u>	
2					
3					
4					

☐ Garbage Grinders/Disposals ☐ Spas/Hot Tubs ☐ Floor/Equipment Drai
☐ Ultra-low Volume Flush Toilets ☐ Other (Specify) _____

APPLICANT'S SIGNATURE: Robert W. Justice DATE: 3/10/09

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787

PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 7/11/2007 DATE ISSUED: 7/13/2007

ENHANCED 9-1-1 ADDRESS:

529 SW MADISON

CT

LAKE CITY FL 32024

PROPERTY APPRAISER PARCEL NUMBER:

07-4S-16-02791-108

Remarks:

LOT 8 WESTWIND ESTATES

Address Issued By:


Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

851

Approved Address

JUL 13 2007

911Addressing/GIS Dept

Columbia County Building Department Culvert Permit

Culvert Permit No.
000001721

DATE 04/02/2009 PARCEL ID # 07-4S-16-02791-108
APPLICANT RONALD JUSTICE PHONE 561.793.4463
ADDRESS 17560 66TH CT LOXAHATCHEE FL 33470
OWNER RONALD & CONNIE JUSTICE PHONE 561.793.4463
ADDRESS 529 SW MADISON CT LAKE CITY FL 32024
CONTRACTOR RONALD JUSTICE PHONE 561.793.4463
LOCATION OF PROPERTY 90-W TO PINEMOUNT RD, TL GO 4 MILES TO MADISN CT, TL GO TO THE
VERY END OF CUL-DE-SAC.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT WEST WIND ESTATES 8

SIGNATURE

Ronald Justice

INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



BUCK STEEL, INC.
6810 LYONS TECH. CIR #105
COCONUT CREEK, FL 33073

RON JUSTICE
JOB NO. 1194R2
BUILDING SIZE:
WIDTH : 40 ft.
LENGTH : 60 ft.
EAVE HT : 16 ft.
JOBSITE : LAKE CITY, FL

DATE: 3/17/09

To Whom It May Concern:

This is to certify that the above referenced building is designed in accordance with the order documentation, the 13TH Edition of the American Institute of Steel Construction (AISC) "Manual of Steel Construction" and the 1986 Edition of American Iron and Steel Institute (AISI) " Cold Formed Steel Design Manual. "The basic loads of the subject building meet or exceed the minimum county climatic data as published in the 1996 edition of the MBMA " Low Rise Building Systems Manual ".

The criteria for application of design loads are follows
Governing Code : FBC 07 (IBC 06)

Roof Dead Load : 2.000 psf plus wt. of metal bldg structure
Roof Live Load : 20.00 psf
Frame Live Load : 20.00 psf W/REDUCTION

Collateral Load	: 0 psf	Roof Snow Load	: 0 psf
Wind Load (3 sec gust)	: 110 mph	Snow Exp. Fac	: 1.00
Enclosure Type	: CLOSED	Snow Imp. Fac.	: 1.00
Wind Exp. Cat	: B		
Wind Imp. Factor	: 1.00		
Ground Snow Load	: 0 psf		

This Letter of Certification applies solely to the building and its component parts as furnished by the Metal Building Manufacturer. Doors, windows and louvers are not structural components of the building. It is the responsibility of the owner to determine if wind lock accessories are supplied if required. Certification specifically excludes any foundation, masonry, or general contract work.

Sincerely,

RICHARD T. SMITH, P.E.

Richard T. Smith

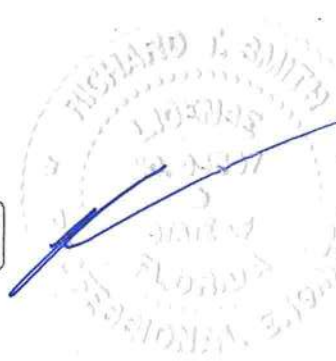
PE # 43547

102 Main Street Ste#212

Lagrange, Georgia Ph: (706) 888-4874

REVIEWED

By Richard at 4:26 pm, Mar 17, 2009





CAL-TECH TESTING, INC.

ENGINEERING & TESTING LABORATORY

P.O. Box 1625, Lake City, FL 32056-1625
4784 Rosselle St. • Jacksonville, FL 32254
2230 Greensboro Hwy., Quincy, FL 32351

27724

Lake City • (386) 755-3631

Fax • (386) 752-5450

Jacksonville • (904) 381-8901

Fax • (904) 381-8901

Quincy • (850) 442-3491

Fax • (850) 442-4001

JOB NO.: 09-147
DATE TESTED: 4/20/09

REPORT OF IN-PLACE DENSITY TEST

ASTM METHOD: ☒ (D-2922) Nuclear ☐ (D-2937) Drive Cylinder ☐ Other

PROJECT: Rem Justice Storage Bldg.

CLIENT: Rem Justice

GENERAL CONTRACTOR: SAC

EARTHWORK CONTRACTOR: SAC

SOIL USE (SEE NOTE): 1- Pad

SPECIFICATION REQUIREMENTS: 95%

TECHNICIAN: C. Day

MODIFIED (ASTM D-1557): ☒

STANDARD (ASTM D-698):

TEST NO.	TEST LOCATION	TEST:	PROCTOR NO.	WET DENS. LBS./CU.FT.	DRY DENS. LBS./CU.FT.	MOIST PERCENT	% MAX. DENS.
		DEPTH ELEV. LIFT					
1	North end approx. center 10' S.	12"	Put	105.9	102.3	3.5	99
2	Approx. center of pad	12"	Put	107.9	103.4	4.3	100
3	South end approx. center 10' N.	12"	Put	108.1	103.6	4.3	101

REMARKS:

PROCTOR NO.	SOIL DESCRIPTION	PROCTOR VALUE	OPT. MOIST.
Put	Light Brown Sand	103.1	10.8

NOTE: 1. Building Fill 2. Trench Backfill 3. Base Course 4. Subbase/Stabilized Subgrade 5. Embankment 6. Subgrade/Natural Soil 7. Other
The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test location and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

REPORT OF GEOTECHNICAL EXPLORATION

**Proposed Metal Building
529 SW Madison Court
Lake City, Columbia County, Florida
CTI Project No. 09-00147-01**

- Prepared For -
Mr. Ron Justice
17560 66TH Court N.
Loxahatchee, Florida 33470

- Prepared by -
Cal-Tech Testing, Inc.
P.O. Box 1625
Lake City, Florida 32056-1625

April 17, 2009

27724



Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

P.O. Box 1625 • Lake City, FL 32056
4784 Rosselle Street • Jacksonville, FL 32254

Tel. (386) 755-3633 • Fax (386) 752-5456

Tel. (904) 381-8901 • Fax (904) 381-8902

LABORATORIES

April 17, 2009

Mr. Ron Justice

17560 66TH Court N.

Loxahatchee, Florida 33470

Subject: Report of Geotechnical Exploration
Proposed Metal Building
529 SW Madison Court
Lake City, Columbia County, Florida
CTI Project No. 09-00147-01

Dear Mr. Justice:

Cal-Tech Testing, Inc. (CTI) has completed the geotechnical exploration for the proposed metal building at the subject site. This exploration was performed in general accordance with our proposal dated March 24, 2009. Authorization to this work and acceptance of the proposal was provided by you on April 8, 2009.

The following report presents the results of our field exploration and testing, an evaluation of the subsurface conditions with respect to available project characteristics, and recommendations to aid in the design and construction of the proposed building.

We have enjoyed assisting you on this project and look forward to serving as your geotechnical and construction materials testing consultant for the remainder of this and future projects. Should you have any questions concerning this report, please contact our office at 386-755-3633.

Sincerely,

CAL-TECH TESTING, INC.

David B. Brown

Executive Vice President

Nabil O. Hmeidi, P.E.

Senior Geotechnical Engineer

Licensed, Florida No. 57842

Distribution: File (1 copy)
Addressee (2 bound copies)

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Attachments

Exhibit 1	Vicinity Map (1 page)
Exhibit 2	Soil Boring Logs (2 pages)
Exhibit 3	Unified Soil Classification System Chart (1 page)
Exhibit 4	Key To Test Data (1 page)

1.0 PROJECT INFORMATION

The purpose of this exploration was to develop information concerning the site and subsurface conditions in order to evaluate site preparation requirements and foundation support recommendations for the proposed metal building. The subject site is located at 529 SW Madison Court in Lake City, Columbia County, Florida. This report briefly describes our field activities and presents our findings.

It is our understanding the proposed building will be 60' by 38', one-story structural steel building to be used as a warehouse/office space. Framing will consist of Concrete Masonry Unit (CMU)/or metal studs supported on a conventional shallow foundation system. Field testing related to drainage or pavement design is beyond the scope of this exploration.

Detailed structural information has not been provided; however, we anticipate individual column loads will not exceed 50 kips. We have assumed that soil-supported ground floor loads (dead load plus live load) in the proposed building will not exceed 150 psf. We have not been provided finished floor elevation for the proposed structures; however, we assume that less than two feet of earthwork fill will be required to achieve desired grade.

2.0 FIELD EXPLORATION

The subsurface conditions at the subject site were explored by drilling two (2) Standard Penetration Test (SPT) borings each extending 15 feet below the existing ground surface. The SPT borings B-1 and B-2 were performed at the approximate northwest and southeast corners of the proposed building, respectively. The locations were based on building perimeters preset by others prior to our arrival at the subject site.

Sampling and penetration procedures of the SPT borings were accomplished in general accordance with ASTM D-1586, "*Penetration Test and Split-Barrel Sampling of Soils*", using a power rotary drill rig (BK-51 with an automatic hammer). The standard penetration tests were performed by driving a standard 1-3/8" I.D. and 2" O.D. split spoon sampler with a 140 pound hammer falling 30 inches. The number of hammer blows required to drive the sampler a total of 18 inches, in 6 inch increments, were recorded. The penetration resistance or "N" value is the summation of the last two 6 inch increments and is illustrated on the attached boring logs adjacent to their corresponding sample depths. The penetration resistance is used as an index to derive soil parameters from various empirical correlations.

The attached record of boring logs presents the descriptions of the subsurface conditions encountered at the time of our field program, and also provide the penetration resistances recorded during the drilling and sampling process. The stratification lines and depth designations on the boring record represent the approximate boundaries between the various soils encountered, as determined in the field by our personnel. In some cases, the transition between the various soils may be gradual.

3.0 SITE AND SUBSURFACE CONDITIONS

3.1 Site Conditions

The existing site conditions were observed by our personnel during our field program. At the time of our visit, the ground surface within the proposed building area appears to have been recently cleared of vegetations and topsoil. An approximately 1½ feet of new fill appears to have been placed within the building area. There was no ponded water observed during our site visit.

3.2 Area Geology/Sinkholes Potential

Published information regarding the geology in this area of Columbia County, Florida indicates the site is situated near the interface of areas designated as Undifferentiated Quaternary Sediments (**Qu**) of the Pleistocene and Holocene epochs; and the Statenville Formation (**Ths**) of the Miocene epoch. Typically, the Undifferentiated Quaternary sediments consist of siliciclastics, organics and freshwater carbonates. The siliciclastics are light gray, tan, brown to dark, unconsolidated to poorly consolidated, clean to clayey, silty, fossiliferous, variably organic-bearing sands to blue green to olive green, poorly to moderately consolidated, sandy, silty, clays. Freshwater carbonates "*marls*" are buff colored to tan, unconsolidated to poorly consolidated, fossiliferous (mollusks) carbonate muds containing organics.

Mainly, the Statenville Formation consists of interbedded sands, clays and dolostones with common to very abundant phosphate grains. The sands are predominate and are light gray to olive gray, poorly indurated, phosphatic, fine to coarse grained with scattered gravel and with minor occurrences of fossils. Clays are yellowish gray to olive gray, poorly consolidated, variably sandy and phosphatic, and variably dolomitic. The dolostones are yellowish gray to light orange, poorly to well indurated, sandy, clayey and phosphatic with scattered mollusk molds and casts.

The limestone in this area consists of carbonate rock and its weathered residuum. In this area of Columbia County, Florida, the limestone is marked by solution features (sinkholes) associated with *karst* terrains. Areas underlain by karst terrains are prone to sinkhole activities due to weathering which is typified by a chemical dissolution process that progresses along joints, fractures and bedding planes in the limestone formation. Sinkholes are primarily caused by an advanced state of internal soil erosion or raveling action, which under certain circumstances can lead to ground subsidences. This internal soil erosion is a very slow process by which soil particle usually migrate under the influence of a hydraulic gradient to underlying karsted and/or fractured limestone formation. This process often results in a highly irregular geological profile that contains deep weathered slots "*chimneys*" filled with soft soils. Voids or caves may also be present within limestone formation. Surface depressions or sinkholes are formed when the soil overburden subsides into these subsurface caverns. There are several indicators generally associated with an advanced state of long term internal soil erosion such as noticeable surface depressions and very loose to soft soil zones just above the bedrock (or weathered bedrock) formation.

Our knowledge of the surrounding areas indicate the presence of active sinkholes within the site vicinity. However, our observation of the subject site and results of the test borings did not reveal presence of active sinkholes within the explored profile. Therefore, it is our opinion the proposed development on this site will have no greater risk of damage due to sinkhole activity than the development of structures in other areas within the immediate vicinity of the subject site. It must be understood that this exploration was not intended to predict or preclude future sinkholes from occurring within the limits of subject site.

3.3 Subsurface Conditions

Initially, the soil profile as disclosed by SPT borings B-1 and B-2 consisted of about 1½ feet of yellowish tan, silty fine sand (SP-SM) underlain by about 4 to 5½ feet of residual yellowish tan, silty fine sand (SP-SM). This stratum is underlain by about 8 to 9½ feet of reddish brown, mottled with light gray, clayey fine sand (SC). Generally, the standard penetration resistance indicates the relative density of the site soils vary from very loose to very dense with “N” values ranging from 2 to exceeding 50 Blow Per Foot (BPF).

The discovery of any site or subsurface condition during any future exploration or any change in project information which deviates from the data and information presented in this evaluation should be reported to us for further evaluation. It should be understood that subsurface conditions may vary between and away from the boring locations, and that transitions between soil types may be gradual. Refer to the attached boring records for more details.

3.4 Groundwater

At the time of completion of drilling, the groundwater was not encountered in any of the SPT borings. We note that due to the relatively short time frame of the field exploration, the groundwater may not have had sufficient time to stabilize. For a true groundwater level reading, piezometers may be required. In any event, fluctuation in groundwater levels should be expected due to seasonal climatic changes, construction activity, rainfall variations, surface water runoff, and other site-specific factors.

4.0 RECOMMENDATIONS FOR FOUNDATION DESIGN & SITE PREPARATION

The recommendations presented in this report are based upon available project information, anticipated loading conditions, and data obtained during our field program. If the structural information is incorrect or the location of the structure changes, please contact this office so our recommendations may be reviewed and/or revised. Discovery of any site or subsurface condition during construction, which deviates from the data collected during this exploration, should be reported to us for evaluation.

4.1 General

Based on our evaluation of the encountered subsoils, anticipated loading conditions and our past experience with similar projects, it is our professional opinion the subject site can be made suitable for the support of the proposed development.

4.2 Foundation Support

Our site observation indicated the presence of loose soils within the upper 3 feet of the existing ground surface. These soils are considered suitable for use as structural fill, however, they are not considered acceptable for the support of the proposed building in their current conditions. To improve the density of these soils, the upper 2 feet of the site soils (including 5 feet outside the perimeter of the building) should be recompacted as indicated herein.

Provided the foundation and site soils are prepared in accordance with the guidelines presented in this report, it is our opinion the proposed structure may be supported on a conventional shallow foundation system. The shallow foundation may be designed for an allowable bearing pressure of 1,500 pounds per square foot (psf) or less supported on **recompacted** soils or newly placed structural fill.

In using net pressures, the weight of the footing and backfill over the footing need not be considered. Hence, only loads applied at or above final grade need to be used for dimensioning footings. However, wall bearing footings should be designed with a minimum width of 18 inches, while the individual column footings should have minimum dimensions of 2 feet by 2 feet.

4.3 Settlement Analyses

Actual magnitude of settlement that will occur beneath foundations will depend upon variations within the subsurface soil profile, actual structural loading conditions, embedment depth of the footings, actual thickness of compacted fill or cut, and the quality of the earthwork operations. Assuming the foundation related site work and foundation design is completed in accordance with the enclosed recommendations, we estimate the total settlement of the structure will be on the order of 1 inch or less. Differential settlements (between adjacent columns or along the length of a continuous wall footing) should be approximately one-half of the total settlement. This settlement is primarily the result of elastic compression of the upper looser sands, and should occur almost immediately following the application of the structural dead load during construction.

4.4 Uplift Resistance

Under wind loading conditions, the foundations will likely be subjected to considerable uplift forces. In order to resist these uplift forces, it may be necessary to increase the footing size (thus increasing the dead weight) or lower the footing to mobilize additional soil weight above the footing. Uplift resistance from the soil may be evaluated as the weight of the soil directly above the footing, plus the shearing resistance along the vertical face of the soil prism. Alternately, the available soil uplift resistance may be calculated as the weight of the soil prism defined by the diagonal line drawn from the top of the footing to the ground surface at an angle of 30 degrees with the vertical. We recommend that a total unit weight of 100 pcf (compacted to 95% of the modified Proctor maximum dry density) be used for well-compacted, suitable fill. Should the

bottom of any structure be below the stabilized seasonal-high groundwater level, these structures must be properly designed to resist the resulting uplift forces due to hydrostatic pressures.

4.5 Lateral Resistance

Lateral loads created by wind loads may be resisted by the passive pressure of the soil acting against the side of the individual footings and/or the friction developed between the base of the foundation system and the underlying soils. For compacted backfill and/or in-situ material, the passive pressure may be taken as an equivalent to the pressure exerted by a fluid weighing 300 pcf for above the ground-water table and 113 pcf below the water level. A coefficient of friction equal to 0.4 may be used for calculating the frictional resistance at the base of the shallow footings. The resistance values discussed herein are based on the assumption that the foundations can withstand horizontal movements on the order of ¼ inch. Lateral resistance determined in accordance with the recommendations provided herein should be considered the total available resistance. Consequently, the design should include a minimum factor of safety of 1.5.

4.6 Lateral Earth Pressures

In general, retaining walls (if any) are subject to "at-rest" or "active" pressures. Retaining walls that are restrained at the top will be subject to "at-rest" pressures due to their restricted movement. These "at-rest" pressures may be calculated as the equivalent pressure exerted by a fluid density of 50 pcf. Where walls are not restrained at the top and thus allowed sufficient movement to mobilize "active" pressures, an equivalent fluid density of 33 pcf should be used in the design.

These values may be used only for walls above the groundwater table. Therefore, the presence of any groundwater due to surface water intrusion should be handled with the use of a drainage layer behind the walls with a collection pipe discharging accumulated water away from the walls. If this is not practical, then the hydrostatic pressure due to water should be included in the design of the walls.

4.7 Drainage Considerations

Adequate drainage should be provided at the site in order to minimize increase in moisture content of the foundation soils. Excessive moisture can significantly reduce the soil's bearing capacity and contribute to foundation settlement. For the protection of the foundation soils, we recommend that the ground water surface be sloped away from all proposed structures.

4.8 Floor Slab

Exposed subgrade should be properly recompact and proofrolled with a fully-loaded, tandem-axle dump-truck or similar pneumatic-tired equipment. Provided the recompaction and proofrolling operations do not indicate significant deflecting or pumping of the existing subgrade, the floor slab may be designed as a slab-on-grade. Any soft or loose soils found during the proofrolling procedure should be undercut and replaced with suitable, well-compacted, engineered fill.

All floor slabs should be supported on at least 4 inches of relatively clean granular material, such as sand, sand and gravel, or crushed stone. This is to help distribute concentrated loads and equalize moisture beneath the slab. This granular material should have 100 percent passing the 1½ -inch sieve and a maximum of 10 percent passing the No. 200 sieve. A vapor retarder may be installed on top of the subgrade to reduce dampness of the surface of the floor slabs. The vapor retarder should consist of a minimum 6-mil thickness overlapping (unsealed) sheets of plastic. In addition, properly constructed jointing will alleviate the potential for cracking and allow for some differential movement.

4.9 Exposed Subgrade

All vegetation, topsoil, and other organic matters should be removed from the building and pavement areas. Following this operation, the exposed soils in the building and pavement areas should be compacted with overlapping passes of a relatively heavy weight vibratory drum roller having a total operating static weight (weight of fuel and water included) of at least 10 tons and a drum diameter of 5 feet. All exposed surfaces should be compacted to a minimum of 95 percent of the modified Proctor maximum dry density (**ASTM D-1557**) to a depth of at least 12 inches below the compacted surface.

4.10 Structural Fill/Backfill

Structural fill should be placed in thin loose lifts not exceeding 12 inches in thickness and compacted with a heavy roller as described above. For walk-behind equipment, a maximum loose lift thickness of 6 inches is recommended. Each lift should be thoroughly compacted with the drum roller to provide densities equivalent to at least 95 percent of the modified Proctor maximum dry density (ASTM D-1557). Structural fill should consist of an inorganic, non-plastic, granular soil containing less than 10 percent material passing the No. 200 mesh sieve (relatively clean sand with a Unified Soil Classification of SP or SP-SM).

