	This Permit Must Be Prominently Posted	on Premises During Con	struction	000026628
APPLICANT MELANIE	RODER	PHONE	623-7829	
ADDRESS 387	SW KEMP COURT	LAKE CITY	## The state of th	FL 32024
OWNER SIM-Q DE	VELOPERS	PHONE	867-0692	Called a Service and April 1990
ADDRESS 453	SW ROSEMARY DRIVE	LAKE CITY		FL 32024
CONTRACTOR DAV	TID SIMQUE	PHONE	867-0692	
LOCATION OF PROPERT	Y 90W, TL ON 252B, TR ON ROSE	MARY DR, 6TH LOT O	N RIGHT	
	-			
TYPE DEVELOPMENT	SFD,UTILITY EST	TIMATED COST OF CO	NSTRUCTION	190600.00
HEATED FLOOR AREA		A 3812.00	HEIGHT _	STORIES 1
FOUNDATION CONC	WALLS FRAMED R	OOF PITCH 10/12	FL	OOR SLAB
LAND USE & ZONING	PRD	MAX	HEIGHT _	
Minimum Set Back Require	ments: STREET-FRONT 25.00	REAR	15.00	SIDE 10.00
NO. EX.D.U. 0	FLOOD ZONE X PP	DEVELOPMENT PERM	MIT NO.	
PARCEL ID 03-4S-16-0	2731-111 SUBDIVISION	N PRESERVE AT LA	UREL LAKES	
LOT 111 BLOCK	PHASE UNIT		L ACRES	
			E ACICES	
000001527	CBC056158	1 Villie		
Culvert Permit No.	Culvert Waiver Contractor's License Num		Applicant/Owner	
WAIVER	X08-009 BK	<u>J</u>		Y Nov. Posidos
Driveway Connection	VIII.0		roved for Issuance	e New Resident
	.5 PER PLAT, ELEVATION CONFRMATIO	N LETTER REQUIRED		
AT SLAB, NOC ON FILE				
				. 2270
			Check # or C	ash 3370
	FOR BUILDING & ZONIN	To the state of the state of		
Temporary Power	FOR BUILDING & ZONIN Foundation	To the state of the state of		(footer/Slab)
Temporary Power		To the state of the state of	ONLY	
Temporary Power Under slab rough-in plumbi	date/app. by Slab	date/app. by	ONLY Monolithic	(footer/Slab) date/app. by Nailing
Under slab rough-in plumbi	foundation Foundation Slab date/app. by	date/app. by	ONLY Monolithic _ Sheathing/	(footer/Slab) date/app. by
Under slab rough-in plumbi	Foundation Foundation Slab Slab date/app. by Rough-in plumbing ab	date/app. by	ONLY Monolithic _ Sheathing/	(footer/Slab) date/app. by Nailing date/app. by
Under slab rough-in plumbi	Foundation Foundation Slab Slab Adate/app. by Rough-in plumbing ab	date/app. by date/app. by ove slab and below wood	ONLY Monolithic _ Sheathing/	(footer/Slab) date/app. by Nailing date/app. by date/app. by
Under slab rough-in plumbi Framing	Foundation Foundation Slab Slab date/app. by Rough-in plumbing ab	date/app. by date/app. by ove slab and below wood	ONLY Monolithic _ Sheathing/	(footer/Slab) date/app. by Nailing date/app. by date/app. by
Under slab rough-in plumbi Framing	Foundation Foundation Slab Slab	date/app. by date/app. by ove slab and below wood date/app. by	ONLY Monolithic _ Sheathing/	(footer/Slab) date/app. by Nailing date/app. by date/app. by l) date/app. by
Under slab rough-in plumbi Framing	Foundation date/app. by ng Slab date/app. by Rough-in plumbing ab b. by Heat & Air Duct date/app. by C.O. Final	date/app. by date/app. by ove slab and below wood	ONLY Monolithic Sheathing/ floor Peri. beam (Linte	(footer/Slab) date/app. by Nailing date/app. by date/app. by
Under slab rough-in plumbi Framing	Foundation date/app. by ng Slab date/app. by Rough-in plumbing ab b. by Heat & Air Duct date/app. by C.O. Final	date/app. by date/app. by ove slab and below wood date/app. by ate/app. by	ONLY Monolithic _ Sheathing/ floor Peri. beam (Linter	(footer/Slab) date/app. by Nailing date/app. by date/app. by l) date/app. by date/app. by
Under slab rough-in plumbi Framing	Foundation date/app. by ng Slab date/app. by Rough-in plumbing ab by Heat & Air Duct date/app. by C.O. Final e/app. by ctricity and plumbing Pump pole	date/app. by date/app. by ove slab and below wood date/app. by date/app. by date/app. by utility Pole	ONLY Monolithic _ Sheathing/ floor Peri. beam (Linte Culvert Pool	(footer/Slab) date/app. by Nailing
Under slab rough-in plumbi Framing	Foundation date/app. by ng Slab date/app. by Rough-in plumbing ab by Heat & Air Duct date/app. by C.O. Final e/app. by date/app. Pump pole ate/app. by Travel Trailer	date/app. by date/app. by ove slab and below wood date/app. by date/app. by date/app. by utility Pole app. by	Monolithic Sheathing/ floor Peri. beam (Linter Culvert Pool e date/app. by	(footer/Slab) date/app. by Nailing
Under slab rough-in plumbi Framing	Foundation date/app. by ng Slab date/app. by Rough-in plumbing ab by Heat & Air Duct date/app. by C.O. Final e/app. by date/app. Pump pole ate/app. by Travel Trailer	date/app. by date/app. by ove slab and below wood date/app. by date/app. by date/app. by utility Pole	ONLY Monolithic _ Sheathing/ floor Peri. beam (Linte Culvert Pool	(footer/Slab) date/app. by Nailing
Under slab rough-in plumbi Framing	Foundation date/app. by ng Slab date/app. by Rough-in plumbing ab by Heat & Air Duct date/app. by C.O. Final e/app. by date/app. pump pole ate/app. by Travel Trailer	date/app. by date/app. by ove slab and below wood date/app. by ate/app. by Utility Pole app. by ate/app. by	Monolithic Sheathing/ floor Peri. beam (Linter Culvert Pool e date/app. by	(footer/Slab) date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by
Under slab rough-in plumbi Framing	Foundation date/app. by ng Slab date/app. by Rough-in plumbing ab by Heat & Air Duct date/app. by C.O. Final e/app. by date/app. Pump pole ate/app. by Travel Trailer	date/app. by date/app. by ove slab and below wood date/app. by ate/app. by Utility Pole app. by ate/app. by 19.06	ONLY Monolithic Sheathing/ floor Peri. beam (Linte Culvert Pool date/app. by Re-roof SURCHARGE	(footer/Slab) date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by FEE \$ 19.06
Under slab rough-in plumbing date/app Electrical rough-in Permanent power date M/H tie downs, blocking, electrical date/app. BUILDING PERMIT FEE \$ MISC. FEES \$ 0.00	Foundation date/app. by ng Slab date/app. by Rough-in plumbing ab by Heat & Air Duct date/app. by C.O. Final e/app. by ctricity and plumbing date/app. Pump pole ate/app. by Travel Trailer date/app. ZONING CERT. FEE \$ 50.00	date/app. by date/app. by ove slab and below wood date/app. by ate/app. by Utility Pole app. by ate/app. by ate/app. by ate/app. by fire FEE \$ 0.00	Monolithic Sheathing/ floor Peri. beam (Linter Pool date/app. by Re-roof SURCHARGE WASTI	(footer/Slab) date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by EFEE \$ 19.06
Under slab rough-in plumbi Framing	Foundation date/app. by ng Slab date/app. by Rough-in plumbing ab by Heat & Air Duct date/app. by C.O. Final e/app. by cetricity and plumbing date/app. Pump pole ate/app. by Travel Trailer date/app. ZONING CERT. FEE \$ 50.00 EE \$ FLOOD ZONE FEE \$ 25.00	date/app. by date/app. by ove slab and below wood date/app. by ate/app. by Utility Pole app. by ate/app. by ate/app. by ate/app. by fire FEE \$ 0.00	Monolithic Sheathing/ floor Peri. beam (Linter Pool date/app. by Re-roof SURCHARGE WASTI	(footer/Slab) date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by FEE \$ 19.06

DEDATE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0801 - 37 Date Received 19/08 By 4 Permit # 1527/26/628
Zoning Official BLK Date 15.01.08 Flood Zone Cht FEMA Map # W/A Zoning PRO Land Use R LO Elevation W/A MFE 116.5 River W/A Plans Examiner OF JTH Date 1-14-08
Comments Floring Trass Annual Elevation Carlymake Latte Pegano
NOC #EH Deed or PA Site Plan D State Road Info Dearent Parcel #
□ Dev Permit # □ In Floodway
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Fax 752-2282
Name Authorized Person Signing Permit Melanie Roder Phone 623-7829
Address 387 SW Kemp court Lake City, FL 32024
Owners Name Sim-Q Developers Phone 867-0692
911 Address 453 SW Rosemary dr. Lake City, FL 32024 Contractors Name Simple Construction LLC Phone
Contractors Name Simple Construction LLC Phone
Address 122 SW Midtown Pl, Suite 101, Lake City, FL 32024
Fee Simple Owner Name & Address
Bonding Co. Name & Address
Architect/Engineer Name & Address Will Mycrs/ Mack Dississary
Mortgage Lenders Name & Address Capital City Bank, 4040 NW 16th Blvd, Gaine Sville FC
Circle the correct power company —FL Power & Light — Clay Elec. — Suwannee Valley Elec. — Progress Energy
Property ID Number 03-45-16-02731-111 Estimated Cost of Construction 190,000
Subdivision Name Preserve at Laurel Lakes Lot III Block Unit 1 Phase
Driving Directions 90W Thon CR 252B TB on SW Bosemary dr, Loth lot
right.
Number of Existing Dwellings on Property
Construction of SFN Total Acreage 33 Lot Size
Construction of SFD Total Acreage 33 Lot Size Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height $29'-0''$
Actual Distance of Structure from Property Lines - Front 38-0' Side 18-0' Side 13-9' Rear 45-2"
Number of Stories Heated Floor Area Total Heated Floor Area Roof Pitch Roof Pitch
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards
of all laws regulating construction in this jurisdiction.

Page 1 of 2 (Both Pages must be submitted together.)

14. 8

spoke to Melvare

Revised 11-13-07

Application #	

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

<u>YOU ARE HEREBY NOTIFIED</u> as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit. **Owners Signature** Affirmed under penalty of perjury to by the Owner and subscribed before me this \(\frac{\lambda}{2} \) day of \(\tag{Tag} Personally known ____ or Produced Identification Linda R. Roder Commission #DD303275 * Expires: Mar 24, 2008 Bonded Thru State of Florida Notary Signature (For the Owner) Atlantic Bonding Co., Inc. **CONTRACTORS AFFIDAVIT:** By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit. Contractor's License Number Contractor's Signature (Permitee) Columbia County Competency Card Number____ Affirmed under penalty of perjury to by the Contractor and subscribed before me this 8 day of Jan 2008 Personally known or Produced Identification Linda R. Roder Commission #DD303275 State of Florida Notary Signature (For the Contractor) Expires: Mar 24, 2008

Bonded Thru Atlantic Bonding Co., Inc. 01-08-'08 12:50 FROM-WESTFIELD GROUP P. DEWITT GASON. CLERK OF COURTS

TERRY MCDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328

Starow 07-2008 Date

כשמש-ככו-מצצ



File No. 07-373

PERMIT NO._

TAX FOLIO NO.: 03-45-16-02731-111

NOTICE OF COMMENCEMENT

STATE OF FLORIDA COUNTY OF COLUMBIA Inst:200812000295 Date:1/7/2008 Time:4:15 PM 2. DC,P.DeWitt Cason,Columbia County Page 1 of 2

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Description of property: 1.

> Lot 111, PRESERVE AT LAUREL LAKE, UNIT 1, a subdivision according to the plat thereof as recorded in Plat Book 9, Pages 18-25 of the public records of Columbia County, Florida.

- General description of improvement: Construction of single family dwelling.
 - 3. Owner information:
- SIM-Q DEVELOPERS, LLC, a Florida Limited Name and address: Liability Company, 122 SW Midtown Place, Suite 101, Lake City, Florida 32024.
 - Interest in property: Fee Simple b.
- Name and address of fee simple title holder (if other than C. Owner):
- Contractor: SIMQUE CONSTRUCTION, LLC, 122 SW Midtown Place, Suite 101, Lake City, Florida 32024.
 - Surety 5.
 - Name and address: None a .
- CAPITAL CITY BANK, 4040 NW 16th Boulevard, Gainesville, Lender: Florida 32605.
- Persons within the State of Florida designated by Owner upon whom or other documents may be served as provided by Section notices 713.13(1)(a)7., Florida Statutes:
- In addition to himself, Owner designates JOHN RAFFERTY, CAPITAL CITY BANK, 4040 NW 16th Boulevard, Gainesville, Florida 32605, 352-372-5162, to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

SIM-Q DEVELOPERS, LLC

BV:

David simque

Managing Member

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 7th day of January 2008, by DAVID SIMQUE, Managing Member of SIM-Q DEVELOPERS, LLC, a Florida Limited Liability Company, on behalf of said company. He is personally known to me and did not take an oath.

Notary Public
My commission expires:



In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

(Signature of First

Terry McDavid

(Typed Name of First Witness)

(Signature of Second Witness)

Myrtle Ann McElroy

(Typed Name of Second Witness)

RESIDENTIAL DEVELOPMENT GROUP,

LLC

(SEAL)

Grancor

BY: DANIEL CRAPPS Managing Member

Grantor

BY: CHARLES S. SPARKS Managing Member

STATE OF Florida COUNTY OF Columbia

The foregoing instrument was acknowledged before me this 7th day of January, 2008, by DANIEL CRAPPS and CHARLES S. SPARKS, as Managing Members of RESIDENTIAL DEVELOPMENT GROUP, LLC, A Florida Limited Liability Company, who are personally known to me and who did not take an oath.

My Commission Expires:

Notary Public

TERMY MCDAVID Limited, typed, or stamped name:
MY COMMISSION EDD SOO768
EXCHANGES LINE DD SOO768

EXPIRES: January 16, 2010 order Thru Holesy Public Undersease



Project Information for:

SEAL TRUSSES

Builder:

Aarom Simque Homes, Inc.

Lot:

111

Subdivision: County:

Preserve Columbia

Truss Count:

73

Design Program: MiTek 20/20 6.3 Building Code:

FBC2004/TPI2002

Truss Design Load Information: Gravity: Wind:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): 55.0

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

Aaron D. Simque Florida Registered Building Contractor License No. RB29003130

Address: Aarom Simque Homes, Inc. Route 9 Box 785-33 Lake City, FL 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

25

26

27

28

J1922699

J1922700

J1922701

J1922702

PB3

PB3A

PB3B

PB4

1/9/08

1/9/08

1/9/08

1/9/08

53

54

55

56

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date	No.	Drwg. #	Truss ID	Date	No.	Drwg. #	Truss ID
1	J1922675	CJ1	1/9/08	29	J1922703	PB4A	1/9/08	57	J1922731	T20
2	J1922676	CJ1A	1/9/08	30	J1922704	PB4B	1/9/08	58	J1922732	T21
3	J1922677	CJ3	1/9/08	31	J1922705	PB4C	1/9/08	59	J1922733	T22
4	J1922678	CJ3A	1/9/08	32	J1922706	PB4D	1/9/08	60	J1922734	T23
5	J1922679	CJ5	1/9/08	33	J1922707	PB4E	1/9/08	61	J1922735	T24
6	J1922680	CJ5A	1/9/08	34	J1922708	PB4F	1/9/08	62	J1922736	T25
7	J1922681	EJ3	1/9/08	35	J1922709	PB4G	1/9/08	63	J1922737	T26
8	J1922682	EJ7	1/9/08	36	J1922710	PB5	1/9/08	64	J1922738	T26G
9	J1922683	EJ7A	1/9/08	37	J1922711	T01G	1/9/08	65	J1922739	T27
10	J1922684	EJ7B	1/9/08	38	J1922712	T02	1/9/08	66	J1922740	T27G
11	J1922685	HJ4	1/9/08	39	J1922713	T02G	1/9/08	67	J1922741	T28
12	J1922686	HJ9	1/9/08	40	J1922714	T03	1/9/08	68	J1922742	T28G
13	J1922687	HJ9A	1/9/08	41	J1922715	T04	1/9/08	69	J1922743	T29
14	J1922688	PB1	1/9/08	42	J1922716	T05	1/9/08	70	J1922744	T30
15	J1922689	PB1A	1/9/08	43	J1922717	T06	1/9/08	71	J1922745	T31
16	J1922690	PB1B	1/9/08	44	J1922718	T07	1/9/08	72	J1922746	T32
17	J1922691	PB1C	1/9/08	45	J1922719	T08	1/9/08	73	J1922747	T32G
18	J1922692	PB1D	1/9/08	46	J1922720	T09	1/9/08			-
19	J1922693	PB2	1/9/08	47	J1922721	T10	1/9/08			
20	J1922694	PB2A	1/9/08	48	J1922722	T11	1/9/08			
21	J1922695	PB2B	1/9/08	49	J1922723	T12	1/9/08			
22	J1922696	PB2C	1/9/08	50	J1922724	T13	1/9/08			
23	J1922697	PB2D	1/9/08	51	J1922725	T14	1/9/08			
24	J1922698	PB2E	1/9/08	52	J1922726	T15	1/9/08			
							The state of the s	-		

J1922727

J1922728

J1922729

J1922730

T16

T17

T18

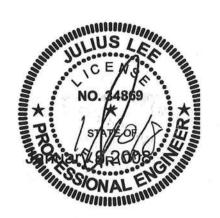
T19

1/9/08

1/9/08

1/9/08

1/9/08



Date

1/9/08

1/9/08

1/9/08

1/9/08

1/9/08

1/9/08

1/9/08

1/9/08

1/9/08

1/9/08

1/9/08

1/9/08

1/9/08

1/9/08

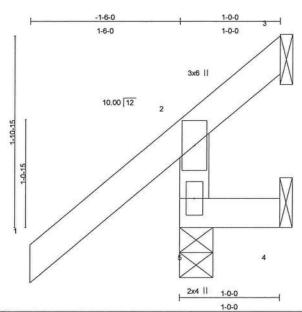
1/9/08

1/9/08

1/9/08

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	CJ1	JACK	4	1		J1922675
OL IL MODOLO	001	or tort	'		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:43 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	0.00	5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	0.00	5	>999	240	in an an out meet	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	100000000000000000000000000000000000000					Weight: 8 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 **WEBS**

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or

1-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=203/0-4-0, 4=-15/Mechanical, 3=-45/Mechanical

Max Horz 5=121(load case 6)

Max Uplift 5=-131(load case 6), 4=-27(load case 6), 3=-45(load case 1) Max Grav 5=203(load case 1), 4=7(load case 2), 3=39(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-5=-179/190, 1-2=0/57, 2-3=-58/30

BOT CHORD 4-5=0/0

JOINT STRESS INDEX

2 = 0.33 and 5 = 0.32

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 Ib uplift at joint 5, 27 lb uplift at joint 4 and 45 lb uplift at joint 3. Continued on page 2

January 9,2008

Scale = 1:10.9

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	CJ1	JACK	4	1		J1922675
SEAL TROOSES	031	JAOK	T	্	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:43 2008 Page 2

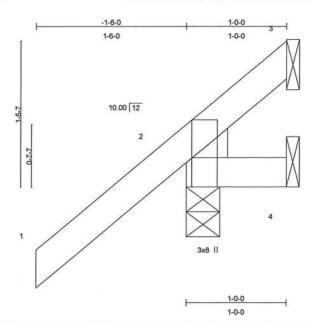
LOAD CASE(S) Standard

us Les se Design Endineer nda PE No. 34868 09 Ceastal Bay Blvd Vnton Beach, Ft. 90435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
			74			J1922676
SEAL TRUSSES	CJ1A	JACK	4	1		
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:43 2008 Page 1



Scale = 1:10.9

Plate Of	fsets (X,)	'): [2:0-3-8,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240	5,100,200,000,000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

1-0-0 oc purlins.

BOT CHORD R

Rigid ceiling directly applied or 10-0-0 oc

Structural wood sheathing directly applied or

bracing.

REACTIONS (lb/size) 2=180/0-4-0, 4=5/Mechanical, 3=-41/Mechanical

Max Horz 2=113(load case 6)

Max Uplift 2=-172(load case 6), 3=-41(load case 1)

Max Grav 2=180(load case 1), 4=14(load case 2), 3=53(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/44, 2-3=-70/48

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.08 and 2 = 0.00

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Truse Cesign Engineer Flonds PE No. 34869 1100 Gestal Bay Blvd Boynton Besch, FL 33435

January 9,2008

Warming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	EL-STRANG RESERVED
SEAL TRUSSES	CJ1A	JACK	4	1		J1922676
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:43 2008 Page 2

NOTES

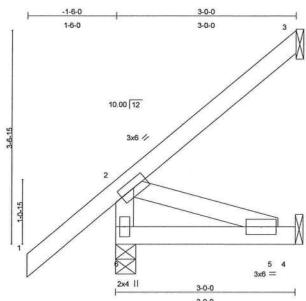
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 172 lb uplift at joint 2 and 41 lb uplift at joint 3.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	CJ3	JACK	2	4		J1922677
SEAL TRUSSES	C33	JACK	2	1 1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:44 2008 Page 1



1-6-0		3-0-0	³ N	Scale = 1:18.2
		/		
	10.00 12	//		
3-6-15	3x6 1/			
	2//			
1-0-15			_	
.,//			5 4	
V	2x4 II	3-0-0	3x6 =	
		3-0-0		

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.00	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	5-6	>999	240	, it was to be a second of	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 18 lb	

	IBA	DI	=R
Ll	JIV	161	- K

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=204/0-4-0, 3=49/Mechanical, 4=14/Mechanical

Max Horz 6=216(load case 6)

Max Uplift 6=-77(load case 6), 3=-48(load case 6), 4=-55(load case 6) Max Grav 6=204(load case 1), 3=49(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

2-6=-190/93, 1-2=0/57, 2-3=-65/22 TOP CHORD

BOT CHORD 5-6=-240/0, 4-5=0/0

WEBS

2-5=0/253

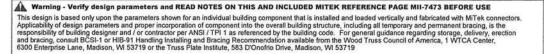
JOINT STRESS INDEX

2 = 0.11, 5 = 0.07 and 6 = 0.07

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	100000000000000000000000000000000000000
SEAL TRUSSES	CJ3	JACK	2	1		J1922677
			17		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:44 2008 Page 2

NOTES

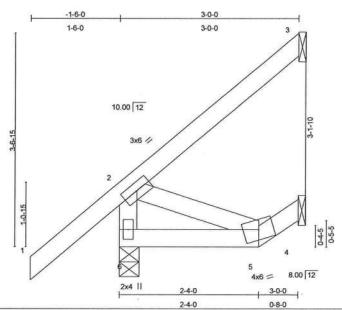
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 6, 48 lb uplift at joint 3 and 55 lb uplift at joint 4.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	CJ3A	SPECIAL	2	1		J1922678
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:44 2008 Page 1



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.01	5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 18 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 WERS

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/size) 6=245/0-4-0, 3=20/Mechanical, 4=3/Mechanical

Max Horz 6=106(load case 2), 3=185(load case 6)

Max Uplift 6=-288(load case 6), 3=-76(load case 2)

Max Grav 6=245(load case 1), 3=106(load case 6), 4=9(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

2-6=-234/348, 1-2=0/57, 2-3=-155/250

BOT CHORD

5-6=-106/0, 4-5=-4/4

WEBS

2-5=0/114

JOINT STRESS INDEX

2 = 0.21, 5 = 0.05 and 6 = 0.19

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 9,2008

Scale = 1:18.2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	CJ3A	SPECIAL	2	1		J1922678
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:44 2008 Page 2

NOTES

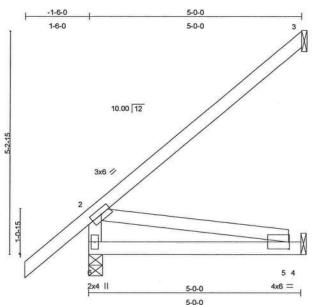
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 288 lb uplift at joint 6 and 76 lb uplift at joint 3.
- 5) Non Standard bearing condition. Review required.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	CJ5	JACK	2	1		J1922679
	1000	The state of the s			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:45 2008 Page 1



Scale = 1:25.5

Plate Of	fsets (X, Y	(): [5:0-0-2,0-1-14]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.03	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	5-6	>999	240	1 - 20-00-000-000-000-000-000-000-000-000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.08	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)						Weight: 28 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 6=257/0-4-0, 3=114/Mechanical, 4=24/Mechanical

Max Horz 6=293(load case 6)

Max Uplift 6=-69(load case 6), 3=-139(load case 6), 4=-37(load case 6) Max Grav 6=257(load case 1), 3=114(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

2-6=-234/96, 1-2=0/57, 2-3=-124/56

BOT CHORD

5-6=-325/1, 4-5=0/0

WEBS

WEBS

2-5=-1/331

JOINT STRESS INDEX

2 = 0.13, 5 = 0.13 and 6 = 0.08

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

E) All hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

January 9,2008

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	CJ5	JACK	2	1		J1922679
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:45 2008 Page 2

NOTES

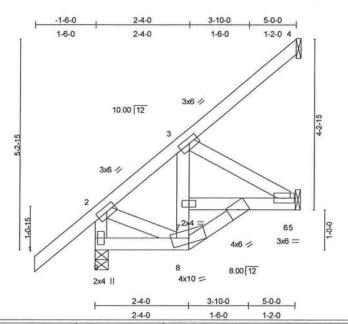
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 6, 139 lb uplift at joint 3 and 37 lb uplift at joint 4.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	CJ5A	SPECIAL	2	1	200	J1922680
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:27:06 2008 Page 1



LOADING (psf) SPACING DEFL 2-0-0 CSI GRIP in (loc) I/defl L/d **PLATES** TCLL 20.0 Plates Increase 1.25 TC 0.23 Vert(LL) 0.01 8 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.07 Vert(TL) -0.016-7 >999 240 **BCLL** 10.0 Rep Stress Incr YES WB 0.31 -0.02Horz(TL) 6 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 35 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

8-10 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=257/0-4-0, 4=69/Mechanical, 6=69/Mechanical

Max Horz 9=293(load case 6)

Max Uplift 9=-68(load case 6), 4=-94(load case 6), 6=-83(load case 6) Max Grav 9=257(load case 1), 4=69(load case 1), 6=81(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-9=-246/90, 1-2=0/57, 2-3=-130/0, 3-4=-81/35

BOT CHORD 8-9=-325/1, 6-7=-175/95, 5-6=0/0

WEBS 7-8=-86/13, 3-7=-78/53, 2-8=0/237, 3-6=-110/203

JOINT STRESS INDEX

2 = 0.11, 3 = 0.10, 6 = 0.06, 7 = 0.10, 8 = 0.39, 9 = 0.09 and 10 = 0.00

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 68 lb uplift at joint 9, 94 lb uplift at joint 4 and 83 lb uplift at joint 6. Continued on page 2

January 9,2008

Scale = 1:27.1

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



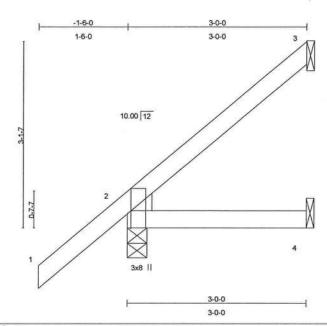
Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	CJ5A	SPECIAL	2	1	J19226	80
OL IL TROOOLO	000/1	OI LOINE	-		Job Reference (optional)	
Builders FirstSource	e, Lake City, FI 3	2055 6.3	00 s Apr 19 2006 N	ITek Ind	lustries, Inc. Wed Jan 09 13:27:06 2008 Page 2	

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	EJ3	MONO TRUSS	6	1		J1922681
02.12.11.00020		merre mese			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:46 2008 Page 1



Scale = 1:18.2

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	2-4	>999	240	2501.0.77970	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 14 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=206/0-4-0, 4=14/Mechanical

Max Horz 2=188(load case 6)

Max Uplift 3=-75(load case 6), 2=-114(load case 6)

Max Grav 3=48(load case 1), 2=206(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-74/23

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.10 and 2 = 0.00

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

E) All hearings age assumed to be SYP No.2 crushing capacity of 565.00 psi

Trues Design Engineer Flonds PE No. 34868 1100 Cassiel Bay Blvd. Boynton Beach. FL 33436

January 9,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	EJ3	MONO TRUSS	6	1	2	J1922681
OD IL INCOCEO		mente intese	ľ		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:46 2008 Page 2

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 3 and 114 lb uplift at joint 2.

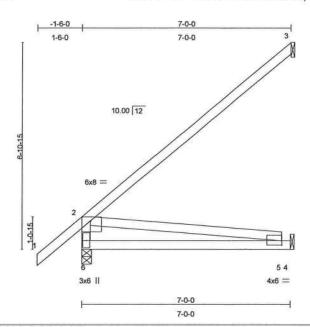
LOAD CASE(S) Standard

an Sesion Engineer PE No. 24868 inastal Bay Blvd 2 Desch. FL 00435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	EJ7	MONO TRUSS	9	1		J1922682
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:46 2008 Page 1



Scale = 1:36.4

Plate Of	fsets (X,)	(): [2:0-3-8,Edge], [5:	0-0-1,0-1-	14]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.69	Vert(LL)	-0.06	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.10	5-6	>779	240	- dones a servicio	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.24	Horz(TL)	-0.02	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 38 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 8-0-2 oc

bracing.

REACTIONS (lb/size) 3=142/Mechanical, 6=317/0-4-0, 4=64/Mechanical

Max Horz 6=271(load case 6)

Max Uplift 3=-122(load case 6), 6=-36(load case 6), 4=-32(load case 6) Max Grav 3=142(load case 1), 6=317(load case 1), 4=110(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/57, 2-3=-164/68, 2-6=-274/102

BOT CHORD

5-6=-620/299, 4-5=0/0

WEBS

2-5=-301/625

JOINT STRESS INDEX

2 = 0.63, 5 = 0.28 and 6 = 0.43

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

E) All hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Truse Design Engineer Florida PE No. 34865 1109 Coastal Bay Blvd Boynton Beach, FL 33435

January 9,2008

🗥 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	EJ7	MONO TRUSS	9	1		J1922682
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:46 2008 Page 2

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint 3, 36 lb uplift at joint 6 and 32 lb uplift at joint 4.

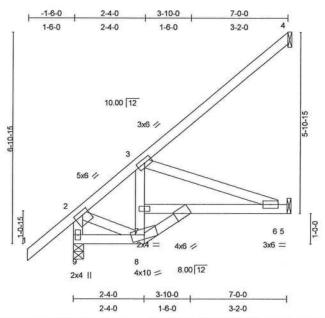
LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 24869 1109 Coestal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	EJ7A	SPECIAL	3	1		J1922683
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:27:55 2008 Page 1



Scale = 1:35.5

Plate Of	fsets (X,Y):	[2:0-2-12,0-1-8]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.04	7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.06	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.67	Horz(TL)	-0.05	5	n/a	n/a		
BCDL	CDL 5.0 Code FBC2004/TPI2002		(Mat	rix)	, ,					Weight: 44 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=109/Mechanical, 9=317/0-4-0, 5=97/Mechanical

Max Horz 9=271(load case 6)

Max Uplift 4=-94(load case 6), 9=-36(load case 6), 5=-60(load case 6) Max Grav 4=109(load case 1), 9=317(load case 1), 5=116(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/57, 2-3=-188/0, 3-4=-124/54, 2-9=-306/99

BOT CHORD

8-9=-212/22, 6-7=-376/223, 5-6=0/0

WEBS

7-8=-77/0, 3-7=-50/64, 2-8=-19/223, 3-6=-236/398

JOINT STRESS INDEX

2 = 0.80, 3 = 0.18, 6 = 0.11, 7 = 0.21, 8 = 0.47, 9 = 0.33 and 10 = 0.00

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at Continued 36 lb applift at joint 9 and 60 lb uplift at joint 5.

Florida PE No. 34868 1109 Coastel Bay Blvd Boynton Beach, FL 33431

January 9,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



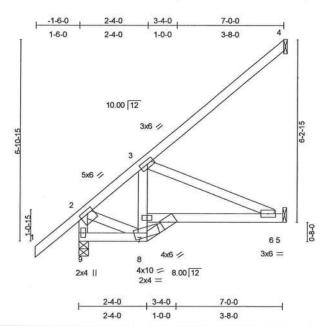
Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	EJ7A	SPECIAL	3	1	J1922	683
					Job Reference (optional) .	
Builders FirstSourc	e, Lake City, FI 3	32055 6.3	00 s Apr 19 2006 M	iTek Ind	lustries, Inc. Wed Jan 09 13:27:55 2008 Page 2	

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	ЕЈ7В	SPECIAL	1	1		J1922684
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:28:43 2008 Page 1



Scale = 1:37.2

Plate Of	fsets (X,Y)	[2:0-2-12,0-1-8]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.03	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.06	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.46	Horz(TL)	-0.03	5	n/a	n/a		
BCDL	[18] [- 18]		(Mati							Weight: 43 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=107/Mechanical, 9=317/0-4-0, 5=100/Mechanical

Max Horz 9=271(load case 6)

Max Uplift 4=-91(load case 6), 9=-36(load case 6), 5=-62(load case 6) Max Grav 4=107(load case 1), 9=317(load case 1), 5=117(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/57, 2-3=-189/0, 3-4=-121/52, 2-9=-307/99

BOT CHORD 8-9=-204/21, 6-7=-320/190, 5-6=0/0

WEBS 7-8=-81/6, 3-7=-53/62, 2-8=-36/233, 3-6=-206/347

JOINT STRESS INDEX

2 = 0.84, 3 = 0.16, 6 = 0.10, 7 = 0.18, 8 = 0.49, 9 = 0.34 and 10 = 0.00

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at Coining 436 banding at joint 9 and 62 lb uplift at joint 5.

