



Alpine, an ITW Company 6750 Forum Drive, Suite 305 Orlando, FL 32821 Phone: (800)755-6001 www.alpineitw.com

Site Information:

Customer: W. B. Howland Company, Inc.

Job Number: 20-4183

Job Description: Woods In-Law Suite

Address:

Job Engineering Criteria:	
Design Code: FBC 2017 RES	IntelliVIEW Version: 19.02.02B
	JRef #: 1WUV2150002
Wind Standard: ASCE 7-10 Wind Speed (mph): 130	Roof Load (psf): 20.00-10.00- 0.00-10.00
Building Type: Closed	Floor Load (psf): None

This package contains general notes pages, 16 truss drawing(s) and 0 detail(s).

Item	Drawing Number	Truss
1	121.20.0802.27947	A01
3	121.20.0802.31773	A03
5	121.20.0802.35203	A05
7	121.20.0802.41323	J01
9	121.20.0802.43923	J02
11	121.20.0802.46733	J03
13	121.20.0802.49853	J04
15	121.20.0802.54777	J05

Item	Drawing Number	Truss
2	121.20.0802.30110	A02
4	121.20.0802.33597	A04
6	121.20.0802.38600	A06
8	121.20.0802.42683	J1A
10	121.20.0802.45203	J2A
12	121.20.0802.47730	J3A
14	121.20.0802.52327	J4A
16	121.20.0802.57413	J5A



General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

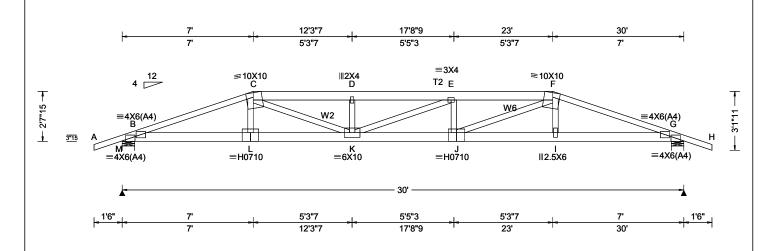
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 13723 Riverport Drive, Suite 200, Maryland Heights, MO 63043; www.alpineitw.com.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 590354 HIPS Ply: 1 Job Number: 20-4183 Cust: R 215 JRef: 1WUV2150002 T6 FROM: CDM DrwNo: 121.20.0802.27947 Qty: 1 Woods In-Law Suite Truss Label: A01 / YK 04/30/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	4
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	١.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.475 E 750 240	L
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.946 E 376 180	I
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.090 I	
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.180 I	١
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0	I
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 2017 RES	Max TC CSI: 0.725	12
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.691	5
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Varies by Ld Case	Max Web CSI: 0.665	"
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)		ľ
	GCpi: 0.18	Plate Type(s):] `
	Wind Duration: 1.60	WAVE, HS	VIEW Ver: 19.02.02B.0122.15	E
Lumban				- (

▲ Maximum Reactions (lbs)						
Gravity Non-Gravity						
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
М	2853	<i>I</i> -	/-	/-	/562	/-
G	2853	/-	/-	/-	/562	/-
Win	d reac	tions b	ased or	n MWFRS		
М	Brg V	Vidth =	8.0	Min Re	q = 2.4	ı
G	Brg V	Vidth =	8.0	Min Re	q = 2.4	ı
Bea	ırings l	И & G а	are a rig	gid surface.		
Mer	nbers	not list	ed have	forces les	s than 3	375#
Max	cimum	Top C	hord F	orces Per	Ply (lb	s)
Cho	ords T	ens.Co	omp.	Chords	Tens.	Comp.
B - (С	1525 -	7992	E-F	1935	- 10144
C - I				F-G	1524	- 7985
D -	F	1051_1	0212			

Maximum Bot Chord Forces Per Ply (lbs)

Chords

I - G

Webs

E - J

J-F

F - I

Tens. Comp.

7493 - 1426

7542 - 1428

Tens. Comp.

-733

- 554

- 38

243

2889

752

Chords Tens.Comp.

7549 - 1430

7499 - 1427

10238 - 1966

Tens.Comp.