4.11 Pavement Subgrade Consideration

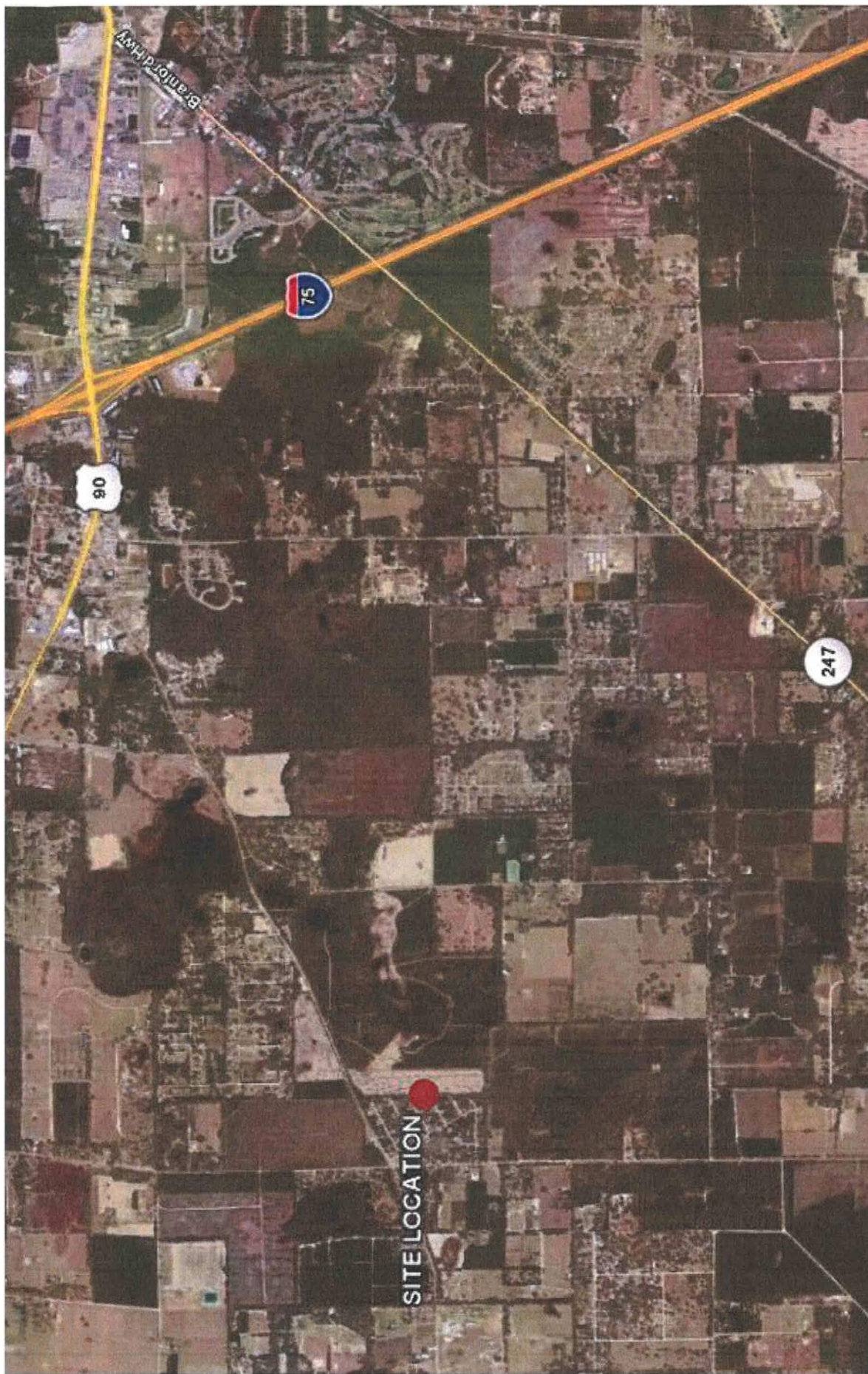
Pavement subgrades should be compacted to a minimum depth of 12 inches to at least 98 percent of the modified Proctor maximum dry density (ASTM D-1557). Any fill utilized to elevate the pavement areas to final subgrade elevation should consist of relatively clean fine sands (inorganic, non-expansive/non-plastic sands containing less than 10 percent, by weight, of fines). Pavement subgrade should be uniformly compacted to a minimum density of 95 percent of the soil's modified Proctor maximum dry density (ASTM D1557).

5.0 REPORT LIMITATIONS

This report has been prepared for the exclusive use of the Mr. Ron Justice of Loxahatchee, Florida. for the specific application to the project discussed herein. Our conclusions and recommendations have been rendered using generally accepted standards of geotechnical engineering practice in the State of Florida. No other warranty is expressed or implied. **CTI** is not responsible for the interpretations, conclusions, opinions, or recommendations of others based on the data contained herein. Field observations, monitoring, and quality assurance testing

during earthwork and foundation installation are an extension of the geotechnical design. We recommend that the owner retain these services and that **CTI** be allowed to continue our involvement in the project through these phases of construction.

ATTACHMENTS



CAL-TECH TESTING, INC.
P.O. Box 1625
Lake City, Florida 32056-1625
Phone: (386) 755-3633
Fax: (386) 752-5456

VICINITY MAP
Geotechnical Exploration
Proposed Metal Building - 529 SW Madison Court
Lake City, Columbia County, Florida
Cal-Tech Testing Project No. 09-00147-01

Figure 1



CAL-TECH TESTING, INC.
3309 SW SR 247
Lake City, Florida 32024
Telephone: (386) 755-3633
Fax: (386) 752-5456

BORING NUMBER B-1

PAGE 1 OF 1

CLIENT Mr. Ron Justice

PROJECT NAME Proposed Metal Building

PROJECT NUMBER 09-00147-01

PROJECT LOCATION 529 SW Madison Ct., Lake City, Columbia County, FL

DATE STARTED 04/14/09 COMPLETED 04/14/09

GROUND ELEVATION _____ HOLE SIZE _____

DRIILLING CONTRACTOR Cal-Tech Testing, Inc.

GROUND WATER LEVELS: _____

DRIILLING METHOD Continuous Flight Auger/Split Spoon

AT TIME OF DRILLING ---

LOGGED BY N.H. CHECKED BY _____

AT END OF DRILLING --- Not Encountered

NOTES BK-51 (manual hammer)

AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	▲ SPT N VALUE ▲			
								20	40	60	80
								PL	MC	LL	
0								20	40	60	80
		LOOSE, yellowish tan, silty fine sand (SP-SM)									
		POSSIBLE FILL UPPER 1.5 FEET	SPT 1	100	2-2-2 (4)						
			SPT 2	100	2-2-2 (4)						
5			SPT 3	100	2-2-2 (4)						
		LOOSE to MEDIUM DENSE, reddish brown, mottled with light gray, clayey fine sand (SC)	SPT 4	100	2-2-2 (4)						
			SPT 5	100	5-7-11 (18)						
10			SPT 6	100	10-12-15 (27)						
			SPT 7	100	9-12-18 (30)						
15											

Bottom of borehole at 15.0 feet.

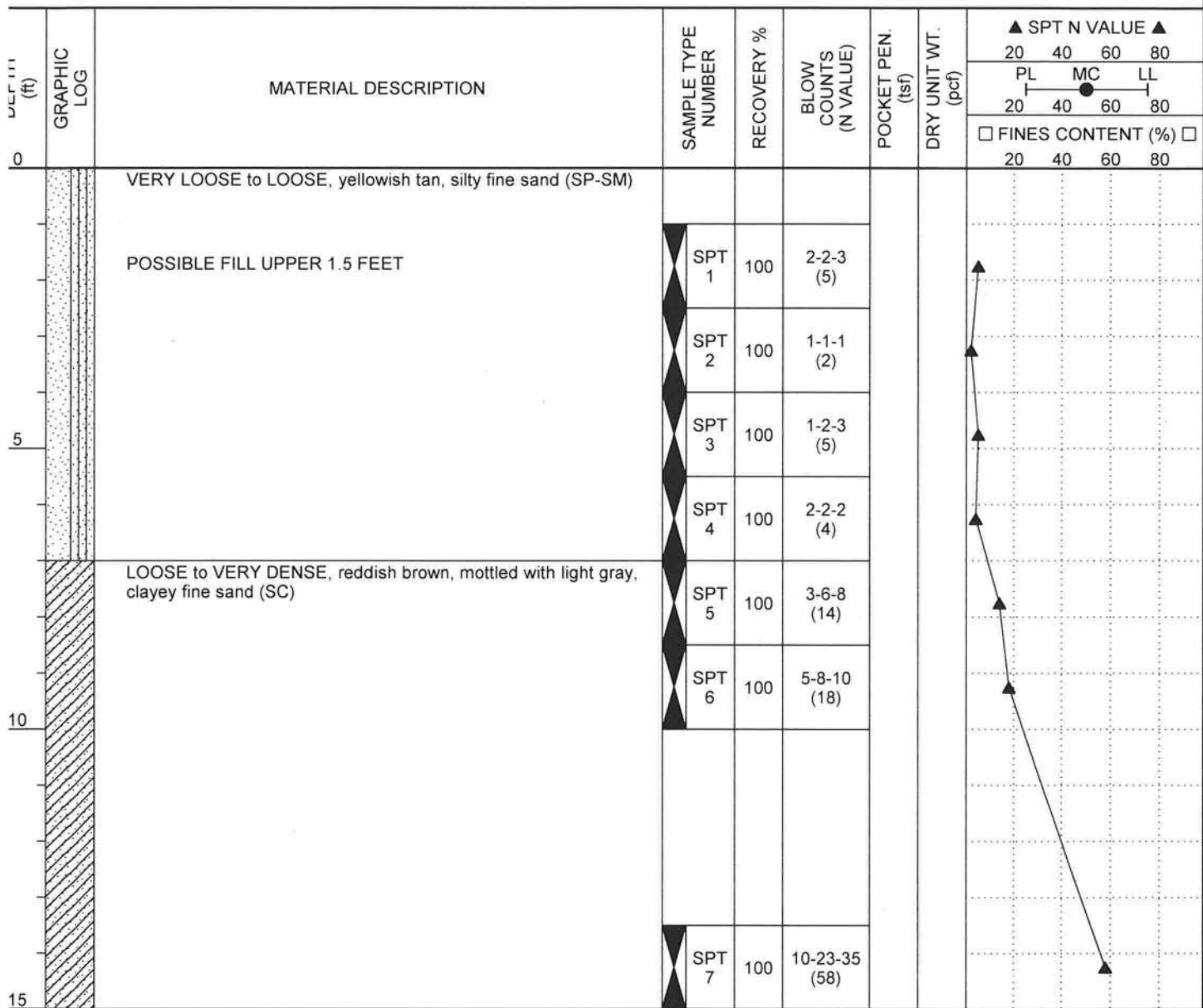


CAL-TECH TESTING, INC.
3309 SW SR 247
Lake City, Florida 32024
Telephone: (386) 755-3633
Fax: (386) 752-5456

BORING NUMBER B-2

PAGE 1 OF 1

CLIENT Mr. Ron Justice PROJECT NAME Proposed Metal Building
PROJECT NUMBER 09-00147-01 PROJECT LOCATION 529 SW Madison Ct., Lake City, Columbia County, FL
DATE STARTED 04/14/09 COMPLETED 04/14/09 GROUND ELEVATION _____ HOLE SIZE _____
DRILLING CONTRACTOR Cal-Tech Testing, Inc. GROUND WATER LEVELS: _____
DRILLING METHOD Continuous Flight Auger/Split Spoon AT TIME OF DRILLING ---
LOGGED BY N.H. CHECKED BY _____ AT END OF DRILLING --- Not Encountered
NOTES BK-51 (manual hammer) AFTER DRILLING ---



Bottom of borehole at 15.0 feet.

UNIFIED SOIL CLASSIFICATION SYSTEM

ASTM DESIGNATION D-2487

MAJOR DIVISIONS			GROUP SYMBOL	TYPICAL NAMES	LABORATORY CLASSIFICATION CRITERIA			
COARSE GRAINED SOILS (More than half of the material is larger than No. 200 sieve)	Gravels (more than half of the coarse fraction is larger than No. 4 sieve)	Clean gravels	GW	Well-graded gravels, gravel-sand mixtures, little or no fines.	Determine percentage of sand and gravel from grain size curve Depending on percentage of fines (fraction smaller than No. 200 Sieve size), coarse grained soils are classified as follows: Less than 5% GW, GP, SW, SP More than 12% ... GM, GC, SM, SC 5 to 12% Borderline cases requiring dual symbols	$C_u = \frac{D_{60}}{D_{10}} > 4 \quad ; \quad 1 < C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} < 3$		
			GP	Poorly graded gravels, gravel-sand mixture, little or no fines.		Not meeting all gradation requirements of GW		
		Gravel with fines	GM	Silty gravels, gravel-sand-silt mixtures.		Atterberg Limits below A-Line or PI less than 4	Above A-Line with PI between 4 and 7 are borderline cases requiring the use of dual symbols.	
			GC	Clayey gravels, gravel-sand-clay mixtures.		Atterberg Limits above A-Line or PI greater than 7		
	Sands (more than half of the coarse fraction is smaller than No. 4 sieve)	Clean sands	SW	Well-graded sands, gravelly sands, little or no fines.		$C_u = \frac{D_{60}}{D_{10}} > 6 \quad ; \quad 1 < C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} < 3$		
			SP	Poorly graded sands, gravelly sands, little or no fines.		Not meeting all gradation requirements of SW		
		Sands with fine	SM	Silty sands, sand-silt mixtures.		Atterberg Limits below A-Line or PI less than 4	Limits plotting in hatched zone with PI between 4 and 7 are borderline cases requiring the use of dual symbols.	
			SC	Clayey sands, sand-clay mixtures.		Atterberg Limits above A-Line or PI greater than 7		

FINE GRAINED SOILS (More than half of the material is finer than No. 200 sieve)	Silts and Clays (LL less than 50)	ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity.	<div>PLASTICITY CHART</div> <div>1. Plot intersection of PI as determined by the Atterberg Limits tests. 2. Points plotted above the A-Line indicate clay soils. 3. Points plotted below the A-Line indicate silt.</div> <div>LL = 43.5 PI = 46.5</div>	
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clay.		
		OL	Organic silts and organic silty clays of low plasticity.		
	Silts and Clays (LL greater than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.		
		CH	Inorganic clays of high plasticity, fat clay.		
		OH	Organic clays of medium to high plasticity, organic silts.		
	Highly Organic Soils	Pt	Peat and other highly organic soils.		

CAL-TECH TESTING, INC.

P.O. Box 1625
 Lake City, Florida 32056-1625
 Phone: 386-755-3633 Fax: 386-752-5456

5% Max. Passing the U.S. No. 200 Sieve SP
 5% - 12% Passing the U.S. No. 200 Sieve SP-SM
 12% - 50% Passing the U.S. No. 200 Sieve SM/SC

KEY TO TEST DATA

STANDARD PENETRATION TEST:

Soil sampling and penetration testing is performed in accordance with ASTM D-1586. The standard penetration resistance ("*N*") is the number of blows of a 140-pound hammer falling 30 inches to drive a 2-inch O.D., 1.4-inch I.D. split spoon sampler one foot.

ROCK CORE DRILLING:

Rock sampling and core drilling is performed in accordance with ASTM D-2113. The rock quality designation percentage (RQD) is determined by summing only pieces of core that are at least 4 inches long, and dividing by the "run" length.

Relation of RQD and In-situ Rock Quality	
RQD (%)	Rock Quality
90 - 100	Excellent
75 - 90	Good
50 - 75	Fair
25 - 50	Poor
0 - 25	Very Poor

RELATIVE DENSITY (SANDS):

Very loose - less than 4 blows/ft.

Loose - 5 to 10 blows/ft.

Medium - 11 to 30 blows/ft.

Dense - 31 to 50 blows/ft.

Very dense - over 50 blows/ft.

CONSISTENCY (SILTS & CLAYS):

Very soft - less than 2 blows/ft.

Soft - 3 to 4 blows/ft.

Medium stiff - 5 to 8 blows/ft.

Stiff - 9 to 15 blows/ft.

Very stiff - 16 to 30 blows/ft.

Hard - 31 to 50 blows/ft.

Very hard - over 50 blows/ft.

HARDNESS (ROCKS):

Soft - Rock core crumbles when handled.

Medium - Can break core with hands.

Moderately hard - Thin edges of rock core can be broken with fingers.

Hard - Thin edges of core can not be broken with fingers.

Very hard - Can not be scratched with knife.

GROUNDWATER:

Water levels shown on boring logs are taken immediately upon completion of boring, and are intended for general information. The apparent level may have been altered by the drilling process. Groundwater levels, if desired, can be monitored over a long time interval.

CAL-TECH TESTING, INC.

P.O. Box 1625

Lake City, Florida 32056-1625

Phone: 386-755-3633 Fax: 386-752-5456

5% Max. Passing the U.S. No. 200 Sieve SP

5% - 12% Passing the U.S. No. 200 Sieve SP-SM

12% - 50% Passing the U.S. No. 200 Sieve SM/SC

COLUMBIA COUNTY FLORIDA DEPARTMENT OF BUILDING AND ZONING OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 07-4S-16-02791-108

Building permit No. 000027724

Use Classification METAL STORAGE BLDG.

Fire: 0.00

Permit Holder RONALD JUSTICE

Waste: 0.00

Owner of Building RONALD & CONNIE JUSTICE

Total: 0.00

Location: 529 SW MADISON CT., LAKE CITY, FL

Date: 08/31/2009



[Signature]
Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address: 536 56 Baya Dr

City: Lake City

Phone: 386-752-1703

Site Location: Subdivision _____

Lot # 8

Block# _____

Permit # 000027724

Address 529 SW Madison Ct Lake City

Product used

Active Ingredient

% Concentration

☒ Premise Imidacloprid 0.1%

☐ Termidor Fipronil 0.12%

☐ Bora-Care Disodium Octaborate Tetrahydrate 23.0%

Type treatment:

☒ Soil

☐ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

Main Body

2400

200

125

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

4/27/09

Date

8:20

Time

Nail S. F295

Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05



DESIGN CRITERIA

Width (ft)	= 40
Length (ft)	= 60
Front Eave Height (ft)	= 16
Back Eave Height (ft)	= 16
Front Roof Slope (Rise/12)	= 1.0:12
Dead Load (psf)	= 2.000
Collateral Load (psf)	= 0
Roof Live Load (psf)	= 20.00
Frame Live Load (psf)	= 20.00 w/REDUCTION
Roof Snow Load (psf)	= 0
Wind Speed (mph)	= 110
Wind Code	= FBC 07 (IBC 06)
Occupancy Category	= II - Normal
Exposure	= B
Closed/Open/Partial	= CLOSED
Importance - Wind	= 1.00
Internal G.Cpi (ENCLOSED)	= +0.18 / -0.18
(PART, ENCLOSED)	= +0.55 / -0.55
(OPEN)	= +0.00 / -0.00

<u>Structural Steel</u>	
ASTM# (Plate)	= A1011
Plate Yield (Fy)	= 50.0 ksi
ASTM# (Bar)	= A592
Plate Yield (Fy)	= 50.0 ksi

<u>Light Gage Steel</u>	
ASTM# (Cold-Form)	= A1011
Cold-Form Yield (Fy)	= 55.0 ksi
ASTM# (Panel)	= A792
Panel Yield (Fy)	= 80.0 ksi

NOTE: ALL CONNECTION BOLTS ARE DESIGNATED IN THESE DRAWINGS AS EITHER A "M" FOR A307 BOLTS OR A "H" FOR A325 BOLTS.

NOTES TO ERECTOR/OWNER:

- [1] MBM IS NOT RESPONSIBLE FOR THE ERECTION OF THE BUILDING, THE SUPPLY OF ANY TOOLS OR EQUIPMENT, OR ANY OTHER FIELD WORK UNLESS MBM HAS BEEN CONTRACTED FOR THESE. MBM DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION OF THE BUILDING, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.
- [2] USE ONLY THE ERECTION DRAWINGS PROVIDED BY MBM AND INCLUDED IN THE ERECTOR'S PACKAGE DELIVERED BY THE TRUCK DRIVER WITH THE BUILDING. MBM IS NOT LIABLE FOR ANY CLAIM RESULTING FROM THE USE OF OTHER DRAWINGS.
- [3] CHECK SLAB AND ANCHOR BOLT PLACEMENTS BEFORE STANDING ANY FRAMING. IF THE SLAB IS NOT SIZED CORRECTLY OR IS OUT OF SQUARE, OR IF THE ANCHOR BOLTS ARE NOT CORRECTLY LOCATED, CALL MBM. MBM IS NOT LIABLE FOR LABOR CHARGES RESULTING FROM STANDING FRAMING ON AN INCORRECT SLAB.
- [4] BEGIN ERECTION WITH A BRACED BAY. INSTALL THE EAVE STRUTS FIRST AND THEN THE PURLINS WHICH FALL AT THE CABLE ATTACHMENT POINTS. NEXT, INSTALL ROOF AND WALL CABLES TO A SNUG CONDITION, SO THAT THE FRAMING IS BRACED. FINISH INSTALLING PURLINS AND GIRTS IN THE BRACED BAY. USING THE THE CABLE BRACING, SQUARE AND PLUMB THE FRAMING. CONTINUE WITH REMAINING BAYS, INSTALLING BRACING AS ADDITIONAL BRACED BAYS ARE ERECTED.
- [5] THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. CONTACT MBM BEFORE MAKING ANY FIELD MODIFICATION TO THE BUILDING. MBM DOES NOT PAY CLAIMS FOR ERROR CORRECTION UNLESS APPROVED IN WRITING BY MBM BEFOREHAND.