January 9,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	100000000000000000000000000000000000000
SEAL TRUSSES	EJ7B	SPECIAL	1	1		J1922684
	A TOTAL CONTROL OF				Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:28:43 2008 Page 2

LOAD CASE(S) Standard

s Les e Design Engineer de PE No. 24866 d Coestel Bay Blvd non Besch. FL 33436



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	VIVVE POVED-POST PR
SEAL TRUSSES	HJ4	JACK	2	1		J1922685
02 12 11100020	1.01	U) (O) (-		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:48 2008 Page 1

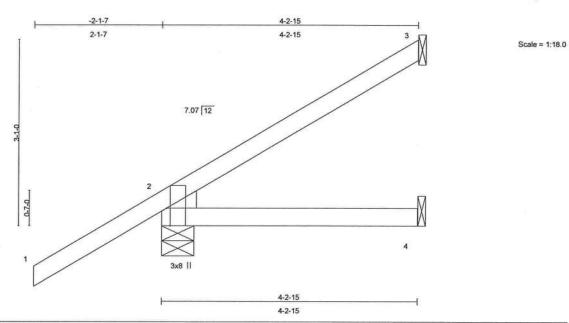


Plate Of	fsets (X,Y	'): [2:0-3-8,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.02	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 18 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-2-15 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=38/Mechanical, 2=217/0-6-6, 4=14/Mechanical

Max Horz 2=114(load case 5)

Max Uplift 3=-23(load case 5), 2=-160(load case 5)

Max Grav 3=83(load case 3), 2=217(load case 1), 4=53(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/49, 2-3=-47/45

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.09 and 2 = 0.00

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb Complificat is in agend 160 lb uplift at joint 2.

Julius Las Truss Design Engineer Flonds PE No. 34866 1 109 Caastal Bay Blvd Boynton Besch, FL 23435

January 9,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	HJ4	JACK	2	1		J1922685
OLAL TROOOLO	1104	UNOIX			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:48 2008 Page 2

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-3=-57(F=-2, B=-2), 2=0(F=5, B=5)-to-4=-11(F=-0, B=-0)

Julius Lee Trues Design Engineer Florida PE No. 34869 1109 Ceastal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	НЈ9	MONO TRUSS	1	1		J1922686
	13.4355	1		100	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:48 2008 Page 1

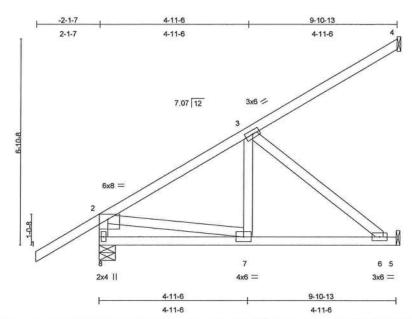


Plate Of	fsets (X, Y	'): [2:0-3-8,Edge]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	0.03	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.07	6-7	>999	240	1	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.24	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 57 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2

2 X 4 SYP No.2 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/size) 8=399/0-6-6, 4=235/Mechanical, 5=266/Mechanical

Max Horz 8=462(load case 5)

Max Uplift 8=-80(load case 5), 4=-289(load case 5), 5=-189(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-8=-400/92, 1-2=0/62, 2-3=-364/0, 3-4=-185/87

BOT CHORD 7-8=-160/122, 6-7=-295/302, 5-6=0/0

WEBS 2-7=-137/440, 3-7=0/170, 3-6=-389/379

JOINT STRESS INDEX

2 = 0.56, 3 = 0.20, 6 = 0.13, 7 = 0.18 and 8 = 0.34

NOTES

WEBS

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb Complificat is in \$290 lb uplift at joint 4 and 189 lb uplift at joint 5.

Truss Design Engineer Florida PE No. 34869 I 109 Coastal Bay Blvd Boynton Beach, FL 93435

January 9,2008

Scale = 1:36.2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occ. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	HJ9	MONO TRUSS	1	1		J1922686
		A CONTRACTOR OF THE CONTRACTOR			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:49 2008 Page 2

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-2(F=26, B=26)-to-4=-134(F=-40, B=-40), 8=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee Truss Design Engineer Florida PE No. 34869 1109 Coastal Bay Blvd Boynton Beach, FL 33435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	НЈ9А	SPECIAL	1	1		J1922687
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:30:00 2008 Page 1

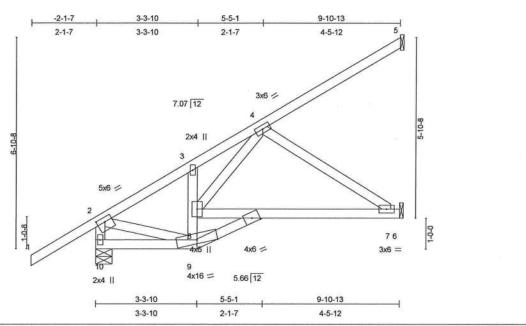


Plate Of	fsets (X,Y):	[2:0-3-0,0-1-12]										
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	1	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.12	7-8	>994	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.27	7-8	>427	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.44	Horz(TL)	-0.05	6	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Mati	rix)						Weight: 60 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 *Except* **WEBS**

3-9 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 10=399/0-6-7, 5=211/Mechanical, 6=290/Mechanical

Max Horz 10=462(load case 5)

Max Uplift 10=-80(load case 5), 5=-258(load case 5), 6=-220(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-10=-393/90, 1-2=0/62, 2-3=-281/0, 3-4=-371/8, 4-5=-167/78

BOT CHORD 9-10=-264/25, 7-8=-383/336, 6-7=0/0

WEBS 8-9=-67/0, 3-8=-164/0, 4-7=-409/467, 2-9=0/281, 4-8=0/231

JOINT STRESS INDEX

2 = 0.69, 3 = 0.11, 4 = 0.24, 7 = 0.14, 8 = 0.16, 9 = 0.13, 10 = 0.29 and 11 = 0.00

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 10, 258 lb uplift at joint 5 and 220 lb uplift at joint 6.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

January 9,2008

Scale = 1:35.4

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	НЈ9А	SPECIAL	1	1		J1922687
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:30:00 2008 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

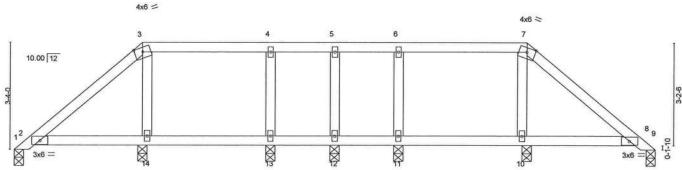
Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-2(F=26, B=26)-to-5=-134(F=-40, B=-40), 10=0(F=5, B=5)-to-9=-8(F=1, B=1), 8=-8(F=1, B=1)-to-6=-25(F=-7, B=-7)



lob	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME
SEAL TRUSSES	PB1	GABLE	1	1	Job Reference (optional)
Builders FirstSource	0. 950	22055 6.		MiTek Ir	ndustries, Inc. Thu Jan 03 16:39:50 2008 Page
Builders FirstSource	0. 950	2055 6.	300 s Feb 15 2006	MiTek Ir	



1	3-10-	4 4-1-12	8-0-0		10-0-0	12-0-0	-	15-1	0-4	16-0-0	20-0-0	
	3-10-	4 0-3-8	3-10-4	-	2-0-0	2-0-0		3-10)-4	0-1-12	4-0-0	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.01	2-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-14	>999	240	*(24.00777.78044.00)	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 81 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		6-0-0 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc
OTHERS	2 X 4 SYP No.3		bracing.

13=209/0-3-8, 9=58/0-3-8

Max Horz 1=90(load case 5)

Max Uplift 1=-20(load case 4), 14=-117(load case 5), 10=-77(load case 7), 12=-18(load case 5), 11=-99(load case 5), 13=-100(load case 4),

9=-26(load case 4)

Max Grav 1=79(load case 10), 14=329(load case 1), 10=329(load case 1), 12=73(load case 1), 11=230(load case 10), 13=230(load case 11), 9=79(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-82/88, 2-3=-57/144, 3-4=0/69, 4-5=0/69, 5-6=0/69, 6-7=0/69, 7-8=-55/144,

8-9=-45/20

BOT CHORD 2-14=-55/108, 13-14=-69/118, 12-13=-69/118, 11-12=-69/118, 10-11=-69/118,

8-10=-55/108

3-14=-270/181, 7-10=-270/181, 5-12=-55/47, 6-11=-198/123, 4-13=-198/123

Julius Lee Truss Ossian Chainser Floride FE No. 14889 1498 Chastel Bay Blod Boynton Beach, FL 33435

JOINT STRESS INDEX

WEBS

2 = 0.25, 3 = 0.47, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.47, 8 = 0.25, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33 and 14 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	0.2455457055
SEAL TRUSSES	PB1	GABLE	1	1	J19	22688
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:50 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 1, 117 lb uplift at joint 14, 77 lb uplift at joint 10, 18 lb uplift at joint 12, 99 lb uplift at joint 11, 100 lb uplift at joint 13 and 26 lb uplift at joint 9.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Ply Job Truss Type Qty AARON SIMQUE / GEORGIA HOME Truss J1922689 SEAL TRUSSES PB1A GABLE 1 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:51 2008 Page 1 6-0-0 14-0-0 20-0-0 6-0-0 8-0-0 6-0-0 Scale = 1:33.9 4x6 = 4x6 > 10.00 12 17 4-0-0 6-1-12 8-0-0 10-0-0 12-0-0 13-10-4 16-0-0 20-0-0 4-0-0 1-10-4 2-0-0 2-0-0 1-10-4 2-1-12 4-0-0 Plate Offsets (X,Y): [3:0-0-0,0-0-0] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES** GRIP in (loc) 20.0 -0.01 10-12 TCLL 1.25 TC >999 360 Plates Increase 0.12 Vert(LL) MT20 244/190 TCDL 1.25 BC >999 7.0 Lumber Increase 0.12 Vert(TL) -0.01 10-12 240 BCLL 10.0 Rep Stress Incr YES WB 0.05 Horz(TL) 0.01 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 103 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins. **WEBS** 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc **OTHERS** 2 X 4 SYP No.3 bracing. REACTIONS (lb/size) 1=100/0-3-8, 15=75/0-3-8, 12=288/0-3-8, 14=206/0-3-8, 18=288/0-3-8, 16=206/0-3-8, 11=100/0-3-8 Max Horz 1=136(load case 5) Max Uplift 1=-34(load case 4), 15=-55(load case 4), 12=-131(load case 7), 14=-55(load case 5), 18=-143(load case 6), 16=-67(load case 5) Max Grav 1=108(load case 10), 15=85(load case 11), 12=288(load case 1), 14=206(load case 1), 18=288(load case 1), 16=206(load case 1), 11=108(load case 11) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-140/134, 2-3=-101/91, 3-4=-78/68, 4-5=-14/73, 5-6=-14/74, 6-7=-14/74, 7-8=-14/73, 8-9=-78/68, 9-10=-92/63, 10-11=-62/1 **BOT CHORD** 2-18=-21/140, 17-18=-21/140, 16-17=-20/140, 15-16=-20/140, 14-15=-20/140, 13-14=-20/140, 12-13=-21/140, 10-12=-21/140 **WEBS** 4-17=-43/30, 8-13=-43/21, 6-15=-89/78, 9-12=-199/202, 7-14=-142/78,

JOINT STRESS INDEX

3-18=-199/202, 5-16=-142/78

2 = 0.35, 3 = 0.33, 4 = 0.24, 5 = 0.33, 6 = 0.33, 7 = 0.33, 8 = 0.24, 9 = 0.33, 10 = 0.35, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = January 9,2008 Continued $\frac{1}{100}$ $\frac{1}{1$





Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	CARCONIDOS SEO TRANSPORTO.
SEAL TRUSSES	PB1A	GABLE	1	1		J1922689
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:51 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 1, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 1, 55 lb uplift at joint 15, 131 lb uplift at joint 12, 55 lb uplift at joint 14, 143 lb uplift at joint 18 and 67 lb uplift at joint 16.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda ME No. 34868 1100 Ceastal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	14000000
SEAL TRUSSES	PB1B	GABLE	1	1		J1922690
			24		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:52 2008 Page 1

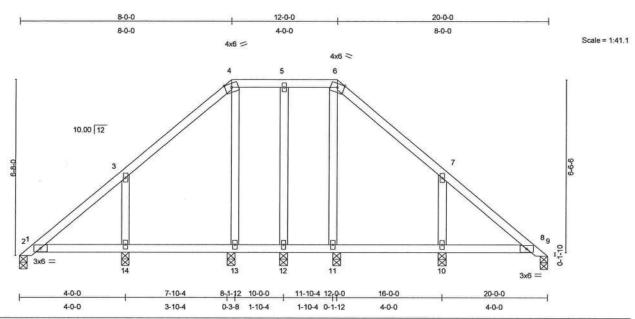


Plate Of	fsets (X,Y	′): [3:0-0-0,0-0-0]		,								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.01	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	8-10	>999	240	11 Vander of the cast that	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 99 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		6-0-0 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc
OTHERS	2 X 4 SYP No.3		bracing.

REACTIONS (lb/size) 1=70/0-3-8, 9=70/0-3-8, 13=239/0-3-8, 11=239/0-3-8, 12=73/0-3-8, 10=287/0-3-8, 14=287/0-3-8

Max Horz 1=-182(load case 4)

Max Uplift 1=-69(load case 4), 13=-74(load case 5), 11=-16(load case 4), 12=-49(load

case 5), 10=-168(load case 7), 14=-174(load case 6)

Max Grav 1=89(load case 5), 9=79(load case 11), 13=239(load case 1), 11=239(load case 1), 12=94(load case 11), 10=289(load case 11), 14=289(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-199/192, 2-3=-162/146, 3-4=-46/131, 4-5=0/155, 5-6=0/155, 6-7=-43/121,

7-8=-88/120, 8-9=-45/1

BOT CHORD 2-14=-37/130, 13-14=-37/130, 12-13=-42/132, 11-12=-42/132, 10-11=-37/130,

8-10=-37/130

WEBS 4-13=-211/101, 6-11=-211/79, 5-12=-84/50, 7-10=-227/262, 3-14=-227/262

Julius Lee Truss Design Engineer Flonds PE No. 34869 1109 Ceastal Bay Blvn Goynton Geach, FL 33436

JOINT STRESS INDEX

2 = 0.26, 3 = 0.33, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.33, 8 = 0.26, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33 and 14 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB1B	GABLE	1	1		J1922690
/					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:52 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 7) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 1, 74 lb uplift at joint 13, 16 lb uplift at joint 11, 49 lb uplift at joint 12, 168 lb uplift at joint 10 and 174 lb uplift at joint 14.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Les Trues Design Engineer Florida PE No. 24869 1109 Coastal Bay Blvd



20-20-999	ACCES.				
	- 1				J192269
SEAL TRUSSES PB	1C GA	ABLE	1	1	
AND THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY.	(Access)				Job Reference (optional)
Builders FirstSource, Lak	e City, FI 32055	6.300 s Feb 15	2006 N	MiTek In	dustries, Inc. Thu Jan 03 16:39:53 2008 Page 1

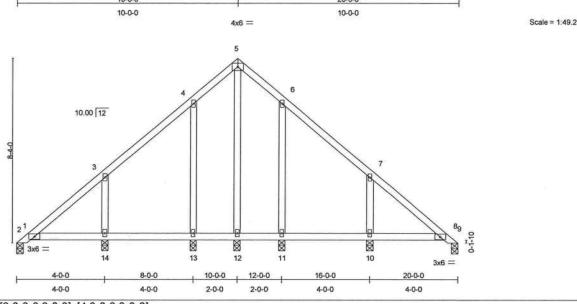


Plate Of	tsets (X, Y	<u>/): [3:0-0-0,0-0-0], [4:</u>	0-0-0,0-0-	.0]								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	8-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 103 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		6-0-0 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc
OTHERS	2 X 4 SYP No.3		bracing.

REACTIONS (lb/size) 1=65/0-3-8, 9=65/0-3-8, 12=143/0-3-8, 10=285/0-3-8, 11=210/0-3-8, 14=285/0-3-8, 13=210/0-3-8

Max Horz 1=228(load case 5)

Max Uplift 1=-79(load case 4), 10=-163(load case 7), 11=-127(load case 7),

14=-171(load case 6), 13=-127(load case 6)

Max Grav 1=127(load case 5), 9=74(load case 11), 12=162(load case 7), 10=288(load case 11), 11=210(load case 1), 14=288(load case 10), 13=210(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

 $1-2=-258/235,\ 2-3=-221/155,\ 3-4=-104/143,\ 4-5=-12/193,\ 5-6=-12/193,\ 6-7=-33/122,$ TOP CHORD

7-8=-162/129, 8-9=-42/0

BOT CHORD 2-14=-44/188, 13-14=-44/188, 12-13=-44/188, 11-12=-44/188, 10-11=-44/188,

8-10=-44/188

5-12=-196/0, 7-10=-226/262, 6-11=-183/212, 3-14=-226/262, 4-13=-183/212

JOINT STRESS INDEX

WEBS

2 = 0.25, 3 = 0.33, 4 = 0.33, 5 = 0.30, 6 = 0.33, 7 = 0.33, 8 = 0.25, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33 and 14 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	Mark Service Service Service
SEAL TRUSSES	PB1C	GABLE	1	1		J1922691
The thorn course of a transport of a transport	and was a pass				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:53 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

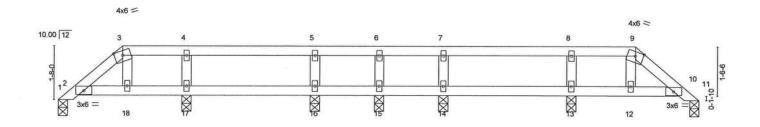
- 6) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 1, 163 lb uplift at joint 10, 127 lb uplift at joint 11, 171 lb uplift at joint 14 and 127 lb uplift at joint 13.
- 8) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonda FE No. 34869 1100 Coastal Bay Blvd Boynton Beach, FL 33435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	≣,
SEAL TRUSSES	PB1D	GABLE	1	1		J1922692
	Notice and the second		1 2		Job Reference (optional)	
Builders FirstSource	e, Lake City, FI	32055 6.	300 s Feb 15 2006	MiTek Ir	ndustries, Inc. Thu Jan 03 16:39:54 20	08 Page 1
2-0-0			18-0-0			0-0-0
2-0-0			16-0-0			2-0-0 Scale = 1:33 0



1	2-1-12	4-0-0	8-0-0		10-0-0	12-0-0		16-	0-0	1	7-10-4 20-0-0	
(i):	2-1-12	1-10-4	4-0-0		2-0-0	2-0-0		4-0	0-0	1	-10-4 2-1-12	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	0.01	18	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	12	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 69 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		6-0-0 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc
OTHERS	2 X 4 SYP No.3		bracing.

REACTIONS (lb/size) 1=121/0-3-8, 11=121/0-3-8, 15=65/0-3-8, 13=262/0-3-8, 14=215/0-3-8, 17=262/0-3-8, 16=215/0-3-8

Max Horz 1=-44(load case 4)

Max Uplift 1=-25(load case 6), 11=-28(load case 7), 15=-22(load case 5), 13=-98(load case 4), 14=-86(load case 5), 17=-106(load case 5), 16=-89(load case 4)

Max Grav 1=121(load case 1), 11=121(load case 1), 15=65(load case 1), 13=269(load case 11), 14=223(load case 10), 17=269(load case 10), 16=223(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-70/39, 2-3=-105/54, 3-4=-67/64, 4-5=-67/65, 5-6=-67/65, 6-7=-67/65,

7-8=-67/65, 8-9=-67/64, 9-10=-105/54, 10-11=-70/39

BOT CHORD 2-18=-17/67, 17-18=-15/67, 16-17=-15/67, 15-16=-15/67, 14-15=-15/67,

13-14=-15/67, 12-13=-15/67, 10-12=-15/67

WEBS 3-18=-4/23, 9-12=-4/23, 6-15=-50/34, 8-13=-209/142, 7-14=-191/137,

4-17=-209/142, 5-16=-191/137

JOINT STRESS INDEX

2 = 0.23, 3 = 0.24, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.33, 8 = 0.33, 9 = 0.24, 10 = 0.23, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33 and 18 = 0.33

Continued on page 2

January 9,2008



ulius Lee russ Design Engineer londs PE No. 34868 189 Ceastal Bay Blon cynton Besch, FL 33436

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB1D	GABLE	1	1		J1922692
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:54 2008 Page 2

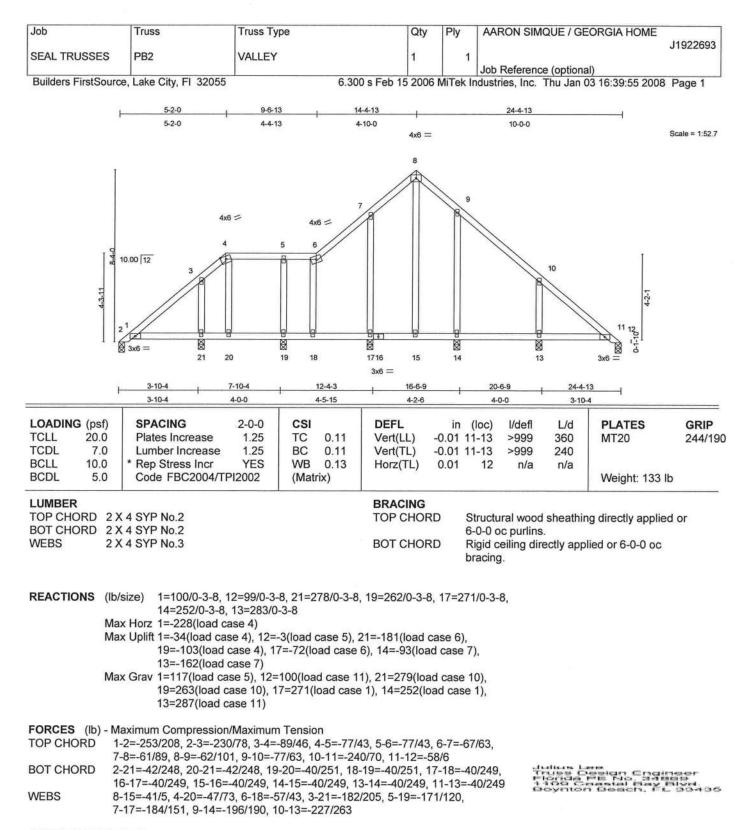
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 1, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1, 28 lb uplift at joint 11, 22 lb uplift at joint 15, 98 lb uplift at joint 13, 86 lb uplift at joint 14, 106 lb uplift at joint 17 and 89 lb uplift at joint 16.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34868 1100 Coestal Bay Blvd Boynton Besch, FL 33435





JOINT STRESS INDEX

2 = 0.35, 3 = 0.33, 4 = 0.24, 5 = 0.33, 6 = 0.29, 7 = 0.33, 8 = 0.30, 9 = 0.33, 10 = 0.33, 11 = 0.29, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.15, 17 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.33 and 21 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB2	VALLEY	1	1		J1922693
02 12 11100020	. 52	77.22.			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:55 2008 Page 2

NOTES

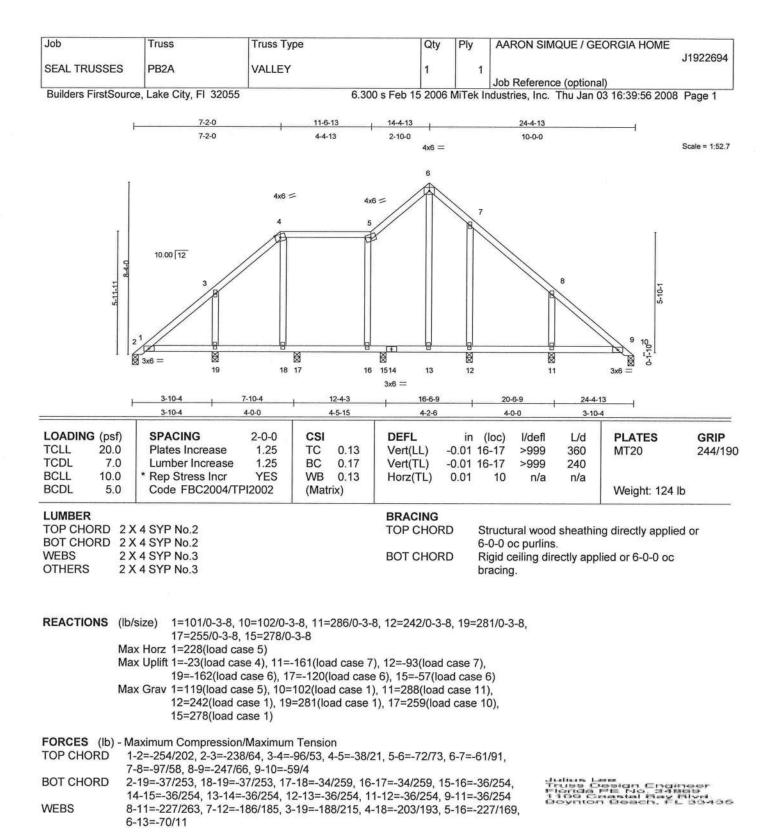
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 1, 3 lb uplift at joint 12, 181 lb uplift at joint 21, 103 lb uplift at joint 19, 72 lb uplift at joint 17, 93 lb uplift at joint 14 and 162 lb uplift at joint 13.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34868 I 109 Crastal Bay Blvd





JOINT STRESS INDEX

2 = 0.36, 3 = 0.33, 4 = 0.39, 5 = 0.68, 6 = 0.30, 7 = 0.33, 8 = 0.33, 9 = 0.29, 11 = 0.33, 12 = 0.33, 13 = 0.33, 14 = 0.23, 16 = 0.33, 18 = 0.33 and 19 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB2A	VALLEY	1	1		J1922694
		7/15-55-1			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:56 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 7) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1, 161 lb uplift at joint 11, 93 lb uplift at joint 12, 162 lb uplift at joint 19, 120 lb uplift at joint 17 and 57 lb uplift at joint 15.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida FE No. 34869 1169 Ceastel Bay Blvd Boynton Beach, Ft. 33435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	Michigano Koy Mana
SEAL TRUSSES	PB2B	GABLE	1	1		J1922695
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:57 2008 Page 1

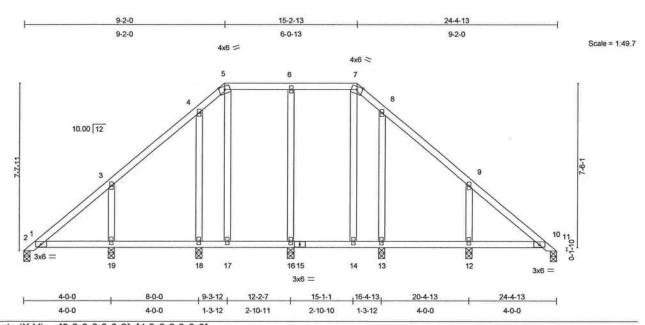


Plate Offsets (X,Y):	[3:0-0-0,0-0-0],	[4:0-0-0,0-0-0]
	- 100 - 100	

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.01	2-19	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-19	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.19	Horz(TL)	0.01	11	n/a	n/a		
BCDL	CDL 5.0 Code FBC2004/TPI2002		(Mat	rix)						Weight: 135 lb		

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=101/0-3-8, 11=101/0-3-8, 16=277/0-3-8, 12=284/0-3-8, 13=248/0-3-8,

19=284/0-3-8, 18=248/0-3-8

Max Horz 1=-209(load case 4)

Max Uplift 1=-70(load case 4), 11=-1(load case 5), 16=-91(load case 5), 12=-168(load case 7), 13=-76(load case 7), 19=-174(load case 6), 18=-95(load case 5)

Max Grav 1=111(load case 10), 11=111(load case 11), 16=277(load case 1), 12=288(load case 11), 13=248(load case 1), 19=288(load case 10),

18=248(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-229/215, 2-3=-182/144, 3-4=-97/133, 4-5=-77/164, 5-6=-19/138, 6-7=-19/138,

7-8=-77/164, 8-9=-97/60, 9-10=-166/67, 10-11=-64/5

BOT CHORD 2-19=-36/191, 18-19=-36/191, 17-18=-36/191, 16-17=-36/191, 15-16=-36/191,

14-15=-36/191, 13-14=-37/191, 12-13=-37/191, 10-12=-37/191

WEBS 5-17=-34/15, 7-14=-34/22, 6-16=-209/103, 9-12=-227/264, 8-13=-188/178,

3-19=-227/264, 4-18=-188/178

JOINT STRESS INDEX

NT STRESS INDEX
2 = 0.31, 3 = 0.33, 4 = 0.33, 5 = 0.24, 6 = 0.33, 7 = 0.24, 8 = 0.33, 9 = 0.33, 10 = 0.31, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 =

January 9,2008 Contline of the page 3217 = 0.33, 18 = 0.33 and 19 = 0.33

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB2B	GABLE	1	1		J1922695
	A STATE OF THE STA		ļ.		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:57 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 7) Bearing at joint(s) 1, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 1, 1 lb uplift at joint 11, 91 lb uplift at joint 16, 168 lb uplift at joint 12, 76 lb uplift at joint 13, 174 lb uplift at joint 19 and 95 lb uplift at joint 18.

