

DATE 04/06/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT

000025701

APPLICANT JOHN DUNN

PHONE 758-1908

ADDRESS 871 SW HILL CREEK DRIVE

LAKE CITY

FL 32025

OWNER JOHN & ELAINE DUNN

PHONE 758.4581

ADDRESS 871 SW HILL CREEK DRIVE

LAKE CITY

FL 32025

CONTRACTOR JOHN DUNN

PHONE 758.4581

LOCATION OF PROPERTY 41S, TR ON TUSTENUGGEE, TL ON HILL CREEK DRIVE, AND IT'S
TH 3RD HOME ON THE L.

TYPE DEVELOPMENT DETACHED GARAGE ESTIMATED COST OF CONSTRUCTION 50000.00

HEATED FLOOR AREA TOTAL AREA HEIGHT 24.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 8'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 05-5S-17-09116-115 SUBDIVISION HILLS AT ROSE CREEK

LOT 15 BLOCK PHASE 1 UNIT TOTAL ACRES 5.03

OWNER

Culvert Permit No. Culvert Waiver Contractor's License Number

Applicant/Owner/Contractor

EXISTING X-07-144

JTH

BLK

N

Driveway Connection Septic Tank Number

LU & Zoning checked by

Approved for Issuance

New Resident

COMMENTS: DETACHED GARAGE. ACCESSORY USE.

Check # or Cash 1273

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by

Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by

Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by

Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by

Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by

M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 250.00 CERTIFICATION FEE \$ 0.00 SURCHARGE FEE \$ 0.00

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$

FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ TOTAL FEE 325.00

INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0703-87 Date Received 3/29 By JW Permit # 25701
 Application Approved by - Zoning Official BLK Date 02.04.07 Plans Examiner AKTH Date 4-6-07
 Flood Zone X plat Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
 Comments Accessory Use 1273

☐ NOC ☒ EH ☐ Deed or PA ☐ Site Plan ☒ State Road Info ☒ Parent Parcel # ☒ Development Permit

Name Authorized Person Signing Permit JOHN DUNN Fax 758-7977 Phone 758-4501 (with # 758-1908)

Address 871 S.W. HILL CREEK DR LAKE CITY FL 32025

Owners Name JOHN & ELAINE DUNN Phone 758-4501

911 Address SAME AS ABOVE

Contractors Name SAME "A" Phone SAME

Address _____

Fee Simple Owner Name & Address _____

Bonding Co. Name & Address _____

Architect/Engineer Name & Address TINA RUFFO & MARK DISAWAY

Mortgage Lenders Name & Address CASH

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 05-58-17-0916-115 Estimated Cost of Construction \$50,000

Subdivision Name HILL'S AT ROSE CREEK Lot 15 Block _____ Unit _____ Phase 1

Driving Directions FROM HWY 90 S. TO TUSTENUGGEE RD S. WEST TO HILL'S AT ROSE CREEK LEFT ON HILL CREEK DR 3RD HOUSE ON LEFT

Type of Construction FRAME - Detached - garage Number of Existing Dwellings on Property 1

Total Acreage 5.03 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 200' Side 77' Side 197' Rear 389'

Total Building Height 24' Number of Stories 1 Heated Floor Area 2916 Roof Pitch 12-8
TOTAL 2916

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Authorized Person by Notarized Letter _____ Contractor Signature _____
 STATE OF FLORIDA _____ Contractors License Number _____
 COUNTY OF COLUMBIA _____ Competency Card Number _____



Sworn to (or affirmed) and subscribed before me _____
 this 29 day of March 20 07 _____
 Personally known _____ or Produced Identification ☒

Notary Signature Laurie Hodson
 (Revised Sept. 2006)

FILED IN 500-461-501-0850

NOTORIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THEIR OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

- ☐ Single Family Dwelling
☐ Farm Outbuilding

☐ Two-Family Residence

☒ Other GARAGE

NEW CONSTRUCTION OR IMPROVEMENT

☒ New Construction

☐ Addition, Alteration, Modification or other Improvement

I JOHN DOWD, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

John Dowd 3.23.07
Owner Builder Signature Date

The above signer is personally known to me or produced identification FL Drivers License

Notary Signature Laurie Hodson Date 3-29-07



(Stamp / Seal)

FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date _____ Building Official/Representative _____

FL DL D-500-462-50-285-0.

@ CAM112M01 S CamaUSA Appraisal System
 3/29/2007 13:48 Legal Description Maintenance
 Year T Property Sel
 2007 R 05-5S-17-09116-115
 871 HILL CREEK DR SW LAKE CITY
 HX DUNN JOHN & ELAINE

	Columbia	County
50000	Land	001 *
	AG	000
360503	Bldg	001
16838	Xfea	002
427341	TOTAL	B*

1	LOT 15, HILLS AT ROSE CREEK S/D PHASE 1., WD 1017-825,,	2
3	WD 1050-603.,	4
5		6
7		8
9		10
11		12
13		14
15		16
17		18
19		20
21		22
23		24
25		26
27		28

Mnt 7/26/2005 KYLIE

F1=Task F3=Exit F4=Prompt F10=GoTo PgUp/PgDn F24=More

Please give this application number on inquiries before the permit is issued / Application No. 0703-87

Owners name JOHN DUNN

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION

We require 24 hours notice for any inspection. Must provide the permit number and the type of inspection needed when you call.

Inspectors are available in the office at 8:00 am until 10:00 am and from 1:30 pm until 3:00 pm - Monday through Friday.

Make sure to call 386-758-1058 for the Final on the Septic system.

improvement will be made to certain real property, and is, the following information is provided in this Notice of

MULT WITH YOUR LENDER OR AN ATTORNEY BEFORE ENT.

7/16/15

Permit Number #25701

1. Description of property: (legal description of the property and street address or 911 address)

871 S.W. HILLCREST DR
LAKE CITY, FL 32025

2. General description of improvement: DETACHED GARAGE

3. Owner Name & Address JOHN & ELAINE DUNN 871 S.W. HILLCREST DR
LAKE CITY, FL 32025 Interest in Property OWNER

4. Name & Address of Fee Simple Owner (if other than owner):

5. Contractor Name JOHN DUNN (OWNER) Phone Number 758-4581
Address 871 S.W. HILLCREST DR LAKE CITY, FL 32025

6. Surety Holders Name _____ Phone Number _____
Address _____

Amount of Bond _____ Inst:2007007892 Date:04/09/2007 Time:10:51

7. Lender Name DC, P. Dewitt Cason, Columbia County B:1115 P:2338
Address _____

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name _____ Phone Number _____
Address _____

9. In addition to himself/herself the owner designates _____ of _____
to receive a copy of the Lien Notice as provided in Section 713.13 (1) -

(a) 7. Phone Number of the designee _____

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified) _____

THE OWNER MUST SIGN THE NOTICE OF COMMENCEMENT AND NO ONE ELSE MAY BE PERMITTED TO SIGN IN HIS/HER STEAD.



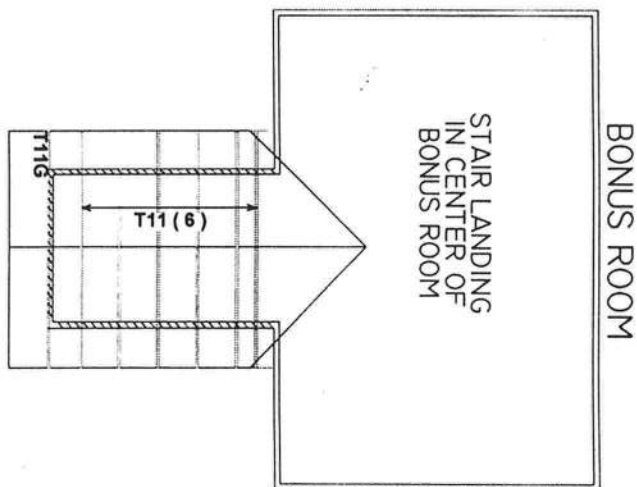
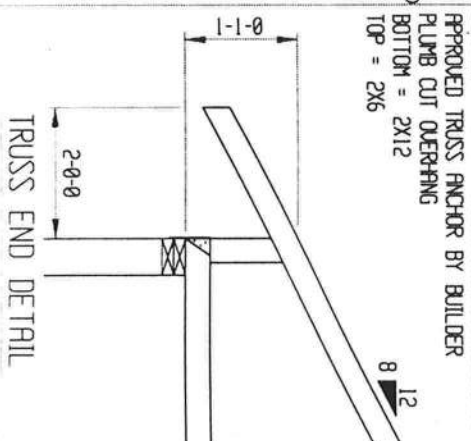
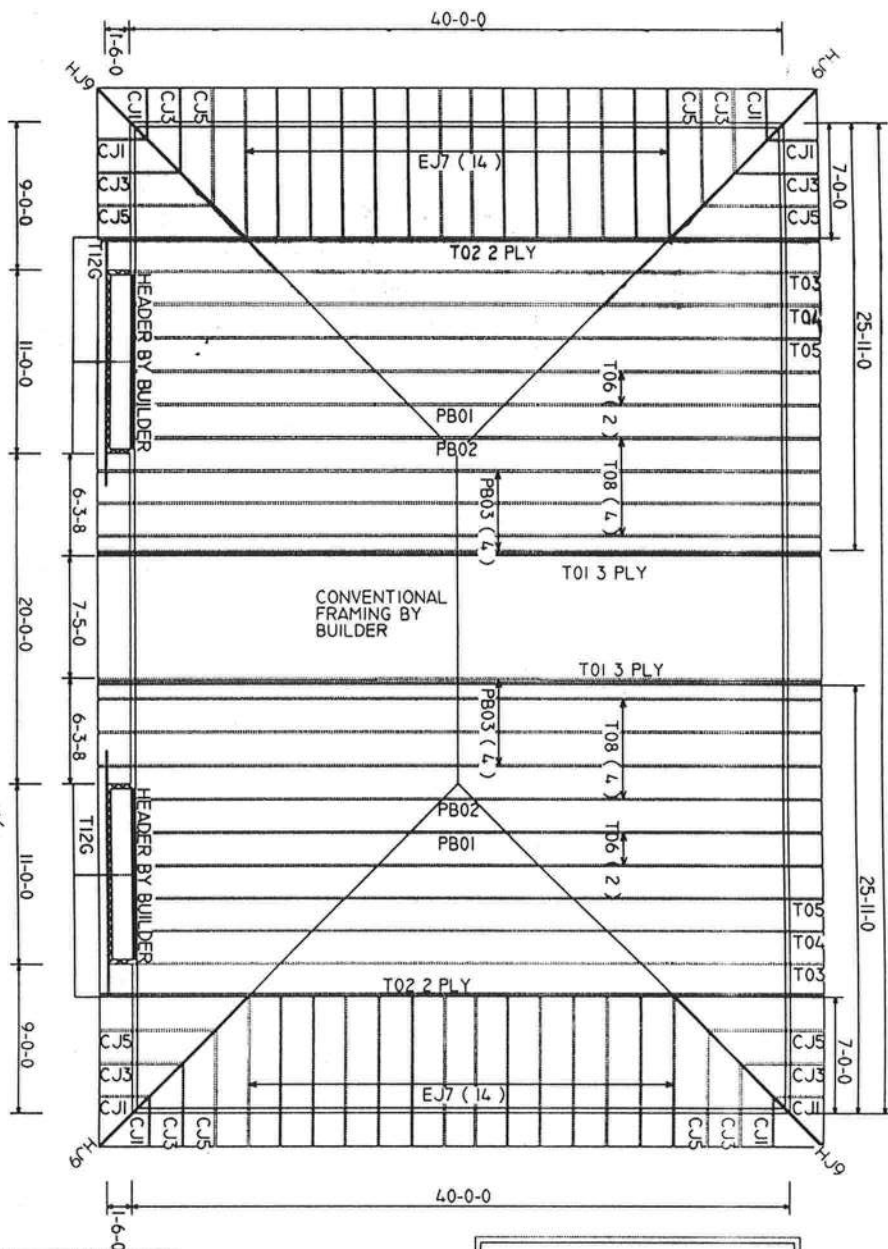
John F. Dunn
Signature of Owner

Sworn to (or affirmed) and subscribed before day of 3-29, 2007.

Laurie Hodson
Signature of Notary

NOTARY STAMP/SEAL

FL OL D-500-466-50. 2850.

[illegible]

Job: L229893 Truss: T01 Truss Type: ATTIC Qty: 2 Ply: 3 JOHN DUNN
6.300 s Apr 19 2006 MITek Industries, Inc. Wed Mar 07 07:44:57 2007 Page 1

Builders FirstSource, Lake City, FL 32055

Scale = 1:73.7

Plate Offsets (X,Y): [4:0-5.0,0-4.8], [5:0-6.0,Edge], [11:0-6.0,Edge], [12:0-5.0,0-4.8], [17:0-3.8,0-6.0], [19:0-3.8,0-6.0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	4-2-0	TC 0.86	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 1.00	Vert(LL) -0.28 17-19 >999 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.63	Vert(TL) -0.45 17-19 >999 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.06 14 n/a n/a		
	Code FBC2004/TPI2002		Wind(LL) 0.20 17 >999 240		
				Weight: 1300 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.10 *Except*
T2 2 X 8 SYP 2400F 2.0E, T2 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 12 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.)
(Switched from sheathed: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 7, 9, 21

REACTIONS (lb/size) 2=7801/0-4-0, 14=10127/0-4-0
Max Horz 2=757(load case 3)
Max Uplift 2=1047(load case 4), 14=2199(load case 5)

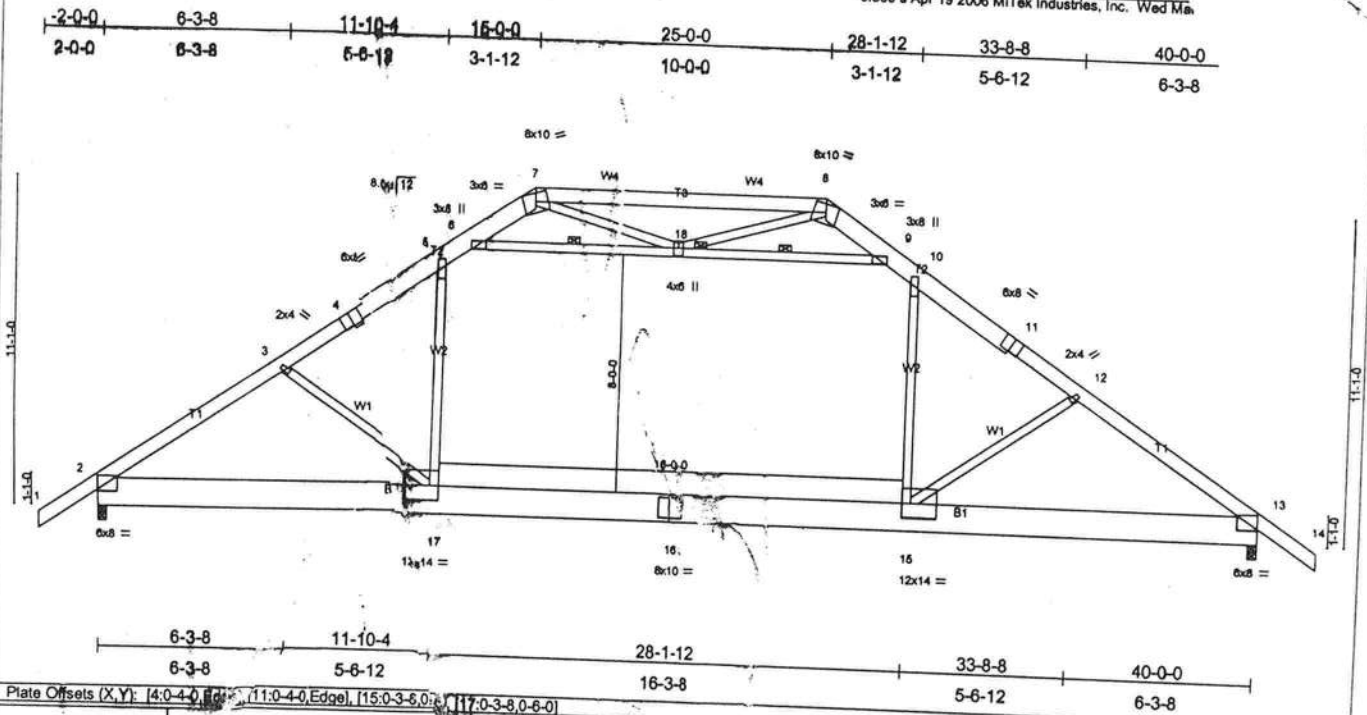
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/102, 2-3=10413/999, 3-4=10118/1064, 4-5=10602/1026, 5-6=7789/1093, 6-7=1265/876, 9-10=1862/1245, 10-11=7624/1010, 11-12=11349/1354, 12-13=12692/2304, 13-14=13757/2634, 14-15=0/102, 7-8=1773/1159, 8-9=1773/1159
BOT CHORD 2-20=1062/8155, 19-20=875/8712, 18-19=699/8536, 17-18=699/8536, 16-17=1043/10033, 14-16=1725/10427
WEBS 6-21=7971/1090, 10-21=7028/620, 5-19=141/4737, 11-17=549/5560, 4-19=1001/443, 4-20=1236/717, 3-20=28/602, 12-17=3380/2039, 12-16=1330/1157, 13-16=183/660, 8-21=109/416, 7-21=1162/1583, 9-21=541/587

JOINT STRESS INDEX
2 = 0.75, 3 = 0.34, 4 = 0.33, 5 = 0.43, 6 = 0.70, 7 = 0.22, 8 = 0.34, 9 = 0.22, 10 = 0.70, 11 = 0.43, 12 = 0.33, 13 = 0.34, 14 = 0.75, 16 = 0.20, 17 = 0.28, 18 = 0.76, 19 = 0.28, 20 = 0.20 and 21 = 0.31

NOTES
1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 12 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Ceiling dead load (5.0 psf) on member(s), 5-6, 10-11, 6-21, 10-21; Wall dead load (5.0psf) on member(s), 5-19, 11-17
7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room, 17-19
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1047 lb uplift at joint 2 and 2199 lb uplift at joint 14.
10) Load case(s) 1, 8, 9, 10 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 2-19=229(F=167), 17-19=229, 14-17=229(F=167), 1-5=112, 5-6=133, 6-7=112, 9-10=112, 10-11=133, 14-15=112, 6-10=21, 7-9=112
Drag: 5-19=21, 11-17=21
Trapezoidal Loads (plf)
Vert: 11=334(F=222) to 14=441(F=328)
2) Attic Floor: Lumber Increase=1.00, Plate Increase=1.00

Job L229893	Truss T08	Truss Type PIGGYBACK ATTIC	Qty 8	Ply 1	JOHN DUNN
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MITek Industries, Inc. Wed Ma		



LOADING (psf)		SPACING		CSJ		DEFL		PLATES		GRIP	
TCCL	20.0	Plates Increase	1.25	TC	0.32	Vx(L)	in (loc)	L/defl	L/d	MT20	244/190
TCOL	7.0	Lumber Increase	1.25	BC	0.90	Ver(TL)	-0.34 15-17	>999	360		
BCCL	10.0	Rep Stress Incr	NO	WB	0.79	Horz(TL)	0.06 13	n/a	n/a		
BCDL	5.0	Code FBC2004/TP12002		(Matrix)		Wind(LL)	0.16 2-17	>999	240		
										Weight: 402 lb	

LUMBER	BRACING
TOP CHORD 2 X 6 SYP No.1D "Except"	TOP CHORD Structural wood sheathing directly applied or 4-1-3 oc purlins.
T2 2 X 8 SYP 2400F 2.0E, T2 2 X 8 SYP 2400F 2.0E	BOT CHORD Rigid ceiling directly applied or 8-6-15 oc bracing.
BOT CHORD 2 X 12 SYP No.2	WEBS 1 Row at midpt 6-18, 9-18
WEBS 2 X 4 SYP No.3	JOINTS 1 Brace at Jt(s): 18

REACTIONS (lb/ft)	2-0-0=2598/0-3-8, 13=2598/0-3-8
Max Lrzd	397 (load case 4)
Max Uplift	397 (load case 5), 13=397 (load case 6)

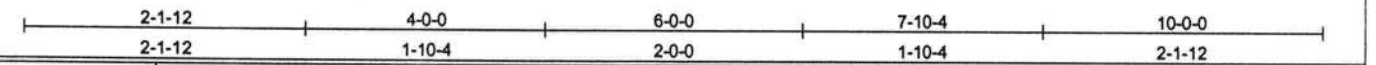
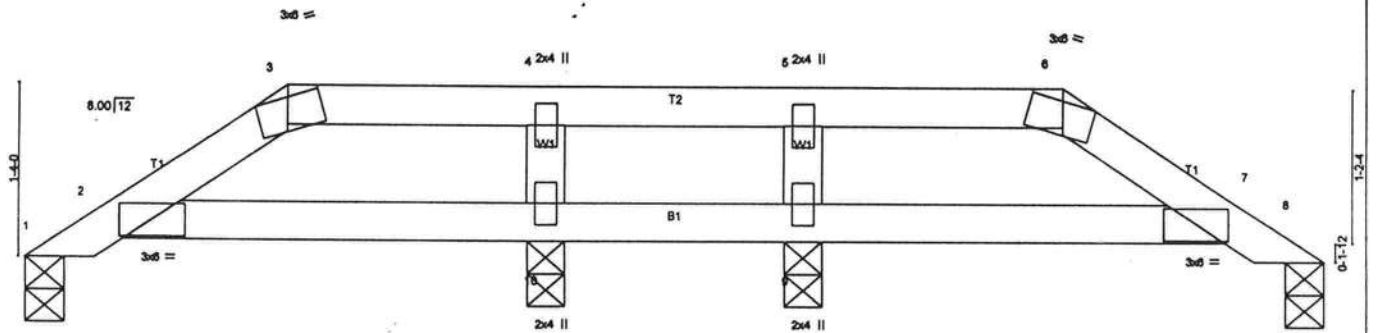
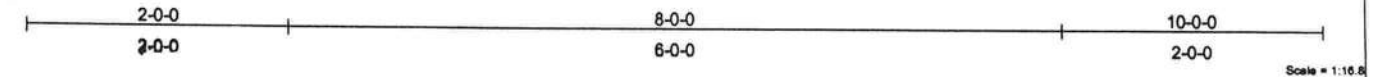
FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	-2-0-0=3938/274, 3-4=3698/209, 4-5=3541/233, 5-6=3473/310, 6-7=785/667, 8-9=766/666, 9-10=2752/310, 10-11=3541/232, 11-12=3698/209, 12-13=3938/275, 13-14=0/49, 14-15=753/44
BOT CHORD	2-17=2463/385, 18-17=282/959, 15-16=282/959, 16-15=0/165
WEBS	5-17=0/1389, 10-15=0/1389, 6-18=2447/102, 9-18=2447/102, 17=237/335, 12-15=237/334, 7-18=73/379, 8-18=72/379

JOINT STRESS INDEX	2 = 0.65, 4 = 0.34, 5 = 0.43, 6 = 0.82, 7 = 0.33, 8 = 0.33, 9 = 0.82, 10 = 0.43, 11 = 0.34, 12 = 0.34, 13 = 0.65, 15 = 0.25, 16 = 0.97, 17 = 0.25 and 18 = 0.36
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- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASE 7-0-2; 110mph (3-second gust); h=20ft; TCCL=4.2psf BCDL=3.0psf category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior (1st floor); Lumber DOL=1.60 plate grip DOL=1.60. This was designed I.C. for members and for, and for MWFRS for reactions specified.
 - Provide adequate drainage to prevent water ponding.
 - Ceiling dead load (5.0 psf) on member(s): 5-6, 9-10, 6-18, 8-18. Wall dead load (10.0 psf) on member(s): 5-17, 10-15
 - Bottom chord live load (40.0 psf) and additional bottom chord load (10.0 psf) applied only to room: 15-17
 - All bearers are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - Provide mechanical connection (by others) of truss to be 1/8" plate capable of withstanding 397 lb uplift at joint 2 and 397 lb uplift at joint 13.

LOAD CASES Standard

Job L229893	Truss PB01	Truss Type HIP PIGGYBACK	Qty 2	Ply 1	JOHN DUNN
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2008 Mitek Industries, Inc. Wed Mar 07 07:38:33 2007 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.16	In (loc) I/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.11	Vert(LL) -0.01 7-9 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.03	Vert(TL) -0.02 7-9 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.01 8 n/a n/a		
	Code FBC2004/TPI2002			Weight: 31 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 1=164/0-3-8, 9=233/0-3-8, 10=233/0-3-8, 8=164/0-3-8
 Max Horz 1=-44(load case 3)
 Max Uplift 1=-58(load case 5), 9=-119(load case 3), 10=-125(load case 4), 8=-60(load case 6)
 Max Grav 1=164(load case 1), 9=252(load case 10), 10=252(load case 9), 8=164(load case 1)

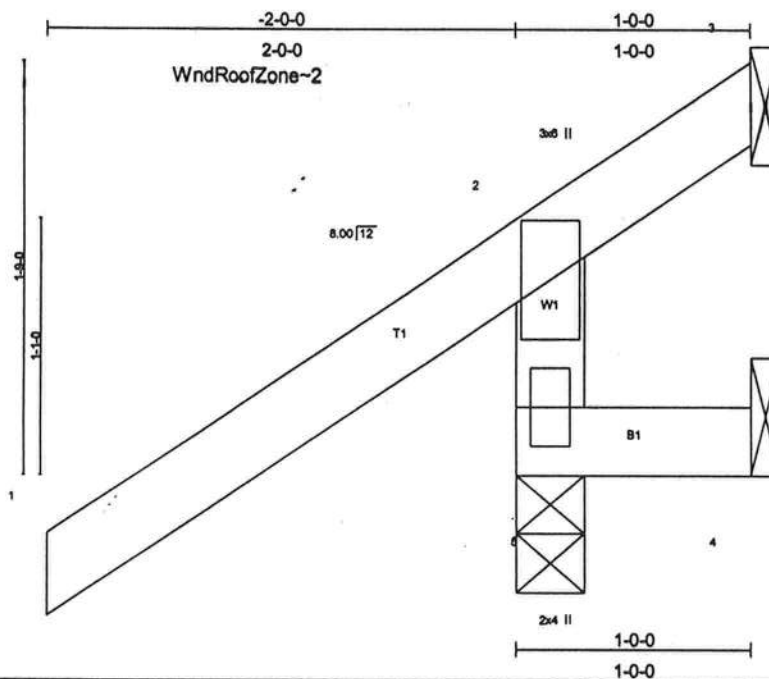
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-84/49, 2-3=-170/83, 3-4=-139/86, 4-5=-139/86, 5-6=-139/86, 6-7=-170/83, 7-8=-84/37
 BOT CHORD 2-10=-67/139, 9-10=-67/139, 7-9=-67/139
 WEBS 5-9=-144/119, 4-10=-144/122

JOINT STRESS INDEX
 2 = 0.25, 3 = 0.06, 4 = 0.07, 5 = 0.07, 6 = 0.06, 7 = 0.25, 9 = 0.07 and 10 = 0.07

- NOTES
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Provide adequate drainage to prevent water ponding.
 - All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 58 lb uplift at joint 1, 119 lb uplift at joint 9, 125 lb uplift at joint 10 and 60 lb uplift at joint 8.

LOAD CASE(S) Standard

Job L229893	Truss CJ1	Truss Type JACK	Qty 8	Ply 1	JOHN DUNN
Builders FirstSource, Lake City, FL 32085			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Mar 07 07:38:08 2007 Page 1		



LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.34	In (loc) 5	MT20	244/190
TCCL 7.0	Lumber Increase 1.25	BC 0.05	Vert(LL) 0.00		
BCCL 10.0	Rep Stress Incr YES	WB 0.00	Vert(TL) 0.00		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)	Horz(TL) -0.00		
				Weight: 8 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 5=299/0-3-8, 4=-20/Mechanical, 3=-93/Mechanical
 Max Horz 5=118(load case 5)
 Max Uplift 5=-237(load case 5), 4=-20(load case 1), 3=-93(load case 1)
 Max Grav 5=299(load case 1), 4=1(load case 3), 3=100(load case 5)

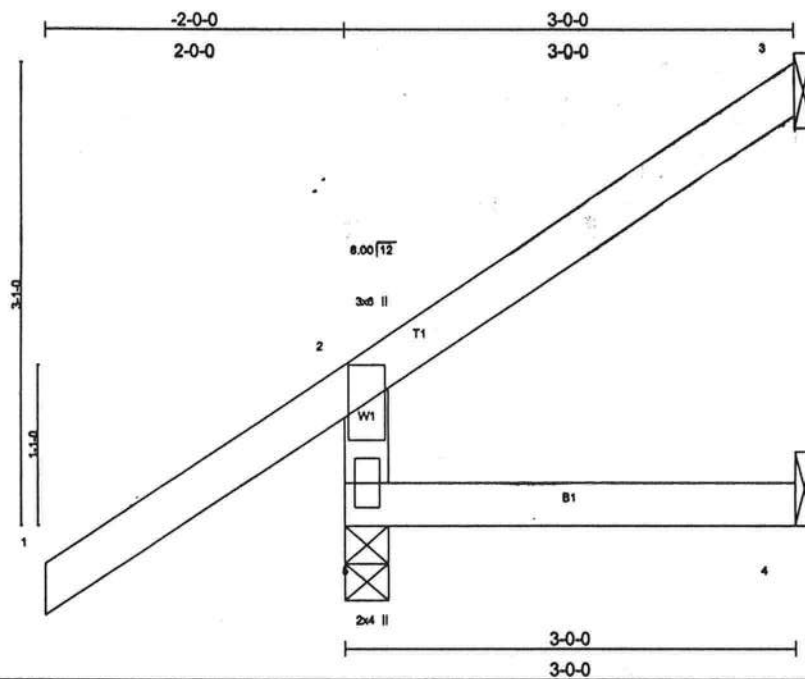
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 2-5=-254/294, 1-2=0/64, 2-3=-77/65
 BOT CHORD 4-5=0/0

JOINT STRESS INDEX
 2 = 0.38 and 5 = 0.29

NOTES
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) All bearings are assumed to be SYP No.2 crushing capacity of 585,000 psi
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 5, 20 lb uplift at joint 4 and 93 lb uplift at joint 3.