Richard T. Smith

PE # 43347
102 Main Street Ste#212
Lagrange, Georgia Pk. (706) 888-4874

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By Richard at 4:27 pm, Mar 17, 2009

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REVISIONS

[1]
[2]
[3]
[4]
[5]

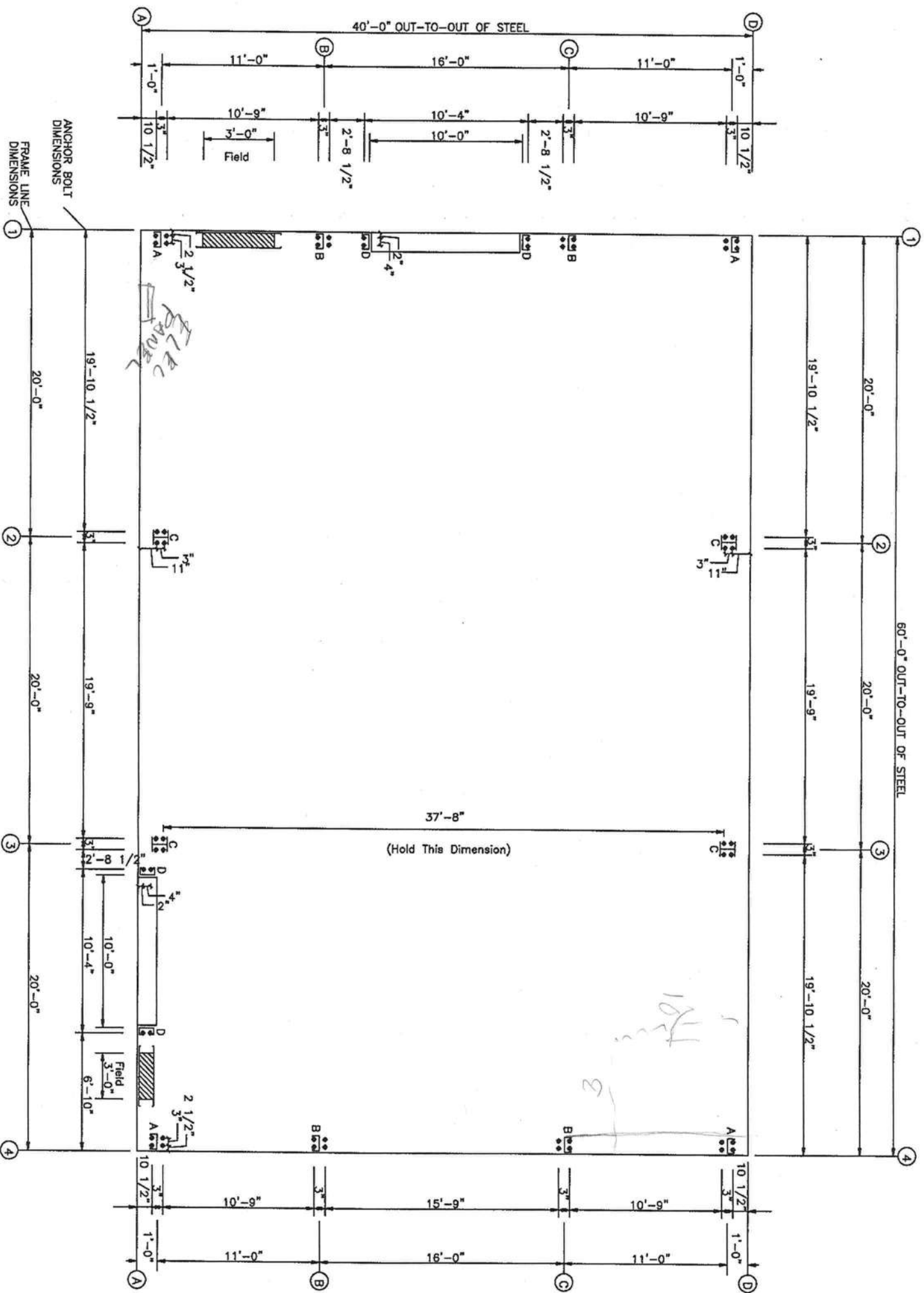
FOR:
RON JUSTICE
529 SW MADISON COURT
LAKE CITY, FL

FROM:
BUCK STEEL, INC.
6810 LYONS TECH. CIR #105
COCONUT CREEK, FL 33073

FLORIDA PRODUCT APPROVAL NUMBER
26 Ga. PBR PANEL FL#5687.2
3070"M" WALKDOOR FL#2501

JOB NO: 1194R2
DATE: 3/17/09
BY: JDH SCALE: NONE
TITLE: COVER PAGE
NUMBER:
PAGE 0

⊕ Dia= 5/8"
⊗ Dia= 3/4"

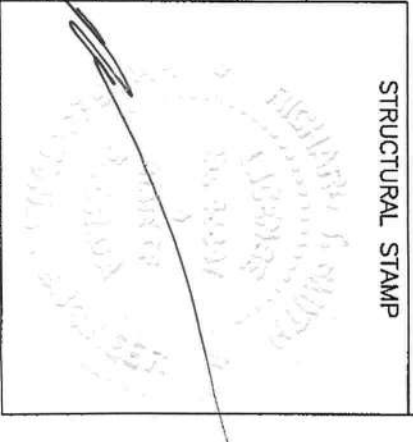


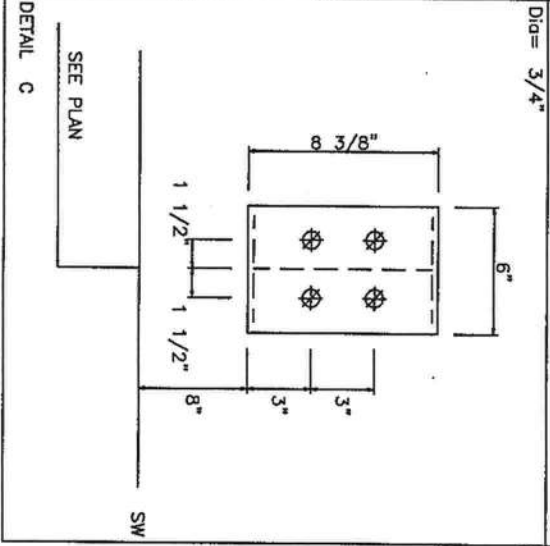
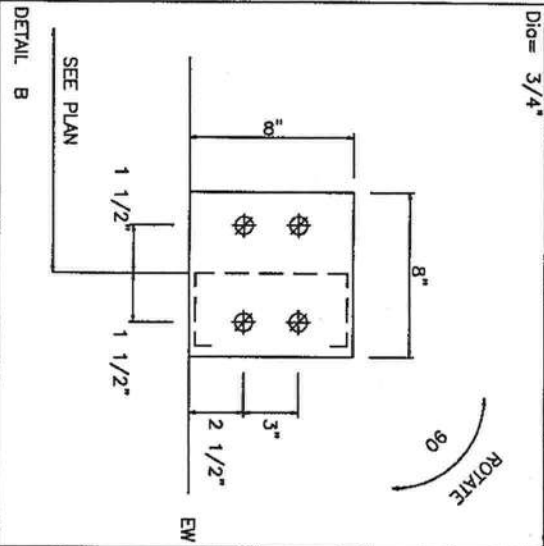
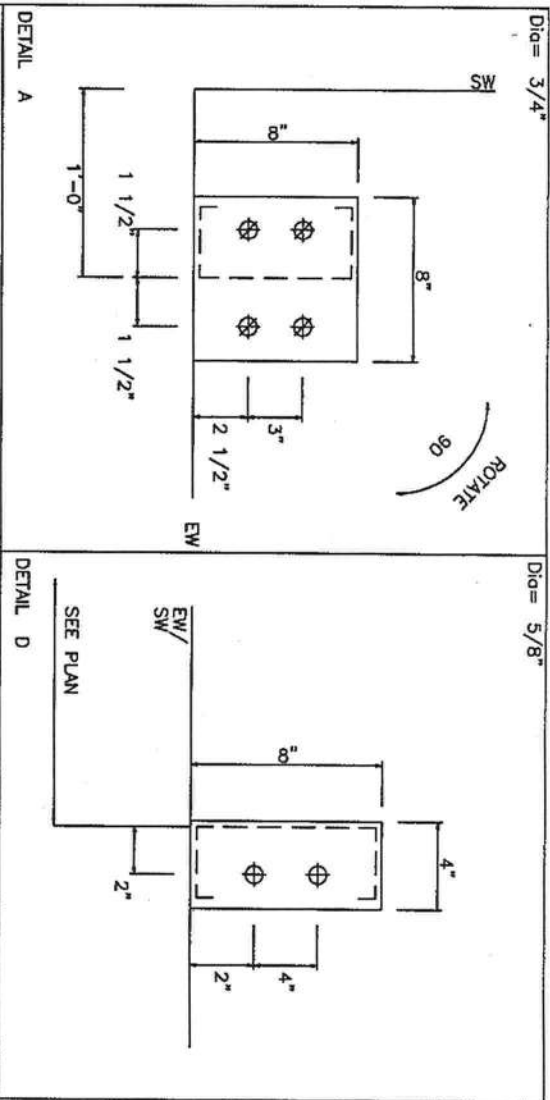
NOTE: FIELD LOCATE
(2) 3070 F.O.

BUCK STEEL, INC.	
CUSTOMER:	RON JUSTICE
JOB NO.:	1194R2
LOCATION:	LAKE CITY, FL
DRAWING NAME:	ANCHOR BOLT LAYOUT
DRAWING NO.:	PAGE 1
DRAWN BY:	JDH
CHECKED BY:	BHM
DATE:	3/17/09
SCALE:	NONE

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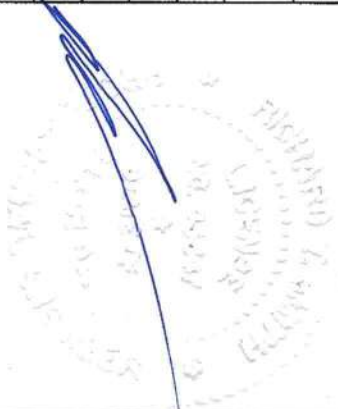


BUCK STEEL, INC.			
CUSTOMER: RON JUSTICE			
JOB NO: 1194R2			
LOCATION: LAKE CITY, FL			
DRAWING NAME: ANCHOR BOLT DETAILS			
DRAWING NO: PAGE 1.1			
DRAWN BY: JDH			
CHECKED BY: BHM			
DATE: 3/17/09			
SCALE: NONE			
REVISIONS			
[1]			
[2]			
[3]			

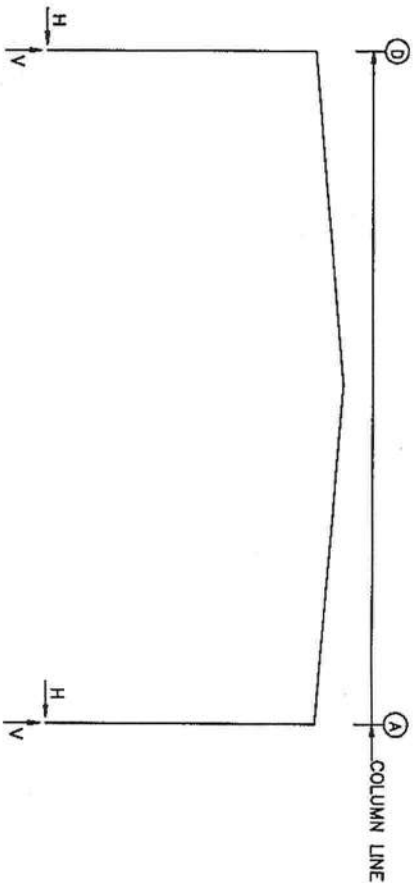
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FRAME LINES: 2 3



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Column Reactions (k)		Anc. Bolt No D(in)	Base Plate (in)		Groat (in)
			Hmax	Vmax		Wid	Len	
2 * D	1	1.9	2.0	4	-3.2	4	0.750	6.000
	1	1.8	6.1	6	-0.1	4	0.750	6.000
	5	3.2	-3.2	2	-1.9	4	0.750	6.000
2 * A	1	-1.8	6.1	7	0.1	4	0.750	6.000
	1					8.380	0.375	0.0

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:

Width (ft) = 40.0
Length (ft) = 60.0
Eave Height (ft) = 16.0/16.0
Roof Slope (Rise/12) = 1.0/1.0
Dead Load (psf) = 2.0
Collateral Load (psf) = 0.0
Roof Live Load (psf) = 20.0
Frame Live Load (psf) = 20.0
Wind Speed (mph) = 110.0
Wind Code = FBC 07 (IBC 06)
Exposure = B
Closed/Open/Partial = C
Importance Wind = 1.00
Importance Seismic = 1.00
Seismic Design Category = B
Seismic Coeff (Fa/Ss) = 0.19

5. Loading conditions are:

- DL+CL+LL
- DL+CL+LL+0.75LL+0.75WL1
- DL+CL+LL+0.75LL+0.75WL1
- 0.60DL+WL2
- 0.60DL+WL2
- 0.60DL+LWmd1+LWIND1-L2E
- 0.60DL+LWmd1+LWIND1-R2E
- 0.60DL+WL2+WS
- DL+CL+0.75LL+0.75WL2+0.75WS
- 0.60DL+WL2+WS
- 0.60DL+WL2+WS
- 0.60DL+WL2+WS

ANCHOR BOLT SUMMARY

Qty	Loc	Dia (in)	Type	Projection (in)
8	DL	5/8"	A307	1.50
32	EW	3/4"	A307	1.50
16	RF	3/4"	A307	2.50

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead	Live	Wind L1	Wind R1	Wind L2	Wind R2
2 * D	A	0.33	1.25	-1.51	4.81	-3.27	-3.91
	A	-0.33	1.25	-1.51	4.79	-0.60	-3.91
2 * A	D	0.33	1.25	-1.51	4.81	-3.27	-3.91
	D	-0.33	1.25	-1.51	4.79	-0.60	-3.91
Frame Column	--Wind Ln1--	--Wind Ln2--	--Seismic L--	--Seismic R--	--LWIND1-L2E--	--LWIND1-R2E--	--LWIND1-R2E--
2 * D	A	0.30	-6.41	-0.45	-4.05	-0.07	-0.06
	A	0.30	-6.41	-0.45	-4.05	-0.07	-0.06
Frame Column	--LWIND2-L3E--	--LWIND2-R3E--	--LWIND2-L3E--	--LWIND2-R3E--	--LWIND2-L3E--	--LWIND2-R3E--	--LWIND2-R3E--
2 * D	A	0.01	-0.42	-0.05	-0.05	-0.05	-0.05
	A	0.01	-0.42	-0.05	-0.05	-0.05	-0.05

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frame Line	Col Line	Dead	Live	Rafter Wind L	Rafter Wind R	Broce Wind L	Broce Wind R	Wind P	Wind S	Wind Ln1	Wind Ln2	Seis L	Seis R
1	D	0.16	0.93	-0.97	-0.54	-0.97	-0.54	0.00	0.00	-0.75	-0.48	0.00	0.01
	D	0.45	3.14	-3.07	-2.00	-3.07	-2.00	-1.72	1.88	-2.50	-1.58	0.00	-0.01
	B	0.45	3.14	-3.07	-2.00	-3.07	-2.00	-1.72	1.88	-2.50	-1.58	0.00	-0.01
1	A	0.16	0.93	-0.97	-0.54	-0.97	-0.54	0.00	0.00	-0.75	-0.48	0.00	0.00
Frame Col	--LWIND1-L--	--LWIND1-R--	--LWIND1-L--	--LWIND1-R--	--LWIND1-L--	--LWIND1-R--	--LWIND1-L--	--LWIND1-R--	--LWIND1-L--	--LWIND1-R--	--LWIND1-L--	--LWIND1-R--	--LWIND1-R--
4	A	0.00	-0.37	0.00	-0.01	0.00	-0.01	0.00	0.00	-0.75	-0.48	0.00	0.00
	B	0.00	-0.37	0.00	-0.01	0.00	-0.01	0.00	0.00	-0.75	-0.48	0.00	0.00
	C	0.00	-0.37	0.00	-0.01	0.00	-0.01	0.00	0.00	-0.75	-0.48	0.00	0.00
4	D	0.00	-0.37	0.00	-0.01	0.00	-0.01	0.00	0.00	-0.75	-0.48	0.00	0.00

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame Line	Col Line	Load Id	Hmax	Vmax	Anc. Bolt No D(in)	Base Plate (in)	Groat (in)
1	D	8	0.0	-2.6	4	0.750	8.000
	D	8	0.0	-2.6	4	0.750	8.000
1	C	10	1.9	-3.5	4	0.750	8.000
	C	10	1.9	-3.5	4	0.750	8.000
1	B	10	1.9	-3.5	4	0.750	8.000
	B	10	1.9	-3.5	4	0.750	8.000
1	A	10	0.0	-0.9	4	0.750	8.000
	A	10	0.0	-0.9	4	0.750	8.000
4	A	8	0.0	-0.9	4	0.750	8.000
	A	8	0.0	-0.9	4	0.750	8.000
4	B	8	0.0	-0.9	4	0.750	8.000
	B	8	0.0	-0.9	4	0.750	8.000
4	C	10	1.9	-3.5	4	0.750	8.000
	C	10	1.9	-3.5	4	0.750	8.000
4	D	10	0.0	-0.9	4	0.750	8.000
	D	10	0.0	-0.9	4	0.750	8.000

BRACING REACTIONS, PANEL SHEAR

Frame Line	Col Line	Dead	Live	Wind L	Wind R	Broce Wind L	Broce Wind R
1	D	1.4	1.8	0.1	0.1	0.1	0.1
	D	1.4	1.8	0.1	0.1	0.1	0.1
1	C	2.6	1.8	0.2	0.1	0.1	0.1
	C	2.6	1.8	0.2	0.1	0.1	0.1
1	B	1.4	1.2	0.1	0.1	0.1	0.1
	B	1.4	1.2	0.1	0.1	0.1	0.1
1	A	2.6	1.8	0.2	0.1	0.1	0.1
	A	2.6	1.8	0.2	0.1	0.1	0.1

BUCK STEEL, INC.

