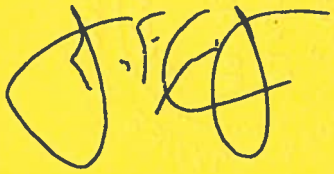
ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1TBI8228Z0211132904

Truss Fabricator:	Anderson Truss Company
Job Identification:	7-276--Freeman Design Group Brown -- , **
Truss Count:	22
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Versions 7.36, 7.24, 7.35.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 32.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed



Seal Date: 10/11/2007

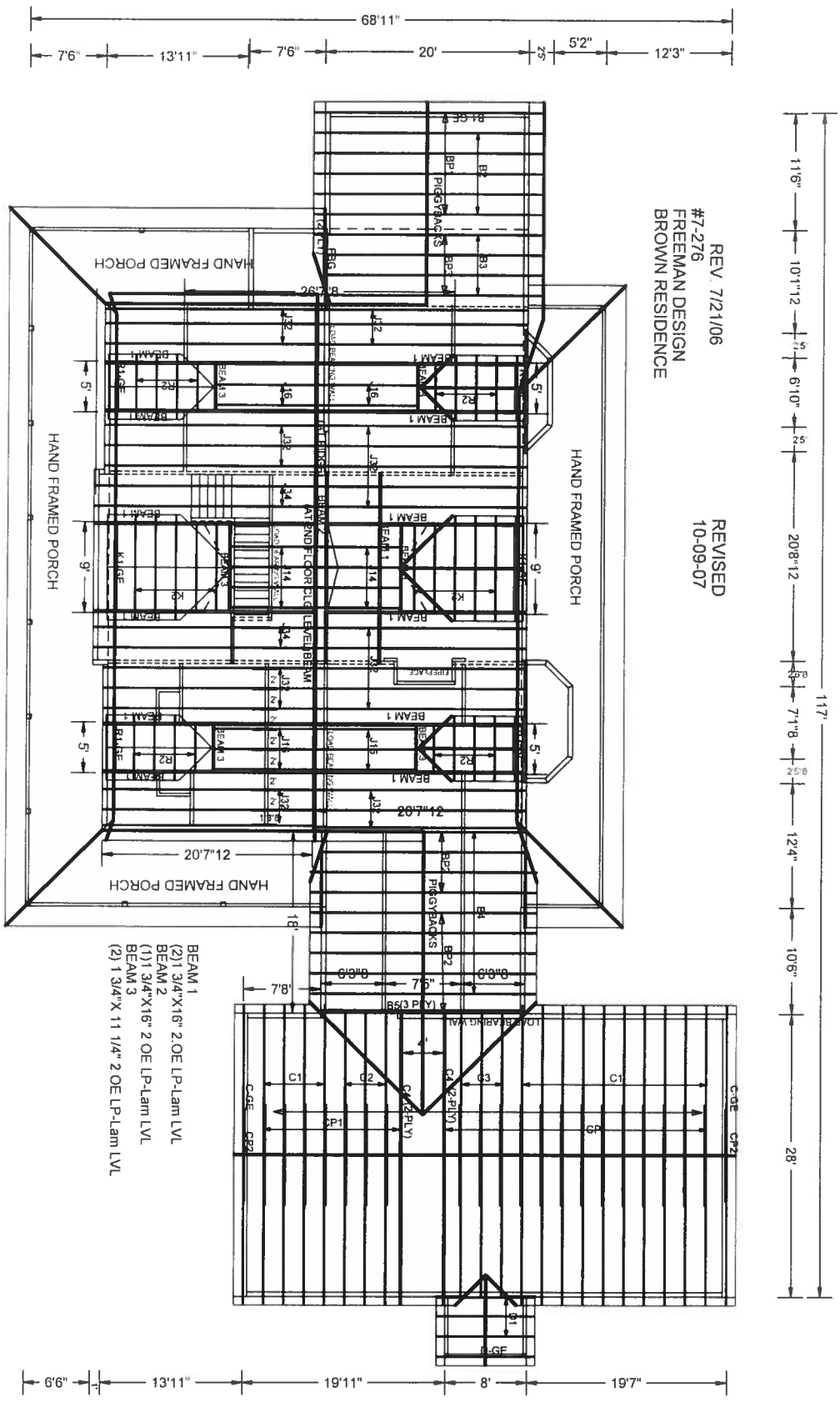
-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: A11030EE-GBLLETIN-BRCLBSUB-PIGBACKA-PIGBACKB-

#	Ref Description	Drawing#	Date
1	54437--B4	07284003	10/11/07
2	54438--B1-GE	07284003	10/11/07
3	54439--B3	07284013	10/11/07
4	54440--B2	07284014	10/11/07
5	54441--B5	07284004	10/11/07
6	54442--C-GE	07284008	10/11/07
7	54443--C4	07284010	10/11/07
8	54444--C1	07284011	10/11/07
9	54445--C3	07284012	10/11/07
10	54446--C2	07284015	10/11/07
11	54447--D-GE	07284006	10/11/07
12	54448--D1	07284007	10/11/07
13	54449--FBG	07284005	10/11/07
14	54450--K1-GE	07284001	10/11/07
15	54451--K2	07284002	10/11/07
16	54452--CP2	07284004	10/11/07
17	54453--CP1	07284009	10/11/07
18	54454--BP1	07284016	10/11/07
19	54455--BP2	07284017	10/11/07
20	54456--BP3	07284018	10/11/07
21	54457--R1-GE	07284019	10/11/07
22	54458--R2	07284020	10/11/07



JOB DESCRIPTION:: Freeman Design Group
/: Brown

JOB NO: 7-276

PAGE NO 1 OF 1

(7-276--Freeman Design Group Brown -- , ** - K1-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

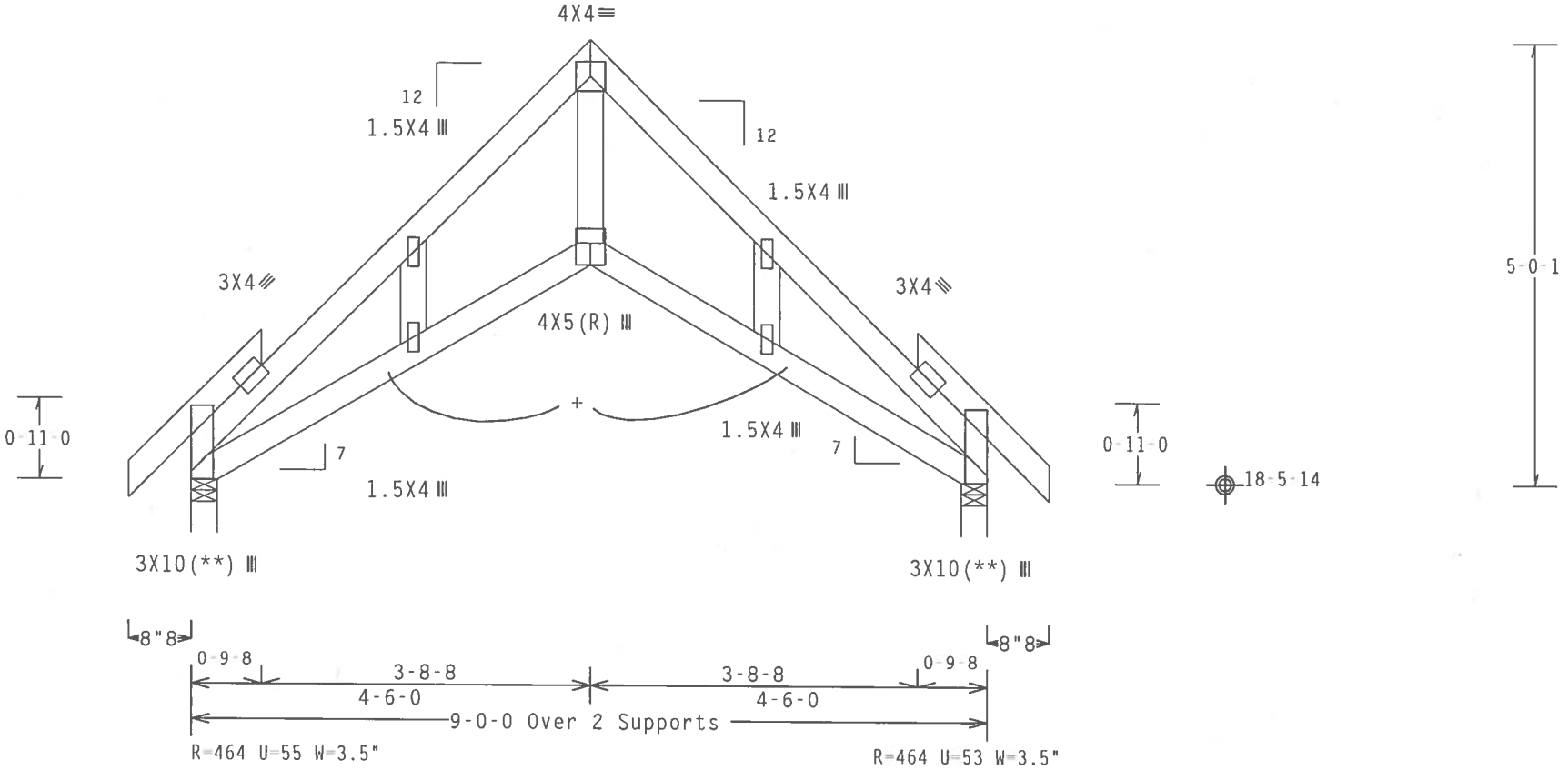
The Building Designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the Building Designer.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 21.10 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

+ Member to be laterally braced for horizontal wind loads. Bracing system to be designed and furnished by others.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.35.0316

QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Oct 10 07

TC LL	20.0 PSF	REF R8228- 39595
TC DL	10.0 PSF	DATE 10/09/07
BC DL	10.0 PSF	DRW HCUSR8228 07282070
BC LL	0.0 PSF	HC-ENG TCE/AP
TOT.LD.	40.0 PSF	SEQN- 11095 REV
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TBG8228Z01

(7-276--Freeman Design Group Brown -- , ** - K2)

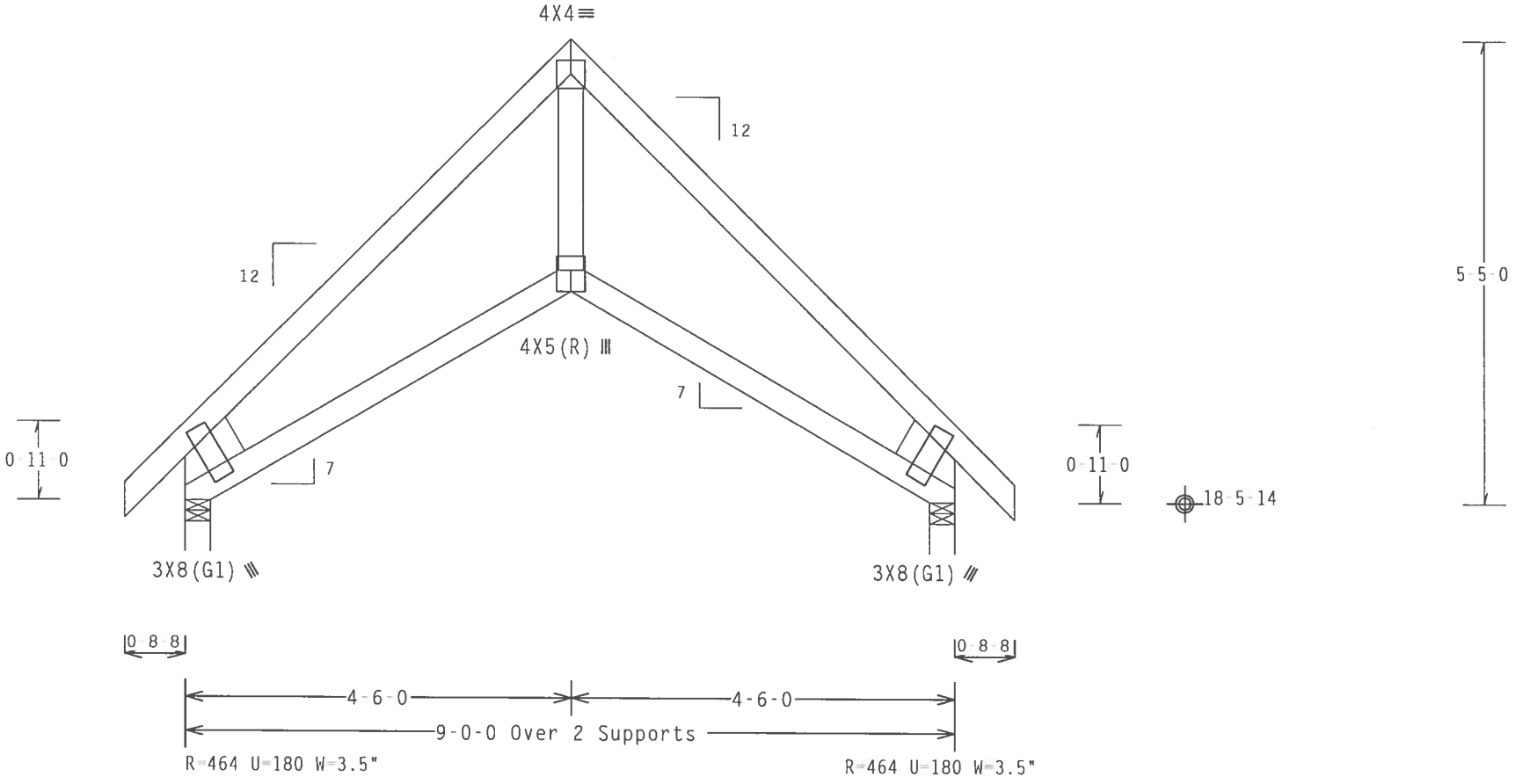
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Stubbed Wedge 2x6 SP #2::Rt Stubbed Wedge 2x6 SP #2:

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 21.30 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1230.17

PLT TYP. Wave

QTY:10 FL/-/4/-/-/R/-

Scale =.5"/Ft.