LOAD CASE(S) Standard

Job L229893	Truss CJ3	Truss Type JACK	Qty 8	Ply 1.	JOHN DUNN
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MITek Industries, Inc. Wed Mar 07 07:38:13 2007 Page 1		



LOADING (psf)	SPACING 2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCCL 20.0	Plates Increase 1.25	TC 0.41	Vert(LL)	-0.00	5	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.07	Vert(TL)	-0.00	4-5	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	-0.01	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)						Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=278/0-3-8, 3=44/Mechanical, 4=29/Mechanical
Max Horz 5=178(load case 5)
Max Uplift 5=151(load case 5), 3=55(load case 5), 4=4(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-5=-223/199, 1-2=0/64, 2-3=-56/16
BOT CHORD 4-5=0/0

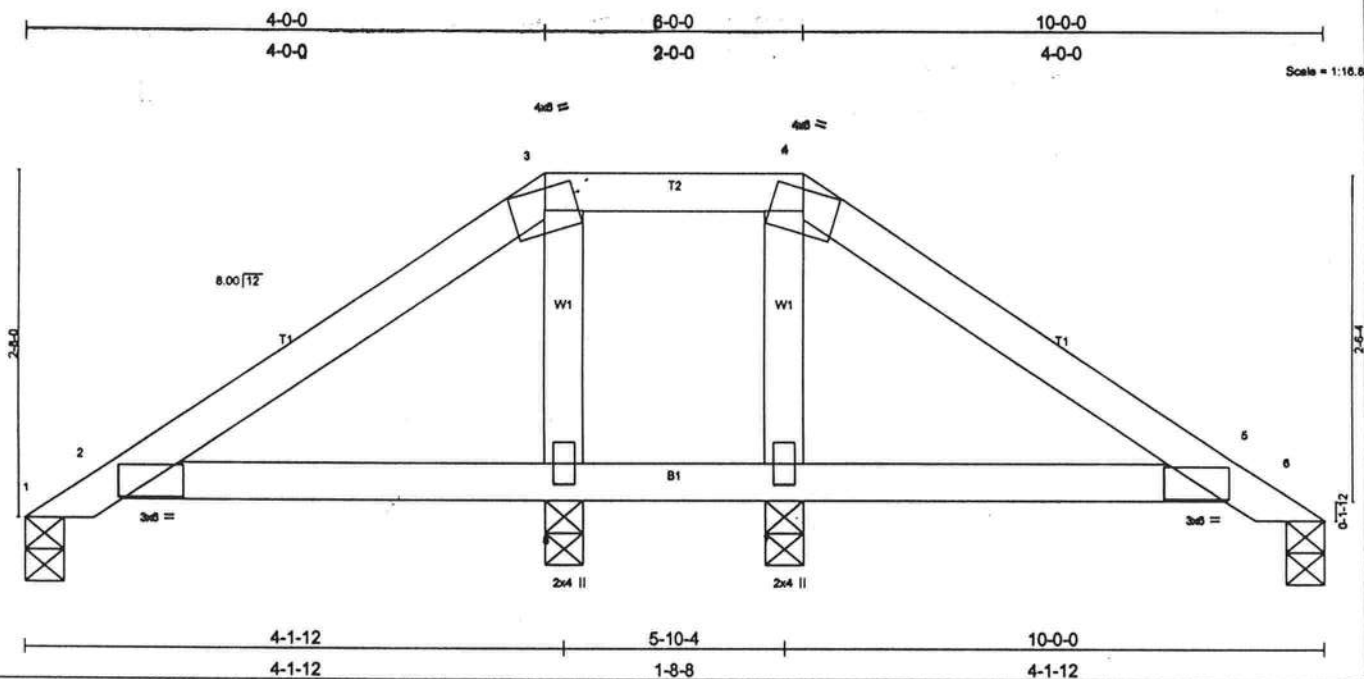
JOINT STRESS INDEX
2 = 0.50 and 5 = 0.40

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 151 lb uplift at joint 5, 55 lb uplift at joint 3 and 4 lb uplift at joint 4.

LOAD CASE(S) Standard

Job L229893	Truss PB02	Truss Type HIP PIGGYBACK	Qty 2	Ply 1	JOHN DUNN
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MITek Industries, Inc. Wed Mar 07 07:38:38 2007 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.13	in (loc)	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.09	Vert(LL) -0.01 5-7 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.05	Vert(TL) -0.01 5-7 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 6 n/a n/a		
	Code FBC2004/TP12002			Weight: 36 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 8-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-0-0 oc bracing.

REACTIONS (lb/size) 1=30/0-3-8, 8=367/0-3-8, 7=367/0-3-8, 6=30/0-3-8
 Max Horz 1=90(load case 4)
 Max Uplift 1=-11(load case 10), 8=-153(load case 5), 7=-137(load case 6), 6=-33(load case 3)
 Max Grav 1=55(load case 9), 8=374(load case 9), 7=374(load case 10), 6=55(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-80/86, 2-3=-97/270, 3-4=-32/196, 4-5=-80/270, 5-6=-27/22
 BOT CHORD 2-8=-178/123, 7-8=-198/128, 5-7=-178/119
 WEBS 3-8=-274/148, 4-7=-274/132

JOINT STRESS INDEX
 2 = 0.23, 3 = 0.34, 4 = 0.34, 5 = 0.23, 7 = 0.10 and 8 = 0.10

NOTES

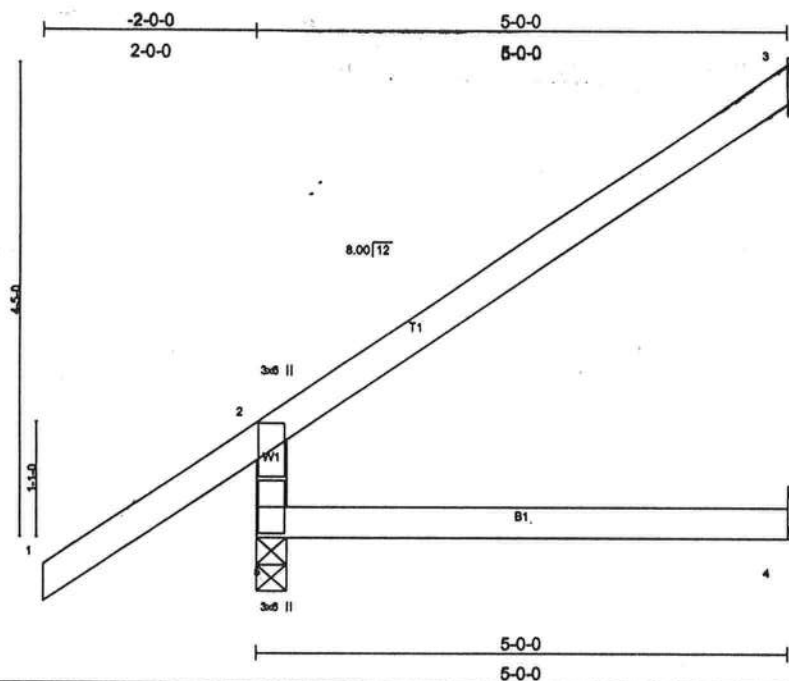
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 585.00 psi
- Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 1, 153 lb uplift at joint 8, 137 lb uplift at joint 7 and 33 lb uplift at joint 6.

LOAD CASE(S) Standard

Job L229893	Truss CJ5	Truss Type JACK	Qty 8	Ply 1	JOHN DUNN
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Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.36	Vert(LL) 0.03	4-5	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.17	Vert(TL) -0.04	4-5	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.03	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)					Weight: 21 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=343/0-3-8, 3=107/Mechanical, 4=68/Mechanical
Max Horz 5=240/load case 5)
Max Uplift 5=145/load case 5), 3=120/load case 5), 4=-7/load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-5=-268/211, 1-2=0/64, 2-3=-96/45
BOT CHORD 4-5=0/0

JOINT STRESS INDEX
2 = 0.44 and 5 = 0.35

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 5, 120 lb uplift at joint 3 and 7 lb uplift at joint 4.

LOAD CASE(S) Standard

Truss Type	Qty	Ply	JOHN DUNN
ATTIC	2	3	Job Reference (optional)

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.29, 14-17=-229(F=-167), 1-5=-29, 5-6=-50, 6-7=-29, 9-10=-29, 10-11=-50, 14-15=-29, 6-10=-21, 7-9=-29

.2(F=-123)
ase=1.25, Plate Increase=1.25

-19=-229, 14-17=-229(F=-167), 1-5=-113, 5-6=-133, 6-7=-112, 9-10=-29, 10-11=-50, 14-15=-29, 6-10=-21, 7-9=-112

-14=-358(F=-328)
er Increase=1.25, Plate Increase=1.25

10) 2nd u...
Uniform Load

Vert: 2-19=-229, 17-19=-229, 14-17=-229(F=-167), 1-5=-29, 5-6=-50, 6-7=-29, 9-10=-112, 10-11=-133, 14-15=-112, 6-10=-21, 7-9=-112

Drag: 5-19=-21, 1, 17=-21

Trapezoidal Loads (plf)

Vert: 11=-334(F=-222)-to-14=-441(F=-328)

Job L229893	Truss T02	Truss Type HIP	Qty 2	Ply 2	JOHN DUNN
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MITek Industries, Inc. Wed Mar 07 07:38:54 2007 Page 1		

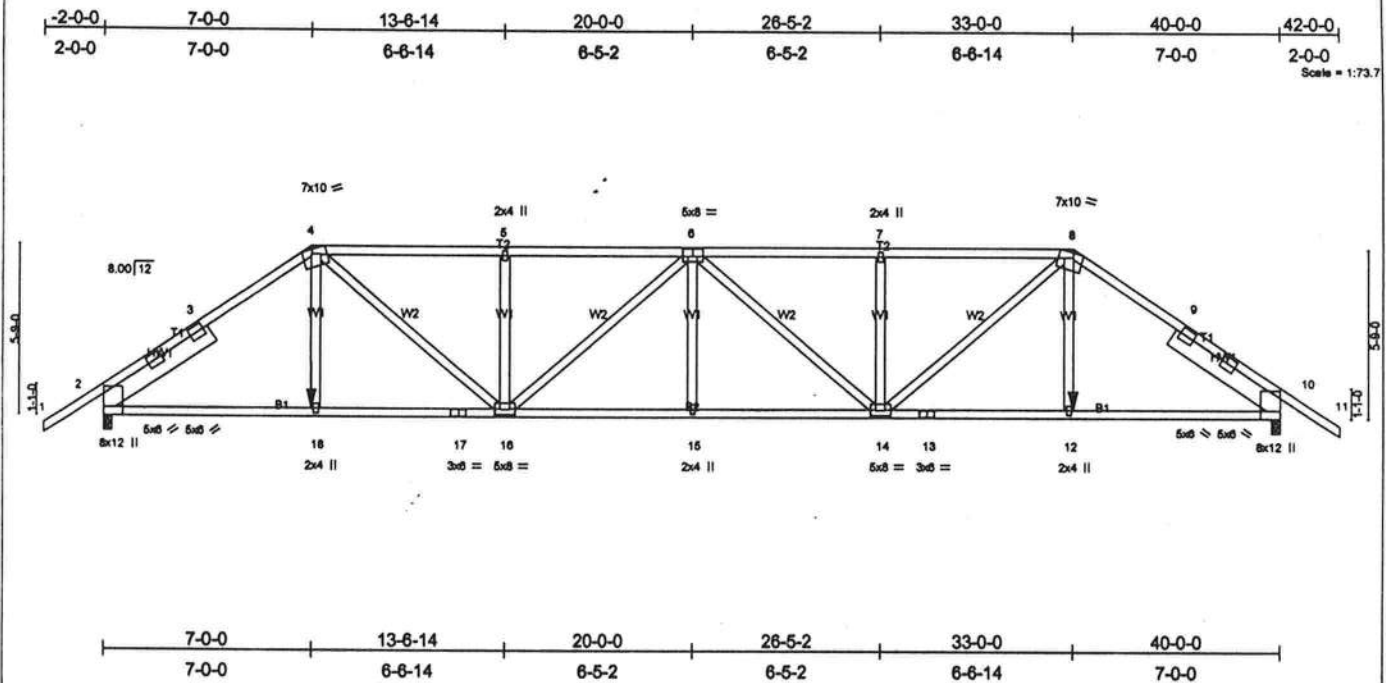


Plate Offsets (X,Y): [4:0-4-0,Edge], [8:0-4-0-3-0], [8:0-4-0,Edge]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)
TCLL 20.0	Plates Increase	1.25	TC 0.95	Vert(LL)	0.36 15 >999
TCDL 7.0	Lumber Increase	1.25	BC 0.85	Vert(TL)	-0.53 15-16 >907
BCCL 10.0	Rep Stress Incr	NO	WB 0.52	Horz(TL)	0.16 10 n/a
BCDL 5.0	Code FBC2004/TP12002		(Matrix)		
			PLATES		GRIP
			MT20		244/190
			Weight: 489 lb		

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.1D "Except"	TOP CHORD Structural wood sheathing directly applied.
T2 2 X 4 SYP No.2, T2 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-7-1 oc bracing.
BOT CHORD 2 X 4 SYP No.2	
WEBS 2 X 4 SYP No.3	
SLIDER Left 2 X 8 SYP No.1D 4-5-7, Right 2 X 8 SYP No.1D 4-5-7	

REACTIONS (lb/size) 2=3607/0-3-8, 10=3607/0-3-8
Max Horiz 2=-192(load case 2)
Max Uplift 2=-2019(load case 3), 10=-2019(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/30, 2-3=-5274/3106, 3-4=-5153/3122, 4-5=-6389/3968, 5-6=-6389/3969, 6-7=-6389/3969, 7-8=-6389/3968, 8-9=-5153/3122, 9-10=-5274/3106, 10-11=0/30
BOT CHORD 2-18=-2560/4088, 17-18=-2568/4109, 16-17=-2568/4109, 15-16=-4332/7025, 14-15=-4332/7025, 13-14=-2379/4109, 12-13=-2379/4109, 10-12=-2372/4088
WEBS 4-18=-308/778, 4-16=-1981/3022, 5-16=-908/963, 6-16=-863/562, 6-15=0/399, 6-14=-863/561, 7-14=-908/963, 8-14=-1981/3022, 8-12=-308/778

JOINT STRESS INDEX
2 = 0.52, 2 = 0.48, 2 = 0.00, 4 = 0.78, 5 = 0.34, 6 = 0.49, 7 = 0.34, 8 = 0.78, 9 = 0.00, 10 = 0.52, 10 = 0.48, 10 = 0.48, 12 = 0.34, 13 = 0.72, 14 = 0.71, 15 = 0.34, 16 = 0.71, 17 = 0.72 and 18 = 0.34

- NOTES
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 - Provide adequate drainage to prevent water ponding.
 - All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psf
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2019 lb uplift at joint 2 and 2019 lb uplift at joint 10.
 - Girder carries hip end with 7-0-0 end setback.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 410 lb up at 33-0-0, and 539 lb down and 410 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-8=-118(F=-64), 8-11=-54, 2-18=-30, 12-18=-65(F=-35), 10-12=-30
Concentrated Loads (lb)
Vert: 18=-539(F) 12=-539(F)

Job L229893	Truss T03	Truss Type HIP	Qty 2	Ply 1	JOHN DUNN
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MITek Industries, Inc. Wed Mar 07 07:38:59 2007 Page 1		

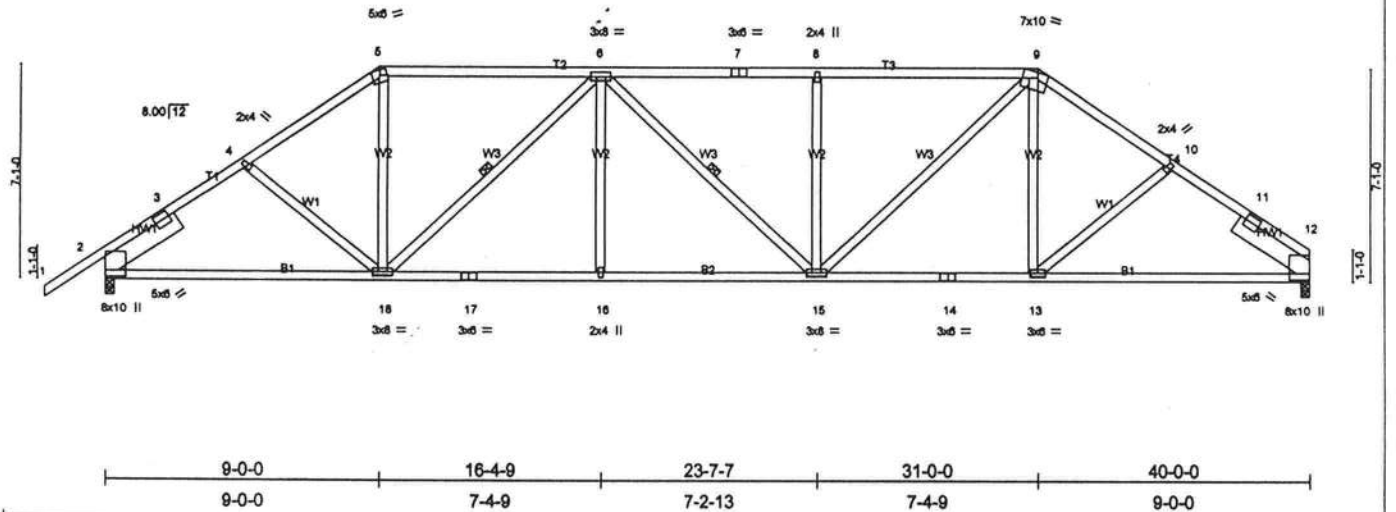
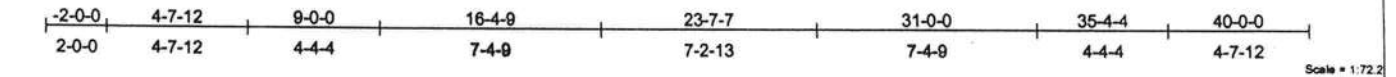
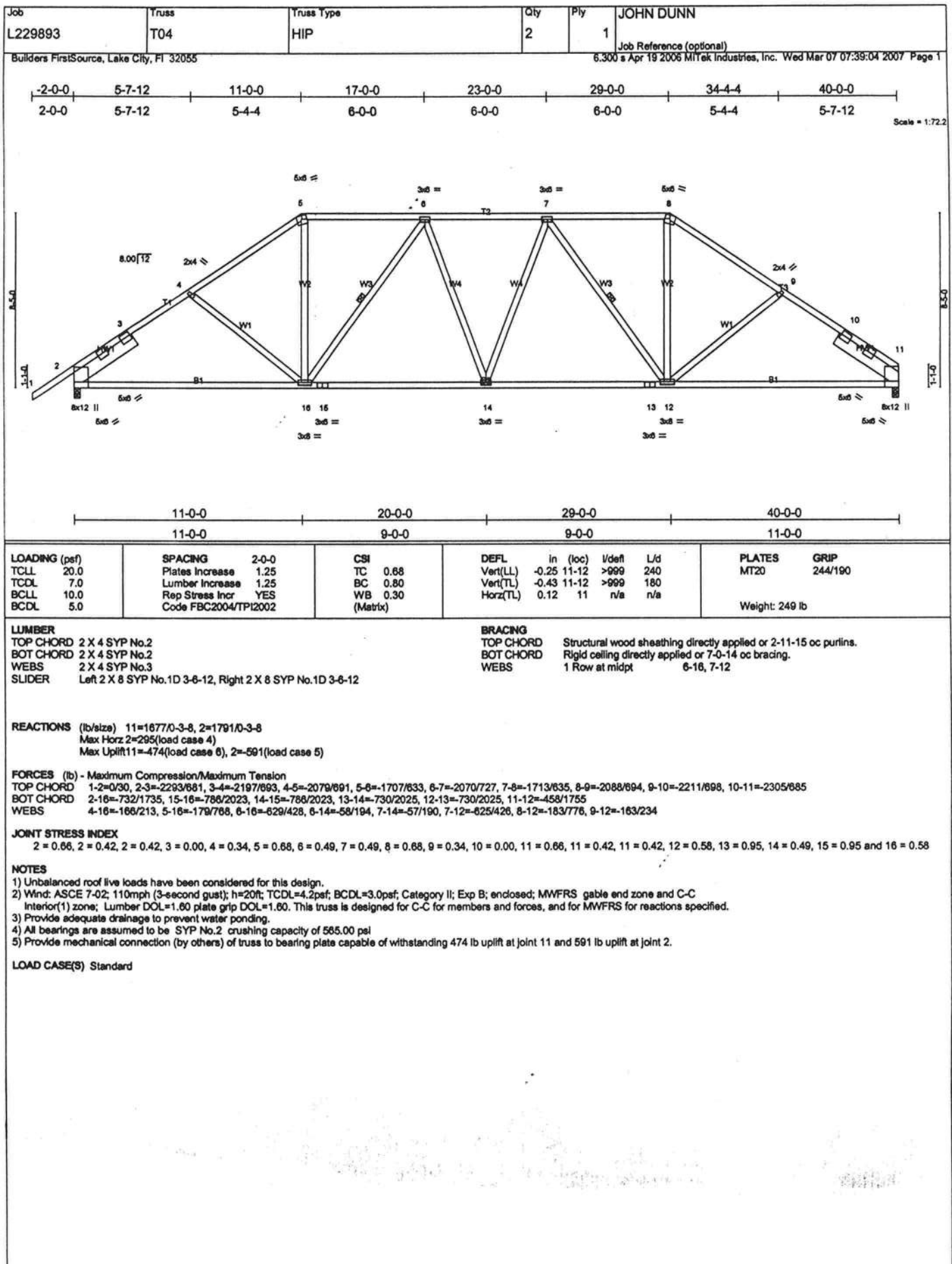


Plate Offsets (X,Y): [2-0-7-13,0-0-1], [9-0-4-0,Edge], [12-0-7-13,0-0-1]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	L/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.62	Vert(LL)	-0.23 15-16	>999	240
TCCL 7.0	Lumber Increase	1.25	BC 0.68	Vert(TL)	-0.38 15-16	>999	180
BCLL 10.0	Rep Stress Incr	YES	WB 0.90	Horz(TL)	0.13 12	n/a	n/a
BCCL 5.0	Code FBC2004/TP12002		(Matrix)				
				PLATES GRIP			
				MT20 244/190			
				Weight: 245 lb			

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-2-8 oc purlins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 6-3-2 oc bracing.
WEBS	2 X 4 SYP No.3	WEBS	1 Row at midpt 6-18, 6-15
SLIDER	Left 2 X 8 SYP No.1D 2-11-8, Right 2 X 8 SYP No.1D 2-11-8		

REACTIONS (lb/size)		12=1677/0-3-8, 2=1791/0-3-8
Max Horz		2=248(load case 4)
Max Uplift		12=531(load case 3), 2=573(load case 5)
FORCES (lb) - Maximum Compression/Maximum Tension		
TOP CHORD		1-2=0/30, 2-3=2284/777, 3-4=2199/785, 4-5=-2142/804, 5-6=-1788/731, 6-7=-2438/992, 7-8=-2438/992, 8-9=-2439/992, 9-10=-2154/809, 10-11=-2217/792, 11-12=-2299/783
BOT CHORD		2-18=-758/1700, 17-18=-1017/2435, 16-17=-1017/2435, 15-16=-1017/2435, 14-15=-571/1783, 13-14=-571/1783, 12-13=-531/1725
WEBS		4-18=-197/205, 5-18=-194/785, 6-18=-952/527, 6-16=0/213, 6-15=-78/77, 8-15=-399/347, 9-15=-529/957, 9-13=-49/199, 10-13=-193/197
JOINT STRESS INDEX		
		2 = 0.81, 2 = 0.84, 3 = 0.00, 4 = 0.34, 5 = 0.78, 6 = 0.57, 7 = 0.48, 8 = 0.34, 9 = 0.66, 10 = 0.34, 11 = 0.00, 12 = 0.77, 12 = 0.84, 13 = 0.35, 14 = 0.69, 15 = 0.94, 16 = 0.34, 17 = 0.91 and 18 = 0.57

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Provide adequate drainage to prevent water ponding.
 - All bearings are assumed to be SYP No.2 crushing capacity of 565,000 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 531 lb uplift at joint 12 and 573 lb uplift at joint 2.
- LOAD CASE(S)** Standard



Job L229893	Truss T05	Truss Type HIP	Qty 2	Ply 1	JOHN DUNN
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MITek Industries, Inc. Wed Mar 07 07:39:10 2007 Page 1		

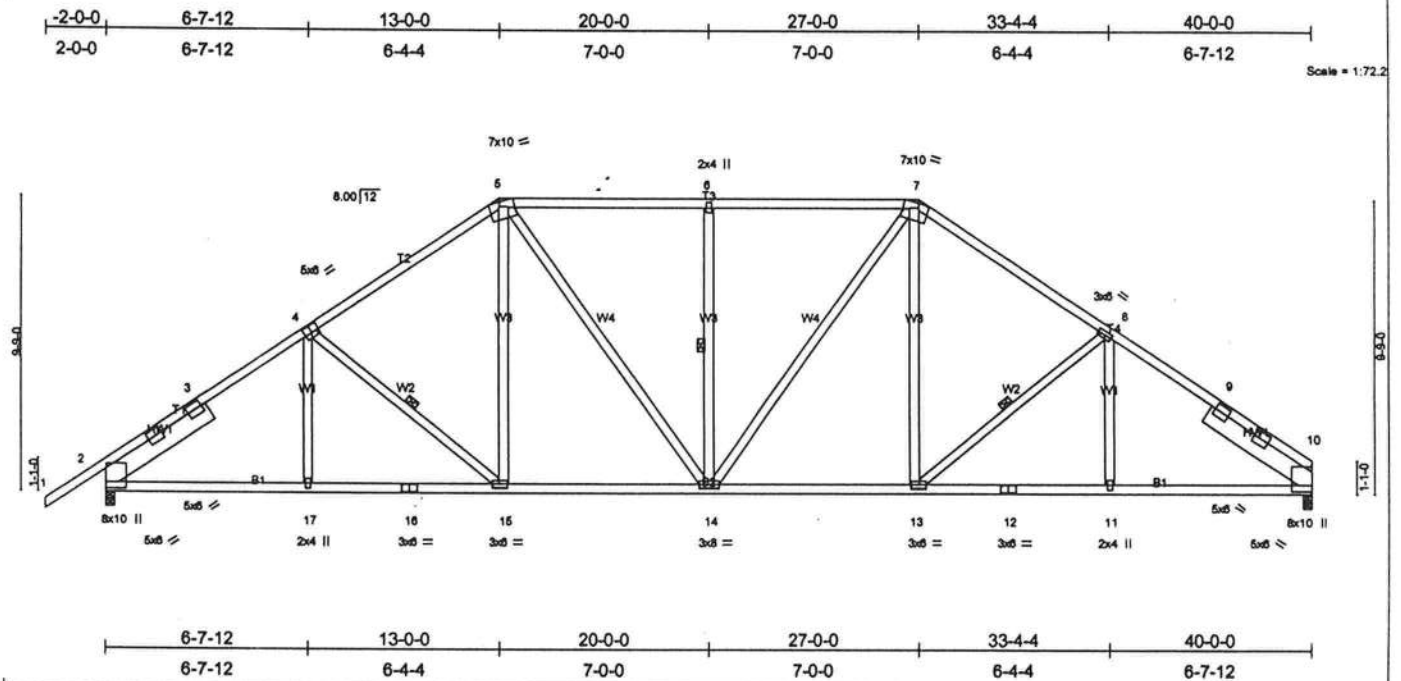


Plate Offsets (X,Y): [2-0-7-13,0-0-1], [4-0-3-0,0-3-0], [5-0-4-0,Edge], [7-0-4-0,Edge], [10-0-7-13,0-0-1]													
LOADING (psf)		SPACING		2-0-0		CSI		DEFL		PLATES		GRIP	
TCLL 20.0		Plates Increase		1.25		TC 0.85		in (loc) l/defl L/d		MT20		244/190	
TCDL 7.0		Lumber Increase		1.25		BC 0.74		Vert(LL) -0.17 13-14 >999 240					
BCLL 10.0		Rep Stress Incr		YES		WB 0.80		Vert(TL) -0.27 13-14 >999 180					
BCDL 5.0		Code FBC2004/TP12002				(Matrix)		Horz(TL) 0.11 10 n/a n/a					
										Weight: 269 lb			

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 7-9-2 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 4-15, 6-14, 8-13
SLIDER Left 2 X 8 SYP No.1D 4-1-13, Right 2 X 8 SYP No.1D 4-1-13	

REACTIONS (lb/size)	10=1677/0-3-8, 2=1791/0-3-8
Max Horz 2=341(load case 4)	
Max Uplift 10=490(load case 6), 2=606(load case 5)	
FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/30, 2-3=-2367/630, 3-4=-2252/661, 4-5=-1964/658, 5-6=-1803/678, 6-7=-1803/678, 7-8=-1971/663, 8-9=-2266/661, 9-10=-2379/655
BOT CHORD	2-17=-669/1784, 16-17=-670/1782, 15-16=-670/1782, 14-15=-571/1595, 13-14=-398/1599, 12-13=-417/1802, 11-12=-417/1802, 10-11=-417/1802
WEBS	4-17=0/163, 4-15=-265/261, 5-15=-125/379, 5-14=-336/471, 6-14=-374/348, 7-14=-335/467, 7-13=-134/387, 8-13=-284/280, 8-11=0/167
JOINT STRESS INDEX	
2 = 0.75, 2 = 0.44, 2 = 0.44, 3 = 0.00, 4 = 0.54, 5 = 0.74, 6 = 0.34, 7 = 0.74, 8 = 0.43, 9 = 0.00, 10 = 0.74, 10 = 0.44, 10 = 0.44, 11 = 0.34, 12 = 0.74, 13 = 0.35, 14 = 0.58, 15 = 0.35, 16 = 0.74 and 17 = 0.34	

- NOTES
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) All bearings are assumed to be SYP No.2 crushing capacity of 585.00 psi
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 490 lb uplift at joint 10 and 606 lb uplift at joint 2.