CUSTOMER: RON JUSTICE

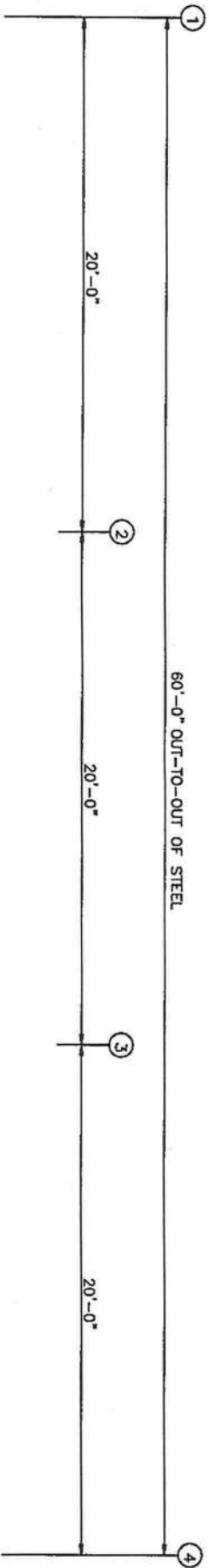
REVISIONS

NO.	DESCRIPTION	DATE
[1]	LAKE CITY, FL	3/17/09
[2]	ANCHOR BOLT REACTIONS	
[3]	PAGE 1.2	

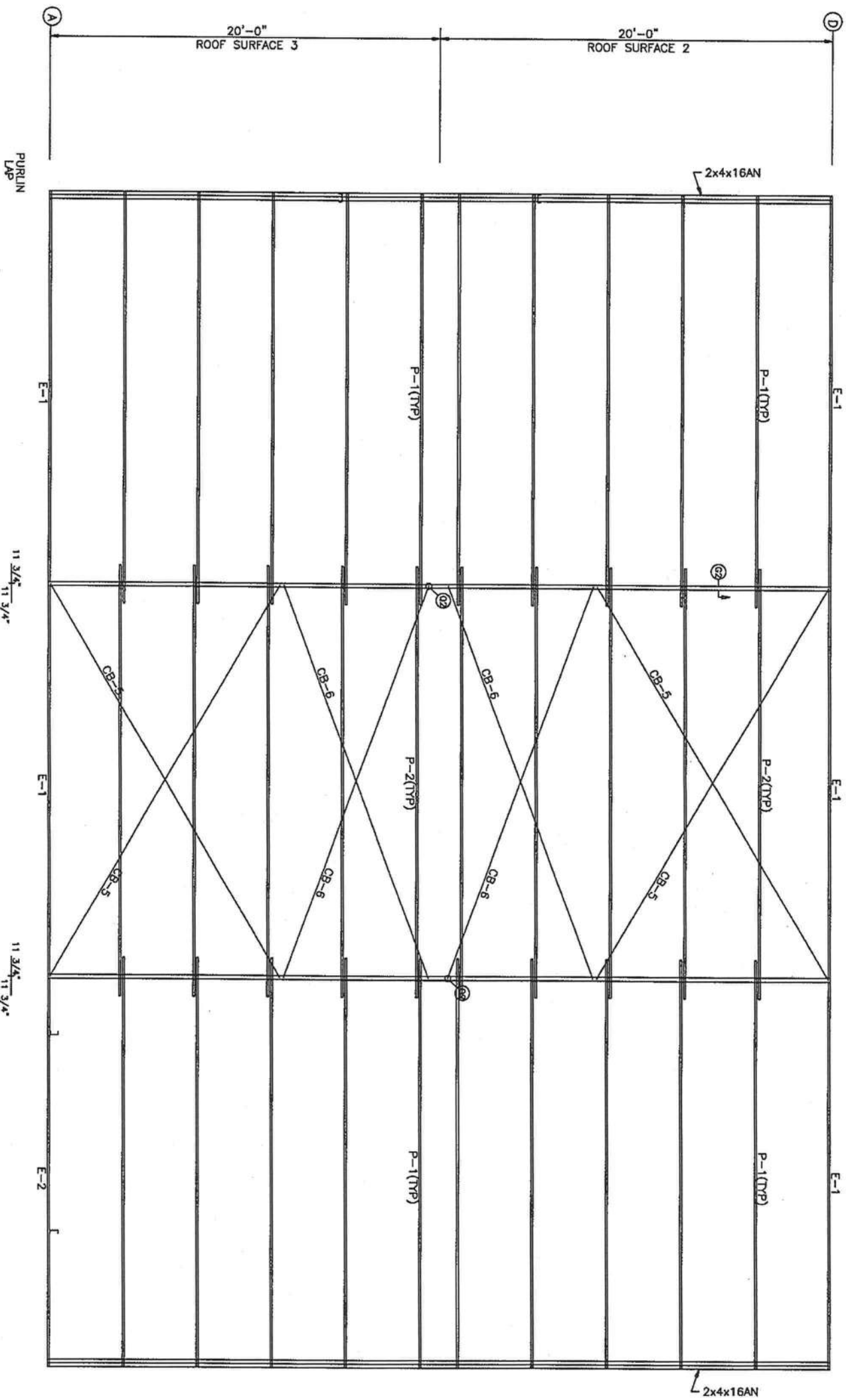
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PE #43547
102 Main Street Ste#112
Lagrange, Georgia Ph: (706) 898-4074



MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8x25Z16	20'-11 1/2"
P-2	8x25Z16	21'-11 1/2"
E-1	8ES14@1	19'-11 1/2"
E-2	8ES14@1	19'-11 1/2"
CB-5	1/4 CBL	22'-10"
CB-6	1/4 CBL	21'-10"



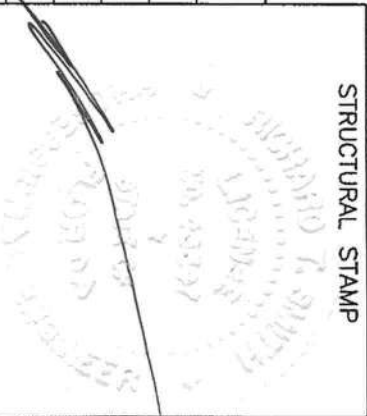
ROOF FRAMING PLAN

BUCK STEEL, INC.	
CUSTOMER: RON JUSTICE	
JOB NO: 1194R2	
DATE: 3/17/09	
LOCATION: LAKE CITY, FL	
DRAWING NAME: ROOF FRAMING LAYOUT	
DRAWING NO: PAGE 2	
DRAWN BY: JDH	
CHECKED BY: BHM	
SCALE: NONE	

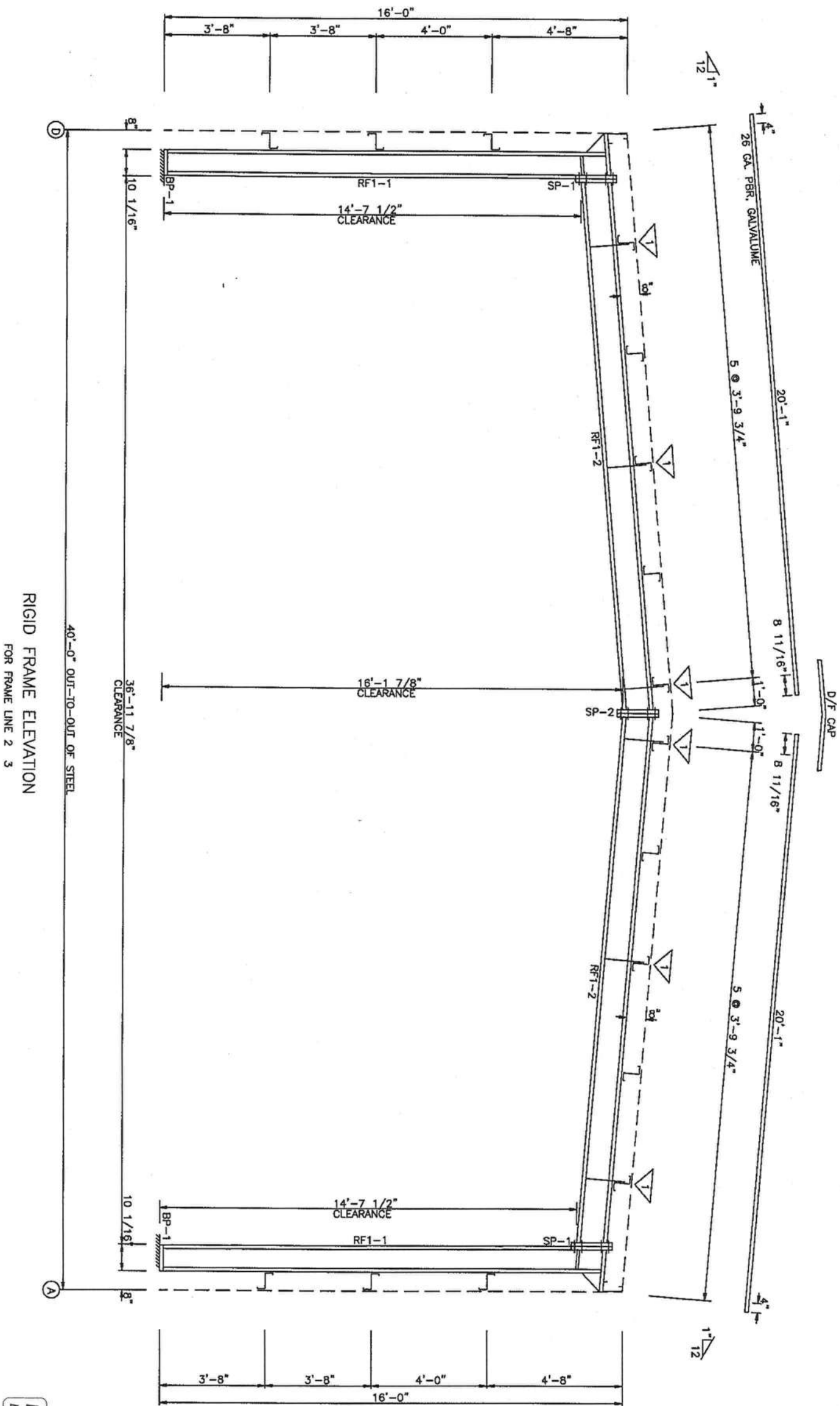
Richard T. Smith
PE # 43547
102 Main Street Ste#212
Lagrange, Georgia Pk. (706) 888-4874

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By Richard at 4:27 pm, Mar 17, 2009

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SPURCE BOLTS		BOLT	
MARK	QUAN	TOP/BOF	INT TYPE DIA LENGTH
SP-1	4	4	0 A325 3/4" 2 1/2"
SP-2	4	4	0 A325 5/8" 2 1/2"
FLANGE BRACE TABLE			
FRAME LINE	2	3	
Δ ID SIDES	MARK	LENGTH	
1	1	FBI	2'-3 1/2"
BASE PLATES		PLATE SIZE	
COL	MARK	WIDTH	THICK LENGTH
BP-1	1	6"	3/8" 8 3/8"



MEMBER SIZE TABLE		OUTSIDE FLANGE		INSIDE FLANGE	
MARK	WEB DEPTH	WEB PLATE THICK	W x I x LENGTH	W x I x LENGTH	W x I x LENGTH
RF1-1	7/16" 9.5	0.134	14'-3 1/4" 5 x 1/4" x 13'-4"	5 x 5/16" x 14'-3 1/4"	5 x 5/16" x 14'-3 1/4"
RF1-2	9/16" 9.5	0.134	18'-5 15/16" 5 x 1/4" x 18'-5 1/8"	5 x 1/4" x 18'-5 1/8"	5 x 1/4" x 18'-5 1/8"

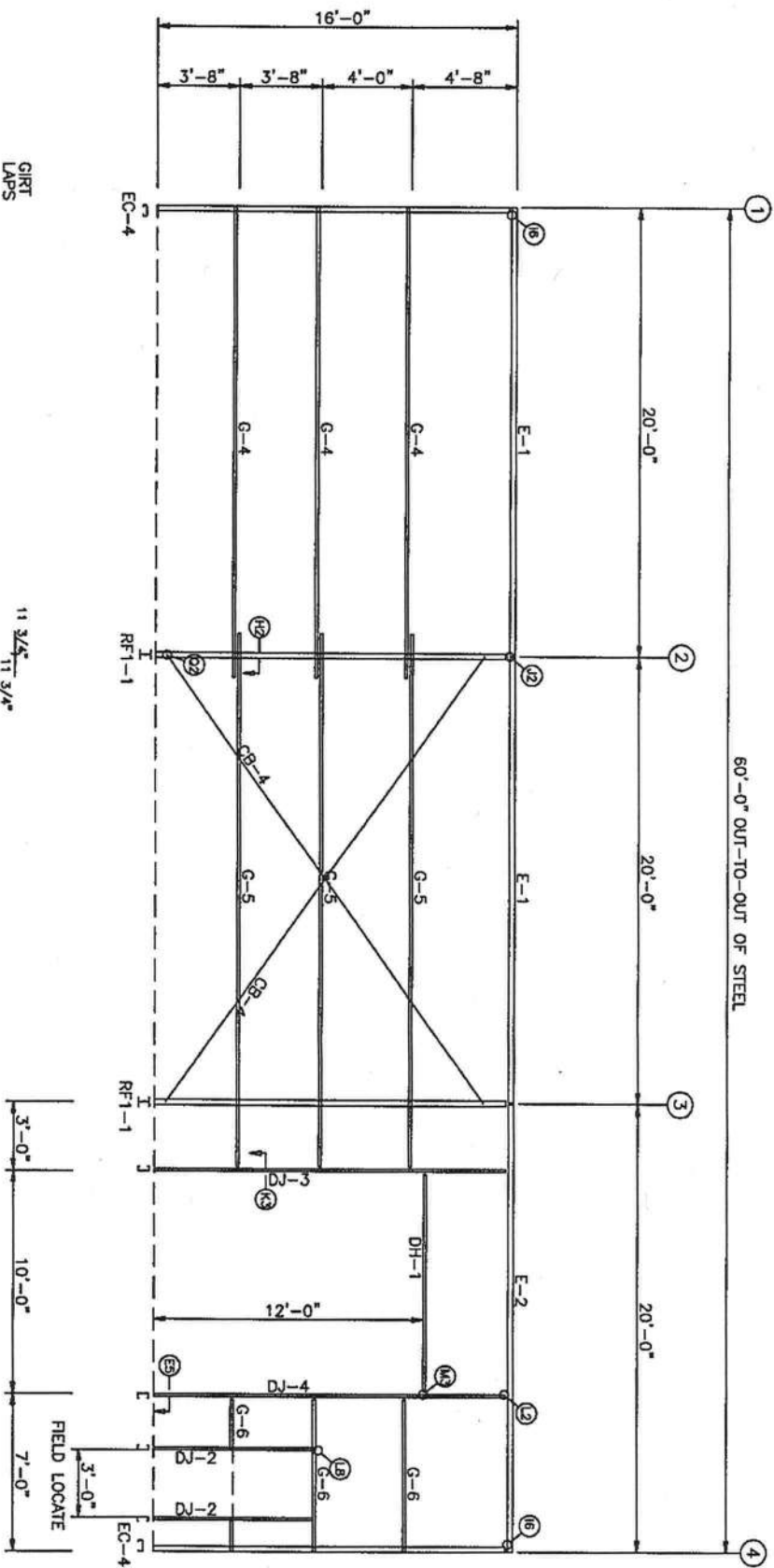
REVISIONS		BUCK STEEL, INC.	
[1]	DATE: 3/17/09	CUSTOMER: RON JUSTICE	JOB NO: 1194R2
[2]	LOCATION: LAKE CITY, FL	DRAWING NAME: RIGID FRAME CROSS SECTION	DRAWING NO: PAGE 2.1
[3]	SCALE: NONE	CHECKED BY: BHM	DRAWN BY: JDH

Richard T. Smith
PE # 43947
102 Main Street Ste#212
Lagrange, Georgia Ph: (706) 888-4874

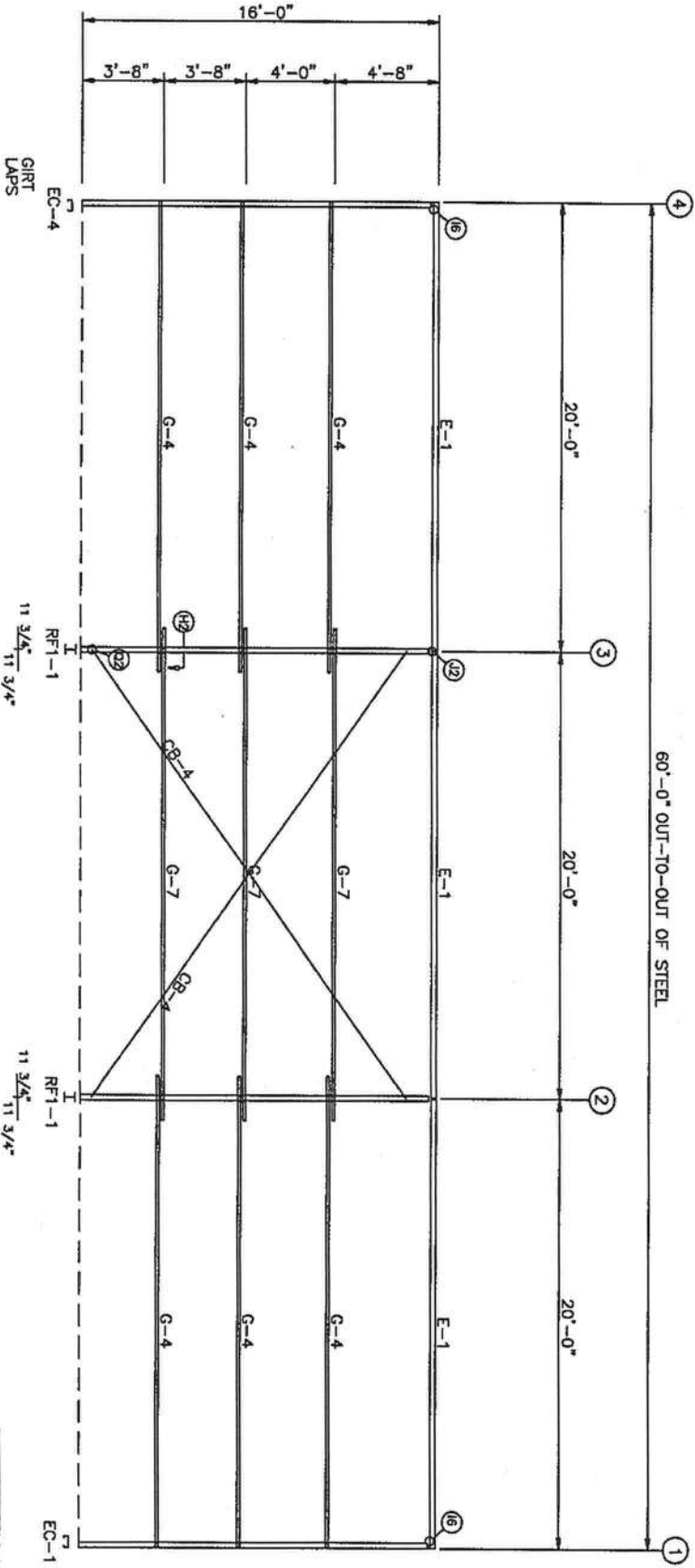
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By Richard at 4:27 pm, Mar 17, 2009

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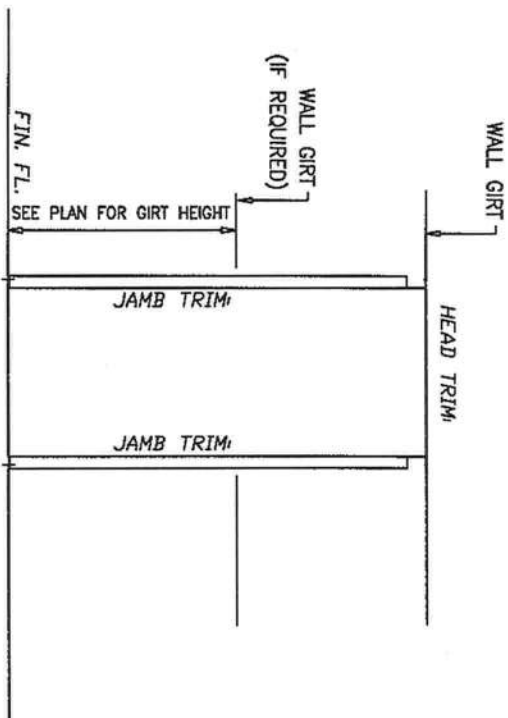
MEMBER TABLE			
FRAME LINE A & D			
MARK	PART	LENGTH	
DJ-2	8x25C16	7'-4"	
DJ-3	8X35C16	15'-4 5/8"	
DJ-4	8X35C16	15'-4 5/8"	
DH-1	8X35C16	10'-0"	
E-1	8ES14@1	19'-11 1/2"	
E-2	8ES14@1	19'-11 1/2"	
G-4	8x25Z16	20'-11 1/2"	
G-5	8x25Z16	23'-7 1/2"	
G-6	8x25Z16	6'-7 1/2"	
G-7	8x25Z16	21'-11 1/2"	
CB-4	1/4 CBL	25'-3"	



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL FRAMING: FRAME LINE D



1. FIELD CUT AND WORK GIRTS, PANELS, AND TRIM AS REQUIRED.
2. REFER TO DETAIL PAGES FOR APPLICABLE TRIM DETAILS (SEE DETAIL PAGE 5.2.)