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Oct 10, 07

TC LL	20.0 PSF	REF R8228-39596
TC DL	10.0 PSF	DATE 10/09/07
BC DL	10.0 PSF	DRW HCUSR8228 07282071
BC LL	0.0 PSF	HC-ENG TCE/AP
TOT.LD.	40.0 PSF	SEQN- 35851
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TBG8228Z01

(7 276 Freeman Design Group Brown , ** B4)

Top chord 2x6 SP #1 Dense
Bot chord 2x10 SP SS
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

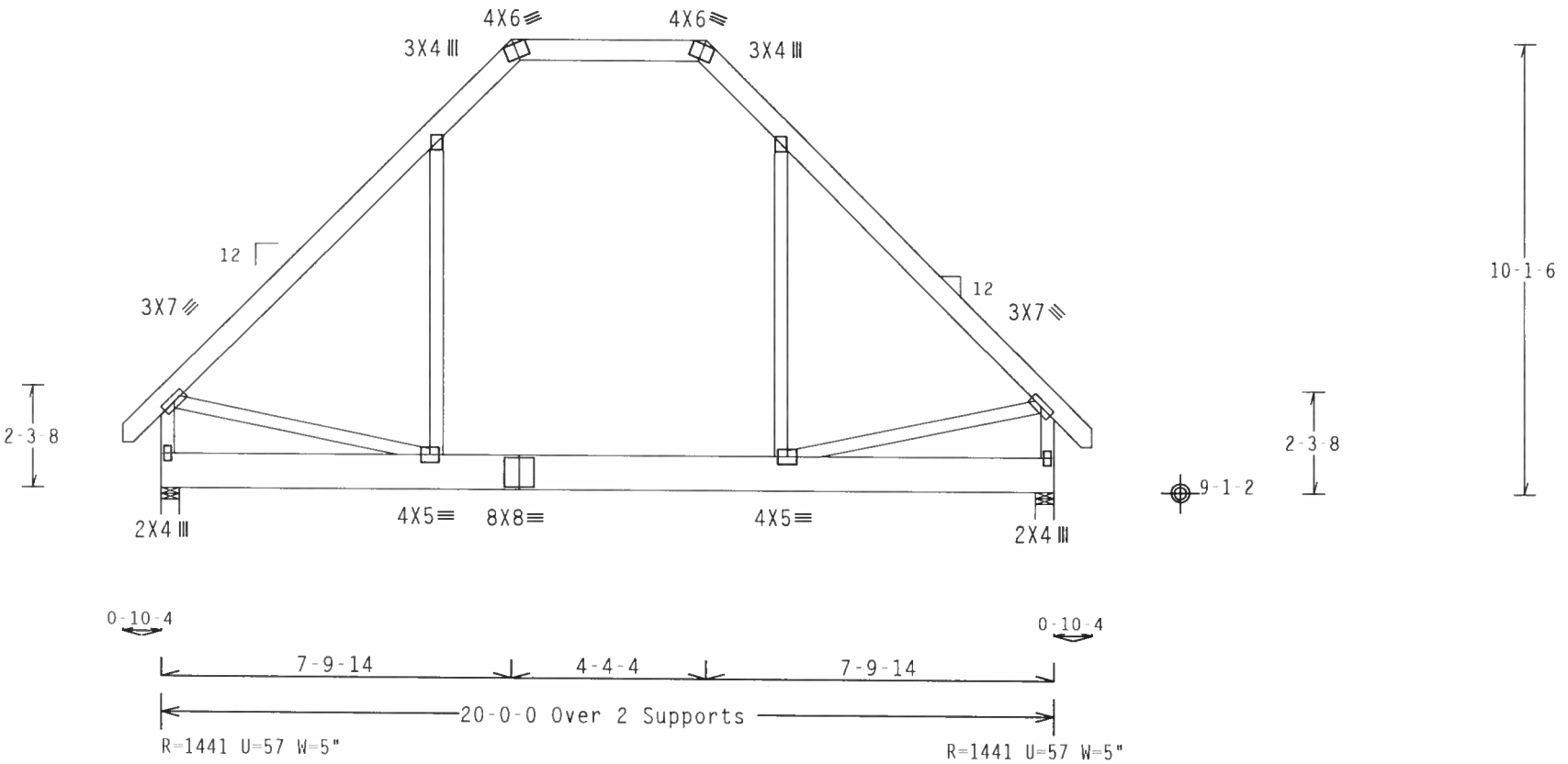
BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-3-0 to 13-8-0.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.35.0316.1

QTY:9 FL/-/4/-/-/R/- Scale =.25"/Ft.

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Oct 10 07

TC LL	20.0 PSF	REF R8228- 39597
TC DL	10.0 PSF	DATE 10/09/07
BC DL	10.0 PSF	DRW HCUSR8228 07282072
BC LL	0.0 PSF	HC-ENG TCE/AP
TOT.LD.	40.0 PSF	SEQN- 11099 REV
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TBG8228Z01

(/ 276 Freeman Design Group Brown , ** B1 GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

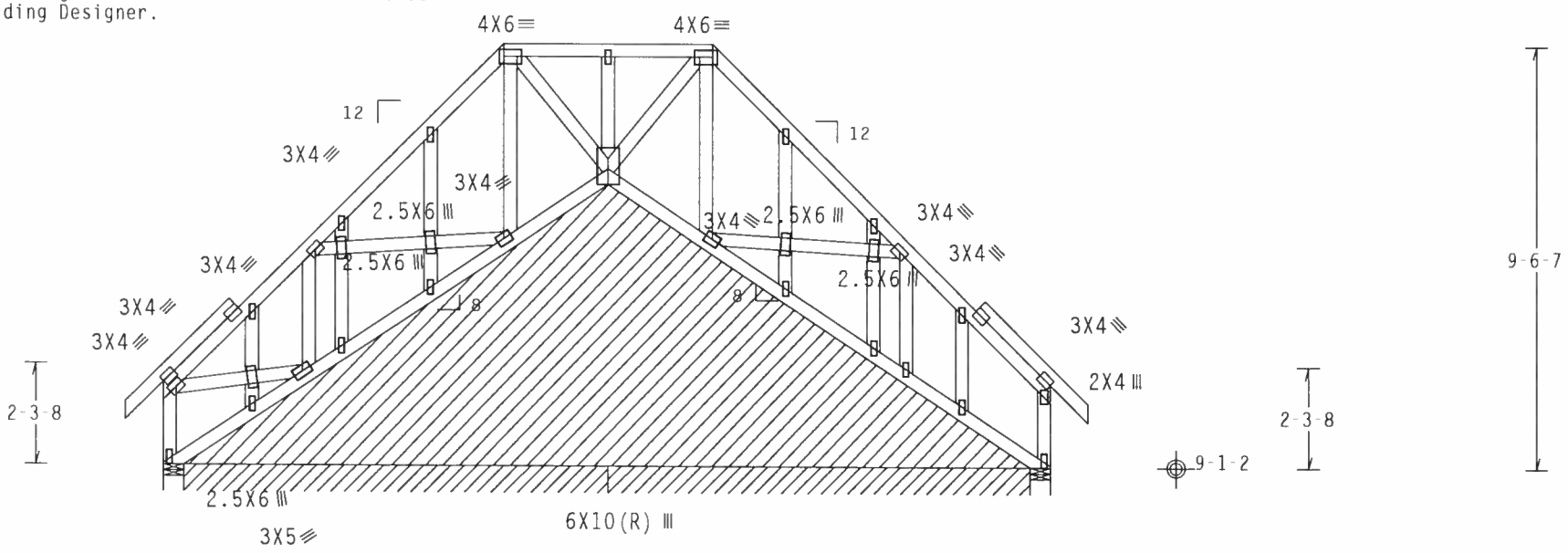
Shim all supports to solid bearing.

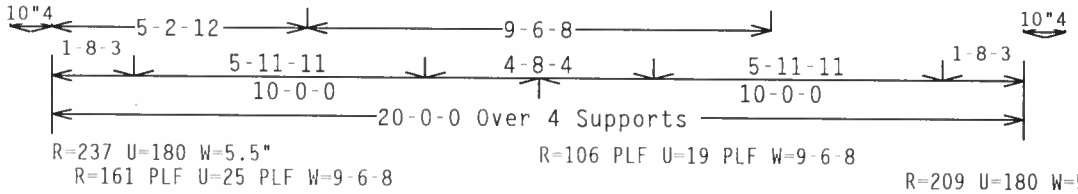
The Building Designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the Building Designer.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

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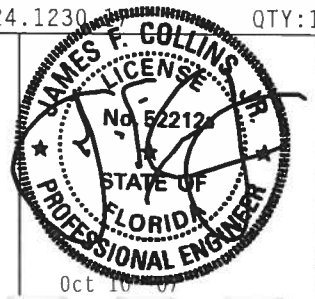


Oct 10 07

TC LL	20.0 PSF	REF	R8228- 39598
TC DL	10.0 PSF	DATE	10/09/07
BC DL	10.0 PSF	DRW	HCUSR8228 07282073
BC LL	0.0 PSF		HC-ENG TCE/AP
TOT.LD.	40.0 PSF	SEQN-	117614 REV
DUR.FAC.	1.25	FROM	JP
SPACING	SEE ABOVE	JREF-	1TBG8228Z01

(7 276 Freeman Design Group Brown , ** CP2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 68 PLF at 0.00 to 68 PLF at 4.59
TC - From 68 PLF at 4.59 to 68 PLF at 9.18
BC - From 4 PLF at 0.00 to 4 PLF at 9.18

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

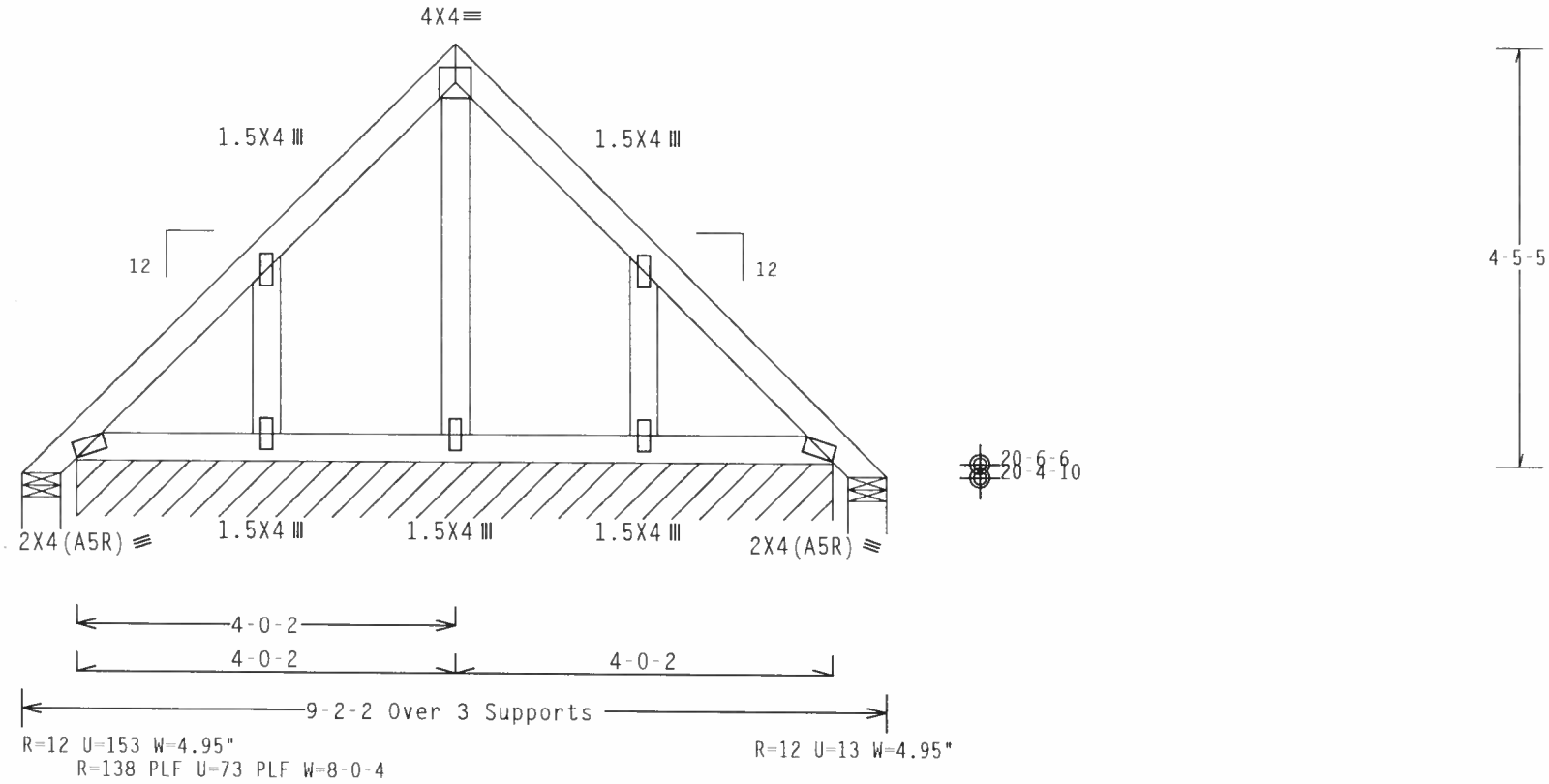
110 mph wind, 22.68 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWGS A11030EE0207 & GBLLETIN0207 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.35.0316.8