LOAD CASE(S) Standard

Job L229893	Truss T06	Truss Type HIP	Qty 4	Ply 1	JOHN DUNN
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Mar 07 07:39:15 2007 Page 1		

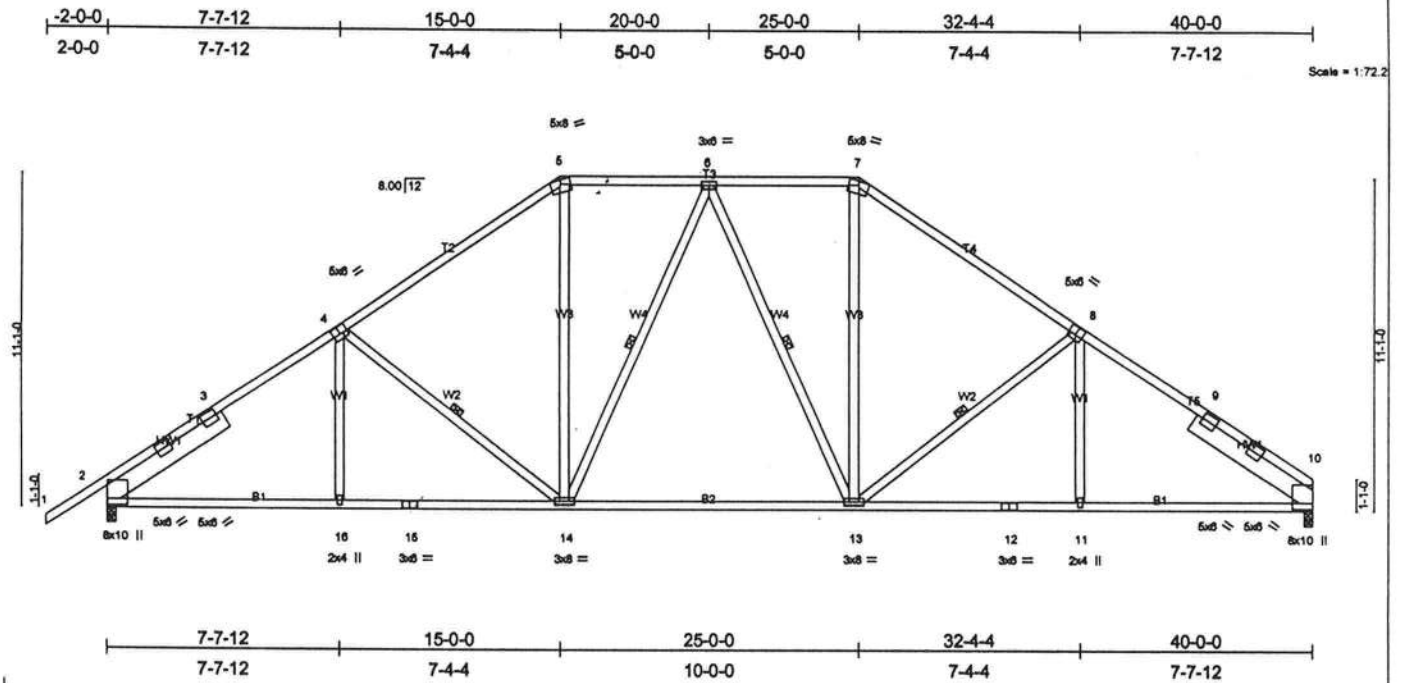


Plate Offsets (X,Y): [2:0-7-13,0-0-1], [4:0-3-0,0-3-0], [8:0-3-0,0-3-0], [10:0-7-13,0-0-1]									
LOADING (psf)	SPACING 2-0-0		CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25		TC 0.88	Vert(LL)	-0.27 13-14	>999	240	MT20	244/190
TCCL 7.0	Lumber Increase 1.25		BC 0.78	Vert(TL)	-0.46 13-14	>999	180		
BCCL 10.0	Rep Stress Incr YES		WB 0.31	Horz(TL)	0.11 10	n/a	n/a		
BCCL 5.0	Code FBC2004/TP12002		(Matrix)						
								Weight: 271 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 8-0-10 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 4-14, 6-14, 6-13, 8-13
SLIDER Left 2 X 8 SYP No.1D 4-9-0, Right 2 X 8 SYP No.1D 4-9-0	

REACTIONS (lb/size)	2=1791/0-3-8, 10=1677/0-3-8
Max Horz Z=387(load case 4)	
Max Uplift Z=620(load case 5), 10=503(load case 6)	
FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/30, 2-3=-2363/857, 3-4=-2235/883, 4-5=-1896/835, 5-6=-1503/827, 6-7=-1505/631, 7-8=-1897/844, 8-9=-2244/697, 9-10=-2371/669
BOT CHORD	2-16=-609/1787, 15-16=-608/1789, 14-15=-608/1789, 13-14=-438/1541, 12-13=-404/1798, 11-12=-404/1798, 10-11=-403/1800
WEBS	4-16=0/192, 4-14=-391/327, 5-14=-147/588, 6-14=-252/308, 6-13=-248/305, 7-13=-150/596, 8-13=-405/347, 8-11=0/196
JOINT STRESS INDEX	
	2 = 0.77, 2 = 0.44, 2 = 0.44, 3 = 0.00, 4 = 0.77, 5 = 0.79, 6 = 0.47, 7 = 0.75, 8 = 0.82, 9 = 0.00, 10 = 0.72, 10 = 0.43, 10 = 0.43, 11 = 0.34; 12 = 0.71, 13 = 0.63, 14 = 0.63, 15 = 0.71 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 585.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 620 lb uplift at joint 2 and 503 lb uplift at joint 10.
LOAD CASE(S) Standard

Job L229893	Truss T11	Truss Type COMMON	Qty 6	Ply 1	JOHN DUNN
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6:300 s Apr 19 2006 Mitek Industries, Inc. Wed Mar 07 07:39:25 2007 Page 1		

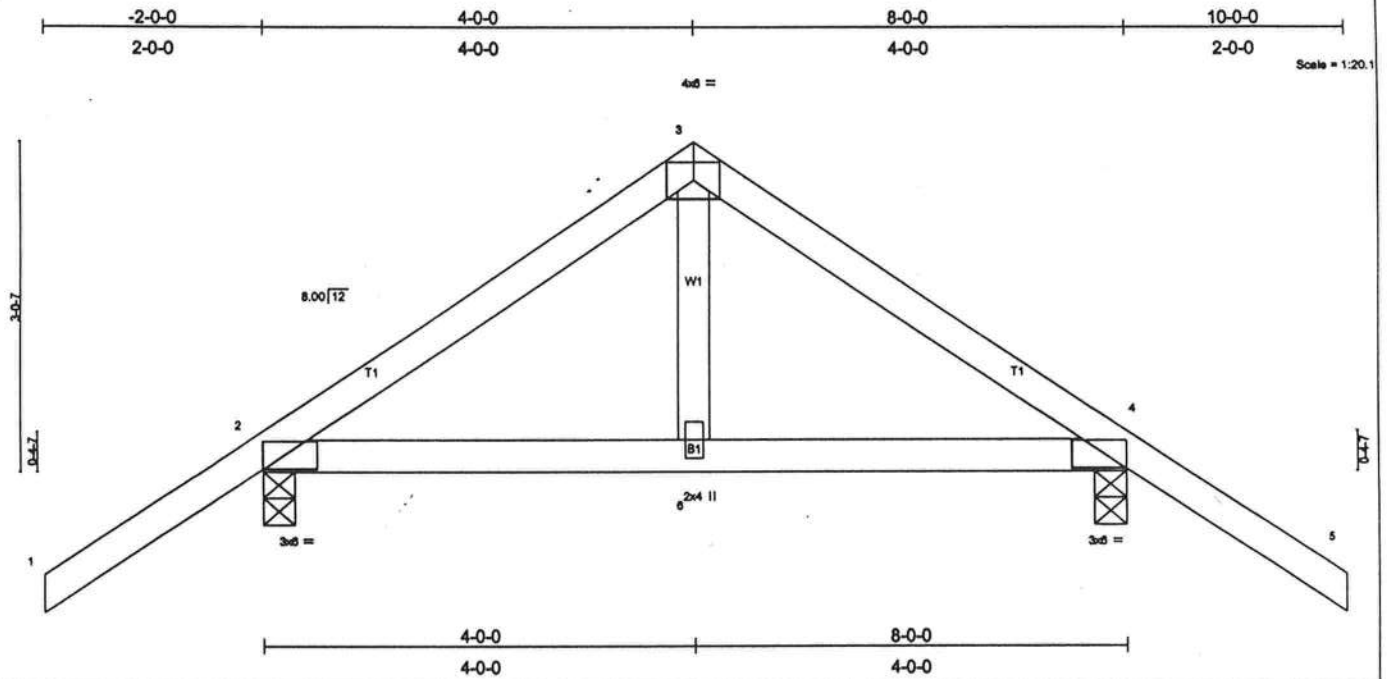


Plate Offsets (X,Y): [2-0-3-9-0-1-8], [4-0-3-9-0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.29	Vert(LL)	-0.01	2-6	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.12	Vert(TL)	-0.01	2-6	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 37 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=440/0-3-8, 4=440/0-3-8
Max Horz 2=101(load case 3)
Max Uplift 2=238(load case 5), 4=238(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/80, 2-3=326/80, 3-4=326/59, 4-5=0/80
BOT CHORD 2-6=0/215, 4-6=0/215
WEBS 3-6=0/113

JOINT STRESS INDEX

2 = 0.60, 3 = 0.17, 4 = 0.60 and 6 = 0.08

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 2 and 238 lb uplift at joint 4.

LOAD CASE(S) Standard

Job L229893	Truss T12G	Truss Type COMMON	Qty 2	Ply 1	JOHN DUNN
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 Mittek Industries, Inc. Wed Mar 07 07:39:34 2007 Page 1		

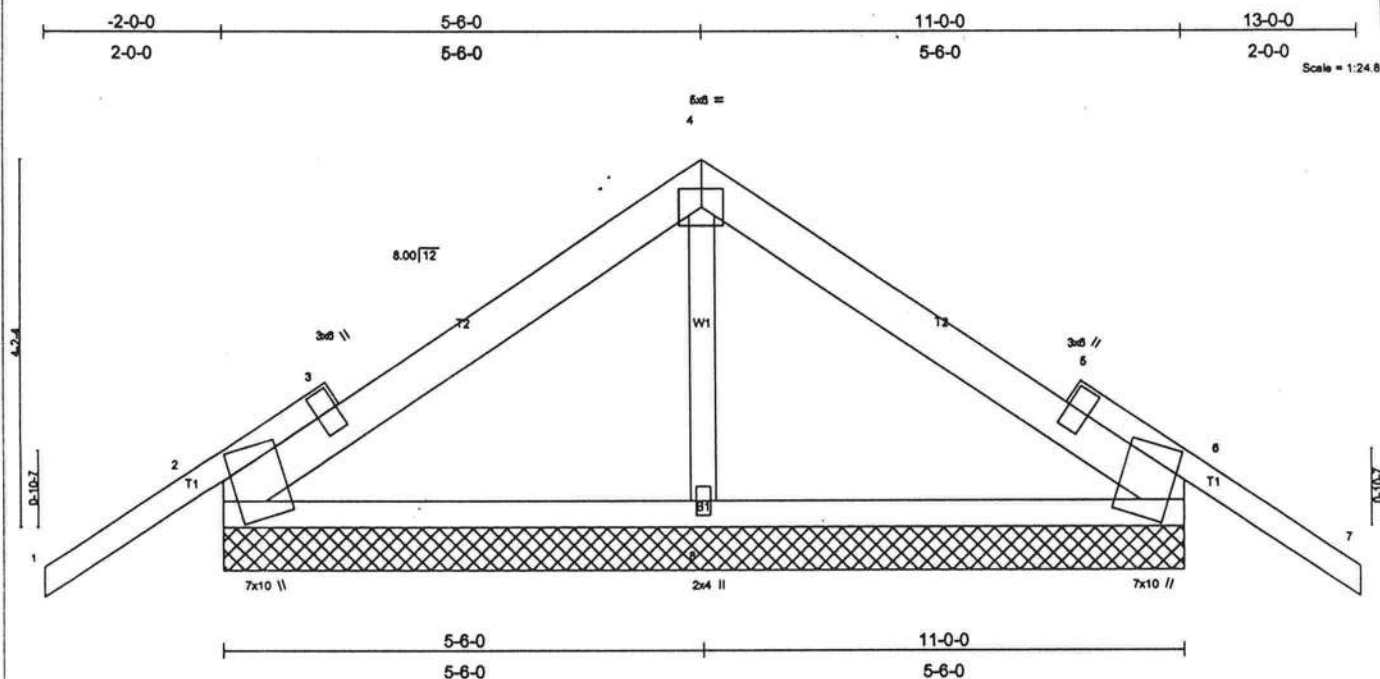


Plate Offsets (X, Y): [2'-0"-3'-13'-0"-2'-0"], [6'-0"-2'-3'-0"-3'-8"]									
LOADING (psf)		SPACING 2'-0"-0"		CSI		DEFL in (loc) l/def L/d		PLATES	GRIP
TCLL 20.0		Plates Increase 1.25		TC 0.49		Vert(LL) -0.02 7 n/r 120		MT20	244/190
TCCL 7.0		Lumber Increase 1.25		BC 0.17		Vert(TL) -0.03 7 n/r 90			
BCLL 10.0		Rep Stress Incr NO		WB 0.11		Horz(TL) 0.00 6 n/a n/a			
BCCL 5.0		Code FBC2004/TP12002		(Matrix)					
Weight: 65 lb									

LUMBER	BRACING
TOP CHORD 2 X 6 SYP No.1D "Except"	TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.
BOT CHORD 2 X 4 SYP No.2	
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=721/11'-0"-0, 6=721/11'-0"-0, 8=599/11'-0"-0
Max Horiz 2=-135(load case 3)
Max Uplift 2=-374(load case 5), 6=-395(load case 6), 8=-126(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-10/83, 2-3=-402/206, 3-4=-230/166, 4-5=-230/166, 5-6=-402/186, 6-7=-10/83
BOT CHORD 2-8=-43/191, 6-8=-43/191
WEBS 4-8=-416/152

JOINT STRESS INDEX
2 = 1.00, 3 = 0.00, 3 = 0.58, 4 = 0.63, 5 = 0.00, 5 = 0.58, 6 = 1.00 and 8 = 0.15

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Gable requires continuous bottom chord bearing.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 374 lb uplift at joint 2, 395 lb uplift at joint 6 and 126 lb uplift at joint 8.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=60), 4-7=-114(F=60), 2-8=30

Add To

Notice of Treatment

11825

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)Address: 536 SE BAYA DR.City Lake City Phone (386) 752-1703Site Location: Subdivision Hills of Rose CreekLot # 15 Block# _____ Permit # 25701Address 871 SW Hill Creek Dr. Lake City

Product used	Active Ingredient	% Concentration
☒ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment:

☒ Soil☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
<u>MAIN BODY</u>	<u>2422</u>	<u>204</u>	<u>125</u>
_____	_____	_____	_____
_____	_____	_____	_____

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

04-20-07

Date

1230

Time

R.D. Crawford

Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05

©

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ENGINEERING SCIENCES

Consultants In: Geotechnical Engineering •
Environmental Sciences • Construction Materials Testing

REPORT ON IN-PLACE DENSITY TESTS

4475 S.W. 35th Terrace • Gainesville, Florida 32608 • (352) 372-3392

Perm. t 25701

CLIENT: John Dunn

PROJECT: 871 Hill Creek Dr (Detached Garage)

AREA TESTED: Fill & prep bldg pad

COURSE: 7/G DEPTH OF TEST: 0'-1'

TYPE OF TEST: ASTM D 2922 DATE TESTED: 4-18-07

NOTE: The below tests DO/~~DO NOT~~ meet the minimum 95 % compaction requirements of maximum density.

REMARKS: Retest

[illegible]

TECH. vw

Anderson Truss

PAGE 1

Anderson Truss Co.

1730 NW Oakland Ave

Lake City, Florida 32055

(386) 752-3103 Fax: (386) 961-8290

JOB #: 7-111

Date: 04/24/2007

Delivery Date:

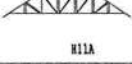
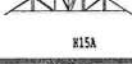
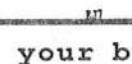
Salesman: SHAWN ANDERSON

Builder:

Job Location/Description:

David King-Dunn Garage

Terms:

QTY	Span	Profile DESCRIPTION	2 X ?		PITCH		OVERHANGS		TRUSS WGT.	
			TOP	BOT	TOP	BOTTOM	LEFT	RIGHT		
2	40'	 H7A	2x4	2x6	8.00	0.00	2'	2'	576.0	
2	40'	 H9A	2x4	2x4	8.00	0.00	2'	2'	249.6	
2	40'	 H11A	2x4	2x4	8.00	0.00	2'	2'	278.4	
2	40'	 H13A	2x4	2x4	8.00	0.00	2'	2'	270.4	
2	40'	 H15A	2x4	2x4	8.00	0.00	2'	2'	281.6	
10	40'	 H15AA	2x4	2x8	8.00	0.00	2'	2'	380.8	
2	40'	 H15A-GOR	2x4	2x8	8.00	0.00	2'	2'	1171.2	
2	11'	 B-G8	2x4	2x4	8.00	0.00	2'	2'	70.4	
2	11'	 B1	2x4	2x4	8.00	0.00	2'	2'	57.6	
1	8'	 C-G8	2x4	2x4	8.00	0.00	2'	2'	56.0	
6	8'	 C1	2x4	2x4	8.00	0.00	2'	2'	43.2	
4	9'10"13	 H27	2x4	2x4	5.66	0.00	2'9"15	0'	49.6	
28	7'	 H27	2x4	2x4	8.00	0.00	2'	0'	30.4	

Thank you for your business. Anderson Truss Company

Anderson Truss

PAGE 2

Anderson Truss Co.
1730 NW Oakland Ave
Lake City, Florida 32055
(386) 752-3103 Fax: (386) 961-8290

JOB #: 7-111
Date: 04/24/2007
Delivery Date: _____
Salesman: SHAWN ANDERSON

Builder:

Job Location/Description:
David King-Dunn Garage

Terms:

QTY	Span	Profile	2 X ?		PITCH		OVERHANGS		TRUSS	
	FTIN16	DESCRIPTION	TOP	BOT	TOP	BOTTOM	LEFT	RIGHT	WGT.	
8	5'	 J5	2x4	2x4	8.00	0.00	2'	0'	24.0	
8	3'	 J3	2x4	2x4	8.00	0.00	2'	0'	16.0	
8	1'	 J1	2x4	2x4	8.00	0.00	2'	0'	8.0	
2	10'	 H15AA-P8	2x4	2x4	8.00	0.00	0'	0'	40.0	
2	10'	 H15AA-P8	2x4	2x4	8.00	0.00	0'	0'	41.6	
8	10'	 H15AA-P8	2x4	2x4	8.00	0.00	0'	0'	43.2	

Thank you for your business. Anderson Truss Company

JOBSITE PACKAGE

IMPORTANT DOCUMENTS ENCLOSED

PLEASE REVIEW

WARNING: The handling, storing, installing and bracing of structural building components requires specialized training, clearly implemented procedures, and careful planning and communication among the contractor, all installation crews and any crane operators. Handling and installing components without appropriate training, planning and communication may result in property damage and/or serious bodily injury.

Prior to component installation, the documents should be examined and disseminated to all appropriate personnel, in addition to proper training and a clear understanding of the installation plan, any applicable fall protection requirements, and the intended bracing requirements.

Examine the structure, including the framing system, bearing locations, and related install installation locations and begin component installation only after any unsatisfactory conditions have been corrected. Do not cut, modify, or repair components and report any damage before installation.

The enclosed documents are offered as minimum guidelines only. Nothing contained in this jobsite package should be construed in any manner as expanding the scope of responsibility of, or imposing any additional liabilities on the component manufacturer.

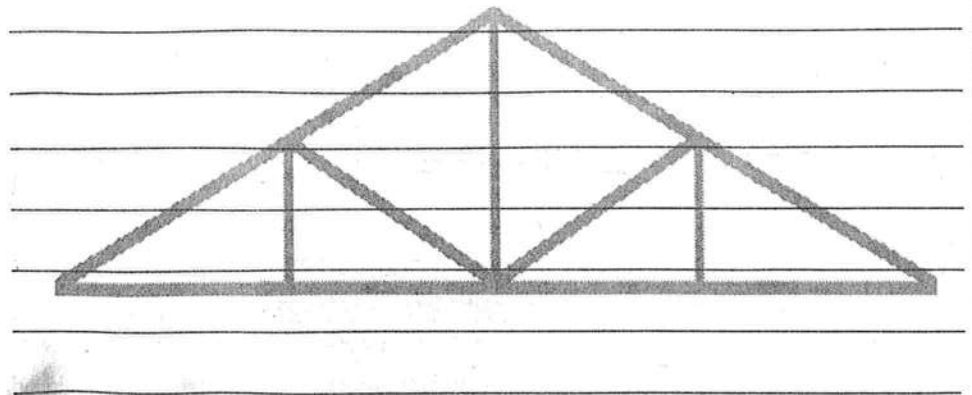
ADVERTENCIA: El manejo, almacenamiento, instalación y soporte de componentes estructurales de construcción requieren capacitación especializada, procedimientos claramente implementados y una planificación y comunicación cuidadosa entre el contratista, todo el personal de instalación y todos los operadores de grúas. Manejar e instalar componentes sin la capacitación, la planificación y la comunicación adecuadas puede ocasionar daños a la propiedad y/o graves lesiones corporales.

Antes de la instalación de componentes, los documentos adjuntos deben ser examinados y difundidos a todo el personal apropiado, además de la capacitación pertinente y un claro entendimiento del plan de instalación, de todo requisito aplicable de protección contra caídas y de los requisitos de soporte deseados.

Examine la estructura, inclusive el sistema de enrejado la ubicación de los apoyos e instalaciones correspondientes y comience la instalación de los componentes sólo después de haber corregido toda condición insatisfactoria. No corte, modifique ni repare componetes e informe cualquier daño descubierto antes de proceder a la instalación.

Los documentos adjuntos se ofrecen solamente como pautas mínimas. Nada de lo incluido en este paquete para la obra debe interpretarse de manera que exceda el alcance de la responsabilidad del fabricante de componentes, ni en forma tal que imponga responsabilidades adicionales sobre éste.

NOTES: _____



FOR TRUSS DESIGN RESPONSIBILITIES SEE REVERSE

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: IT6S487-Z0124151314

Truss Fabricator: Anderson Truss Company
Job Identification: 7-111-David King-Dunn Garage
Truss Count: 19
Model Code: Florida Building Code 2004 and 2006 Supplement
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software, Version 7.24.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61615-31.003(5a) of the FAC
Minimum Design Loads: Roof - 32.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-02 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: TCFILLER-BCFILLER-CNBRGBLK-A11015EE-GBLLETIN-A11030EE-PIGBACKA-PIGBACKB-



Seal Date: 04/24/2007

-Truss Design Engineer-
Arthur R. Fisher

Florida License Number: 59687
1950 Marley Drive
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	64989--H7A		07114002	04/24/07
2	64990--H9A		07114005	04/24/07
3	64991--H11A		07114006	04/24/07
4	64992--H13A		07114007	04/24/07
5	64993--H15A		07114014	04/24/07
6	64994--H15AA		07114001	04/24/07
7	64995--H15A-GDR		07114015	04/24/07
8	64996--B-GE		07114011	04/24/07
9	64997--B1		07114010	04/24/07
10	64998--C-GE		07114017	04/24/07
11	64999--C1		07114009	04/24/07
12	65000--HJ7		07114004	04/24/07
13	65001--EJ7		07114013	04/24/07
14	65002--J5		07114003	04/24/07
15	65003--J3		07114008	04/24/07
16	65004--J1		07114012	04/24/07
17	65005--H15AA-PB1		07114016	04/24/07
18	65006--H15AA-PB2		07114018	04/24/07
19	65007--H15AA-PB3		07114002	04/24/07



TC LL	20.0	PSF	REF R487 - - 64990
TC DL	10.0	PSF	DATE 04/24/07
BC DL	10.0	PSF	DRW HCURS487 07114005
BC LL	0.0	PSF	HC-ENG DAL/AF *
TOT.LD.	40.0	PSF	SEQN - 39321
DUR.FAC.	1.25		FROM JP
SPACING	24.0"		JREF - 1T6S487_Z01

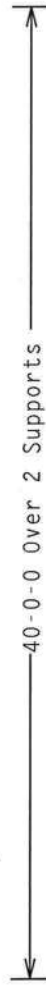
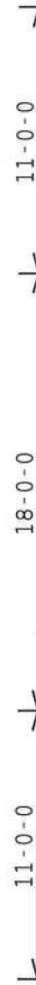
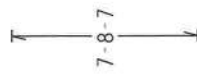
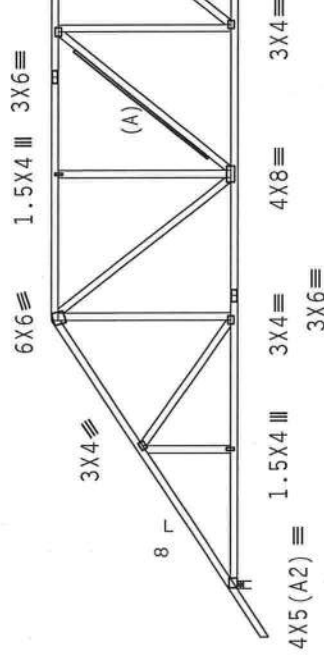
	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

110 mph wind, 17.37 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

(A) 1x4 SP #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

 $3 \times 4 =$ 

R=1818 U=196 W=3.5"

R=1818 U=196 W=3.5"

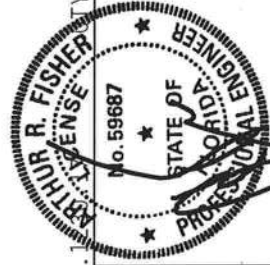
Design Crit: TPI-2002(STD)/FBC
 $\text{Ca/RT}=1.00(1.25)$
$$Ca/RT=1.00(1.25)/10(0)$$

4.1.

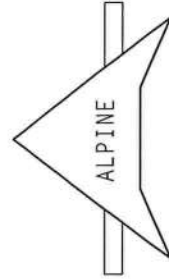
ARTIST'S CONSENT: 2 FL/1-141-1-1R/1-

Scale = .125"/Ft.

TC LL	20.0	PSF	REF	R487--	64991
TC DL	10.0	PSF	DATE	04/24/07	
BC DL	10.0	PSF	DRW	HCUSR487	07114006
BC LL	0.0	PSF	HC-ENG	DAL/AF	*
TOT.LD.	40.0	PSF	SEQN-	39326	
DUR.FAC.	1.25		FROM	JP	
SPACING	24.0"		JREF-	1T6S487_Z01	



Apr 2, '07



ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

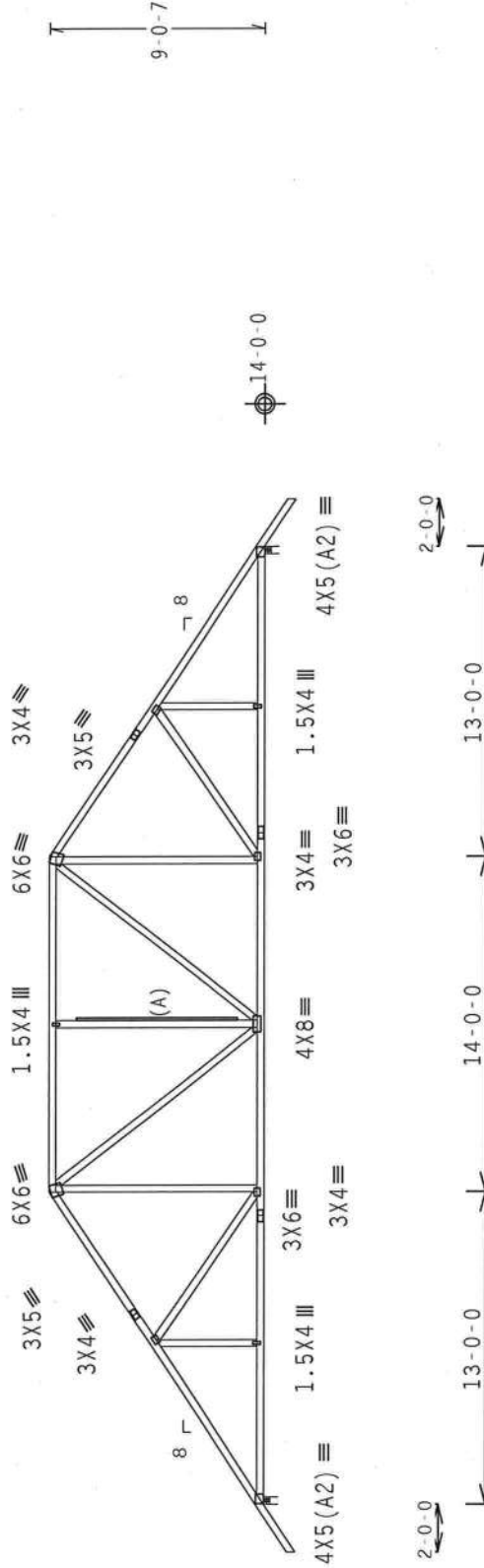
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 18.04 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $lw=1.00$ $G_{CPI}(+/-)=-0.18$

(A) 1x4 SP #3 or better "I" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.



R-1818 U=199 W=3.5"

40-0-0 Over 2 Supports

R-1818 U=199 W=3.5"

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

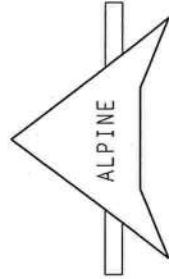
7.24.1

FL/-/4/-/R/-

Scale = .125"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/P&J) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2010/160A (44/55/55) ASTM A553 GRADE 40/60 (4, 47/55) GALV. STEEL. APPLY TO ALL TRUSSES. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANCH A3 OF TPI-2002 SEC. 3.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group, Inc.
Haines City, FL 33844
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Apr 4 '07

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487--	64992
DATE	04/24/07	
DRW	HCUSR487	07114007
HC-ENG	DAL/AF	*
SEQN	39331	
FROM	JP	
JREF	1T6S487_Z01	

[illegible]

Bot chord ZX8 SP SS :BZ ZX10 SP SS:
:B3 2x4 SP #2 Dense:

Webs 2x4 SP #3

Filler 2x4 SP #2 Dense

SPECIAL LOADS

----- (LUMBER DUR.FAC. = 125 / PLATE DUR.FAC. = 1.25)		
TC - From	64 PLF at -2.00 to	64 PLF at 7.98
TC - From	64 PLF at 7.98 to	64 PLF at 12.00
TC - From	88 PLF at 12.00 to	88 PLF at 13.43
TC - From	64 PLF at 13.43 to	64 PLF at 15.00
TC - From	64 PLF at 15.00 to	64 PLF at 25.00
TC - From	64 PLF at 25.00 to	64 PLF at 26.57
TC - From	88 PLF at 26.57 to	88 PLF at 28.29
TC - From	64 PLF at 28.29 to	64 PLF at 32.18
TC - From	64 PLF at 32.18 to	64 PLF at 42.00
PLT - From	20 PLF at 14.59 to	20 PLF at 25.41
BC - From	5 PLF at -2.00 to	5 PLF at 0.00
BC - From	20 PLF at 0.00 to	20 PLF at 12.00
BC - From	120 PLF at 12.00 to	120 PLF at 28.29
BC - From	20 PLF at 28.29 to	20 PLF at 42.00
BC - From	5 PLF at 40.00 to	5 PLF at 44.00
BC -	141 LB Conc. Load at 12.00, 28.00	

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 18.70 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCp1(+/-)=0.18

Wind reactions based on MWFRS pressures.

