ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1T48487-Z0722165019

Truss Fabricator: Anderson Truss Company

Job Identification: 7-032--GARY JOHNSON BLAKLEY RES. -- , **

Truss Count: 5

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Version 7.24.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB-A11015EE-GBLLETIN-

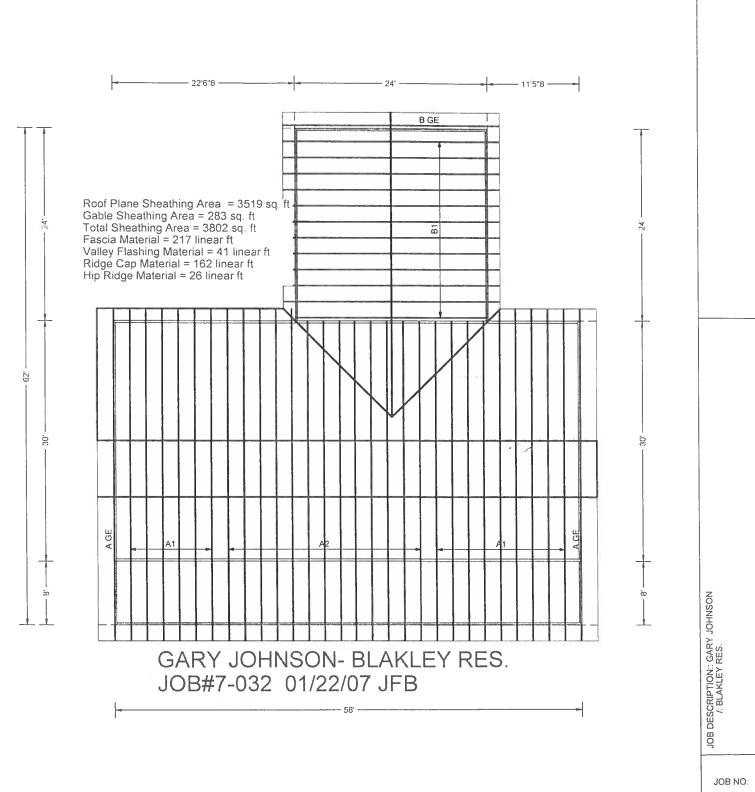
#	Ref Description	Drawing#	Date
1	33440 A1	07022206	01/22/07
2	33441 A2	07022207	01/22/07
3	33442A GE	07022210	01/22/07
4	33443 B1	07022208	01/22/07
5	33444 B GE	07022209	01/22/07

Seal Date: 01/22/2007

-Truss Design Engineer-Arthur R. Fisher Florida License Number: 59687 1950 Marley Drive Haines City, FL 33844

FILE CUPY





7-032

PAGE NO: 1 OF 1

Top chord 2x4 SP # Bot chord 2x4 SP # Webs 2x4 SP # PLT In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. Wind reactions based on MWFRS pressures ITW Building Components Group, Inc.
Haines City, FL 33844
Fr Certificate CAuthorization (7 032 GARY JOHNSON TYP. 0 - 4 - 3ALPINE Wave 2-0-0 $2.5 \times 6 (A1) =$ #2 Dense #2 Dense #3 R=246 U=180 W=3.5" BLAKLEY RES. **WARNING** TRUSSES REQUIRE EXTREME CARE IN FARRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST. (BUILDING COMPONENT SAFEY INFORMATION), PUBLISHED BY FIT (RBUSS PLAIF INSTITUTE, 2718 HORAT HE STREET, SUITE 137. ALEXANDRA, VA, 22131) AND WICA (MODO TRUSS COUNCEL O MERICA, 6300 CHIERDRA SELVEN, HADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE UNICTIONS. UNLESS OTHERNISE LINE, HADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE UNICTIONS. UNLESS OTHERNISE LINEALED DE GROOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. **IMPORTANT**TURNISH A CORY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BUILDING COMPONENTS GRADUP. THE SYSTAM, BOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRISS. IN CONTIDENANCE WITH PTI OR FARRICATION, HANDING, SHIPPING, INSTALLING A BRACTING OF TRISSES.

IN CONTIDENANCE WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY AFRA) AND TRI.