PLT TYP. Wave QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Oct 10 07

TC LL	20.0 PSF	REF R8228- 39599
TC DL	10.0 PSF	DATE 10/09/07
BC DL	2.0 PSF	DRW HCUSR8228 07282074
BC LL	0.0 PSF	HC-ENG TCE/AP
TOT.LD.	32.0 PSF	SEQN- 11102 REV
DUR.FAC.	1.25	FROM JP
SPACING	SEE ABOVE	JREF- 1TBG8228Z01

(7-276 - Freeman Design Group Brown -- , ** - FBG)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3 :W2, W4 2x4 SP #2 Dense:

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 60 PLF at 0.00 to 60 PLF at 7.96
BC - From 20 PLF at 0.00 to 20 PLF at 7.96
BC - 1569 LB Conc. Load at 0.56, 2.56, 4.56, 6.56

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

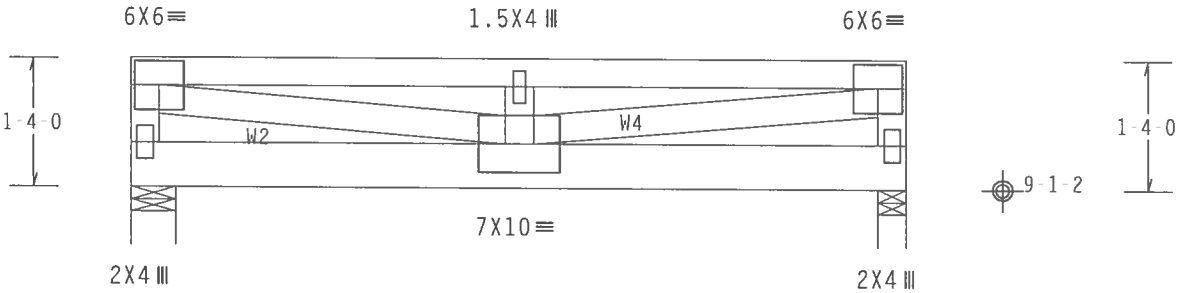
Truss must be installed as shown with top chord up.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Box or Gun (0.128"x3.25", min.) nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 4.00" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



7-11-8 Over 2 Supports

R=3785 U=407 W=5.5"

R=3128 U=337 W=3.5"

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.35.0316.10 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS JR. LICENSED No 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Oct 10 07

TC LL	20.0 PSF	REF	R8228- 39600
TC DL	10.0 PSF	DATE	10/09/07
BC DL	10.0 PSF	DRW	HCUSR8228 07282076
BC LL	0.0 PSF	HC-ENG	TCE/AP
TOT.LD.	40.0 PSF	SEQN-	11105 REV
DUR.FAC.	1.25	FROM	JP
SPACING	SEE ABOVE	JREF-	1TBG8228Z01

(/ 276 Freeman Design Group Brown , ** D GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:
:Stack Chord SC2 2x4 SP #2 Dense:

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

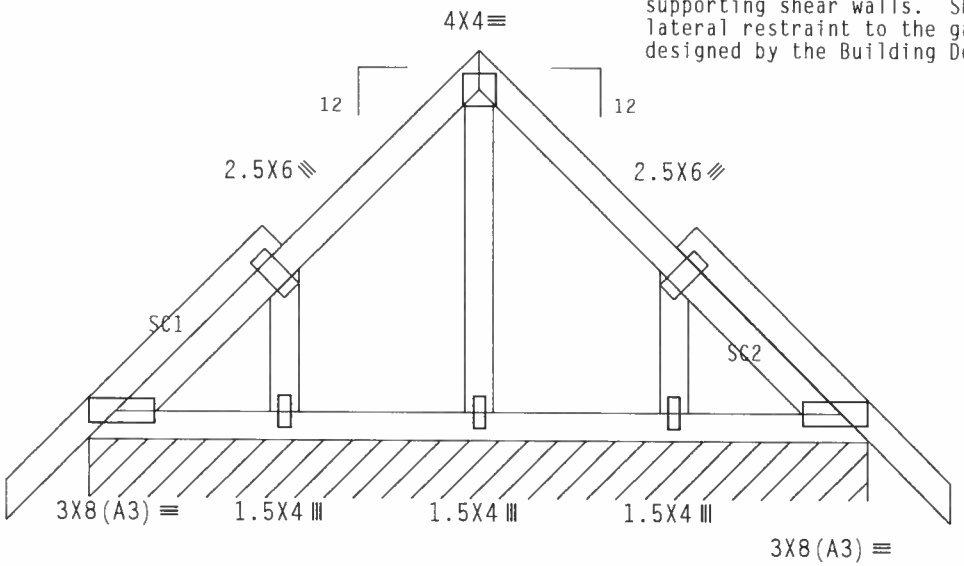
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

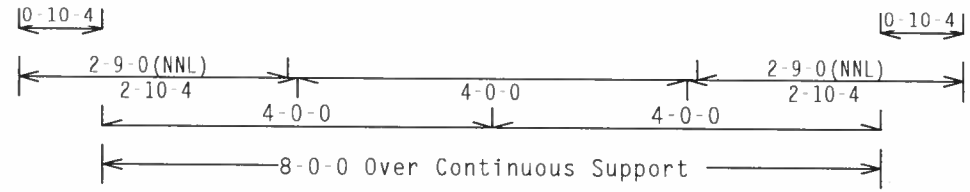
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The Building Designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the Building Designer.





R=153 PLT U=40 PLT W=8-0-0

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1230

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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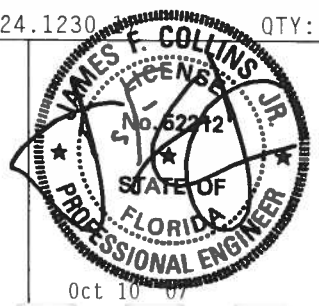


Oct 10 07

TC LL	20.0 PSF	REF	R8228- 39601
TC DL	10.0 PSF	DATE	10/10/07
BC DL	10.0 PSF	DRW	HCUSR8228 07283001
BC LL	0.0 PSF	HC-ENG	TCE/AP *
TOT.LD.	40.0 PSF	SEQN-	167057
DUR.FAC.	1.25	FROM	JP
SPACING	SEE ABOVE	JREF-	1TBG8228Z01

(7 276 Freeman Design Group Brown , ** D1)

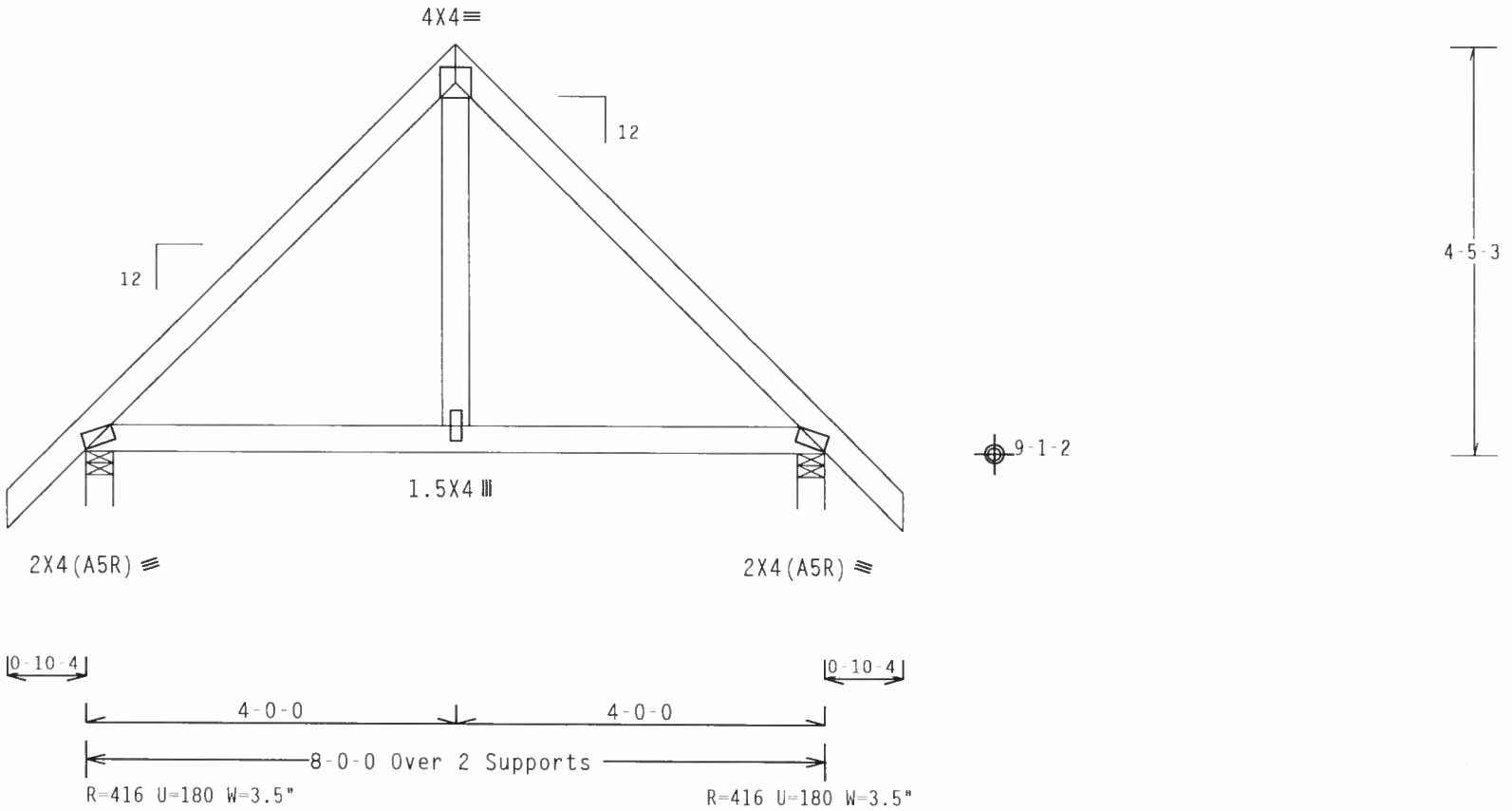
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1230

PLT TYP. Wave

QTY:3 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Oct 10 07

TC LL	20.0 PSF	REF R8228- 39602
TC DL	10.0 PSF	DATE 10/09/07
BC DL	10.0 PSF	DRW HCUSR8228 07282081
BC LL	0.0 PSF	HC-ENG TCE/AP *
TOT.LD.	40.0 PSF	SEQN- 167053
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TBG8228Z01

(7 276 Freeman Design Group Brown , ** C GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

The Building Designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the Building Designer.

110 mph wind, 15.46 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

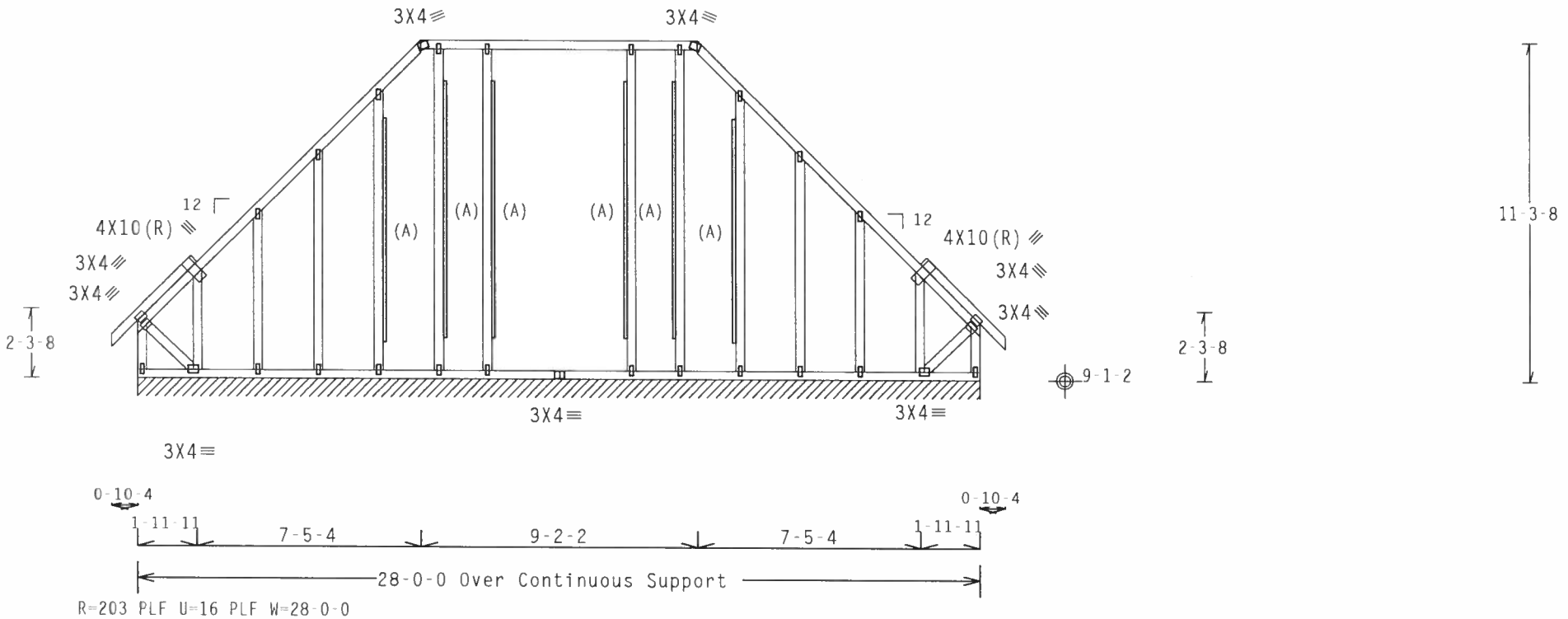
Wind reactions based on MWFRS pressures.