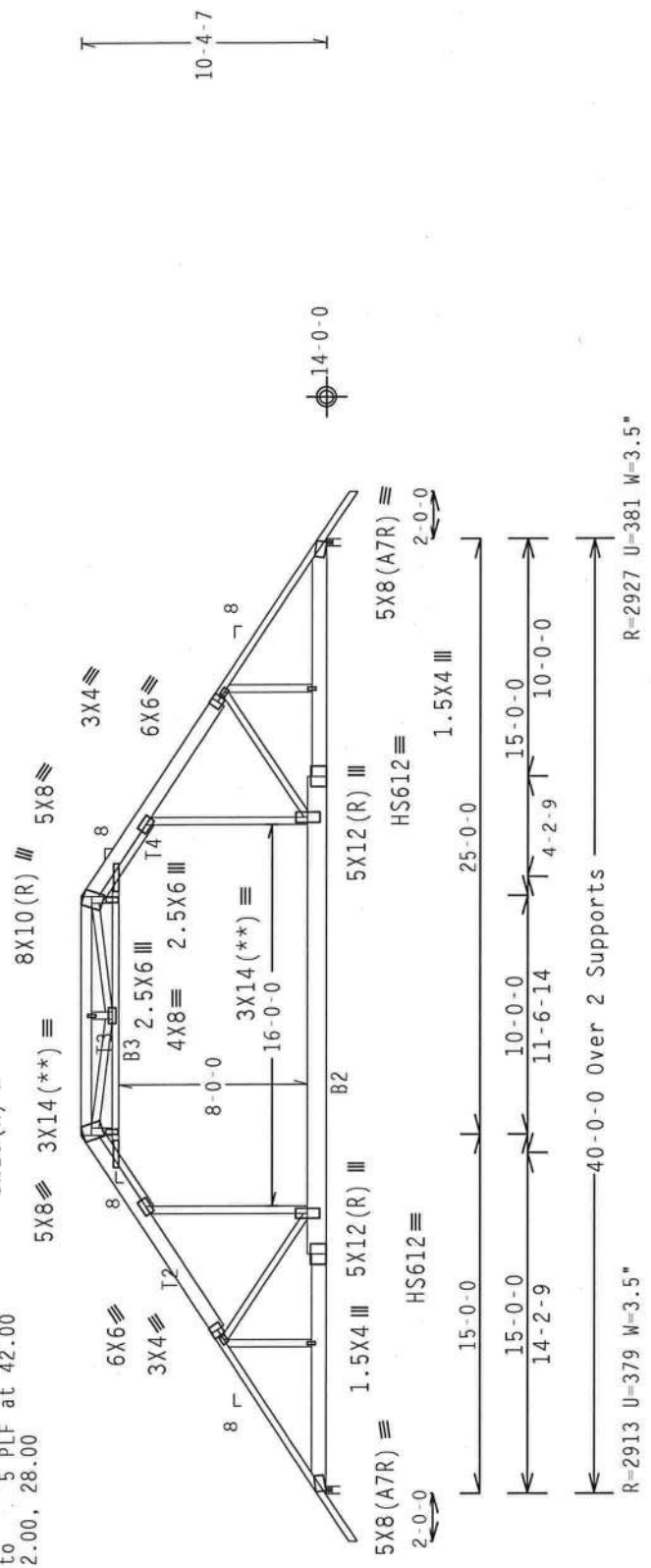
Calculated horizontal deflection is 0.12" due to live load and 0.20" due to dead load.

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

Laterally brace TC below filler at 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=2913 U=379 W=3.5"

R=2927 U=381 W=3.5"

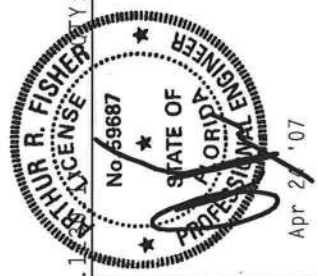
Design Crit: TPI-2002(STD)/FBC

$$Cq/RT = 1.00(1.25) / 10(0)$$

Scale = .125"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WOOD TRUSS COUNCIL OF AMERICA, ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN COMPLIANCE WITH THIS DESIGN CONFORMS WITH ALL APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. 5.3.5. ITM BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA. (44-H/55YR) ALUMINUM ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, 40/60 (4.8 K/IN) GALV. STEEL, APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX 43 OF TP11-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2.



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Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

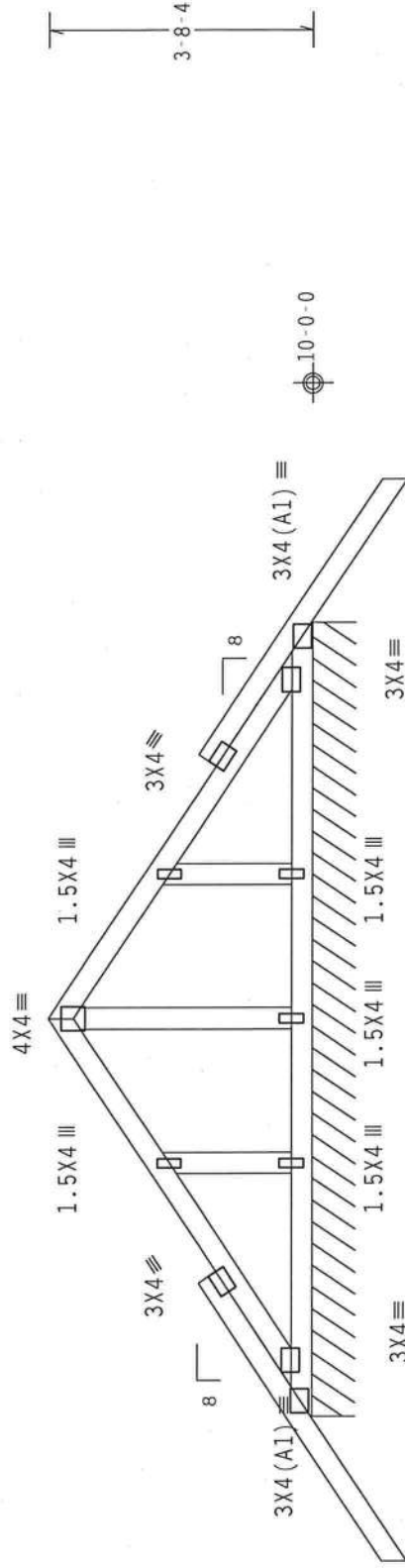
Wind reactions based on MMFRS pressures.

Truss spaced at 24.0" OC designed to support 2-0-0 Top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord
must not be cut or notched.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC
DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

See DWGS A11015EE0207 & GBLLEIINO207 for more requirements.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



R=204 PLF U=76 PLF W=11-0-0

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

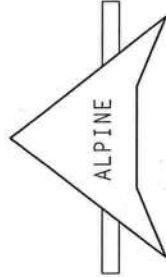
7.24.1

TY:2 FL/-/4/-/-/R/-

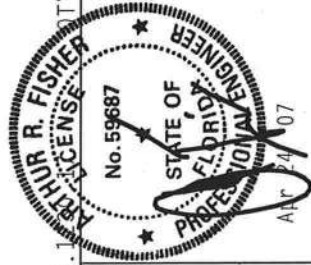
Scale = .375"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48371) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG PLATES TO PLATE FACE OR END OF TRUSS. 2018/1600 (4-11/15/18) ASH A055 GRADE 40/60 (4-11/15/18) GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.2. A SEAL ON THIS DRAWING INDICATES THE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group, Inc.
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TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

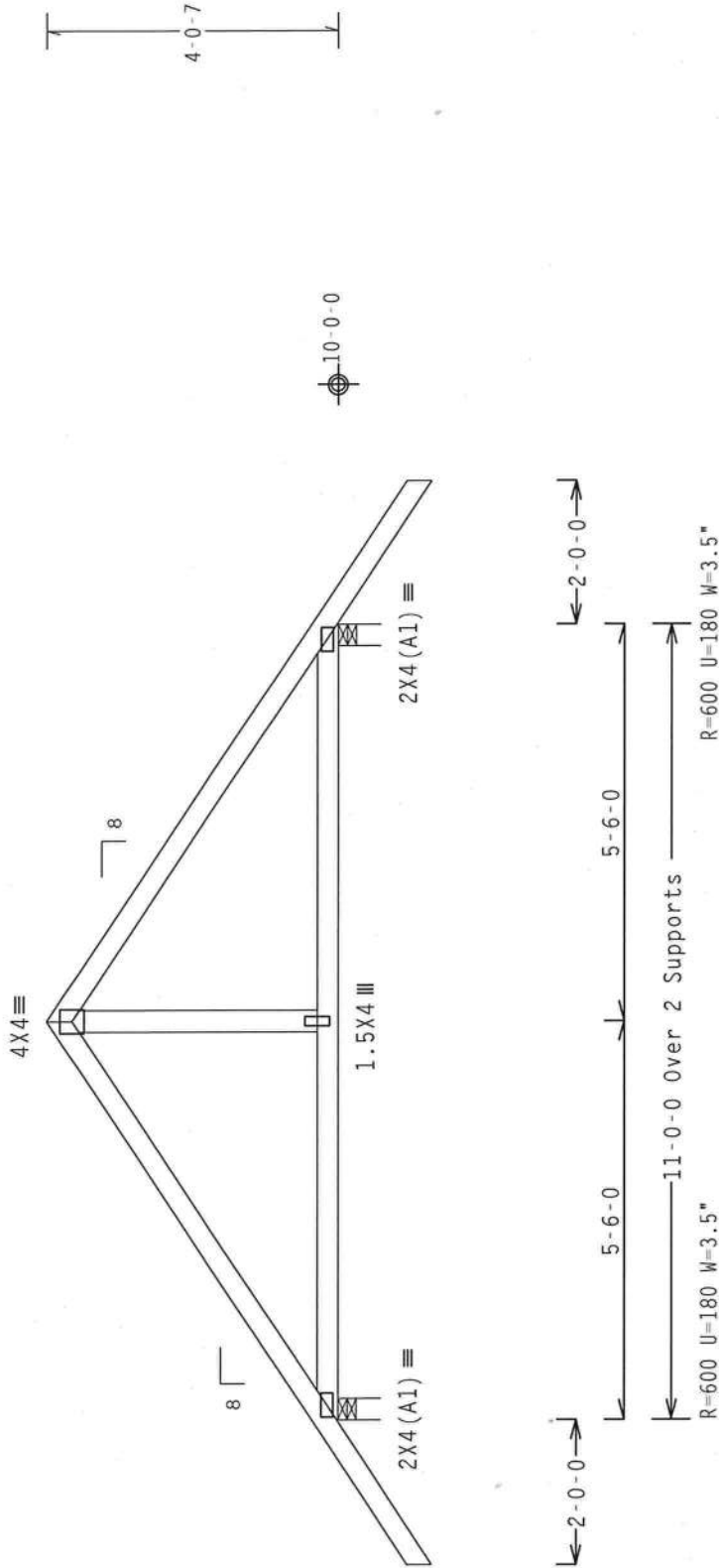
REF	R487--	64996
DATE	04/24/07	
DRW	HCUSR487	07114011
HC-ENG	DAL/AF	
SEQN	39264	REV
FROM	JP	
JREF	1T6S487_Z01	

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

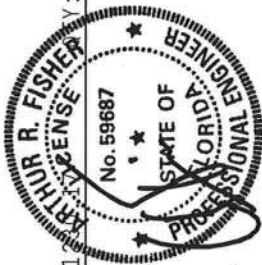
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

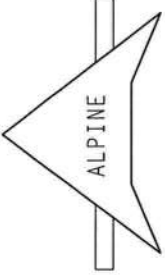


Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.12

PLT TYP. Wave	FL/-/4/-/1/R/-	Scale = .375"/Ft.
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48071) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.	TC LL 20.0 PSF	REF R487-- 64997
IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PJ) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (W-H/SS/K) ASTM A653 GRADE 40/60 (W, K/M,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16GA-2. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS ARE IN FEET AND INCHES. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS ARE IN FEET AND INCHES. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.	TC DL 10.0 PSF	DATE 04/24/07
	BC DL 10.0 PSF	DRW HCUSR487 07114010
	BC LL 0.0 PSF	HC-ENG DAL/AF *
	TOT.LD. 40.0 PSF	SEQN- 39205
	DUR.FAC. 1.25	FROM JP
	SPACING 24.0"	JREF- 1T6S487_Z01



Apr 24 '07

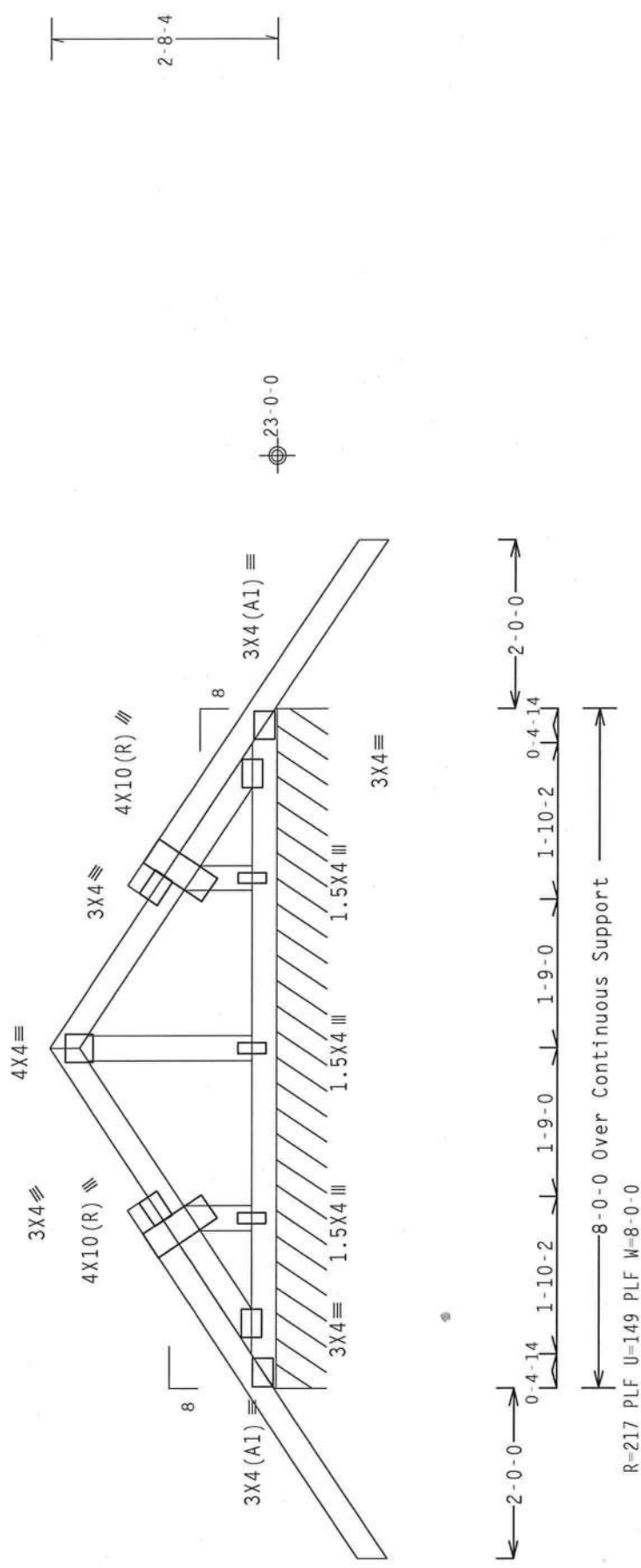


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110 mph wind, 23.86 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

See DWGS A11030EE0207 & GBLLETIN0207 for more requirements.

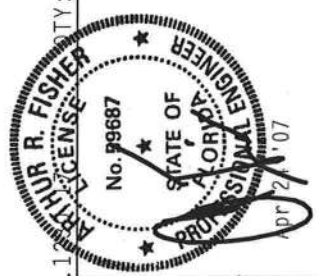
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/

Scale = .5"/Ft.

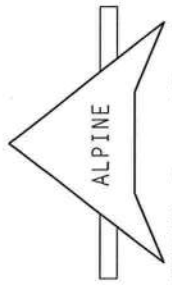
TC LL	20.0 PSF	REF	R487 - -	64998
TC DL	10.0 PSF	DATE	04/24/07	
BC DL	10.0 PSF	DRW	HCUSR487	07114017
BC LL	0.0 PSF	HC-ENG	DAL/AF	
TOT.LD.	40.0 PSF	SEQN -	39258	REV
DUR.FAC.	1.25	FROM	JP	
SPACING	24.0"	JREF -	1T6S487_Z01	



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****IMPORTANT**** TURKISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING, OR BRACING OF TRUSSES.

THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (44/18/55K) ASTM A663 GRADE 40/60 (A, K/H-55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMER A3/TPI 1



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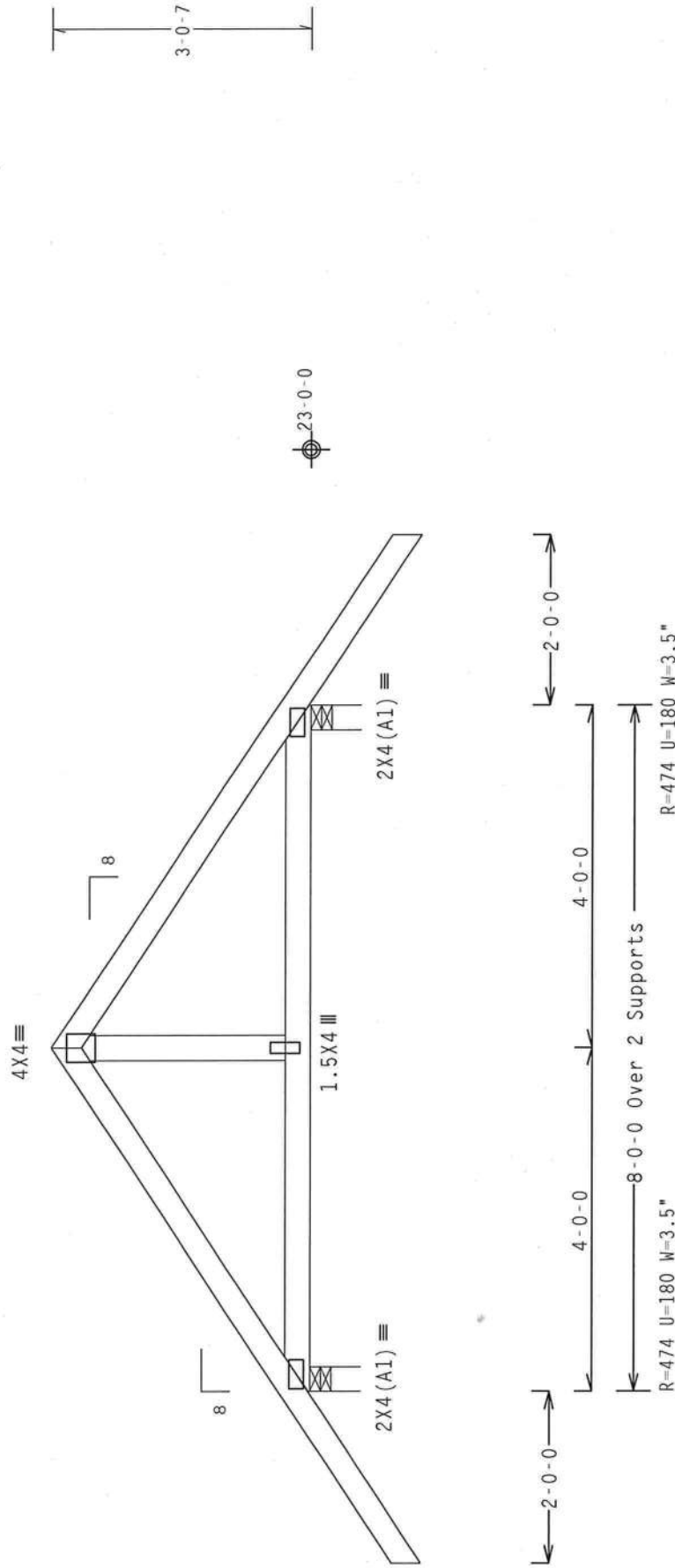
(7-111-David King-Dunn Garage - C1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MMFRS pressures.

110 mph wind, 24.04 ft mean hgt, ASCE 7-02, CLOSED bldg, Located
anywhere in roof, CAT II, Exp B, wind TC DL=5.0 psf, wind BC
DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.24.1

FL/-/4/-/-/R/-

Scale = .5"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487--	64999
DATE	04/24/07	
DRW	HCUSR487	07114009
HC-ENG	DAL/AF	*
SEQN-	39201	
FROM	JP	
JREF-	1T6S487_Z01	



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****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGNED CONFORMS WITH APPLICABLE PROVISIONS OF ROS (NATIONAL DESIGN SPEC. BY AIA/ASA) AND TPI. ITW BCG PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1601Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



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Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

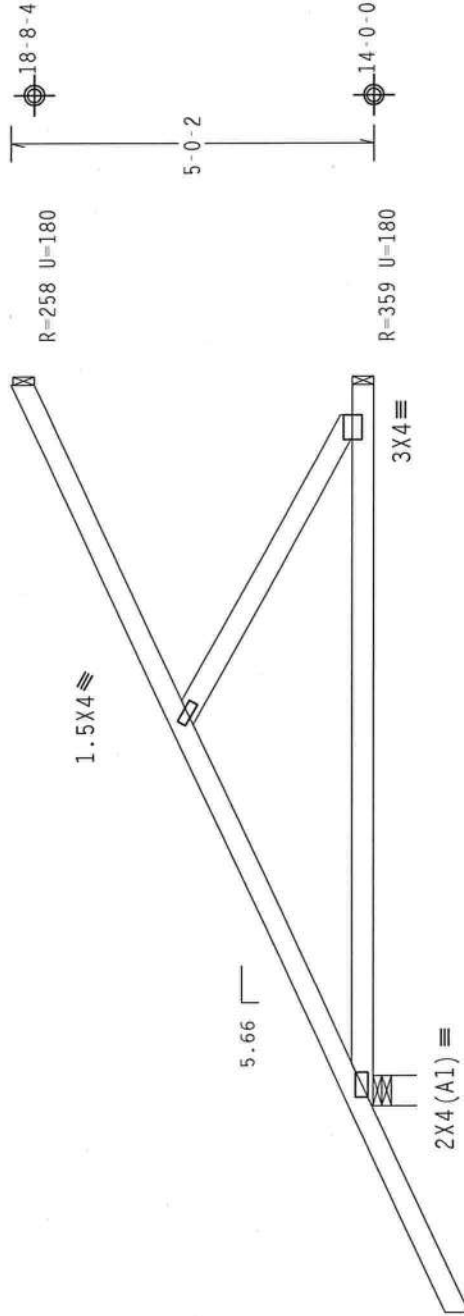
Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 16.01 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Hipjack supports 7-0-0 setback jacks with no webs.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



2-9-15

9-10-13 Over 3 Supports
R=553 U=180 W=4.95"

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24

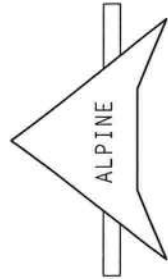
PLT TYP. Wave

Scale = .375"/Ft.

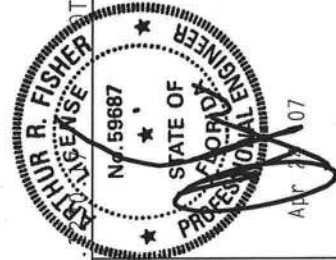
TY:4 FL/-/4/-/-/R/-

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487--	65000
DATE	04/24/07	
DRW	HCUSR487	07114004
HC-ENG	DAL/AF	
SEQN-	39457	
FROM	JP	
JREF-	1T6S487_Z01	



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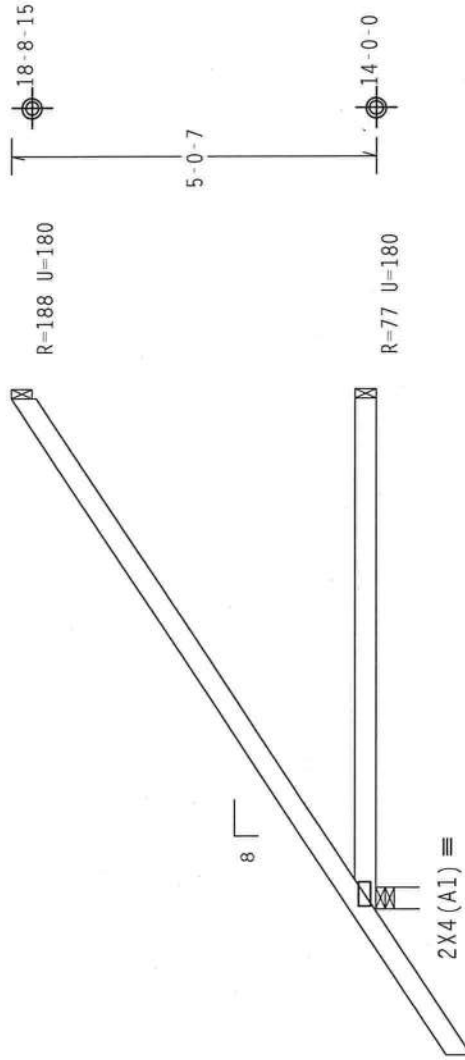
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Wind reactions based on MWFRS pressures.

Provide { 2 } 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide { 2 } 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 16.04 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpl(+/-)=0.18

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



← 2'-0" →

← 7'-0" Over 3 Supports →

R=461 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

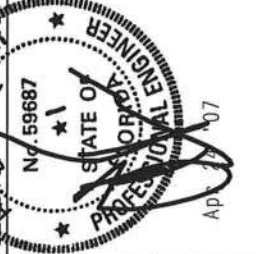
7.24.1

FL/-/4/-/R/-

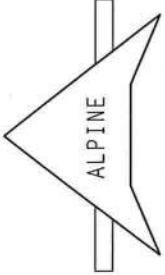
Scale = .375"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (M-J/SS/KS) ASTM A653 GRADE 40/60 (M, K/M,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R487--	65001
TC DL	10.0 PSF	DATE	04/24/07
BC DL	10.0 PSF	DRW HCUSR487	07114013
BC LL	0.0 PSF	HC-ENG DAL/AF	*
TOT.LD.	40.0 PSF	SEON-	39292
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1T6S487_Z01



ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

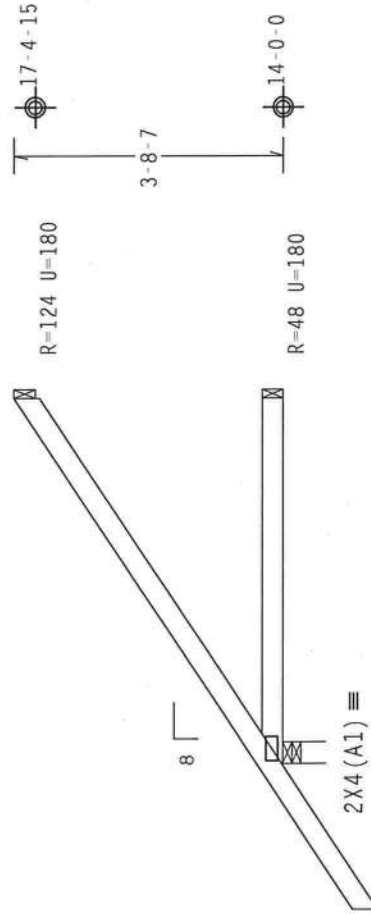
Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense

110 mph wind, 15.37 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



→ 2-0-0 →

←5-0-0 Over 3 Supports →
R=387 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/

PLT TYP. Wave

19TH DISTRICT SHEAR & MCMURDO COUNTY: 8 FL/-14/-1/-1R/-

Scale = .375"/Ft.

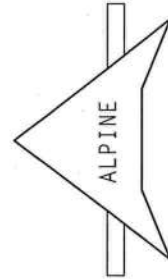
TC LL	20.0 PSF	REF R487 - - 65002
TC DL	10.0 PSF	DATE 04/24/07
BC DL	10.0 PSF	DRW HCURS487 07114003
BC LL	0.0 PSF	HC-ENG DAL/AF *
TOT.LD.	40.0 PSF	SEQN- 39296
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1T6S487_Z01



Apr 24 '07

WARNING: THESE REQUIRE EXTREME CARE IN ERECTION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION PUBLISHED BY THE MANUFACTURER. 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND AARON MOORE TRUSS COMPANY OF AMERICA, ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THE SE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID BRACING.

*****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. THE DESIGN SPEC. BY ACP&P AND TP1. THE BCG CONNECTOR PLATES ARE MADE TO CONFORM TO THE NATIONAL DESIGN SPEC. BY ACP&P AND TP1. THE BCG PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2 THROUGH 160A-7. THE TRUSS SHALL BE MADE OF 100% CARBON STEEL. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2.

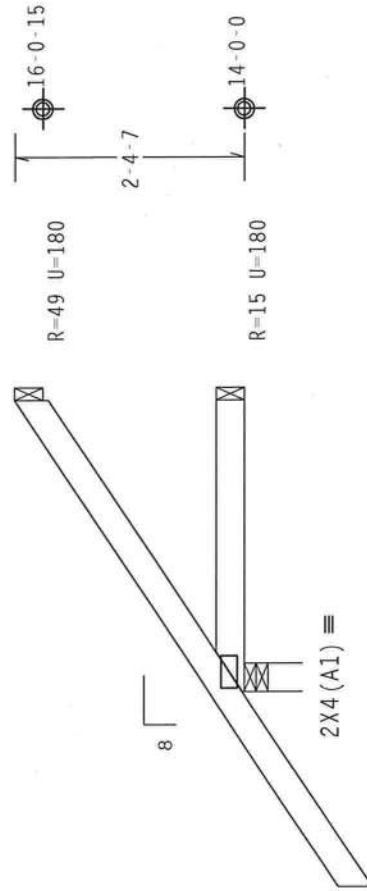


ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpf (+/-)=0.18

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

factor for dead load is 1.50.



3-0-0 Over 3 Supports

R=325 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC

 $Cq/RT=1.00(1.25)/10(0) \quad 7.$

FL/-/4/-/-/R/-

Scale = .5"/Ft.