DESIGN CONTIDENS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY AFRA) AND TRI.

CONNECTOR TYLES, ARE MADE OF ZO/18176AG, MULISSY, ASTH ASS, DEADE 40/60 (M. X/H.SS) SALV. SIEL, APPLY

PHATES TO FACH FACE OF TRUSS AND. UNIESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, Z

PHATES TO FACH FACE OF TRUSS AND. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF IPI1 2002 SEC.3
BRANING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY 8-1-12 1.5X4 * 3×5≡ 4×5= Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 6-0-0 A1) R=1812 U=180 W=3.5" 3×5≡ 4X5≡ AHY ည် 8 3 X 4 ≡ 5×6# 0-0-SOLELY FOR THE TRUSS COMPONENT 0ver RESPONSIBILITY OF 7 - 0 - 0 ယ Supports 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (A) Continuous lateral bracing equally spaced on 3 X 6 ≡ 5×5= 4 X 8 ≡ 8 3×5// 1.5X4 III 3 X 4 **//** 15-0-0 * BC LL ВС TC DL DUR.FAC. TC LL TOT.LD. FL/-/4/-7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC DL=5.0 2 6 R=1273 U=180 W=3.5" $2.5 \times 6 (A1) =$ /-/R/-40.0 20.0 PSF 1.25 10.0 PSF 10.0 PSF 0.0 PSF member V 6.0 SEQN-DATE REF FROM HC-ENG DRW HCUSR487 07022206 Scale =.1875"/Ft. R487--JB/AF 18651 01/22/07 33440

Certifi----

Author

SPACING

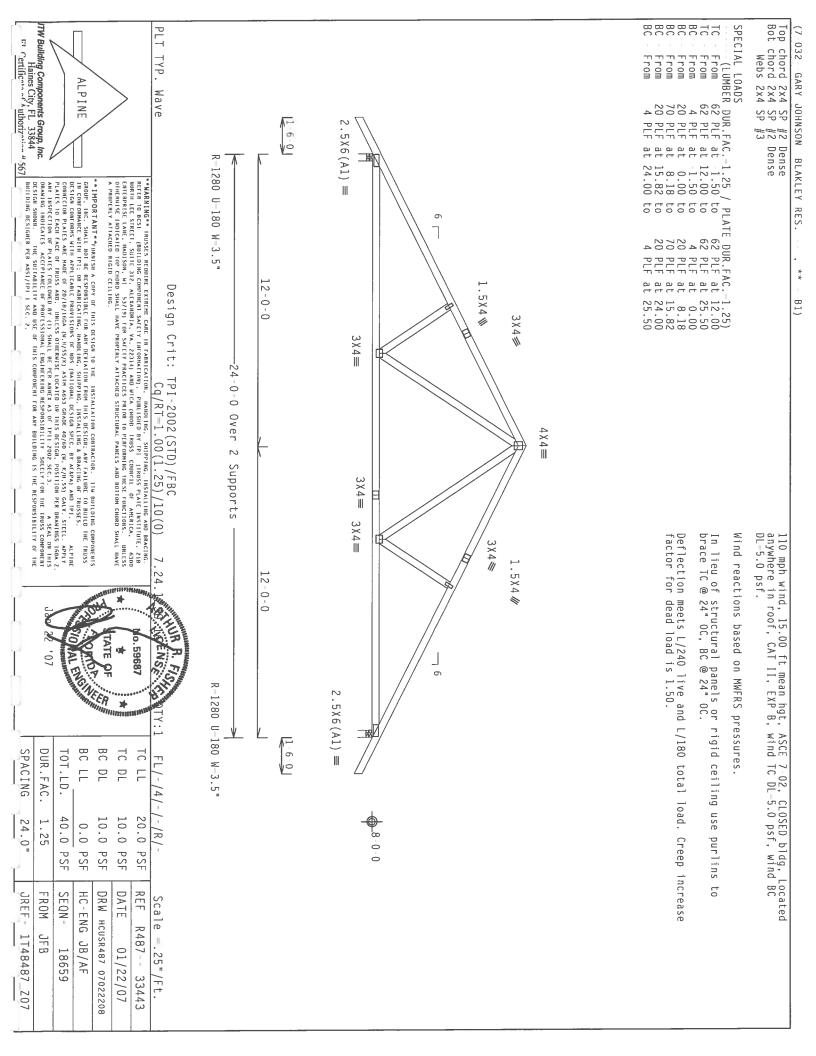
24.0"