9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda Fit No. 34569 1 100 Caestel Bay Blvd Boynton Besch, Ft. 33435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	14000000
SEAL TRUSSES	PB2C	GABLE	1	1		J1922696
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:58 2008 Page 1

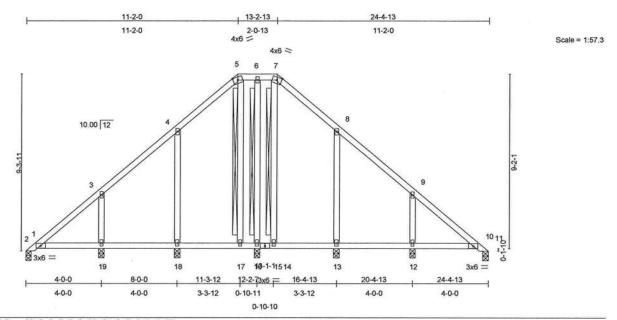


Plate Of	fsets (X,Y	(): [3:0-0-0,0-0-0], [4:	0-0-0,0-0-	0]								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	-0.01	2-19	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.01	2-19	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 145 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -5-17, 7-14, 6-16

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=91/0-3-8, 11=91/0-3-8, 16=298/0-3-8, 12=282/0-3-8, 13=251/0-3-8,

19=282/0-3-8, 18=251/0-3-8 Max Horz 1=-256(load case 4)

Max Uplift 1=-95(load case 4), 16=-30(load case 5), 12=-159(load case 7),

13=-149(load case 7), 19=-168(load case 6), 18=-150(load case 6)

Max Grav 1=141(load case 5), 11=95(load case 11), 16=298(load case 1),

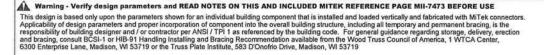
12=282(load case 1), 13=259(load case 11), 19=282(load case 1),

18=259(load case 10)

Julius Les Truss Design Engineer Flonda PE No. 34869 1109 Calestal Bay Blvd Boynton Besch, FL 33435

January 9,2008

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB2C	GABLE	1	1		J1922696
			100	1 22/	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:58 2008 Page 2

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-289/267, 2-3=-242/182, 3-4=-128/171, 4-5=-62/232, 5-6=0/226, 6-7=0/226, 7-8=-62/232, 8-9=-66/80,

9-10=-147/85, 10-11=-55/4

BOT CHORD 2-19=-36/177, 18-19=-36/177, 17-18=-36/177, 16-17=-35/177, 15-16=-35/177, 14-15=-35/177, 13-14=-35/177,

12-13=-35/177, 10-12=-35/177

WEBS 5-17=-84/46, 7-14=-84/0, 6-16=-100/18, 9-12=-223/257, 8-13=-210/243, 3-19=-223/257, 4-18=-210/243

JOINT STRESS INDEX

2 = 0.28, 3 = 0.33, 4 = 0.33, 5 = 0.24, 6 = 0.33, 7 = 0.24, 8 = 0.33, 9 = 0.33, 10 = 0.28, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.15, 16 = 0.33, 17 = 0.33, 18 = 0.33 and 19 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 7) Bearing at joint(s) 1, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 1, 30 lb uplift at joint 16, 159 lb uplift at joint 12, 149 lb uplift at joint 13, 168 lb uplift at joint 19 and 150 lb uplift at joint 18.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Line Truse Design Engineer Plonda PE No. 24889 I 109 Gestal Bay Blvd Boynton Besch, FL 99435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	100000000000000000000000000000000000000
SEAL TRUSSES	PB2D	GABLE	6	1		J1922697
02.2 11.00020	. 525	0,1522			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:59 2008 Page 1

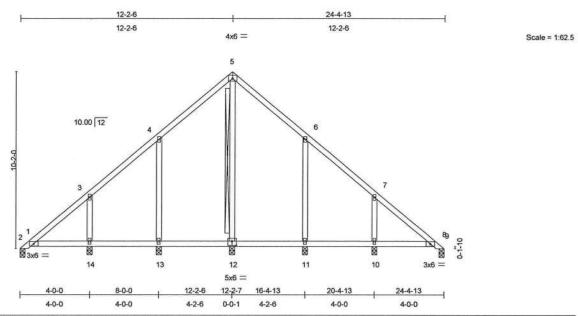


Plate Offsets (X,Y): [3:0-0-0,0-0-0], [4:0-0-0,0-0-0], [12:0-3-0,0-

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.01	2-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	2-14	>999	240	100	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.16	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	100 march 100 ma					Weight: 121 lb	

2 X 4 SYP No.2
2 X 4 SYP No.2
2 X 4 SYP No.3
2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS T-Brace: 2 X 4 SYP No.3 -

5-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=90/0-3-8, 9=90/0-3-8, 12=293/0-3-8, 10=281/0-3-8, 11=256/0-3-8, 14=281/0-3-8, 13=256/0-3-8

Max Horz 1=-279(load case 4)

Max Uplift 1=-102(load case 4), 9=-2(load case 5), 10=-153(load case 7),

11=-169(load case 7), 14=-163(load case 6), 13=-167(load case 6)

Max Grav 1=158(load case 5), 9=91(load case 11), 12=293(load case 1), 10=281(load case 1), 11=267(load case 11), 14=281(load case 1), 13=267(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-318/289, 2-3=-268/188, 3-4=-157/178, 4-5=-65/226, 5-6=-65/226, 6-7=-55/84,

7-8=-168/86, 8-9=-52/5

BOT CHORD 2-14=-39/195, 13-14=-39/195, 12-13=-39/195, 11-12=-39/195, 10-11=-39/195,

8-10=-39/195

WEBS 5-12=-248/12, 7-10=-218/250, 6-11=-232/267, 3-14=-219/250, 4-13=-232/267 Continued on page 2

January 9,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB2D	GABLE	6	1		J1922697
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:39:59 2008 Page 2

JOINT STRESS INDEX

2 = 0.29, 3 = 0.33, 4 = 0.33, 5 = 0.30, 6 = 0.33, 7 = 0.33, 8 = 0.29, 10 = 0.33, 11 = 0.33, 12 = 0.19, 13 = 0.33 and 14 = 0.33

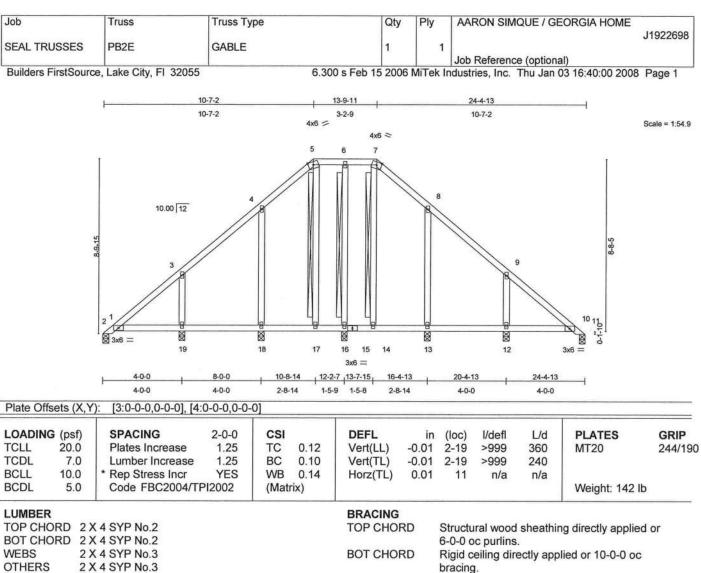
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 1, 2 lb uplift at joint 9, 153 lb uplift at joint 10, 169 lb uplift at joint 11, 163 lb uplift at joint 14 and 167 lb uplift at joint 13.
- 8) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Florida FE No. 24869 1109 Coastel Bay Blvd Boynton Beach, Ft. 33435





WEBS

T-Brace: 2 X 4 SYP No.3 -

5-17, 7-14, 6-16

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=106/0-3-8, 11=106/0-3-8, 16=268/0-3-8, 12=286/0-3-8, 13=248/0-3-8,

19=286/0-3-8, 18=248/0-3-8

Max Horz 1=-242(load case 4)

Max Uplift 1=-80(load case 4), 16=-49(load case 5), 12=-161(load case 7),

13=-131(load case 7), 19=-170(load case 6), 18=-135(load case 6)

Max Grav 1=130(load case 5), 11=108(load case 11), 16=268(load case 1),

12=286(load case 1), 13=253(load case 11), 19=286(load case 1),

18=253(load case 10)

January 9,2008

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB2E	GABLE	1	1		J1922698
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:01 2008 Page 2

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-272/247, 2-3=-226/158, 3-4=-110/146, 4-5=-81/201, 5-6=-14/191, 6-7=-14/191, 7-8=-81/201, 8-9=-90/52,

9-10=-165/59, 10-11=-62/4

BOT CHORD 2-19=-36/190, 18-19=-36/190, 17-18=-36/190, 16-17=-35/190, 15-16=-35/190, 14-15=-35/190, 13-14=-35/190,

12-13=-35/190, 10-12=-35/190

WEBS 5-17=-51/31, 7-14=-51/7, 6-16=-143/49, 9-12=-225/261, 8-13=-202/224, 3-19=-225/261, 4-18=-202/224

JOINT STRESS INDEX

2 = 0.30, 3 = 0.33, 4 = 0.33, 5 = 0.24, 6 = 0.33, 7 = 0.24, 8 = 0.33, 9 = 0.33, 10 = 0.30, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.15, 16 = 0.33, 17 = 0.33, 18 = 0.33 and 19 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 1, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 1, 49 lb uplift at joint 16, 161 lb uplift at joint 12, 131 lb uplift at joint 13, 170 lb uplift at joint 19 and 135 lb uplift at joint 18.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34868 1 100 Ceasial Bay Blvd Bovnton Beson, FL 33436



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGI	
SEAL TRUSSES	PB3	VALLEY	1	1		J1922699
Builders FirstSource,	Lake City, FI 32055	6.300 s	Apr 19 2006 N	/liTek Ind	Job Reference (optional) dustries, Inc. Wed Jan 09 13:52	2:44 2008 Page 1
-	3-10-5	11-0-14			19-8-0	1
	3-10-5 4x6	7-2-10			8-7-2	Scale = 1:42.
	2	3	4x6 ≈ 4			
10	.00 12	0	5			
1						
Z-1-15				3	6	\$P

		3-10-4	7-10-	4		11-9-12	15	5-9-12		19-8-0	1	
		3-10-4	4-0-0			3-11-8		4-0-0		3-10-4		
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.01	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	7-9	>999	240	1	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.01	8	n/a	n/a		

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

5.0

WEBS

2 X 4 SYP No.3

OTHERS

2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

Weight: 110 lb

REACTIONS (lb/size) 9=289/0-3-8, 12=250/0-3-8, 13=256/0-3-8, 10=226/0-3-8, 15=118/0-3-8,

8=103/0-3-8

Code FBC2004/TPI2002

Max Horz 15=-191(load case 4)

Max Uplift 9=-164(load case 7), 12=-107(load case 4), 13=-46(load case 5), 10=-119(load

(Matrix)

case 4), 15=-80(load case 6), 8=-45(load case 5)

Max Grav 9=289(load case 1), 12=262(load case 11), 13=256(load case 1), 10=231(load case 11), 15=121(load case 10), 8=103(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 14-15=-121/116, 1-14=-100/114, 1-2=-75/98, 2-3=-16/140, 3-4=-16/140, 4-5=-61/175,

5-6=-106/130, 6-7=-213/135, 7-8=-59/31

BOT CHORD 13-14=-100/202, 12-13=-98/203, 11-12=-98/203, 10-11=-98/202, 9-10=-98/202,

7-9=-98/202

WEBS 6-9=-227/265, 3-12=-220/123, 2-13=-217/110, 4-11=-41/16, 5-10=-183/155

JOINT STRESS INDEX

1 = 0.43, 2 = 0.38, 3 = 0.34, 4 = 0.25, 5 = 0.34, 6 = 0.34, 7 = 0.30, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34 and 14 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. Continued on page 2

January 9,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	14000000
SEAL TRUSSES	PB3	VALLEY	1	1		J1922699
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:52:44 2008 Page 2

NOTES

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MT20 unless otherwise indicated.

- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 15, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 9, 107 lb uplift at joint 12, 46 lb uplift at joint 13, 119 lb uplift at joint 10, 80 lb uplift at joint 15 and 45 lb uplift at joint 8.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

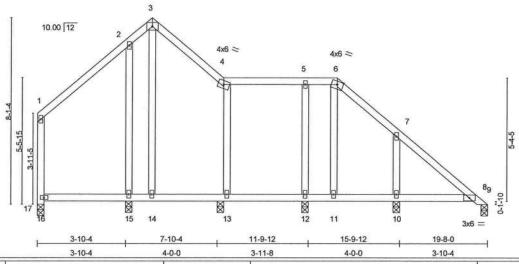
Julius Lee Truse Design Engineer Flonds PE No. 34869 1109 Ceastel Bay Blvd Bovnton Besch, FL 33435



lob	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME
	K	7		0.0480	J1922700
SEAL TRUSSES	PB3A	VALLEY	1	1	
					Job Reference (optional)



Scale = 1:47.3



LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.18	Vert(LL)	-0.01	8-10	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.01	8-10	>999	240	100000000000000000000000000000000000000	
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0		Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 116 lb	

L			

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 **WEBS OTHERS** 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS 10=285/0-3-8, 15=246/0-3-8, 13=252/0-3-8, 12=224/0-3-8, 17=129/0-3-8, (lb/size)

9=105/0-3-8

Max Horz 17=-217(load case 4)

Max Uplift 10=-149(load case 7), 13=-77(load case 7), 12=-103(load case 4), 17=-96(load

case 6), 9=-65(load case 5)

Max Grav 10=286(load case 11), 15=246(load case 1), 13=253(load case 11), 12=228(load case 11), 17=135(load case 10), 9=117(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 16-17=-135/144, 1-16=-114/133, 1-2=-85/112, 2-3=-70/192, 3-4=-84/163, 4-5=-81/114,

5-6=-81/114, 6-7=-135/143, 7-8=-235/162, 8-9=-69/43

BOT CHORD 15-16=-123/223, 14-15=-123/223, 13-14=-123/223, 12-13=-120/224, 11-12=-120/224,

10-11=-119/222, 8-10=-119/222

7-10=-205/217, 2-15=-187/117, 3-14=-126/52, 4-13=-211/212, 6-11=-58/74, 5-12=-169/107333 **WEBS**

JOINT STRESS INDEX

1 = 0.53, 2 = 0.34, 3 = 0.31, 4 = 0.35, 5 = 0.34, 6 = 0.25, 7 = 0.34, 8 = 0.35, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 0.34 and 16 = 0.42

NOTES

1) Unbalanced roof live loads have been considered for this design.

January 9,2008

Continued on page 2

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	РВЗА	VALLEY	1	1		J1922700
kee					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:32:42 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 17, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 10, 77 lb uplift at joint 13, 103 lb uplift at joint 12, 96 lb uplift at joint 17 and 65 lb uplift at joint 9.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 24869 1100 Coastel Bay Blyd Boynton Beach, FL 33436



Job	Truss	Truss Type		Qty	Ply	AARON SI	MQUE / GEORGIA HOME	J1922701
SEAL TRUSSES	РВ3В	VALLEY		1	1	164 - 1600 - 1500 - 1500 - 1500		31922701
Builders FirstSource	e, Lake City, FI 320	55	6.300	s Apr 19 2006	MiTek Inc	Job Referer lustries, Inc.	nce (optional) Wed Jan 09 13:33:24 2008	Page 1
	-102	23						
	4-11		10-1-8 5-1-10		-0-14 -11-6		19-8-0 4-7-2	
		4x6 =	- / /-	,				Scale = 1:47
	2.	3						
	10.00 12	2						
		1						
		Z -	1 4					
		11 11	B	4x6 <		4x6 ≥		
	1 //	11 11		. 5		400		
	4 1 1			5 6		7	Ţ	
	1 1						I	
	8-14 11-5 11-5 11-5 11-5						ु. जु.	
	8-14-8 3-11-8 1						3-8-5	
	(a)	2 Q		6		7		
		16 15	B 88 14	6			89 07	
	(a)	16 15 7-10-	14	6	15-9-12	7		
	18 8	16 15	14	13 12	15-9-12 4-0-0	7	3x6 =	
LOADING (psf)	18 3-10-4 3-10-4 SPACING	7-10- 4-0-0 2-0-0	14 4 CSI	13 12 11-9-12 3-11-8	4-0-0 in (I	7 11 10 11 10 1/defl	3x6 = 19-8-0 3-10-4 L/d PLATES	GRIP
L OADING (psf)	3-10-4 3-10-4 SPACING Plates Increase	2-0-0 1.25	14 4 0 CSI TC 0.18	13 12 11-9-12 3-11-8 DEFL Vert(LL)	4-0-0 in (I -0.01 11-	7 11 10 0c) I/defl -12 >999	3x6 = 19-8-0 3-10-4 L/d PLATES 360 MT20	GRIP 244/190
. OADING (psf) FCLL 20.0 FCDL 7.0 BCLL 10.0	18 3-10-4 3-10-4 SPACING	2-0-0 1.25 e 1.25	14 4 CSI	13 12 11-9-12 3-11-8 DEFL Vert(LL)	4-0-0 in (I	7 11 10 0c) I/defl -12 >999	3x6 = 19-8-0 3-10-4 L/d PLATES	

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 WFBS

OTHERS

2 X 4 SYP No.3

TOP CHORD Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 16=240/0-3-8, 14=235/0-3-8, 12=262/0-3-8, 18=131/0-3-8, 9=115/0-3-8,

10=259/0-3-8

Max Horz 18=-217(load case 4)

Max Uplift 14=-89(load case 7), 12=-143(load case 5), 18=-94(load case 6), 9=-59(load case

5), 10=-162(load case 4)

Max Grav 16=240(load case 1), 14=242(load case 11), 12=262(load case 1), 18=135(load

case 10), 9=115(load case 1), 10=266(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 17-18=-135/145, 1-17=-113/133, 1-2=-84/113, 2-3=-67/193, 3-4=-79/165, 4-5=-130/152,

5-6=-120/115, 6-7=-120/116, 7-8=-208/152, 8-9=-67/40

BOT CHORD 16-17=-122/220, 15-16=-122/220, 14-15=-122/220, 13-14=-122/220, 12-13=-120/219,

11-12=-120/219, 10-11=-117/211, 8-10=-117/211

11-12=-120/219, 10-11=-117/211, 8-10=-117/211 2-16=-182/117, 4-14=-177/165, 3-15=-121/50, 5-13=-54/57, 7-11=-204/198, 6-12=-159/180/2013 FEE FIG. 2318-254/57 **WEBS**

JOINT STRESS INDEX

1 = 0.54, 2 = 0.34, 3 = 0.31, 4 = 0.34, 5 = 0.30, 6 = 0.34, 7 = 0.29, 8 = 0.59, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 0.34 and 17 = 0.47

1) Unbalanced roof live loads have been considered for this design.

January 9,2008

Continued on page 2

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	KONS CALESTON SERVIN
SEAL TRUSSES	PB3B	VALLEY	1	1		J1922701
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:33:24 2008 Page 2

NOTES

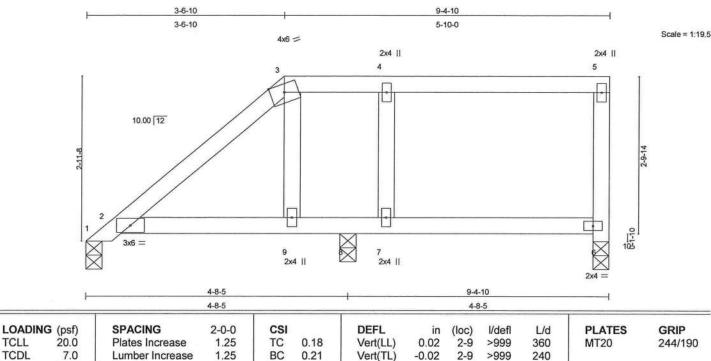
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 18, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 14, 143 lb uplift at joint 12, 94 lb uplift at joint 18, 59 lb uplift at joint 9 and 162 lb uplift at joint 10.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida ME No. 34866 1 100 Gastal Bay Blyd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME
					J1922702
SEAL TRUSSES	PB4	GABLE	1	1	
			120		Job Reference (optional)
Builders FirstSource	e, Lake City, FI 32	2055 6.3	300 s Apr 19 2006 N	liTek Ind	lustries, Inc. Wed Jan 09 13:34:05 2008 Page 1



LUMBER

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

10.0

5.0

BRACING

Horz(TL)

0.01

10

TOP CHORD Structural wood sheathing directly applied or 6-0-0

n/a

Weight: 39 lb

oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

REACTIONS (lb/size) 1=132/0-3-8, 10=131/0-3-8, 8=320/0-3-8

Max Horz 1=92(load case 6)

Rep Stress Incr

Code FBC2004/TPI2002

Max Uplift 10=-61(load case 4), 8=-101(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-101/0, 2-3=-78/26, 3-4=-29/19, 4-5=-29/20, 6-10=-131/93, 5-6=-101/97

YES

BOT CHORD 2-9=-27/35, 8-9=-20/29, 7-8=-20/29, 6-7=-20/29

WEBS 3-9=-99/125, 4-7=-172/181

JOINT STRESS INDEX

2 = 0.32, 3 = 0.15, 4 = 0.10, 5 = 0.47, 6 = 0.42, 7 = 0.10 and 9 = 0.07

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

WB

(Matrix)

0.05

- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 10 and 101 lb uplift at joint 8. Continued on page 2

January 9,2008

👠 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult 8CS-1 or HIL9-11 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4	GABLE	1	1		J1922702
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:34:05 2008 Page 2

NOTES

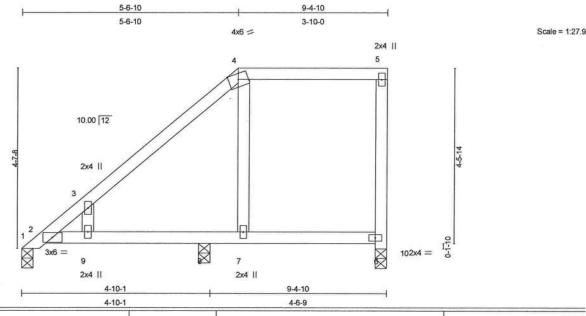
7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4A	MONO HIP PIGGYBACK	1	1		J1922703
		7455 A S & C			Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:34:31 2008 Page 1



LOADIN		SPACING	2-0-0	CSI	0.45	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	0.03	9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.03	9	>999	240	0.535.07.00	
BCLL	10.0	 Rep Stress Incr 	YES	WB	0.07	Horz(TL)	-0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mati	rix)	50-111-001-00-00-011-020		31.45.15	R.F.A.TA		Weight: 42 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=137/0-3-8, 10=135/0-3-8, 8=311/0-3-8

Max Horz 1=146(load case 6)

Max Uplift 10=-59(load case 4), 8=-149(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-199/0, 2-3=-256/25, 3-4=-94/44, 4-5=-20/15, 6-10=-135/102, 5-6=-87/81

BOT CHORD 2-9=-24/26, 8-9=-24/26, 7-8=-24/26, 6-7=-15/19

WEBS 4-7=-212/260, 3-9=-171/240

JOINT STRESS INDEX

2 = 0.09, 3 = 0.12, 4 = 0.49, 5 = 0.40, 6 = 0.48, 7 = 0.15 and 9 = 0.14

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula.
 Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 10 and 149 lb uplift at joint 8. Continued on page 2

Truss Design Engineer Florida PE No. 34899 1109 Coastal Bay Blyd Boynton Besch. FL 33435

January 9,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4A	MONO HIP PIGGYBACK	1	1		J1922703
AMERICANO, TANAMARKANIA					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:34:31 2008 Page 2

NOTES

7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

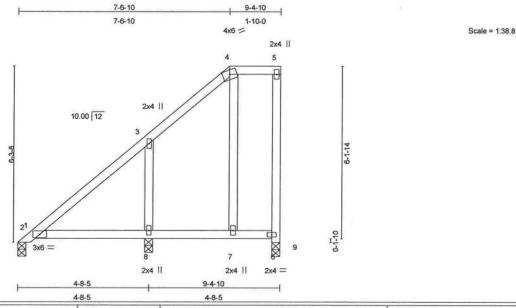
LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonds PE No. 34869 1109 Coestal Bay Blyd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4B	GABLE	1	1		J1922704
			1.70		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:05 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.02	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.02	2-8	>999	240	75.53	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.08	Horz(TL)	-0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 52 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

OTHERS 2 X 4

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=121/0-3-8, 8=342/0-3-8, 9=120/0-3-8

Max Horz 1=199(load case 6)

Max Uplift 8=-202(load case 6), 9=-38(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-312/0, 2-3=-268/70, 3-4=-87/5, 4-5=-9/7, 6-9=-120/103, 5-6=-68/62

BOT CHORD 2-8=-10/10, 7-8=-10/10, 6-7=-8/8

WEBS 4-7=-55/86, 3-8=-243/311

JOINT STRESS INDEX

2 = 0.48, 3 = 0.15, 4 = 0.06, 5 = 0.24, 6 = 0.31, 7 = 0.05 and 8 = 0.17

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

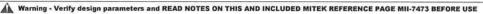
2) Provide adequate drainage to prevent water ponding.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Truse Design Engineer Florida PE No. 34868 1100 Caestel Bay Blvd Boynton Beach, FL 53436





Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4B	GABLE	1	1		J1922704
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:05 2008 Page 2

NOTES

- 5) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 8 and 38 lb uplift at joint 9.
- 7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

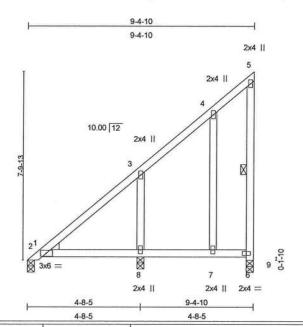
LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Florida PE No. 34869 1100 Ceastal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4C	GABLE	1	1		J1922705
	A second second			- 2	Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:35:11 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.02	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.02	2-8	>999	240	With the second second	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.08	Horz(TL)	-0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Mat	rix)	SOURCE STATE OF STATE					Weight: 56 lb	

JM		

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 **OTHERS** 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

BRACING

WEBS

TOP CHORD Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals. **BOT CHORD**

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

5-9

REACTIONS (lb/size) 1=117/0-3-8, 8=349/0-3-8, 9=116/0-3-8

Max Horz 1=243(load case 6)

Max Uplift 8=-216(load case 6), 9=-74(load case 6)

Max Grav 1=130(load case 6), 8=349(load case 1), 9=116(load case 1)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-398/0, 2-3=-357/75, 3-4=-130/9, 4-5=-49/34, 6-9=-116/123, 5-6=-61/73 **BOT CHORD** 2-8=-7/6, 7-8=-7/6, 6-7=-7/6

WEBS 3-8=-247/318, 4-7=-61/101

JOINT STRESS INDEX

2 = 0.48, 3 = 0.16, 4 = 0.05, 5 = 0.31, 6 = 0.30, 7 = 0.06 and 8 = 0.18

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

January 9,2008

Scale = 1:45.1

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4C	GABLE	1	1		J1922705
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:35:11 2008 Page 2

NOTES

- 4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216 lb uplift at joint 8 and 74 lb uplift at joint
- 6) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4D	GABLE	1	1		J1922706
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:35:48 2008 Page 1

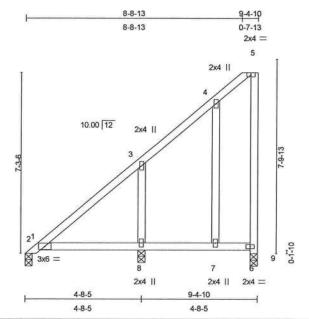


Plate Offsets (X,Y): [5:0-0-0,0-0-0]												
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.02	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.02	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.08	Horz(TL)	-0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Mat	rix)	3.76				11.0.50	Weight: 54 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 **WEBS OTHERS** 2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=118/0-3-8, 8=348/0-3-8, 9=117/0-3-8

Max Horz 1=236(load case 6)

Max Uplift 8=-216(load case 6), 9=-68(load case 6)

Max Grav 1=123(load case 6), 8=348(load case 1), 9=117(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-384/0, 2-3=-342/74, 3-4=-116/9, 4-5=-37/30, 6-9=-117/114, 5-6=-62/65

BOT CHORD 2-8=-7/7, 7-8=-7/7, 6-7=-7/7 **WEBS** 3-8=-247/318, 4-7=-60/99

JOINT STRESS INDEX

2 = 0.48, 3 = 0.16, 4 = 0.05, 5 = 0.57, 6 = 0.30, 7 = 0.06 and 8 = 0.18

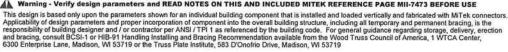
NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building Coffee ignershould yerify capacity of bearing surface.

January 9,2008

Scale = 1:43.8

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4D	GABLE	1	1		J1922706
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:35:48 2008 Page 2

NOTES

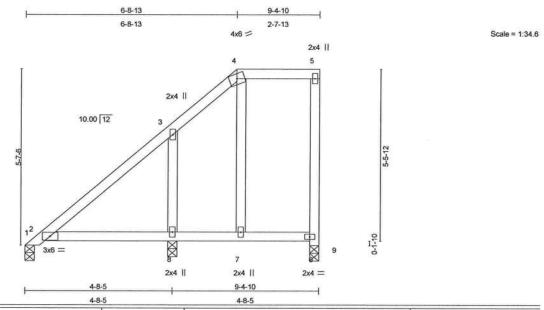
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216 lb uplift at joint 8 and 68 lb uplift at joint
- 6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4E	GABLE	1	1		J1922707
	HOS the		0	820	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:06 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.02	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.02	2-8	>999	240	a traveración de la constante	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Mat	rix)	31. 15					Weight: 50 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

OTHERS 2 X 4 SYP No.3 BRACING

BOT CHORD

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=123/0-3-8, 8=339/0-3-8, 9=121/0-3-8

Max Horz 1=177(load case 6)

Max Uplift 8=-190(load case 6), 9=-44(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-269/0, 2-3=-224/67, 3-4=-83/7, 4-5=-10/7, 6-9=-121/95, 5-6=-84/80

BOT CHORD 2-8=-10/11, 7-8=-10/11, 6-7=-8/10

WEBS 4-7=-47/70, 3-8=-232/292

JOINT STRESS INDEX

2 = 0.50, 3 = 0.14, 4 = 0.06, 5 = 0.28, 6 = 0.28, 7 = 0.04 and 8 = 0.16

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 9,2008

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4E	GABLE	1	1	3	J1922707
			20		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:07 2008 Page 2

NOTES

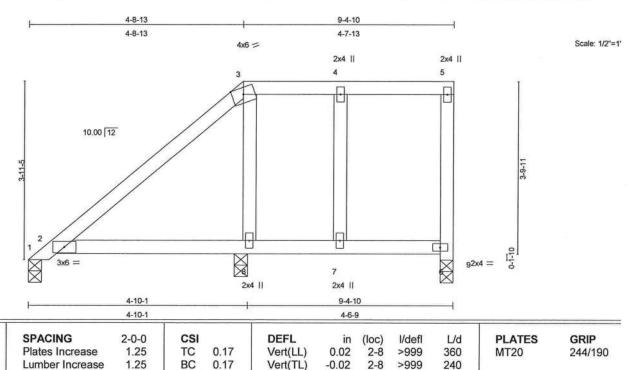
- 5) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 8 and 44 lb uplift at joint 9.
- 7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34888 1 109 Coastal Bay Blvd



J1922708
GYBACK 1 1 Let Defended (author)
Job Reference (optional)
;



LUMBER

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

20.0

7.0

10.0

5.0

BRACING TOP CHORD

Horz(TL)

0.01

9

Structural wood sheathing directly applied or 6-0-0

Weight: 44 lb

oc purlins, except end verticals.

n/a

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

REACTIONS

(lb/size) 1=132/0-3-8, 8=333/0-3-8, 9=118/0-3-8

Max Horz 1=124(load case 6)

Rep Stress Incr

Code FBC2004/TPI2002

Max Uplift 8=-138(load case 6), 9=-64(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-160/0, 2-3=-108/55, 3-4=-14/12, 4-5=-13/13, 6-9=-118/81, 5-6=-81/73

YES

WB

(Matrix)

0.07

BOT CHORD 2-8=-18/23, 7-8=-13/13, 6-7=-13/13

WEBS 3-8=-221/263, 4-7=-52/59

JOINT STRESS INDEX

2 = 0.53, 3 = 0.52, 4 = 0.03, 5 = 0.27, 6 = 0.26, 7 = 0.03 and 8 = 0.15

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 8 and 64 lb uplift at joint 9.

January 9,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4F	MONO HIP PIGGYBACK	1	1		J1922708
			- AS	20	Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:36:32 2008 Page 2

7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

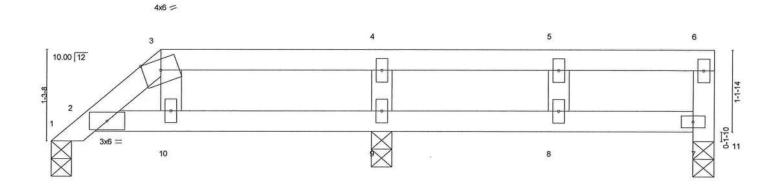


Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4G	GABLE	1	1		J1922709
		33. 33.			Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:36:59 2008 Page 1

9-4-10





	4-8-5								4-8	-5		
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.01	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.01	10	>999	240	000000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	1 (1.50000 11.00 (1.5000 11.00 (1.5000 11.00 (1.5000 11.00 (1.5000 11.00 (1.5000 11.00 (1.5000 11.00 (1.5000 1					Weight: 31 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0
BOT CHORD	2 X 4 SYP No.2		oc purlins, except end verticals.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2 X 4 SYP No.3		, , , , ,

REACTIONS (lb/size) 1=143/0-3-8, 9=298/0-3-8, 11=142/0-3-8

4-8-5

Max Horz 1=39(load case 6)

Max Uplift 1=-23(load case 5), 9=-100(load case 4), 11=-52(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-83/8, 2-3=-156/85, 3-4=-119/92, 4-5=-119/93, 5-6=-119/93, 7-11=-142/107, 6-7=-90/80

BOT CHORD 2-10=-101/121, 9-10=-93/119, 8-9=-93/119, 7-8=-93/119

WEBS 3-10=-15/51, 4-9=-211/201, 5-8=-51/69

JOINT STRESS INDEX

2 = 0.20, 3 = 0.08, 4 = 0.11, 5 = 0.04, 6 = 0.58, 7 = 0.61, 8 = 0.04, 9 = 0.11 and 10 = 0.04

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Truss Design Engineer Flonds FE No. 34865 1109 Casstal Bay Blyd Boynton Beach, FL 33435

January 9,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB4G	GABLE	1	1		J1922709
02 12 11100020	1.010	0,1022			Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:36:59 2008 Page 2

NOTES

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1, 100 lb uplift at joint 9 and 52 lb uplift at joint 11.
- 8) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB5	PIGGYBACK	11	1		J1922710
SEAL TRUSSES	FBS	TIGGTBACK 3x6	= 11	1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:08 2008 Page 1

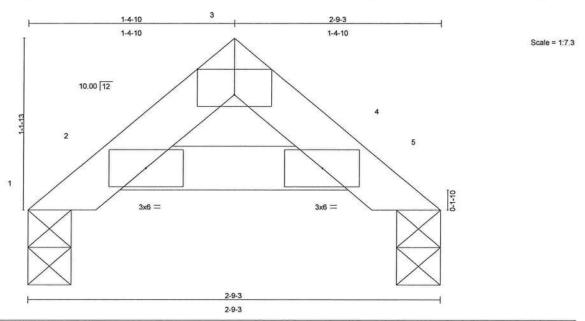


Plate Of	fsets (X, Y	'): [3:0-3-0,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.07	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.00	2	>999	240	10.000.0000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	**************************************					Weight: 8 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

2-9-3 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=88/0-3-8, 5=88/0-3-8

Max Horz 1=-31(load case 4)

Max Uplift 1=-16(load case 6), 5=-16(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-51/35, 2-3=-93/49, 3-4=-93/49, 4-5=-51/35

BOT CHORD 2-4=-13/87

JOINT STRESS INDEX

2 = 0.09, 3 = 0.07 and 4 = 0.09

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Continuous continuous surface.

Truse Design Engineer Flonda FE No. 34868 I 100 Chastel Bay Blyd Boynton Besch, FL 93436

January 9,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building sock. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	PB5	PIGGYBACK	11	1		J1922710
			12-10		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:08 2008 Page 2

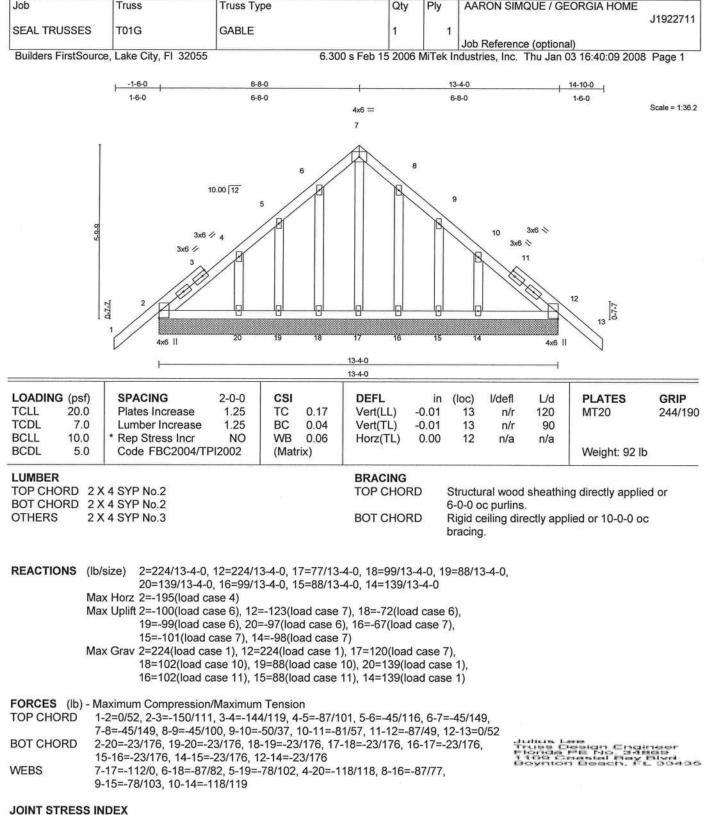
NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 1 and 16 lb uplift at joint 5.
- 7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonda PE No. 34866 1109 Geastal Bay Blvd.