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION PUBLISHED BY TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304, AND WEA (WOOD TRUSS COUNCIL OF AMERICA), 1000 ERIE STREET, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID BRACING.

****IMPORTANT**** TURKISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE BUILDING OR THE TRUSS SYSTEMS DURING THE CONSTRUCTION OF THE TRUSSES. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) 13TH EDITION. CONNECTOR PLATES ARE MADE OF 70/18/16GA (4/8X55/8) ASH ON THIS DESIGN. POSITION PER DRAWINGS 160A-2. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABEX 43 OF TP1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER A513/TP1 1 SEC. 2.



Apr 24 '07

TC LL	20.0 PSF	REF R487 - - 65003
TC DL	10.0 PSF	DATE 04/24/07
BC DL	10.0 PSF	DRW HCURS487 07114008
BC LL	0.0 PSF	HC-ENG DAL/AF *
TOT.LD.	40.0 PSF	SEQN- 39461
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1T6S487_Z01



ITW Building Components Group, Inc.
Haines City, FL 33844

FL Certificate of Authorization # 567

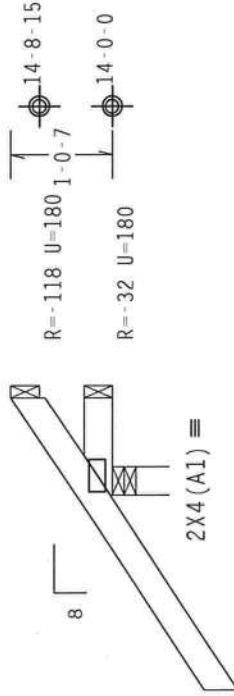
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Wind reactions based on MWFRS pressures.

Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



2-0-0
1-0-0 Over 3 Supports
R=372 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC
 $C_q/R_T=1.00(1.25)/10(0)$ 7.24.12

PLT TYP. Wave

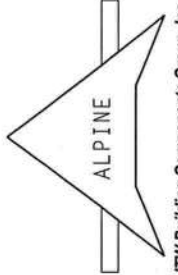
Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. 1TH BCG CONNECTION PLATES ARE MADE OF 2018/1816GA (M-H/SS/K3) ASTM A653 GRADE 40/60 (M, K/P,SS) GALV. STEEL. APPLY ANY RESPECTIVE PLATES FOLLOWED BY SHOWN DIMENSIONS. DIMENSIONS ARE IN INCHES UNLESS OTHERWISE NOTED. 2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487--	65004
DATE	04/24/07	
DRW	HCUSR487	07114012
HC-ENG	DAL/AF	
SEON-	39305	
FROM	JP	
JREF-	1T6S487_Z01	



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Haines City, FL 33844
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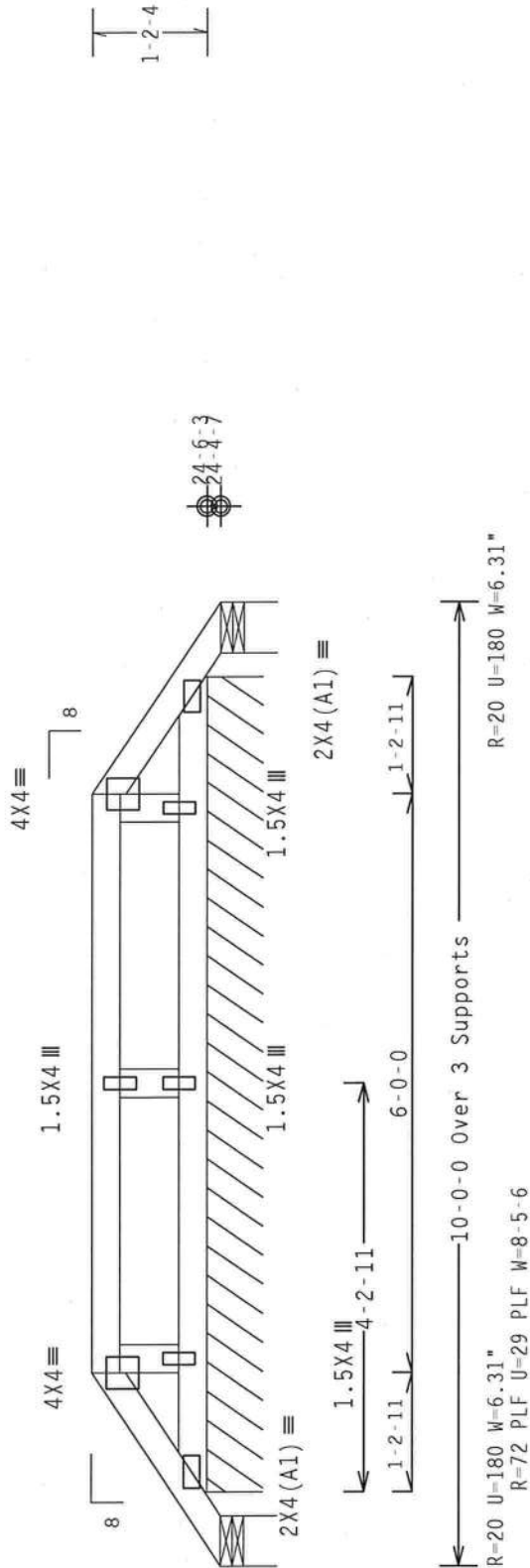
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 25.04 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge. CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. $I_w=1.00$ GCpi(+/-)=0.18

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



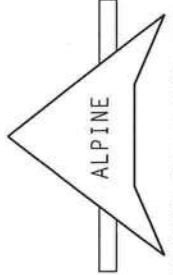
Design Crit: TPI-2002(STD)/FBC

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NFCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE TRUSS IS TO BE BUILT IN CONFORMANCE WITH THE DESIGN. ALL TRUSS MATERIALS SHALL BE PROVIDED BY THE INSTALLER. THE TRUSS IS TO BE BUILT IN CONFORMANCE WITH THE DESIGN. ALL TRUSS MATERIALS SHALL BE PROVIDED BY THE INSTALLER. THE TRUSS IS TO BE BUILT IN CONFORMANCE WITH THE DESIGN. ALL TRUSS MATERIALS SHALL BE PROVIDED BY THE INSTALLER.



PLT TYP. Wave	Scale = .5" / Ft.
TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	2.0 PSF
BC LL	0.0 PSF
TOT. LD.	32.0 PSF
DUR. FAC.	1.25
SPACING	24.0"
REF	R487-- 65005
DATE	04/24/07
DRW	HCUSR487 07114016
HC-ENG	DAL/AF
SEQN-	39283
FROM	JP
JREF-	1T6S487_Z01



ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

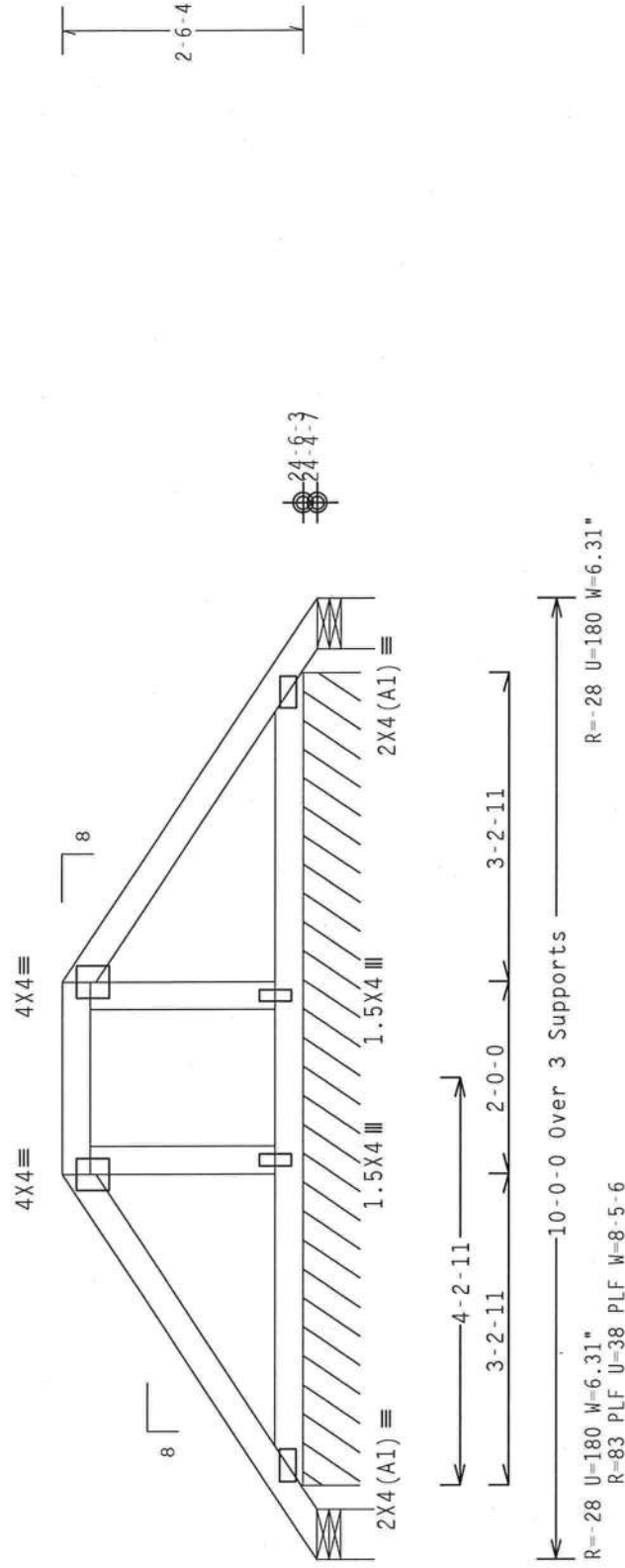
	Top	Bot	Dense
chord	2x4	2x4	SP #2
chord	2x4	2x4	SP #2
webs	2x4	2x4	SP #3

110 mph wind, 25.70 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf, $I_w=1.00$ GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" ON CENTER UNLESS OTHERWISE SPECIFIED.

Design Crit: TPI-2002(STD)/FBC
Ca/RT=1.00(1.25)

PLT TYP. Wave

12

QTY: 2 FL/-/4/-/-/R/-/

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. TRUSSES MUST BE PROPERLY SUPPORTED AND BRACED TO PREVENT BUCKLING. (SEE BRACING PLATE TYPOT, 2100 NORTH LEE STREET, SUITE 302, WESTLAND, MI 48094) WHEN TRUSS CONNECTIONS ARE MADE TO STEEL MEMBERS, THE TRUSS CONNECTIONS MUST BE MADE TO THE WEBS OF THE STEEL MEMBERS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID BEAMING.

TC II	20.0 PSF
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REF R487-- 65006

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
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KZ1	K407	030000
DATE	04/24/07	

70.0 10.0 PSF

DATE 04/24/01

BC DL 2.0 PSF

DRW HCUSR487 07114018

BC LL 0.0 PSF

HC-ENG DAL/AF

TOT ID 32.0 PSE

SEON- 39276

[illegible]

FROM	TO
CELESTINE	05270

DUR. FAC. 1.25

FROM JP

James City, FL 32844
FL Certificate of Authorization # 567

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

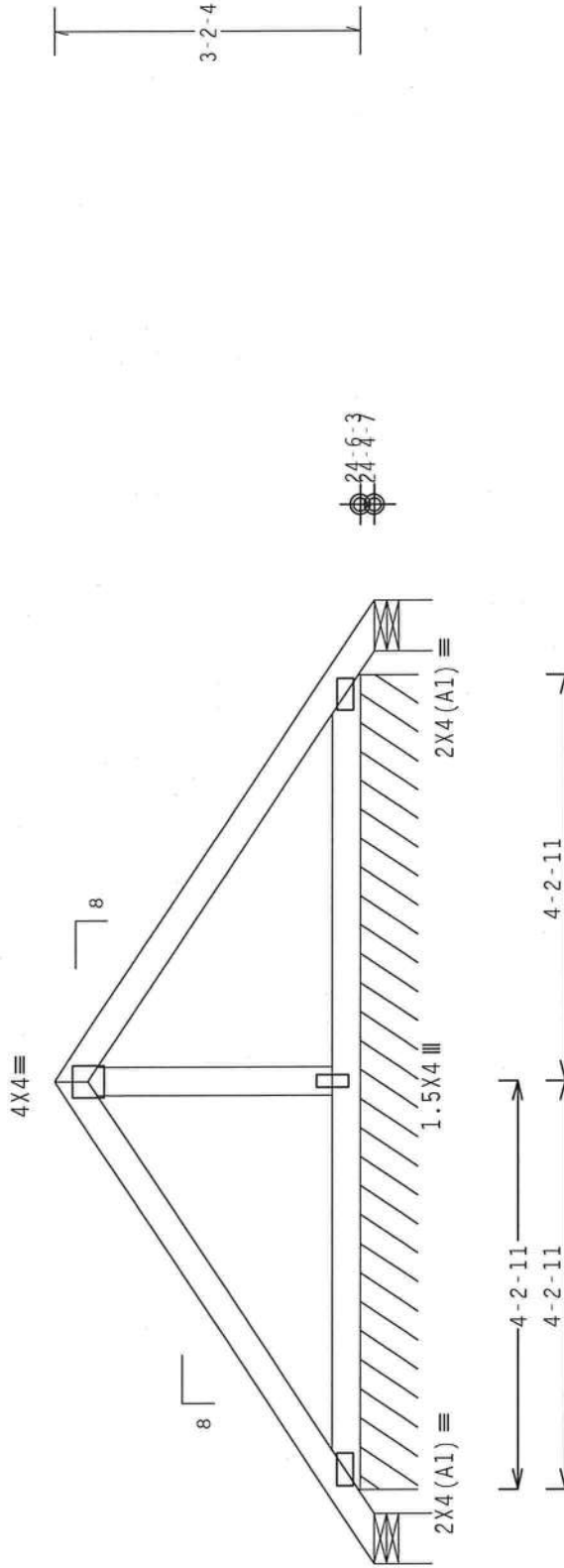
Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC,
UNLESS OTHERWISE SPECIFIED.

110 mph wind, 26.04 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. $lw=1.00 GCpi(+/-)=0.18$

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.



R=-62 U=180 W=6.31"
R=91 PLF U=44 PLF W=8-5-6
10-0-0 Over 3 Supports
R=-62 U=180 W=6.31"

Design Crit: TPI-2002(STD)/FBC

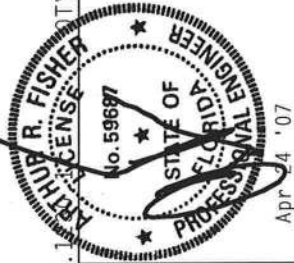
Cq/RT=1.00(1.25)/10(0) 7.24.1

PLT TYP. Wave

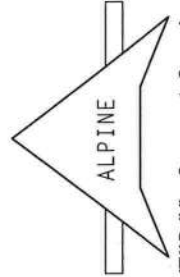
Scale = .5" / Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI1: STEEL TRUSS DESIGN. ITW BCG SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. ANY INSPECTION OF TRUSSES SHALL BE PERFORMED BY A QUALIFIED INSPECTOR. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R487--	65007
TC DL	10.0 PSF	DATE	04/24/07	
BC DL	2.0 PSF	DRW	HCUSR487	07114002
BC LL	0.0 PSF	HC-ENG	DAL/AF	
TOT.LD.	32.0 PSF	SEQN-	39272	
DUR.FAC.	1.25	FROM	JP	
SPACING	24.0"	JREF-	1T6S487_Z01	



ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

TOP CHORD FILLER DETAIL

- + 2X4 CONTINUOUS LATERAL BRACING AT 24" O.C. MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH (2) 16d COMMON (0.162"x 3.5".MIN) NAILS.
- BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.
- ++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.
- +++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED 48" OC MAXIMUM.
- * 8/12 MAXIMUM PITCH.
- ** 2X8.25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGBACKB0699 FOR PIGGYBACK SPECIAL PLATE INFORMATION.

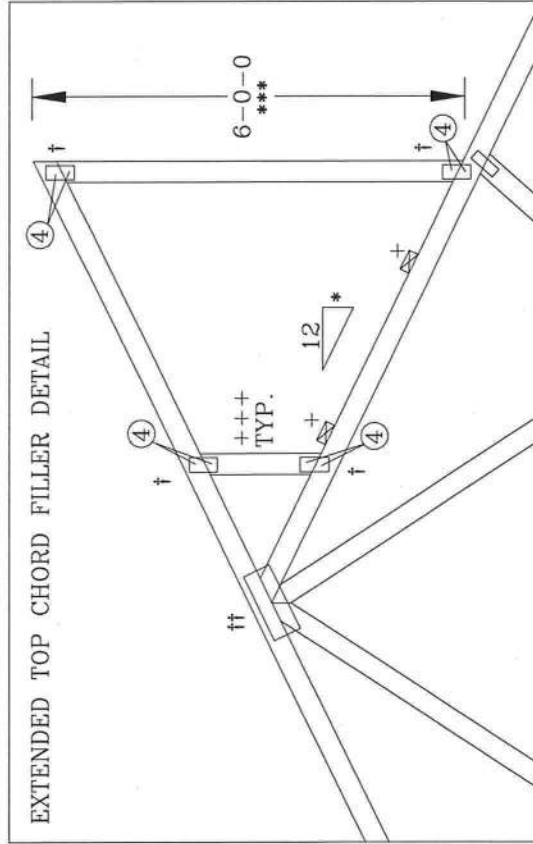
*** 6'0" MAXIMUM HEIGHT.

† W2X4 OR 3X6 TRULOX.

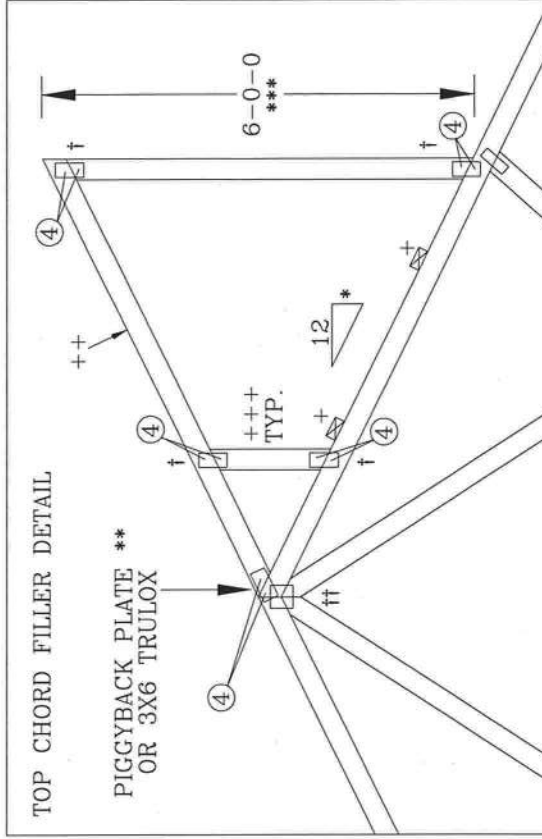
†† REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

0.120"x 1.375" NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF EACH TRUSS PLY. SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS.

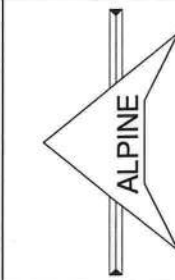
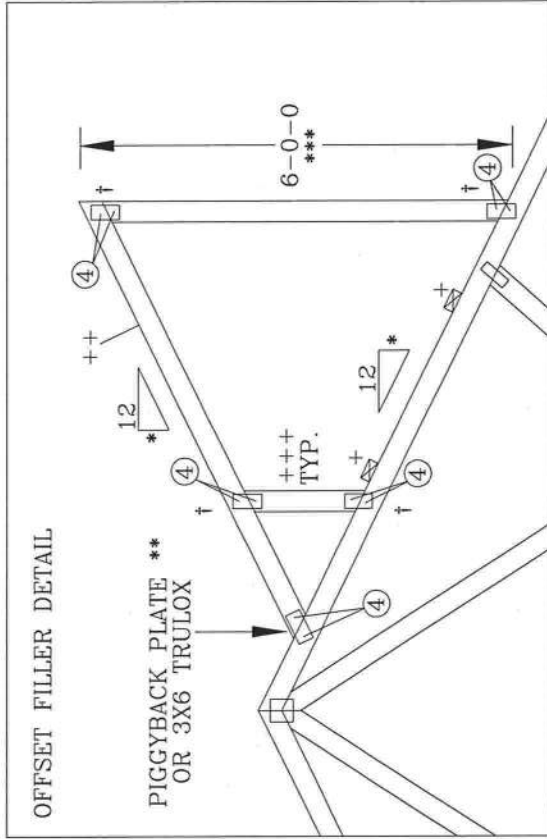
EXTENDED TOP CHORD FILLER DETAIL



TOP CHORD FILLER DETAIL



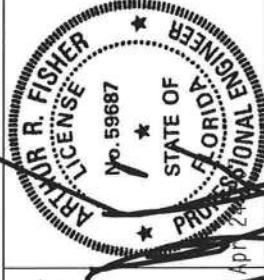
OFFSET FILLER DETAIL



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 208 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22304, AND TICA (WOOD TRUSS COUNCIL OF AMERICA) 3500 WILSON AVENUE, WILSON, NC 27157, FOR THE LATEST RECOMMENDED DESIGN SPECIFICATIONS AND FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN ACCORDANCE WITH THE DESIGN SPECIFICATIONS AND RECOMMENDATIONS SHALL BE THE RESPONSIBILITY OF THE DESIGN CONTRACTOR. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA 60K/55K/50 ASTM A573 GRADE 40/60 60K/55K GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL IN THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



THIS DRAWING REPLACES DRAWING 884.080

TC LL	MAX 30	PSF	REF	TC-FILLER
TC DL	MAX 15	PSF	DATE	2/23/07
BC DL	MAX 10	PSF	DRWG	TCFILLER0207
BC LL	0	PSF	-ENG	SIP/KAR
TOT. LD.	MAX 55	PSF		
DUR. FAC.	1.15	OR 1.33		
SPACING	24.0"			

BOTTOM CHORD FILLER DETAIL

* OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION.

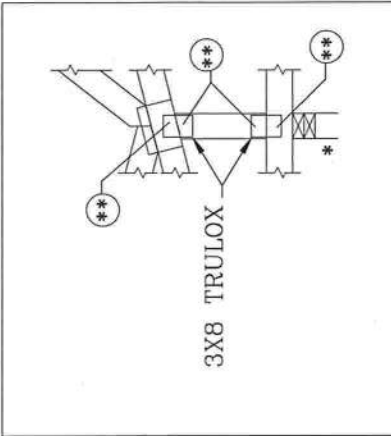
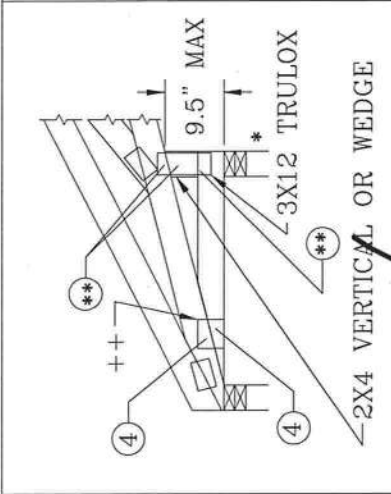
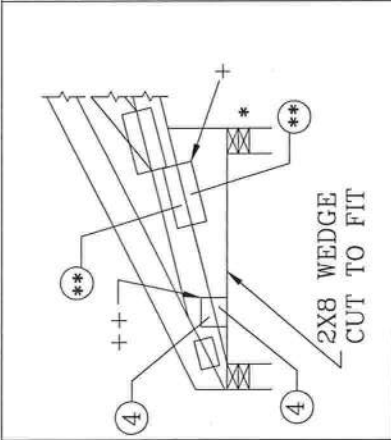
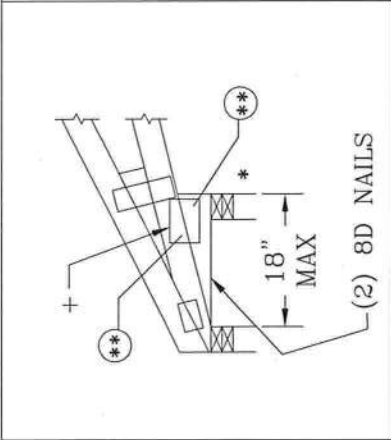
+ 3X4 WAVE OR 4X8 TRULOX
++ 2X4 WAVE OR 3X6 TRULOX

0.120" X 1.375", NAILS, REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF THE TRUSS. SEE DWG. 160T1 FOR NAILING AND TRULOX PLATE REQUIREMENTS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

ALL TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES MAY BE REQUIRED TO ACCOMMODATE REQUIRED NAILS (**)

FILLER BOTTOM CHORD OR WEDGE SPECIES	MAXIMUM REACTION		MINIMUM BEARING AREA	** REQUIRED NAILS PER FACE WITH TRULOX PLATES					
	DOWNWARD	UPLIFT		1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.	
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	12	11	10	9	8	
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	6	
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6	
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8	
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7	
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"	9	8	7	7	6	

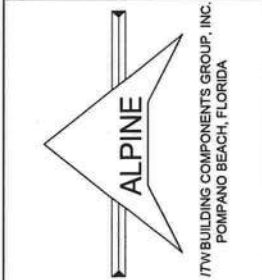
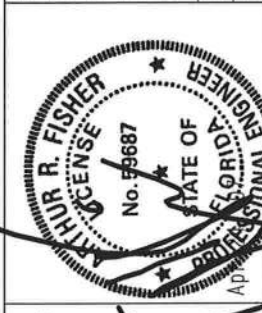


THIS DRAWING REPLACES DRAWINGS A115 A115/R & 884.132

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TC LL	—	PSF	REF	BC FILLER
TC DL	—	PSF	DATE	2/23/07
BC DL	10.0	PSF	DRWG	BCFILLER0207
BC LL	—	PSF	—	ENG DLJ/KAR
TOT. LD.	—	PSF		
DUR. FAC.	1.0/1.15/1.25/1.33			
SPACING	24.0"			



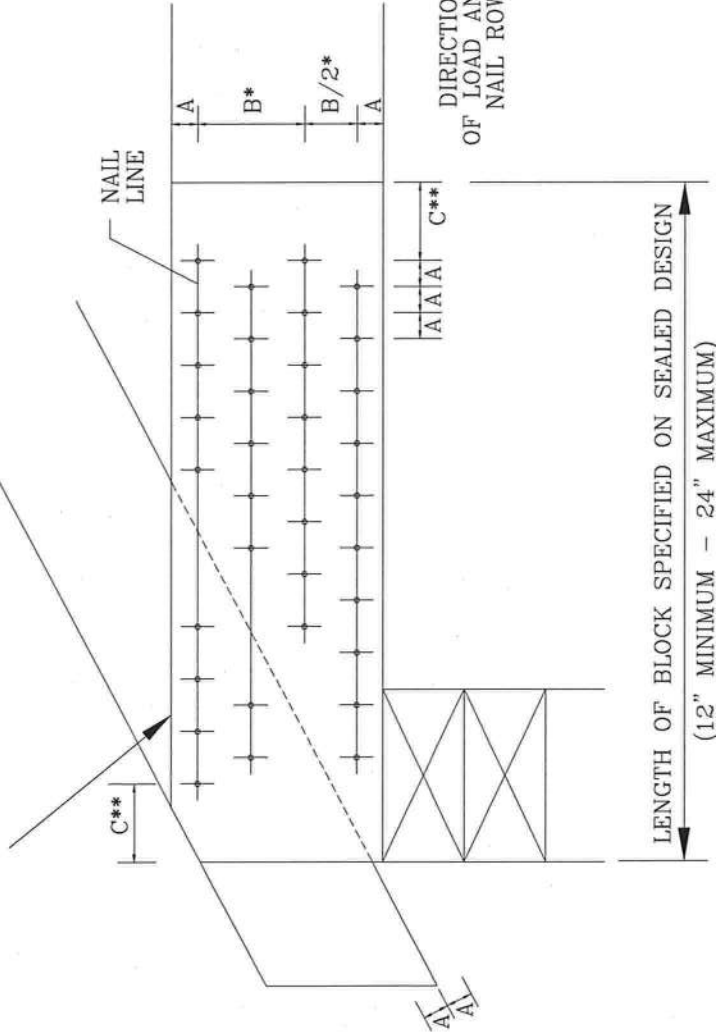
BEARING BLOCK NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

- A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C - END DISTANCE (15 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
• SPACING MAY BE REDUCED BY 50%
• SPACING MAY BE REDUCED BY 33%

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN VALUE (Fc-perp) IS AT LEAST THAT OF THE CHORD.