JREF - 1T48487_Z07

Haines City, FL 33844 Shim all supports to solid bearing In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. Wind reactions based on MWFRS pressures Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 (7-032--GARY JOHNSON TYP. -4-3 ALPINE Wave 2-0- $2.5 \times 6 (A1) =$ Ó - 4 567 BLAKLEY RES. 118 **IMPORTANT**FIRMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE RUSS IN CONTROLLED THIS PROPERTY OF THE **MARNING** RUSSES REQUER CYREME CARE IN FARRICATION, FAMOLISME, SHIPPING, HISTAILLING AND BRACHIG, REFIRE TO BEST (BUILDING COMPONENT SAFETY INFORMATION), POULISMED BY TPI (TRUSS PLATE INSTITUTE, 21B MORTH LEE SIREE, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (400D TRUSS COUNCIL OF AMERICA, 6300 ERHERPAISE LAME, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PREFORMING HIGSE FUNCTIONS. DRILES OTHERWISE HOLDERS AND SOME HAVE HAVE PROPERLY ATTACHED STRUCTURAL FOR TO PROPERLY ATTACHED STRUCTURAL FARETS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL FARETS. U=180 W=3.5" φ 1.5X4≢ 3-8 12 PER ANSI/IPI 1 SEC. 2-* 3 X 5 ≡ 5 X 8 = Design Crit: 4×6≡ A2) R=2294 U=193 W=3.5" 4 X 6 ≡ 4 X 5 ≡ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 38 14-8 4 X 5 ≡ 5×6€ 0-0 ά 0ver W Supports 110 mph wind, 15.00 ft mean hgt, located within 4.50 ft from roof DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Calculated horizontal deflection is 0.11" 0.17" due to dead load. 5 X 5 == 5 X 8 ≡ Jan 22 '07 SCENSE No. 59687 3X5// 1.5X4 III 3 X 4 // 5 * Ö Ö ASCE 7 TC DL TC LL SPACING DUR.FAC. ВС ВС TOT.LD. -02. CAT 민 14/due to live load and CLOSED II, EXP R=1173 U=180 W=3.5" 3X6(A1) ≡ 40.0 20.0 24.0" 10.0 PSF 1.25 10.0 PSF 0.0 PSF bldg, not B, wind TC 160 PSF DATE REF JREF -FROM SEQN-DRW HCUSR487 07022207 HC-ENG Scale R487-JF8 1T48487_Z07 =.1875"/Ft JB/AF 18655 01/22/07 33441

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 ITW Building Components Group, Inc.
Haines City, FL 33844

"' Certific" ' uthoriz" " 567 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. See DWGS Al1015EE1106 & GBLLETIN1106 for more requirements. Wind reactions based on MWFRS pressures Note: All Plates PLT TYP. (7-032--GARY JOHNSON 0 - - - - - 3 ALPINE Wave 2-0-0 $2.5 \times 6 (A1) =$ Are 1.5X4 Except As Shown. R = 105BLAKLEY RES. @-QO-9 3X7≡ 3 X 4 ≡ **IMPORTANT**FURNISH A COPY OF THIS DESIGN 10 THE INSTALLATION CONTRACTOR. ITH BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM HIS DESIGN ANY FAILURE TO BUILD THE INSESSIBLE CONTROLLED AND THE PROPERTY OF T **HARNING** HOUSES REQUIRE LYBEM CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PRUBLISHED BY THE (RUSS PLATE INSTITUTE, 218 HORTH LEE STREET, SHITE 312, ALEXANDRIA, VA, 22314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 CHIEBPINS LAHE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOUSEAUTH OF HORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TRUSCHED SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PLF NOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR DESIGNER PER ANSI/IPI 1 SEC. 2. ω U-50 PLF 1 W=8-0-0 Design Crit: 14-2-5 R = 116GE) 3×5≡ -22 PLF U=21 10 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 4 PLF W=29-38 3X6≢ 0 ά 0 8 0ver 6 ω Supports Ň Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. factor for dead load is 1.50. 3 X 6≡ 5 X 5 == STATE/OF NAL ENGRIER GENS lo. 59687 4 3×5/ * BC LL BC DL TC DL DUR.FAC. SPACING TOT.LD. FL/-/4/-6 3 X 4 ₩ 2.5X6(A1) =273 U=223 W=3.5" 1-6-0 /-/R/ 40.0 10.0 20.0 3 X 4 // 24.0" 10.0 PSF 1.25 0.0 Creep increase PSF PSF PSF DATE REF JREF FROM SEQN-DRW HCUSR487 07022210 HC-ENG Scale = .1875"/Ft. R487--1748487_207 JB/AF 01/22/07 18673 33442