763

2947 - 569

239 -711

B - L

Top chord: 2x4 SP M-31; T2 2x6 SP 2400f-2.0E; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3; W2,W6 2x4 SP #2;

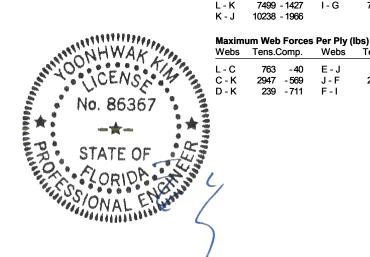
Special Loads

Dur.Fac.=1.	.25 / Plate D	our.Fac.=1.2	25)
61 plf at	-1.50 to	61 plf at	7.00
31 plf at	7.00 to	31 plf at	23.00
61 plf at	23.00 to	61 plf at	31.50
4 plf at	-1.50 to	4 plf at	0.00
20 plf at	0.00 to	20 plf at	7.03
10 plf at	7.03 to	10 plf at	22.97
20 plf at	22.97 to	20 plf at	30.00
4 plf at	30.00 to	4 plf at	31.50
Conc. Load	at 7.06, 9.	06,11.06,13	3.06
18.94,20.94,	22.94		
Conc. Load	at 7.03,22	.97	
Conc. Load	at 9.06,11	.06,13.06,1	5.00
20.94			
	61 plf at 31 plf at 61 plf at 4 plf at 20 plf at 10 plf at 20 plf at 4 plf at Conc. Load 18.94,20.94, Conc. Load	61 plf at -1.50 to 31 plf at 7.00 to 61 plf at 23.00 to 4 plf at -1.50 to 20 plf at 0.00 to 10 plf at 30.00 to 20 plf at 22.97 to 4 plf at 30.00 to Conc. Load at 7.06, 9. 18.94,20.94,22.94 Conc. Load at 9.06,11	31 plf at 7.00 to 31 plf at 61 plf at 23.00 to 61 plf at 4 plf at -1.50 to 4 plf at 20 plf at 0.00 to 20 plf at 10 plf at 7.03 to 10 plf at 20 plf at 22.97 to 20 plf at 4 plf at 30.00 to 4 plf at 4 plf at 30.00 to 4 plf at Conc. Load at 7.06, 9.06,11.06,13 (8.94,20.94,22.94) Conc. Load at 7.03,22.97 Conc. Load at 9.06,11.06,13.06,1

Wind loads and reactions based on MWFRS. Uplifts based on an elevation at or above 1000 ft.

Additional Notes

Refer to General Notes for additional information The overall height of this truss excluding overhang is



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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

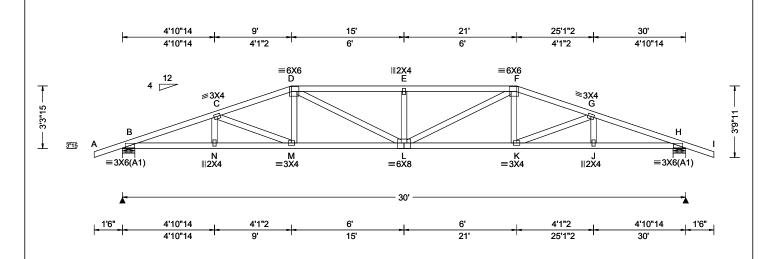
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.



For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

SEQN: 590360 HIPS Ply: 1 Job Number: 20-4183 Cust: R 215 JRef: 1WUV2150002 T2 FROM: CDM Qty: 2 DrwNo: 121.20.0802.30110 Woods In-Law Suite Truss Label: A02 / YK 04/30/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	4
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	١.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.242 E 999 240	L
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.482 E 739 180	l
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.064 J	ŀ
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.128 J	١
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0	E
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 2017 RES	Max TC CSI: 0.441	!
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.758	5
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.211	1:
'	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		ľ
	GCpi: 0.18	Plate Type(s):		_ -՝
	Wind Duration: 1.60	WAVE	VIEW Ver: 19.02.02B.0122.15	E
Lumber	•	•		۱ -