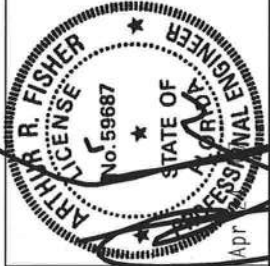
MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES		
	A	B*	C**
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"
10d BOX (0.128"X 3",MIN)	7/8"	1 5/8"	2"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"
20d BOX (0.148"X 4",MIN)	1"	1 7/8"	2 1/4"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"
10d COMMON (0.148"X 3",MIN)	1"	1 7/8"	2 1/4"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"
16d COMMON (0.162"X 3.5",MIN)	1"	2"	2 1/2"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"
GUN (0.120"X 3",MIN)	3/4"	1 1/2"	1 7/8"
GUN (0.131"X 3",MIN)	7/8"	1 5/8"	2"

THIS DRAWING REPLACES DRAWING B139 AND CNBRGBLK0699

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REF	BEARING BLOCK
DATE	2/23/07
DRWG	CNBRGBLK0207
-ENG	SJP/KAR



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

MAX GABLE CABLE VERTICAL LENGTH		2X4		BRACE		NO BRACES		(1) 1X4 "L" BRACE		(2) 2X4 "L" BRACE		(1) 2X6 "L" BRACE		(2) 2X6 "L" BRACE	
GABLE VERTICAL SPACING	SPECIES	GRADE	#1 / #2	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF	STUD	#1 / #2	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	12' 5"	12' 9"	14' 0"	14' 0"
	HF		#3	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	12' 4"	12' 4"	14' 0"	14' 0"
	SP		#1	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	10' 7"	10' 7"	14' 0"	14' 0"
	DFL		#2	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	12' 5"	13' 5"	14' 0"	14' 0"
16" O.C.	SPF	STUD	#3	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	12' 5"	12' 8"	14' 0"	14' 0"
	HF		#1 / #2	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	12' 5"	12' 6"	14' 0"	14' 0"
	SP		#3	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	10' 10"	10' 10"	14' 0"	14' 0"
	DFL		#1	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	STUD	#2	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF		#3	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP		#1	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	14' 0"	14' 0"
	DFL		#2	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	HEM-FIR
#1 / #2	#2
STUD	STUD
#3	#3
STANDARD	STANDARD

DOUGLAS FIR-LARCH

#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR	DOUGLAS FIR-LARCH
#1 & BTR	#1
#1	#2

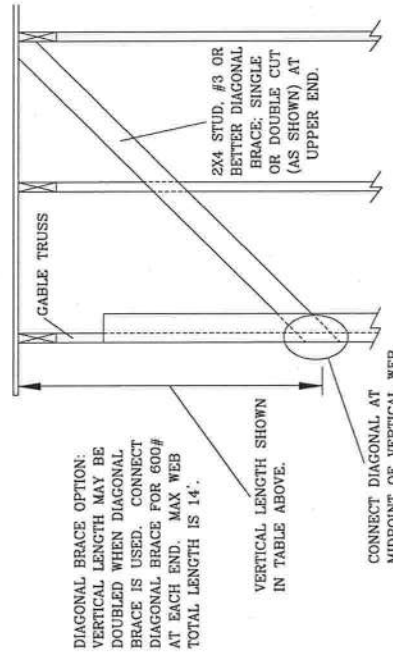
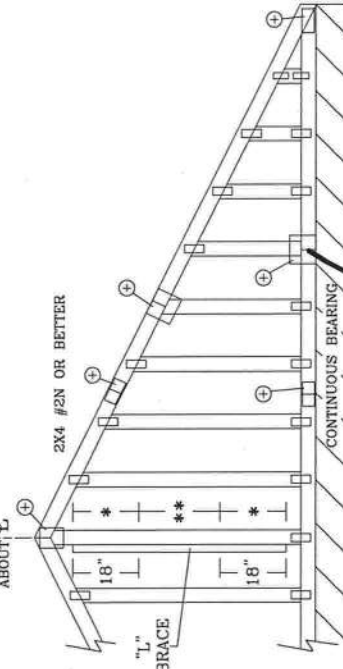
SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2

GABLE TRUSS DETAIL NOTES:

- LIVE LOAD DEFLECTION CRITERIA IS L/240.
- PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
- GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
- ATTACH EACH "L" BRACE WITH 10d NAILS.
- * FOR (1) "L" BRACE: SPACE NAILS AT 2' O.C. IN 18" END ZONES AND 4' O.C. BETWEEN ZONES.
- ** FOR (2) "L" BRACES: SPACE NAILS AT 3' O.C. IN 18" END ZONES AND 6' O.C. BETWEEN ZONES.
- "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

* REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

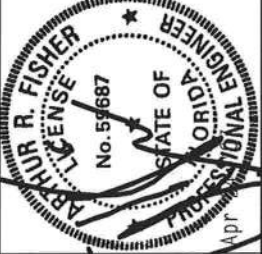


DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 600# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



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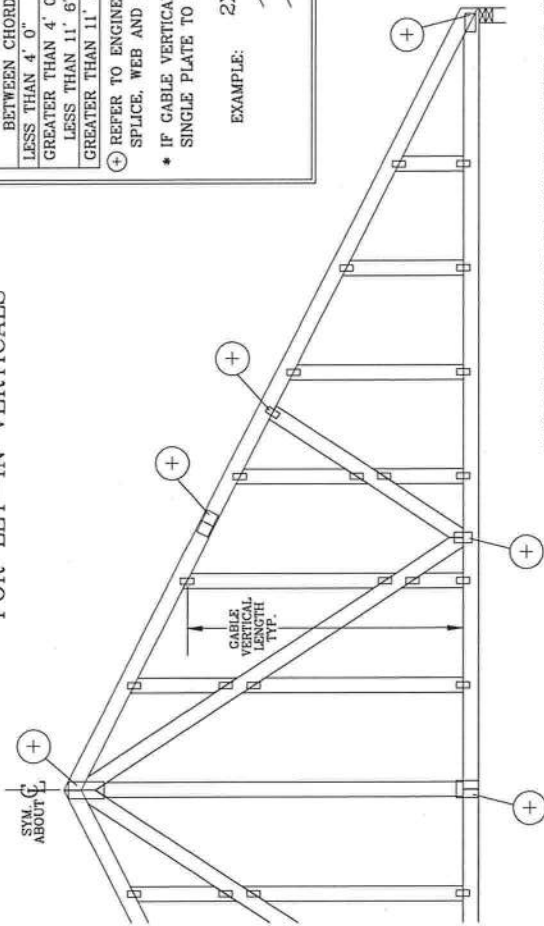
REF	ASCE7-02-GAB11015
DATE	2/23/07
DRWG	A11015EE0207
-ENG	

MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"



ITV BUILDING COMPONENTS GROUP, INC.
POMPAÑO BEACH, FLORIDA

GABLE DETAIL FOR LET-IN VERTICALS

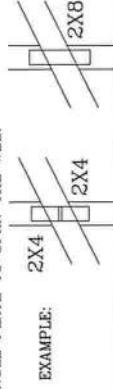


GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.



EXAMPLE:

PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH

HAND DRIVEN NAILS:

- 10d COMMON (0.148" X 3" MIN) TOENAILS AT 4" O.C. PLUS
- (4) 16d COMMON (0.162" X 3.5" MIN) TOENAILS IN TOP AND BOTTOM CHORD.

GUN DRIVEN NAILS:

- 8d COMMON (0.131" X 2.5" MIN) TOENAILS AT 4" O.C. PLUS
- (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS

A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08015EC0207, A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08030EC0207

ASCE 7-02 GABLE DETAIL DRAWINGS

A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08015EE0207, A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08030EE0207

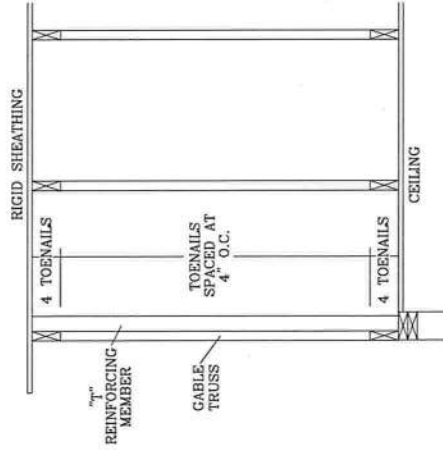
ASCE 7-05 GABLE DETAIL DRAWINGS

A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08015E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08030E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI

WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE

VERTICAL LENGTH.



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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2X8 "T" REINFORCING MEMBER

2X4 "T" REINFORCING MEMBER

TOENAIL

TOENAIL

TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH

MEAN ROOF HEIGHT = 30 FT

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

"L" BRACE INCREASE (FROM ABOVE) = 10% = 1.10

(1) 2X4 "L" BRACE LENGTH = 6' 7"

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH

1.10 x 6' 7" = 7' 3"

THIS DRAWING REPLACES DRAWINGS GAB98117 876.719 & HC26294035

REF LET-IN VERT

DATE 2/23/07

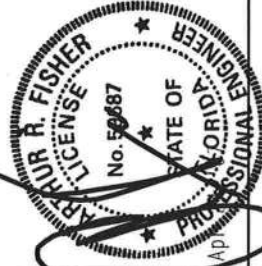
DRWG GBLLETIN0207

-ENG DLJ/KAR

MAX TOT. LD. 60 PSF

DUR. FAC. ANY

MAX SPACING 24.0"

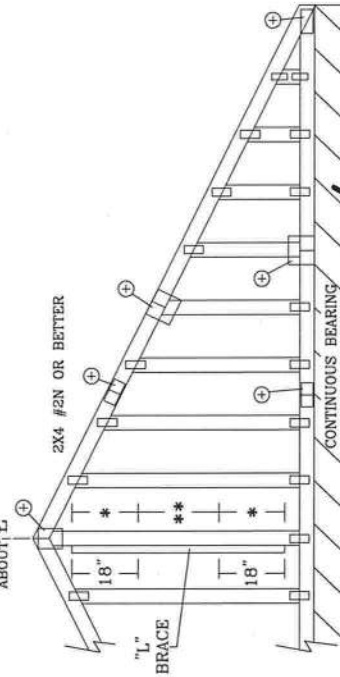


2X4 GABLE VERTICAL SPACING / SPECIES		BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE •		(1) 2X4 "L" BRACE •		(2) 2X4 "L" BRACE ••		(1) 2X6 "L" BRACE •		(2) 2X6 "L" BRACE •	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
O.C.	SPF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
	HF	STUD	3' 7"	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 1"	6' 1"	8' 3"	8' 3"	9' 6"	9' 6"	12' 11"	12' 11"
		#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
24"	SP	#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
	DFL	STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	14' 0"
		#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 10"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"
O.C.	SPF	#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
	HF	STUD	4' 1"	8' 0"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	7' 6"	10' 1"	10' 1"	11' 8"	11' 8"	14' 0"	14' 0"
		#1	4' 7"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#2	4' 6"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
16"	SP	#3	4' 4"	6' 10"	6' 10"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
	DFL	STUD	4' 4"	6' 9"	6' 9"	8' 7"	8' 11"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 10"	5' 10"	7' 8"	7' 8"	10' 3"	10' 4"	11' 11"	11' 11"	14' 0"	14' 0"
		#1 / #2	4' 7"	8' 0"	8' 2"	9' 5"	9' 8"	11' 3"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
O.C.	SPF	STUD	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STANDARD	4' 6"	6' 7"	6' 7"	8' 8"	8' 8"	11' 3"	11' 3"	13' 6"	13' 6"	14' 0"	14' 0"
		#1	5' 1"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 11"	7' 11"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
12"	DFL	STUD	4' 9"	7' 9"	7' 9"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 7"	6' 9"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"

DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONNECT
DIAGONAL BRACE FOR 700
AT EACH END. MAX WEB
TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN
IN TABLE ABOVE.

CONNECT DIAGONAL AT
MIDPOINT OF VERTICAL.



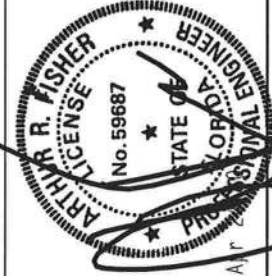
REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



ITW BUILDING COMPONENTS GROUP, INC.
POMONA BEACH, FL 33462

***WARNING:** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION PUBLISHED BY THE MANUFACTURER. INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22314 AND VITA CORDO TRUSS COUNCIL IN AMERICA, 6300 ENTERPRISE LN, WILSON, WI 53179 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

***IMPORTANT:** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD IN CONFORMANCE WITH ITPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS NATIONAL DESIGN SPEC. BY AF&PA AND TPI. ALL BCG CONNECTOR PLATES ARE MADE OF 2018/16GA C/V/H/S3/K0 ASTM A573 GRADE 40/60 C/V/H/S3 GALV. STEEL. APPLY PLATES TO EACH FACE OF TOP CHORDS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 1-2008 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEER. THE SEAL IS NOT VALID FOR THIS TRUSS DESIGN. DESIGN SHOWN IS THE SUITABLE AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING ENGINEER, PER ITPI 1-2008 TPI 1-SEC. 2



REF	ASCE7-02-GABI1030
DATE	2/23/07
DRWG	A11030EE0207
-ENG	

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

DOUGLAS FIR-LARCH

#3	
STUD	
STANDARD	

GROUP B:

HEM-FIR
#1 & BTR
μs

SOUTHERN PINE DOUGLAS FIR-LARCH

1	1
2	2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER
CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORT'S LOAD FROM 4' 0"

OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) L BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR
PEAK, SPLICE, AND HEEL PLATES

PIGGYBACK DETAIL

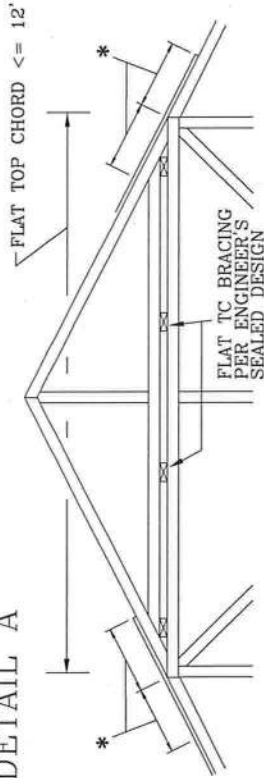
100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

80 MPH WIND, 30.00 FT MEAN HGT, SBC, ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. PROVIDE DIAGONAL BRACING OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

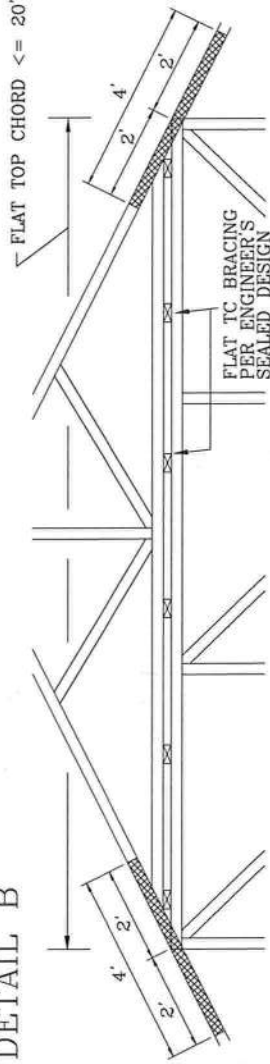
DETAIL A



PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS.

* 12" MIN RIGID SHEATHING OVERLAP WITH 8d COMMON (0.131"x2.5") OR GUN NAILS IN OVERLAP ZONE SPACED AT 4" O.C.

DETAIL B

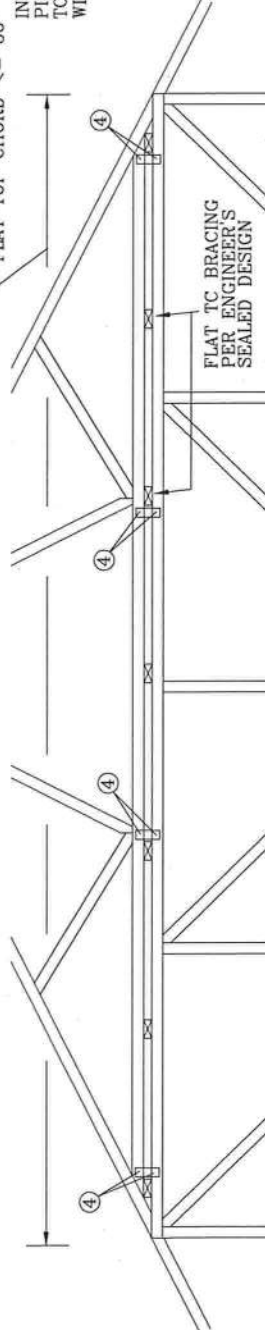


PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS AND SECURED WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY) ATTACHED WITH 10d COMMON NAILS AT 4" O.C.

DETAIL C

CAP TRUSS TOENAILED TO TOP CHORD BRACING AND SECURED WITH 3X8 TRULOX PLATES (EACH FACE) AT EACH END AND AT 1/3 POINTS. CIRCLED NUMBER INDICATES REQUIRED NUMBER OF 0.120" X 1.375" NAILS PER FACE. SEE DRAWING 160TL FOR TRULOX INFORMATION.

FLAT TOP CHORD <= 30'



IN LIEU OF TRULOX CONNECTORS, ALPINE 62PB SPECIAL PIGGYBACK CONNECTORS MAY BE USED. SHOP APPLY TOOTHED PORTION, FIELD ATTACH TO MATING TRUSS WITH (4) 0.120" X 0.375" NAILS MINIMUM EACH FACE.

(4) 8d COMMON NAILS (0.131"x2.5")

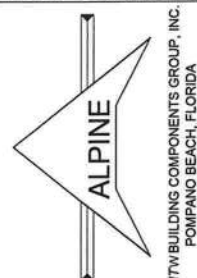
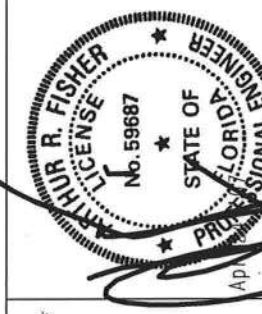
8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES. ATTACH WITH (8) 8d COMMON NAILS PER GUSSET, (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

THIS DRAWING REPLACES DRAWINGS 581,670 & 961,860

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22314) AND VITA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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TC LL	PSF	REF	PIGGYBACK
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	PIGBACKA0207
BC LL	PSF	-ENG	DLJ/KAR
TOT. LD.	MAX 60 PSF		
DUR. FAC.	1.15		
SPACING	24.0"		



ITV BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

THIS DRAWING SPECIFIES REPAIRS FOR A TRUSS WITH CRACKED OR BROKEN WEBS.

THIS DESIGN IS VALID ONLY FOR SINGLE PLY TRUSSES WITH 2X4 #3, STUD, OR STANDARD CRACKED OR BROKEN WEBS. NO MORE THAN 1 CRACK OR BREAK PER WEB AND 2 CRACKED OR BROKEN WEBS PER TRUSS ARE ALLOWED. CONTACT THE TRUSS MANUFACTURER FOR ANY REPAIRS THAT DO NOT COMPLY WITH THIS DETAIL.

CRACKED OR BROKEN WEB REPAIR DETAIL

(B) = DAMAGED AREA, 0" MIN TO 12" MAX LENGTH OF CRACK OR BREAK IN WEB.

(S) = (2) 2X4 SCABS, SAME GRADE, SPECIES AS WEB MEMBER. MINIMUM

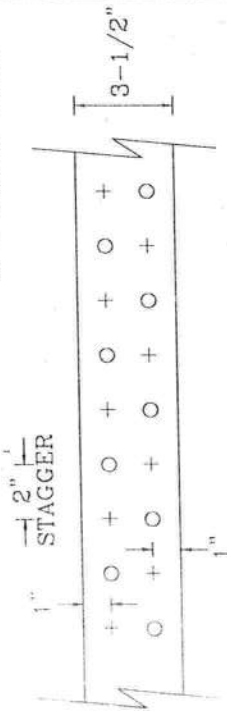
LENGTH OF SCAB MUST BE THE GREATER OF:

- 68" + LENGTH OF DAMAGED AREA (B), MINIMUM OF 34" ON BOTH SIDES OF THE DAMAGED AREA.
 - OR
 - 80% OF THE ORIGINAL WEB LENGTH.
- ATTACH ONE SCAB TO EACH FACE OF THE WEB WITH A DOUBLE ROW OF 10d COMMON NAILS SPACED 4" OC STAGGERED. REFER TO NAIL SPACING DETAIL FOR ADDITIONAL NAIL SPACING INFORMATION.

NOTE: FIELD REPAIRS MUST COMPLY WITH ALPINE DESIGNS AND SPECIFICATIONS.

10d COMMON (0.148" x 3") NAILS:
DOUBLE ROW, STAGGERED

4" OC
TYPICAL
+ = BACK FACE
O = FRONT FACE



NAIL SPACING DETAIL

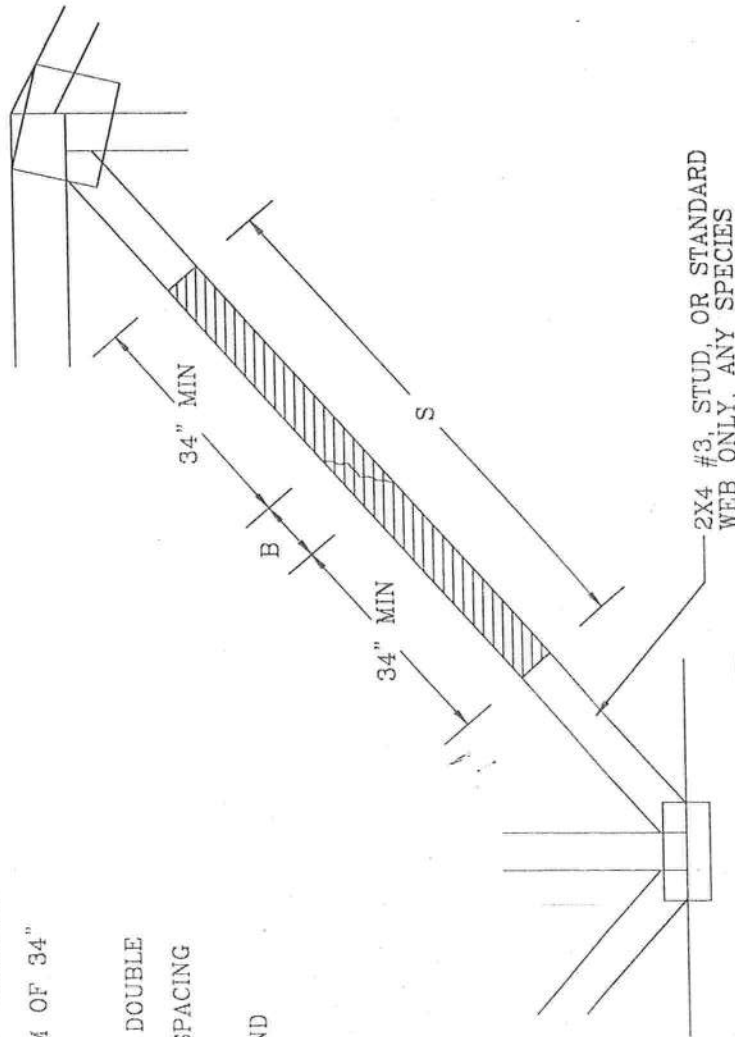
TRUSS REPAIR

DAMAGED TRUSSES MUST BE CAREFULLY EVALUATED TO DETERMINE THE EXTENT OF DAMAGE AND THE FEASIBILITY OF REPAIR. IN SOME CASES, THE PRUDENT SOLUTION IS TO SCRAP THE DAMAGED TRUSSES AND REBUILD. INTERMEDIATE REPAIRS ARE ALLOWED AND EXCESS CONNECTOR STRESS FROM BENDING OR SHOCK CANNOT BE READILY DETECTED. THEREFORE, IT IS VITAL THAT THE TRUSS FABRICATOR AND BUILDING CONTRACTOR CONSIDER THE CAUSE OF THE DAMAGE IN THEIR DECISION WHETHER TO REPAIR OR REBUILD.

REPAIR WORK SHOWN ON THIS DRAWING APPLIES ONLY TO THOSE SECTIONS OF THE TRUSS REPORTED BY THE TRUSS MANUFACTURER TO HAVE BEEN DAMAGED. A QUALIFIED THIRD PARTY INSPECTOR SHALL CHECK TRUSSES TO DETERMINE THE EXTENT OF ANY FURTHER DAMAGE, IF ANY, AND VERIFY THAT REPAIRS HAVE BEEN PERFORMED AS INDICATED ON THIS DRAWING.



ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA



THIS DRAWING REPLACES DRAWINGS HC25094073 & 958.849

REF	WEB REPAIR
DATE	11/26/03
DRWG	REPWEBSC1103
-ENG	MLH/KAR

SPACING 24.0"

TOP CHORD FILLER DETAIL

- + 2X4 CONTINUOUS LATERAL BRACING AT 24" OC MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH (2) 16d NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.
- ++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.
- +++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED 48" OC MAXIMUM.
- * 8/12 MAXIMUM PITCH.
- ** 2X8.25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGBACKB0399 FOR PIGGYBACK SPECIAL PLATE INFORMATION.

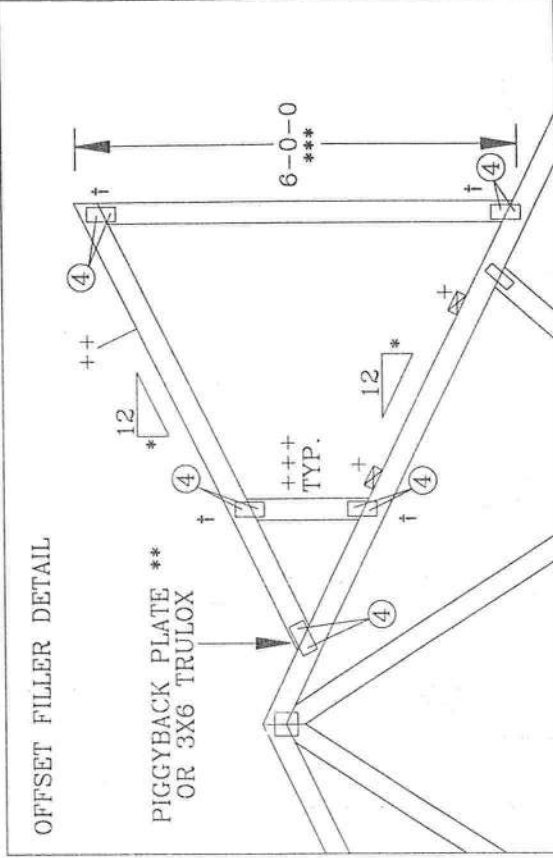
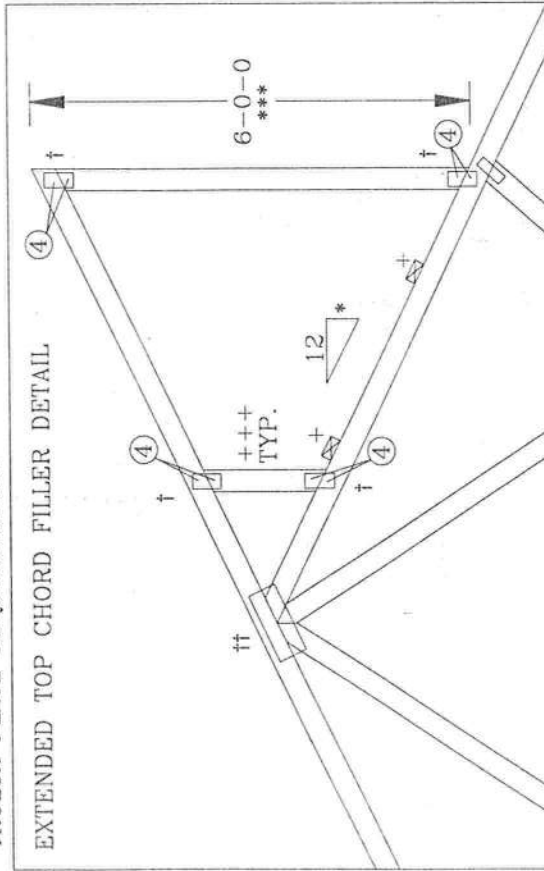
*** 6'0" MAXIMUM HEIGHT.

† W2X4 OR 3X6 TRULOX.

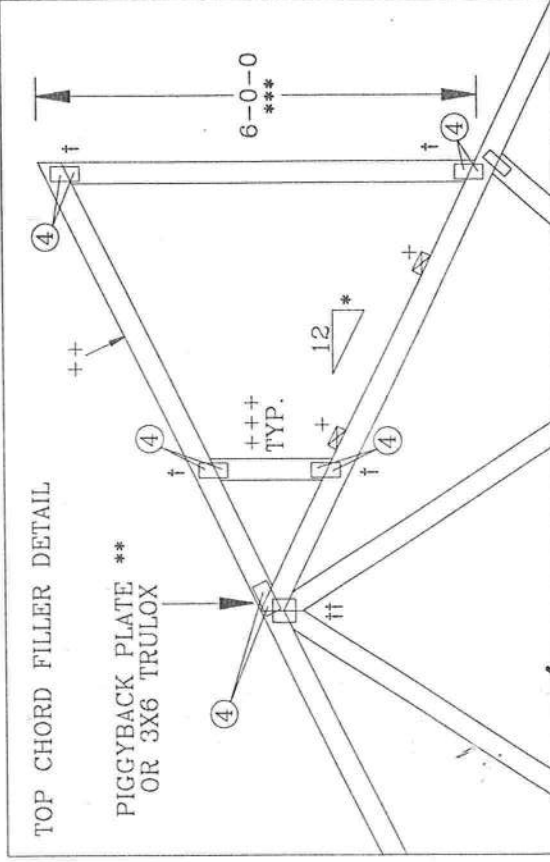
†† REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

11 GAUGE (0.120")X1.375" NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF EACH TRUSS PLY. SEE DWG 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS.

EXTENDED TOP CHORD FILLER DETAIL

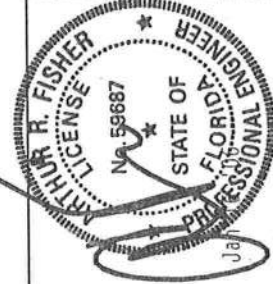


TOP CHORD FILLER DETAIL



THIS DRAWING REPLACES DRAWING 884,080

TC LL	MAX 30 PSF	REF	TC-FILLER
TC DL	MAX 15 PSF	DATE	11/26/03
BC DL	MAX 10 PSF	DRWG	TCFILLER1103
BC LL	0 PSF	-ENG	SJP/KAR
TOT. LD.	MAX 55 PSF		
DUR. FAC.	1.15 OR 1.33		
SPACING	24.0"		



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 563 DUNDRIED DR., SUITE 200, MADISON, WI 53719) AND VTC (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL BE FABRICATED FROM ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA 4X4/5X4 ASTM A653 GRADE 40/60 (K/K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3.1. A SEAL FOR THE TRUSS COMPONENT DESIGN SHOWN. THE PROFESSIONAL ENGINEER'S SEAL IS REQUIRED FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/TPI 1 SEC. 2.



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POMPANO BEACH, FLORIDA

BOTTOM CHORD FILLER DETAIL

* OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION.