See DWGS A11030EE0207 & GBLLETIN0207 for more requirements.

(A) 1x4 #3 or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230.17 QTY:2 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Oct 10

TC LL	20.0 PSF	REF	R8228- 39603
TC DL	10.0 PSF	DATE	10/10/07
BC DL	10.0 PSF	DRW	HCUSR8228 07283002
BC LL	0.0 PSF	HC-ENG	TCE/AP
TOT.LD.	40.0 PSF	SEQN-	167135 REV
DUR.FAC.	1.25	FROM	JP
SPACING	SEE ABOVE	JREF-	1TBG8228Z01

(7-276--Freeman Design Group Brown -- , ** - CP1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

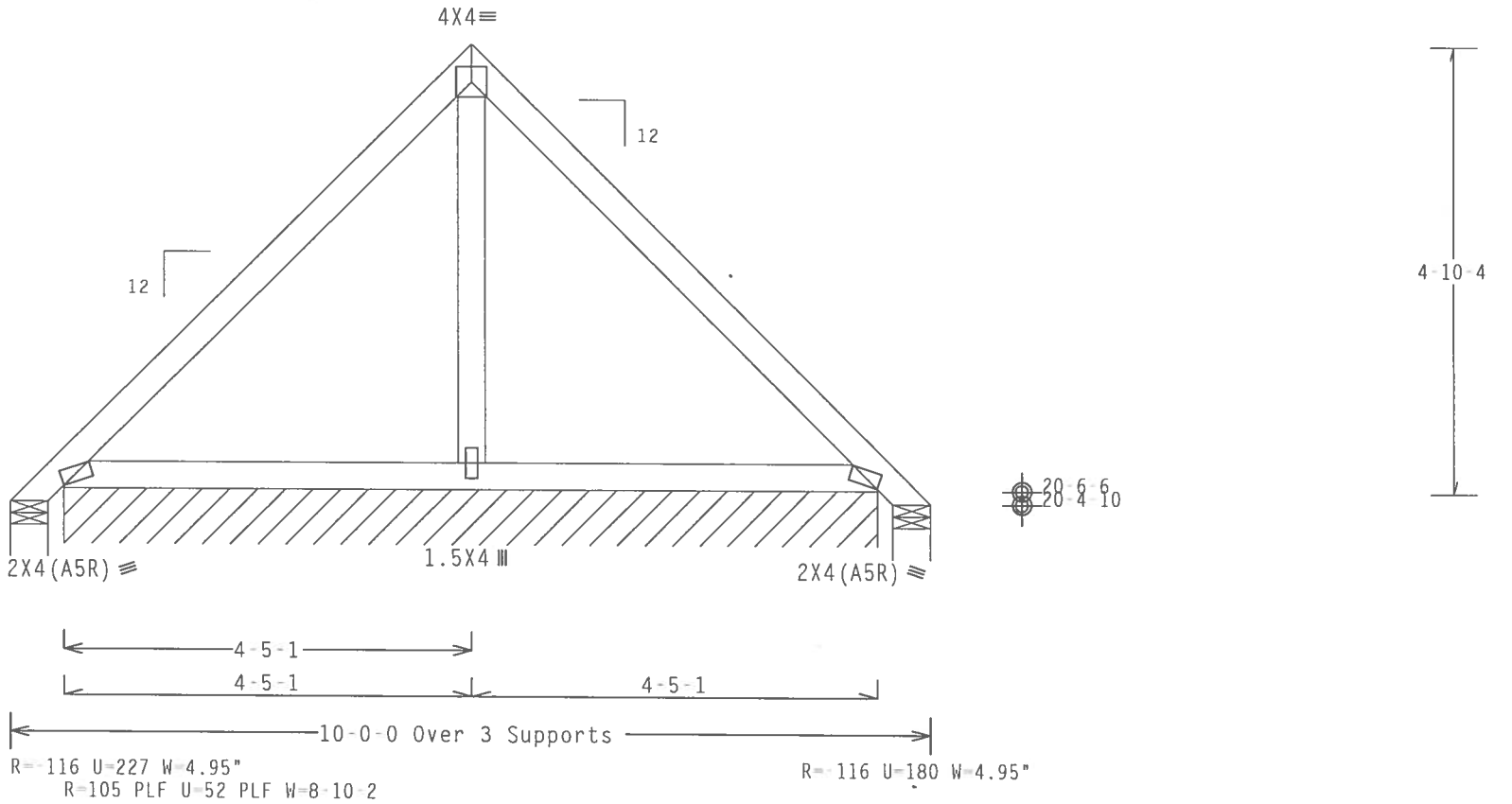
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified.

110 mph wind, 22.89 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1230.1

PLT TYP. Wave

QTY:22 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Oct 10-07

TC LL	20.0 PSF	REF R8228- 39604
TC DL	10.0 PSF	DATE 10/10/07
BC DL	2.0 PSF	DRW HCUSR8228 07283003
BC LL	0.0 PSF	HC-ENG TCE/AP
TOT.LD.	32.0 PSF	SEQN- 167064
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TBG8228Z01

(7-276--Freeman Design Group Brown -- , ** - C4)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC	From	104 PLF	at	0.00	to	104 PLF	at 9.00
TC	From	104 PLF	at	9.00	to	104 PLF	at 19.00
TC	From	104 PLF	at	19.00	to	104 PLF	at 28.85
BC	From	30 PLF	at	0.00	to	30 PLF	at 8.00
BC	From	30 PLF	at	8.00	to	30 PLF	at 9.45
BC	From	130 PLF	at	9.45	to	130 PLF	at 18.55
BC	From	30 PLF	at	18.55	to	30 PLF	at 20.00
BC	From	30 PLF	at	20.00	to	30 PLF	at 28.00
BC	From	9 PLF	at	28.00	to	9 PLF	at 28.85

Left end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Truss to be spaced at 19" oc from one side and at 51.75" oc from opposite side.

2 COMPLETE TRUSSES REQUIRED

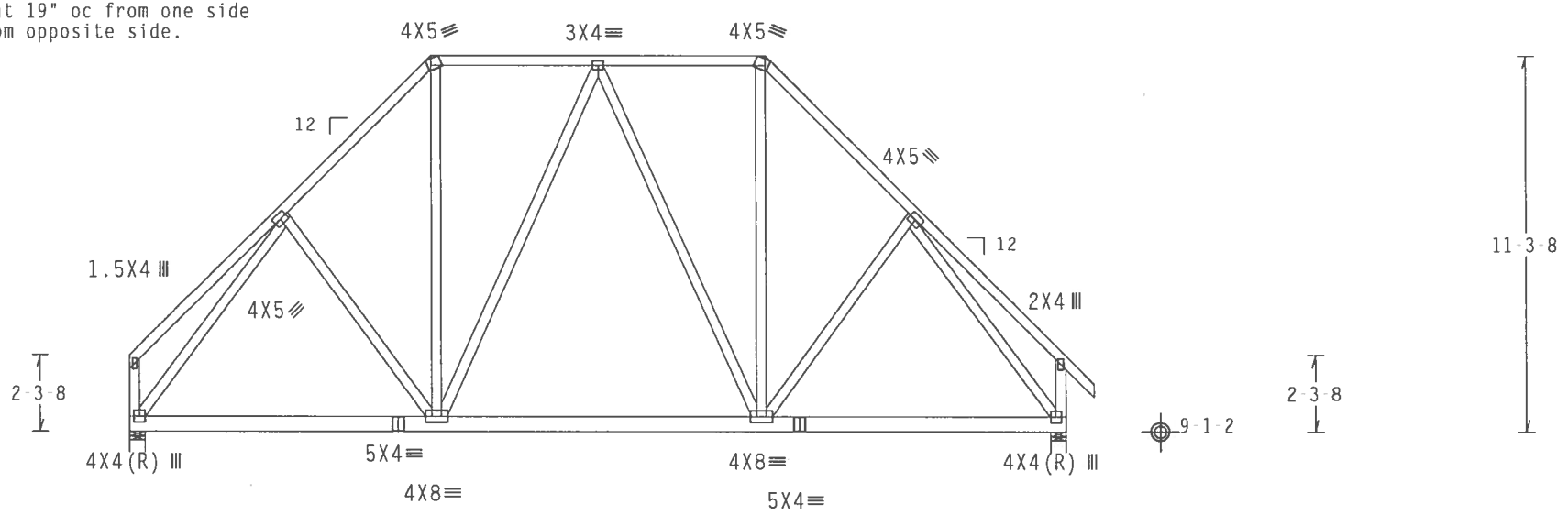
Nailing Schedule: (12d Box or Gun (0.128"x3.25", min.) nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.46 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230.17 QTY:2 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Oct 10,2007

TC LL	20.0 PSF	REF	R8228-39605
TC DL	10.0 PSF	DATE	10/10/07
BC DL	10.0 PSF	DRW	HCUSR8228 07283004
BC LL	0.0 PSF	HC-ENG	TCE/AP
TOT.LD.	40.0 PSF	SEQN-	167149
DUR.FAC.	1.25	FROM	JP
SPACING	SEE ABOVE	JREF-	1TBG8228Z01

(7 276 Freeman Design Group Brown , ** C1)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	68	PLF at	-0.85	to	68	PLF at	9.00
TC - From	68	PLF at	9.00	to	68	PLF at	19.00
TC - From	68	PLF at	19.00	to	68	PLF at	28.85
BC - From	6	PLF at	-0.85	to	6	PLF at	0.00
BC - From	20	PLF at	0.00	to	20	PLF at	8.00
BC - From	20	PLF at	8.00	to	20	PLF at	9.45
BC - From	120	PLF at	9.45	to	120	PLF at	18.71
BC - From	20	PLF at	18.71	to	20	PLF at	20.00
BC - From	20	PLF at	20.00	to	20	PLF at	28.00
BC - From	6	PLF at	28.00	to	6	PLF at	28.85

110 mph wind, 15.46 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

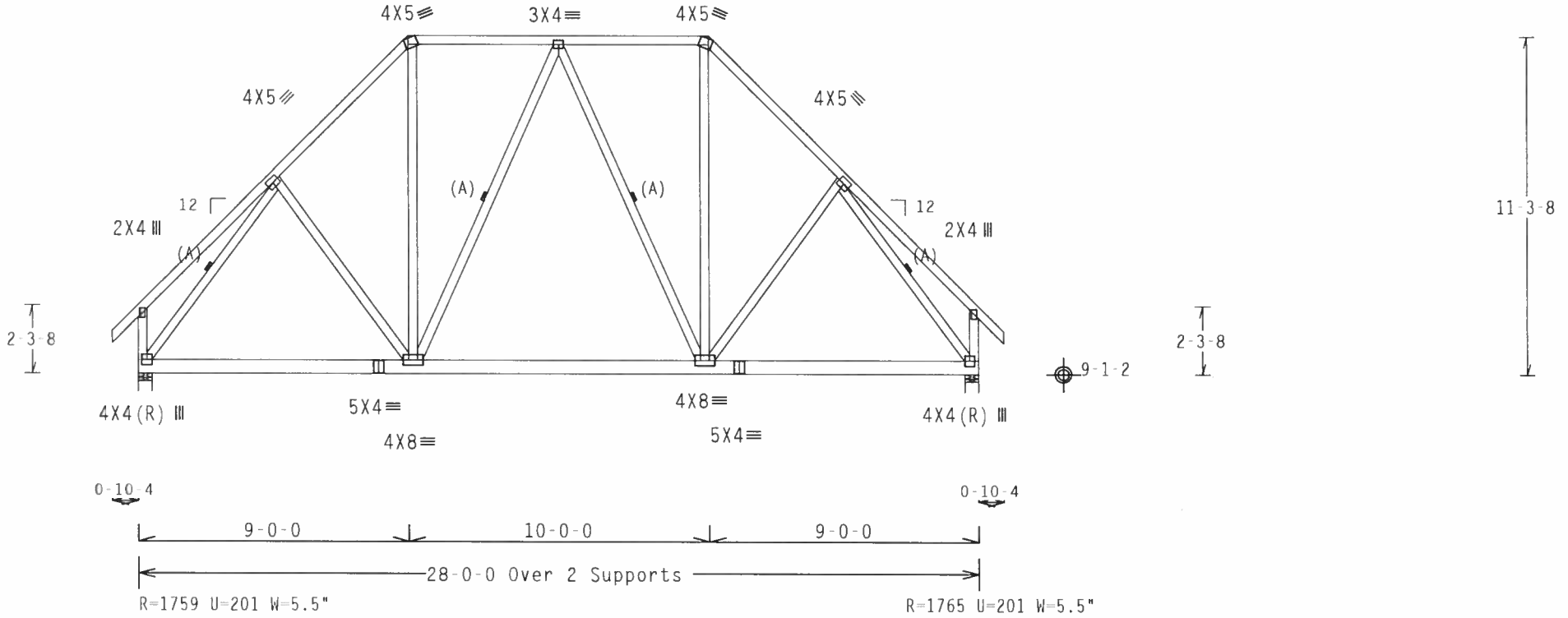
Wind reactions based on MWFRS pressures.