▲ Ma	▲ Maximum Reactions (lbs)					
	Gravity Non-Gravity					
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
В	1314	/-	/-	/756	/251	/90
Н	1314	/-	/-	/756	/251	/-
Win	d read	ctions ba	sed or	MWFRS		
В	Brg V	Vidth = 8	3.0	Min Re	eq = 1.6	;
Н	Brg V	Vidth = 8	3.0	Min Re	q = 1.6	;
Bea	rings I	B & H aı	e a rig	id surface.	•	
Men	nbers	not liste	d have	forces les	s than 3	375#
Max	imun	Top C	hord F	orces Per	Ply (lb	s)
Cho	rds 1	ens.Co	mp.	Chords	Tens.	Ćomp.
В-0	5	793 - 3	8013	E-F	834	- 2991
_ C - ն)	734 - 2	2662	F-G	734	- 2662
D - E	Ξ	834 - 2	2991	G-H	792	- 3013

Maximum Bot Chord Forces Per Ply (lbs)

-682

- 684

- 583

Chords

K-J

J - H

Webs

Tens. Comp.

Tens. Comp.

-602

-702

- 701

- 165

2495

2812

2814

555

Chords Tens.Comp.

2814

2812

2495

Tens.Comp.

555 - 165

158 - 386

B - N

N - M

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

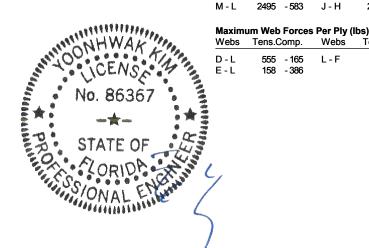
Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

Refer to General Notes for additional information The overall height of this truss excluding overhang is



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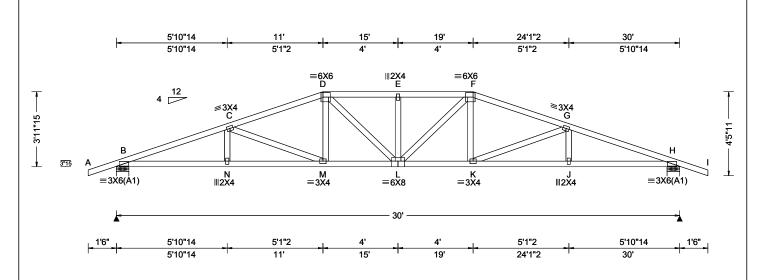
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SEQN: 590363 HIPS Ply: 1 Job Number: 20-4183 Cust: R 215 JRef: 1WUV2150002 T3 FROM: CDM DrwNo: 121.20.0802.31773 Qty: 2 Woods In-Law Suite Truss Label: A03 / YK 04/30/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	4
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	١.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.205 E 999 240	L
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.408 E 873 180	E
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.063 J	H
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.126 J	١
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0	E
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 2017 RES	Max TC CSI: 0.398	Ľ
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.752	ľ
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.348	Ľ
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		ľ
	GCpi: 0.18	Plate Type(s):		Į -`
	Wind Duration: 1.60	WAVE	VIEW Ver: 19.02.02B.0122.15	Į
Lumber				- (

▲ Maximum Reactions (lbs)						
	Gravity Non-Gravity					
Loc R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
B 1314	1 /-	/-	/761	/250	/104	
H 1314	1 /-	/-	/761	/250	/-	
Wind rea	actions b	ased on	MWFRS			
B Brg	Width =	8.0	Min Re	q = 1.6	;	
H Brg	Width =	8.0	Min Re	q = 1.6	;	
Bearings	в В & На	are a rigio	d surface.	-		
Member	s not list	ed have	forces les	s than 3	375#	
Maximu	m Top C	hord Fo	orces Per	Ply (lb	s)	
Chords	Tens.Co	omp.	Chords	Tens.	Ćomp.	
B-C	763 -	2996	E-F	687	- 2409	
C-Ď		2426	F-G	666	- 2426	
D-E	687 -	2409	G-H	763	- 2996	

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

Refer to General Notes for additional information The overall height of this truss excluding overhang is



Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp.

Maximum Bot Chord Forces Per Ply (lbs)

-649

Chords

K-J

J - H

K-G

Tens. Comp.

- 520

-669

-668

- 577

2255

2790

2794

161

Chords Tens.Comp.