Top Bot In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. ITW Building Components Group, Inc. Haines City, FL 33844 F1 Certificate of A uthorization # 567 See DWGS Al1015EE1106 & GBLLETIN1106 for more requirements. Wind reactions based on MWFRS pressures Note: All Plates Are 1.5X4 Except As Shown. PLT TYP. (7-032--GARY JOHNSON BLAKLEY RES. chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 ALPINE Wave R=120 PLF U=11 $2X4(A1) \equiv$ 3X4// 1D-6-**IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLIATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN ANY FAILURE TO BUILD HIE HUSS LIN CONTROLLED WITH HIS PRESENCE AND THIS CHARLED FOR THE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY AFAVA) AND IPL. ALPHE CONNECTOR PLATES ARE HADE OF FOUNDESTORS OF HOS (MATIONAL DESIGN SPEC, BY AFAVA) AND IPL. ALPHE CONNECTOR PLATES ARE HADE OF FOUNDESTORS OF HIS SOCIAL BRACING OF HERSEL APPLY PLATES TO EACH FACE OF TRUSS AND. HIS SOCIAL FOR HIS DESIGN, POSITION FER DRAHLIGS TO ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AND THIS DOSTORS SEC.3. A SEAL ON THIS DRAHLIG HOS CONTROLLED BY (I) SHALL BE FER ANNEX AND THIS DOSTORS SEC.3. A SEAL ON THIS DRAHLIG HOS AND THIS DOSTORS SEC.3. A SEAL ON THIS DRAHLIG HOS FOLLOWED BY (I) SHALL BE FER ANNEX AND THIS DOSTOR TO BE HE THAN SECONDARY HIS DRAHLIG HOS AND THIS SOCIAL FOR THE THE SECONDARY HIS DRAHLIG HOS AND THE SULTABLILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE A PROPERLY ATTACHED RIGID CEILING. PLF W=24-0-3X4# 6 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 3X4 / В GE) 24-0-0 Over Continuous Support $3 \times 4 =$ 5 X 4 ≡ 3 X 4 ≡ Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chamust not be cut or notched. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. factor for dead load is 1.50. 3 X 4 ₩ Jan 15968 ____6 3×4*∰* ENGINEER -6 G 6 10 $2X4(A1) \equiv$ * 3X4₩ TC DL ВС ВС TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/ DL 40.0 10.0 20.0 10.0 PSF 1.25 0.0 Creep increase Ö PSF PSF PSF PSF Top chord SEQN-REF FROM DATE DRW HCUSR487 07022209 HC-ENG Scale = .25"/Ft. R487--JB/AF 18666 01/22/07 33444

BUILDING DESIGNER PER

SPACING

24.0"

JREF-

1T48487_Z07

BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

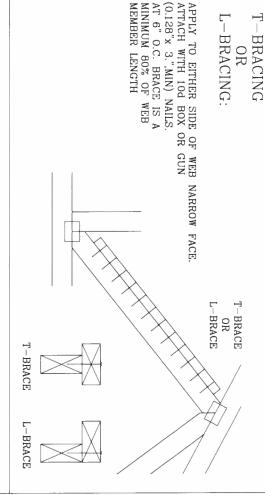
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. BRACING. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE

1-2X8	2X6	1 ROW	2X8
2-2X6(*)	2X6	2 ROWS	2X8
1-2X6	2X4	1 ROW	2X6
2-2X4(*)	2X6	2 ROWS	2X6
1-2X4	2X4	1 ROW	2X3 OR 2X4
2-2X4	2X6	2 ROWS	2X3 OR 2X4
BRACING	ALTERNATIVE BRACING T OR L-BRACE SCAB BR	SPECIFIED CLB	WEB MEMBER
SCAB BRACE		BRACING	SIZE