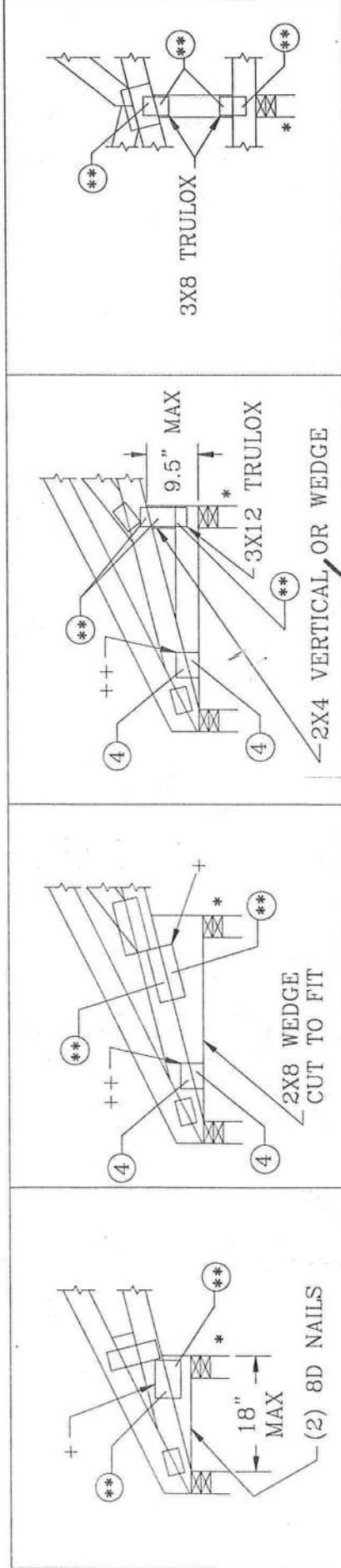
+ 3X4 WAVE OR 4X8 TRULOX
++ 2X4 WAVE OR 3X6 TRULOX

11 GAUGE (0.120")X1.375" NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF THE TRUSS. SEE DWG 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

ALL TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES MAY BE REQUIRED TO ACCOMMODATE REQUIRED NAILS (**)

FILLER BOTTOM CHORD OR WEDGE SPECIES	MAXIMUM REACTION		MINIMUM BEARING AREA	** REQUIRED NAILS PER FACE WITH TRULOX PLATES						
	DOWNWARD	UPLIFT		1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.		
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	12	11	10	9	8		
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	6		
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6		
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8		
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7		
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"	9	8	7	7	6		



ALPINE
ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

THIS DRAWING REPLACES DRAWINGS A115 A115/R & 884.132

TC LL	—	PSF
TC DL	—	PSF
BC DL	10.0	PSF
BC LL	—	PSF
TOT. LD.	—	PSF
DUR. FAC.	1.0/1.15/1.25/1.33	
SPACING	24.0"	

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 DUNDON DR., SUITE 200, MADISON, WI 53719) AND AISC (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

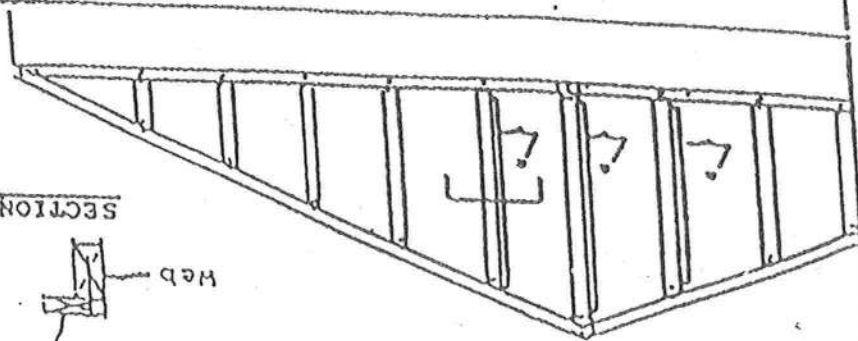
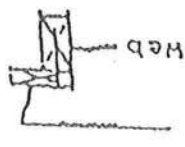
****IMPORTANT**** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (W/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY TPI SHALL BE PER ANNEX A3 OF TPI 1-03. NO SECONDARY DESIGN SHALL BE USED WITHOUT THE WRITTEN APPROVAL OF THE DESIGNER. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER

EXAMPLES OF WEB BRACES

L-BRACE

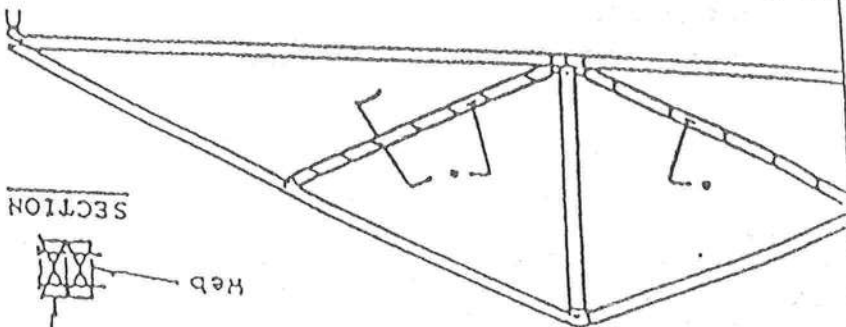
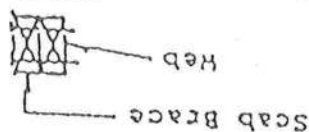
SECTION



1x4 "L" brace. Same grade as web member. Attach with 8d nails @ 6" o.c. Bracing material to be supplied by erection contractor.

SCAB BRACE

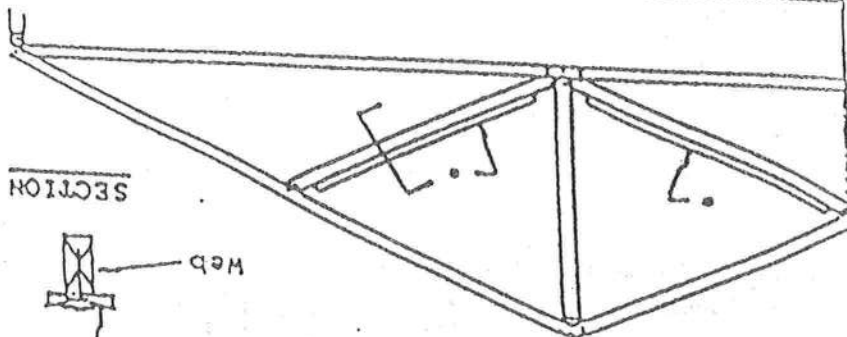
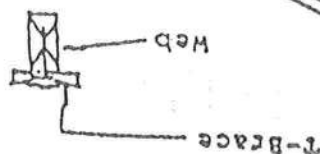
SECTION



Scab brace, same size, grade and length as web member. Attach with 16d nails @ 4" o.c. Bracing material to be supplied by erection contractor.

T-BRACE

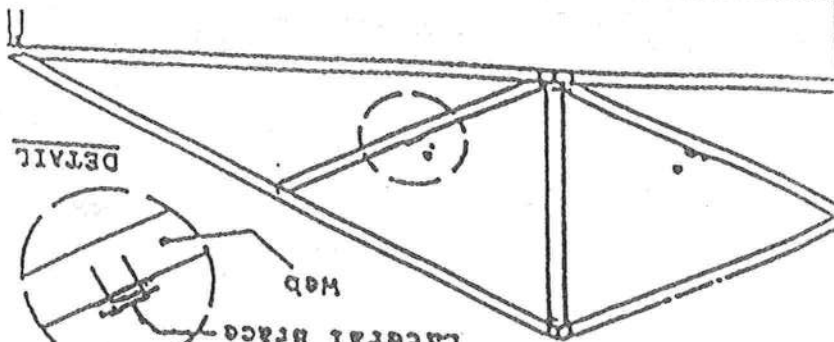
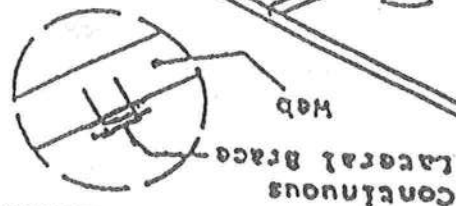
SECTION



1x4 "T" Brace. Same grade as web member. Attach with 8d nails @ 6" o.c. Bracing material to be supplied by erection contractor.

CONTINUOUS LATERAL BRACE

DETAIL



1x4 #3 Hem-Plk or better continuous lateral bracing to be equally spaced. Attach with (2) 10d nails. Bracing material to be supplied and attached at both ends to a suitable support by erection contractor.

SUPPORT REPORT
WIND CODE: ASCE 7-02

JOB DESCRIPTION: 7-111
WIND MPH: 110

BLDG TYPE:

CLOSED

TRUSS DESC	TRUSS SPAN-ft	SUPPORT SIZE-in.	SUPPORT TYPE	BEARING XLOC-ft.	BEARING YLOC-ft.	REACT. MAX.+#	REACT. MAX.-#	MAX WIND UPLFT.-#
H7A	40.000	3.500	WALL	0.000	14.000	3470		-287
H7A	40.000	3.500	WALL	39.708	14.000	3470		-287
H9A	40.000	3.500	WALL	0.000	14.000	1818		-192
H9A	40.000	3.500	WALL	39.708	14.000	1818		-192
H11A	40.000	3.500	WALL	0.000	14.000	1818		-195
H11A	40.000	3.500	WALL	39.708	14.000	1818		-195
H13A	40.000	3.500	WALL	0.000	14.000	1818		-198
H13A	40.000	3.500	WALL	39.708	14.000	1818		-198
H15A	40.000	3.500	WALL	0.000	14.000	1818		-201
H15A	40.000	3.500	WALL	39.708	14.000	1818		-201
H15AA	40.000	3.500	WALL	0.000	14.000	2913		-379
H15AA	40.000	3.500	WALL	39.708	14.000	2927		-381
H15A-GDR	40.000	3.500	WALL	0.000	14.000	7080		-921
H15A-GDR	40.000	3.500	WALL	39.708	14.000	9473		-1232
B-GE	11.000	132.000	WALL	0.000	10.000	2248		-839
B1	11.000	3.500	WALL	0.000	10.000	600		-180
B1	11.000	3.500	WALL	10.708	10.000	600		-180
C-GE	8.000	96.000	WALL	0.000	23.000	1740		-1189
C1	8.000	3.500	WALL	0.000	23.000	474		-180
C1	8.000	3.500	WALL	7.708	23.000	474		-180
HJ7	9.899	4.950	WALL	0.000	14.000	553		-180
HJ7	9.899	1.500	NAILED	9.899	18.686	258		-180
HJ7	9.899	1.500	NAILED	9.899	14.000	359		-180
EJ7	7.000	3.500	WALL	0.000	14.000	461		-180
EJ7	7.000	1.500	NAILED	7.000	14.000	77		-180
EJ7	7.000	1.500	NAILED	7.000	18.746	188		-180
J5	5.000	3.500	WALL	0.000	14.000	387		-180
J5	5.000	1.500	NAILED	5.000	14.000	48		-180
J5	5.000	1.500	NAILED	5.000	17.413	124		-180
J3	3.000	3.500	WALL	0.000	14.000	325		-180
J3	3.000	1.500	NAILED	3.000	14.000	27		-180
J3	3.000	1.500	NAILED	3.000	16.080	49		-180
J1	1.000	3.500	WALL	0.000	14.000	372		-180
J1	1.000	1.500	NAILED	1.000	14.000	19	-32	-180
J1	1.000	1.500	NAILED	1.000	14.747	47	-118	-180
H15AA-PB	10.000	6.310	WALL	0.000	24.371	20		-180
H15AA-PB	10.000	101.381	WALL	0.776	24.517	605		-245
H15AA-PB	10.000	6.310	WALL	9.474	24.371	20		-180

H15AA-PB	10.000	6.310	WALL	0.000	24.371	49	-28	-180
H15AA-PB	10.000	101.381	WALL	0.776	24.517	701		-319
H15AA-PB	10.000	6.310	WALL	9.474	24.371	26	-28	-180

H15AA-PB	10.000	6.310	WALL	0.000	24.371	81	-62	-180
H15AA-PB	10.000	101.381	WALL	0.776	24.517	769		-369
H15AA-PB	10.000	6.310	WALL	9.474	24.371	52	-62	-180

=====

HANGER SUMMARY FOR: 7-111

Quantity Hanger

=====

=====

HANGER DETAIL FOR: 7-111

Quantity Hanger

=====

CERTIFICATE OF OCCUPANCY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 05-5S-17-09116-115

Building permit No. 000025701

Use Classification DETACHED GARAGE

Fire: 0.00

Permit Holder JOHN DUNN

Waste: _____

Owner of Building JOHN & ELAINE DUNN

Total: 0.00

Location: 871 SW HILL CREEK DR, LAKE CITY, FL 32025

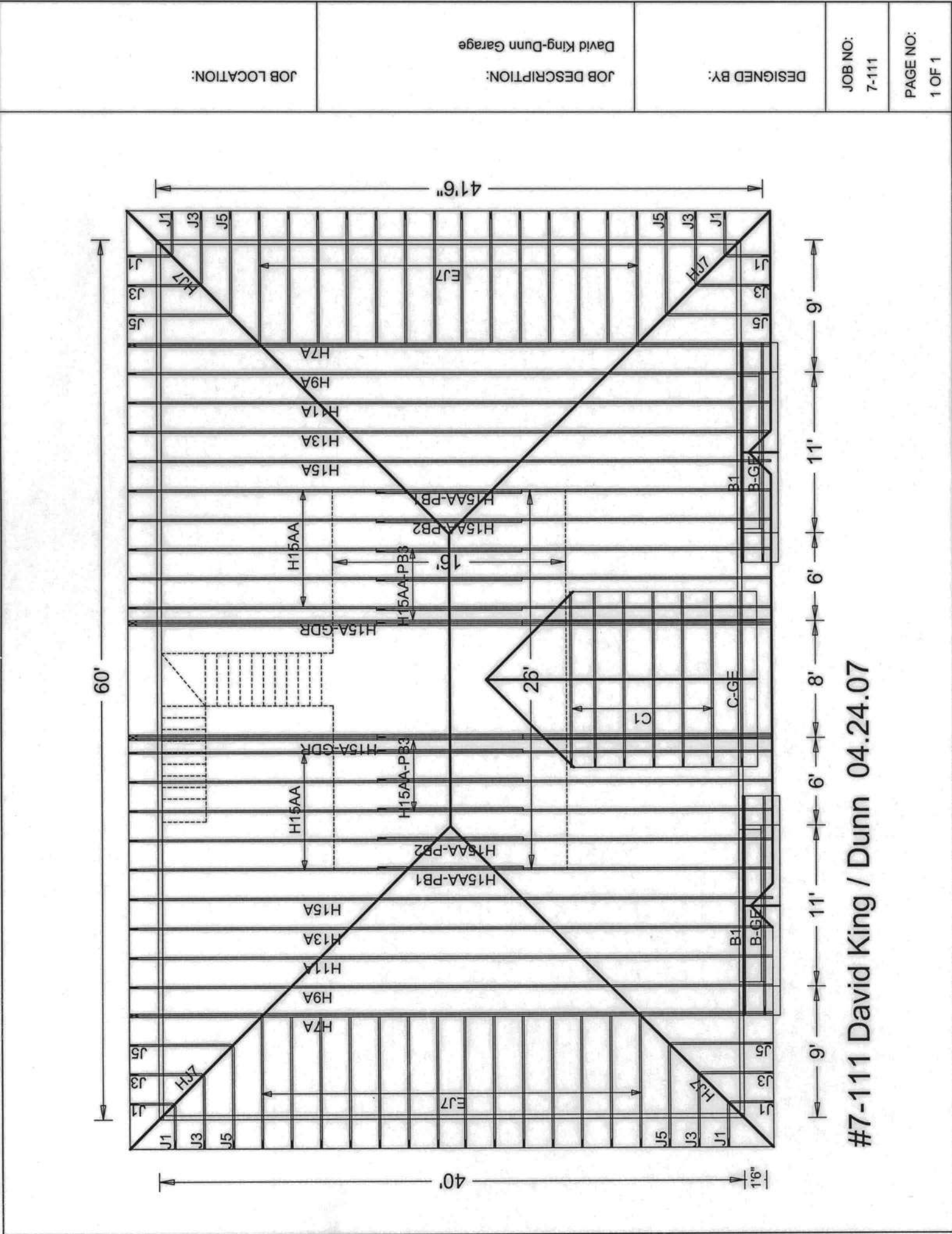
Date: 12/21/2007



Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

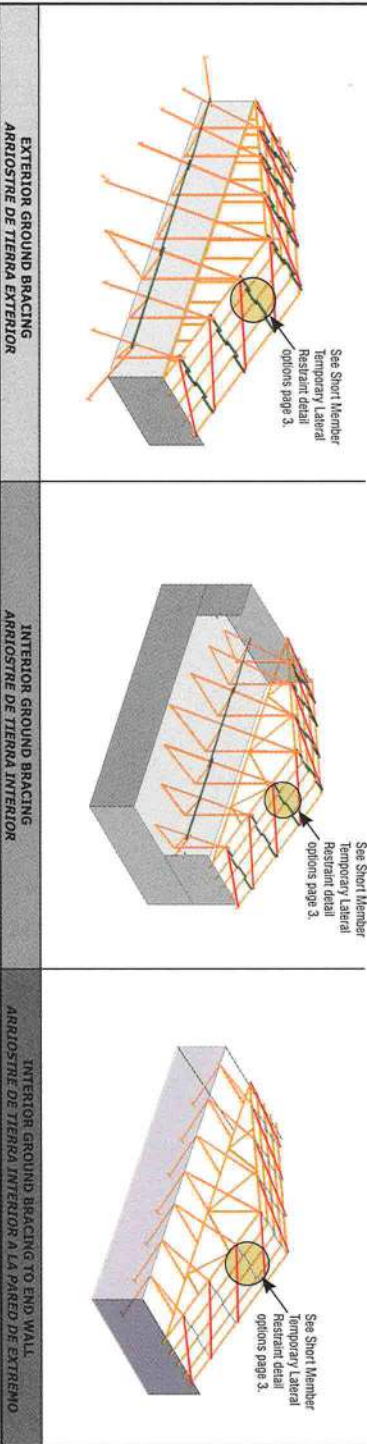




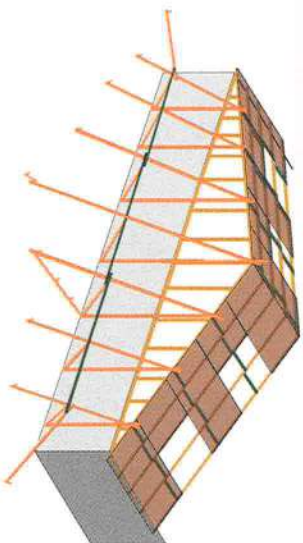
#7-111 David King / Dunn 04.24.07

5. Install Top Chord Diagonal Bracing
Instale Arriostre Diagonal en la Cuerda Superior

- Attach Diagonal Bracing to the first five trusses. Examples of Diagonal Bracing on first five trusses. Coloque Arriostre Diagonal a los primeros cinco trusses. Ejemplos de Arriostre Diagonal en los primeros cinco trusses.



- Or start applying Structural Sheathing. Example of Structural Sheathing installed on first five trusses. O empiece en aplicar el Entablado Estructural. Ejemplo de Entablado Estructural instalado en los primeros cinco trusses.

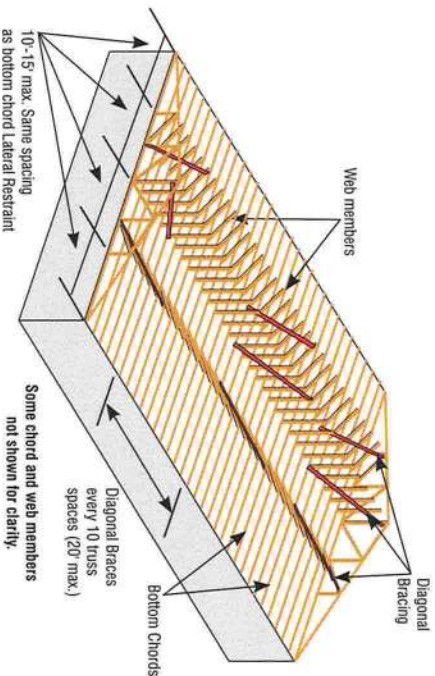


6. Install Web Member Diagonal Bracing
Instale el Arriostre Diagonal de Miembros Secundarios

- Temporary web member Diagonal Bracing acts with the top chord and bottom chord temporary Lateral Restraint and Diagonal Bracing to form triangulation perpendicular to the plane of the truss and prevents trusses from leaning or doming. El Arriostre Diagonal de los miembros secundarios temporales trabaja con la Restricción Lateral y el Arriostre Diagonal Temporal de la cuerda superior e inferior para formar una triangulación perpendicular al plano del truss y evita que los trusses se inclinen o caligan como domínos.

- Install Diagonal Bracing at about 45° on web members (Verticals whenever possible) located at or near bottom chord Lateral Restraint locations. Web Diagonal Bracing must extend from the top chord to the bottom chord. Repeat at the interval shown in the Figure at right. Instale Arriostre Diagonal a aproximadamente 45 grados en los miembros secundarios (verticales cuando sea posible); colocados en o cerca de las ubicaciones de Restricción Lateral de la cuerda inferior. Repita a los intervalos mostrados en la Figura a la derecha.

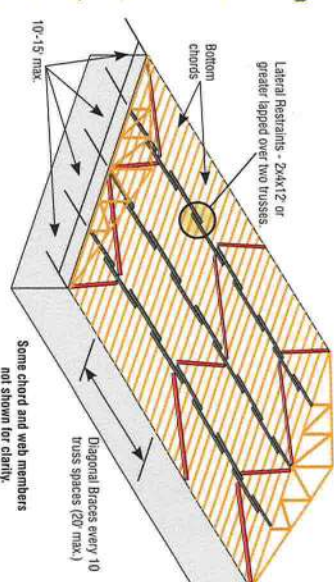
- The requirements for web member Permanent Individual Truss Member Restraint are specified on the Truss Design Drawing. Refer to BCSI-B3 Summary Sheet – Permanent Restraint/Bracing of Chords & Web Members for more information. Los requisitos para Restricción Permanente de Miembros Individuales de Truss para miembros secundarios son especificados en el Dibujo del Diseño de Truss. Vea el resumen BCSI-B3 – Restricción/Arriostre Permanente de las Cuerdas y los Miembros Secundarios para más información.



- Mono pitch trusses, deep flat trusses and other types of trusses with deep ends also require Temporary Lateral Restraint and Diagonal Bracing on the long web members at the deep end of the truss. Trusses de una sola pendiente, trusses planos y otros tipos de trusses con extremos profundos también requieren Restricción Lateral Temporal y Arriostre Diagonal en los miembros secundarios largos al parte profundo del truss.

7. Install Bottom Chord Lateral Restraint and Diagonal Bracing
Instale la Restricción Lateral y el Arriostre Diagonal de la Cuerda Inferior

- Bottom Chord Temporary Lateral Restraint and Diagonal Bracing stabilizes the bottom chord plane during installation and helps maintain proper spacing of the trusses. Restricción Lateral Temporal y Arriostre Diagonal de la Cuerda Inferior estabilizan el plano de la cuerda inferior durante la instalación y asiste en mantener espaciamiento apropiado de los trusses. Instálle Temporal Lateral Restraint at 15' on-center maximum. Remove, if desired, after the permanent ceiling diaphragm is in place. Instale la Restricción Lateral Temporal a 15 pies en el centro como máximo. Qútelos, si así lo desea, después que el diafragma permanente del techo esté colocado. Install Permanent Lateral Restraint at 10' on-center maximum. Specified spacing may be less; check with the Truss Design Drawing and/or the Building Designer. Instale la Restricción Lateral Temporal a 10 pies en el centro como máximo. El espaciamiento especificado puede ser menor; verifique con el Dibujo del Diseño de Truss y/o el Diseñador del Edificio.
- Install Diagonal Bracing at intervals of 20' maximum. Instale Arriostres Diagonales a intervalos de 20 pies máximo.

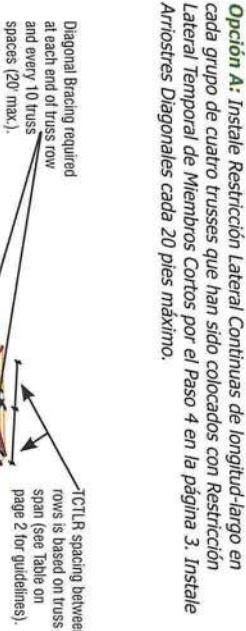


- IMPORTANT SAFETY WARNING!** Do not remove ground bracing until all top chord, bottom chord and web Lateral Restraint and Bracing is installed on at least the first five trusses.

- ADVERTENCIA IMPORTANTE DE SEGURIDAD!** No remueva el arriostre de tierra hasta que toda la Restricción Lateral y Arriostre de la cuerda superior, la cuerda inferior y los miembros secundarios estén instalados en por lo menos los primeros cinco trusses.

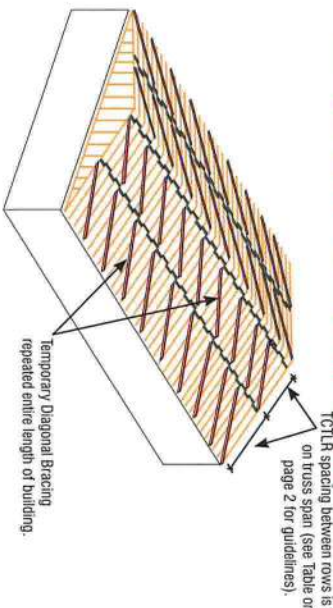
8. Repeat Steps Four Through Seven on Groups of Four Trusses Using Option A or B
Repita los Pasos Cuatro a Siete en Grupos de Cuatro Trusses Usando la Opción A o la Opción B

- Option A: Install long-length Continuous Lateral Restraints on each group of four trusses that have been set with Short Member Temporary Lateral Restraint per Step 4 on page 3. Install Diagonal Braces every 20' maximum. Opción A: Instale Restricción Lateral Continuas de longitud largo en cada grupo de cuatro trusses que han sido colocados con Restricción Lateral Temporal de Miembros Cortos por el Paso 4 en la página 3. Instale Arriostres Diagonales cada 20 pies máximo.



- WARNING!** After the initial set of five trusses are installed and braced (i.e., Lateral Restraint and Diagonal Bracing), DO NOT set more than four trusses when using Short Member Temporary Lateral Restraint before you STOP, and Diagonally Brace as shown. This approach will NOT work without Diagonal Bracing applied early and often.

- ADVERTENCIA!** Después de que el primer grupo de cinco trusses sea instalado y arriostreado (Ej. Restricción Lateral y Arriostre Diagonal), NO coloque más de cuatro trusses cuando se usa Restricción Lateral Temporal de los Miembros Cortos antes que PARA, y arriostrelo diagonalmente como mostrado. Este enfoque NO funcionará sin aplicar el Arriostre Diagonal temprano y con frecuencia.



- ENSURE THAT ALL TRUSSES ARE PROPERLY DIAGONALLY BRACED** Apply Structural Sheathing early and often. DO NOT wait until all trusses are set to apply Structural Sheathing. ASEGÚRESE QUE TODOS LOS TRUSSES ESTÉN ARRIOSTRADOS DIAGONALMENTE APROPIADAMENTE. Aplique Entablado Estructural temprano y con frecuencia, NO ESPERE hasta que todos los trusses estén colocados para aplicar Entablado Estructural.



- WARNING!** Remove only as much top chord Temporary Lateral Restraint and Diagonal Bracing as is necessary to nail down the next sheet of Structural Sheathing. DO NOT EXCEED TRUSS DESIGN LOAD WITH CONSTRUCTION LOADS. (SEE BCSI-B4) **ADVERTENCIA!** Quite sólo tanta Restricción Lateral Temporal y Arriostre Diagonal de la cuerda superior como sea necesario para clavar la siguiente hoja de Entablado Estructural. NO EXCEDA LA CARGA DE DISEÑO DE TRUSS CON CARGAS DE CONSTRUCCION. (VEA BCSI-B4)...

STEPS TO SETTING TRUSSES

PASOS PARA EL MONTAJE DE TRUSSES

Establish Ground Bracing Procedure: Exterior or Interior

Establezca el Procedimiento de Arriostre de Tierra: Exterior o Interior

Exterior Ground Bracing

ARRIOSTRE DE TIERRA EXTERIOR

Interior Ground Bracing

ARRIOSTRE DE TIERRA INTERIOR

Interior Ground Bracing to End Wall

ARRIOSTRE DE TIERRA INTERIOR A LA PARED DE EXTREMO

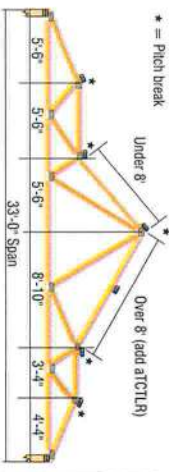
2. Determine the locations for TCTLR and Ground Brace Verticals

Determine las ubicaciones para TCTLR y las Verticales de Arriostre de Tierra

- Use truss span to determine spacing for Top Chord Temporary Lateral Restraint (TCTLR) from table at right. Use el vano de truss para determinar el espaciamiento para Restricción Lateral Temporal de la Cuerda Superior (TCTLR) en la tabla a la derecha.

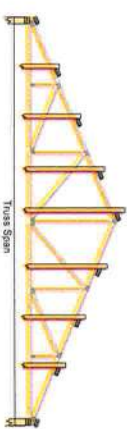
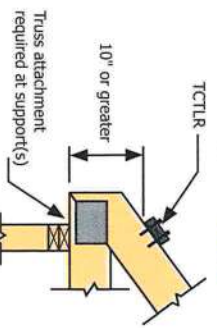
Maximum Top Chord Temporary Lateral Restraint Spacing**	
Truss Span	Top Chord Temporary Lateral Restraint (TCTLR) Spacing
Up to 30'	10' on-center maximum
30' - 45'	8' on-center maximum
45' - 60'	6' on-center maximum
60' - 80' +	4' on-center maximum

*Consult a Professional Engineer for trusses longer than 60'. **For trusses in Post-Frame buildings, see also BCS1-810.

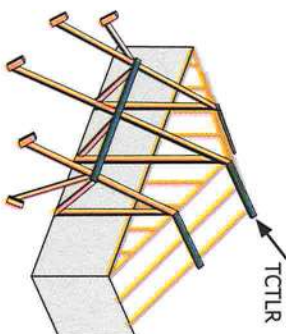


- Locate additional TCTLR at each change of pitch. Localice TCTLR adicional en cada cambio de inclinación.

- Locate additional TCTLR over bearings if the heel height is 10" or greater. Localice TCTLR adicional sobre los soportes si la altura del talón (heel height) es de 10 pulgadas o más.



- Locate a Ground Brace Vertical at each TCTLR location. Localice una Vertical de Arriostre de Tierra en cada lugar de TCTLR.



3. Set First Truss and Fasten Securely to Ground Brace Verticals

Coloque el Primer Truss y Fíjelo Seguramente a los Verticales de Arriostre de Tierra.

- Set first truss (or Gable End Frame) and fasten securely to Ground Brace Verticals and to the wall, or as directed by the Building Designer. Ex-amples of first truss installed. Coloque el primer truss (o Armazón Hastial) y fíjelo seguramente a las Verticales de Arriostre de Tierra y a la pared, o como se dirige el Diseña-dor del Edificio. Ejemplos del primer truss instalado.