2794

2790 - 650

2255 - 501

> 161 - 577

B - N

N - M

M - L

C - M

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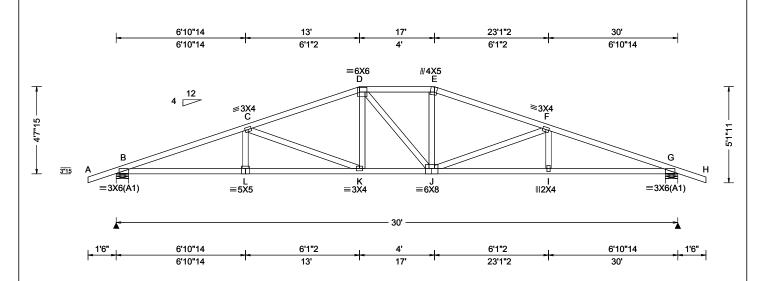
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SEQN: 590366 HIPS Ply: 1 Job Number: 20-4183 Cust: R 215 JRef: 1WUV2150002 T4 FROM: CDM DrwNo: 121.20.0802.33597 Qty: 2 Woods In-Law Suite Truss Label: A04 / YK 04/30/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.191 K 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.381 K 936 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.063 I
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.126 I
NCBCLL: 10.00	Mean Height: 15.00 ft	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf BCDL: 5.0 psf	Bldg Code: FBC 2017 RES	Max TC CSI: 0.531
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.745
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.658
- -	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 19.02.02B.0122.15

▲ M	▲ Maximum Reactions (lbs)					
	Gravity Non-Gravity					
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
В	1314	<i>I</i> -	/-	/763	/249	/119
G	1314	/-	/-	/763	/249	/-
Win	d rea	ctions b	ased or	MWFRS		
В	Brg \	Vidth =	8.0	Min Re	q = 1.6	i
G	Brg \	Vidth =	8.0	Min Re	eq = 1.6	i
Bea	rings	B&Ga	are a rig	id surface.	•	
Men	nbers	not list	ed have	forces les	s than 3	375#
Мах	imur	n Top (hord F	orces Per	Ply (lb	s)
Cho	rds '	Tens.Co	omp.	Chords	Tens.	Ćomp.
B - 0	c	726 -	2953	E-F	596	- 2187
c-i ا	-	-	2198		725	- 2954
D - I	E	597 -	2028			

Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

Refer to General Notes for additional information The overall height of this truss excluding overhang is



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.		
B-L	2747	- 608	J - I	2742	- 627	
L-K	2742	- 609	I-G	2748	- 626	
K - J	2026	- 420				

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.		Webs	Tens. Comp.	
C-K D-K		- 769 - 60	J-E J-F	406 205	- 71 - 779
D - K	407	-00	J - I	200	-119

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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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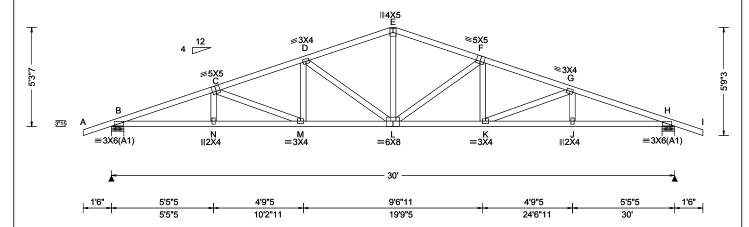
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SEQN: 590369 HIPS Ply: 1 Job Number: 20-4183 Cust: R 215 JRef: 1WUV2150002 T1 FROM: CDM DrwNo: 121.20.0802.35203 Qty: 1 Woods In-Law Suite Truss Label: A05 / YK 04/30/2020





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.196 L 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.390 L 913 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.064 J
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.127 J
NCBCLL: 10.00	Mean Height: 15.00 ft	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf BCDL: 5.0 psf	Bldg Code: FBC 2017 RES	Max TC CSI: 0.349
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.757
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.449
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 19.02.02B.0122.15