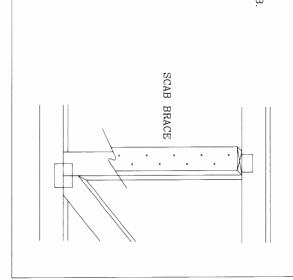
T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

* FACE OF WEB CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH



SCAB BRACING:

(0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM
BOZ OF WEB MEMBER LENGTH ATTACH WITH 10d BOX OR GUN NO MORE THAN (1) SCAB PER FACE. APPLY SCAB(S) TO WIDE FACE OF WEB.



THIS
DRAWING
REPLACES
DRAWING
579,640

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#WARNHOFM TRUSSES REDUIRE EXTREME CARE IN FABRICATING, HANDLING, SHPPING, INSTALLING AND BRACKING. REFER TO BESI (BUILDING COMPONENT SAFETY INTORHATION), PUBLISHED BY TEI (TRUSS PLAT INSTITUTE, 218 NURTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WITCA (WOOD TRUSS COLUNCIL AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIGR TO PERFORMING THE: FUNCTIONS. UNILESS OTHERWISE INDICATED, TO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

RAVINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY REAL BY THE TRUSS COMPONENT DESIGN SHOWN. THE FOR THE TRUSS COMPONENT DESIGN SHOWN. THE FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING

7-02: 110 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, 1.00, EXPOSURE

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STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE					
4 11	1 -	5.0"	5' 3"	5' 4"	4' 9"	4' 9"	4' 9"	4' 11"	4' 5"	4' 6"	4' 6"	4' 9"	4' 10"	4' 4"	4' 4"	4' 4"	4' 5"	3' 10"	4, 0,"	4' 0"	4' 2"	4' 3"	3' 9"	3' 9"	3' 9"	3' 10"	BRACES	N O					
7 5		8' 5"	8, 5,	8, 5,	7' 3"	B, 5,	8, 5,	æ, 5.	6' 5"	7' 6"	7' 7"	7' 8"	7' 8"	6, 4,"	7' 4"	7' 4"	7' 8"	5 3"	6' 1"	ල, ව,	6' 8"	6' 8"	25,	6' 0"	6' 0"	6' 8"	GROUP A	(1) 1X4 "L"					
7 5		8' 5"	9' 1"	9' 1"	7' 3"	8, 5,	8' 5"	8' 8"	6' 5"	7' 6"	7' 7"	8' 3"	8, 3,	6' 4"	7' 4"	7' 4"	7' 10"	5 3	6' 1"	6' 2"	7' 2"	7' 2"	5' 2"	6' 0"	6'0"	6' 10"	GROUP B	" BRACE *					
9 10	٦.	10' 0"	10' 0"	10' 0"	9' 7"	10' 0"	10' 0"	10' 0"	8' 6"	9' 1"	9' 1"	9' 1"	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6'11"	7' 11"	7' 11"	7' 11"	7' 11"	6' 9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) 2X4 "L"					
9 10		10' 6"	10' 9"	10' 9"	9' 7"	10' 0"	10'0"	10' 3"	8' 6"	9' 6"	9' 6"	9' 9"	9' 9"	8' 4"	9' 1"	9' 1"	9' 4"	6' 11"	8'0"	8' 1"	8' 6"	В' 6"	6'9"	7' 11"	7' 11"	8' 1"	GROUP B	" BRACE *					
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12: 3:	1 -	12' 6"	12' 10"	12' 10"	11' 11"	11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10' 10"	10' 10"	10' 10"	11' 1"	9' 4"	9' 11"	9' 11"	10' 2"	10' 2"	9' 1"	9' 5"	9' 5"	9' 8"	GROUP B	" BRACE **					
14 0	1 -	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14'0"	14' 0"	12' 11"	14' 0"	14' 0"	14'0"	10' 10"	12' 5"		12' 5"	12' 5"	10' 7"	12' 3"	-	12' 5"	GROUP A	(1) 2X6 "L"					
14 0	`	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"		14' 0"	10' 10"	-	12' 8"		13' 5"			12' 4"		GROUP B	" BRACE *					
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DOUGLAS FIR-LARCH