2 = 0.51, 3 = 0.00, 3 = 0.16, 3 = 0.16, 4 = 0.07, 5 = 0.05, 6 = 0.05, 7 = 0.08, 8 = 0.05, 9 = 0.05, 10 = 0.07, 11 = 0.00, 11 = 0.16, 11 = 0.16, 12 = 0.51, 14 = 0.07, 15 = 0.06, 16 = 0.05, 17 = 0.03, 18 = 0.05, 19 = 0.06 and 20 = 0.07

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T01G	GABLE	1	1	J1922	2711
021217100020	10.0	O' IDEE	1.		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:09 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.

8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint 2, 123 lb uplift at joint 12, 72 lb uplift at joint 18, 99 lb uplift at joint 19, 97 lb uplift at joint 20, 67 lb uplift at joint 16, 101 lb uplift at joint 15 and 98 lb uplift at joint 14.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

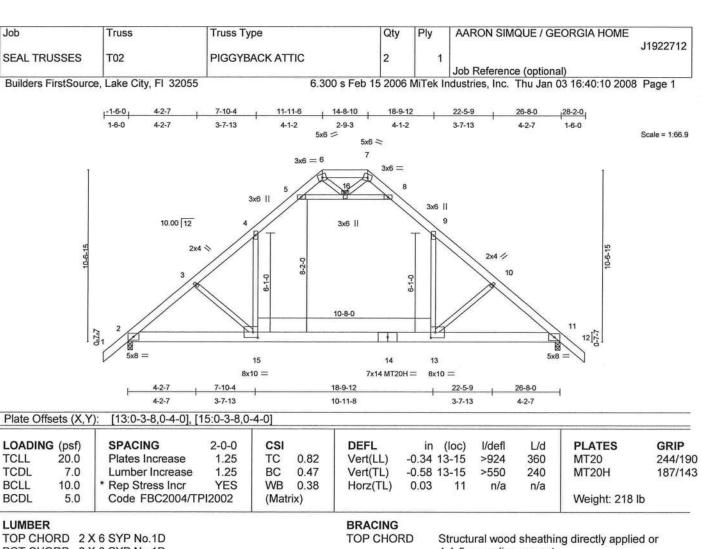
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-7=-64(F=-10), 7-13=-64(F=-10), 2-12=-10

Julius Lee Truse Design Engineer Florida PE No. 24869 1100 Geestal Bay Blvd





BOT CHORD 2 X 8 SYP No.1D

WEBS

2 X 4 SYP No.3

4-1-5 oc purlins, except

2-0-0 oc purlins (10-0-0 max.): 6-7.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS

1 Row at midpt

5-8

REACTIONS (lb/size) 11=1601/0-3-8, 2=1601/0-3-8

Max Horz 2=276(load case 5)

Max Uplift 11=-71(load case 7), 2=-71(load case 6)

TOP CHORD

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/53, 2-3=-2310/250, 3-4=-2112/234, 4-5=-1303/297, 5-6=0/488, 7-8=0/488,

8-9=-1303/297, 9-10=-2112/234, 10-11=-2311/250, 11-12=0/53, 6-7=0/699 2-15=-29/1711, 14-15=0/1404, 13-14=0/1404, 11-13=-29/1711

BOT CHORD WEBS

4-15=-1/1051, 9-13=-1/1051, 5-16=-2151/359, 8-16=-2151/359, 3-15=-440/237,

10-13=-440/237, 6-16=-102/163, 7-16=-102/163

JOINT STRESS INDEX

2 = 0.46, 3 = 0.33, 4 = 0.33, 5 = 0.74, 6 = 0.52, 7 = 0.52, 8 = 0.74, 9 = 0.33, 10 = 0.33, 11 = 0.46, 13 = 0.20, 14 = 0.38, 15 = 0.380.20 and 16 = 0.40

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

ius Lee lee Design Engineer rida PE No. 24888 99 Coestal Bay Blvd vinton Besch, FL 23425



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	Name and the same and the
SEAL TRUSSES	T02	PIGGYBACK ATTIC	2	1		J1922712
			-		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:11 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-16, 8-16; Wall dead load (5.0 psf) on member(s).4-15, 9-13
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 11 and 71 lb uplift at joint 2.

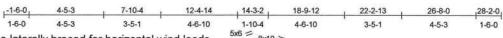
LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonds ME No. 34868 I 100 Coestel Bay Blvd Boymon Beach, FL 93435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T02G	GABLE	1	1		J1922713
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:38:48 2008 Page 1



8x10 =

+ Member must be laterally braced for horizontal wind loads.

Scale = 1:64.8

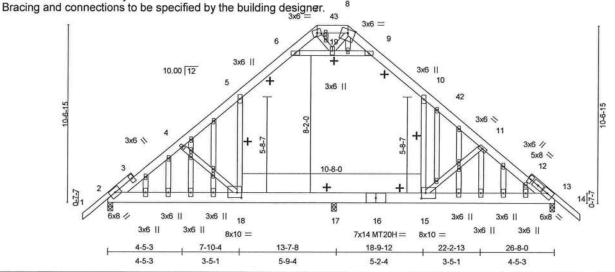


Plate Of	fsets (X,Y):	[15:0-3-1,0-3-14], [1	8:0-3-3,0-3	3-10]						101		
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.17	13-15	>915	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.24	13-15	>644	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.02	13	n/a	n/a	1471216/8000 CHELL	115570AUT. 1517AU
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	1860					Weight: 250 II	0

LUMBER

TOP CHORD 2 X 6 SYP No.1D *Except*

1-3 2 X 4 SYP No.2, 12-14 2 X 4 SYP No.2

BOT CHORD 2 X 8 SYP No.1D

WEBS 2 X 4 SYP No.3 **OTHERS** 2 X 4 SYP No.3 BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins, except

2-0-0 oc purlins (10-0-0 max.): 7-8.

BOT CHORD

Rigid ceiling directly applied or 9-1-14 oc bracing.

REACTIONS (lb/size) 2=1159/0-3-8, 13=1123/0-3-8, 17=1308/0-3-8

Max Horz 2=-354(load case 4)

Max Uplift 2=-350(load case 6), 13=-338(load case 7), 17=-56(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/59, 2-3=-1436/313, 3-4=-1394/321, 4-5=-1224/288, 5-6=-894/351, 6-7=-6/118,

8-9=-46/141, 9-10=-942/354, 10-42=-1084/293, 11-42=-1216/295, 11-12=-1367/323,

12-13=-1404/307, 13-14=0/50, 7-43=-19/193, 8-43=-19/193

BOT CHORD 2-18=-315/1119, 17-18=-149/795, 16-17=-149/795, 15-16=-149/795, 13-15=-140/1108

WEBS 5-18=-7/294, 10-15=0/214, 6-19=-1040/452, 9-19=-979/417, 4-18=-463/300,

11-15=-447/304, 7-19=-192/200, 8-19=-130/215

JOINT STRESS INDEX

2 = 0.83, 3 = 0.00, 3 = 0.67, 4 = 0.34, 5 = 0.16, 6 = 0.37, 7 = 0.36, 8 = 0.15, 8 = 0.00, 9 = 0.35, 10 = 0.16, 11 = 0.63, 12 = 0.27, 12 = 0.27, 12 = 0.270.34, 12 = 0.28, 13 = 0.79, 15 = 0.11, 16 = 0.11, 18 = 0.10, 19 = 0.36, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 23 = 0.34, 24 = 0.325 = 0.16, 26 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.16, 29 = 0.16, 30 = 0.34, 31 = 0.16, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 360.16, 37 = 0.34, 37 = 0.34, 38 = 0.16, 39 = 0.16, 40 = 0.34 and 41 = 0.16

NOTES

1) Unbalanced roof live loads have been considered for this design.

January 9,2008

Continued on page 2

航 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	A MANAGER AND A MARKET AND A MA
SEAL TRUSSES	T02G	GABLE	1	1		J1922713
		2 1/1/2 1/2/2		- 3	Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:38:48 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) Ceiling dead load (5.0 psf) on member(s). 5-6, 9-10, 6-19, 9-19; Wall dead load (5.0psf) on member(s).5-18, 10-15
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 17-18, 15-17
- 11) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 350 lb uplift at joint 2, 338 lb uplift at joint 13 and 56 lb uplift at joint 17.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 14) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 2-18=-10, 15-18=-110, 13-15=-10, 1-5=-64(F=-10), 5-6=-74(F=-10), 6-7=-64(F=-10), 8-9=-91(F=-37), 9-10=-101(F=-37), 10-42=-91(F=-37), 14-42=-54, 7-43=-64(F=-10), 8-43=-91(F=-37), 6-9=-10

Drag: 5-18=-10, 10-15=-10

Julius Lee Truss Design Engineer Flonda PE No. 34868 1109 Coasial Bay Blyd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T03	PIGGYBACK ATTIC	9	1		J1922714
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:14 2008 Page 1

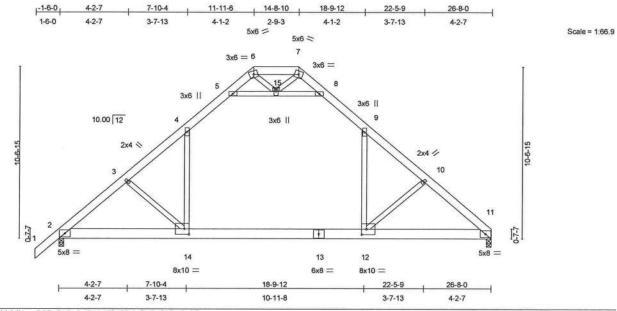


Plate Offsets (X,Y): [12:0-3-8,0-4-0], [14:0-3-8,0-4-0] LOADING (psf) **SPACING** 2-0-0 CSI DEFL **PLATES** GRIP in (loc) I/defl L/d TCLL 20.0 Plates Increase 1.25 TC 0.82 Vert(LL) -0.34 12-14 >923 360 MT20 244/190 7.0 TCDL 1.25 BC Lumber Increase 0.47 Vert(TL) -0.58 12-14 >550 240 BCLL 10.0 Rep Stress Incr YES WB 0.38 Horz(TL) 0.03 11 n/a n/a BCDL Code FBC2004/TPI2002 5.0 (Matrix) Weight: 213 lb

LUMBER

TOP CHORD 2 X 6 SYP No.1D BOT CHORD 2 X 8 SYP No.1D

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-0-12 oc purlins, except

2-0-0 oc purlins (10-0-0 max.): 6-7.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS

1 Row at midpt

5-8

REACTIONS (lb/size) 2=1604/0-3-8, 11=1510/0-3-8

Max Horz 2=301(load case 5) Max Uplift 2=-72(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-2316/260, 3-4=-2118/245, 4-5=-1308/305, 5-6=0/492, 7-8=0/490,

8-9=-1306/302, 9-10=-2123/253, 10-11=-2323/272, 6-7=-3/703

BOT CHORD 2-14=-118/1715, 13-14=0/1409, 12-13=0/1409, 11-12=-137/1726

4-14=-4/1053, 9-12=-21/1063, 5-15=-2162/378, 8-15=-2158/371, 3-14=-439/234,

10-12=-453/260, 6-15=-103/166, 7-15=-101/162

JOINT STRESS INDEX

2 = 0.46, 3 = 0.33, 4 = 0.34, 5 = 0.74, 6 = 0.52, 7 = 0.52, 8 = 0.74, 9 = 0.34, 10 = 0.33, 11 = 0.46, 12 = 0.20, 13 = 0.66, 14 = 0.46, 12 = 0.46, 12 = 0.46, 14 = 00.20 and 15 = 0.40

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

January 9,2008

Continued on page 2



inns Leer ruse Ossign Engineer londa PE No. 34866 188 Coastal Bay Blvd cynton Beach, FL 35435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T03	PIGGYBACK ATTIC	9	1		J1922714
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:14 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-15, 8-15; Wall dead load (5.0 psf) on member(s).4-14, 9-12
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 2.

LOAD CASE(S) Standard

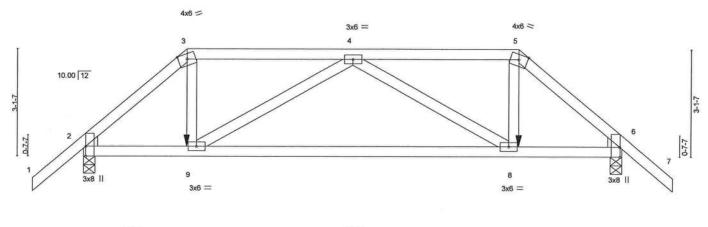
Julius Lee Truss Design Engineer Flonda PE No. 24889 1 109 Ceastel Bay Blyd Boynton Besch, FL 33435





6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:15 2008 Page 1





3-0-0	12-8-0	15-8-0
3-0-0	9-8-0	3-0-0

Plate Of	fsets (X, Y	'): [2:0-3-8,Edge], [6:	0-3-8,Edg	e]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.14	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.27	8-9	>686	240	AND MINESTER	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.18	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 80 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=680/0-4-0, 6=680/0-4-0

Max Horz 2=-76(load case 3)

Max Uplift 2=-227(load case 4), 6=-227(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-787/267, 3-4=-541/232, 4-5=-541/232, 5-6=-787/267, 6-7=0/45

BOT CHORD 2-9=-212/525, 8-9=-354/810, 6-8=-154/525

WEBS 3-9=-82/326, 4-9=-332/228, 4-8=-332/228, 5-8=-82/326

JOINT STRESS INDEX

2 = 0.42, 2 = 0.00, 3 = 0.26, 4 = 0.10, 5 = 0.26, 6 = 0.42, 6 = 0.00, 8 = 0.21 and 9 = 0.21

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colinade page 2

January 9,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T04	HIP	1	1		J1922715
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:15 2008 Page 2

NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 2 and 227 lb uplift at joint 6.

7) Girder carries hip end with 3-0-0 end setback.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

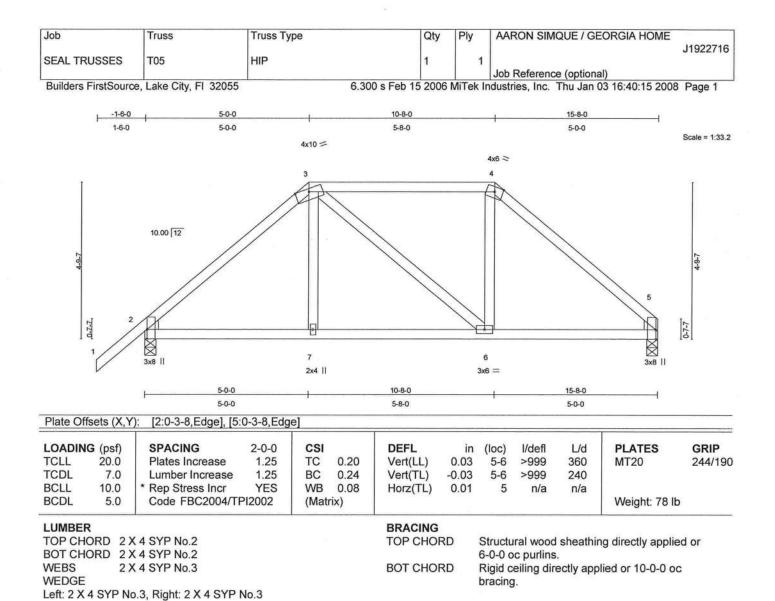
Vert: 1-3=-54, 3-5=-63(F=-9), 5-7=-54, 2-9=-10, 8-9=-12(F=-2), 6-8=-10

Concentrated Loads (lb)

Vert: 9=-48(F) 8=-48(F)

Julius Lee Truse Design Engineer Flonda FE No. 34868 1100 Coastel Bay Blyd





REACTIONS (lb/size) 2=586/0-4-0, 5=486/0-4-0

Max Horz 2=142(load case 5)

Max Uplift 2=-161(load case 6), 5=-85(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-605/278, 3-4=-398/307, 4-5=-614/292

BOT CHORD 2-7=-143/384, 6-7=-142/386, 5-6=-110/396

WEBS 3-7=0/164, 3-6=-96/86, 4-6=-11/166

JOINT STRESS INDEX

2 = 0.33, 2 = 0.00, 3 = 0.62, 4 = 0.62, 5 = 0.41, 5 = 0.00, 6 = 0.11 and 7 = 0.12

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T05	HIP	1	1		J1922716
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:15 2008 Page 2

NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 2 and 85 lb uplift at joint 5.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T06	SPECIAL	1	1		J1922717
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:40:32 2008 Page 1

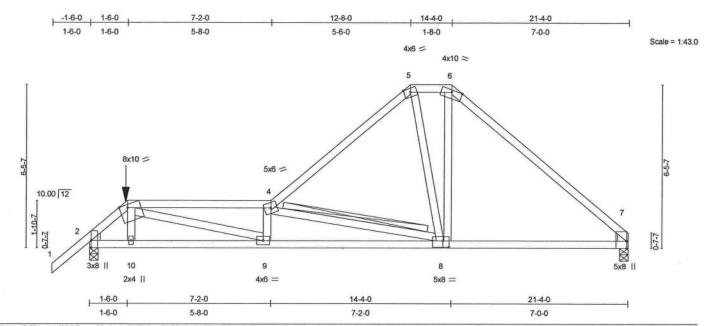


Plate Of	fsets (X,Y)	[2:0-3-8,Edge], [3:0	-3-13,Edge], [7:0-3-	·8,Edge],	[8:0-3-0,0-3-0]						
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	0.15	9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.25	8-9	>991	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.48	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 114 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

D Rigi

Rigid ceiling directly applied or 10-0-0 oc bracing,

Structural wood sheathing directly applied or 4-3-10

Except:

6-5-13 oc bracing: 8-9.

WEBS

T-Brace: 2 X 4 SYP No.3 - 4-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum

end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=772/0-4-0, 7=669/0-4-0

Max Horz 2=189(load case 5)

Max Uplift 2=-238(load case 6), 7=-119(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-885/376, 3-4=-1988/977, 4-5=-741/372, 5-6=-558/413, 6-7=-850/386

BOT CHORD 2-10=-219/606, 9-10=-214/609, 8-9=-933/2022, 7-8=-145/548

WEBS 3-10=0/141, 3-9=-722/1434, 4-9=-369/262, 4-8=-1562/847, 5-8=-200/301, 6-8=-142/319

JOINT STRESS INDEX

2 = 0.59, 2 = 0.00, 3 = 0.40, 4 = 0.67, 5 = 0.73, 6 = 0.95, 7 = 0.53, 7 = 0.00, 8 = 0.74, 9 = 0.67 and 10 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is Codesigned for Garage 2 for members and forces, and for MWFRS for reactions specified.

Truss Design Engineer Florida FE No. 34995 1 199 Caastel Bay Blvri Boynton Besch, FL 99435

January 9,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T06	SPECIAL	1	1		J1922717
	7.258				Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:40:32 2008 Page 2

NOTES

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 2 and 119 lb uplift at joint 7.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

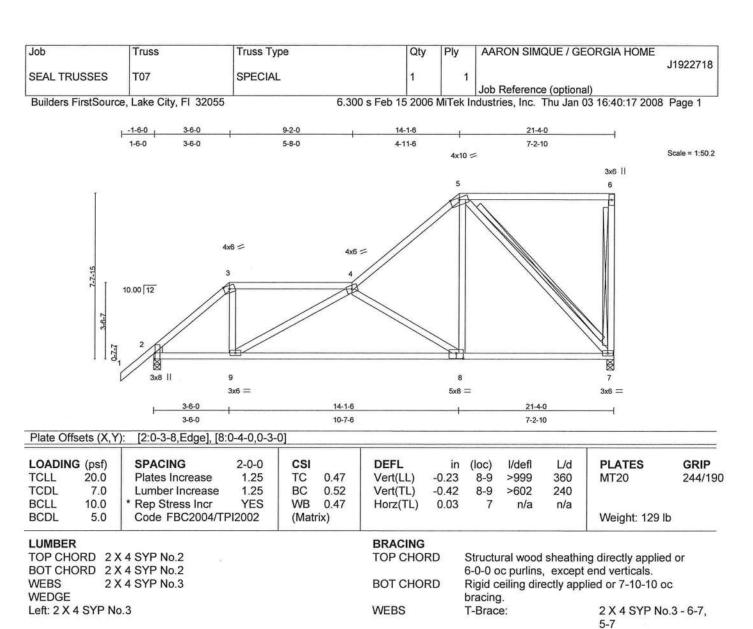
LOAD CASE(S) Standard

Vert: 3=-7(F)

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-4=-54, 4-5=-54, 5-6=-54, 6-7=-54, 2-7=-10
 Concentrated Loads (lb)

Julius Lee Truss Cesign Engineer





Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 7=669/0-4-0, 2=766/0-4-0

Max Horz 2=285(load case 6)

Max Uplift 7=-166(load case 5), 2=-205(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-914/301, 3-4=-637/282, 4-5=-663/239, 5-6=-24/13, 6-7=-180/123

BOT CHORD 2-9=-461/622, 8-9=-640/1069, 7-8=-274/464

WEBS 3-9=-35/399, 4-9=-500/204, 4-8=-727/436, 5-8=-169/520, 5-7=-635/377

JOINT STRESS INDEX

2 = 0.58, 2 = 0.00, 3 = 0.39, 4 = 0.78, 5 = 0.87, 6 = 0.35, 7 = 0.37, 8 = 0.63 and 9 = 0.34

Truss Cesian Engineer Florida PE No. 24865 1100 Coastal Bay Blvd Boynton Beach, FL 30435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T07	SPECIAL	1	1	8.	J1922718
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:17 2008 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 166 lb uplift at joint 7 and 205 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Closign Engineer Flonda PE No. 34868 1 109 Ceastel Bay Blvd Boynton Beach, FL 93495

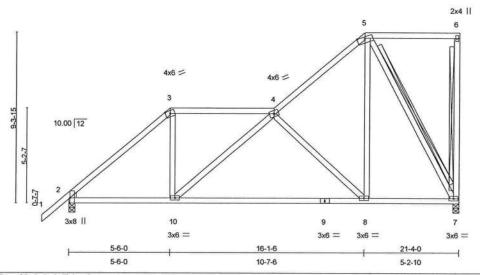


Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T08	SPECIAL	1	1		J1922719
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:18 2008 Page 1



Scale = 1:59.4



ets (X,Y	'): [2:0-3-8,Edge]										
(psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.23	8-10	>999	360	MT20	244/190
7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.42	8-10	>597	240	0.000.000000	55-1045 S 504
10.0	* Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.02	7	n/a	n/a		
5.0	Code FBC2004/TF	PI2002	(Mat	rix)	The state of the s		0.000		15.000	Weight: 140 lb	
	(psf) 20.0 7.0 10.0	(psf) SPACING 20.0 Plates Increase 7.0 Lumber Increase 10.0 * Rep Stress Incr	(psf) SPACING 2-0-0 20.0 Plates Increase 1.25 7.0 Lumber Increase 1.25 10.0 * Rep Stress Incr YES	(psf) SPACING 2-0-0 CSI 20.0 Plates Increase 1.25 TC 7.0 Lumber Increase 1.25 BC 10.0 * Rep Stress Incr YES WB	(psf) SPACING 2-0-0 CSI 20.0 Plates Increase 1.25 TC 0.27 7.0 Lumber Increase 1.25 BC 0.52 10.0 * Rep Stress Incr YES WB 0.51	(psf) SPACING 2-0-0 CSI DEFL 20.0 Plates Increase 1.25 TC 0.27 Vert(LL) 7.0 Lumber Increase 1.25 BC 0.52 Vert(TL) 10.0 * Rep Stress Incr YES WB 0.51 Horz(TL)	(psf) SPACING 2-0-0 CSI DEFL in 20.0 Plates Increase 1.25 TC 0.27 Vert(LL) -0.23 7.0 Lumber Increase 1.25 BC 0.52 Vert(TL) -0.42 10.0 * Rep Stress Incr YES WB 0.51 Horz(TL) 0.02	(psf) SPACING 2-0-0 CSI DEFL in (loc) 20.0 Plates Increase 1.25 TC 0.27 Vert(LL) -0.23 8-10 7.0 Lumber Increase 1.25 BC 0.52 Vert(TL) -0.42 8-10 10.0 * Rep Stress Incr YES WB 0.51 Horz(TL) 0.02 7	(psf) SPACING 2-0-0 CSI DEFL in (loc) l/defl 20.0 Plates Increase 1.25 TC 0.27 Vert(LL) -0.23 8-10 >999 7.0 Lumber Increase 1.25 BC 0.52 Vert(TL) -0.42 8-10 >597 10.0 * Rep Stress Incr YES WB 0.51 Horz(TL) 0.02 7 n/a	(psf) SPACING 2-0-0 CSI DEFL in (loc) l/defl L/d 20.0 Plates Increase 1.25 TC 0.27 Vert(LL) -0.23 8-10 >999 360 7.0 Lumber Increase 1.25 BC 0.52 Vert(TL) -0.42 8-10 >597 240 10.0 * Rep Stress Incr YES WB 0.51 Horz(TL) 0.02 7 n/a n/a	(psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d PLATES 20.0 Plates Increase 1.25 TC 0.27 Vert(LL) -0.23 8-10 >999 360 MT20 7.0 Lumber Increase 1.25 BC 0.52 Vert(TL) -0.42 8-10 >597 240 10.0 * Rep Stress Incr YES WB 0.51 Horz(TL) 0.02 7 n/a n/a

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
WEDGE	
Left: 2 X 4 SYI	P No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 9-2-3 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 - 6-7, 5-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=669/0-4-0, 2=766/0-4-0

Max Horz 2=339(load case 6)

Max Uplift 7=-192(load case 6), 2=-191(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-880/257, 3-4=-588/283, 4-5=-473/137, 5-6=-7/5, 6-7=-120/79

BOT CHORD 2-10=-465/582, 9-10=-465/724, 8-9=-465/724, 7-8=-192/307

3-10=0/310, 4-10=-181/47, 4-8=-588/383, 5-8=-204/511, 5-7=-613/382 **WEBS**

JOINT STRESS INDEX

2 = 0.58, 2 = 0.00, 3 = 0.65, 4 = 0.80, 5 = 0.83, 6 = 0.44, 7 = 0.44, 8 = 0.35, 9 = 0.28 and 10 = 0.35

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	Carrier Control (School of Control of Contro
SEAL TRUSSES	T08	SPECIAL	1	1		J1922719
			1 70		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:18 2008 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 192 lb uplift at joint 7 and 191 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Cestion Engineer Flonda PE No. 34869 I 109 Cestial Rey Blvd Boynton Besch, FL 35436



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	Т09	SPECIAL	1	1		J1922720
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:18 2008 Page 1

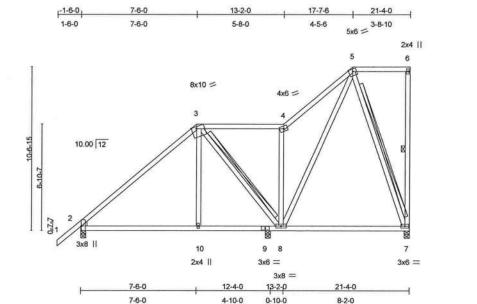


Plate Offsets (X,Y): [2:0-3-8,Edge], [3:0-3-13,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL L/d **PLATES** GRIP I/defl in (loc) TCLL 20.0 Plates Increase 1.25 TC 0.48 Vert(LL) 7-8 >948 360 -0.12MT20 244/190 TCDL 7.0 1.25 BC 0.32 Lumber Increase Vert(TL) -0.227-8 >490 240 BCLL 10.0 Rep Stress Incr YES WB 0.68 0.01 Horz(TL) 7 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 150 lb

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
WEDGE	
1 -4. 0 V 4 CV	7 N = 2

Left: 2 X 4 SYP No.3

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 3-4, 5-6.

Rigid ceiling directly applied or 9-6-13 oc

bracing.

WEBS 1 Row at midpt

T-Brace:

2 X 4 SYP No.3 - 3-8,

Scale = 1:70.3

5-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

JOINTS

Brace must cover 90% of web length.

1 Brace at Jt(s): 6

REACTIONS (lb/size) 7=558/0-4-0, 2=680/0-4-0, 9=198/0-4-0

Max Horz 2=379(load case 6)

Max Uplift 7=-178(load case 6), 2=-140(load case 6), 9=-81(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/45, 2-3=-697/123, 3-4=-375/72, 4-5=-581/213, 5-6=-13/0, 6-7=-80/59

BOT CHORD

2-10=-382/428, 9-10=-381/429, 8-9=-381/429, 7-8=-116/157

WEBS

3-10=0/187, 3-8=-84/228, 4-8=-565/277, 5-8=-306/566, 5-7=-453/351

JOINT STRESS INDEX

2 = 0.70, 2 = 0.00, 3 = 0.74, 4 = 0.47, 5 = 0.36, 6 = 0.38, 7 = 0.48, 8 = 0.64, 9 = 0.46 and 10 = 0.33

Continued on page 2

January 9,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T09	SPECIAL	1	1		J1922720
02 12 11100020	1.00	Of Low LE		100	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:18 2008 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 7, 140 lb uplift at joint 2 and 81 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34889 1109 Coastel Bay Blvd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T10	SPECIAL	4			J1922721
SEAL TRUSSES	110	SPECIAL	1	1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:19 2008 Page 1



Scale = 1:66.5

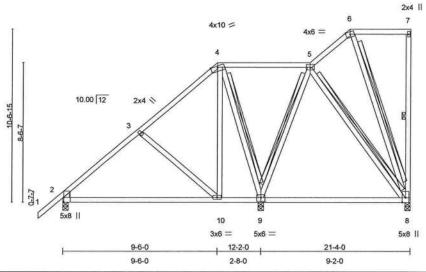


Plate Offsets (X,Y): [2:0-3-8,Edge], [9:0-3-0,0-3-0]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.17	2-10	>861	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.30	2-10	>483	240	S-CAT PROPERTY AND	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.18	Horz(TL)	-0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)	1 2 2					Weight: 164 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
WEDGE	

Left: 2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5, 6-7.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

T-Brace:

2 X 4 SYP No.3 - 4-9,

5-9, 5-8, 6-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

JOINTS

WEBS

1 Brace at Jt(s): 7

REACTIONS (lb/size) 8=272/0-4-0, 2=467/0-4-0, 9=696/0-4-0

Max Horz 2=379(load case 6)

Max Uplift 8=-100(load case 5), 2=-76(load case 6), 9=-234(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-404/0, 3-4=-211/13, 4-5=-41/23, 5-6=-115/25, 6-7=-15/0, 7-8=-97/60

BOT CHORD 2-10=-339/244, 9-10=-158/104, 8-9=-82/67

WEBS 3-10=-187/239, 4-10=-114/237, 4-9=-410/356, 5-9=-260/97, 5-8=-44/74,

6-8=-103/120

JOINT STRESS INDEX

Continued 57, 3ag 6.90, 3 = 0.33, 4 = 0.85, 5 = 0.72, 6 = 0.29, 7 = 0.45, 8 = 0.31, 9 = 0.74 and 10 = 0.34

January 9,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T10	SPECIAL	1	1		J1922721
B 11 E 10		STATE SERVICE STATE OF THE STAT		220	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:19 2008 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint 8, 76 lb uplift at joint 2 and 234 lb uplift at joint 9.

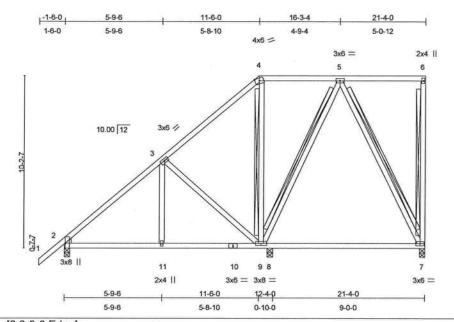
LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Florida ME No. 34899 1109 Chestel Bay Blvd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T11	MONO HIP	1	1		J1922722
OD IL INGOGEO	1	INOTIO TIII	13		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:20 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.12	7-8	>871	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.20	7-8	>537	240	Control of the Contro	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.31	Horz(TL)	-0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Mat	rix)						Weight: 151 lb	

LOMBEK	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
WEDGE	
Left: 2 X 4 SYI	No.3

LUMBED

.

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
Rigid ceiling directly applied or 9-3-6 oc

bracing.
WEBS T-Brace:

2 X 4 SYP No.3 - 6-7, 4-9, 5-9, 5-7

Scale: 3/16"=1"

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=452/0-4-0, 2=603/0-4-0, 8=380/0-4-0

Max Horz 2=367(load case 6)

Max Uplift 7=-204(load case 5), 2=-151(load case 6), 8=-18(load case 6) Max Grav 7=452(load case 1), 2=603(load case 1), 8=524(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-628/174, 3-4=-324/171, 4-5=-168/217, 5-6=-13/0, 6-7=-118/84

BOT CHORD 2-11=-441/394, 10-11=-441/394, 9-10=-441/394, 8-9=-157/156, 7-8=-157/156

WEBS 3-11=0/193, 3-9=-297/294, 4-9=-137/98, 5-9=-143/43, 5-7=-334/354

JOINT STRESS INDEX

2 = 0.58, 2 = 0.00, 3 = 0.43, 4 = 0.59, 5 = 0.46, 6 = 0.43, 7 = 0.45, 9 = 0.61, 10 = 0.24 and 11 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T11	MONO HIP	1	1		J1922722
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:20 2008 Page 2

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 7, 151 lb uplift at joint 2 and 18 lb uplift at joint 8.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T12	SPECIAL	1	1		J1922723
02.27700020	1.12	0. 20., 12	1.		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:21 2008 Page 1

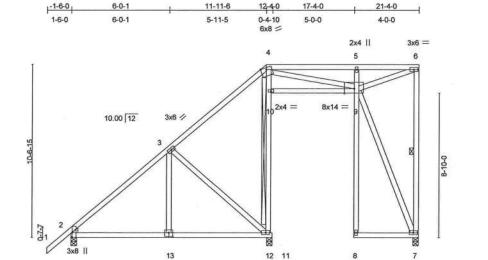


Plate Offsets (X,Y): [2:0-3-8,Edge], [13:0-3-0,0-3-0] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES** GRIP in (loc) TCLL 20.0 Plates Increase 1.25 TC 0.22 Vert(LL) -0.03 12-13 >999 360 MT20 244/190 TCDL BC 0.28 7.0 Lumber Increase 1.25 Vert(TL) -0.05 12-13 >999 240 **BCLL** 10.0 WB 0.35 Rep Stress Incr YES Horz(TL) 0.08 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 Weight: 163 lb (Matrix)

6-1-15

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

4-12 2 X 4 SYP No.3, 5-8 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

BRACING

3x6 =

12,4-0

0-2-0

TOP CHORD Structure

5-0-0

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 4-6.