Richard T. Smith

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Lagrange, Georgia Pn: (706) 898-4874

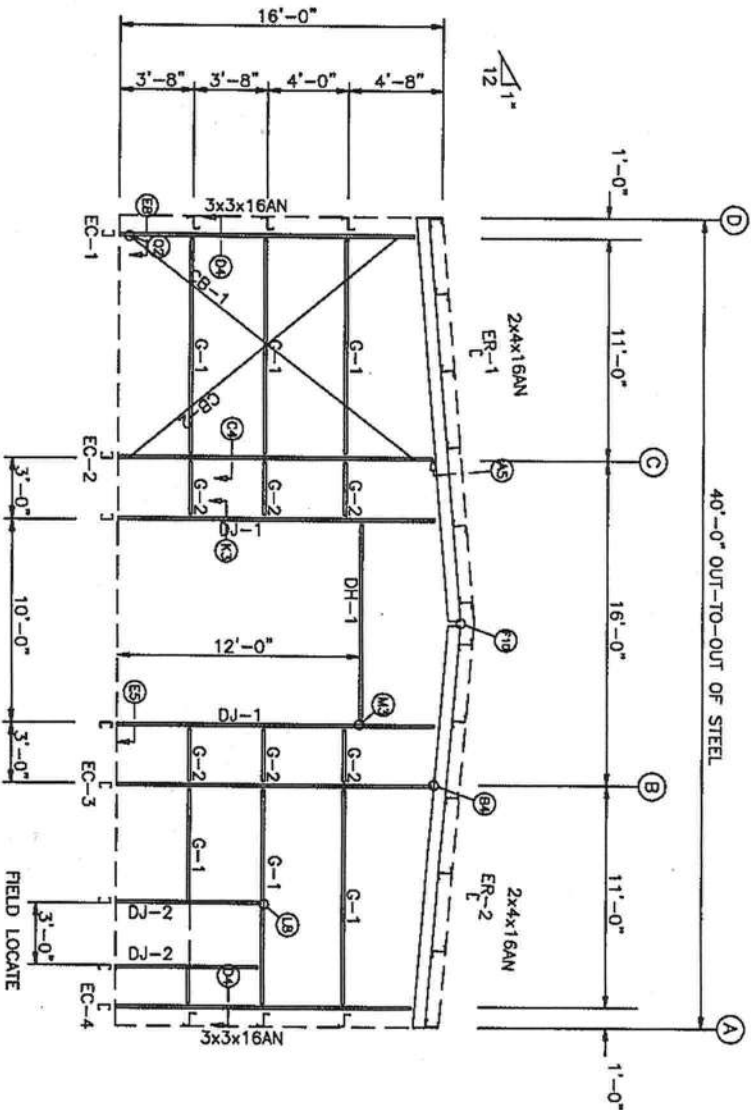
REVIEWED
By Richard at 4:27 pm, Mar 17, 2009

BUCK STEEL, INC.			
CUSTOMER: RON JUSTICE			
JOB NO: 1194R2		DATE: 3/17/09	
REVISIONS			
[1]	LOCATION: LAKE CITY, FL		
[2]	DRAWING NAME: SIDEWALL FRAMING LAYOUT		
[3]	DRAWING NO: PAGE 3		DRAWN BY: JDH
			CHECKED BY: BHM
	SCALE: NONE		

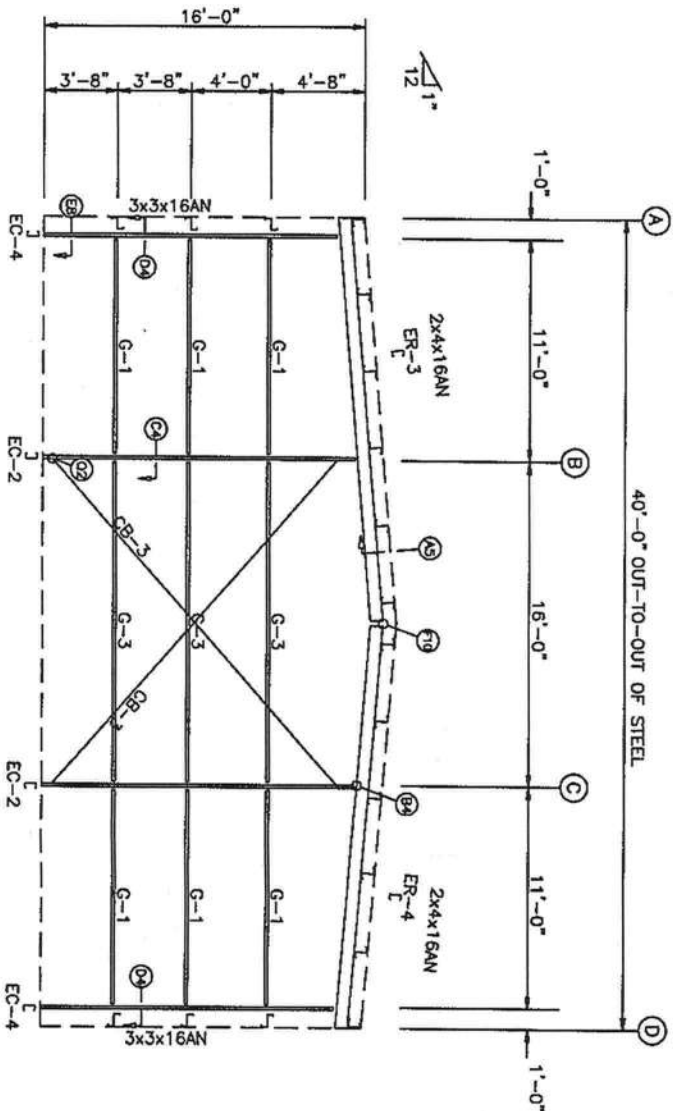
STRUCTURAL STAMP

BOLT TABLE			
FRAME LINE 1 & 4		QUAN	TYPE DIA
LOCATION			LENGTH
ER-1/ER-2		4	A325 5/8" 2"
ER-3/ER-4		4	A325 5/8" 2"
Columns/Rof		2	A325 5/8" 1 1/2"
Jamb		2	A325 5/8" 1 1/2"

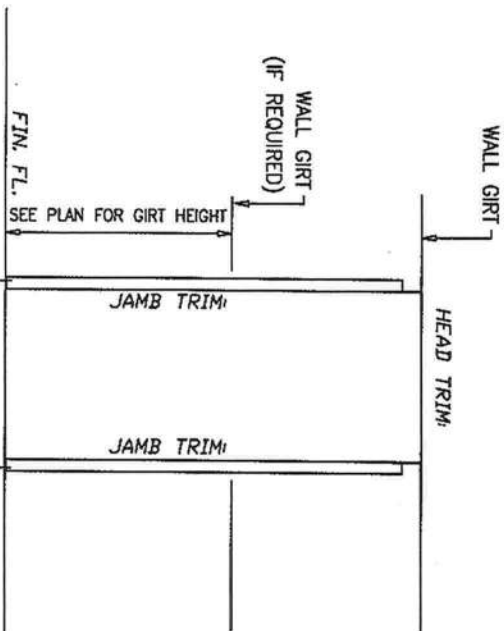
MEMBER TABLE		
FRAME LINE 1 & 4		
MARK	PART	LENGTH
EC-1	8X35C16	14'-5 15/16"
EC-2	8X35C12	15'-4 15/16"
EC-3	8X35C12	15'-4 15/16"
EC-4	8X35C16	14'-5 15/16"
ER-1	8X35C14	20'-0 9/16"
ER-2	8X35C14	20'-0 9/16"
ER-3	8X35C14	20'-0 9/16"
ER-4	8X35C14	20'-0 9/16"
DJ-1	8X35C16	15'-7 15/16"
DJ-2	8X25C16	7'-4"
DH-1	8X35C16	10'-0"
G-1	8X25Z16	10'-7 1/2"
G-2	8X25Z16	2'-7 1/2"
G-3	8X25Z16	15'-11 1/2"
CB-1	1/4 CBL	18'-9"
CB-2	1/4 CBL	18'-1"
CB-3	1/4 CBL	22'-2"



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 4



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BUCK STEEL, INC.

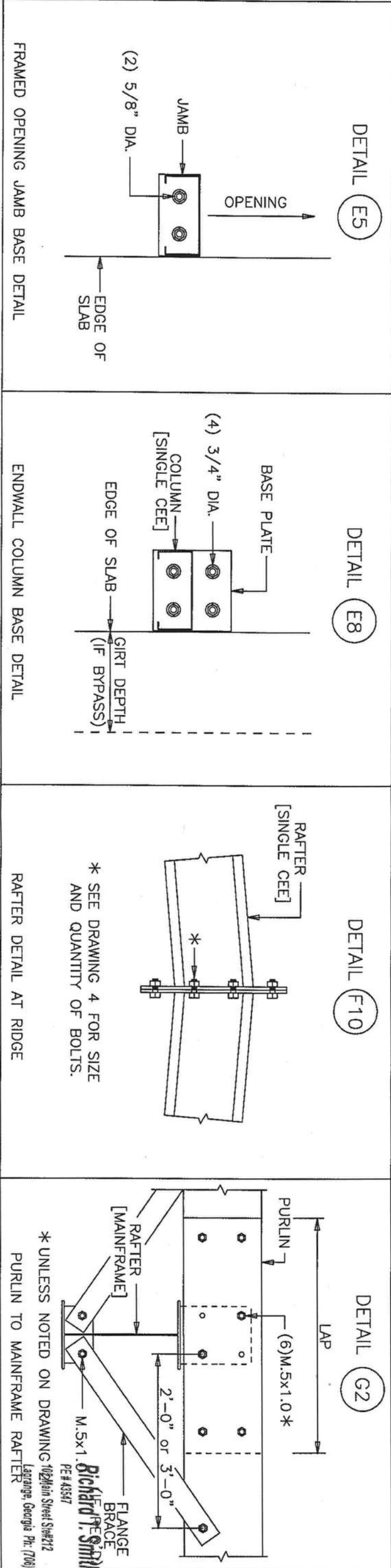
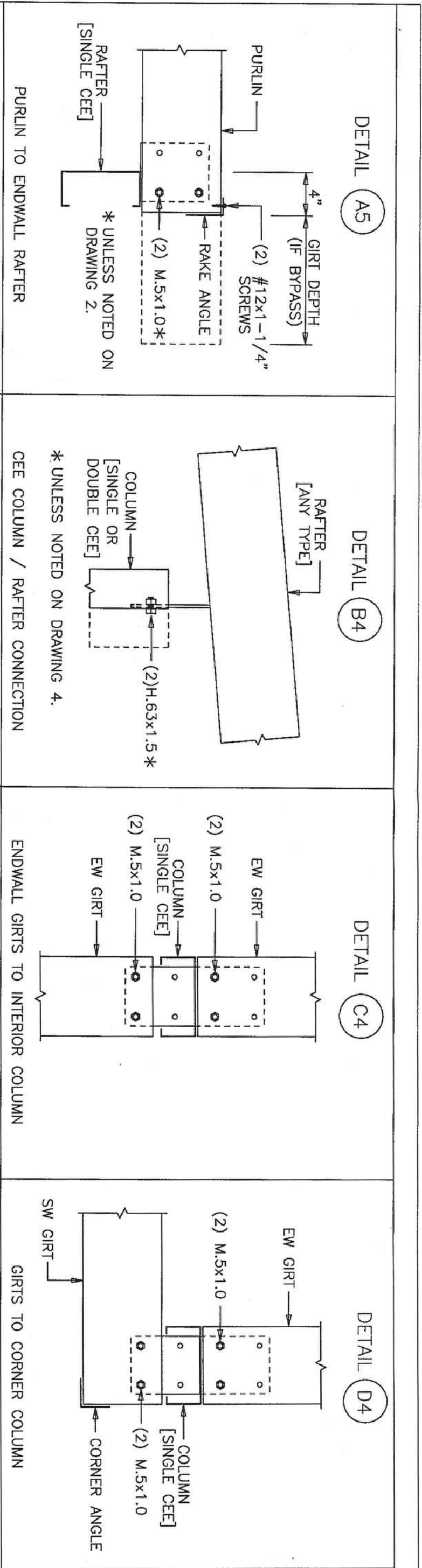
CUSTOMER:
RON JUSTICE

JOB NO.: 1194R2
DATE: 3/17/09

LOCATION: LAKE CITY, FL

DRAWING NAME: ENDWALL FRAMING LAYOUT

DRAWING NO.: PAGE 4
DRAWN BY: JDH
CHECKED BY: BHM



REVISIONS		CUSTOMER: BUCK STEEL, INC.	
[1]	DATE: 3/17/09	JOB NO: 1194R2	
[2]	DATE: 3/17/09	LOCATION: LAKE CITY, FL	
[3]	DATE: 3/17/09	DRAWING NO: PAGE 5	
DRAWING NAME: FRAMING DETAILS		DRAWN BY: JDH	
SCALE: NONE		CHECKED BY: BHM	
STRUCTURAL STAMP			

REVIEWED

By Richard at 4:27 pm, Mar 17, 2009

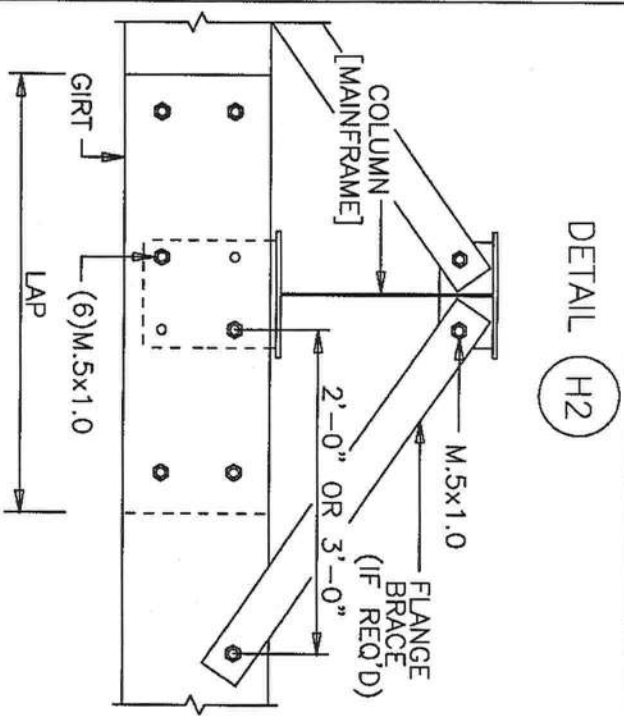
Richard I. Smith

PE # 43547

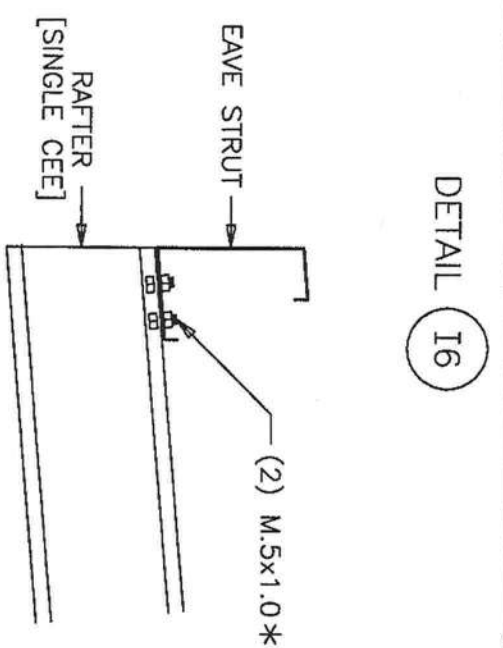
102 Main Street, Ste 212

Lagrange, Georgia 30240

Ph: (706) 898-4674

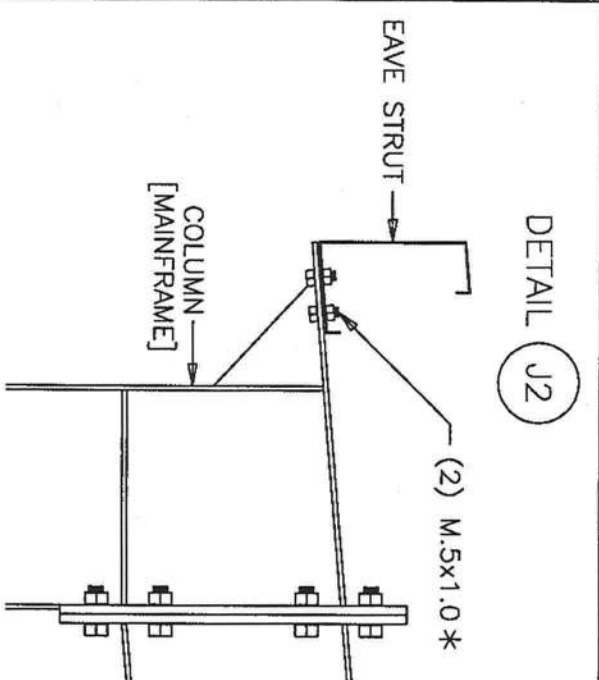


GIRT TO MAINFRAME COLUMN



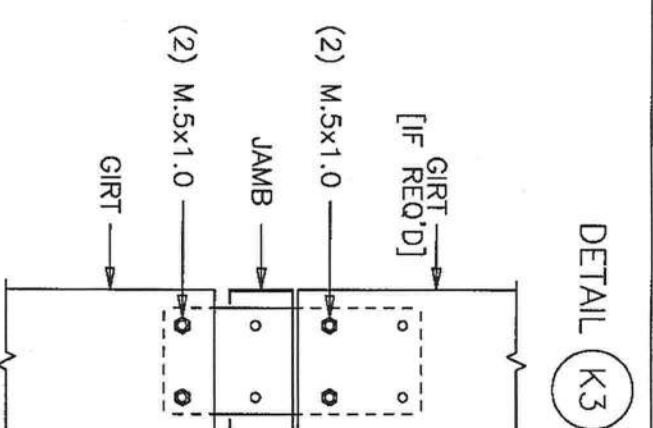
* UNLESS NOTED ON DRAWING 2.

EAVE STRUT CONNECTION AT ENDWALL

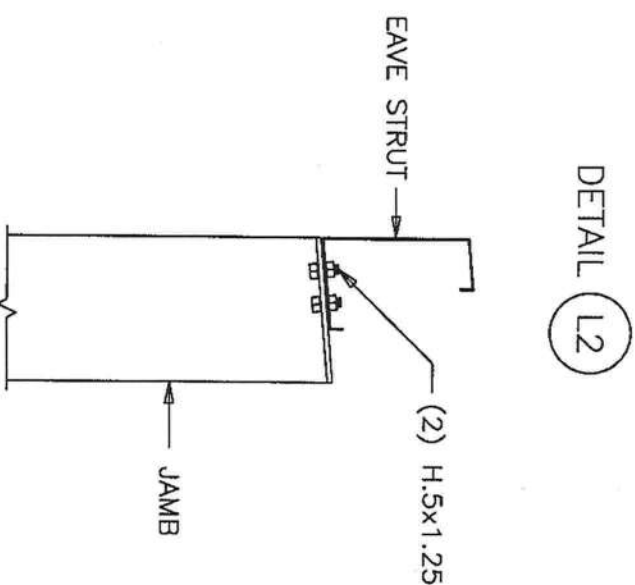


* UNLESS NOTED ON DRAWING 2.

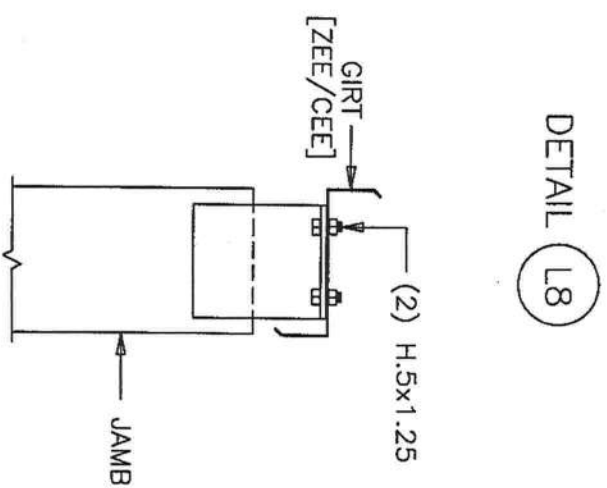
EAVE STRUT CONNECTION AT MAINFRAME



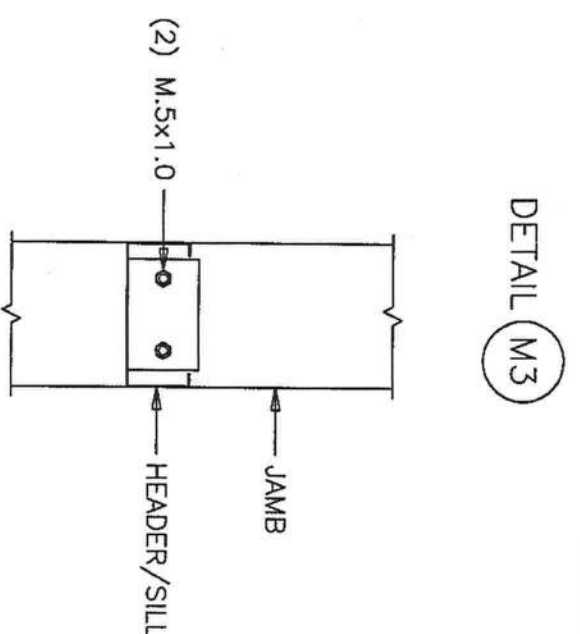
GIRTS TO JAMB



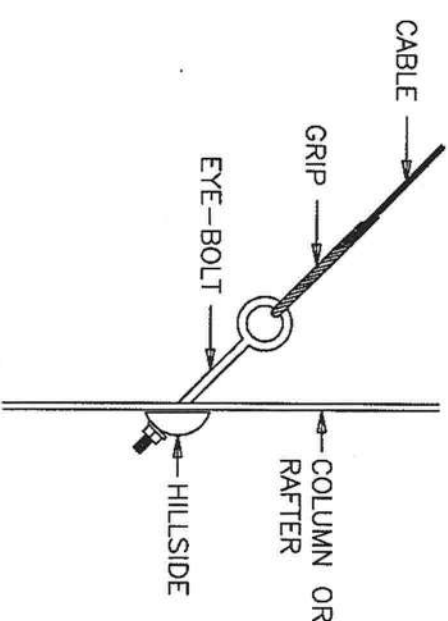
FRAMED OPENING JAMB TO EAVE STRUT



FRAMED OPENING JAMB TO GIRT



FRAMED OPENING HEADER/SILL TO JAMB



CABLE INSTALLATION DETAIL

Richard T. Smith
102 Main Street Ste#212
Lagrange, Georgia Ph: (706) 888-4874

REVIEWED
By Richard at 4:27 pm, Mar 17, 2009

BUCK STEEL, INC.

CUSTOMER:
RON JUSTICE

REVISIONS

JOB NO:
1194R2

LOCATION:
LAKE CITY, FL

DRAWING NAME:
FRAMING DETAILS

DRAWING NO:
PAGE 5.1

DATE:
3/17/09

SCALE:
NONE

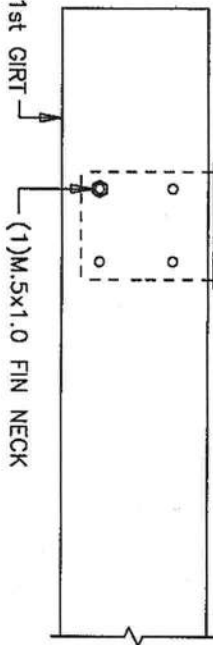
CHECKED BY:
BHM

DRAWN BY:
JDH

STRUCTURAL STAMP

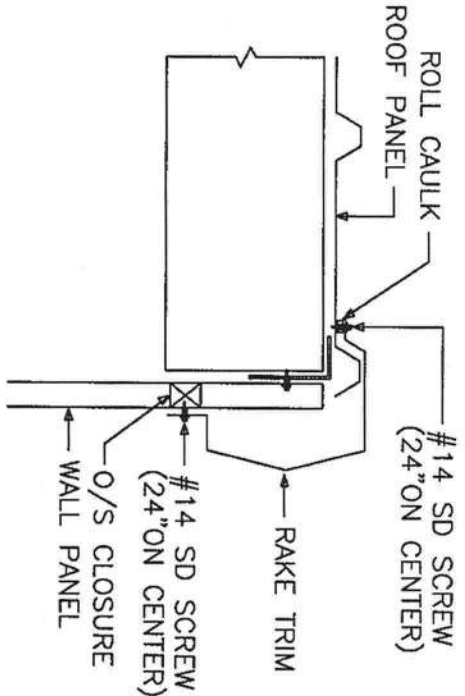
OSHA REQUIREMENT

NOTE: THE FIRST GIRT AT A LAPPED CONDITION MUST BE SECURED TO THE COLUMN WITH (1)M.5x1.0 FIN NECK BOLT W/ NUT BEFORE INSTALLING THE SECOND GIRT AND STANDARD CONNECTION BOLTS.