SOUTHERN PINE #3 STUD

STANDARD

STANDARD #3 STUD

GROUP

Ä

#1 & BTR #1

HEM-FIR

#1 / #2 STANDARD
#3 STUD

#3

STANDARD

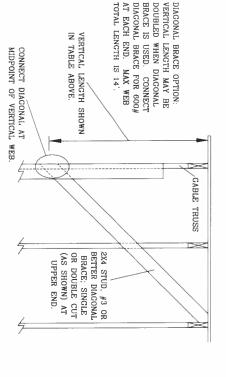
STUD

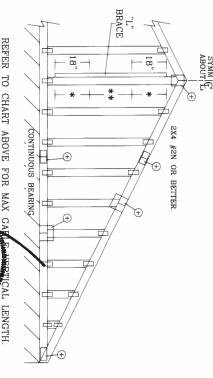
BRACING GROUP SPECIES AND GRADES:

GROUP

A

HEM-FIR





GABLE
TRUSS
DETAIL
NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

#2

#2 #

GABLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER LIVE LOAD DEFLECTION CRITERIA IS L/240. CONTINUOUS BEARING (5 PSF TC DEAD LOAD). OUTLOOKERS WITH 2' 0" OVERHANG, OR 12"

PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES. ** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES

MEMBER LENGTH.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB

+ REFEI PEAK,	GREAT	LESS	LESS 1	VE	GAB
R TO CON	GREATER THAN 11' 6"	LESS THAN 11' 6"	LESS THAN 4' 0"	VERTICAL LENGTH	LE VER
AND HE	11' 6"	1.6.	. 0	ENGTH	TICAL F
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	2		+-	NO	GABLE VERTICAL PLATE SIZES
IGN FOR	2.5X4	2X4	1X4 OR 2X3	NO SPLICE	IZES
	_		_	_	

REVARANINGEM TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST CBUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CTRUSS PLATE INSTITUTE, 218 NURTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA CWOIDD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, HADISON, WI 53719) TOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS DITHEMISE INDICATED, TOP GHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE APPROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE APPROPERLY ATTACHED RIGID CEILING.

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

ALPINE

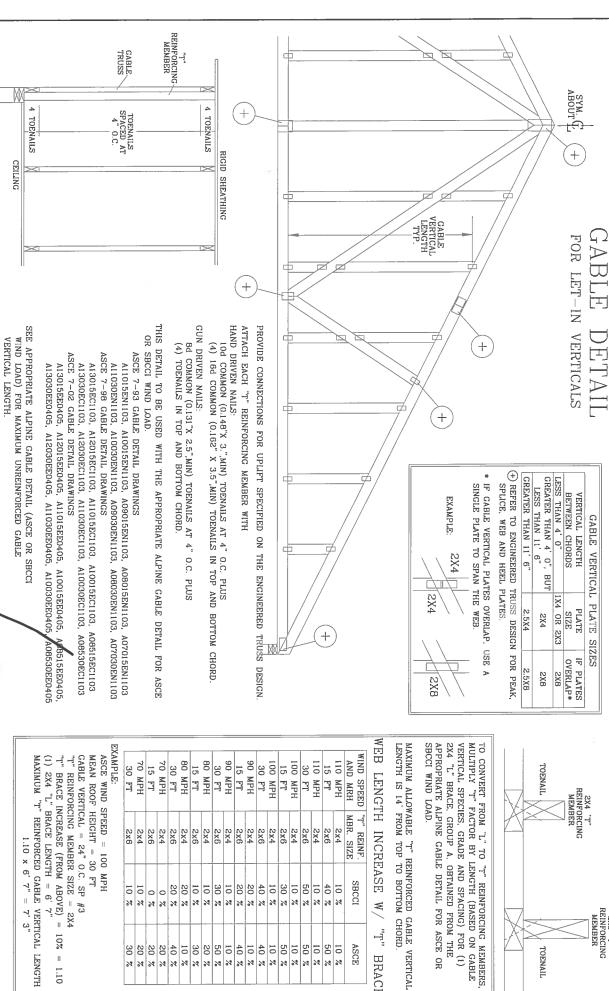
No.59687 댦유 JAIOP. # min MAX. TOT. LD. 60 PSF DRWG DATE REF -ENG