3x6 =

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:

2x4 II

4-0-0

T-Brace:

2 X 4 SYP No.3 -

Scale = 1:66.7

WEBS JOINTS 1 Row at midpt

1 Brace at Jt(s): 6

10-12 6-7

REACTIONS

WEBS

(lb/size) 7=273/0-4-0, 2=464/0-4-0, 12=700/0-4-0

Max Horz 2=379(load case 6)

Max Uplift 7=-108(load case 4), 2=-30(load case 6), 12=-291(load case 6) Max Grav 7=285(load case 11), 2=464(load case 1), 12=700(load case 1)

5x6 =

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-414/0, 3-4=-155/102, 4-5=-499/204, 5-6=-470/210, 6-7=-267/140

BOT CHORD 2-13=-227/234, 12-13=-227/234, 11-12=0/0, 10-12=-471/362, 4-10=-438/376,

9-10=-48/83, 8-9=0/55, 5-9=-257/179, 7-8=-1/2

3-13=0/209, 3-12=-309/303, 4-9=-261/456, 7-9=-1/2, 6-9=-224/502

Julius Lee Truss Clesign Engineer Flonde FE No. 34868 1166 Caastal Bay Blvd Boynton Beach, FL 33436

JOINT STRESS INDEX

2 = 0.58, 2 = 0.00, 3 = 0.43, 4 = 0.51, 5 = 0.33, 6 = 0.34, 7 = 0.47, 8 = 0.33, 9 = 0.32, 10 = 0.82, 12 = 0.35 and 13 = 0.44

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T12	SPECIAL	1	1		J1922723
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:21 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 7, 30 lb uplift at joint 2 and 291 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonda PE No. 34868 1189 Coastal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T13	SPECIAL	3	1		J1922724
		0, 252			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:22 2008 Page 1

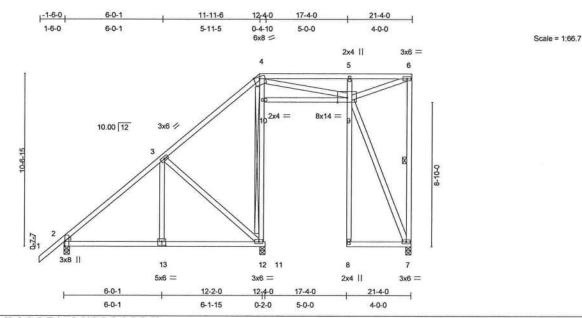


Plate Offsets (X,Y): [2:0-3-8,Edge], [13:0-3-0,0-3-0] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES GRIP** in (loc) TCLL 20.0 Plates Increase 1.25 TC 0.22 Vert(LL) -0.03 12-13 >999 360 MT20 244/190 TCDL 7.0 1.25 BC 0.28 Lumber Increase Vert(TL) -0.05 12-13 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.35 0.08 Horz(TL) 7 n/a n/a **BCDL** Code FBC2004/TPI2002 5.0 (Matrix) Weight: 163 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

4-12 2 X 4 SYP No.3, 5-8 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

BRACING

WEBS

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing. Except:

1 Row at midpt

T-Brace:

2 X 4 SYP No.3 -

10-12 6-7

REACTIONS (lb/size) 7=273/0-4-0, 2=464/0-4-0, 12=700/0-4-0

Max Horz 2=379(load case 6)

Max Uplift 7=-108(load case 4), 2=-30(load case 6), 12=-291(load case 6) Max Grav 7=285(load case 11), 2=464(load case 1), 12=700(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-414/0, 3-4=-155/102, 4-5=-499/204, 5-6=-470/210, 6-7=-267/140

BOT CHORD 2-13=-227/234, 12-13=-227/234, 11-12=0/0, 10-12=-471/362, 4-10=-438/376,

9-10=-48/83, 8-9=0/55, 5-9=-257/179, 7-8=-1/2

3-13=0/209, 3-12=-309/303, 4-9=-261/456, 7-9=-1/2, 6-9=-224/502 **WEBS**

ulius Lae nuss Cesign Engineer londa PE No. 34866 100 Caastal Ray Blvd Cynton Beach, FL 33436

JOINT STRESS INDEX

2 = 0.58, 2 = 0.00, 3 = 0.43, 4 = 0.51, 5 = 0.33, 6 = 0.34, 7 = 0.47, 8 = 0.33, 9 = 0.32, 10 = 0.82, 12 = 0.35 and 13 = 0.44

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T13	SPECIAL	3	1		J1922724
		9. 707 -			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:22 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 7, 30 lb uplift at joint 2 and 291 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonda PE No. 34868 1 109 Coastal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T14	SPECIAL	1	1	19	J1922725
	323	5. 25. 12	1.5		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:23 2008 Page 1



Scale = 1:70.7

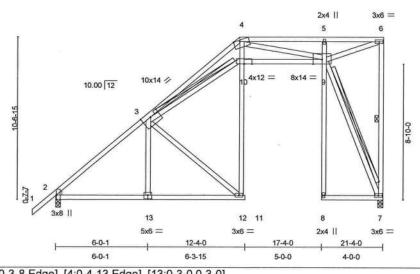


Plate Offsets (X,Y): [2:0-3-8,Edge], [4:0-4-13,Edge], [13:0-3-0,0-3-0] SPACING LOADING (psf) 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP 20.0 TC TCLL Plates Increase 1.25 0.52 Vert(LL) 0.46 >550 360 MT20 11 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.76 Vert(TL) -0.5611 >452 240 MT20H 187/143 BCLL 10.0 Rep Stress Incr YES WB 0.86 Horz(TL) 1.09 7 n/a n/a **BCDL**

JOINTS

LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 *Except* 3-2-5 oc purlins, except end verticals, and 5-8 2 X 4 SYP No.3 2-0-0 oc purlins (4-11-6 max.): 4-6. Rigid ceiling directly applied or 4-7-13 oc **WEBS** 2 X 4 SYP No.3 BOT CHORD

(Matrix)

WEDGE

5.0

Left: 2 X 4 SYP No.3

Code FBC2004/TPI2002

bracina. **WEBS** 1 Row at midpt

T-Brace:

2 X 4 SYP No.3 - 7-9,

Weight: 174 lb

3-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

1 Brace at Jt(s): 6

REACTIONS (lb/size) 7=670/0-4-0, 2=767/0-4-0

Max Horz 2=379(load case 6)

Max Uplift 7=-209(load case 5), 2=-156(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-863/217, 3-4=-3134/2227, 4-5=-1535/1034, 5-6=-1428/975,

6-7=-650/445

BOT CHORD 2-13=-490/572, 12-13=-489/573, 11-12=0/0, 10-12=-386/498, 4-10=-1407/1861,

9-10=-1785/2391, 8-9=0/56, 5-9=-267/190, 7-8=-2/4

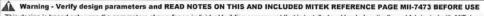
3-13=0/197, 4-9=-887/778, 7-9=-2/2, 6-9=-1046/1531, 3-10=-2085/2677,

3-12=-719/620

Continued on page 2

WEBS

January 9,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T14	SPECIAL	1	1		J1922725
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:23 2008 Page 2

JOINT STRESS INDEX

2 = 0.58, 2 = 0.00, 3 = 0.41, 4 = 0.97, 5 = 0.81, 6 = 0.84, 7 = 0.47, 8 = 0.33, 9 = 0.41, 10 = 0.66, 12 = 0.80 and 13 = 0.42

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Special provisions shall be made by the building designer to allow for 0.88in of horizontal movement due to live load and 1.09in due to total load.
- 4) Provide adequate drainage to prevent water ponding.
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 209 lb uplift at joint 7 and 156 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34869 1109 Gestel Bey Blvd Bovnton Desch El. 33435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	-93114-911066603399793
SEAL TRUSSES	T15	ROOF TRUSS	3	1		J1922726
	112.2.2		100		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:23 2008 Page 1



Scale = 1:67.5

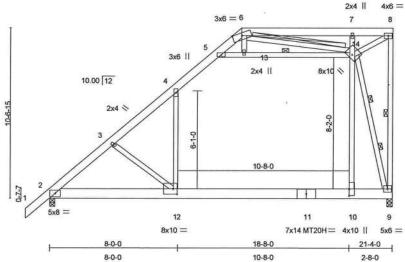


Plate Offsets (X,Y): [10:0-5-0,0-0-0], [12:0-3-0,0-4-0], [14:0-5-0,0-2-8]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.33	10-12	>764	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.58	10-12	>435	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.02	9	n/a	n/a	III December of the second	
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	2 8					Weight: 213 lb	

LUMBER	
TOP CHORD	2 X 6 SYP No.1D
BOT CHORD	2 X 8 SYP No.1D
WEBS	2 X 4 SYP No.3 *Except*
	9-14 2 X 4 SYP No.2

BRACING TOP CHORD

Structural wood sheathing directly applied or 5-3-4 oc purlins, except end verticals, and

2-0-0 oc purlins (9-9-2 max.): 6-8.

BOT CHORD

Rigid ceiling directly applied or 5-8-6 oc

bracing.

WEBS

1 Row at midpt

8-9, 5-14 9-14

2 Rows at 1/3 pts T-Brace:

2 X 4 SYP No.3 -

6-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

JOINTS

1 Brace at Jt(s): 8

REACTIONS (lb/size) 9=1527/0-4-0, 2=1268/0-4-0

Max Horz 2=385(load case 6) Max Uplift 2=-35(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-1642/7, 3-4=-1422/0, 4-5=-820/91, 5-6=-75/276, 6-7=-641/1616,

7-8=-661/1629, 8-9=-433/1039

BOT CHORD 2-12=-380/1241, 11-12=-178/886, 10-11=-178/886, 9-10=-177/856

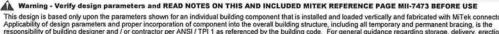
4-12=0/716, 6-13=0/125, 5-13=-1214/138, 13-14=-1201/144, 10-14=0/1668,

7-14=-473/307, 6-14=-1324/687, 8-14=-1885/763, 9-14=-3428/710, 3-12=-453/257

Continued on page 2

WEBS

January 9,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T15	ROOF TRUSS	3	1		J1922726
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:24 2008 Page 2

JOINT STRESS INDEX

2 = 0.45, 3 = 0.33, 4 = 0.25, 5 = 0.41, 6 = 0.95, 7 = 0.33, 8 = 0.52, 9 = 0.68, 10 = 0.37, 11 = 0.09, 12 = 0.18, 13 = 0.33 and 14 = 0.82

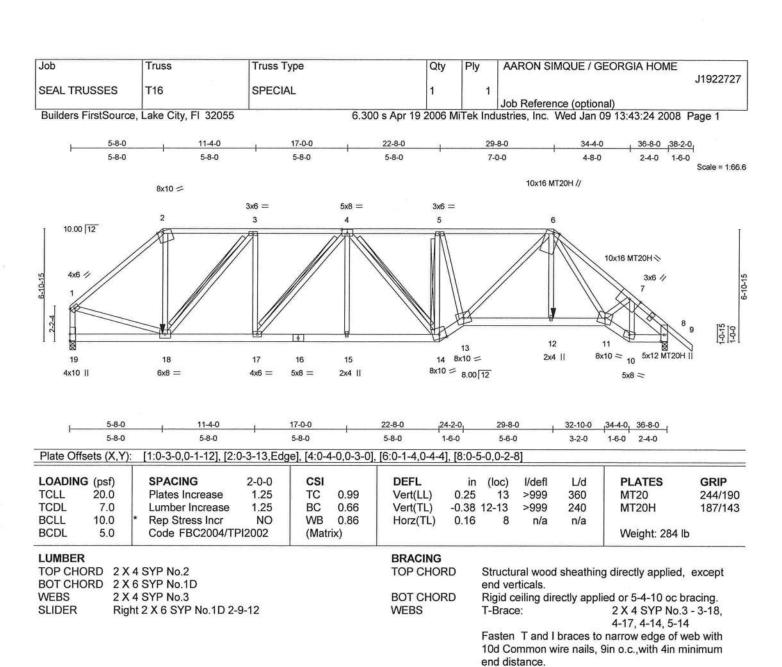
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are MT20 plates unless otherwise indicated.
- 6) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-13, 13-14; Wall dead load (5.0 psf) on member(s).4-12, 10-14
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 10-12
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Cesign Engineer Florida PE No. 34866 1189 Coesiel Bay Blyd Boyston Beeck E. 1944





REACTIONS (lb/size) 19=2525/0-4-0, 8=2503/0-4-0

Max Horz 19=-190(load case 3)

Max Uplift 19=-1240(load case 4), 8=-1285(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2738/1392, 2-3=-2066/1142, 3-4=-3235/1824, 4-5=-3624/2102, 5-6=-3887/2217,

6-7=-3853/2121, 7-8=-3237/1715, 8-9=0/30, 1-19=-2478/1240

BOT CHORD 18-19=-167/216, 17-18=-1818/3235, 16-17=-2092/3724, 15-16=-2092/3724,

14-15=-2092/3724, 13-14=-2323/4130, 12-13=-1561/2896, 11-12=-1550/2875,

10-11=-1162/2271, 8-10=-1064/2077

2-18=-620/1258, 3-18=-1851/1154, 3-17=-303/712, 4-17=-764/497, 4-15=0/205,

4-14=-185/87, 5-14=-1732/1065, 5-13=-445/1000, 6-12=-389/730, 6-11=-60/137,

7-11=-759/1237, 7-10=-967/505, 1-18=-1113/2042, 6-13=-837/1455

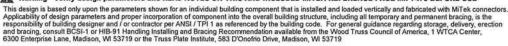
JOINT STRESS INDEX

1 = 0.81, 2 = 0.94, 3 = 0.64, 4 = 0.68, 5 = 0.72, 6 = 0.84, 7 = 0.34, 7 = 0.21, 8 = 0.84, 10 = 0.51, 11 = 0.80, 12 = 0.55, 13 = 0.75, 14 = 0.81, 2 = 0.81,0.76, 15 = 0.34, 16 = 0.79, 17 = 0.33, 18 = 0.80 and 19 = 0.37January 9,2008

Continued on page 2

WEBS

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Brace must cover 90% of web length.

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T16	SPECIAL	1	1		J1922727
	20,7953	F-	100		Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:43:24 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1240 lb uplift at joint 19 and 1285 lb uplift at joint 8.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

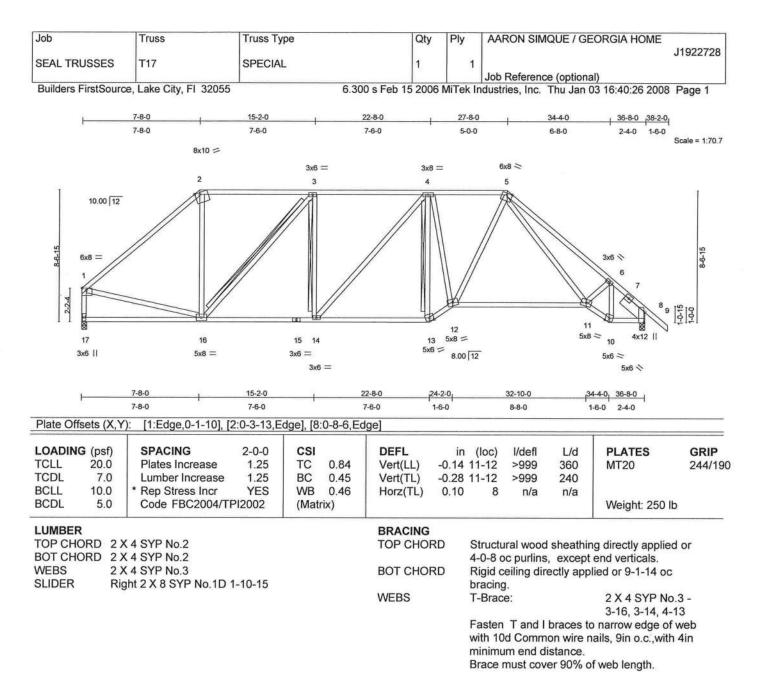
Vert: 1-2=-54, 2-6=-117(F=-63), 6-9=-54, 18-19=-10, 14-18=-22(F=-12), 13-14=-22(F=-12), 12-13=-22(F=-12), 11-12=-10, 10-11=-10, 8-10=-10

Concentrated Loads (lb)

Vert: 18=-411(F) 12=-411(F)

Julius Lee Truss Design Engineer Flonda FE No. 34888 1199 Cessiel Bay Blvd Acceton Gesch, FL 33425





REACTIONS (lb/size) 17=1167/0-4-0, 8=1251/0-4-0

Max Horz 17=-236(load case 4)

Max Uplift 17=-282(load case 5), 8=-297(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1276/648, 2-3=-889/621, 3-4=-1280/812, 4-5=-1321/798, 5-6=-1848/943,

6-7=-1376/706, 7-8=-1424/685, 8-9=0/26, 1-17=-1124/586

BOT CHORD 16-17=-236/286, 15-16=-424/1280, 14-15=-424/1280, 13-14=-400/1246,

12-13=-468/1471, 11-12=-316/1102, 10-11=-464/1127, 8-10=-404/943

2-16=-111/400, 3-16=-657/301, 3-14=-7/196, 4-14=-91/105, 4-13=-749/270, 4-12=-35/401, 5-12=-243/582, 5-11=-212/407, 6-11=-131/588, 6-10=-731/320,

1-16=-277/761

January 9,2008

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T17	SPECIAL	1	1		J1922728
1.50 Decision Of the decision of the same state	1 20000	I Industrial Production Comp. 1			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:26 2008 Page 2

JOINT STRESS INDEX

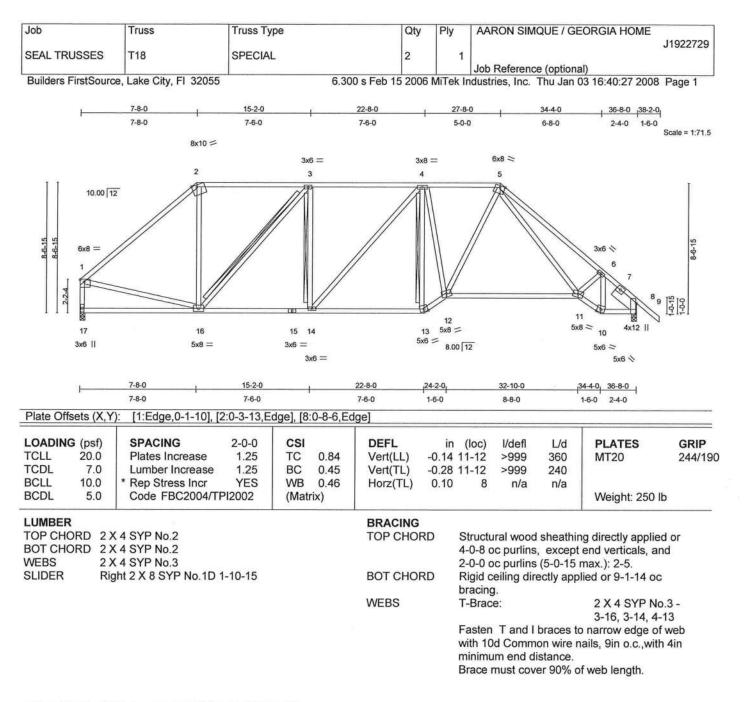
1 = 0.71, 2 = 0.79, 3 = 0.38, 4 = 0.59, 5 = 0.75, 6 = 0.43, 7 = 0.00, 8 = 0.79, 8 = 0.51, 10 = 0.43, 11 = 0.72, 12 = 0.66, 13 = 0.79, 10 = 0.79, 0.60, 14 = 0.38, 15 = 0.41, 16 = 0.37 and 17 = 0.39

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 17 and 297 lb uplift at joint 8.

LOAD CASE(S) Standard





REACTIONS (lb/size) 17=1167/0-4-0, 8=1251/0-4-0

Max Horz 17=-236(load case 4)

Max Uplift 17=-282(load case 5), 8=-297(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1276/648, 2-3=-889/621, 3-4=-1280/812, 4-5=-1321/798, 5-6=-1848/943,

6-7=-1376/706, 7-8=-1424/685, 8-9=0/26, 1-17=-1124/586

BOT CHORD 16-17=-236/286, 15-16=-424/1280, 14-15=-424/1280, 13-14=-401/1246,

12-13=-468/1472, 11-12=-316/1103, 10-11=-464/1127, 8-10=-404/944

2-16=-111/400, 3-16=-657/301, 3-14=-7/197, 4-14=-91/105, 4-13=-749/270,

4-12=-35/401, 5-12=-244/582, 5-11=-212/407, 6-11=-131/589, 6-10=-731/320,

1-16=-277/761

Truse Design Engineer Florida PE No. 24ges 1100 Coastal Bay Blvd Boynton Beach, FL 22425

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T18	SPECIAL	2	1		J1922729
		1-1			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:27 2008 Page 2

JOINT STRESS INDEX

1 = 0.71, 2 = 0.79, 3 = 0.38, 4 = 0.59, 5 = 0.74, 6 = 0.43, 7 = 0.00, 8 = 0.79, 8 = 0.51, 10 = 0.43, 11 = 0.72, 12 = 0.66, 13 = 0.60, 14 = 0.38, 15 = 0.41, 16 = 0.37 and 17 = 0.39

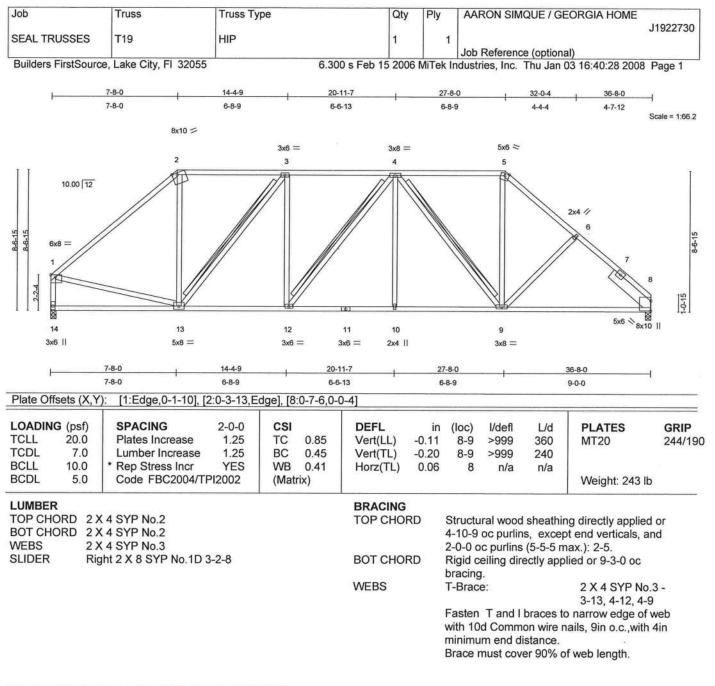
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 17 and 297 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Plonda PE No. 34868 I 109 Coastal Bay Blvd





REACTIONS (lb/size) 8=1169/0-4-0, 14=1169/0-4-0

Max Horz 14=-227(load case 4)

Max Uplift 8=-281(load case 4), 14=-283(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-1277/650, 2-3=-888/622, 3-4=-1251/799, 4-5=-994/669, 5-6=-1317/759, TOP CHORD

6-7=-1418/751, 7-8=-1507/737, 1-14=-1125/589

BOT CHORD 13-14=-247/274, 12-13=-438/1251, 11-12=-463/1291, 10-11=-463/1291,

9-10=-463/1291, 8-9=-419/1013

2-13=-122/403, 3-13=-652/298, 3-12=-18/225, 4-12=-96/73, 4-10=0/186,

4-9=-544/267, 5-9=-259/477, 6-9=-95/162, 1-13=-273/755

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T19	HIP	1	1	J	J1922730
OD IL THOOGES	1.10				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:28 2008 Page 2

JOINT STRESS INDEX

1 = 0.72, 2 = 0.74, 3 = 0.39, 4 = 0.56, 5 = 0.64, 6 = 0.33, 7 = 0.00, 8 = 0.60, 8 = 0.53, 9 = 0.56, 10 = 0.33, 11 = 0.46, 12 = 0.39, 13 = 0.36 and 14 = 0.40

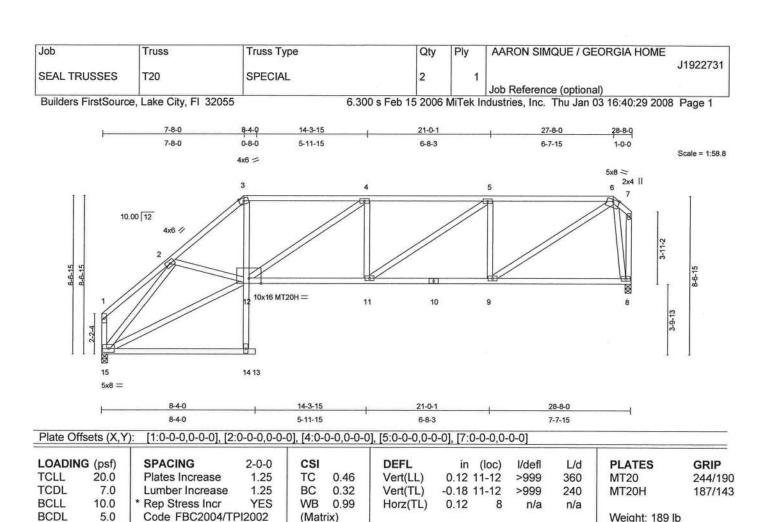
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 281 lb uplift at joint 8 and 283 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 24869 1109 Ceastel Bay Blvd Bovnton Beach, Ft. 83436





LUMBER

WEBS

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

13-15 2 X 4 SYP No.1D, 3-14 2 X 4 SYP No.1D

2 X 4 SYP No.3

BRACING TOP CHORD

BOT CHORD

ORD Structural wood sheathing directly applied or

4-11-13 oc purlins, except end verticals, and

2-0-0 oc purlins (4-11-9 max.): 3-6.

Rigid ceiling directly applied or 6-9-9 oc

bracing.

REACTIONS (lb/size) 8=909/0-4-0, 15=912/0-4-0

Max Horz 15=187(load case 6)

Max Uplift 8=-317(load case 5), 15=-225(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-161/118, 2-3=-1541/930, 3-4=-1189/804, 4-5=-1497/909, 5-6=-1179/692,

6-7=-71/82, 1-15=-168/131, 7-8=-105/118

BOT CHORD 14-15=-6/32, 13-14=0/0, 12-14=0/127, 3-12=-382/644, 11-12=-877/1497,

10-11=-659/1179, 9-10=-659/1179, 8-9=-117/208

WEBS 4-12=-448/224, 4-11=-146/184, 5-11=-262/383, 5-9=-582/411, 6-9=-653/1170,

6-8=-988/627, 2-15=-1390/802, 2-12=-120/307, 12-15=-808/957

JOINT STRESS INDEX

1 = 0.23, 2 = 0.42, 3 = 0.77, 4 = 0.35, 5 = 0.35, 6 = 0.45, 7 = 0.52, 8 = 0.67, 9 = 0.67, 10 = 0.42, 11 = 0.34, 12 = 0.63, 14 = 0.38 and 15 = 0.46

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

Julius Lee Truse Design Engineer Floride PE No. 34889 1109 Chestal Bay Blvd Boynton Beach, FL 93436



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T20	SPECIAL	2	1		J1922731
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:29 2008 Page 2

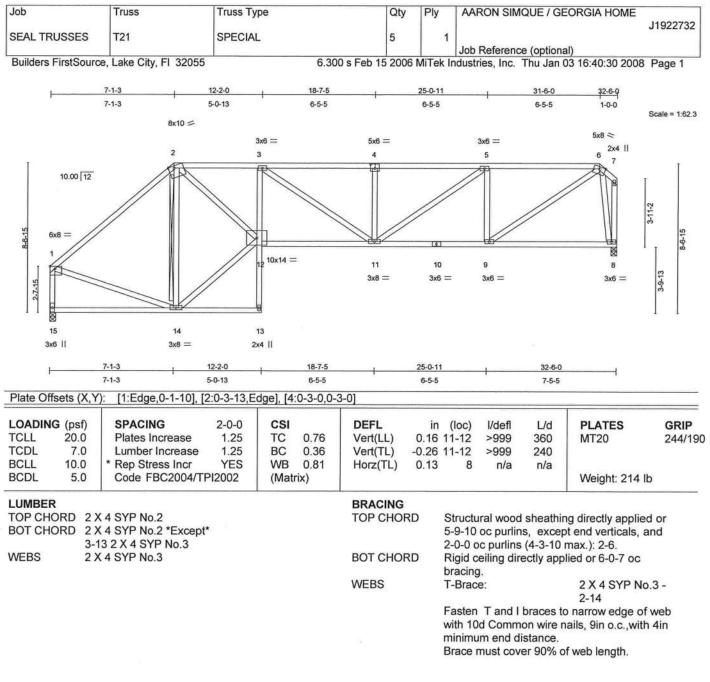
NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 3x6 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 8 and 225 lb uplift at joint 15.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Plonda PE No. 34869 1 100 Coestal Bay Blyd Bayoton Basen El 10446





REACTIONS (lb/size) 15=1031/0-4-0, 8=1031/0-4-0

Max Horz 15=172(load case 6)

Max Uplift 15=-280(load case 5), 8=-361(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1045/504, 2-3=-1853/1126, 3-4=-1866/1085, 4-5=-1866/1085, 5-6=-1351/772,

6-7=-71/68, 1-15=-989/485, 7-8=-95/100

BOT CHORD 14-15=-319/105, 13-14=-13/14, 12-13=0/52, 3-12=-334/223, 11-12=-1114/1885,

10-11=-739/1351, 9-10=-739/1351, 8-9=-126/229

2-14=-739/486, 12-14=-586/898, 2-12=-833/1517, 3-11=-62/74, 4-11=-357/260,

5-11=-380/627, 5-9=-712/470, 6-9=-745/1366, 1-14=-203/639, 6-8=-1093/673

Trues Design Engineer Flonda PE No. 34868 1199 Caastal Bay Blyd Boynton Besch, FL 33435

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T21	SPECIAL	5	1		J1922732
	7.058				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:30 2008 Page 2

JOINT STRESS INDEX

1 = 0.58, 2 = 0.57, 3 = 0.47, 4 = 0.50, 5 = 0.36, 6 = 0.48, 7 = 0.57, 8 = 0.65, 9 = 0.78, 10 = 0.47, 11 = 0.59, 12 = 0.61, 13 = 0.33, 14 = 0.85 and 15 = 0.33

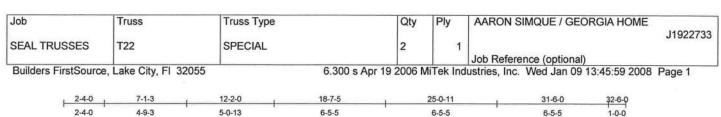
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 15 and 361 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Les Truss Cosign Engineer Florida PE No. 34869 1199 Coestel Bay Blvd





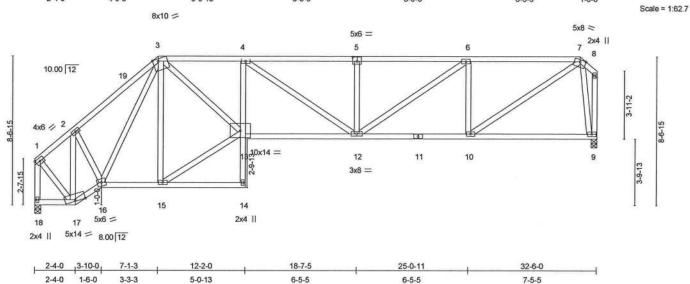


Plate Of	fsets (X,Y)	: [3:0-3-13,Edge], [5:	0-3-0,0-3-0)]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	0.18	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.30	12-13	>999	240	05000000000	-001/00 tol/00/00
BCLL	10.0	* Rep Stress Incr	NO	WB	0.88	Horz(TL)	0.16	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	419-200-380 MC10010					Weight: 226 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

4-14 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-11-15 oc purlins, except end verticals, and 2-0-0

oc purlins (3-8-9 max.): 3-7.

BOT CHORD

Rigid ceiling directly applied or 5-7-12 oc bracing.

(lb/size) 18=1943/0-4-0, 9=1109/0-4-0 REACTIONS

Max Horz 18=172(load case 6)

Max Uplift 18=-557(load case 5), 9=-386(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1299/573, 2-19=-1815/974, 3-19=-1331/831, 3-4=-2204/1289, 4-5=-2106/1193,

5-6=-2106/1193, 6-7=-1477/830, 7-8=-73/65, 1-18=-1926/872, 8-9=-94/96

BOT CHORD 17-18=-222/23, 16-17=-730/1047, 15-16=-653/1084, 14-15=-17/24, 13-14=0/65,

4-13=-228/177, 12-13=-1275/2241, 11-12=-797/1477, 10-11=-797/1477, 9-10=-133/244

WEBS 2-17=-1567/867, 2-16=-227/524, 3-16=-176/107, 3-15=-609/421, 13-15=-745/1244,

3-13=-805/1490, 4-12=-160/137, 5-12=-359/259, 6-12=-442/765, 6-10=-789/506,

7-10=-807/1501, 1-17=-641/1384, 7-9=-1168/706

JOINT STRESS INDEX

1 = 0.71, 2 = 0.62, 3 = 0.43, 4 = 0.50, 5 = 0.52, 6 = 0.45, 7 = 0.53, 8 = 0.62, 9 = 0.67, 10 = 0.88, 11 = 0.53, 12 = 0.73, 13 = 0.68, 14 = 0.71, 2 = 0.71,0.45, 15 = 0.73, 16 = 0.42, 17 = 0.53 and 18 = 0.70

NOTES

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is Coffee and for MWFRS for reactions specified.