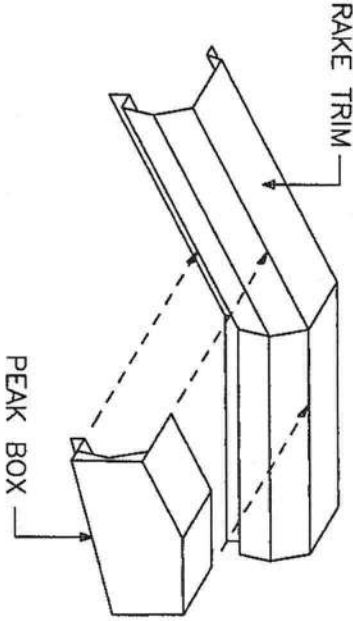
* Directive Number: CPL 2-1.34
* Title: Inspection policy and procedures for OSHA's steel erection standards for construction
* Standard Number: 1926
* Information Date: 03/22/2002

LAPPED GIRTS TO MAINFRAME COLUMN



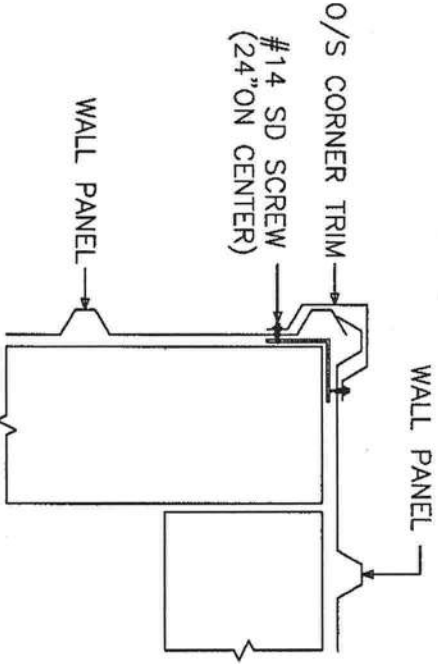
TRIM_3

RAKE TRIM DETAIL



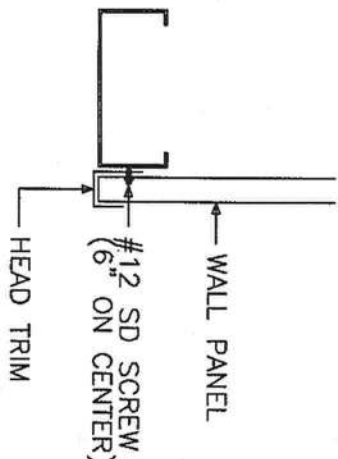
TRIM_4

PEAK BOX DETAIL



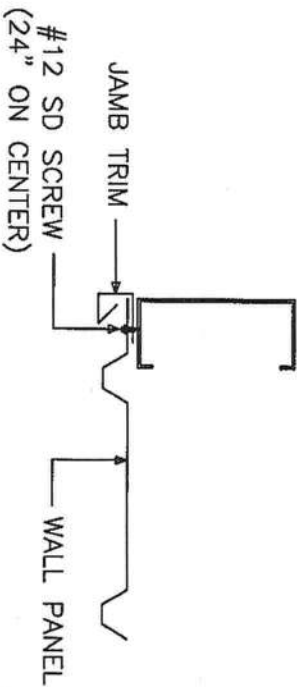
TRIM_5

O/S CORNER DETAIL



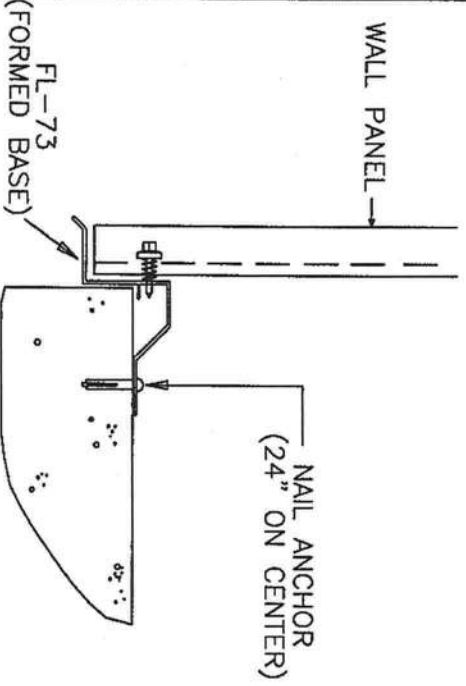
TRIM_6

HEAD TRIM DETAIL AT HEADER



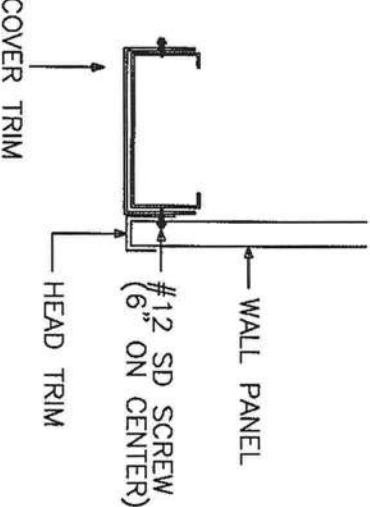
TRIM_8

JAMB TRIM DETAIL AT JAMB



TRIM_9

FORMED BASE DETAIL



TRIM_10

COVER TRIM DETAIL AT HEADER

REVIEWED
By Richard at 4:27 pm, Mar 17, 2009

BUCK STEEL, INC.

CUSTOMER:
RON JUSTICE

JOB NO.:
1194R2

LOCATION:
LAKE CITY, FL

DRAWING NAME:
FRAMING DETAILS

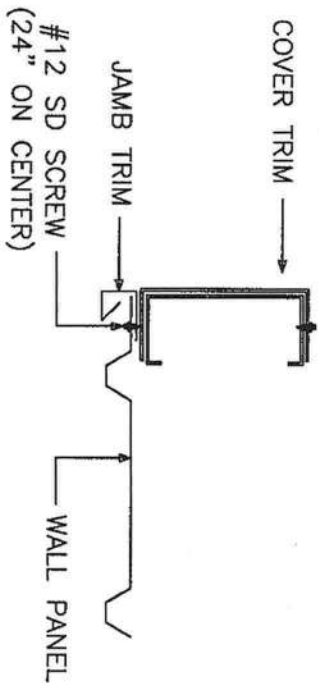
DRAWING NO.:
PAGE 5.2

DRAWN BY:
JDH

CHECKED BY:
BHM

STRUCTURAL STAMP





TRIM_11

COVER TRIM DETAIL AT JAMB

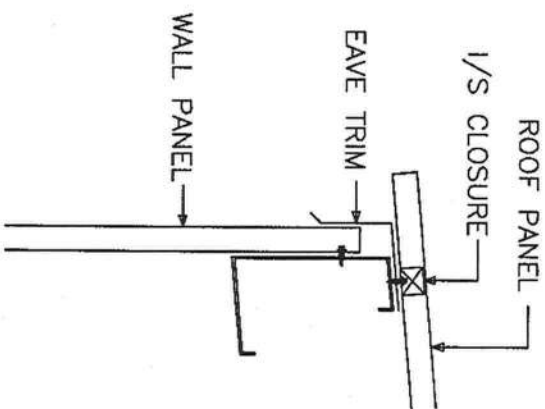
NOTE: THE PROPER TIGHTENING AND INSPECTION OF ALL FASTENERS IS THE RESPONSIBILITY OF THE ERECTOR. ALL HEAVY STRUCTURAL (A325/A490) BOLTS AND NUTS MUST BE TURNED BY THE TURN-OF-NUT METHOD SHOWN BELOW. A325 AND A490 BOLTS ARE DESIGNATED BY MARK WITH A "H". (SEE H63x2.0 OR H75x2.75)

TURN-OF-NUT TIGHTENING: BOLTS SHALL BE INSTALLED IN ALL HOLES OF THE CONNECTION AND BROUGHT TO A SNUG-TIGHT CONDITION. SNUG TIGHT IS DEFINED AS THE TIGHTNESS THAT EXISTS WHEN THE FLETS OF THE JOINT ARE IN FIRM CONTACT. THIS MAY BE ACHIEVED BY A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH. SNUG TIGHTENING SHALL PROGRESS SYSTEMATICALLY FROM THE MOST RIGID PART OF THE CONNECTION TO THE FREE EDGES UNTIL ALL BOLTS ARE SIMULTANEOUSLY SNUG TIGHT AND THE CONNECTION IS FULLY COMPRESSED. FOLLOWING THIS INITIAL OPERATION, ALL BOLTS IN THE CONNECTION SHALL BE TURNED AN ADDITIONAL 1/3 TURN. THE TURN-OF-NUT METHOD SPECIFIED IN THE TABLE BELOW. DURING THE OPERATION THERE SHALL BE NO ROTATION OF THE PART NOT TURNED BY THE WRENCH. TIGHTENING SHALL PROGRESS SYSTEMATICALLY FROM THE MOST RIGID PART OF THE JOINT TO THE FREE EDGES.

NUT ROTATION FROM SNUG-TIGHT CONDITION

BOLT LENGTH UP TO AND INCLUDING 4 DIAMETERS	REQUIRED ROTATION
OVER 4 DIAMETERS BUT NOT EXCEEDING 8 DIAMETERS	1/2 TURN
OVER 8 DIAMETERS BUT NOT EXCEEDING 12 DIAMETERS	2/3 TURN

NOTES: [1] NUT ROTATION IS RELATIVE TO BOLT OR BOLT BEING TURNED. [2] APPLICABLE ONLY TO CONNECTIONS IN WHICH ALL MATERIAL WITHIN THE GRIP OF THE BOLT IS STEEL.

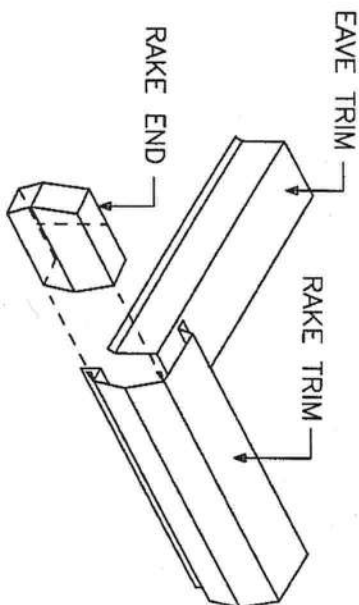


TRIM_12

EAVE TRIM DETAIL

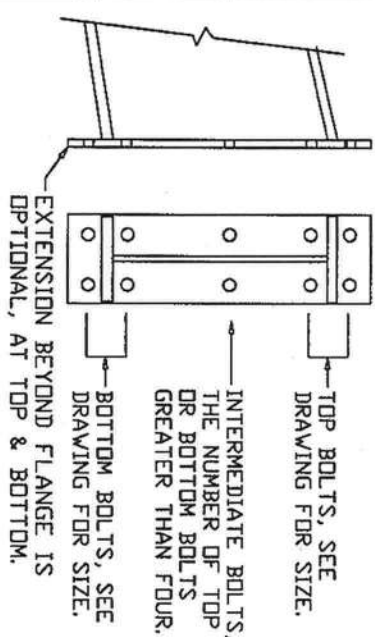
TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK.
- [2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- [3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- [4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.



TRIM_13

RAKE END DETAIL



U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK

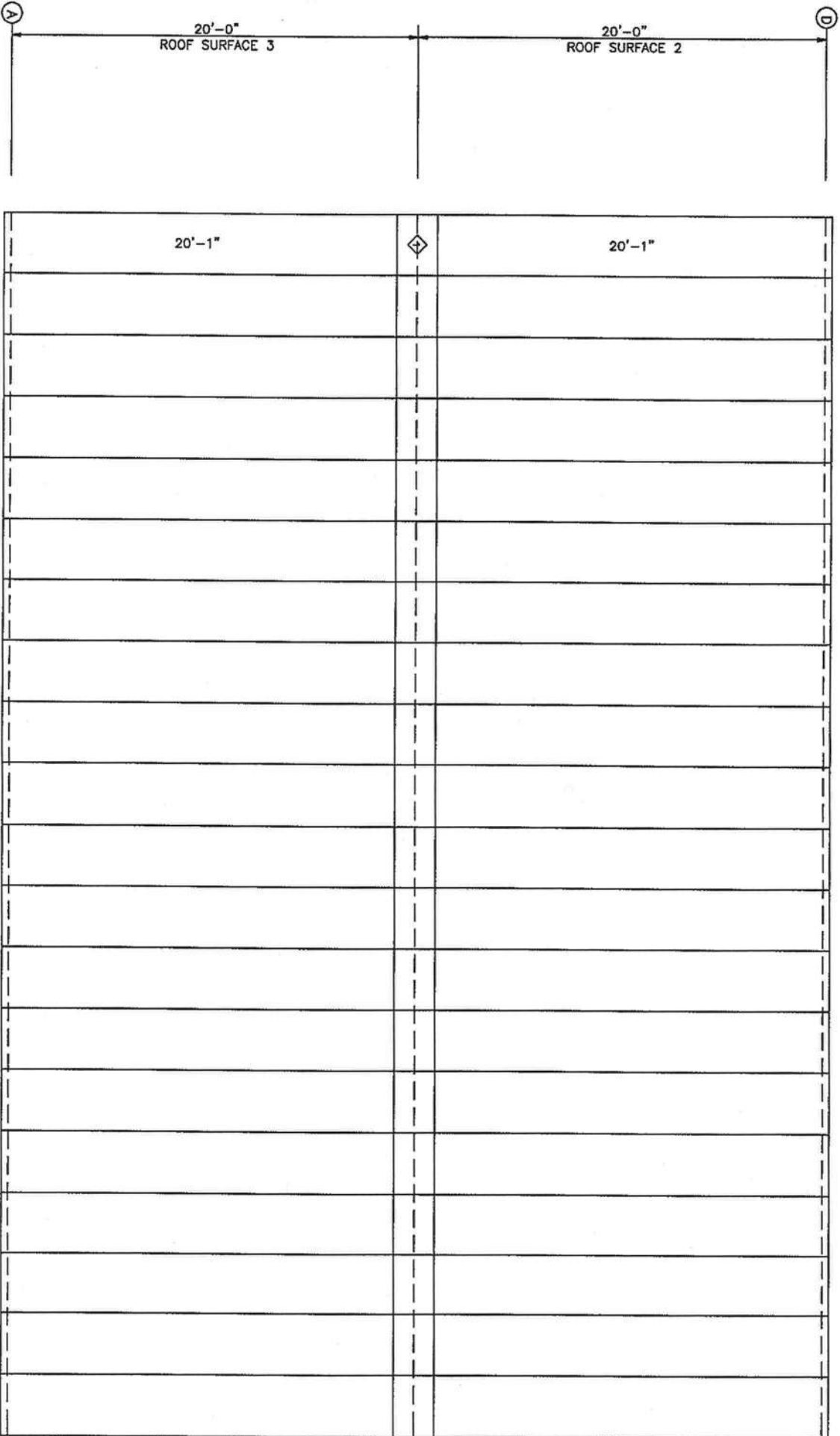
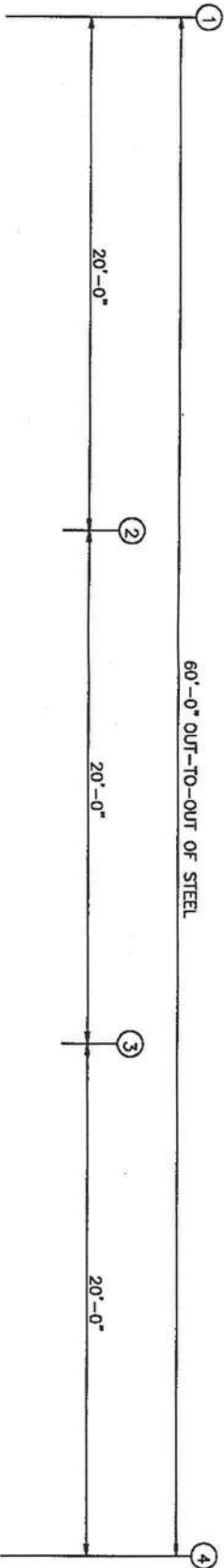
BUCK STEEL, INC.			
CUSTOMER: RON JUSTICE			
JOB NO: 1194R2		DATE: 3/17/09	
REVISIONS	LOCATION: LAKE CITY, FL		
[1]	DRAWING NAME: FRAMING DETAILS		
[2]	DRAWING NO: PAGE 5.3		DRAWN BY: JDH
[3]			CHECKED BY: BHM

Richard T. Smith
PE #43547
102 Main Street Ste#112
Lagrange, Georgia Pk: (706) 888-4874

REVIEWED
By Richard at 4:27 pm, Mar 17, 2009

STRUCTURAL STAMP

TRIM TABLE		
ROOF PLAN		
QID	PART	LENGTH
1	D/F CAP	3'-0"



ROOF SHEETING PLAN
PANELS: 26 GA. PBR -- GALVALUME

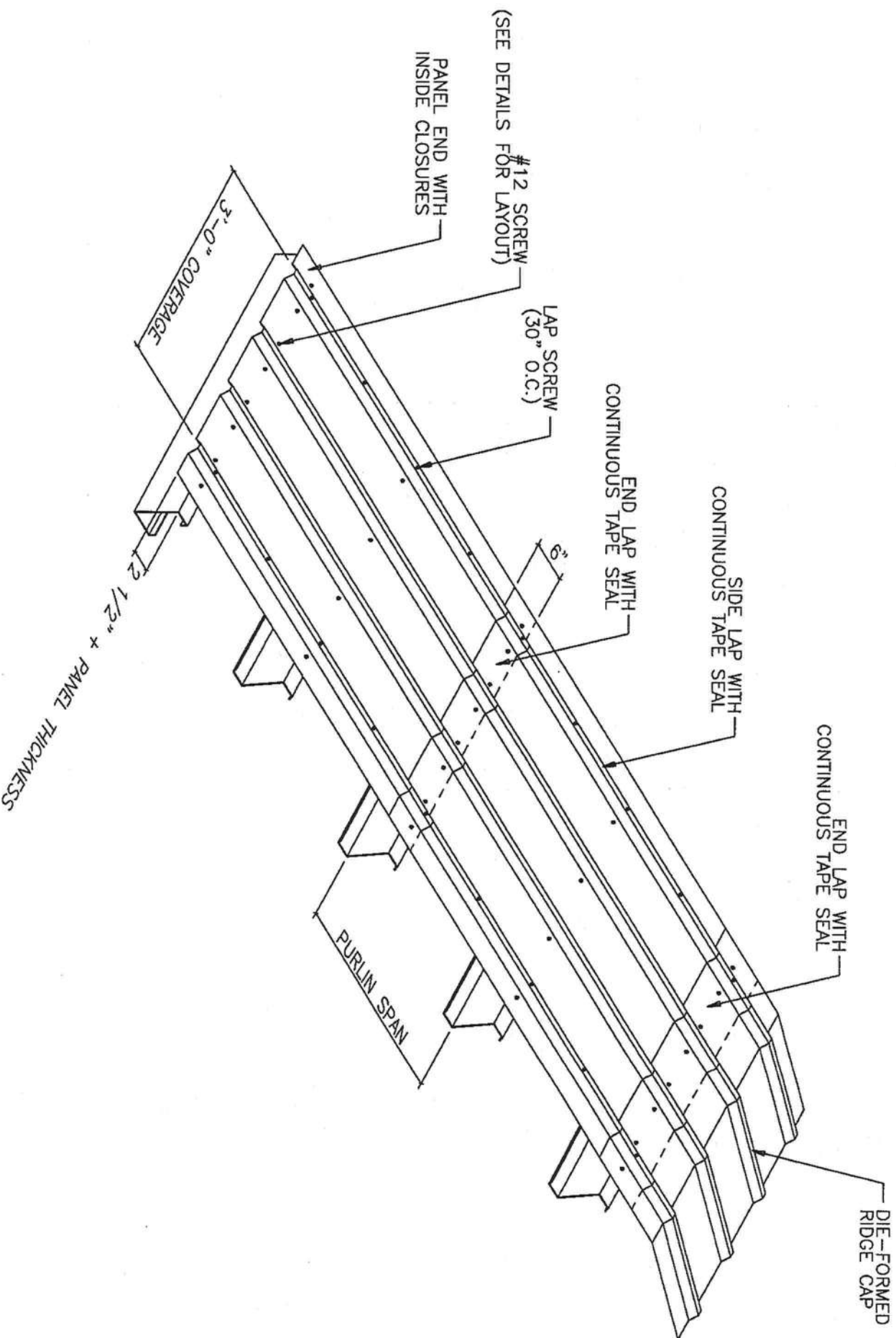
BUCK STEEL, INC.	
CUSTOMER: RON JUSTICE	
JOB NO: 1194R2	DATE: 3/17/09
LOCATION: LAKE CITY, FL	
DRAWING NAME: ROOF PANELS & TRIM	
DRAWING NO: PAGE 6	DRAWN BY: JDH
SCALE: NONE	
CHECKED BY: BHM	