NUR R. F. CENSE MAX. SPACING

24.0

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TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE SBCCI WIND LOAD. 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR TOENAIL TOENAIL

2X4 "T" REINFORCING MEMBER

2X6 "T" REINFORCING MEMBER

٦,

WEB LENGTH INCREASE W/ BRACE

																		_			
30 FT	70 MPH	15 FT	70 MPH	30 FT	80 MPH	15 FT	80 MPH	30 FT	90 MPH	15 FT	90 MPH	30 FT	100 MPH	15 FT	100 MPH	30 FT	110 MPH	15 FT	110 MPH	AND MRH	WIND SPEED
2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	MBR. SIZE	"T" REINF.												
10 %	2 01	0 %	0 %	20 %	20 %	10 %	10 %	30 %	2 01	20 %	20 %	40 %	10 %	30 %	10 %	50 %	10 %	40 %	10 %	PBCCI	3
30 %	20 %	20 %	20 %	40 %	7 01	30 %	20 %	50 %	2 01	40 %	2 01	40 %	10 %	50 %	10 %	50 %	2 01	50 %	10 %	ADUL	1

EXAMPLE:

GABLE VERTICAL = 24" O.C. SP #3 MEAN ROOF HEIGHT = 30 FT ASCE WIND SPEED = 100 MPH "T" REINFORCING MEMBER SIZE = 2X4

 2X4 "L" BRACE LENGTH = 6' 7" "T" BRACE INCREASE (FROM ABOVE) = 10%

= 1.10

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH $1.10 \times 6' 7'' = 7' 3''$

REPLACES DRAWINGS GAB98117 876,719 & HC26294035

The state of	1	4 4 4 4	1	1				4 4 4 4
S.						REF	LET-IN VERT	VERT
						DATE	11/1/06	
•						DRWG	DRWG GBLLETIN1106	V1106
71 ? ?!!!!!!!!						-ENG	-ENG DLJ/KAR	
SINEE MAX	MAX	MAX TOT. LD. 60 PSF	LD.	60	PSF			
	DUR.	DUR. FAC.		ANY				
	NAV SDACING	ט ע ב		יי עי	2			

WANDERFANTS. FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL AND TER RESPONSIBLE FOR ANY DEVILATION FROM THIS DESIGN, ANY FAILURE TO BRILLIO THE TRUSS. IN CONFIDENCE THE THIS OF FARRICATING, HANDLING, SHIPPING, INSTALLING & BRACHING OF REUSSES. BESIGN CONFEDENCY WITH APPLICABLE PROVISIONS OF INS. WANTIONAL DESIGN SPEC, BY AFEND AND THE ALPINE CONNECTION PLATES ARE MADE OF 201201/66A (**)HASS, OTHER MISS. GRADE 40.66 (**)L/FASS, GALV, STEEL APPLY PLATES. TO EACH FACE OF TRUSS. AND UNLESS TOLLOWED BY (1) SHALL BE PER ANNEX ASD TOPP 1—2000 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROPESSIONAL ENGINEERING RESPONSIBILITY SOLICITY FOR THE TRUSS. COMPONENT DESIGN SHOWN. THE SUITABLE FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING IS THE RESPONSIBILITY OF THE BUILDING IS THE RESPONSIBILITY OF THE BUILDING. **AVARNING** TRUSSES REDUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST OBLILDING CUPPIDIKIT SAFETY INFORMATION, PUBLISHED BY TPI CTRUSS PLATE INSTITUTE, 218 NIRTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND VTCA VADIO TRUSS COLUCIL OF AMERICA, 6300 ENTERPRISE LN, HADISON, VI 53719, FOR SAFETY PRACTICES PRIBE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERVISE INDICATED, TOP GHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE ROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE ROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE APPRIERED.

TA HUN H. FI.O. CENSE MORIO. STATE 0 No. 58687 ONAL

MAX SPACING

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ALPINE ENGINEERED PRODUCTS, INC. POMPANO BEACH, FLORIDA

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