January 9,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T22	SPECIAL	2	1		J1922733
			-		Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:45:59 2008 Page 2

NOTES

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 3x6 MT20 unless otherwise indicated.

- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 557 lb uplift at joint 18 and 386 lb uplift at joint 9.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard Except:

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 3-19=-54, 3-7=-54, 7-8=-54, 17-18=-10, 16-17=-10, 14-16=-10, 9-13=-10

Trapezoidal Loads (plf)

Vert: 1=-269(F=-60)-to-19=-209

Julius Lee Tuuse Design Engineer Flonda PE No. 34869 1 100 Caastal Bay Blvd Bovnton Besch, FL 23436



Job AARON SIMQUE / GEORGIA HOME Truss Truss Type Qty Ply J1922734 SEAL TRUSSES T23 SPECIAL 3 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:47:16 2008 Page 1 16-10-9 7-1-3 9-6-0 31-6-0 35-10-4 40-6-0 42-0-0 2-4-0 24-1-7 2-4-0 4-9-3 2-4-13 7-4-9 7-2-13 7-4-9 4-4-4 4-7-12 1-6-0 Scale = 1:70.9 5x8 = 5x8 > 3x8 = 2x4 II 5x8 = 3 4 5 6 7 10.00 12 3x6 / 2x4 // 17 5x6 = 8x10 II 5x6 = 20 19 16 15 14 13 12 5x6 \ 5x14 = 3x8 = 3x6 = 3x8 II 3x8 = 8.00 12 5x6 = 2x4 II 2-4-0 3-10-0 8-0-0 9-6-0 16-10-9 24-1-7 31-6-0 40-6-0 2-4-0 1-6-0 4-2-0 1-6-0 7-4-9 7-2-13 7-4-9 9-0-0 Plate Offsets (X,Y): [6:0-4-0,0-3-0], [10:0-7-6,0-0-4] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES GRIP** in (loc) TCLL 20.0 Plates Increase 1.25 TC 0.70 Vert(LL) 0.13 14-15 >999 360 MT20 244/190 TCDI 7.0 Lumber Increase 1.25 BC 0.51 Vert(TL) -0.24 14-15 >999 240 0.59 BCLL 10.0 Rep Stress Incr NO WB Horz(TL) 0.12 10 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 297 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 3-7-13 **BOT CHORD** 2 X 4 SYP No.2 oc purlins, except end verticals, and 2-0-0 oc **WEBS** 2 X 4 SYP No.3 purlins (4-5-3 max.): 3-7. SLIDER Right 2 X 8 SYP No.1D 3-2-8 **BOT CHORD** Rigid ceiling directly applied or 8-0-12 oc bracing. **WEBS** T-Brace: 2 X 4 SYP No.3 - 4-16, 5-15, 6-12 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length. REACTIONS (lb/size) 20=2218/0-4-0, 10=1436/0-4-0 Max Horz 20=-237(load case 4) Max Uplift 20=-621(load case 5), 10=-370(load case 4) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-1482/685, 2-21=-2114/1056, 3-21=-1628/913, 3-4=-1405/831, 4-5=-1735/1035, 5-6=-1735/1035, 6-7=-1196/764, 7-8=-1567/875, 8-9=-1664/864, 9-10=-1755/852, 10-11=0/26, 1-20=-2196/1034 **BOT CHORD** 19-20=-155/232, 18-19=-524/1213, 17-18=-503/1333, 16-17=-582/1586, 15-16=-502/1387, 14-15=-610/1669, 13-14=-610/1669, 12-13=-610/1669, 10-12=-457/1179 WEBS 2-19=-1825/799, 2-18=-202/720, 3-17=-324/751, 4-17=-106/203, 4-16=-733/323, 4-15=-278/569, 5-15=-407/293, 6-15=-101/141, 6-14=0/207, 6-12=-769/365, 7-12=-313/6/10 8-12=-112/137, 1-19=-764/1586, 3-18=-120/115

JOINT STRESS INDEX

1 = 0.81, 2 = 0.72, 3 = 0.57, 4 = 0.69, 5 = 0.34, 6 = 0.40, 7 = 0.68, 8 = 0.34, 9 = 0.00, 10 = 0.62, 10 = 0.65, 12 = 0.57, 13 = 0.54 and the results of the second seco Continued on page 7216 = 0.65, 17 = 0.51, 18 = 0.43, 19 = 0.60 and 20 = 0.36

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T23	SPECIAL	3	1		J1922734
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:47:16 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 621 lb uplift at joint 20 and 370 lb uplift at joint 10.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard Except:

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

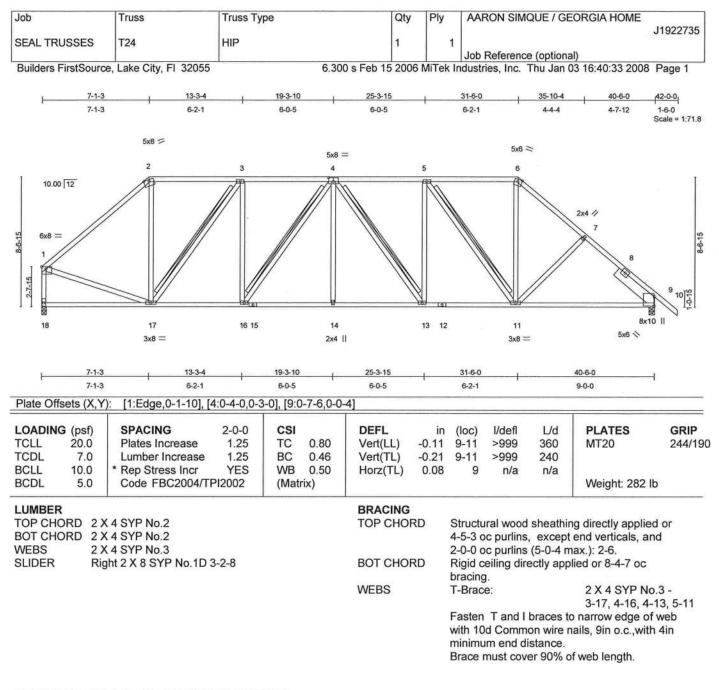
Vert: 3-21=-54, 3-7=-54, 7-11=-54, 19-20=-10, 18-19=-10, 17-18=-10, 16-17=-10, 10-16=-10

Trapezoidal Loads (plf)

Vert: 1=-269(F=-60)-to-21=-209

Julius Les Truse Design Engineer Floride PE No. 34866 1109 Grastal Bay Blvd





REACTIONS (lb/size) 18=1290/0-4-0, 9=1374/0-4-0

Max Horz 18=-236(load case 4)

Max Uplift 18=-338(load case 5), 9=-350(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1332/687, 2-3=-940/643, 3-4=-1399/871, 4-5=-1493/922, 5-6=-1122/730,

6-7=-1480/837, 7-8=-1579/826, 8-9=-1669/815, 9-10=0/26, 1-18=-1249/650

BOT CHORD 17-18=-202/267, 16-17=-492/1399, 15-16=-576/1587, 14-15=-576/1587,

13-14=-576/1587, 12-13=-528/1493, 11-12=-528/1493, 9-11=-431/1121

2-17=-162/470, 3-17=-837/386, 3-16=-105/352, 4-16=-342/169, 4-14=0/187,

4-13=-205/121, 5-13=-66/256, 5-11=-690/326, 6-11=-312/590, 7-11=-102/141,

1-17=-305/880

Julius Lee Truse Design Engineer Flonds PE No. 24869 1160 Ceastel Bay Blvd Bovoton Besch, FL 23435

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T24	HIP	1	1		J1922735
OD IL MOGGEO	1.2.1	· · · · · · · · · · · · · · · · · · ·			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:33 2008 Page 2

JOINT STRESS INDEX

1 = 0.57, 2 = 0.69, 3 = 0.40, 4 = 0.34, 5 = 0.40, 6 = 0.63, 7 = 0.33, 8 = 0.00, 9 = 0.60, 9 = 0.60, 11 = 0.56, 12 = 0.48, 13 = 0.40, 14 = 0.33, 15 = 0.52, 16 = 0.40, 17 = 0.79 and 18 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 338 lb uplift at joint 18 and 350 lb uplift at joint 9.
- 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Julius Lee Truse Clesion Engineer Florida PE No. 34869 1109 Grestel Bay Blvd Bovnton Beach, FL 23435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T25	SPECIAL	3	1		J1922736
OD IL TROODES	120	OI LOIAL	"		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:34 2008 Page 1

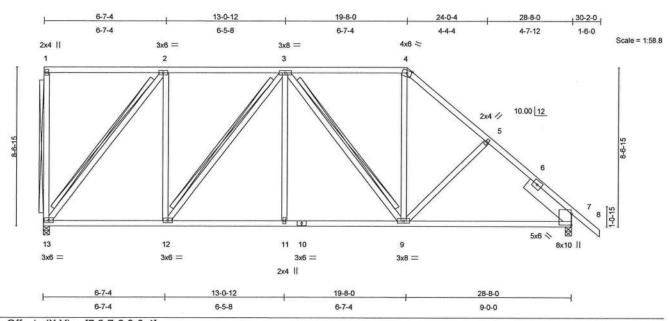


Plate Offsets (X,Y): [7:0-7-6,0-0-4] LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.34 Vert(LL) -0.10>999 360 244/190 7-9 MT20 TCDL 7.0 Lumber Increase 1.25 BC 0.40 Vert(TL) -0.187-9 >999 240 **BCLL** 10.0 Rep Stress Incr YES WB 0.57 0.04 Horz(TL) n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 204 lb

LU	IVI		K		
	-	01		-	

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

SLIDER

Right 2 X 8 SYP No.1D 3-2-8

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-10-2 oc purlins, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 1-4.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -1-13, 2-13, 3-12, 3-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

JOINTS

1 Brace at Jt(s): 1

REACTIONS (lb/size) 13=911/0-4-0, 7=996/0-4-0

Max Horz 13=-291(load case 7)

Max Uplift 13=-276(load case 5), 7=-200(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-13=-155/110, 1-2=-16/8, 2-3=-578/332, 3-4=-705/466, 4-5=-950/501,

5-6=-1054/494, 6-7=-1144/471, 7-8=0/26

BOT CHORD

12-13=-166/578, 11-12=-206/801, 10-11=-206/801, 9-10=-206/801, 7-9=-197/750

WEBS

2-13=-914/526, 2-12=-146/404, 3-12=-364/246, 3-11=0/176, 3-9=-158/146,

4-9=-96/318, 5-9=-83/177

Engineer

JOINT STRESS INDEX

Contihued 87, 3ag 0.29, 3 = 0.56, 4 = 0.61, 5 = 0.33, 6 = 0.00, 7 = 0.60, 7 = 0.41, 9 = 0.56, 10 = 0.27, 11 = 0.33, 12 = 0.39 and 13a uary 9,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T25	SPECIAL	3	1		J1922736
02.211.00020	1.20	0, 20, 12			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:34 2008 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 276 lb uplift at joint 13 and 200 lb uplift at joint 7.
- 6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonda PE No. 24865 1109 Ceastal Bay Blvd.



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T26	SCISSOR	3	1		J1922737
02 12 11100020	1.20	Colocolt	"		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:35 2008 Page 1

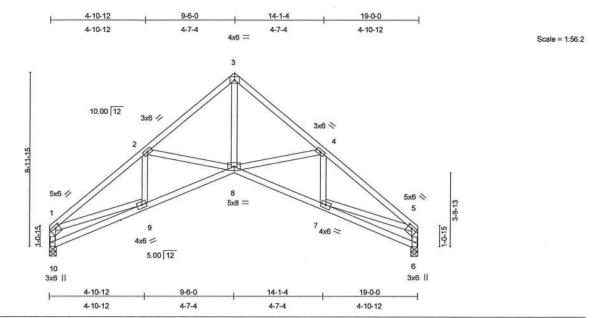


Plate Offsets	(X,Y):	[1:0-2-12,0-1-8],	[5:0-2-12,0-1-8]
		7		

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	0.13	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.08	7-8	>999	240	and the second second	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.54	Horz(TL)	-0.12	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	-(Mat	rix)						Weight: 113 lb	

LUMBEK	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 6-1-15 oc

bracing.

REACTIONS (lb/size) 10=599/0-4-0, 6=599/0-4-0

Max Horz 10=-217(load case 4)

Max Uplift 10=-349(load case 6), 6=-349(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-107

1-2=-1073/1650, 2-3=-864/1195, 3-4=-864/1195, 4-5=-1073/1650, 1-10=-630/951,

5-6=-630/951

BOT CHORD

9-10=-336/211, 8-9=-1012/833, 7-8=-1012/833, 6-7=-273/170

WEBS

2-9=-245/55, 2-8=-202/524, 3-8=-1345/752, 4-8=-202/524, 4-7=-245/55,

1-9=-858/621, 5-7=-858/621

JOINT STRESS INDEX

1 = 0.79, 2 = 0.43, 3 = 0.43, 4 = 0.43, 5 = 0.79, 6 = 0.65, 7 = 0.28, 8 = 0.49, 9 = 0.28 and 10 = 0.65

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Les Truss Design Engineer Fuela PE No. 34868 1100 Caastal Bay Blod Boynton Beach, Ft. 33435

Continued on page 2

January 9,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T26	SCISSOR	3	1		J1922737
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:35 2008 Page 2

NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 10, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 349 lb uplift at joint 10 and 349 lb uplift at joint 6.

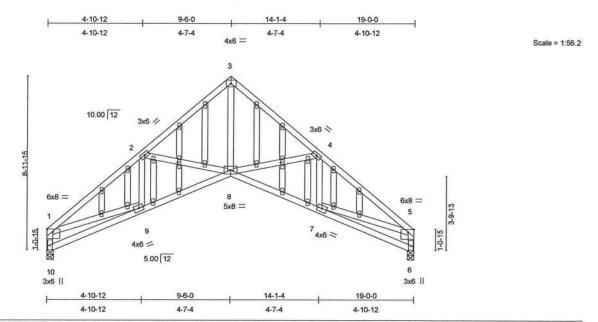
LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonds ME No. 24869 1 109 Geestel Bey Blyd Boynton Besch El 22435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T26G	GABLE	3	1		J1922738
OLAL INCOOLS	1200	OADEL	3	,	Job Reference (optional)	9

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:36 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	0.15	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	-0.09	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.62	Horz(TL)	-0.14	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)						Weight: 149 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		5-6-10 oc purlins, except end verticals.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 5-7-0 oc
OTHERS	2 X 4 SYP No.3		bracing.

REACTIONS (lb/size) 10=692/0-4-0, 6=692/0-4-0

Max Horz 10=-271(load case 4)

Max Uplift 10=-553(load case 6), 6=-553(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1243/1917, 2-3=-1001/1407, 3-4=-1001/1407, 4-5=-1243/1917,

1-10=-732/1127, 5-6=-732/1127

BOT CHORD 9-10=-424/257, 8-9=-1234/962, 7-8=-1234/962, 6-7=-317/195

WEBS 2-9=-210/40, 2-8=-226/566, 3-8=-1535/863, 4-8=-226/566, 4-7=-210/40,

1-9=-1025/718, 5-7=-1025/718

Truse Design Engineer Flonds FE No. 34869 1109 Ceastal Bay Blvn Boynton Beach, Ft. 33435

JOINT STRESS INDEX

1 = 0.50, 2 = 0.43, 3 = 0.50, 4 = 0.43, 5 = 0.50, 6 = 0.69, 7 = 0.28, 8 = 0.49, 9 = 0.28, 10 = 0.69, 11 = 0.33, 12 = 0.33, 13 = 0.45, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.33, 21 = 0.33,

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T26G	GABLE	3	1		J1922738
OLAL INCOOLS	1200	OABEE			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:36 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 10, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 553 lb uplift at joint 10 and 553 lb uplift at joint 6.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-64(F=-10), 3-5=-64(F=-10), 8-10=-10, 6-8=-10

Julius Lee Truse Design Engineer Florida PE No. 34868 1100 Geastel Bay Blvd Bovnton Besch, FL 33435



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T27	COMMON	1	1		J1922739
				1 .	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:37 2008 Page 1

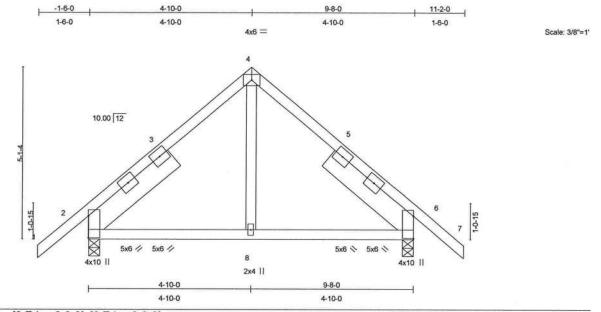


Plate Of	fsets (X,Y	'): [2:Edge,0-0-0], [6:	Edge,0-0-	0]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	0.02	6-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.01	6-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 67 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

SLIDER

Left 2 X 8 SYP No.1D 3-3-12,

Right 2 X 8 SYP No.1D 3-3-12

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=390/0-4-0, 6=390/0-4-0

Max Horz 2=132(load case 5)

Max Uplift 2=-247(load case 6), 6=-247(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/26, 2-3=-328/362, 3-4=-215/385, 4-5=-215/385, 5-6=-328/362, 6-7=0/26

BOT CHORD

2-8=-119/165, 6-8=-119/165

WEBS

4-8=-289/147

JOINT STRESS INDEX

2 = 0.31, 2 = 0.06, 2 = 0.06, 3 = 0.00, 4 = 0.38, 5 = 0.00, 6 = 0.31, 6 = 0.06, 6 = 0.06 and 8 = 0.10

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp Hilliam B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; porch left and Hilliam B; enclosed; because the second gust and the second gust are second gust by the second gust are second gust by the second gust are second gust by the second gust by t right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

January 9,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	V1000000000000000000000000000000000000
SEAL TRUSSES	T27	COMMON	1	1		J1922739
CONTENT MATERIAL CONTENTS OF THE STATE OF TH					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:37 2008 Page 2

NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 2 and 247 lb uplift at joint 6.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T27G	GABLE	1	1		J1922740
02.12.11.00020	1.2.0	3,132			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:38 2008 Page 1

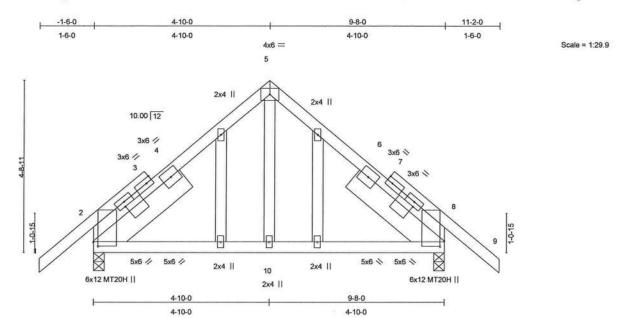


Plate Offsets (X,Y): [2:0-1-8,0-1-8], [2:1-3-8,0-2-4], [8:0-1-13,0-1-8], [8:1-3-5,0-2-0]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.02	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.01	8-10	>999	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.09	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)						Weight: 80 lb	

11	11	ME	RE	R

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

OTHERS 2 X 4 SYP No.3

SLIDER Left 2 X 8 SYP No.1D 3-0-13, Right 2 X 8 SYP No.1D 3-0-13

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=451/0-3-8, 8=451/0-3-8

Max Horz 2=-143(load case 4)

Max Uplift 2=-357(load case 6), 8=-357(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/52, 2-3=-332/347, 3-4=-276/350, 4-5=-232/360, 5-6=-232/360, 6-7=-276/350,

7-8=-332/347, 8-9=0/52

BOT CHORD 2-10=-125/180, 8-10=-125/180

WEBS 5-10=-251/120

JOINT STRESS INDEX

2 = 0.88, 2 = 0.01, 2 = 0.01, 3 = 0.00, 3 = 0.02, 3 = 0.07, 4 = 0.00, 5 = 0.38, 6 = 0.00, 7 = 0.00, 7 = 0.07, 7 = 0.02, 8 = 0.88, 8 = 0.01, 8 = 0.01, 10 = 0.10, 11 = 0.00, 12 = 0.00, 13 = 0.00 and 14 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

Julius Les Truse Design Engineer Flonda PE No. 24868 1109 Ceastal Bay Blvd Bovnton Besch, FL 33495



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T27G	GABLE	1	1	J19227	740
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:38 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 2 and 8.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 357 lb uplift at joint 2 and 357 lb uplift at joint 8.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 2-8=-10, 1-5=-64(F=-10), 5-9=-64(F=-10)

Julius Lee Truse Design Engineer Florida PE No. 24869 1100 Createl Bay Blvd Boynton Beach, FL 33435





6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:39 2008 Page 1

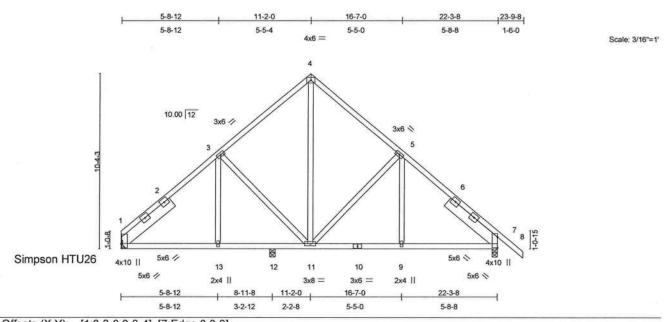


Plate Of	tsets (X, Y	(): [1:0-3-0,0-0-4], [7:	Edge,0-0-	-0]							7	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	0.07	1-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	-0.06	9-11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 156 lb	

	M		-	
u	IVI	п	_	к

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

SLIDER

Left 2 X 8 SYP No.1D 3-10-7,

Right 2 X 8 SYP No.1D 3-10-9

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=656/Mechanical, 7=760/0-4-0, 12=91/0-4-0

Max Horz 1=-288(load case 4)

Max Uplift 1=-298(load case 6), 7=-236(load case 7), 12=-33(load case 6) Max Grav 1=656(load case 1), 7=760(load case 1), 12=101(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-779/630, 2-3=-611/659, 3-4=-565/503, 4-5=-563/502, 5-6=-726/479,

6-7=-834/458, 7-8=0/26

BOT CHORD 1-13=-326/510, 12-13=-326/510, 11-12=-326/510, 10-11=-174/525, 9-10=-174/525,

7-9=-174/525

3-13=-227/115, 3-11=-235/455, 4-11=-431/301, 5-11=-248/241, 5-9=0/175

Julius Lee Truss Cesian Engineer Flonda FE No. 34889 1109 Geastal Bay Blvd.

JOINT STRESS INDEX

1 = 0.71, 1 = 0.14, 1 = 0.14, 2 = 0.00, 3 = 0.43, 4 = 0.48, 5 = 0.43, 6 = 0.00, 7 = 0.61, 7 = 0.15, 7 = 0.15, 9 = 0.33, 10 = 0.23, 11 = 0.56 and 13 = 0.33

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

January 9,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	HENDON-DARRESTON OF THE
SEAL TRUSSES	T28	COMMON	3	1		J1922741
OLAL MODDLO	120	COMMON			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:39 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 298 lb uplift at joint 1, 236 lb uplift at joint 7 and 33 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonda PE No. 34868 1199 Coastal Bay Blyd



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T28G	GABLE	1	1		J1922742
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:50:04 2008 Page 1

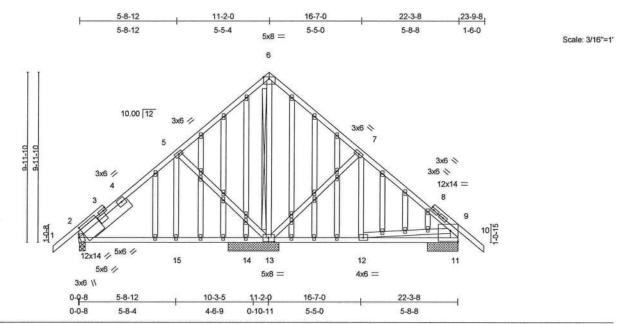


Plate Of	fsets (X,Y):	[2:0-4-4,0-5-4], [2:1	-6-11,0-2-0], [2:Edg	e,0-0-10]	, [9:Edge,0-7-8	3], [13:0-	4-0,0-3	3-0]			
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	0.06	2-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.03	2-15	>999	240	MANUSCONE .	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.41	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Mat	rix)						Weight: 237 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** 2 X 4 SYP No.2

2 X 4 SYP No.3 *Except* WEBS

9-11 2 X 6 SYP No.1D

OTHERS 2 X 4 SYP No.3

SLIDER Left 2 X 8 SYP No.1D 3-7-7 BRACING

TOP CHORD

BOT CHORD

WEBS

Rigid ceiling directly applied or 10-0-0 oc bracing. 2 X 4 SYP No.3 - 6-13 T-Brace:

Structural wood sheathing directly applied or 6-0-0

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum

end distance.

Brace must cover 90% of web length.

oc purlins, except end verticals.

REACTIONS

(lb/size) 2=706/0-4-0, 13=1104/2-11-11, 11=701/1-9-11, 14=8/2-11-11

Max Horz 2=-355(load case 4)

Max Uplift 2=-579(load case 6), 13=-766(load case 6), 11=-464(load case 7), 14=-62(load

Max Grav 2=713(load case 10), 13=1104(load case 1), 11=701(load case 1), 14=40(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-32/75, 2-3=-565/545, 3-4=-453/503, 4-5=-407/487, 5-6=-173/183, 6-7=-161/175,

7-8=-278/240, 8-9=-547/348, 9-10=-40/92, 9-11=-674/584

2-15=-350/314, 14-15=-355/312, 13-14=-355/312, 12-13=-52/303, 11-12=-220/327

BOT CHORD WEBS 5-15=-243/145, 5-13=-419/672, 6-13=-494/314, 7-13=-423/451, 7-12=0/173, 9-12=-53/215

idikis Lere NUSS Design Engineer Ionde PE No. 24869 100 Caasial Bay Blvri Ioynton Besch, FL 33436

JOINT STRESS INDEX

2 = 0.80, 2 = 0.11, 2 = 0.09, 2 = 0.61, 3 = 0.00, 3 = 0.00, 3 = 0.15, 4 = 0.00, 5 = 0.45, 6 = 0.80, 7 = 0.45, 8 = 0.00, 8 = 0.34, 8 = 0.47, 8= 0.47, 9 = 0.45, 11 = 0.00, 12 = 0.26, 13 = 0.39, 14 = 0.34, 15 = 0.34, 16 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 18 = 0.34, 19 = 0.34, 19 = 0.34, 1820 = 0.34, 21 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 28 = 0.34, 29 = 0.34, 300.34, 31 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 34 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.34 and 39 = 0.34 January 9,2008

Continued on page 2

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T28G	GABLE	1	1		J1922742
02.2 11.00020	1.200	0,1022			Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 13:50:04 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 579 lb uplift at joint 2, 766 lb uplift at joint 13, 464 lb uplift at joint 11 and 62 lb uplift at joint 14.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 10) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

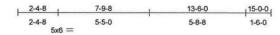
Vert: 1-6=-91(F=-37), 6-9=-91(F=-37), 9-10=-91(F=-37), 2-11=-10

Julius Lee Truse Cesign Engineer Florida PE No. 24869 1189 Ceastel Bay Blvd Boydon Baser Et 2244



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T29	COMMON	2	1		J1922743
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:41 2008 Page 1



Scale: 3/16"=1"

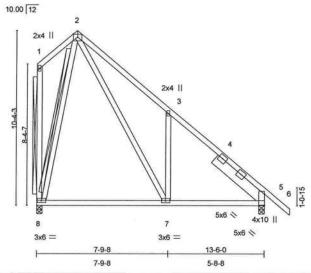


Plate Of	fsets (X,Y	'): [5:Edge,0-0-0]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.08	7-8	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.14	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.98	Horz(TL)	0.01	5	n/a	n/a		
BCDL	BCDL 5.0 Code FBC2004/TPI2002		(Mat	rix)						Weight: 112 lb		

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

SLIDER

Right 2 X 8 SYP No.1D 3-10-9

BRACING

TOP CHORD

BOT CHORD

6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc

Structural wood sheathing directly applied or

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 - 1-8, 2-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=423/0-4-0, 5=513/0-4-0

Max Horz 8=-309(load case 7)

Max Uplift 8=-185(load case 7), 5=-88(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-40/60, 2-3=-456/355, 3-4=-299/63, 4-5=-490/44, 5-6=0/26, 1-8=-56/43

BOT CHORD 7-8=-26/334, 5-7=0/277

WEBS 2-7=-430/406, 3-7=-281/411, 2-8=-375/327

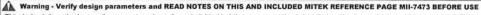
JOINT STRESS INDEX

1 = 0.28, 2 = 0.38, 3 = 0.20, 4 = 0.00, 5 = 0.30, 5 = 0.09, 5 = 0.09, 7 = 0.32 and 8 = 0.49

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

January 9,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occup. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T29	COMMON	2	1		J1922743
			-		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:41 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 8 and 88 lb uplift at joint 5.

LOAD CASE(S) Standard

January 9,2008



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	104040004004000
SEAL TRUSSES	T30	MONO TRUSS	6	1		J1922744
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:41 2008 Page 1

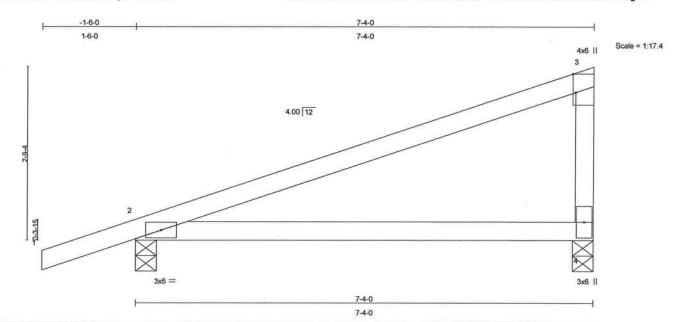


Plate Of	fsets (X, Y	(): [3:0-3-8,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	0.19	2-4	>446	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.10	2-4	>846	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	August 100 to 100 to					Weight: 28 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.
BOT CHORD	2 X A SVP No

2 X 4 SYP No.3 WEBS

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=214/0-4-0, 2=325/0-4-0

Max Horz 2=102(load case 4)

Max Uplift 4=-154(load case 4), 2=-216(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/24, 2-3=-189/165, 3-4=-154/174

BOT CHORD

2-4=-234/123

JOINT STRESS INDEX

2 = 0.49, 3 = 0.68 and 4 = 0.75

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 Coltinged anipiate and 216 lb uplift at joint 2.

January 9,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	VALUE OF THE PARTY
SEAL TRUSSES	T30	MONO TRUSS	6	1		J1922744
SEAL TROSSES	130	WONO TROSS	0	,	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:41 2008 Page 2

LOAD CASE(S) Standard

January 9,2008





6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:42 2008 Page 1

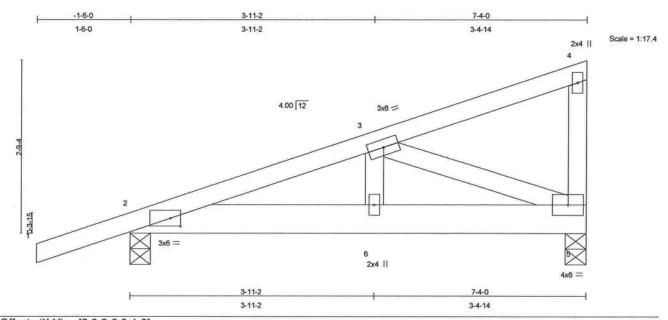


Plate Of	fsets (X, Y	<u>(): [2:0-2-0,0-1-8]</u>										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.02	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.03	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.21	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)	, ,					Weight: 40 lb	

	M		_	п
	IVI	ь	_	ĸ

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 5=577/0-4-0, 2=688/0-4-0

Max Horz 2=103(load case 3)

Max Uplift 5=-254(load case 3), 2=-318(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/28, 2-3=-928/333, 3-4=-74/8, 4-5=-76/38

BOT CHORD

2-6=-366/850, 5-6=-366/850

WEBS

3-6=-174/392, 3-5=-862/369

JOINT STRESS INDEX

2 = 0.43, 3 = 0.28, 4 = 0.48, 5 = 0.24 and 6 = 0.28

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 5 and 318 lb uplift at joint 2.

Ebhirder ar isagiezin span(s): 8-9-8 from 0-0-0 to 7-4-0

January 9,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T31	MONO TRUSS	1	1		J1922745
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:42 2008 Page 2

NOTES

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

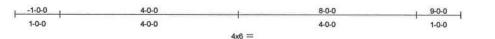
Vert: 1-4=-54, 2-5=-113(F=-103)

January 9,2008





6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:42 2008 Page 1



Scale = 1:24.3

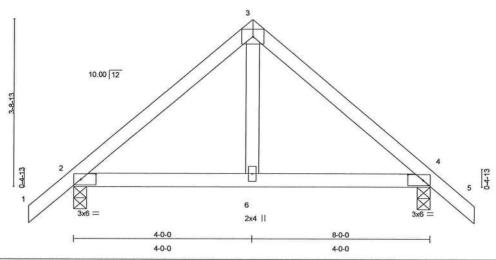


Plate Offsets	(X,Y)	[2:0-4-	-1,0-1-8],	[4:0-4-1	,0-1-8]
---------------	-------	---------	------------	----------	---------

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	4-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 37 lb	

 10.1	m	_	
 IJν	ıĸ	-	ĸ

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=309/0-3-8, 4=309/0-3-8

Max Horz 2=-93(load case 4)

Max Uplift 2=-101(load case 6), 4=-101(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/34, 2-3=-274/122, 3-4=-274/122, 4-5=0/34

BOT CHORD

2-6=-5/158, 4-6=-5/158

WEBS

3-6=0/133

JOINT STRESS INDEX

2 = 0.32, 3 = 0.29, 4 = 0.32 and 6 = 0.09

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truse Design Engineer Florida FE No. 34869 1109 Coestal Bay Blvd Boynton Beach, FL 33435

January 9,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	The second secon
SEAL TRUSSES	T32	COMMON	2	1		J1922746
The second secon					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:42 2008 Page 2

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 2 and 101 lb uplift at joint 4.