Exterior Ground Bracing

ARRIOSTRE DE TIERRA EXTERIOR

Interior Ground Bracing

ARRIOSTRE DE TIERRA INTERIOR

Interior Ground Bracing to End Wall

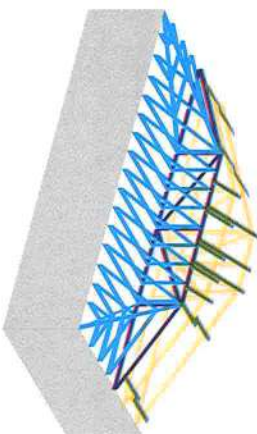
ARRIOSTRE DE TIERRA INTERIOR A LA PARED DE EXTREMO

First truss must be attached securely at each bearing and to all Ground Brace Verticals prior to removing the hoisting supports.

El primer truss tiene que ser sujetado seguramente a cada soporte y a todas las Verticales de Arriostre de Tierra, antes de quitar los soportes de levantar.

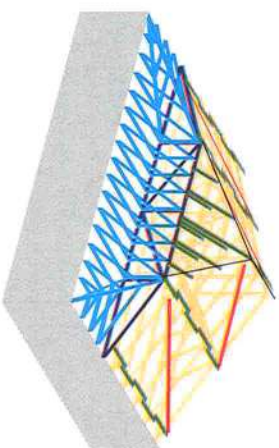
IMPORTANT SAFETY WARNING!

ADVERTENCIA IMPORTANTE DE SEGURIDAD!



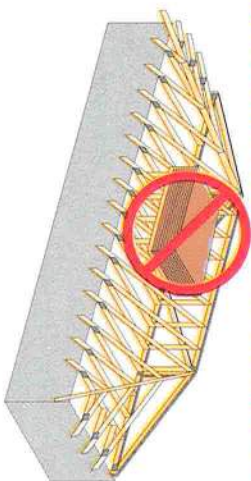
Step 6 Install pitched trusses using the guidelines provided in Steps 2-8 on pages 2-5 of this document.

Step 6 Instale trusses pendientes usando las directrices provistos en Pasos 2-8 en páginas 2-5 de este documento.



Warning! DO NOT stack materials or stand on end jacks as it causes instability in the hip gilder.

¡Advertencia! NO amontone materiales ni pise en los gatos extremos por así causa inestabilidad en el travesaño de cadena.



LONG SPAN TRUSS INSTALLATION

INSTALACIÓN DE TRUSSES DE VANOS LARGOS

WARNING! TRUSSES OVER 60' IN LENGTH ARE EXTREMELY DANGEROUS TO INSTALL AND REQUIRE MORE DETAILED SAFETY AND HANDLING MEASURES THAN SHORTER SPAN TRUSSES

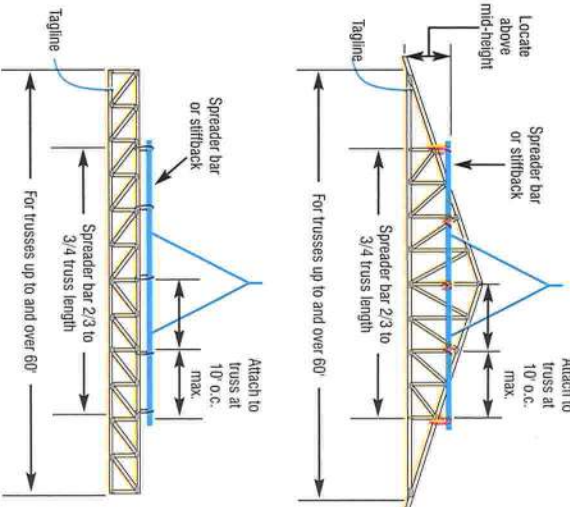
¡ADVERTENCIA! TRUSSES MÁS DE 60 PIES DE LARGO SON EXTREMAMENTE PELIGROSOS A INSTALAR Y REQUIEREN MEDIDAS DE SEGURIDAD Y MANEJO MÁS DETALLADAS QUE TRUSSES DE VANOS MÁS CORTOS

BEFORE STARTING

ANTES DE EMPEZAR

- Hire a Professional Engineer to provide a temporary restraint/bracing plan and to supervise the erection process. Contrate un Ingeniero Profesional para proveer un plan de restric-ción/arriostre temporal y para supervisar el proceso de erección.
- Develop a safe, effective truss installation method and inform all crew members of their roles. Desarrolle a safe, efectiva truss installation method and inform all crew members of their roles.
- Elabore un método seguro y efectivo de la instalación de trusses e informe todos tripulantes de sus papeles.
- Use installers who have experience installing trusses greater than 60' in span. Use instaladores que tienen experiencia en instalar trusses más que 60 pies de vano.
- Ensure that the walls and supporting structure are stable and ad-equately restrained and braced. Asegure que las paredes y la estructura soportante son estables y adecuadamente restringidos y arriostados.

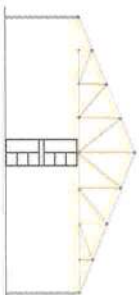
- Have all necessary lifting equipment and building materials on hand. Make sure the crane operator understands the special hoisting requirements of Long Span Trusses (see BCS1-B1). Tenga todo equipo de levantar y los materiales de construcción nec-esarios a la mano. Asegúrese que el operador de grúa entiende los requisitos especiales de levantar los Trusses de Vanos Largos (vea BCS1-B1).



TIPS FOR SAFE AND EFFICIENT INSTALLATIONS

CONSEJOS PARA INSTALACIONES SEGURAS Y EFICIENTES

- Assemble the first five trusses with all Structural Sheathing, restraint and bracing, either as the trusses are installed or built as a unit on the ground and lifted into place. Monte los primeros cinco trusses con todo el Entablado Estructural, restricción y arriostre, cualquier cuando los trusses son instalados o son contruidos como una unidad en la tierra y levantados en lugar.
- Keep trusses straight during hoisting to minimize bending out-of-plane. Mantenga los trusses derechos durante el levantamiento para mini-mizar el doblado fuera-de-plano.
- Add a temporary center support to provide greater stability and leave in place until all Permanent Building Stability Bracing is in-stalled. Agregue un soporte temporal en el centro para proveer más es-tabilidad y quedar en lugar hasta que todo el Arriostre de Estabilidad Permanente de Construcción es instalado.



- Apply Structural Sheathing to the top chord as the trusses are being installed. Aplique el Entablado Estructural a la cuerda superior mientras los trusses están instalados.
- Install all Permanent Building Stability Bracing immediately. Instale todo el Arriostre de Estabilidad Permanente de Construcción inmediatamente.

FIELD ASSEMBLY AND OTHER SPECIAL CONDITIONS
MONTAJE DE CAMPO Y OTROS CONDICIONES ESPECIALES

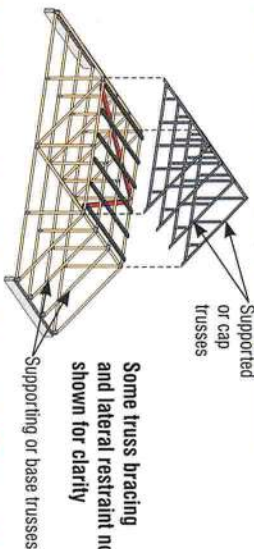
Certain sizes or shapes of wood trusses require some assembly at the jobsite. For these trusses, refer to the Truss Design Drawings for specific instructions on assembly methods, unless the Construction Documents indicate otherwise.

Ciertos tamaños o formas de trusses de madera requieren algún montaje en la obra. Para estos trusses, refiera a los Dibujos del Diseño de Truss para instrucciones específicas sobre métodos de montaje, a menos que los Documentos de la Construcción indiquen de lo contrario.

The Contractor is responsible for proper field assembly.
El Contratista es responsable para el montaje de campo apropiado.

PIGGYBACK TRUSSES
TRUSSES DE PIGGYBACK

- ✓ The supporting trusses shall be completely installed with all Permanent Building Stability Bracing as required BEFORE installing the supported trusses.
Los trusses soportantes serán completamente instalados con todo el Arriostre de Estabilidad Permanente de Construcción como se requiere ANTES de instalar los trusses soportados.

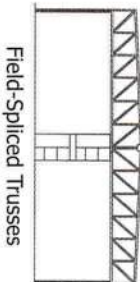


WARNING! Adequately restrain and brace the flat portion of the supporting trusses to prevent buckling.

- ✓ For more information on how to permanently laterally restrain and diagonally brace piggyback truss systems refer to BCSI-B3. *** Para más información sobre como restringir lateralmente permanentemente y arriostrear diagonalmente sistemas de trusses de piggyback refiera al BCSI-B3. ***

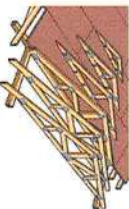
FIELD SPICED TRUSSES
TRUSSES EMPALMADOS EN CAMPO

- ✓ Splicing can be performed on the ground before installation or the truss sections can be supported by temporary shoring after being hoisted into place and the splices installed from a safe working surface.
Empalmado puede ser hecho en la tierra antes de la instalación o las secciones de trusses pueden ser soportados por "shoring" temporal después de que sean levantados en lugar y los empalmes son instalados de una superficie de trabajo segura.

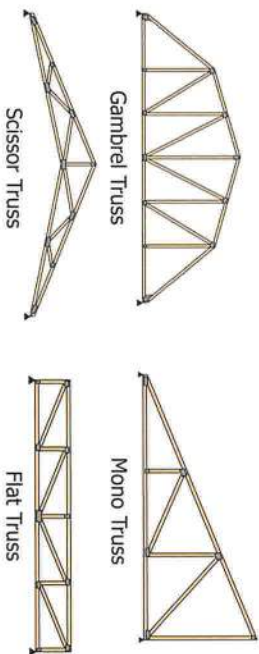


VALLEY SET FRAME INSTALLATION
INSTALACIÓN DE ARMAZÓN DE CONJUNTO DE VALLE

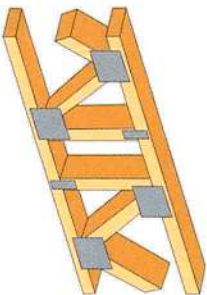
- ✓ The top chords of the supporting trusses must be braced properly. This can be accomplished by attaching Structural Sheathing or with rows of Lateral Restraint, spaced no more than the maximum on center spacing specified on the Truss Design Drawing, and Diagonal Bracing.
Las cuerdas superiores de los trusses de soporte tienen que ser arriostrados apropiadamente. Esto puede ser hecho por sujetar el Entablado Estructural o con filas de Restricción Lateral, espaciados no más que el máximo en el espaciamiento del centro que es especificado en el Dibujo del Diseño de Truss, y Arriostre Diagonal.



This document applies to all 2x_ oriented sloped and flat chord trusses such as:
Este documento aplica a todos trusses de cuerda pendiente y planas de 2x orientación tales como:



For parallel chord trusses manufactured with 3x2 or 4x2 lumber, see BCSI-B7 Summary Sheet – Temporary & Permanent Restraint/Bracing for Parallel Chord Trusses. ***
Para trusses de cuerdas paralelas fabricados con madera 3x2 o 4x2, Resumen BCSI-B7 – Restricción/Arriostre Temporal y Permanente Para Trusses de Cuerdas Paralelas. ***



For trusses spaced more than 2'-0" on center, see BCSI-B10 Summary Sheet – Post Frame Truss Installation & Temporary Restraint/Bracing. ***
Para trusses espaciados más de 2 pies en el centro, vea el Resumen BCSI-B10 – Instalación y Restricción/Arriostre Temporal de Armazón de Poste. ***

***Contact the component manufacturer for more information or consult a Professional Engineer for assistance.

This document summarizes the information provided in Section B2 of the 2006 Edition of Building Component Safety Information BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses. Copyright © 2004 - 2006 WYCA – Representing the Structural Building Components Industry and Truss Plate Institute. All Rights Reserved. This guide or any part thereof may not be reproduced in any form without the written permission of the publishers. Printed in the United States of America.



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B2 Truss Installation & Temporary Restraint/Bracing
Instalación de Trusses & Restricción/Arriostre Temporal

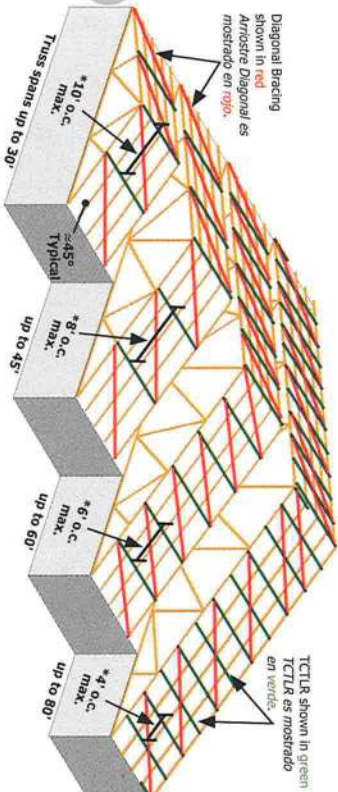
FOR TRUSSES UP TO 2'-0" ON-CENTER AND 80'-0" IN LENGTH
PARA TRUSSES HASTA 2 PIES EN CENTRO Y HASTA 80 PIES DE LONGITUD

- ⚠ Spans over 60' require more complex Temporary Installation Restraint Bracing. Consult a Professional Engineer.
Vanos más de 60 pies requieren Instalación Temporal de Restricción/Arriostre más complejo. Consulte un Ingeniero Profesional.

- ⚠ Disregarding handling, installing, restraining and bracing safety recommendations is the major cause of truss erection/installation accidents. El no seguir las recomendaciones de manejo, instalación, restricción y arriostre es la causa principal de los accidentes durante la erección/instalación de trusses.

- ⚠ Lateral Restraint is not adequate without Diagonal Bracing. Restricción Lateral no es adecuada sin Arriostre Diagonal. Always diagonally brace for safety! ¡Siempre arriostre diagonalmente para seguridad!

MAXIMUM SPACING FOR TOP CHORD TEMPORARY LATERAL RESTRAINT (TCTLR)
ESPACIAMIENTO MÁXIMO PARA ARRIOSTRE LATERAL TEMPORAL DE LA CUERDA SUPERIOR (TCTLR)

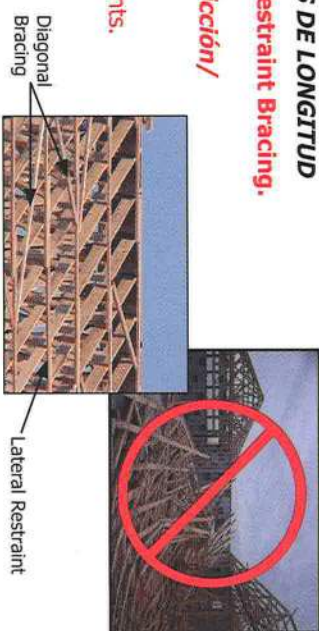


The graphic at left shows the maximum on-center spacing (see * at left) of TCTLR based on truss span from the table in Step 2 on page 2.

- Ground bracing not shown for clarity.
- Apply Diagonal Bracing or Structural Sheathing immediately. For spans over 60' applying Structural Sheathing immediately is the preferred method.

El dibujo a la izquierda muestra el espaciamiento máximo en el centro (vea * a la izquierda) del TCTLR basado en los vanos de trusses de la tabla en el Paso 2 en la página 2.

- No se muestra el arriostre de tierra para claridad.
- Aplique inmediatamente Arriostre Diagonal o Entablado Estructural (structural sheathing). Para vanos más de 60 pies el método preferido es entablarlo inmediatamente.



CHECK THESE ITEMS BEFORE STARTING ERECTION/INSTALLATION AND CORRECT AS NEEDED
REVISE ESTOS PUNTOS ANTES DE EMPEZAR LA ERECCIÓN/INSTALACIÓN Y CORRIJA COMO ES NECESARIO

- ✓ Building dimensions match the Construction Documents.
Dimensiones del edificio concuerdan con Planos de Construcción.
- ✓ Bearing Supports are accurately and securely installed.
Soportes de Soporter son instalados correctamente y seguramente.
- ✓ Load bearing supports (e.g., walls, columns, headers, beams, etc.) are plumb and properly braced.
Soportes que sostener cargas (Ej.: paredes, columnas, vigas de cabeza, vigas, etcétera) son plomadas y correctamente arriostradas.
- ✓ Hangers, tie-downs, restraint and bracing materials are on site and accessible.
Colgadores (hangers), soportes de anclaje (tie-downs) y materiales de restricción y arriostre están accesibles en la obra.
- ✓ Erection/Installation crew is aware of installation plan and Lateral Restraint/Diagonal Bracing requirements.
La tripulación de erección/instalación es consciente del plan de instalación y requisitos de restricción/arriostre.
- ✓ Multi-ply trusses, including girders, are correctly fastened together prior to lifting into place.
Trusses de varias capas, incluyendo girders, are correctly fastened together prior to lifting into place.
- ✓ Trusses de varias capas, incluyendo travesaños, son fijados juntos correctamente antes de levantarlos en lugar.

- ✓ Any truss damage is reported to Truss Manufacturer. Refer to BCSI-B5 Summary Sheet – Truss Damage, Jobsite Modifications and Installation Errors. *** Do not install damaged trusses unless instructed to do so by the Building Designer, Truss Designer or Truss Manufacturer.
Algun daño a los trusses ha sido reportado al Fabricante de Trusses. Vea el resumen BCSI-B5 – Daño a los Trusses, Modificaciones en la Obra y Errores de Instalación. *** No instale trusses dañados a menos que se diferjen el Diseñador del Edificio, Diseñador del Truss o Fabricante del Truss.

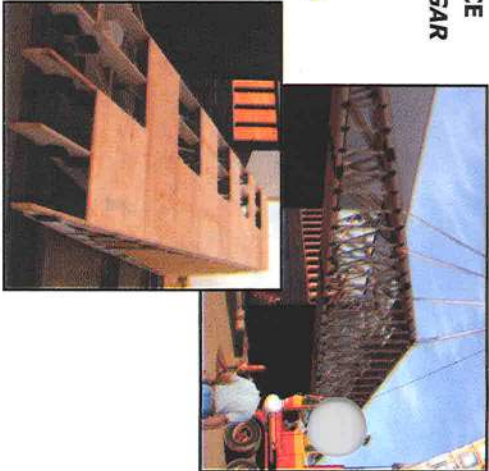
- ✓ Ground Bracing procedure for first truss is based on site and building configuration.
El procedimiento de Arriostre de Tierra para el primer truss es basado en el terreno y configuración del edificio.



If ground level is too far from truss for exterior Ground Bracing, use interior Ground Bracing. Si el nivel del terreno es demasiado lejos para usar Arriostre de Tierra exterior, use Arriostre de Tierra interior.

ALTERNATE INSTALLATION METHOD: BUILD IT ON THE GROUND AND LIFT IT INTO PLACE
MÉTODO ALTERNO DE INSTALACIÓN: CONSTRUIRLO EN LA TIERRA Y LEVANTARLO EN LUGAR

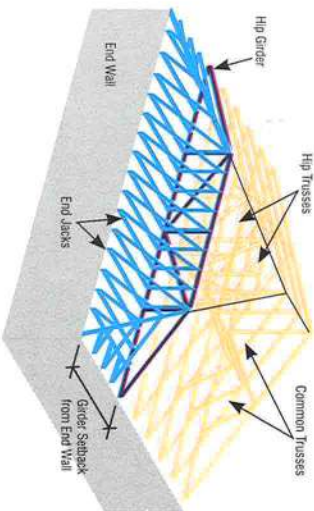
- Ensure level bearing, set, position, plumb and properly restrain and brace the set of trusses on the ground. Asegúrese que el soporte es nivelado; colocar, posicionar, nivelar y restringir y arriostrar apropiadamente el grupo de trusses en la tierra.
- Apply Structural Sheathing for stability. Begin at the heel and alternate 4'x8" and 4'x4" sheets up to the peak. Aplique el Entablado Estructural para estabilidad. Empezar en el talón y alternar 4 pies x 8 pies y 4 pies x 4 pies láminas hasta el pico.
- Install web and bottom chord Lateral Restraint and Diagonal Bracing as required by the Building Designer. requiere el Diseñador del Edificio.
- Pick up the assembly and set it in place. Levante el ensamble y colóquelo en lugar.
- Be sure to get the proper Professional Engineering guidance to lift the entire system into place safely and efficiently. Asegúrese de obtener la guía de Ingeniería Profesional apropiada para levantar el sistema entero a su lugar en forma segura y eficiente.



HIP SET ASSEMBLY & BRACING
ENSAMBLAJE Y ARRIOSTRE DEL GRUPO DE CADERA

Permanently connecting the end jacks to the end wall and girder as early in the installation process as possible dramatically increases the stability of the hip girder and the safety of the structure.

Conectando permanentemente los gatos extremos a la pared extremo y travesaños tan temprano en el proceso de instalación como sea posible aumenta drásticamente la estabilidad del travesaño de cadera (hip girder) oy la seguridad de la estructura.



Use the following sequence:
Use la siguiente secuencia:

Step 1 Position the hip girder on the bearing walls at the specified end wall setback and permanently attach to supports.

Paso 1 Posicione el travesaño de cadera en las paredes de soporte al "setback" de la pared extremo especificado y sujete a los soportes permanentemente.

WARNING! Whenever possible, connect multi-ply Girder Trusses together and install the end jack hangers (if required) prior to erection/installation.

¡ADVERTENCIA! Siempre que sea posible, conecte los trusses de travesaño de varias-capas juntos y instale los colgadores de gato extremo (si requerido) antes de la erección/instalación.

Note: All piles of a multi-ply girder shall be attached per the fastening schedule on the TDD before attaching any framing members or applying any loads.

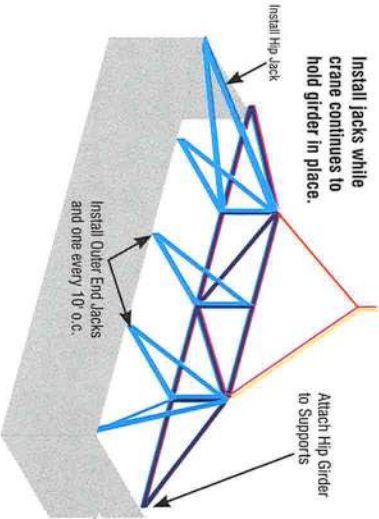
Nota: Todas capas de un travesaño de varias capas sujetarán por el horario de fijar en el Dibujo del Diseño de Truss antes de sujetar algunos miembros o de aplicar algunas cargas.

Step 2 Install Hip Jacks, and End Jacks at 10' on center while crane continues to hold girder in place.

Paso 2 Instale Gatos de Cadera y Gatos Extremos a 10 pies en centro mientras la grúa continúa tener el travesaño en lugar.

Note: Attach jack trusses to Top Chord and Bottom Chord of Girder Truss. The Connection at the top is important as it helps to stabilize the Top Chord of the girder.

Nota: Sujete trusses de gatos a la Cuerda Superior y la Cuerda Inferior del Truss de Travesaño. La conexión por encima es importante como ayuda a estabilizar la Cuerda Superior del travesaño.

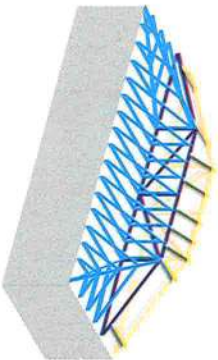


Step 3 Install all the corner and end jacks with all permanent truss-to-bearing connections (e.g., hangers and tie-downs).

Paso 3 Instale todos los gatos extremos y gatos de rincón con todos conexiones permanentes de truss-a-soporte (Ej. Colgadores soportes de anclaje.

Step 4 Install the next hip truss with 2x4 Short Member Temporary Lateral Restraints cut to a minimum of 32" long. Attach each Short Member Temporary Lateral Restraint to the Top Chord of the hip girder and adjacent hip truss with two nails at each end. Place Short Member Temporary Lateral Restraint at hip breaks and space according to the guidelines of this document.

Paso 4 Instale el próximo truss de cadera con 2x4 Restricciones Temporales Laterales para Miembros Cortos cortadas a 32 pulgadas de largo en mínimo. Sujete cada Restricción Lateral Temporal del Miembro Corto a la Cuerda Superior del travesaño de cadera y al truss de cadera contiguo con 2 clavos a cada extremo. Puse Restricción Lateral Temporal del Miembro Corto a roturas de cadera (hip breaks) y espacian según las directrices de este documento.

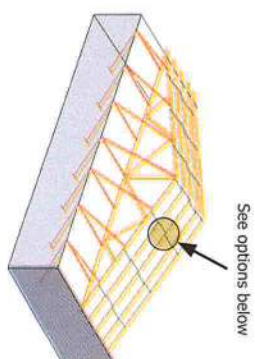
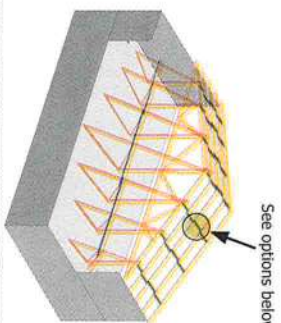
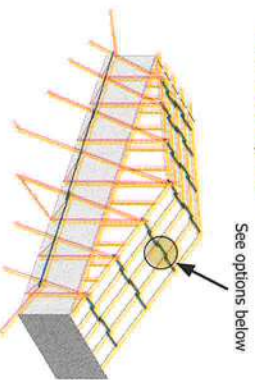


Step 5 Install remaining hip trusses. For the flat portion of each hip truss use Short Member Temporary Lateral Restraints that are at least double the length of the first set of Short Member Temporary Lateral Restraints (see Figure on next page). For the sloped sides of the trusses, install Short Member Temporary Lateral Restraints according to one of the three options on page 3.

Paso 5 Instale el resto de los trusses de cadera. Para la parte plana de cada truss de cadera use Restricciones Laterales Temporales de Miembros Cortos que son por lo menos dobles la longitud del primer grupo de Restricciones Temporales Laterales de Miembros Cortos (Vea la figura en la próxima página). Para los lados pendientes de los trusses, instale Restricciones Laterales Temporales de Miembros Cortos según uno de las tres opciones en la página 3.

4. Set Trusses 2, 3, 4 & 5 with TCTLR in Line with Ground Bracing
Coloque los Trusses 2, 3, 4 y 5 con TCTLR en Línea con los Arriostrés de Tierra

- Attach trusses securely at all bearings, shimming bearings as necessary. Examples of first five trusses set. Sujete seguramente los trusses a todos los soportes, rellenar sólidamente los soportes como sea necesario. Ejemplos de los primeros cinco trusses colocados.

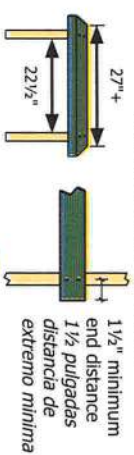


EXTERIOR GROUND BRACING ARRIOSTRÉ DE TIERRA EXTERIOR	INTERIOR GROUND BRACING ARRIOSTRÉ DE TIERRA INTERIOR	INTERIOR GROUND BRACING TO END WALL ARRIOSTRÉ DE TIERRA INTERIOR A LA PARED DE EXTREMO
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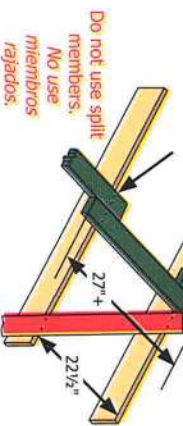
- Three options for installing Short Member Temporary Lateral Restraint. Tres opciones para instalar Restricción Lateral Temporal de los Miembros Cortos.

Option 1
Short Member Temporary Lateral Restraint Installed on Top of Trusses

Restricción Lateral Temporal de los Miembros Cortos
Instalados Encima de Trusses



2 nails at every connection
2 clavos en cada conexión

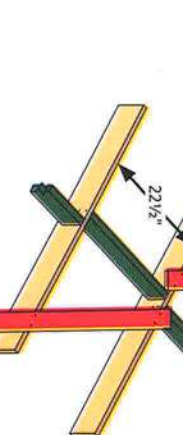


Option 2
Short Member Temporary Lateral Restraint Installed Between Trusses

Restricción Lateral Temporal de los Miembros Cortos
Instalados Entre Trusses

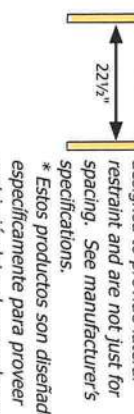


Do not use split members.
No use miembros rajados.

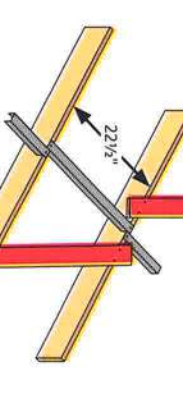


Option 3
Proprietary Metal Restraint Products*

Productos de Restricción de Metal Patentado*
* These products are specifically designed to provide lateral restraint and are not just for spacing. See manufacturer's specifications.



* Estos productos son diseñados específicamente para proveer restricción lateral y no solamente para espaciamiento. Vea las especificaciones del fabricante.



Each truss must be attached securely at each bearing and all Top Chord Temporary Lateral Restraints installed before removing the hoisting supports.
¡ADVERTENCIA IMPORTANTE DE SEGURIDAD! Superiores instaladas antes de remover los soportes de levantar.

LATERAL RESTRAINT/BRACING MATERIAL AND CONNECTIONS
CONEXIONES Y MATERIALES DE RESTRICCIÓN/ARRIOSTRÉ

- Minimum size of Lateral Restraint and bracing material is 2x4 stress-graded lumber or approved proprietary metal restraint/bracing, unless otherwise specified by the Building Designer. El tamaño del material de Restricción Lateral y arriostré debe ser por lo menos 2x4 madera graduada por esfuerzo o restricción/arriostré de metal patentado aprobado, a menos que especifique el Diseñador del Edificio.
- All bracing and Lateral Restraint members must be connected to each truss with at least 2 nails (see minimum sizes shown below), except for the short member restraints shown in Step 4, Option 2 (see above), which require 2-16d deformed-shank (i.e., ring- or screw-shank) nails. Todos los miembros de Restricción Lateral y arriostré tienen que ser conectados a cada truss con un mínimo de 2 clavos (ver los tamaños mostrados en el Paso 4, Opción 2 (vea arriba), cuales requieren 2-16d clavos mínimos mostrados abajo) excepto para las restricciones de miembros cortos con largos deformados (Ej. Largos de anillos o tornillos).
- Drive nails flush, or use double-headed nails for easiest removal. Penetre los clavos al ras, o use clavos de dos cabezas para quitarlos más fácilmente.

10d (0.128x3")
12d (0.128x3.25")
16d (0.131x3.5")