End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1230.1

PLT TYP. Wave

QTY:14 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Oct 10,2007

TC LL	20.0 PSF	REF	R8228- 39606
TC DL	10.0 PSF	DATE	10/09/07
BC DL	10.0 PSF	DRW	HCUSR8228 07282077
BC LL	0.0 PSF	HC-ENG	TCE/AP
TOT.LD.	40.0 PSF	SEQN-	167141
DUR.FAC.	1.25	FROM	JP
SPACING	SEE ABOVE	JREF-	1TBG8228Z01

(7-276--Freeman Design Group Brown -- , ** - C3)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC	From	68 PLF	at	0.00	to	68 PLF	at 9.00
TC	From	68 PLF	at	9.00	to	68 PLF	at 19.00
TC	From	68 PLF	at	19.00	to	68 PLF	at 28.00
BC	From	20 PLF	at	0.00	to	20 PLF	at 9.29
BC	From	120 PLF	at	9.29	to	120 PLF	at 18.55
BC	From	20 PLF	at	18.55	to	20 PLF	at 28.00

Wind reactions based on MWFRS pressures.

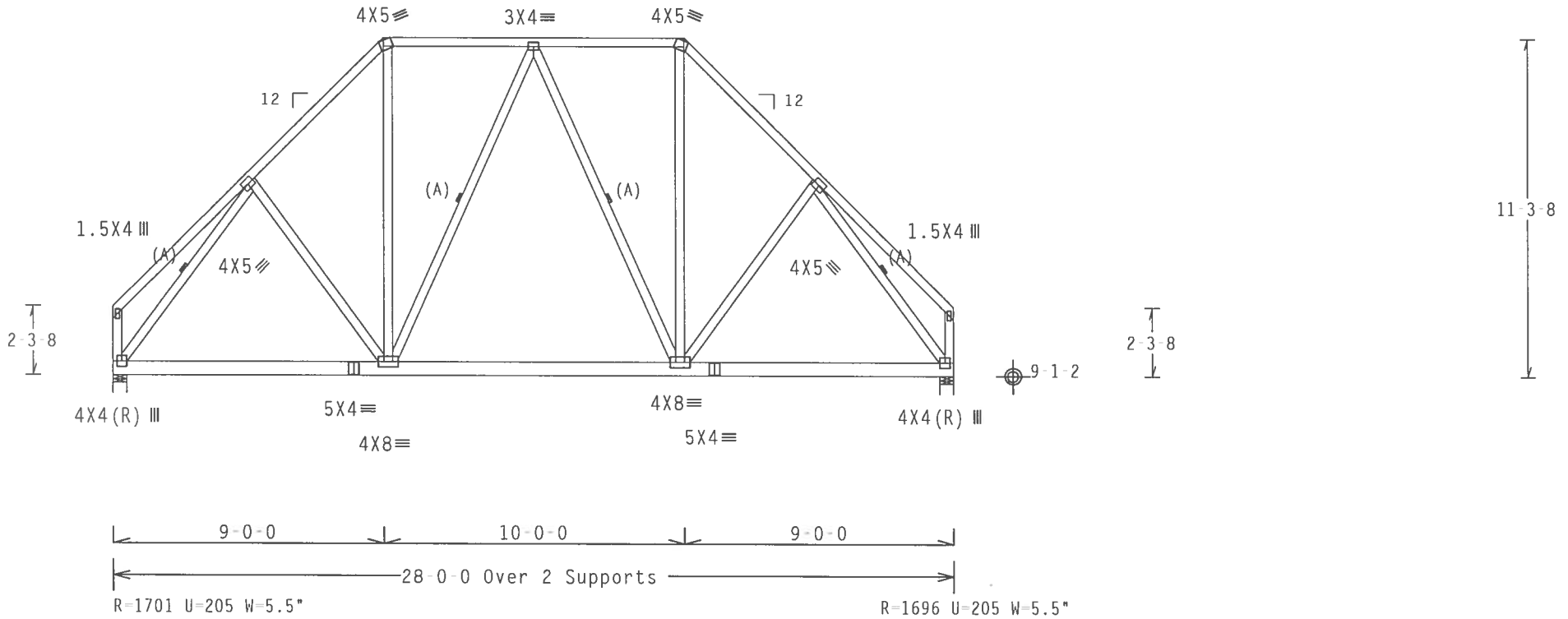
110 mph wind, 15.89 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1230

PLT TYP. Wave QTY:3 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Oct 10 07

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	SEE ABOVE

REF	R8228- 39607
DATE	10/09/07
DRW	HCUSR8228 07282079
	HC-ENG TCE/AP
SEQN-	167082
FROM	JP
JREF-	1TBG8228Z01

(7-276--Freeman Design Group Brown -- , ** - B3)

Top chord 2x6 SP #1 Dense
Bot chord 2x8 SP SS
Webs 2x4 SP #3

End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-3-0 to 13-8-0.

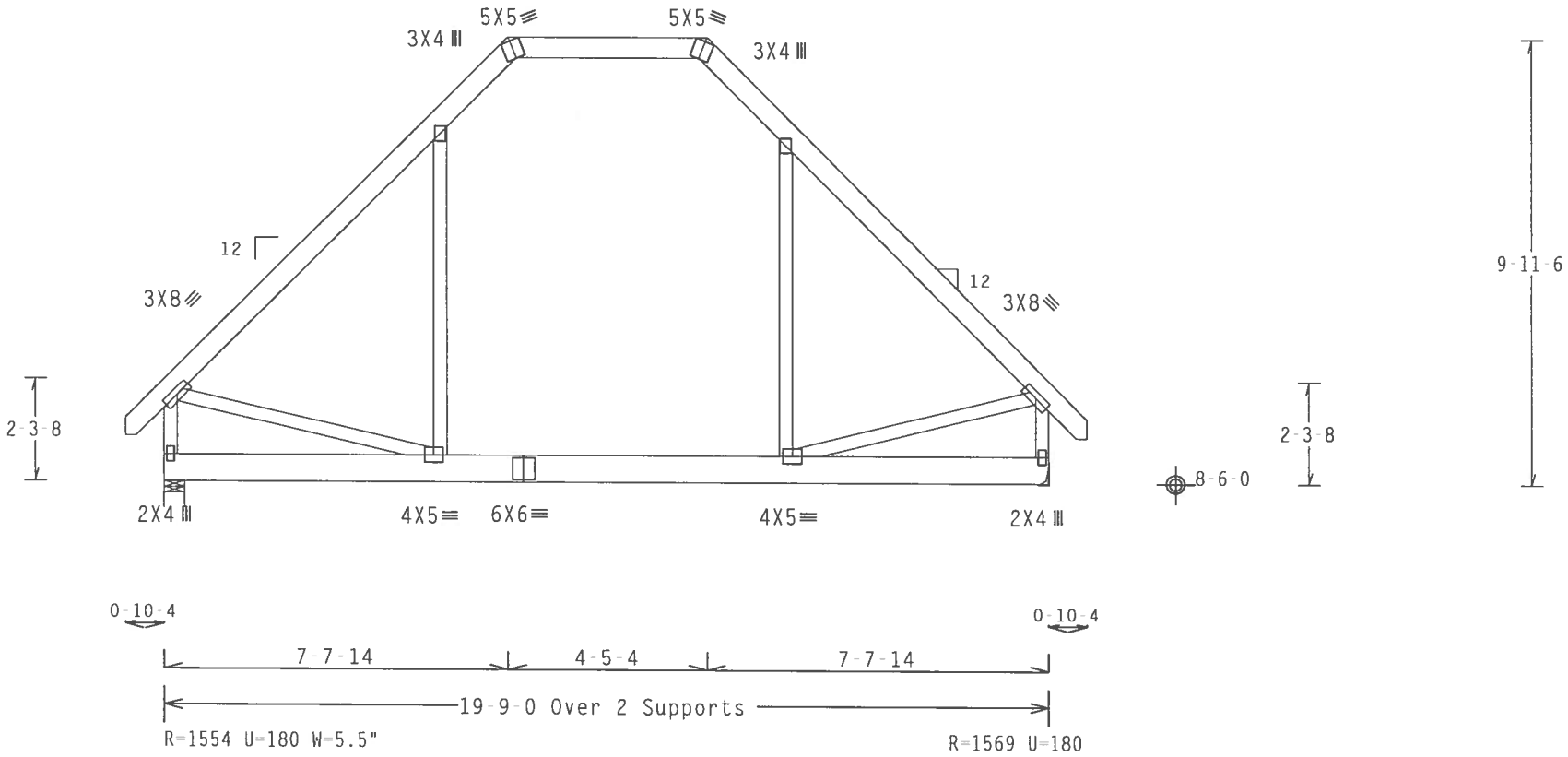
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.08" due to live load and 0.20" due to dead load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:4 FL/-/4/-/-/R/- Scale =.25"/Ft.

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JAMES F. COLLINS, JR. LICENSE No.52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Oct 10 '07

TC LL	20.0 PSF	REF R8228-39608
TC DL	10.0 PSF	DATE 10/09/07
BC DL	10.0 PSF	DRW HCUSR8228 07282075
BC LL	0.0 PSF	HC-ENG TCE/AP
TOT.LD.	40.0 PSF	SEQN- 117638
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TBG8228Z01

(7 276 Freeman Design Group Brown , ** B2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

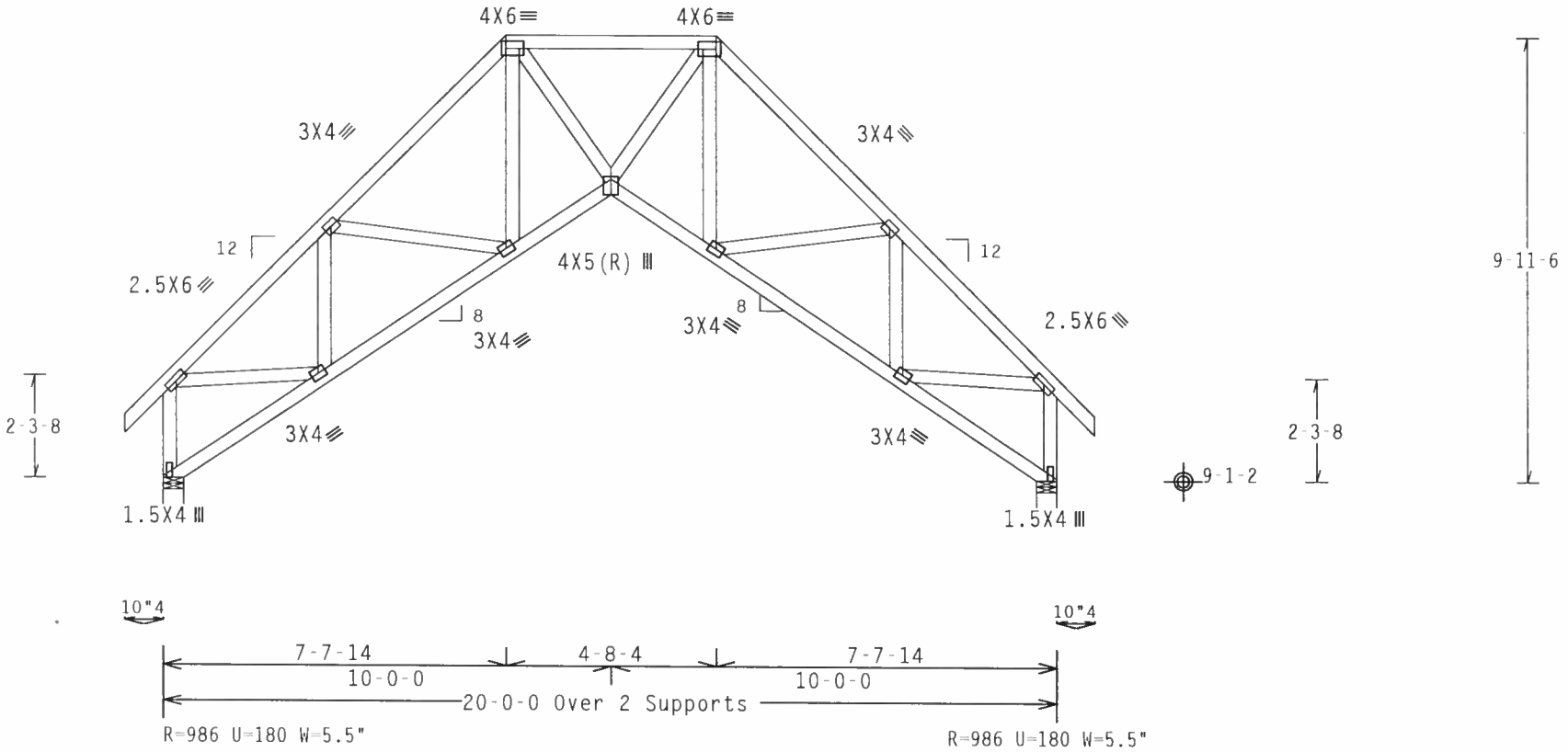
End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:5 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Oct 10 '07