STRUCTURAL STAMP

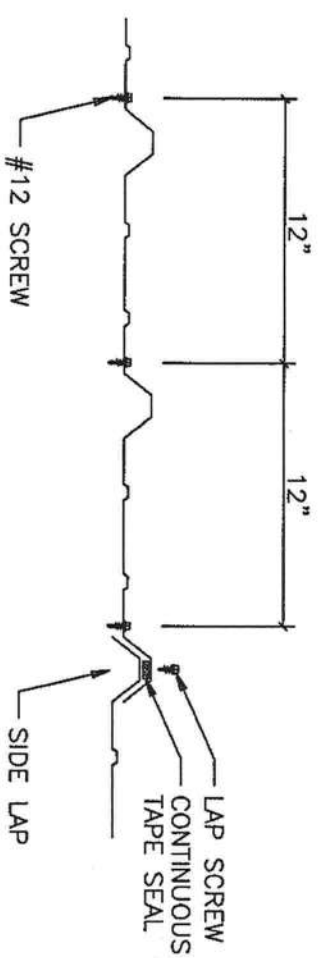
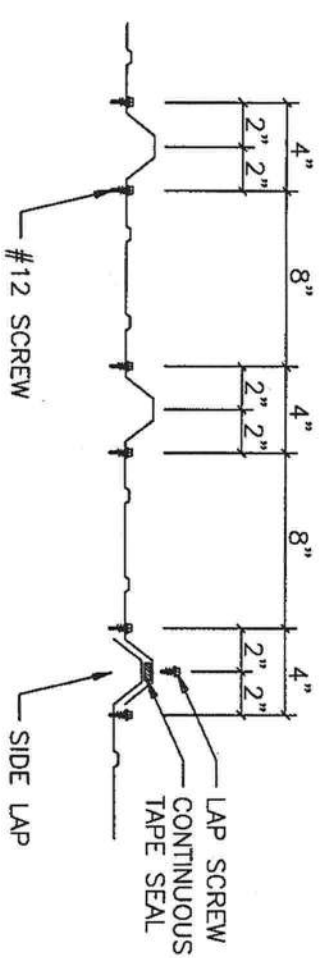
REVIEWED
By Richard at 4:27 pm, Mar 17, 2009

Richard T. Smith
PE # 43547
102 Main Street Ste#212
Lagrange, Georgia Ph: (706) 888-4874





DETAIL AT PANEL END



DETAIL AT INTERIOR OF PANEL

Richard T. Smith

PE # 43547

102 Main Street Ste#212

Lagrange, Georgia Pk: (706) 888-4814

REVIEWED

By Richard at 4:27 pm, Mar 17, 2009

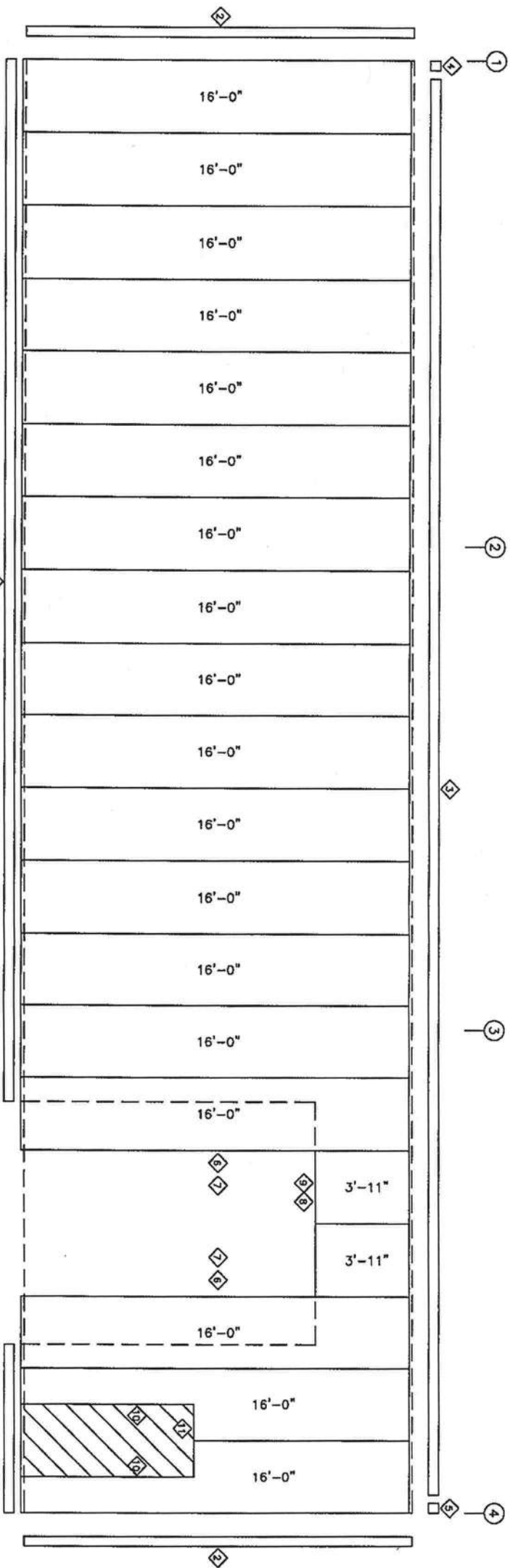
NOTES:

- [1] ALL END LAPS MUST BE A MINIMUM OF 6".
- [2] METAL SHAVINGS MUST BE SWEEP FROM THE ROOF EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.
- [4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

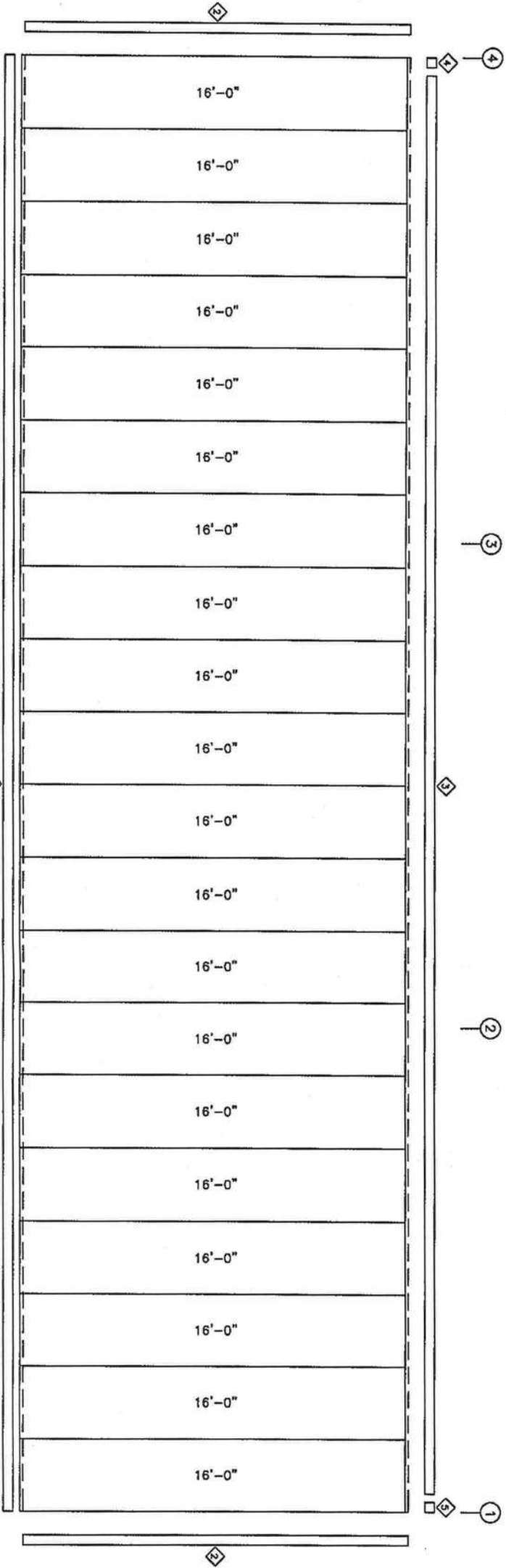
BUCK STEEL, INC.			
CUSTOMER: RON JUSTICE			
JOB NO: 1194R2		DATE: 3/17/09	
[1]	LOCATION: LAKE CITY, FL		
[2]	DRAWING NAME: ROOF PANEL DETAILS		
[3]	DRAWING NO: PAGE 6.1	DRAWN BY: JDH	SCALE: NONE CHECKED BY: BHM

STRUCTURAL STAMP

TRIM TABLE			
FRAME LINE A & D			
ID	PART	LENGTH	DETAIL
1	FL-73	20'-0"	TRIM_9
2	O/S CORN	16'-0"	TRIM_5
3	EAVE TRM	20'-3"	TRIM_12
4	R END LH	6"	TRIM_13
5	R END RH	6"	TRIM_13
6	CT8	12'-1"	TRIM_11
7	R JAMB	12'-3"	TRIM_8
8	CT8	10'-0"	TRIM_10
9	R HEAD	10'-3"	TRIM_6
10	R JAMB	7'-3"	TRIM_8
11	R HEAD	3'-3"	TRIM_6



SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 GA. PBR - LIGHT STONE



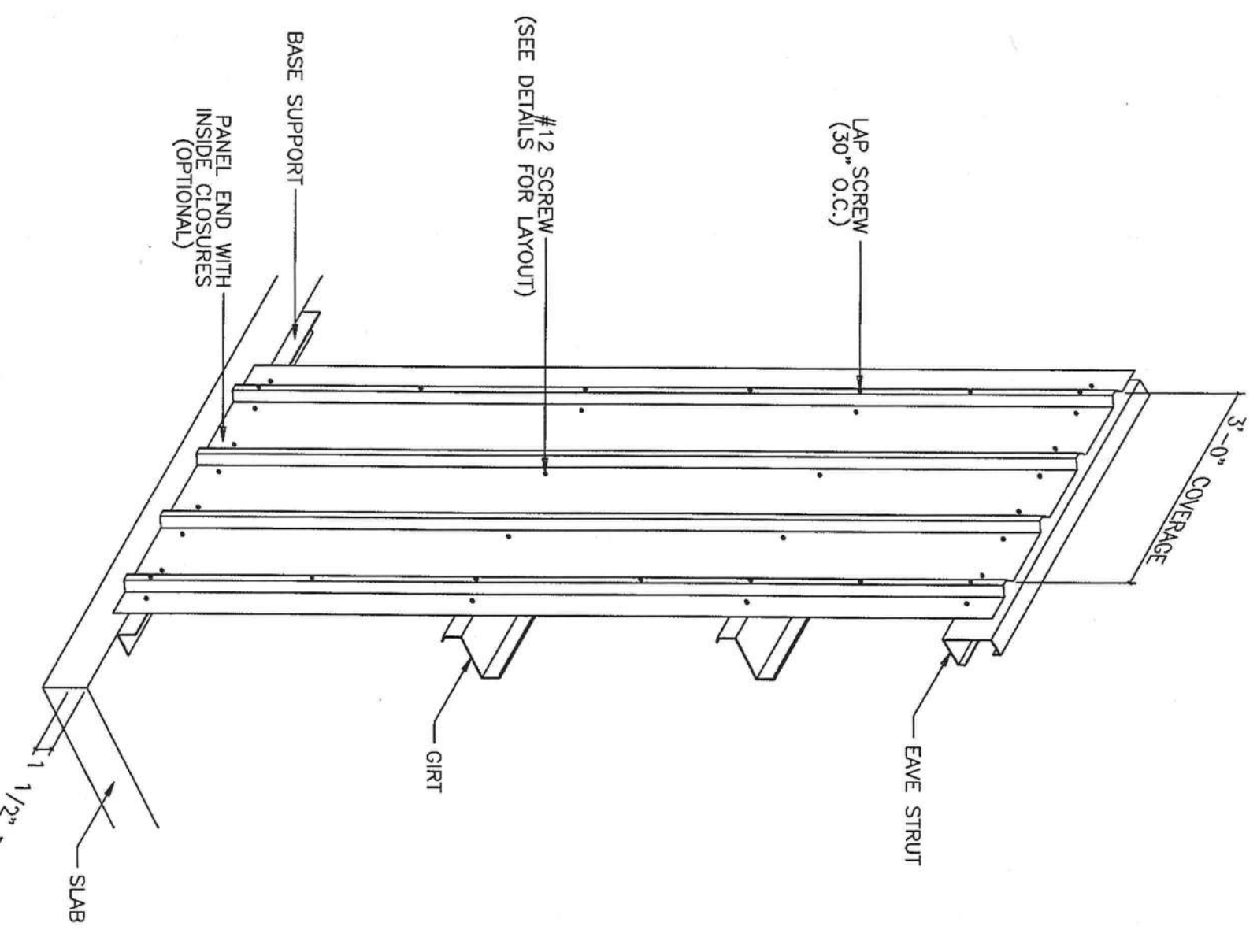
SIDEWALL SHEETING & TRIM: FRAME LINE D
PANELS: 26 GA. PBR - LIGHT STONE

BUCK STEEL, INC.			
CUSTOMER: RON JUSTICE			
JOB NO: 1194R2		DATE: 3/17/09	
[1]	LOCATION: LAKE CITY, FL		
[2]	DRAWING NAME: SIDEWALL PANELS & TRIM		SCALE: NONE
[3]	DRAWING NO: PAGE 7	DRAWN BY: JDH	CHECKED BY: BHM

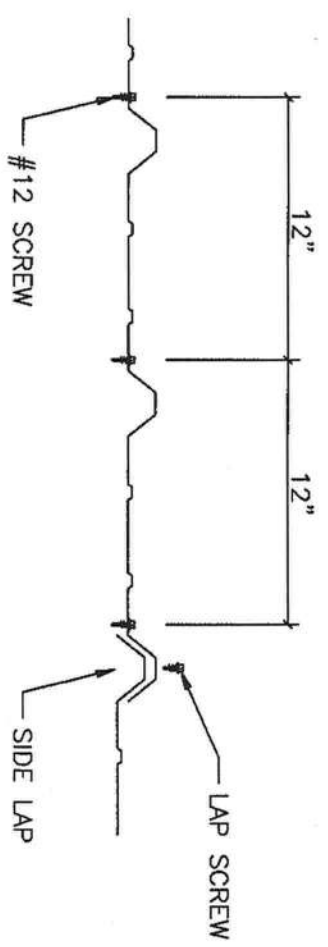
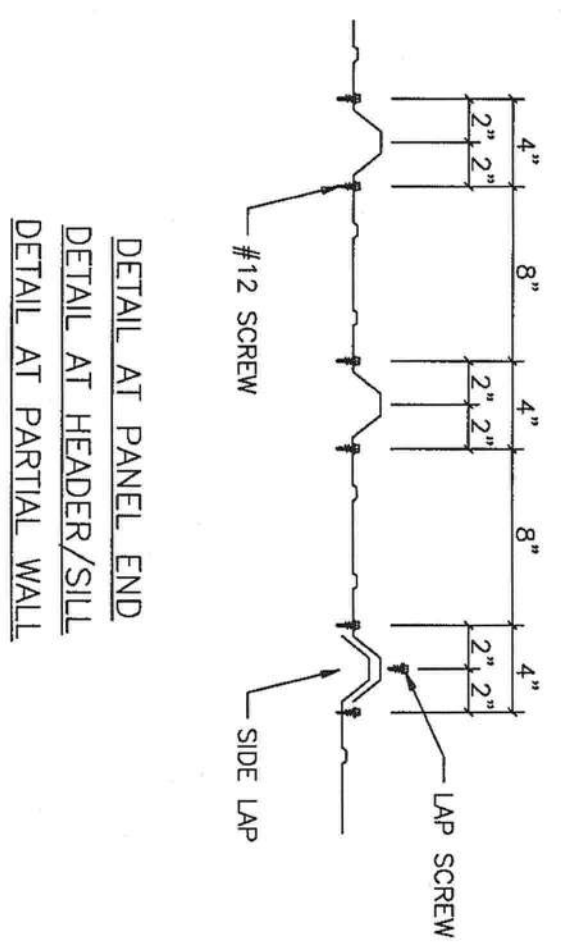
Richard T. Smith
PE # 43547
102 Main Street Ste#212
Lagrange, Georgia Pk: (706) 888-4874

REVIEWED
By Richard at 4:27 pm, Mar 17, 2009

STRUCTURAL STAMP



- NOTES:
- [1] METAL SHAVINGS MUST BE SWEEP FROM THE ROOF EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
 - [2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

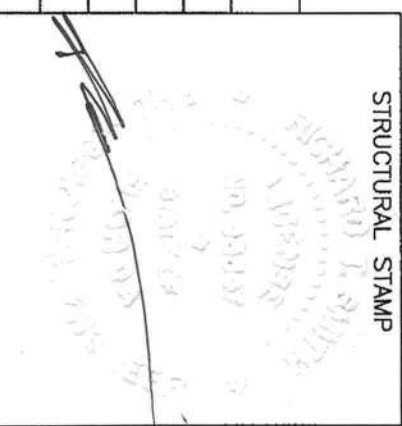


DETAIL AT INTERIOR OF PANEL

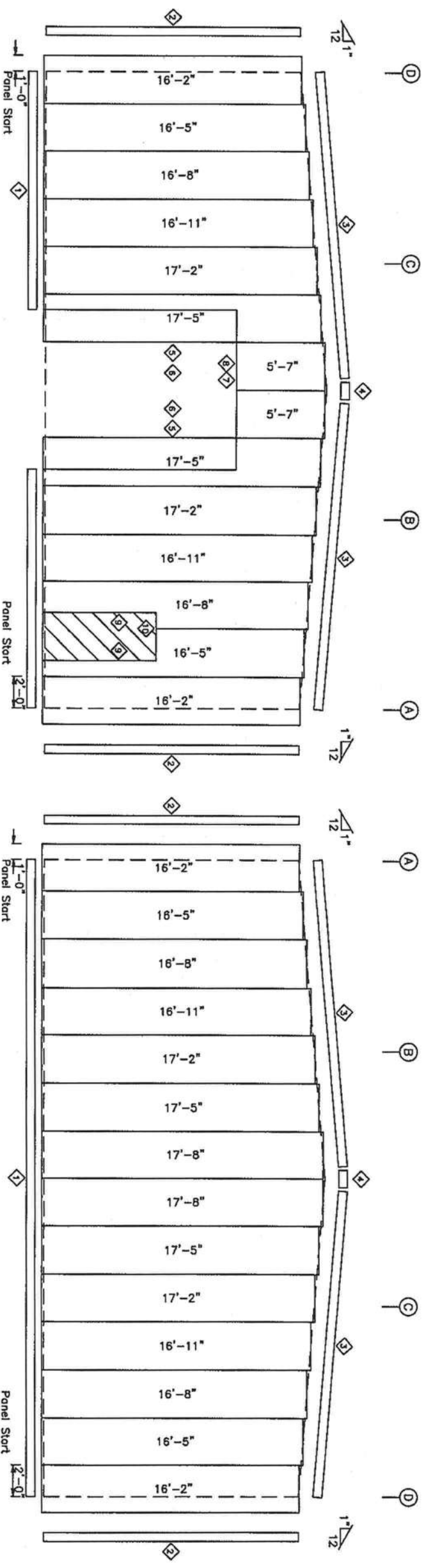
BUCK STEEL, INC.			
CUSTOMER: RON JUSTICE			
JOB NO: 1194R2		DATE: 3/17/09	
LOCATION: LAKE CITY, FL			
DRAWING NAME: SIDEWALL PANEL DETAILS			
DRAWING NO: PAGE 7.1			
DRAWN BY: JDH			
CHECKED BY: BHM			

Richard T. Smith
PE # 43547
102 Main Street Ste#212
Lagrange, Georgia Ph: (706) 898-4374

REVIEWED
By Richard at 4:27 pm, Mar 17, 2009



TRIM TABLE			
FRAME LINE 1 & 4			
QID	PART	LENGTH	DETAIL
1	FL-73	20'-0"	TRIM_9
2	O/S CORN	16'-0"	TRIM_5
3	RAKE TRM	20'-1"	TRIM_3
4	PEAK BOX	1'-4"	TRIM_4
5	CT8	12'-1"	TRIM_11
6	R JAMB	12'-3"	TRIM_8
7	CT8	10'-0"	TRIM_10
8	R HEAD	10'-3"	TRIM_6
9	R JAMB	7'-3"	TRIM_8
10	R HEAD	5'-3"	TRIM_6