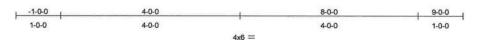
LOAD CASE(S) Standard

January 9,2008



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T32G	GABLE	1	1		J1922747
	1015.000				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:43 2008 Page 1



Scale = 1:24.3

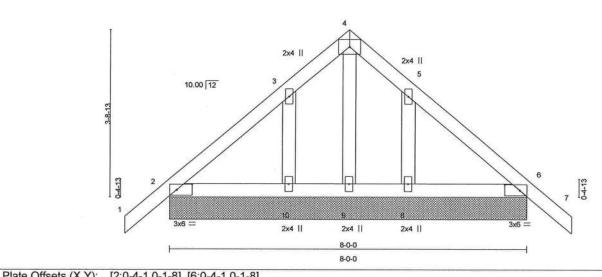


Plate Of	isets (A, 1). [2.0-4-1,0-1-6], [6.	.0-4-1,0-1-	0]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.08	Vert(LL)	-0.00	6	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	0.00	6	n/r	90		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)						Weight: 43 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=142/8-0-0, 6=142/8-0-0, 9=28/8-0-0, 10=154/8-0-0, 8=154/8-0-0

Max Horz 2=-93(load case 4)

Max Uplift 2=-50(load case 6), 6=-64(load case 7), 10=-79(load case 6), 8=-77(load

case /)

Max Grav 2=142(load case 1), 6=142(load case 1), 9=67(load case 7), 10=155(load case 10), 8=155(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-65/61, 3-4=-41/105, 4-5=-41/105, 5-6=-55/41, 6-7=0/33

BOT CHORD 2-10=-4/121, 9-10=-4/121, 8-9=-4/121, 6-8=-4/121

WEBS 4-9=-91/0, 3-10=-123/150, 5-8=-123/150

JOINT STRESS INDEX

2 = 0.12, 3 = 0.07, 4 = 0.04, 5 = 0.07, 6 = 0.12, 8 = 0.08, 9 = 0.03 and 10 = 0.08

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Les Truss Design Engineer Florida FE No. 34889 1109 Coestal Bay Blvd Boynton Beach, FL 9349:

January 9,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / GEORGIA HOME	
SEAL TRUSSES	T32G	GABLE	1	1		J1922747
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 03 16:40:43 2008 Page 2

NOTES

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 2, 64 lb uplift at joint 6, 79 lb uplift at joint 10 and 77 lb uplift at joint 8.

LOAD CASE(S) Standard

January 9,2008

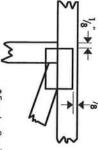


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless plates to both sides of truss and Dimensions are in inches. Apply securely seat. dimensions indicate otherwise



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 × 4

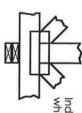
dimension is the length parallel The first dimension is the width perpendicular to slots. Second to slots.

LATERAL BRACING



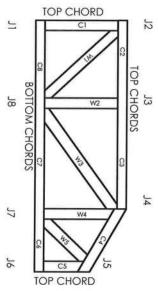
continuous lateral bracing. Indicates location of required

BEARING



which bearings (supports) occur Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

ICBO BOCA

9667, 9432A

3907, 4922

96-31, 96-67

SBCCI

WISC/DILHR

960022-W, 970036-N

561

NER



MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Damage or Personal Injury

Failure to Follow Could Cause Property

5 ω 12 Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

Unless otherwise noted, locate chord splices

Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.

Cut members to bear tightly against each

- Unless otherwise noted, moisture content of at 1/4 panel length (± 6" from adjacent joint.
- 0 lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant or Camber is a non-structural consideration and is the responsibility of truss fabricator. General preservative treated lumber.
- 00 Plate type, size and location dimensions shown indicate minimum plating requirements.

practice is to camber for dead load deflection

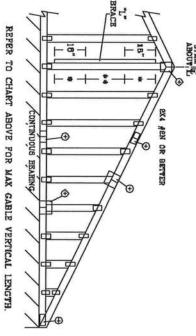
- 9 Lumber shall be of the species and size, and grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- Do not overload roof or floor trusses with stacks of construction materials.
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling, erection and installation of trusses.
- © 1993 MiTek® Holdings, Inc.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

		MAX GABLE VERTI	CAL LENGT	CH
		12 201421	24" O.C.	GABL
		SPF 43 CONNECT DIAGONAL AT MEDIUM OF VERTICAL LENGTH SI 14: CONNECT DIAGONAL AT MEDIUM OF VERTICAL LENGTH SI 14: CONNECT DIAGONAL AT MEDIUM OF VERTICAL	SPF SP DFL	GABLE VERTICAL SPACING SPECIES
	STACE PLANT	STANDARD	#3 STUD STANDA #2 #2 #3 STUD STANDA #3 #3 STUD STANDA	AL BRACE
	ARMING** TRU ZING. REFER TI E (NSTITUTE, 1 WERICA, 6300 SE FUNCTIONS.	G. B. C.		BRACES
	SSES REQUIRE D BCS) 1-03 (8 SS D DUNDTRID ENTERPRISE LIN UNLESS DITER S AND BOTTON	/ 	5 5 5 5 4 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1	(1) 1X4 GROUP
	"" WARNING" TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 BOILDING COMPORNT SAFE THE PROPERTY OF THE STORE THE STORE INSTALLING AND PUBLISHED BY 18TH STORE THE STORE INSTALLING AND REFER FACTORS. CHARGE THE STORE THE STORE IN THE STORE THE STOR	6. 8, 6, 10. 6. 10. 6. 10. 6. 8, 6. 10. 6. 0. 6. 6. 8. 6. 8. 6. 2. 6. 2. 6. 2. 6. 2. 6. 2. 6. 11. 6.		°L" BRACE A GROUP
	IN FABRICATING WENT SAFETY IN WENT SAFETY IN FOR SAFETY IN STRIP FOR SAF IN O, TOP CHORD I WAVE A PROPER		က က တ တ တ တ တ တ တ	• (1) 2X4 *
	LT ATTACHED S		ପ୍ରୀଷ୍ଟ୍ରୀଷ୍ଟ୍ର ଫ୍ରିସ୍ସ୍ସ୍ୟୁସ୍	(1) 2X4 "L" BRACE *
	IGID CEILING			
	FIRMS AND CLINCS S COLUNCIL LING AND CLINCS COLUNCIL LING COLUNCIL LING COLUMN	2 SIG (2) (2)	2 B B B 7 B B B B B B B B B B B B B B B	(2) 2X4 "L" BRACE ** (1) 2X6 "L" GROUP A GROUP B GROUP A
STATE O	CONS. ENC CONS. ENC DELEAN HEACH	BEATER BETTER BE		(1) 2X6 T
of Florida	INEERS NAMED IN STATE OF THE PARTY OF THE PA		11. 2. 10° 0. 10° 0. 11° 8. 8. 10° 11° 8. 10° 10° 4. 10° 3. 10° 10° 10° 10° 10° 10° 10° 10° 10° 10°	63
	P.A.	VERTICAL LENGTH	12' 11' 12' 12	
MAX. TOT. LD.			12 11 12 11 12 11 12 11 12 11 12 11 12 11 12 11 12 11 12 11 12 12	BRACE **
60 PSF 24.0"	מוטוט	GROUP B HEM-PIR HI & BITE HI & BITE GABLE TRUSS DETA LIVE LOAD DEPLECTION CHITERIA PROVIDE UPLIT CONNECTIONS F CONTINUOUS BEARING (8 PSIP GABLE END SUPPORTS LOAD FIR OUTLIDAKERS MITH 2 O OVER HACE SUPPORTS LOAD FIR OUTLIDAKERS MITH 2 O OVER HACE SUPPORTS LOAD FIR OUTLIDAKERS MITH 2 O OVER HOR (1) "L" BRACES SPACE HOR (2) "L" BRACES SPACE HOR (3) "L" BRACES SPACE HOR (4) "C" GREATER THAN 10" B" GREATER THAN 11" 6" GREATER THAN 11" 6" GREATER THAN 11" 6" GREATER TO COMMON TRUSS HERET TO COMMON TRUSS HEMPLOYMENT CONTROLL SPACE HOR (1) "L" BRACES SPACE HOR (BRACING GROUP A: SPRUCE-PINE-WB 41 / 42 STANDARD DOUGLAS FIR-LARCH STUD STUNDARD STUNDARD STUNDARD STUNDARD STUNDARD	
ENG	REF ASCEY-02-GAB13015 DATE 11/26/03 DRWG miek std cable 15 e ht	GROUP B: HEM-PIR HE		i c
	-CAB13015)3 Ble 15 e ht	TES: 10. 11ES: 10. 11ES: 10. 11E 0.0. 1	AND GRADES: HEM-PIR STUD STANDARD STUD TIHERN PINE #3 STUD STUD THERN PINE #3 STUD THERN PINE #3	

ASCE 7-02: 130 MPH WIND SPEED, 30, MEAN HEIGHT, ENCLOSED, I II 1.00, EXPOSURE 0

_	_	_	_	_		_	_	_	_			_			_	_	_	_	_	_	_	_	_		_				
		,	M	Α	X		(j A	4]	3]		E		V	F	R	r	ľ	C	A	L		L	E	'N	1(1	ΓH	
		1	2	91		0	.(3			1	6	"		0	.(3			2	4	33		C),(С		SPACING	GARI
		レデー	1	V.	2	TII	I I	015	CDT			1	<i>ن</i> .	2	111	H	OI I	CDT		しずし	1	<i>V.</i>)	TII	I I	טלי	C D I	SPACING SPECIES	GABLE VERTICAL
	STANDARD	STUD	‡ 3	#2	#1	STANDARD	STUD	* 3	\$1 / #2	STANDARD	STUD	† 3	#2	# 3	STANDARD	STUD	#3	\$1 / #Z	STANDARD	STUD	‡ 3	#2	11	STANDARD	STUD	# 3	\$1 / #2	GRADE	BRACE
	4' 0"	4. U	4. 23	4' 4'	4. 5.	3' 11"	3' 11.	3' 11"	4.0	3' B.		3. 8,	3' 11"	4. 0.					3' 0"						3' 1"	3' 1"	ω. ω.	BRACES	Z S
	5 6	6. 4.	6' 6"	6' 11"	6' 11"		6' 3			-	5 6	5. 7.	8' 4"	6.	4. 8.	5, 5,	U. U.	6 4	3' 10"	4' 8"	4. 6.	5' 6"	5' 6"	3' 9"	4' 6"	4' 5	5. 6.	GROUP A	(1) 134
	5' 6"	6. 4.	6' 5"	7º 6°	7' 6"	5' 4"	6' 3"		7. 5.	4' 9"	5' 6"	6. 7.	8' 10"	B 10"	4. 8.	6' 5"		6. 6.	3' 10"	4' 6"	4. 6.	5' 11"	5' 11"	3. 9.	4' 5"	4. 5.	6.8.	GROUP B	"L" BRACE .
	7' 3"	8' 3"	8, 3,	8' 3"	8, 3,	7' 1"	8' 3"	8' 3"	8' 3"	6' 3"	7' 3"	7' 4"	7' 8"	7' 6"	6. 5.	7. 22.	7 2	7' 6"	5" 1"	5' 11"	6. 0.	6, 8,	6' 6"	6. 0.	5' 10"		6 6.	GROUP A	Н
3	7' 3"	8. 6.	8, 6,		B' 11"	7, 1,	B' 3°	8' 3"	8. 8.	6' 3"	7' 3"	7. 4"	8' 1"	B' 1"	6. 2.	7' 2"	7' 2"	7. 8.	5 1	5' 11"	6. 0.	7' 0"	7' 0"	5. 0.	5' 10"		6. 8.	GROUP B	(1) 2X4 "L" BRACE .
NI MAS	8.8	9. 10.	1.00	9' 10"		9' 6"	9' 10"	9, 10,	9. 10	8 5	a' 11"	8. 11	8' 11"	8' 11"	8' 3"	8' 11"	B' 11"	8, 11.	8° 11"	7' 10"	7' 10"	7' 10"	7' 10"	6. 9.	7' 10"	7' 10"	7 10	GROUP A	(2) 2X4 "L"
	8, 8,	10. 4.	10' 4"	10' 7"	10' 7"	g, e,	9' 10"		10, 1,		8, 5,	9. 6.	8, 3,	8, 2,	8' 3"	B' 11"		9. 2.	6, 11,	8' 0"	8. 1.	- 1	1	6. 9.	7' 10"		8.0.	GROUP	BRACE **
	11' 4"		12, 11,				12, 10.			8, 8,	11' 4"	11. 5.	11. 9.	11, 8,	9. 7"		11' 2"	11. 9.	B! 0*	8 3	9. 4.		10' 3"			9' 1"	10' 3"	B GROUP A	(1) 2X6
	11' 4"		13' 3'		13' 11"		12' 10"	12' 11"	13' 4"	9, 9,,	11, 4,			12' B"	8. 4.	11. 1	11' 2"	12' 1"	8.0.	9 3	9. 4."	11, 1		7 10"	9′ 1°	9' 1"	10' 7'	GROUP	"L" BRACE *
	14' 0"	14. 0	14' 0"		14 0	14' 0"	14' 0"	14.0	14. 0.	13' 3"	14.0	14. 0.	14' 0"	14 0	12. 11.	14' 0"	14' 0"	14. 0.		12' 3"	12. 3.	12' 3"	12' 3"	10. 7.	12' 3"	12' 3"	12. 3.	B GROUP A GROUP B	(2) ZXB "L" BRACE
	14' 0"	14. 0	14' 0"	14' 0"	14. 0.		14. 0"	14 0	14 0"	13' 3"	14.0			14' 0"	12. 11.	14" D"		14. 0.			12. 8.	13′ 2"	13' 2"	10. 7.	12, 3,	12' 3"	12' 7"	GROUP B	BRACE **
CABBLE BUD SUPPORTS LOAD FROM 4 0	CUMILINGUID BEARING (O FOF IC DEAD LIAD).	CONTINUE OF THE CONNECTIONS FOR 180 PLF OVER	The same of the sa	ו אומשתמט אחושיים ומפון חאם שעון	CABLE INUSS DETAIL NOTES:				#2	_	SOUTHERN PINE DOUGLAS FIR-LARCH		#1 & BTR	HEM-PIR	GROUP B:			STATUARD	I	\$13	DOUGLAS FIR-LARCH SOUTHERN PINE	STOP STOP	100	A-MI	JUP A:		BRACING GROUP SPECIES AND GRADES:		



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WINN DIAGONAL
BRACE IS USSED. CONNECT
BRACE IS USSED. CONNECT
BRACE IS USSED. TOWN BED
AT EACH END. MAX WEB
TOWAL LENGTH IS 14.

GABLE TRUSS

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZX4 SP OR
DIT-L #Z OR
BETTER DIAGONAL
BRACE, SINGLE
OR DOUBLE
CUT (AS SHOWN)
AT UPPER END

CONNECT DIAGONAL AT AMERICAL WEB.

† |o| | |o| |c| |

CABLE
TRUSS
DETAIL
NOTES:

ABLE END SUPPORTS LOAD FROM 4' 0"
DUTLIDEMERS WITH 2' 0" DVERHANG, DR 12"
PLYWOOD OVERHANG. E LOAD DEPLECTION CRITERIA IS L/240. ONTINUOUS BEARING (6 PSF TC DEAD LOAD).

ATTACH EACH 'L' BRACE WITH 10d NAILS.

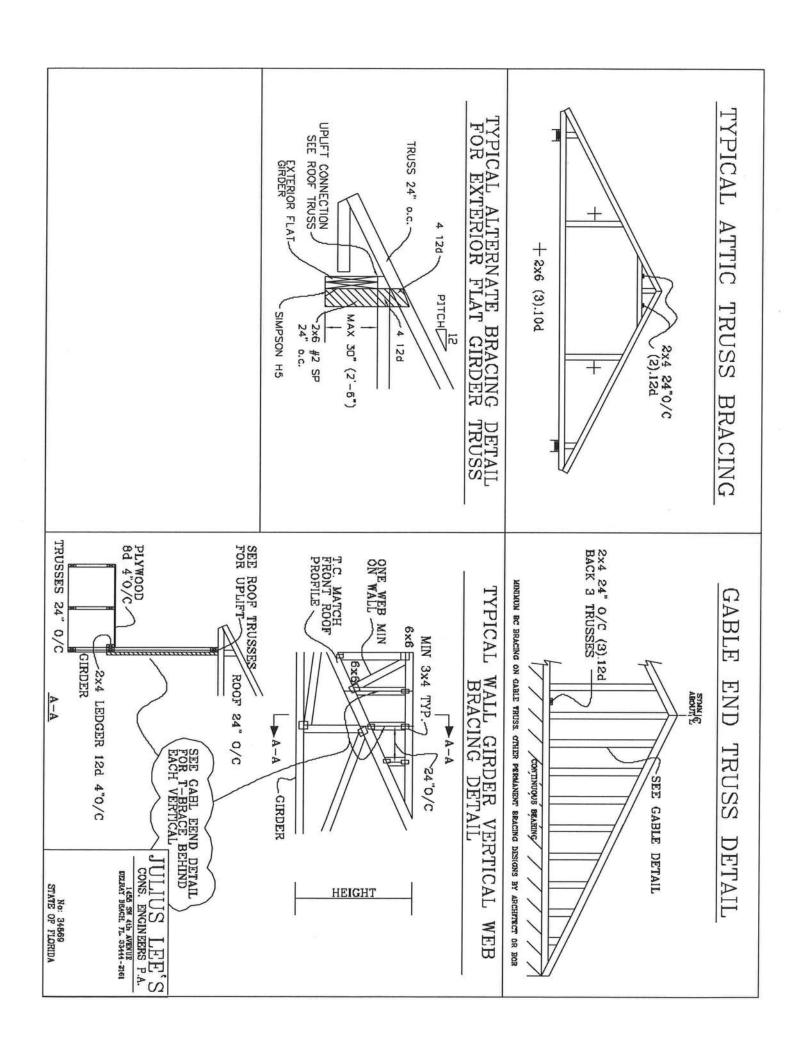
FOR (1) 'L' BRACE: SPACE NAILS AF 2° O.C.

FOR (2) 'L' BRACES: SPACE NAILS AT 3° O.C.

IN 18° END ZONES AND 6° O.C. BETWEEN ZONES. "I." BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

FOR	PLATES.	HERL F	COMMON	PEAK, SPI
L	2.5X4	I.	R THAN 11' 6	REATE
	274	TUB	THAN 1 D	REATE
EX3	IX4 DR		HAN 4' 0"	PSS T
S	NO SPLI		TITICAL LENGTH	VER
"	E SIZES	PLATE	E VERTICAL	GABLE

			STRUCTURAL PANELS AND BUTTON CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CELLING	PLATE INSTITUTE, 383 D'OUPRIO DE, SUITE 200, MAJISON, ME SZYPR) AND VICA (MODD TRUSS CONCIL. OF AMERICA, 630 ENTERPRISE LA, MAGISON, VI SZYPR) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS, UNILESS OTHERMES INDICATED, THE PLATED SAME HAVE PROPERLY ATTACHED.	MAYARADACIA TRUSSES REQUIRE EXTREME CARE IN FAMILIANG, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 CHUILIANG COMPONENT SAFETY (MEDRANTIDA), PUBLISHED BY FPI CIRUSS	•
STATE OF FLORIDA	7.000			DELRAY BEACH, PL 33444-2161	CONS. ENGINEERS P.A.	S'AE'I SUL'IN
MAX. SPACING 24.0"		MAX. TOT. LD. 60 PSF				
			-ENG	DWG MITEK STD GABLE SO' E HT	DATE 11/26/03	REF ASCEY-02-GABI3030



TOP CHORD BOT CHORD WEBS 2X4 2X4 30,00 222 BETTER BETTER BETTER

PIGGYBACK DETAIL

TYPE

SPANS

Ϋ́

30

34

8 5

52

2.5X4

2.6X4

5X8

5X8

5X6 3X5 REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS. REFER TO ENCINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, I MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C. WIND TC DL=6 PSF, WIND EC DL=6 PSF

D C Ħ >

5X4 EXG. 4X8 2X4

5X6

6X5

1.5X4

1.5X4

1.5X4 5X6

4X6 OR 3X6 TRULOX AT 4' OC, HOTATED VERTICALLY

FRONT FACE (B,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. LOCATION IS ACCEPTABLE XX Z 20' FLAT TOP CHORD MAX SPAN 品 TYP W MAX SIZE OF ZX12 W C-TYP. D-SPLICE

ř.	= mm>
	A CONT
	SOFE
	TION PER
	N T N
	E S
	PER
	Z P S
	0.43
	R + 6
	ING NO.
	182
	SEX
	HH
	25.5
	NAILS, MEMBER TRULO
	ੂ ਤੇ

TOWAS GOW
WEB LENGT
o' TO 7'9"
7'9" TO 10'
10' TO 14'

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS BYTH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

N 3

	STHIL
	DRAWING
	REPLACES
	THIS DRAWING REPLACES DRAWINGS 634,016 834,017 & 847,04
	634,016
	634,017
	8
	847.0
-1	-

		NAVARNINGME TRUSSES REQUIRE EXTREME (N. FABRICATING, HANDLING, SHIPPING, INSTALLING AND BAACING, REFER TO SEST I-DD GUILLING COMPONENT SAFETY INFORMATION), PUBLICINED BY TRI CRUSS PLATE INSTITUTE, 383 O'D-DUTRID DR. SUNITE 289, MANDEON, VI. 52799) AND WITCA COUDD BRASS COLNETIL OF MERCH, 5500 CHERRERSES CH. HANDLESS L. 1, 23739 FOR SMILLT HAVE PROPERTY ATTACHED THE SERVICE IN HANDLESS L. 1, 23739 FOR SMILLT HAVE PROPERTY ATTACHED STRUCTURAL PANCLS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CEILING.	*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.
STATE OF FLORIDA	:	JULIUS LEEKS P.A. 1400 SW 4th AVENUE DELEANY BEACH, FL. 33444-2161	THIS DRAWI
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	MAX LOADING 55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC.	NG REPLACES DRAWINGS
		DATE 09/12/07 DRWGMITEK STD PIGGY -ENG JL	9 69

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

*

"* ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

(2) 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET NUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

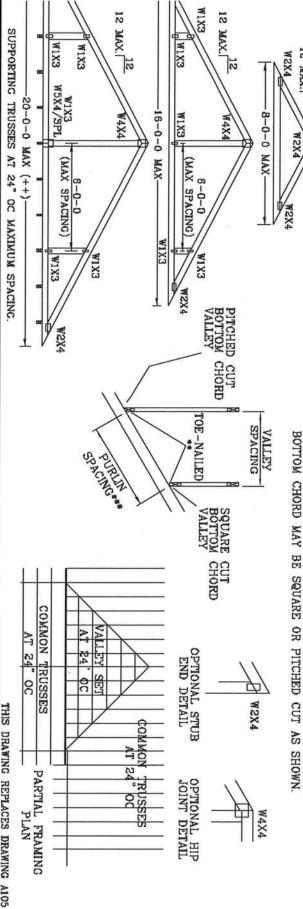
**+ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES

LARGER AS REQ'D

4-0-0 MAX

12 NAX.

NOT EXCEED 12'0".



			IG.	PERFORMING	TALLING AND		
STATE OF FLORIDA	No: 34869			DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S	
SPACING	DUR.FAC. 1.25	TOT. LD.	BC LL	BC DL	TC DL	TC LL	
	55	32	0	C,	~2	20	ŀ
24"	1.25	40	0	5	15	8	
		PSF	PSF	PSF	PSF 1	PSF	
			PSF -ENG JL	PSF DRWG	DATE	PSF REF	1
			JL	VALTRUSS1103	11/26/03	VALLEY DETAIL	The same of the sa

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

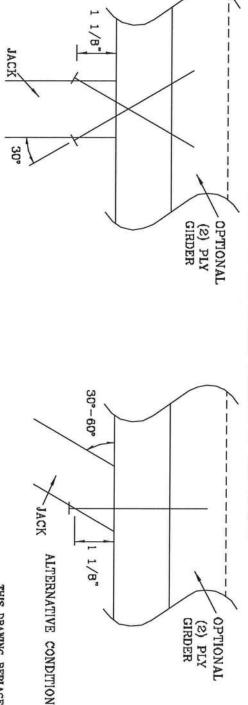
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUN VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

ALL MALLING MAY DE MILLIMINE CONTROLLED TO THE DESCRIPTION OF TAKEN	5 493#	4 394#	3 296#	2 197#	TUE-NAILS 1 PLY	NUMBER OF SOUTHERN PINE	
WILL WILL II	639#	511#	383#	256#	2 PLIES 1 PLY	N PINE	
77	452#	361#	271#	181#	1 PLY	DOUGLAS	
1	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH	
	390#	312#	234#	156#	1 PLY		
	507#	406#	304#	203#	2 PLIES	HEM-FIR	
	384#	307#	230#	- 154#	1 PLY	SPRUCE	
	496#	397#	298#	199#	2 PLIES	SPRUCE PINE FIR	

TAN



GIRDER

OPTIONAL (2) PLY

THIS DRAWING REPLACES DRAWING 7840	
WING REPLACES DRA	SIE
REPLACES DRA	DRA WIN
DRA	
DRAWING 7840	
G 7840	DRAWIN
	G 7840

			STRUCTURAL PANELS AND BOTTON CARDS SHALL HAVE A PROPERLY ATTACHED RIGID CELLING	PLATE INSTITUTE, 383 PONCERIO DR., SUTTE 200, NADISON, ME 30719) AND VTCA (NOOD TRUSS COLNCIL. Of Andrica, 6300 enterprise LNI, Maricon, VII 30719) for Safety Practices, Pride to Perdoning These Functions. United the Prince Indicated. The Chron trust have proper vitables.	***VARNING*** TRUSSES REDURE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BESI 1-03 COULDING COMPONENT SAFETY INTORNATION, PUBLISHED BY TPI CROUSS		
STATE OF FLORIDA	No: 34869			DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S	
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL	THIS
	1.00	PSF	PSF	PSF	PSF	PSF REF	DRAWING REI
			-ENG JL	DRWG	DATE		PLACES D
			JL	DRWG CNTONAIL1103	09/12/07	TOE-NAIL	THIS DRAWING REPLACES DRAWING 784040

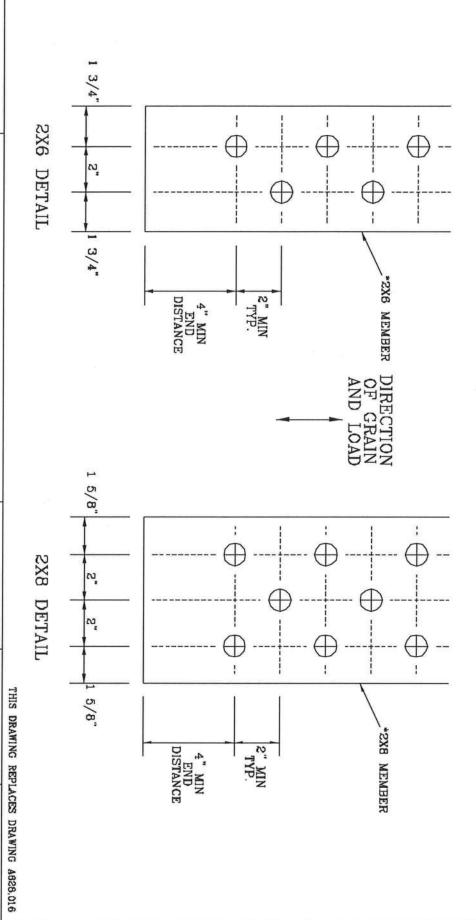
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



VARRING* TRUSSES REQUIRE EXTECNE CASE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BACCING, RETER TO BEST LONG GUILLING COPPONENT SAFETY INFEDIMENTS, POULTED BY THE ITEMSS PLAIT INSTITUTE, 590 CONCRETED DE XITTE 200, MISSISSIN, V. 37199 AND VICEA CAUDD TRUSS COUNCIL THE AMERICA, 6300 ENTERPRISE LA, MANDEON, VI. 37199 FOR SAFETY HAVE PROPERTY ATTACHED REPORTED TO THE AMERICA FAMILIANCE AND SAFETY ATTACHED ROUNCE STRUCTURAL PARELS AND SOTTOM COURD SHALL HAVE A PROPERTY ATTACHED ROUNCE CELLING.

			SPACING	STATE OF FLORIDA	
			DUR. FAC.	W. 24000	
		PSF	TOT. LD.		
JL	-ENG JL	PSF	BC LL		
CNBOLTSP1103	DRWG	PSF	BC DL	DELRAY BEACH, FL 33444-2161	ERF DRMING
11/26/03	DATE	PSF	TC DL	CONS. ENGINEERS P.A.	
BOLT SPACING	REF	PSF	TC LL	N, HH I SIII IIII	

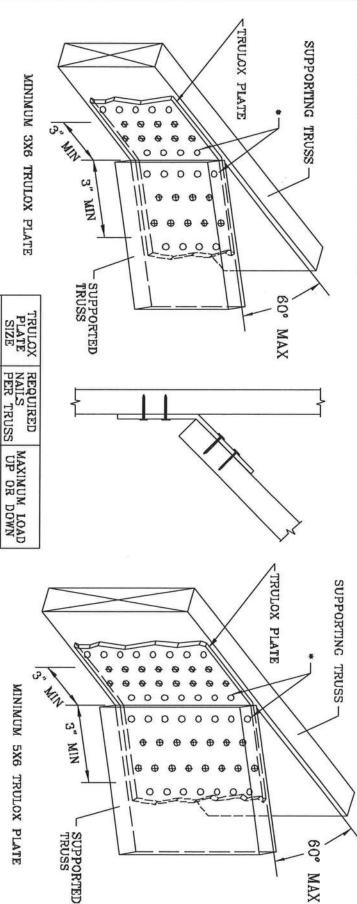
TRULOX CONNECTION

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (+).

NAILS MAY BE OMITTED FROM THESE ROWS THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING INFORMATION NOT SHOWN. THIS DETAIL FOR LUMBER, PLATES, AND OTHER



TRISSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIP
OR TO JUST 1-02 GUILLING COMPONENT SAFETY INFORMATION, PUBL
OR TO JUST SAFETY INFORMATION, PUBL
OR JUST SAFETY INFORMATION, SUITE 201, MAISSON, VI. 33719 MO VICE
OR SAFETY PACATICES POR
ONE, UNLISS OTHERVISE WIDDATED, ITP GOODS DALL, HAVE FOR
MELS AND JUSTION CHORD SHALL HAVE A PROPERLY ATTACHED RIGH
MELS AND JUSTION CHORD SHALL HAVE A PROPERLY ATTACHED RIGH
ONE.

5X6 3X6

15 9

#066 350#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

REF DATE

-ENG

DRWG CNTRULOX1103

11/26/03 TRULOX

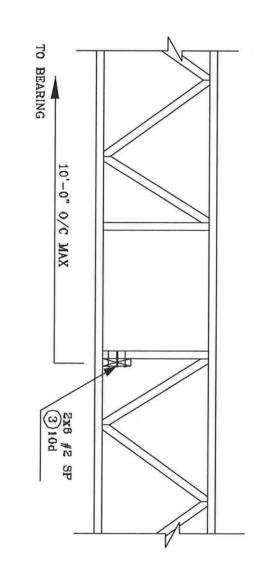
CONS. ENGINEERS P.A. DELRAY BEACH, IL 38444-2151

No: 34869 STATE OF FLORIDA

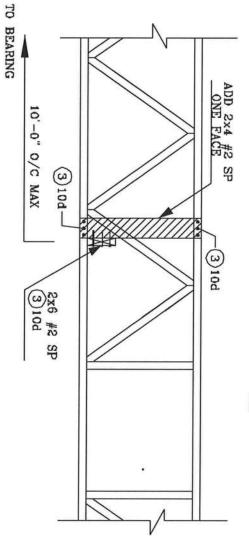
MAXIMUM LOAD UP OR DOWN

MINIMUM 5X6 TRULOX PLATE

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S CONS. ENGINEERS P.A.

1125 SM 4th NEWIZ
DEERN BEACH. TL. 33444-2361

No: 34869 STATE OF FLORIDA

COLUMBIA COUNTY 9-1-1 ADDRESSING / GIS DEPARTMENT

P. O. Box 1787, Lake City, FL 32056-1787 Telephone: (386) 758-1125 * Fax: (386) 758-1365 * E-mail: ron_croft@columbiacountyfla.com

ADDRESS ASSIGNMENT DATA

The Columbia County Board of County Commissioners has passed Ordinance 2001-9, which provides for a uniform numbering system. A copy of this ordinance is available in the Clerk of Court records, located in the courthouse. This new numbering system will increase the efficiency of POLICE, FIRE AND EMERGENCY MEDICAL vehicles responding to calls within Columbia County by immediately identifying the location of the caller.

Residential or Other Structure on Parcel Number: 03-4S-16-02731-111 (LOT 111 PRESERVE AT LAUREL LAKE UNIT 1.)

Address Assignment: 453 SW ROSEMARY DR, LAKE CITY, FL, 32024

Any questions concerning this information should be referred to the Columbia County 9-1-1 Addressing / GIS Department at the address or telephone number above.