TC LL	20.0 PSF	REF	R8228- 39609
TC DL	10.0 PSF	DATE	10/09/07
BC DL	10.0 PSF	DRW	HCUSR8228 07282078
BC LL	0.0 PSF		HC-ENG TCE/AP
TOT.LD.	40.0 PSF	SEQN-	117606
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TBG8228Z01

(7 276 Freeman Design Group Brown , ** C2)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

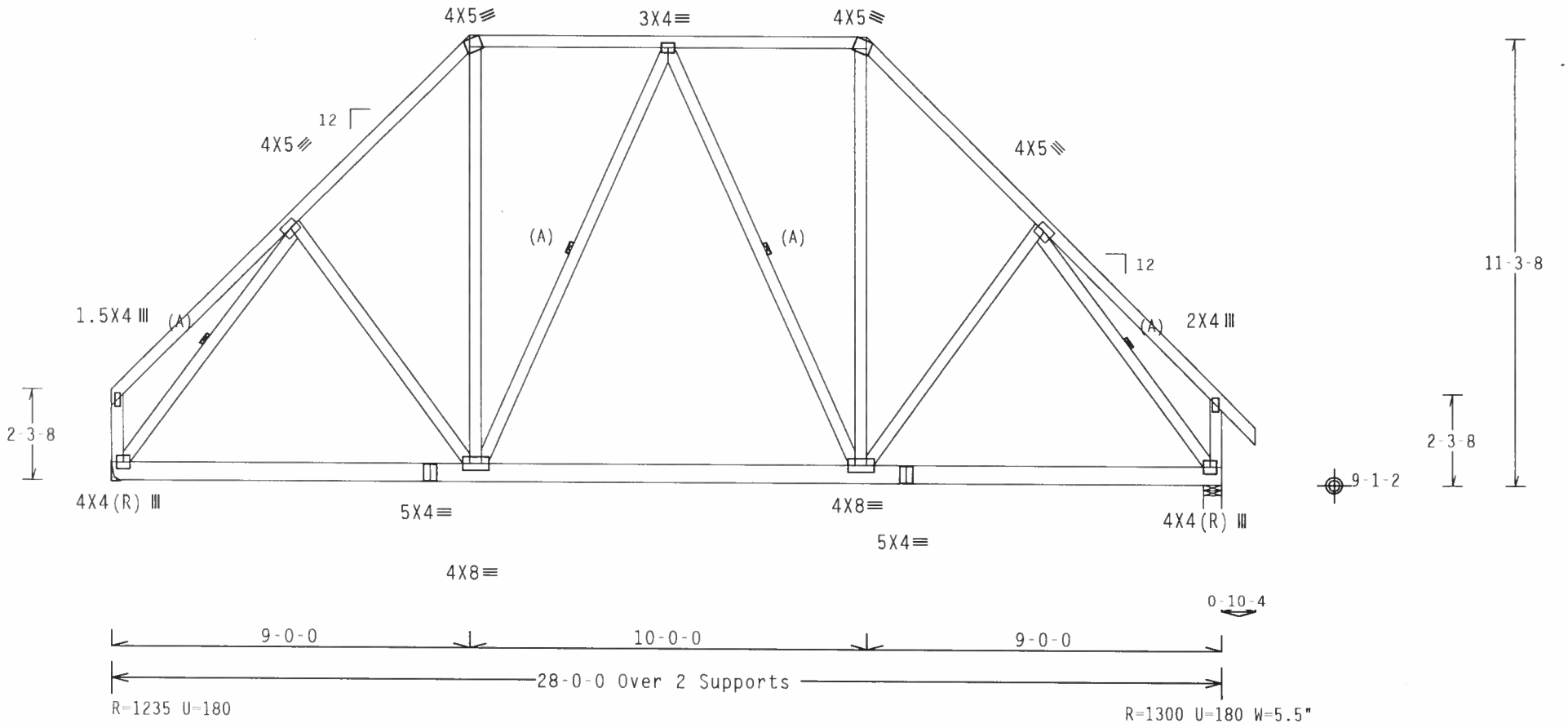
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.46 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1230

QTY:3 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

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Oct 10 07

TC LL	20.0 PSF	REF	R8228- 39610
TC DL	10.0 PSF	DATE	10/09/07
BC DL	10.0 PSF	DRW	HCUSR8228 07282080
BC LL	0.0 PSF	HC-ENG	TCE/AP
TOT.LD.	40.0 PSF	SEQN-	167145
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TBG8228Z01

(7-276 Freeman Design Group Brown -- , ** - BP1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

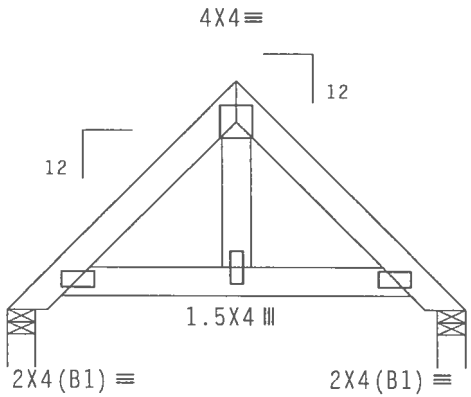
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

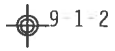
Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified.

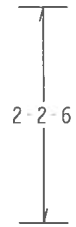
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

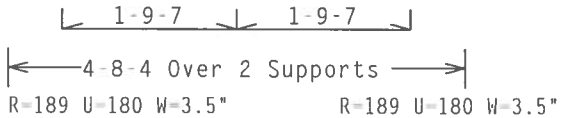
Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.









Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

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Oct 10 07

TC LL	20.0 PSF	REF	R8228- 39611
TC DL	10.0 PSF	DATE	10/10/07
BC DL	10.0 PSF	DRW	HCUSR8228 07283005
BC LL	0.0 PSF		HC-ENG TCE/AP
TOT.LD.	40.0 PSF	SEQN-	167032
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TBG8228Z01

(7-276--Freeman Design Group Brown -- , ** - BP2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

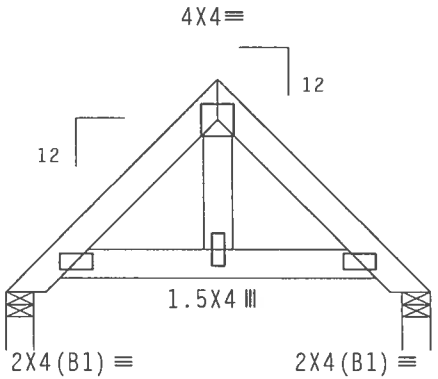
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



9-1-2

2-0-6

1-7-7 1-7-7

4-4-4 Over 2 Supports

R=174 U=180 W=3.5" R=174 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:10 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

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JAMES F. COLLINS JR. LICENSE NO. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Oct 10 07

TC LL	20.0 PSF	REF	R8228- 39612
TC DL	10.0 PSF	DATE	10/10/07
BC DL	10.0 PSF	DRW	HCUSR8228 07283006
BC LL	0.0 PSF		HC-ENG TCE/AP
TOT.LD.	40.0 PSF	SEQN-	167112
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TBG8228Z01

(7-276-Freeman Design Group Brown -- , ** - BP3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

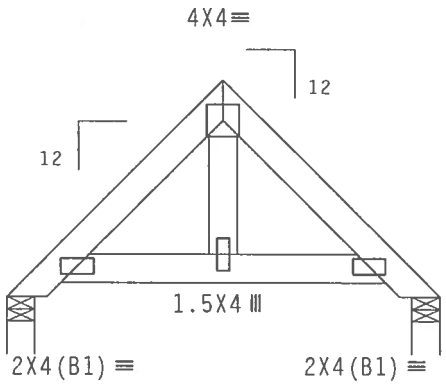
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



9-1-2

2-0-14

1-7-15 1-7-15

4-5-4 Over 2 Supports

R=178 U=180 W=3.5" R=178 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

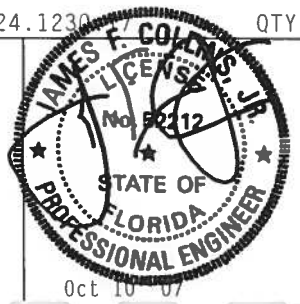
Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:4 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Oct 10 '07

TC LL	20.0 PSF	REF	R8228-39613
TC DL	10.0 PSF	DATE	10/10/07
BC DL	10.0 PSF	DRW	HCUSR8228 07283007
BC LL	0.0 PSF	HC-ENG	TCE/AP
TOT.LD.	40.0 PSF	SEQN-	117645
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TBG8228Z01

(7 276 Freeman Design Group Brown , ** B5)

Top chord 2x6 SP #1 Dense
Bot chord 2x10 SP SS
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 68 PLF at -0.04 to 68 PLF at 6.25
TC - From 97 PLF at 6.25 to 97 PLF at 7.97
TC - From 97 PLF at 7.97 to 97 PLF at 12.14
TC - From 97 PLF at 12.14 to 97 PLF at 13.67
TC - From 68 PLF at 13.67 to 68 PLF at 19.96
BC - From 20 PLF at -0.04 to 20 PLF at 6.25
BC - From 120 PLF at 6.25 to 120 PLF at 13.67
BC - From 20 PLF at 13.67 to 20 PLF at 19.96
BC - 143 LB Conc. Load at 6.25
BC - 1378 LB Conc. Load at 13.60
BC - 1235 LB Conc. Load at 15.60, 17.60

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

3 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d_Box_or_Gun_(0.128"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 3.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.30 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

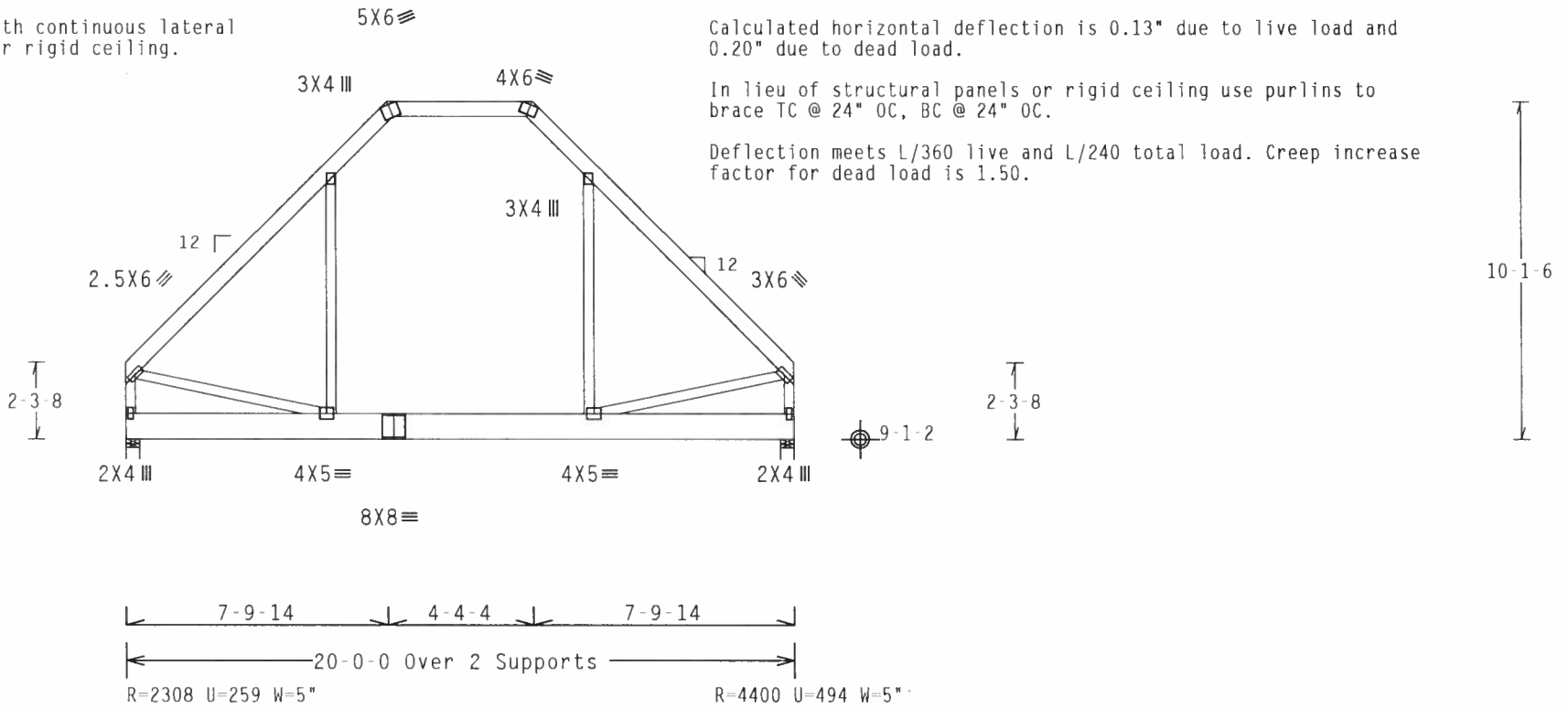
Wind reactions based on MWFRS pressures.