ENDWALL SHEETING & TRIM: FRAME LINE 1

ENDWALL SHEETING & TRIM: FRAME LINE 4

PANELS: 26 GA. PBR - LIGHT STONE

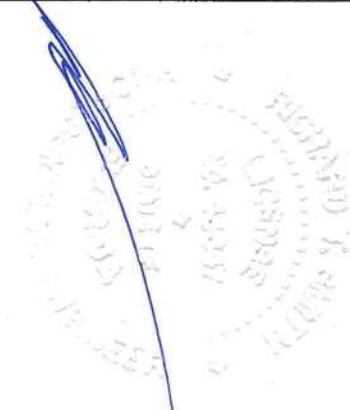
PANELS: 26 GA. PBR - LIGHT STONE

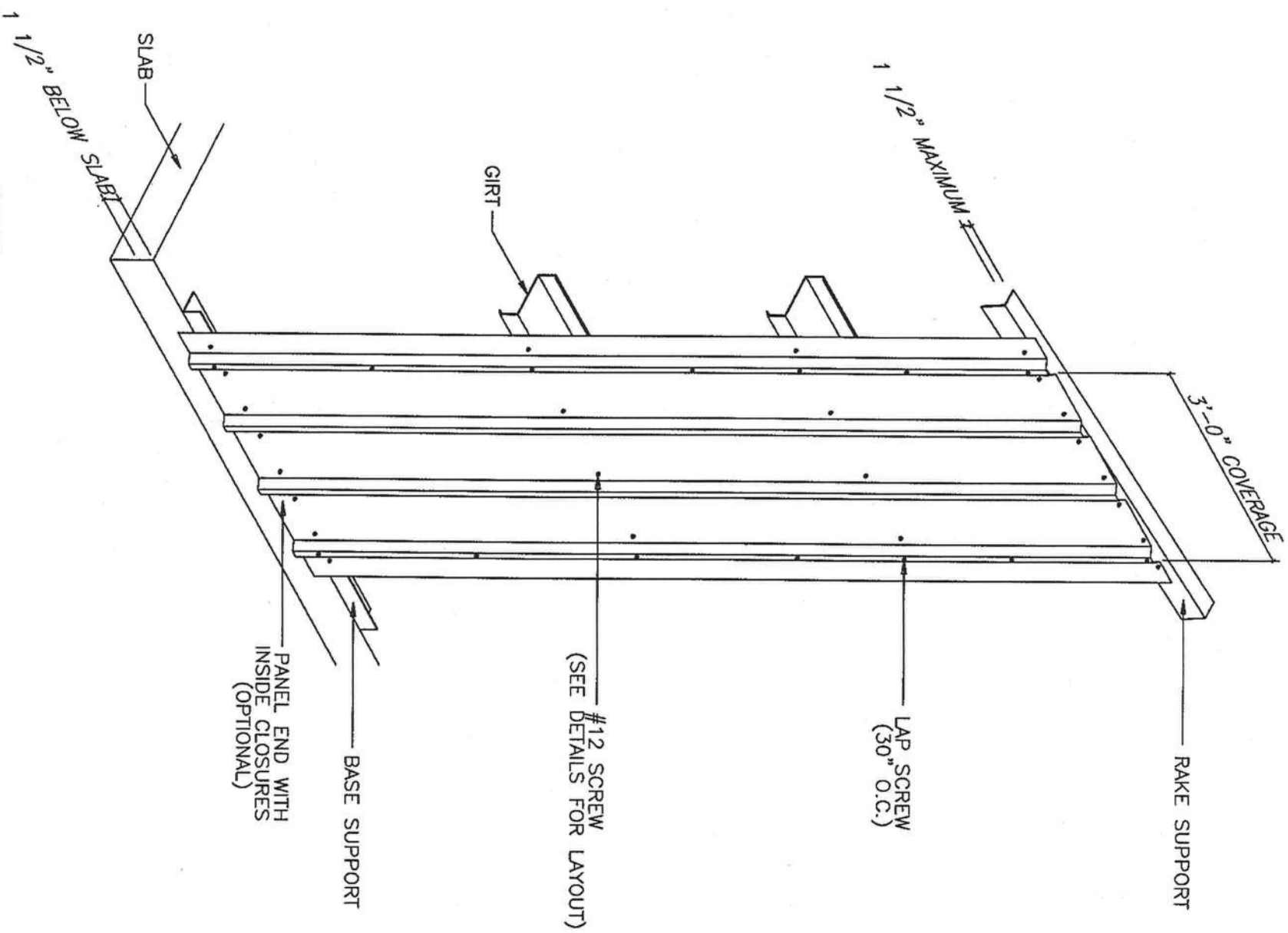
Richard T. Smith
PE # 43547
102 Main Street Ste#112
Lagrange, Georgia Ph: (706) 888-4874

REVIEWED
By Richard at 4:27 pm, Mar 17, 2009

BUCK STEEL, INC.	
CUSTOMER:	RON JUSTICE
JOB NO:	1194R2
DATE:	3/17/09
LOCATION:	LAKE CITY, FL
DRAWING NAME:	ENDWALL PANELS & TRIM
DRAWING NO:	PAGE 8
DRAWN BY:	JDH
CHECKED BY:	BHM
SCALE:	NONE

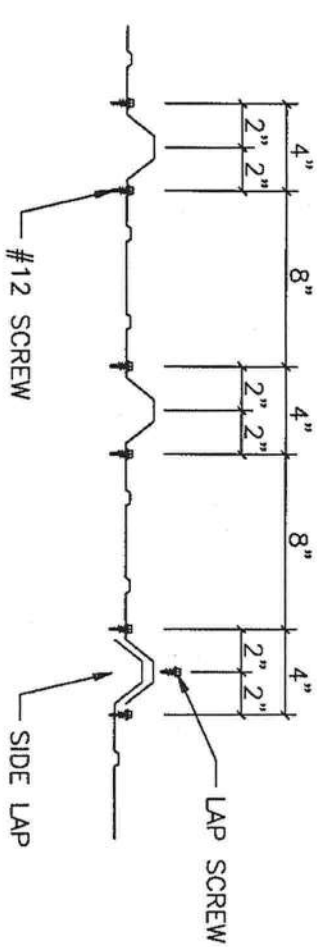
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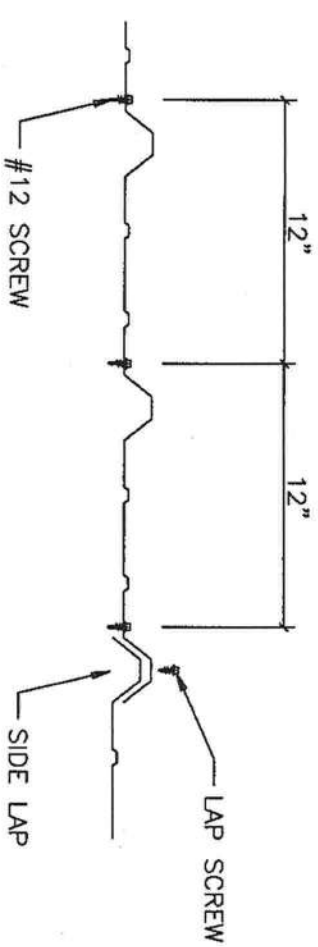


NOTES:

- [1] METAL SHAVINGS MUST BE SWEEP FROM THE ROOF EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.



DETAIL AT PANEL END
DETAIL AT HEADER/SILL
DETAIL AT PARTIAL WALL



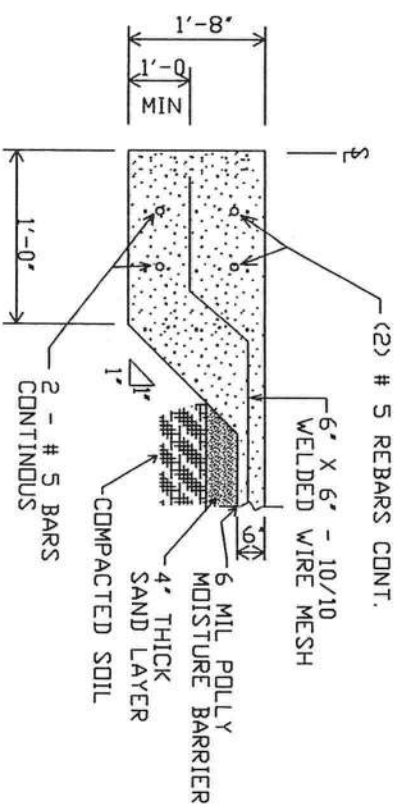
DETAIL AT INTERIOR OF PANEL

BUCK STEEL, INC.			
CUSTOMER:			
RON JUSTICE			
JOB NO:		DATE:	
1194R2		3/17/09	
REVISIONS			
[1]	LOCATION:		
	LAKE CITY, FL		
[2]	DRAWING NAME:		SCALE:
	ENDWALL PANEL DETAILS		NONE
[3]	DRAWING NO:	DRAWN BY:	CHECKED BY:
	PAGE 8.1	JDH	BHM

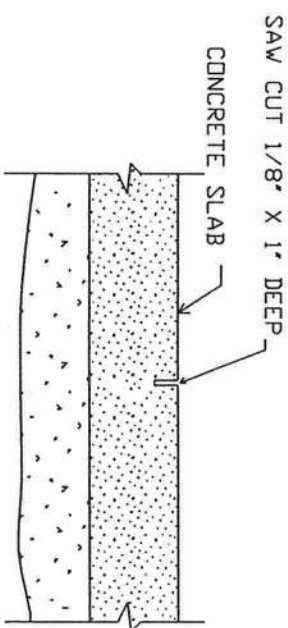
Richard T. Smith
PE # 43547
102 Main Street Ste#212
Lagrange, Georgia Ph: (706) 888-4874

REVIEWED
By Richard at 4:27 pm, Mar 17, 2009

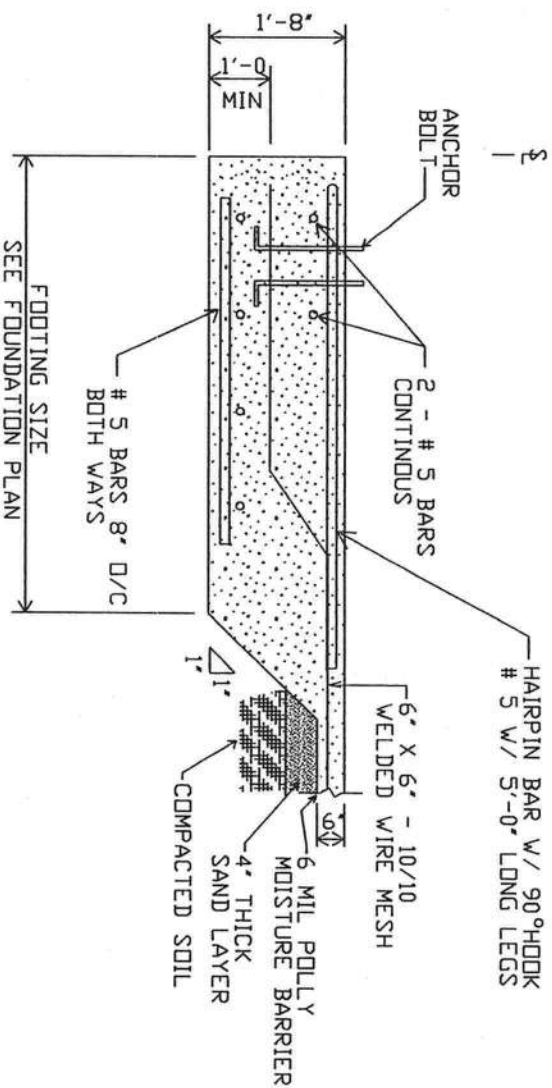




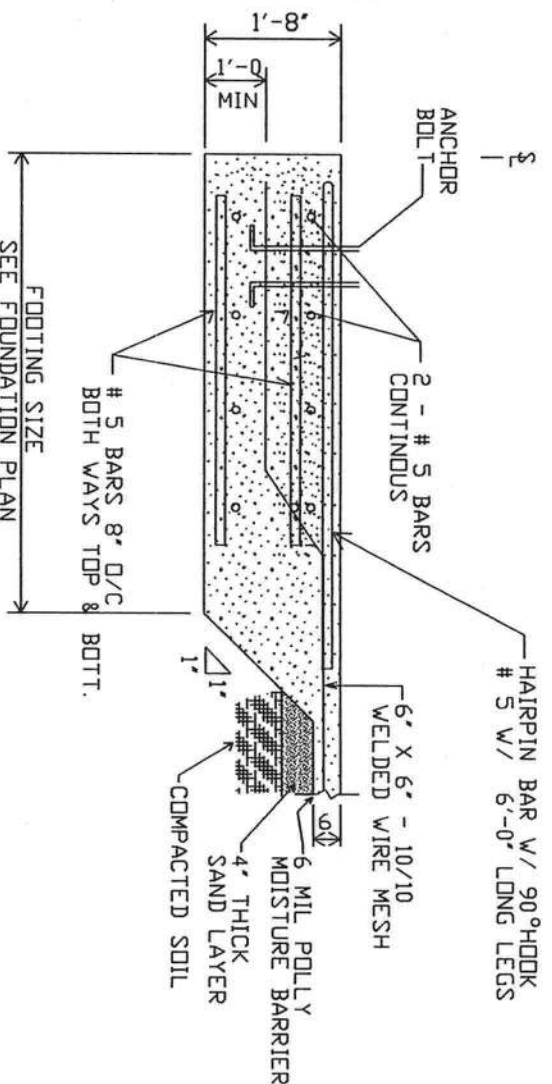
SECT. B-B
NOT TO SCALE



SECT. D-D (Const. Joint)
NOT TO SCALE



SECT. A-A
NOT TO SCALE



SECT. F-F
NOT TO SCALE

- GENERAL FOUNDATION NOTES
1. ALL REINFORCING STEEL SHALL BE GRADE 60 DEFORMED BARS.
 2. ALL POURED IN PLACE SHALL BE F'C 3000 PSI MIN @ 28 DAYS.
 3. WELDED WIRE MESH SHALL MEET ASTM A-185. FOUNDATION AND FOOTING SIZING BASED ON ASSUMED SOIL BEARING CAPACITY OF 2500 PSF.
 4. THE UPPER 12" OF BEARING SOIL IN FOOTING SHALL BE COMPACTED TO 98% OF THE STANDARD PROCTOR.
 5. MIN. REINFORCING STEEL COVER AT EARTH : 3"
 6. SLUMP RANGE AT POINT OF DISCHARGE: 3'-6"
 7. OVERLAP ALL WVF A MINIMUM OF 8"
 8. LAP ALL REINFORCING STEEL A MINIMUM OF 48 DIAMETERS.
 9. REMOVE TOPSOIL & ORGANIC MATERIAL FROM TOP 12" OF EXISTING GRADE.
 10. FDN. IS DESIGNED TO MEET THE 2004 ED. OF THE FBC CODE. SECT 106.35 W/06 AMEND. W.S. IS 120 MPH. EXP. B

12. PROOF ROLL OF 5' OUTSIDE BUILDING FOOTPRINT WITH VIBRATORY COMPACTOR.
13. FILL TO WITHIN 4" OF FINISHED FLOOR ELEVATION WITH CLEAN SAND FILL.
14. COMPACT TOP 6" OF FILL MATERIAL TO 95% OF MODIFIED PROCTOR DENSITY. (MIN)
15. SAW INDICATED CRACK CONTROL JOINTS WITHIN 8 HOURS OF PLACEMENT OF CONCRETE.
16. SOIL IN FOOTING TRENCHES SHALL BE FREE OF ORGANIC MATERIAL OR CLAY. IF EITHER IS ENCOUNTERED IN FOOTING TRENCHES, REMOVE IT & REPLACE WITH COMPACTED SAND.
17. CONTRACTOR TO REVIEW FOUNDATION DRAWINGS AND CHECK FOR COMPLIANCE WITH ERECTION DRAWINGS BEFORE COMMENCEMENT OF CONSTRUCTION. ANY DISCREPANCY SHOULD BE BROUGHT TO ENGINEER'S ATTENTION.

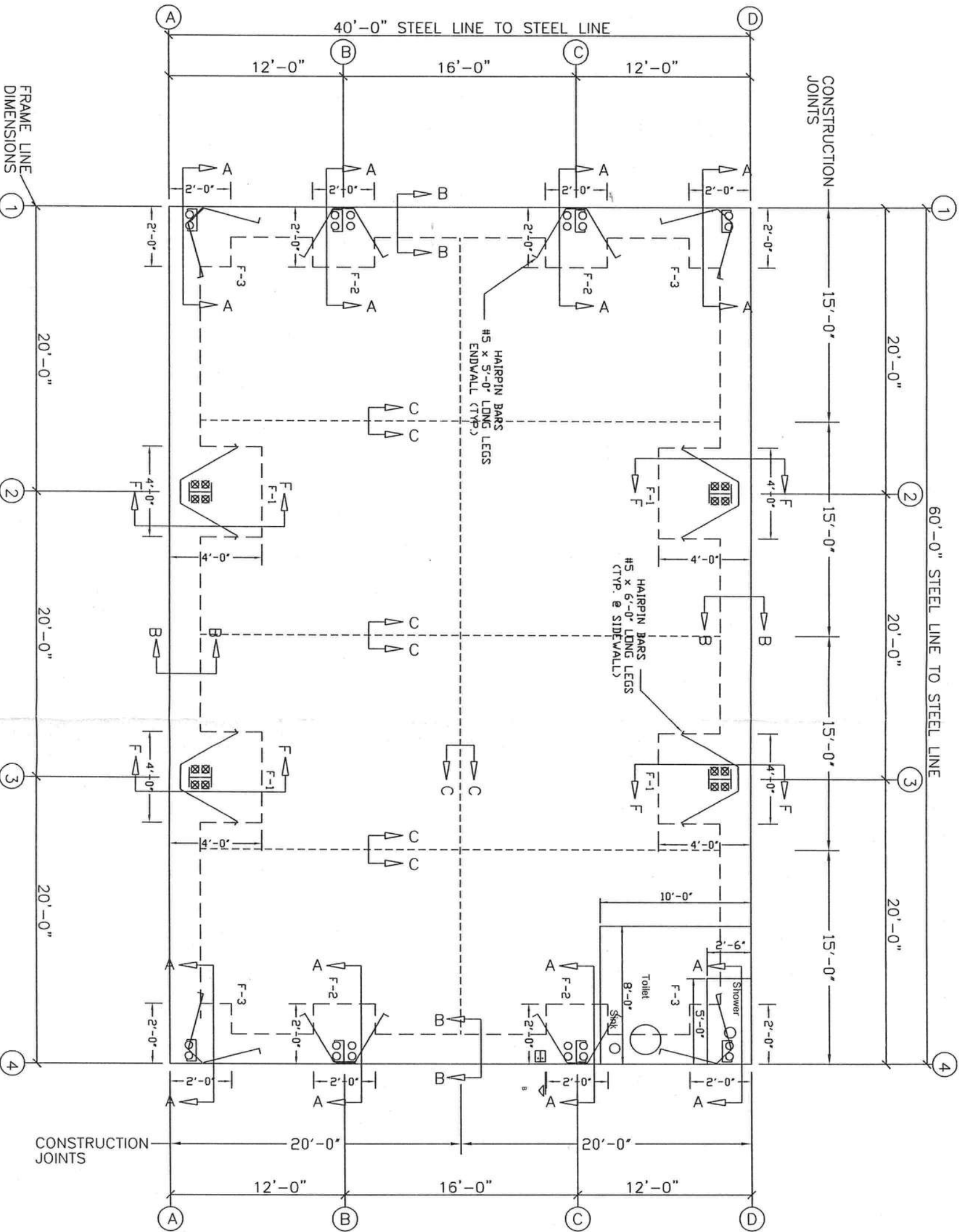
REVISIONS			
REV.	DESCRIPTION	DATE	DLR
1			
DRAWING STATUS			
<input checked="" type="checkbox"/> FOR CONSTRUCTION <input type="checkbox"/> FOR PERMIT ONLY <input type="checkbox"/> FOR APPROVAL <input type="checkbox"/> OTHER, EXPLAIN			
BUCK STEEL, INC.		RON JUSTICE	
PROJECT	400' x 300' x	FOUNDATION DETAIL	PAGE
ID	1194	DESIGN: CE	DRAFT: CE
PROJECT	LAKE CITY, FL	DATE: 3/1/09	SHEET
		FNDWG-2	

SEAL

Not to be used

3/1/09

ANCHOR BOLT SUMMARY						
QNT	LDC	DIA. (IN)	PROJ LENGTH (IN)	TOTAL LENGTH (IN)	BEND (IN)	
Ø 8	DJ	5/8"	3.00	9.0	3.00	
Ø 32	EW	3/4"	3.00	12.0	3.00	
Ø 16	RF	3/4"	3.00	18.0	3.00	



FOUNDATION LAYOUT PLAN

REVISIONS					DRAWING STATUS				
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD	[X] FOR CONSTRUCTION		
							[] FOR PERMIT ONLY		
							[] FOR APPROVAL		
							[] OTHER, EXPLAIN		
BUCK STEEL INC.							PROJECT		
400 x 600 x							ID		
1194							PROJECT		
LAKE CITY, FL							ADDRESS		
RON JUSTICE							FOUNDATION LAYOUT PLAN		
3/4/09							DESIGN CVE		
							DRAFT CVE		
							CHECK		
							DATE: 3/1/09		
							SHEET		
							FNDWG-1		

CHARLES EMBDEN P.E.
26 HARRIS DRIVE
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SEAL

Not to scale