▲ Maximum Reactions (lbs)							
	Gravity	-	Non-Gravity				
Loc R+	/ R-	/Rh	/ Rw	/ U	/ RL		
B 1314	/-	/-	/763	/83	/133		
H 1314	/-	/-	/763	/83	/-		
Wind rea	ctions bas	ed on I	MWFRS				
B Brg V	Vidth = 8.	0	Min Re	q = 1.6			
H Brg V	H Brg Width = 8.0 Min Reg = 1.6						
Bearings	B & H are	a rigid	surface.	-			
Members	not listed	have f	orces les	s than 3	375#		
Maximum Top Chord Forces Per Ply (lbs)							
Chords	Tens.Com	p.	Chords	Tens.	Ćomp.		
В-С	655 - 30	11	E-F	495	- 1930		
」C-D	591 - 25	28	F-G	590	- 2527		
D-E	495 - 19	30	G - H	656	- 3012		

Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

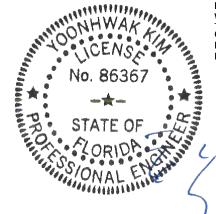
Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

Refer to General Notes for additional information The overall height of this truss excluding overhang is



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.		_
B - N	2810	- 549	L-K	2335	- 458	
N - M	2806	- 550	K-J	2808	- 569	
M - L	2336	- 44 0	J - H	2812	- 568	

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.		Webs	Tens. (Comp.
C - M	117	- 486	L-F	188	-682
D-L	189	- 684	K-G	118	- 490
F-I	912	- 189			

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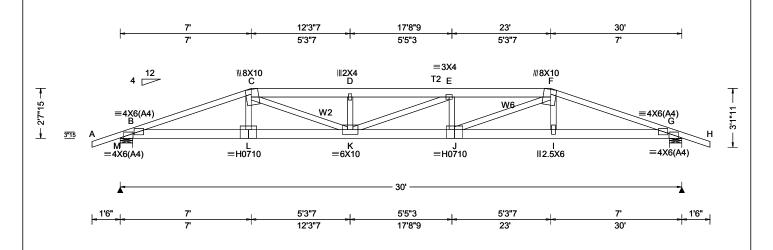
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6750 Forum Drive Suite 305 Orlando FL, 32821 SEQN: 590357 HIPS Ply: 1 Job Number: 20-4183 Cust: R 215 JRef: 1WUV2150002 T14 FROM: CDM DrwNo: 121.20.0802.38600 Qty: 1 Woods In-Law Suite Truss Label: A06 / YK 04/30/2020



▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw / U /RL 2782 /-/-/544 /-2782 /-/544 /-/-Wind reactions based on MWFRS Brg Width = 8.0 Min Reg = 2.3М Brg Width = 8.0 Min Reg = 2.3Bearings M & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 1467 - 7765 1852 - 9866 C-D 1868 - 9935 F-G 1465 - 7759 D-E 1868 - 9932

Top chord: 2x4 SP M-31; T2 2x6 SP 2400f-2.0E; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3; W2,W6 2x4 SP #2;

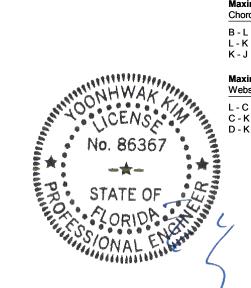
Special Loads

-(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) 61 plf at 31 plf at 61 plf at TC: From -1.50 to 7.00 to 61 plf at 31 plf at TC: From 23.00 TC: From 61 plf at 23.00 to -1.50 to BC: From 4 plf at 4 plf at 0.00 BC: From 20 plf at 0.00 to 20 plf at 7.03 10 plf at 20 plf at 4 plf at BC: From BC: From 7.03 to 10 plf at 22.97 22.97 to 20 plf at 30.00 From 4 plf at 30.00 to 4 plf at 31 182 lb Conc. Load at 7.06, 9.06,11.06,13.06 BC: From 31.50 15.00,16.94,18.94,20.94,22.94 BC: 530 lb Conc. Load at 7.03,22.97 BC: 126 lb Conc. Load at 9.06,11.06,13.06,15.00 16.94,18.94,20.94

Wind loads and reactions based on MWFRS. Uplifts based on an elevation at or above 1000 ft.

Additional Notes

Refer to General Notes for additional information The overall height of this truss excluding overhang is



B - L 7333 - 1374 7280 - 1370 7286 - 1372 1 - G 7327 - 1373 L-K K-J 9958 - 1883 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. L - C E - J 237 -713 730

J-F

F - I

Chords

Tens. Comp.

2819

719

- 525

-34

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp.

2875 - 539

234 - 691

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