End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Calculated horizontal deflection is 0.13" due to live load and 0.20" due to dead load.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.35.0316.18 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Oct 10,07

TC LL	20.0 PSF	REF R8228- 39614
TC DL	10.0 PSF	DATE 10/10/07
BC DL	10.0 PSF	DRW HCUSR8228 07283008
BC LL	0.0 PSF	HC-ENG TCE/AP
TOT.LD.	40.0 PSF	SEQN- 11118 REV
DUR.FAC.	1.25	FROM JP
SPACING	SEE ABOVE	JREF- 1TBG8228Z01

(7 276 Freeman Design Group Brown , ** R1 GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

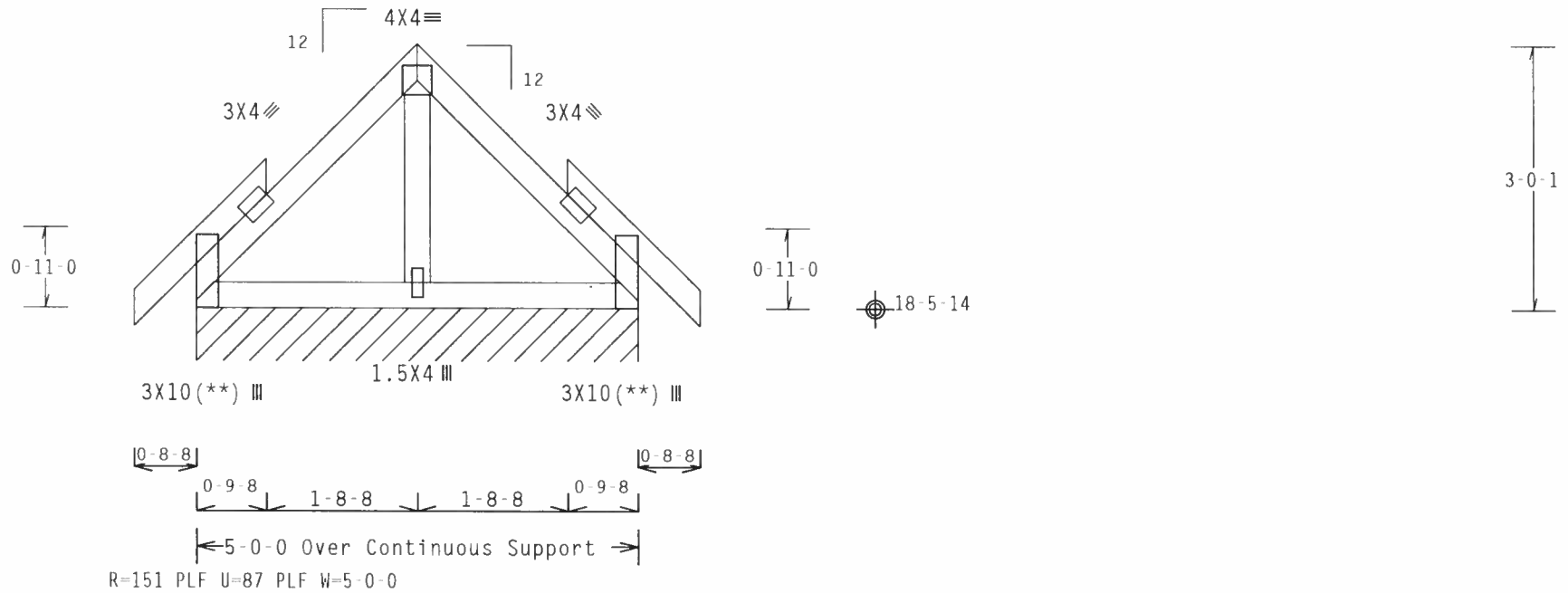
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 20.10 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Right end vertical not exposed to wind pressure.

See DWGS A11030EE0207 & GBLLETIN0207 for more requirements.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.35.0316 8 QTY:4 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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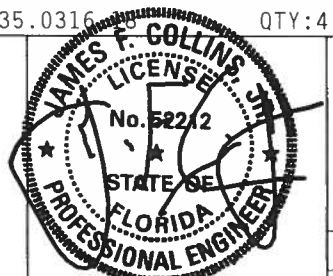


Oct 10 07

TC LL	20.0 PSF	REF	R8228- 39615
TC DL	10.0 PSF	DATE	10/09/07
BC DL	10.0 PSF	DRW	HCUSR8228 07282083
BC LL	0.0 PSF	HC-ENG	TCE/AP
TOT.LD.	40.0 PSF	SEQN-	11110 REV
DUR.FAC.	1.25		
SPACING	SEE ABOVE	JREF-	1TBG8228Z01

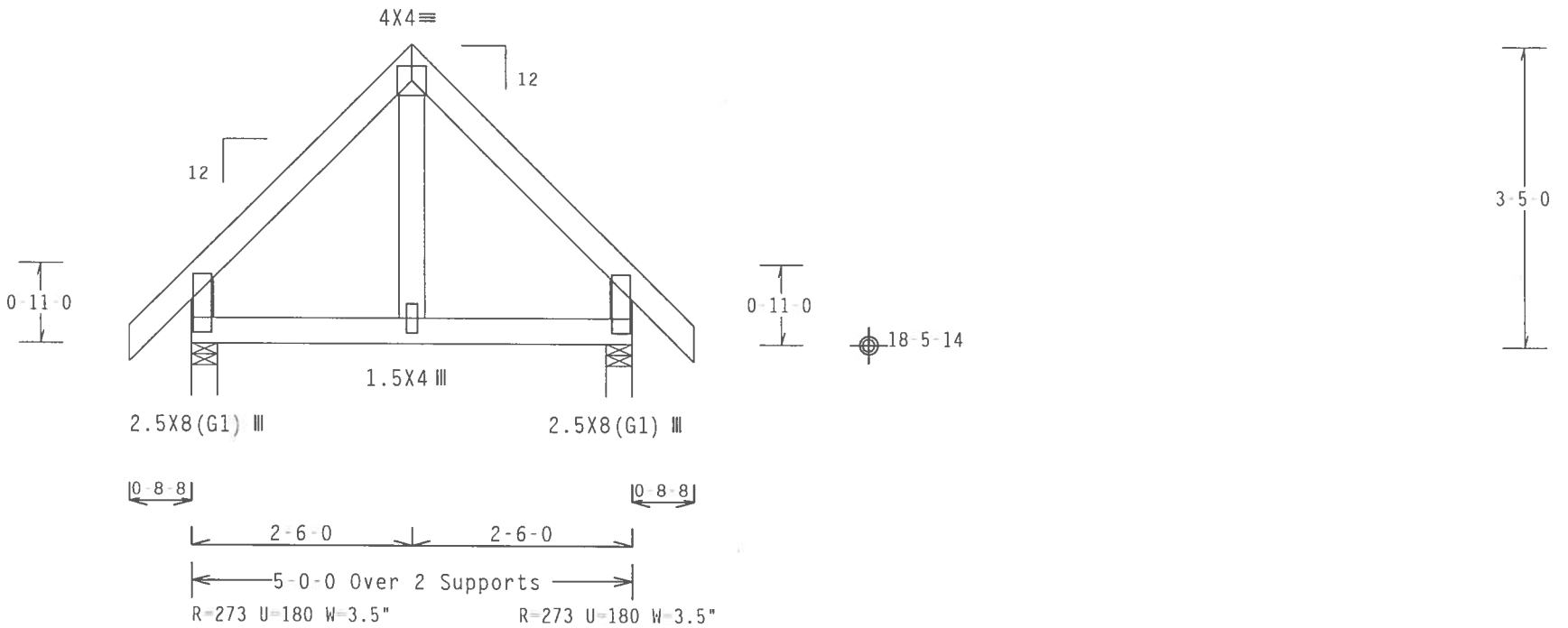
(7 276 Freeman Design Group Brown , ** R2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Stubbed Wedge 2x6 SP #2::Rt Stubbed Wedge 2x6 SP #2:

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 20.30 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:16 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS JR. LICENSE No 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Oct 10 07

TC LL	20.0 PSF	REF	R8228- 39616
TC DL	10.0 PSF	DATE	10/09/07
BC DL	10.0 PSF	DRW	HCUSR8228 07282082
BC LL	0.0 PSF	HC-ENG	TCE/AP *
TOT.LD.	40.0 PSF	SEQN-	115270
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TBG8228Z01

ASCE 7-02: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 1"	6' 1"	8' 3"	8' 3"	9' 6"	9' 6"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 10"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"
		#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STUD	4' 1"	8' 0"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	7' 6"	10' 1"	10' 1"	11' 8"	11' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 7"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#2	4' 6"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	6' 10"	6' 10"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	6' 9"	6' 9"	8' 7"	8' 11"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 10"	5' 10"	7' 8"	7' 8"	10' 3"	10' 4"	11' 11"	11' 11"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 7"	8' 0"	8' 2"	9' 5"	9' 8"	11' 3"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 6"	6' 7"	6' 7"	8' 8"	8' 8"	11' 3"	11' 3"	13' 6"	13' 6"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 11"	7' 11"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	7' 9"	7' 9"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 7"	6' 9"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

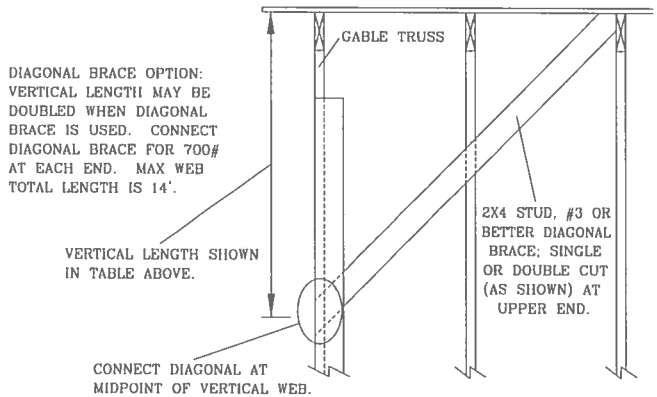
GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

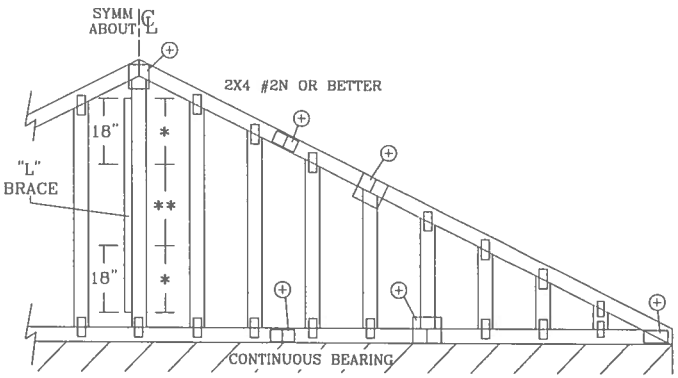
ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

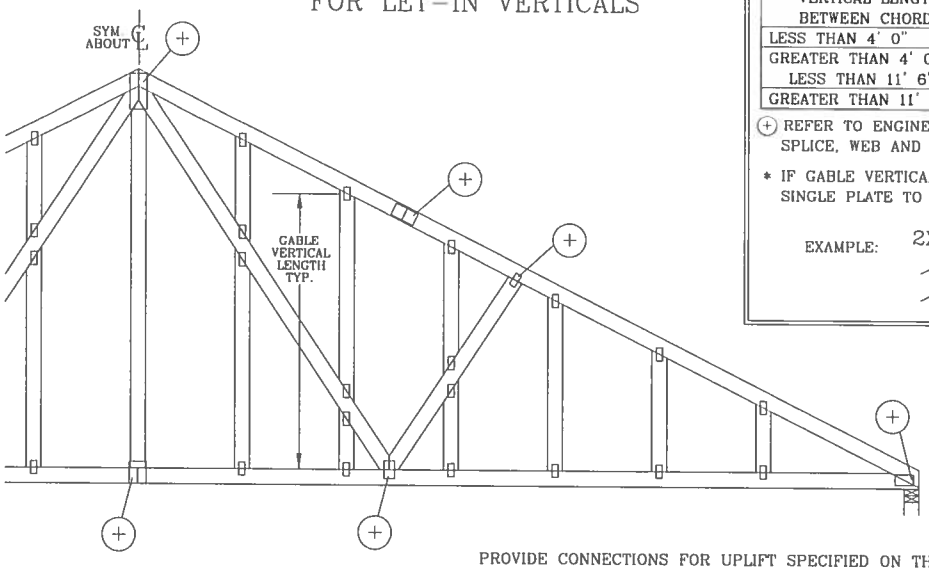
OCT 10 '07

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11030
DATE	2/23/07
DRWG	A11030EE0207
-ENG	

GABLE DETAIL
FOR LET-IN VERTICALS

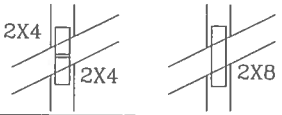


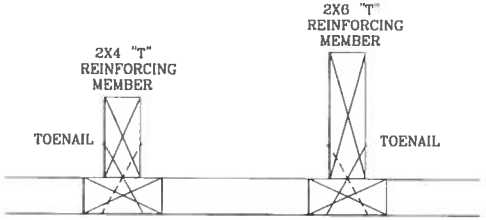
GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE: 



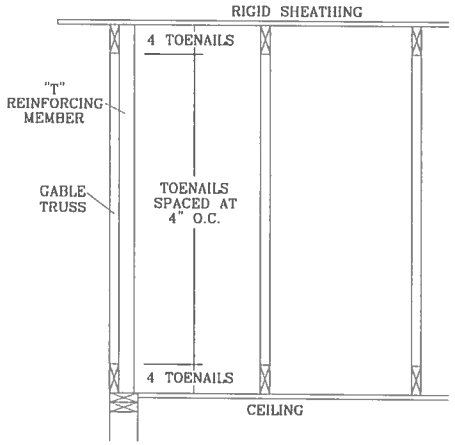
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH 15 FT	2x4	10 %	10 %
	2x6	40 %	50 %
110 MPH 30 FT	2x4	10 %	10 %
	2x6	50 %	50 %
100 MPH 15 FT	2x4	10 %	10 %
	2x6	30 %	50 %
100 MPH 30 FT	2x4	10 %	10 %
	2x6	40 %	40 %
90 MPH 15 FT	2x4	20 %	10 %
	2x6	20 %	40 %
90 MPH 30 FT	2x4	10 %	10 %
	2x6	30 %	50 %
80 MPH 15 FT	2x4	10 %	20 %
	2x6	10 %	30 %
80 MPH 30 FT	2x4	20 %	10 %
	2x6	20 %	40 %
70 MPH 15 FT	2x4	0 %	20 %
	2x6	0 %	20 %
70 MPH 30 FT	2x4	10 %	20 %
	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH

HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.

GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207

ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207, A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08530EC0207

ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207, A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08530EE0207

ASCE 7-05 GABLE DETAIL DRAWINGS
A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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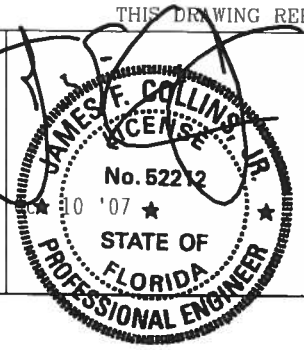


MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

REF	LET-IN VERT
DATE	2/23/07
DRWG	GBLLETIN0207
-ENG	DLJ/KAR

PIGGYBACK DETAIL

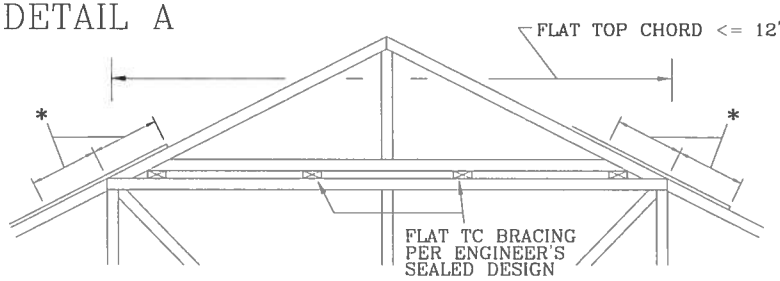
100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

80 MPH WIND, 30.00 FT MEAN HGT, SBC, ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATLY BRACED BY SHEATHING OR PURLINS. PROVIDE DIAGONAL BRACING OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

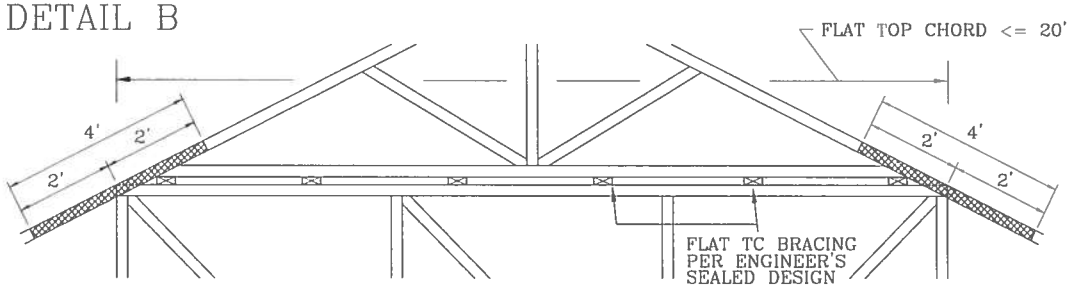
DETAIL A



PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS.

* 12" MIN RIGID SHEATHING OVERLAP WITH 8d COMMON (0.131"x2.5") OR GUN NAILS IN OVERLAP ZONE SPACED AT 4" O.C.

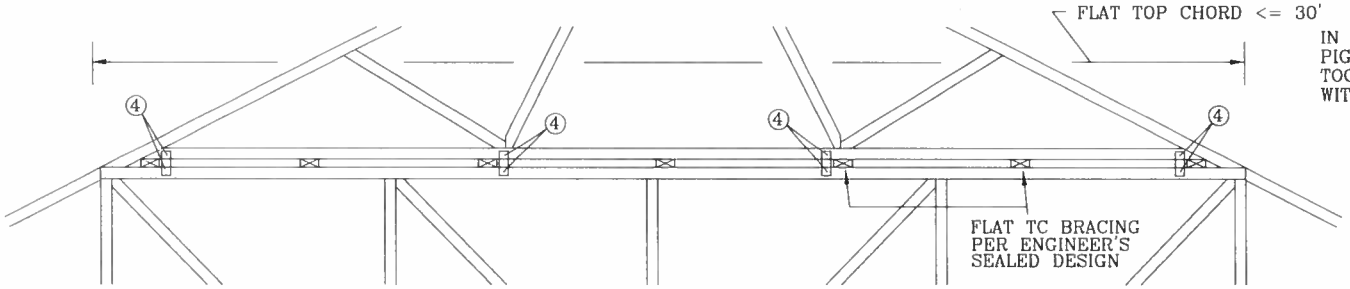
DETAIL B



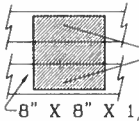
PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"X3") NAILS AND SECURED WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY) ATTACHED WITH 10d COMMON NAILS AT 4" O.C.

DETAIL C

CAP TRUSS TOENAILED TO TOP CHORD BRACING AND SECURED WITH 3X8 TRULOX PLATES (EACH FACE) AT EACH END AND AT 1/3 POINTS. CIRCLED NUMBER INDICATES REQUIRED NUMBER OF 0.120" X 1.375" NAILS PER FACE. SEE DRAWING 160TL FOR TRULOX INFORMATION.



IN LIEU OF TRULOX CONNECTORS, ALPINE 62PB SPECIAL PIGGYBACK CONNECTORS MAY BE USED. SHOP APPLY TOOTHED PORTION, FIELD ATTACH TO MATING TRUSS WITH (4) 0.120" X 0.375" NAILS MINIMUM EACH FACE.



8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 8d COMMON NAILS PER GUSSET, (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

THIS DRAWING REPLACES DRAWINGS 581,670 & 961,860

ALPINE

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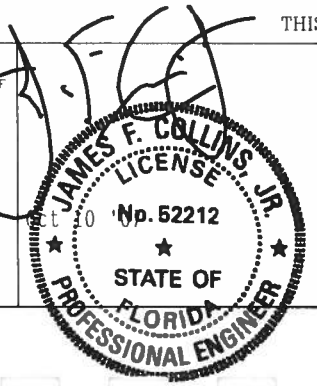


TC LL	PSF	REF	PIGGYBACK
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	PIGBACKA0207
BC LL	PSF	-ENG	DLJ/KAR
TOT. LD. MAX	60 PSF		
DUR. FAC.	1.15		
SPACING	24.0"		

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

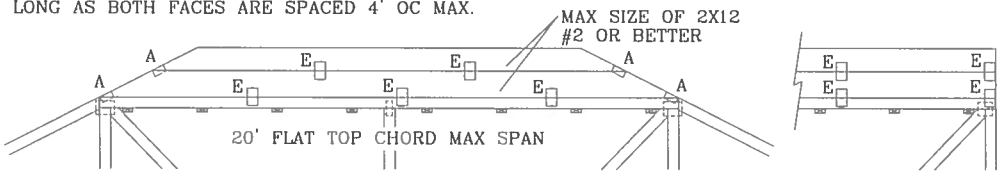
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

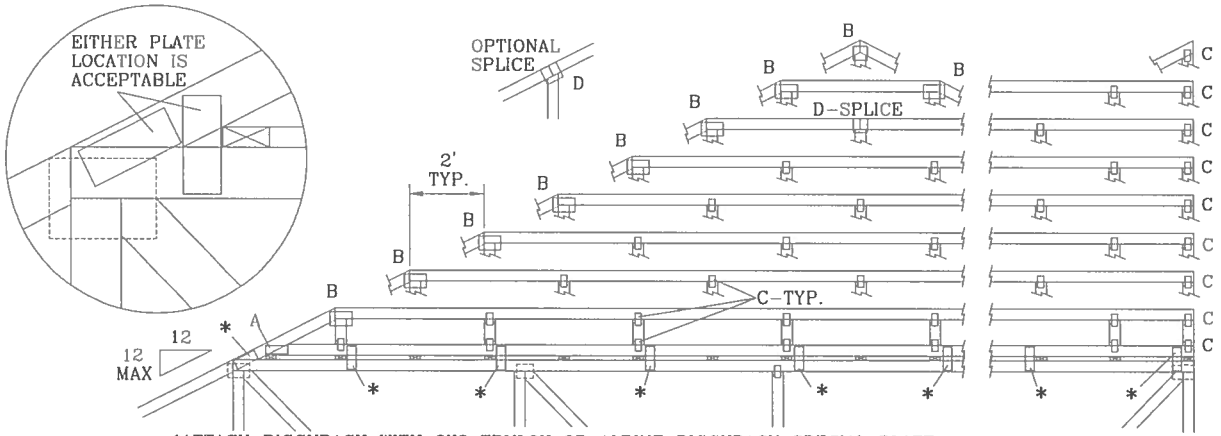
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

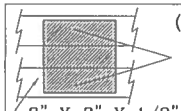
110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.



(4) 6d BOX (0.099"X 2.",MIN) NAILS.

8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET.

(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

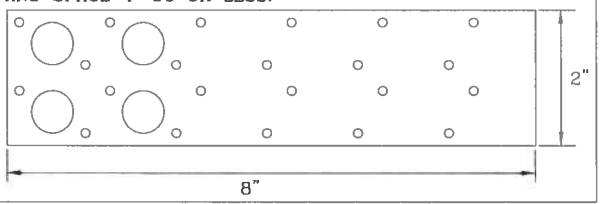
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5",MIN) NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5",MIN) NAILS AT 4" OC

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

ALPINE
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POMPANO BEACH, FLORIDA

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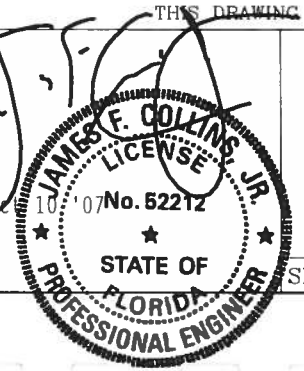


OC 10 '07

MAX LOADING

55 PSF AT 1.33 DUR. FAC.

50 PSF AT 1.25 DUR. FAC.

47 PSF AT 1.15 DUR. FAC.

SPACING 24.0"

REF	PIGGYBACK
DATE	2/23/07
DRWG	PIGBACKB0207
-ENG	DLJ/KAR

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

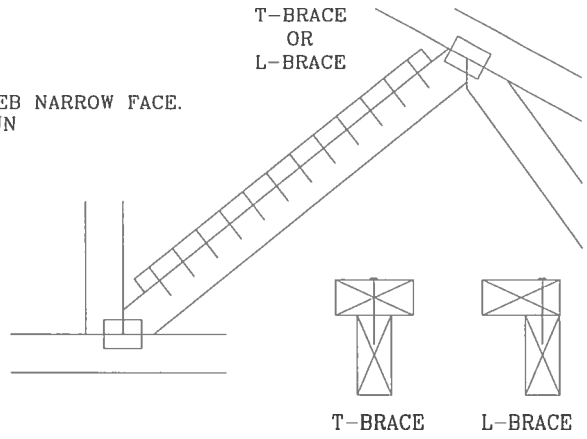
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

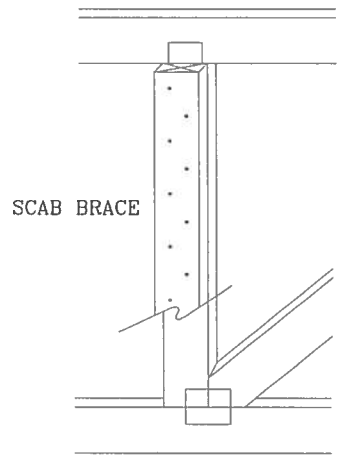
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW, BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W,K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
02 10 '07

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 2/23/07
BC DL	PSF	DRWG BRCLBSUB0207
BC LL	PSF	-ENG MLH/KAR
TOT. LD.	PSF	
DUR. FAC.		
SPACING		