COLUMBIA COUNTY 9-1-1 ADDRESSING APPROVED Project Name:

OWNER/AGENT: ____

Address:

City, State:

Sim-Q Developers

Lake City, FL 32025-

Lot: 111, Sub: The Preserves, Plat:

Aaron Simque Homes

Columbia County

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

BUILDING OFFICIAL:

DATE:

Permitting Office:

City, State: Owner: Climate Zone:	Lake City, FL 32 Model Home North	2025-		Permit Number: 7 Jurisdiction Number:	221000
a. U-factor: (or Single or Do b. SHGC: (or Clear or Tin 8. Floor types a. Slab-On-Grade F b. N/A	multi-family if multi-family boms se? r area (ft²) area: (Label reqd. by 13- uble DEFAULT) 7a. (D	Description Area	a. b. c. 13. a. b. c.	Cooling systems Central Unit N/A N/A Heating systems Electric Heat Pump N/A N/A	Cap: 62.0 kBtu/hr SEER: 13.00
c. N/A 9. Wall types a. Frame, Wood, Ex b. Frame, Wood, Ad c. N/A d. N/A e. N/A 10. Ceiling types a. Under Attic b. N/A c. N/A 11. Ducts a. Sup: Unc. Ret: U b. N/A	djacent	R=13.0, 1863.7 ft ² _ R=13.0, 333.0 ft ²	a. b. c.	Hot water systems Electric Resistance N/A Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) HVAC credits (CF-Ceiling fan, CV-Cross ventilatio HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	Cap: 80.0 gallons EF: 0.90
I hereby certify that this calculation are Code. PREPARED BY DATE:	1-3-0	Total base	Rev spe calc with Before this com		S CREAT CONTRACTOR OF THE STATE

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 111, Sub: The Preserves, Plat: , Lake City, FL, 32025-

PERMIT #:

	BASE					AS-	BUI	LT			
GLASS TYPES .18 X Condition Floor A	oned X B	SPM =	Points	Type/SC		hang Len	Hgt	Area X	SPM X	SOF	= Points
.18 280	9.0	18.59	9399.0	1.Double, Clear	W	1.5	8.0	30.0	38.52	0.96	1107.0
, same	2.2.2		5.5.5.5.5.	2.Double, Clear	w	1.5	11.0		38.52	0.99	3988.0
				3.Double, Clear	N	5.5	11.0		19.20	0.83	335.0
				4.Double, Clear	w	9.5	11.0		38.52	0.56	1735.0
				5.Double, Clear	S	5.5	11.0		35.87	0.66	473.0
				6.Double, Clear	N	1.5	11.0	2.5 M	19.20	0.99	795.0
				7.Double, Clear	N	1.5	11.0		19.20	0.99	75.0
l				8.Double, Clear	N	1.5	11.0	15/87	19.20	0.99	151.0
				9.Double, Clear	NE	1.5	10.0	12,000	29.56	0.98	347.0
				10.Double, Clear	E	1.5	10.0	18.0	42.06	0.98	740.0
				11.Double, Clear	SE	1.5	10.0	12.0	42.75	0.98	500.0
				12.Double, Clear	E	13.0	11.0	13.3	42.06	0.47	264.0
				13.Double, Clear	E	8.5	11.0	60.0	42.06	0.58	1474.0
				14.Double, Clear	S	1.5	9.0	30.0	35.87	0.94	1015.0
				14.Double, Clear	3	1.5	9.0	30.0	33.67	0.94	1015.0
				As-Built Total:				455.3			12999.0
WALL TYPES	Area)	K BSPM	= Points	Туре		R-	Value	Area	X SP	M =	Points
Adjacent	333.0	0.70	233.1	1. Frame, Wood, Exterior			13.0	1863.7	1.50		2795.5
Exterior	1863.7	1.70	3168.3	2. Frame, Wood, Adjacent			13.0	333.0	0.60		199.8
											,,,,,,
Base Total:	2196.7		3401.4	As-Built Total:				2196.7			2995.3
DOOR TYPES	Area >	K BSPM	= Points	Туре				Area	X SPI	M =	Points
Adjacent	0.0	0.00	0.0	1.Exterior Insulated				18.0	4.10		73.8
Exterior	38.0	6.10	231.8	2.Exterior Insulated				20.0	4.10		82.0
		0.10	201.0	Z.E.KOTOT MOGISTOG				20.0	1.10		02.0
Base Total:	38.0		231.8	As-Built Total:				38.0			155.8
CEILING TYPE	S Area >	(BSPM	= Points	Туре	F	R-Valu	e A	Area X S	SPM X S	CM =	Points
Under Attic	2809.0	1.73	4859.6	1. Under Attic		;	30.0	3100.0	1.73 X 1.00		5363.0
Base Total:	2809.0		4859.6	As-Built Total:				3100.0			5363.0
FLOOR TYPES	Area >	BSPM	= Points	Туре		R-	Value	Area	X SPI	M =	Points
Slab	262.0(p)	-37.0	-9694.0	Slab-On-Grade Edge Insu	lation		50	262.0(p	-36.20		-9484.4
Raised	0.0	0.00	0.0	1. Olab-Oll-Glade Luge Ilist	nation		3.0	202.0(p	-30.20		-5404.4
	0.0	0.00	0.0								- 1
Base Total:			-9694.0	As Built Total				262.0			0404.4
Dase I Utal.			-9094.0	As-Built Total:				262.0			-9484.4

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 111, Sub: The Preserves, Plat: , Lake City, FL, 32025- PERMIT #:

	BASE		AS-BUILT	
INFILTRATION	Area X BS	PM = Points	Area X SPM = P	oints
	2809.0 10	28679.9	2809.0 10.21 28	3679.9
Summer Bas	se Points: 3	6877.7	Summer As-Built Points: 407	08.6
Total Summer : Points	X System = Multiplier	Cooling Points		oling ints
36877.7	0.3250	11985.2	:	39.7 39.7

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 111, Sub: The Preserves, Plat: , Lake City, FL, 32025- PERMIT #:

	BASE	=				AS-	BUI	LT			
GLASS TYPE: .18 X Conditi		D\A/D&4 =	Deint								
Floor A		OVVEIVI =	Points	Type/SC		rhang Len	Hgt	Area X	WPM X	wo	F = Point
.18 280	9.0	20.17	10198.0	1.Double, Clear	W	1.5	8.0	30.0	20.73	1.01	628.0
				2.Double, Clear	W	1.5	11.0	105.0	20.73	1.00	2185.0
ř .				3.Double, Clear	N	5.5	11.0	21.0	24.58	1.01	520.0
				4.Double, Clear	W	9.5	11.0	80.0	20.73	1.15	1910.0
				5.Double, Clear	S	5.5	11.0	20.0	13.30	1.64	436.0
				6.Double, Clear	N	1.5	11.0	42.0	24.58	1.00	1032.0
				7.Double, Clear	N	1.5	11.0	4.0	24.58	1.00	98.0
				8.Double, Clear	N	1.5	11.0	8.0	24.58	1.00	196.0
				9.Double, Clear	NE	1.5	10.0	12.0	23.57	1.00	282.0
				10.Double, Clear	Е	1.5	10.0	18.0	18.79	1.01	342.0
				11.Double, Clear	SE	1.5	10.0	12.0	14.71	1.03	181.0
				12.Double, Clear	E	13.0	11.0	13.3	18.79	1.34	334.0
				13.Double, Clear	E	8.5	11.0	60.0	18.79	1.22	1374.0
				14.Double, Clear	S	1.5	9.0	30.0	13.30	1.02	408.0
				As-Built Total:				455.3			9926.0
WALL TYPES	Area X	BWPM	= Points	Туре		R-\	/alue	Area	X WPM	=	Points
Adjacent	333.0	3.60	1198.8	1. Frame, Wood, Exterior		1	3.0	1863.7	3.40		6336.6
Exterior	1863.7	3.70	6895.7	2. Frame, Wood, Adjacent			3.0	333.0	3.30		1098.9
Base Total:	2196.7		8094.5	As-Built Total:				2196.7			7435.5
DOOR TYPES	Area X	BWPM	= Points	Туре				and the second	X WPM	=	Points
Adjacent	0.0	0.00	0.0							3,5-3,0	
Exterior	38.0	0.00 12.30	0.0	1.Exterior Insulated				18.0	8.40		151.2
EXCHO	30.0	12.30	467.4	2.Exterior Insulated				20.0	8.40		168.0
Base Total:	38.0		467.4	As-Built Total:				38.0			319.2
CEILING TYPE	S Area X	BWPM	= Points	Туре	R-V	/alue	Are	a X W	PM X WC	M =	Points
Under Attic	2809.0	2.05	5758.4	1. Under Attic		3	0.0	3100.0 2.	.05 X 1.00		6355.0
Base Total:	2809.0		5758.4	As-Built Total:				3100.0			6355.0
FLOOR TYPES	Area X	BWPM	= Points	Туре		R-V	alue		X WPM	=	Points
Slab	262.0(p)	8.9	2331.8	Slab-On-Grade Edge Insul	ation						
Raised	0.0	0.00	0.0	Glab-Oll-Glade Edge Illsul	ation		5.0 2	62.0(p	7.60		1991.2
			12.72								
Base Total:			2331.8	As-Built Total:				262.0			1991.2

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 111, Sub: The Preserves, Plat: , Lake City, FL, 32025- PERMIT #:

BASE			AS-	BUILT			
INFILTRATION Area X BWPM = P	s			Area	a X WP	M =	Points
2809.0 -0.59 -1	3			280	9.0 -0.5	59	-1657.3
Winter Base Points: 2519	8 Winter As	-Built P	oints:			2	4369.6
Total Winter X System = Heating Points Multiplier Points	Total Component (System - Po	X Cap Ratio	X Duct Multiplier (DM x DSM x A	Multiplier	X Credi Multipli		Heating Points
25192.8 0.5540 1395	24369.6	Heat Pump 1.000 1.00	62000 btuh ,EFF (1.069 x 1.169 1.162	F(7.7) Ducts:Uno 9 x 0.93) 0.443 0.443	c(S),Unc(R), 0.950 0.95)	R6.0 11915.4 1915.4

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 111, Sub: The Preserves, Plat: , Lake City, FL, 32025- PERMIT #:

	В	BASE						Α	S-BUII	_T		
WATER HEA Number of Bedrooms	TING X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier	X Credit Multiplie	
4		2635.00		10540.0	80.0	0.90	4		1.00	2693.56	1.00	10774.2
					As-Built To	otal:						10774.2

				CODE	C	OMPLI	ANCE	S1	ATUS	3			
		BAS	SE							AS	-BUILT		
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
11985		13957		10540		36482	11440		11915		10774		34129

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 111, Sub: The Preserves, Plat: , Lake City, FL, 32025- PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS SECTION		REQUIREMENTS FOR EACH PRACTICE				
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.				
Exterior & Adjacent Walls 606.1.ABC.1.2.1		Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.				
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is seale to the perimeter, penetrations and seams.				
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.				
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm fro conditioned space, tested.				
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.				
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.				

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS SECTION Water Heaters 612.1		REQUIREMENTS Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.		
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.		
Air Distribution Systems	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.			
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.		
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.		

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.8

The higher the score, the more efficient the home.

Model Home, Lot: 111, Sub: The Preserves, Plat: , Lake City, FL, 32025-

1.	New construction or existing	New			Cooling systems		
2. 3.	Single family or multi-family Single family		_	a	Central Unit	Cap: 62.0 kBtu/hr	
3. 4.	Number of units, if multi-family Number of Bedrooms	1	_		U.S.	SEER: 13.00	_
5.	Is this a worst case?	. 4	_	Ь	. N/A		_
6.	Conditioned floor area (ft²)	No	-		SVII		_
7.	Glass type 1 and area: (Label reqd.)	2809 ft ²	_	C.	N/A		-
	U-factor:			10			-
a.	(or Single or Double DEFAULT)	Description Area			Heating systems		
b.	SHGC:	ra. (Doie Default) 455.3 ft ²	_	a.	Electric Heat Pump	Cap: 62.0 kBtu/hr HSPF: 7.70	
59	(or Clear or Tint DEFAULT)	7b. (Clear) 455.3 ft ²	_	b.	N/A		
	Floor types						
	Slab-On-Grade Edge Insulation	R=5.0, 262.0(p) ft	_	c.	N/A		
	N/A		_				
	N/A		_	14.	Hot water systems		
	Wall types			a.	Electric Resistance	Cap: 80.0 gallons	_
	Frame, Wood, Exterior	R=13.0, 1863.7 ft ²	-			EF: 0.90	
	Frame, Wood, Adjacent	R=13.0, 333.0 ft ²	_	b.	N/A		_
	N/A						_
	N/A		_	c.	Conservation credits		_
	N/A		_		(HR-Heat recovery, Solar		
	Ceiling types				DHP-Dedicated heat pump)		
	Under Attic	R=30.0, 3100.0 ft ²	-	15.	HVAC credits	PT,	_
	N/A				(CF-Ceiling fan, CV-Cross ventilation,	è	
	N/A		_		HF-Whole house fan,		
	Ducts				PT-Programmable Thermostat,		
	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 55.0 ft	_		MZ-C-Multizone cooling,		
D.	N/A		_		MZ-H-Multizone heating)		
I cer	tify that this home has complie	d with the Florida Energy	Effic	iency	Code For Building	THEST	
Cons	struction through the above ene	rgy saving features which	will i	be ins	talled (or exceeded)	NOT THE REAL PROPERTY.	à
in th	is home before final inspection	. Otherwise, a new EPL I	Display	y Car	d will be completed		B
base	d on installed Code compliant i	features.				7	112
Buile	der Signature:		Date:			B	ij
	ress of New Home:					I COD WE TRUST	
*NO	TE: The home's estimated ener	gy performance score is	only a	vaila	ble through the FLA/RES compute	er program.	

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001527

DATE 01/1	16/2008	PARC	EL ID # 03-4S-16	6-02731-111				
APPLICANT	MELANIE RO	DER		PHONE	623-7829			
ADDRESS _	387 SW KEN	MP COURT		LAKE CITY		FL	32024	
OWNER SI	IM-Q DEVELOPE	ERS		PHONE	867-0692			
ADDRESS _4	453 SW ROSE	EMARY DRIVE		LAKE CITY		FL	32024	
CONTRACTO	OR DAVID SIM	QUE		PHONE	867-0692			
LOCATION O	F PROPERTY	90W, TL ON 252	2B, TR ON ROSEMA	RY DR, 6TH LOT	ON RIGHT			
SUBDIVISION	N/LOT/BLOCK	C/PHASE/UNIT	PRE. LAUREL LAK	KES	111			
SIGNATURE	Molarid	Cella						
	INSTALL	TION DECLUE	DEMENTS					
	**	Will be 18 inches	in diameter with a	total length of	32 feet leavi	ina 24	feet of	
	driving surfa	ce. Both ends will ced concrete slab	be mitered 4 foot	with a 4:1 slo	pe and pour	ed wit	h a 4 inch	
	INSTALLAT	ION NOTE: Turn	outs will be requir	ed as follows:				
	b) the driv	eway to be served	and existing drivey d will be paved or f	formed with cor	ncrete.			
	concrete	or paved drivewa	e or paved a minin ay, whichever is gr	eater. The widt				
	current a	and existing paved	d or concreted turn	nouts.				
	Culvert installation shall conform to the approved site plan standards.							
	Department	of Transportation	Permit installation	on approved sta	ndards.			
	Other							

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00





OCCUPANCY

COLUMBIA COUNTY, FLORIDA

partment of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 03-4S-16-02731-111

Fire: 46.34

Building permit No. 000026628

Permit Holder DAVID SIMQUE

Use Classification SFD, UTILITY

10.01

Owner of Building SIM-Q DEVELOPERS

Location: 453 SW ROSEMARY DRIVE., LAKE CITY, FL

Date: 08/01/2008

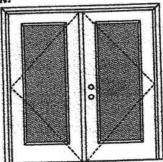
rupie M. Kusa

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:

Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door

Design Pressure +40.5/-40.5

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES: 1/4 GLASS:











1/2 GLASS:



12 RAL, 23 RAL, 2

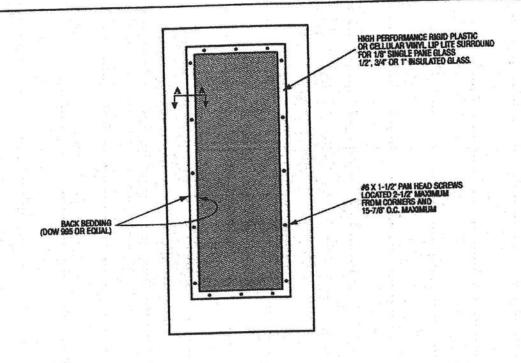




"This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.



GLASS INSERT IN DOOR OR SIDELITE PANEL



TYPICAL RIGID PLASTIC LIP LITE SURROUND 1-9/16 DOOR 1-9/16 DOOR 1-1/2' PAME GLASS, 1/2', 34' OR 1' 34' OR 1' TRIPLE PAME BLASS, 1/2', 34' OR 1' BUSULATED GLASS 1/2' SW OR 1' TRIPLE PAME BUSULATED GLASS





WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES: 3/4 GLASS:



















CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH MIAMI-DADE BCCO PA202

COMPANY NAME CITY, STATE

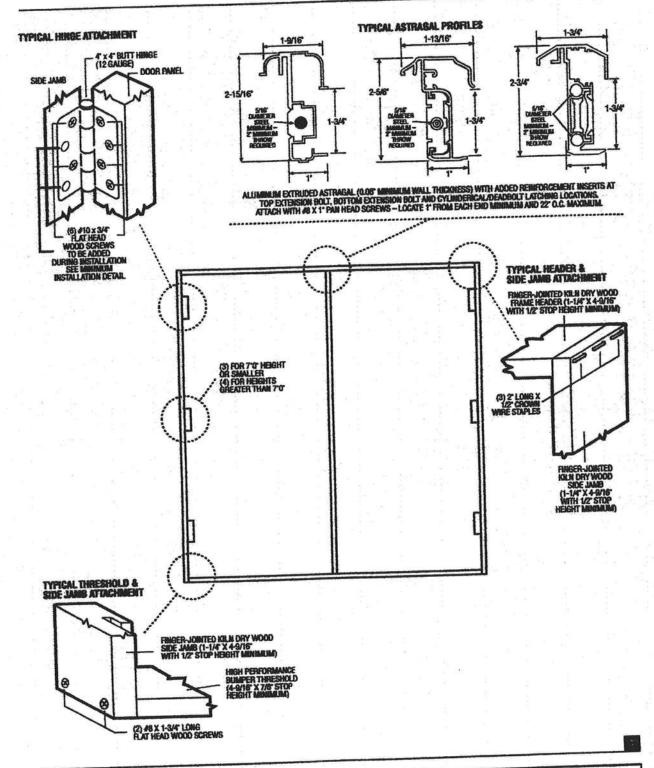
To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer Kurt Balthazor, P.E. - License Number 56533



MAD-WL-MA0012-02

OUTSWING UNITS WITH DOUBLE DOOR

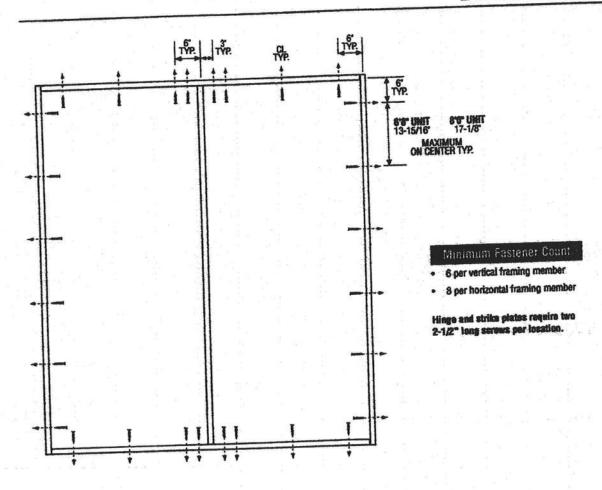








DOUBLE DOOR



Latching Hardware:

Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

- 1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
- The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
- 3. Wood bucks by others, must be anchored properly to transfer loads to the structure.









Submit Surcharge Stats & Facts Publications FBC Staff BCIS Site Map Links Search ABOUT DOA / DOA PROGRAMS CONTACT DOA



Product Approval **USER:** Public User

Product Approval Menu > Product or Application Search > Application List > Application Detail

 COMMUNITY PLANNING OFFICE OF THE EMERGENCY DEVELOPMENT HOUSING & COMMUNITY MANAGEMENT

> Application Type Application Status Code Version

Comments

Archived

2004

Revision FL1956-R1

Approved

Address/Phone/Email **Product Manufacturer**

TAMKO Building Products, Inc.

Joplin, MO 64802 PO Box 1404 fred_oconnor@tamko.com (800) 641-4691 ext 2394

Authorized Signature

fred_oconnor@tamko.com Frederick O'Connor

Address/Phone/Email Technical Representative

PO Box 1404 Joplin, MO 64802 Frederick J. O'Connor (800) 641-4691

fred_oconnor@tamko.com

7 of S

Address/Phone/Email Quality Assurance Representative

Subcategory Category

Roofing

Asphalt Shingles

Compliance Method Certification Mark or Listing

Certification Agency Underwriters Laboratories Inc.

Referenced Standard and Year (of Standard)

Standard

ASTM D 3462

Year 2001

Equivalence of Product Standards Certified By

Product Approval Method

Method 1 Option A

Date Pending FBC Approval Date Validated Date Submitted Date Approved 06/29/2005 06/25/2005 06/20/2005 06/09/2005

Summary of Products

) of 5

Model, Number or Name	Model, Number or Name Description	1077 - 2 10 10 10 10 10 10 10 10 10 10 10 10 10	FL #	
			Model, Number o	
	Description			

slopes of 2:12 or greater. Not approved for use in HVHZ. Next Back DCA Administration

Department of Community Affairs

Florida Building Code Online

© 2000-2005 The State of Florida. All rights reserved. Copyright and Disclaimer Codes and Standards 2555 Shumard Oak Boulevard Tallahassee, Florida 32399-2100 (850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:









2/14/2007 11:22 AN

5 of 5



Northbreak Division

333 Pfinjskin Road Northdrook, 1, 60062-2096 USA www.il.com tel: 1,847,272,5600

June 17, 2005

Tamko Roofing Products Ms. Kerri Eden P.O. Box 1404 220 W. 4th Street Joplin, MO 64802-1404

Our Reference: R2919

This is to confirm that "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage 50 AR", "Glass-Seal AR" manufactured at Tuscaloosa, AL and "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage XL AR", "Heritage 50 AR" manufactured at Frederick, MD and "Heritage 30 AR", "Heritage XL AR", and "Heritage 50 AR" manufactured in Dallas, TX are UL Listed asphalt glass mat shingles and have been evaluated in accordance with ANSI/UL 790, Class A (ASTM E108), ASTM D3462, ASTM D3161 or UL 997 modified to 110 mph when secured with four nails.

Let me know if you have any further questions.

Very truly yours,

Alpesh Patel (Ext. 42522)

Engineer Project

Fire Protection Division

Reviewed by,

Randall K. Laymon (Ext. 42687)

Engineer Sr Staff

Fire Protection Division



Application Instructions for

• HERITAGE® VINTAGE™ AR — Phillipsburg, KS LAMINATED ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.

IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

I. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-andgroove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced

TAMKO does not recommend re-roofing over existing roof.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

- 1. Vapor Condensation
- 2. Buckling of shingles due to deck movement.
- 3. Rotting of wood members.
- 4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents. FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

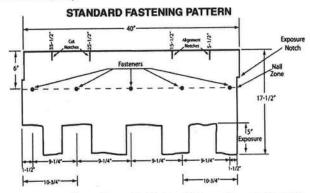
3. FASTENERS

WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, this will result in the termination of TAMKO's liabilities under the limited warranty. TAMKO will not be responsible for damage to shingles caused by winds in excess of the applicable miles per hour as stated in the limited warranty. See limited warranty for details.

FASTENING PATTERNS: Fasteners must be placed 6 in. from the top edge of the shingle located horizontally as follows:

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1-1/2 in. back from each end, one 10-3/4 in. back from each end and one 20 in. from one end of the shingle for a total of 5 fasteners. (See standard fastening pattern illustrated below).



2) Mansard or Steep Slope Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) Use standard nailing instructions with four additional nails placed 6 in. from the butt edge of the shingle making certain nails are covered by the next (successive) course of shingles. (Continued)

Visit Our Web Site at www.tamko.com

Central District Northeast District Southeast District Southwest District Western District 220 West 4th St., Joplin, MO 64801 4500 Tamko Dr., Frederick, MD 21701 2300 35th St., Tuscaloosa, AL 35401 7910 S. Central Exp., Dallas, TX 75216 5300 East 43rd Ave., Denver, CO 80216 800-641-4691 800-368-2055 800-228-2656 800-443-1834 800-530-8868 05/06

1

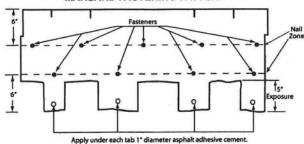


(CONTINUED from Pg. 1)

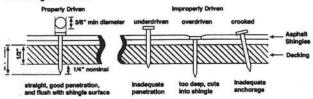
HERITAGE® VINTAGE™ AR – Phillipsburg, KS LAMINATED ASPHALT SHINGLES

Each shingle tab must be sealed underneath with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a \$.25 piece and applied to shingles with a 5 in. exposure, use 9 fasteners per shingle.

MANSARD FASTENING PATTERN



NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12 gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in. into the roof deck. Where the deck is less than 3/4 in. thick, the nails should be long enough to penetrate completely through plywood decking and extend at least 1/8 in. through the roof deck. Drive nail head flush with the shingle surface.



4. UNDERLAYMENT

UNDERLAYMENT: An underlayment consisting of asphalt saturated felt must be applied over the entire deck before the installation of TAMKO shingles. Failure to add underlayment can cause premature failure of the shingles and leaks which are not covered by TAMKO's limited warranty. Apply the felt when the deck is dry. On roof decks 4 in. per foot and greater apply the felt parallel to the eaves lapping each course of the felt over the lower course at least 2 in. Where ends join, lap the felt 4 in. If left exposed, the underlayment felt may be adversely affected by moisture and weathering. Laying of the underlayment and the shingle application must be done together.

Products which are acceptable for use as underlayment are:

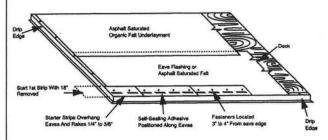
- TAMKO No. 15 Asphalt Saturated Organic Felt
- A <u>non-perforated</u> asphalt saturated organic felt which meets ASTM: D226, Type I or ASTM D4869, Type I
- Any TAMKO <u>non-perforated</u> asphalt saturated organic felt
- TAMKO TW Metal and Tile Underlayment, TW Underlayment and Moisture Guard Plus® (additional ventilation maybe required. Contact TAMKO's technical services department for more information)

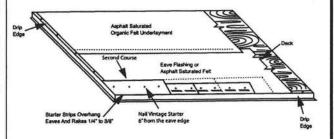
In areas where ice builds up along the eaves or a back-up of water from frozen or clogged gutters is a potential problem, TAMKO's Moisture Guard Plus® waterproofing underlayment (or any specialty eaves flashing product) may be applied to eaves, rakes, ridges, valleys, around chimneys, skylights or dormers to help prevent water damage. Contact TAMKO's Technical Services Department for more information. TAMKO does not recommend the use of any substitute products as shingle underlayment.

5. APPLICATION INSTRUCTIONS

STARTER COURSE: Two starter course layers must be applied prior to application of Heritage Vintage AR Shingles.

The first starter course may consist of TAMKO Shingle Starter, three tab self-sealing type shingles or a 9 inch wide strip of mineral surface roll roofing. If three tab self-sealing shingles are used, remove the exposed tab portion and install with the factory applied adhesive adjacent to the eaves. If using three tab self-sealing shingles or shingle starter, remove 18 in. from first shingle to offset the end joints of the Vintage Starter. Attach the first starter course with approved fasteners along a line parallel to and 3 in. to 4 in. above the eave edge. The starter course should overhang both the eave and rake edge 1/4 in. to 3/8 in. Over the first starter course, install Heritage Vintage Starter AR and begin at the left rake edge with a full size shingle and continue across the roof nailing the Heritage Vintage Starter AR along a line parallel to and 6 in. from the eave edge.





Note: Do not allow Vintage Starter AR joints to be visible between shingle tabs. Cutting of the starter may be required.

HERITAGE VINTAGE STARTER AR 12 1/2" x 36" 20 PIECES PER BUNDLE 60 LINEAL FT. PER BUNDLE

(Continued)

Visit Our Web Site at www.tamko.com

Central District Northeast District Southeast District Southwest District Western District 220 West 4th St., Joplin, MO 64801 4500 Tamko Dr., Frederick, MD 21701 2300 35th St., Tuscaloosa, AL 35401 7910 S. Central Exp., Dallas, TX 75216 5300 East 43rd Ave., Denver, CO 80216 800-641-4691 800-368-2055 800-228-2656 800-443-1834 800-530-8868 05/06

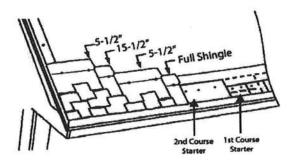
2



(CONTINUED from Pg. 2)

• HERITAGE® VINTAGE™ AR — Phillipsburg, KS LAMINATED ASPHALT SHINGLES

SHINGLE APPLICATION: Start the first course at the left rake edge with a full size shingle and overhang the rake edge 1/4 in. to 3/8 in.. To begin the second course, align the right side of the shingle with the 5-1/2 in. alignment notch on the first course shingle making sure to align the exposure notch. (See shingle illustration on next page) Cut the appropriate amount from the rake edge so the overhang is 1/4" to 3/8". For the third course, align the shingle with the 15-1/2 in. alignment notch at the top of the second course shingle, again being sure to align the exposure notch. Cut the appropriate amount from the rake edge. To begin the fourth course, align the shingle with the 5-1/2 in. alignment notch from the third course shingle while aligning the exposure notch. Cut the appropriate amount from the rake edge. Continue up the rake in as many rows as necessary using the same formula as outlined above. Cut pieces may be used to complete courses at the right side. As you work across the roof, install full size shingles taking care to align the exposure notches. Shingle joints should be no closer than 4 in.



6. LOW SLOPE APPLICATION

On pitches 2 in. per foot to 4 in. per foot cover the deck with two layers of underlayment. Begin by applying the underlayment in a 19 in. wide strip along the eaves and overhanging the drip edge by 1/4 to 3/4 in. Place a full 36 in. wide sheet over the 19 in. wide starter piece, completely overlapping it. All succeeding courses will be positioned to overlap the preceding course by 19 in. If winter temperatures average 25°F or less, thoroughly cement the laps of the entire underlayment to each other with plastic cement from eaves and rakes to a point of a least 24 in. inside the interior wall line of the building. As an alternative, TAMKO's Moisture Guard Plus self-adhering waterproofing underlayment may be used in lieu of the cemented felts.

7. VALLEY APPLICATION

TAMKO recommends an open valley construction with Heritage Vintage AR shingles.

To begin, center a sheet of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment in the valley.

After the underlayment has been secured, install the recommended corrosion resistant metal (26 gauge galvanized metal or an equivalent) in the valley. Secure the valley metal to the roof deck. Overlaps should be 12" and cemented.

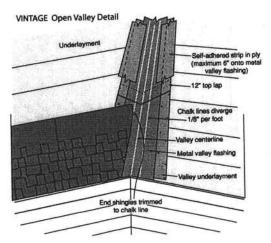
Following valley metal application; a 9" to 12" wide strip of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment should be applied along the edges of the metal valley flashing (max. 6" onto metal valley flashing) and on top of the valley underlayment. The valley will be completed with shingle application.

SHINGLE APPLICATION INSTRUCTIONS (OPEN VALLEY)

- Snap two chalk lines, one on each side of the valley centerline over the full length of the valley flashing. Locate the upper ends of the chalk lines 3" to either side of the valley centerline.
- The lower end should diverge from each other by 1/8" per foot.
 Thus, for an 8' long valley, the chalk lines should be 7" either side of the centerline at the eaves and for a 16' valley 8".

As shingles are applied toward the valley, trim the last shingle in each course to fit on the chalk line. Never use a shingle trimmed to less than 12" in length to finish a course running into a valley. If necessary, trim the adjacent shingle in the course to allow a longer portion to be used.

- Clip 1" from the upper corner of each shingle on a 45° angle to direct water into the valley and prevent it from penetrating between the courses.
- Form a tight seal by cementing the shingle to the valley lining with a 3" width of asphalt plastic cement (conforming to ASTM D 4586).



· CAUTION:

Adhesive must be applied in smooth, thin, even layers.

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.

(Continued)

Visit Our Web Site at www.tamko.com

Central District Northeast District Southeast District Southwest District Western District 220 West 4th St., Joplin, MO 64801 4500 Tamko Dr., Frederick, MD 21701 2300 35th St., Tuscaloosa, AL 35401 7910 S. Central Exp., Dallas, TX 75216 5300 East 43rd Ave., Denver, CO 80216 800-641-4691 800-368-2055 800-228-2656 800-443-1834 800-530-8868

05/06



(CONTINUED from Pg. 3)

• HERITAGE® VINTAGE™ AR — Phillipsburg, KS LAMINATED ASPHALT SHINGLES

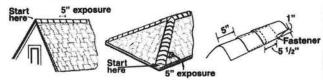
8. HIP AND RIDGE FASTENING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener on each side, 5-1/2 in. back from the exposed end and 1 in. up from the edge. TAMKO recommends the use of TAMKO Heritage Vintage Hip & Ridge shingle products.

Fasteners should be 1/4 in. longer than the ones used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLE IN COLD WEATHER.

Direction of prevailing wind



THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

TAMKO®, Moisture Guard Plus®, Nail Fast® and Heritage® are registered trademarks and Vintage™ is a trademark of TAMKO Building Products, Inc.

Visit Our Web Site at www.tamko.com

Central District Northeast District Southeast District Southwest District Western District 220 West 4th St., Joplin, MO 64801 4500 Tamko Dr., Frederick, MD 21701 2300 35th St., Tuscaloosa, AL 35401 7910 S. Central Exp., Dallas, TX 75216 5300 East 43rd Ave., Denver, CO 80216 800-641-4691 800-368-2055 800-228-2656 800-443-1834 800-530-8868

05/06

Dec. 489.30

THIS INSTRUMENT WAS PREPARED BY:

TERRY Medavid FOST OFFICE BOX 1328 LARE CITY, PL 32056-1328

RETURN TO:

TERRY MCDAVID FOST OFFICE BOX 1328 LAKE CITY, FL 32086-1328

File No. 07-373

Property Appraiser's Identification Number 03-45-16-02731-111



STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and forgology
is a true copy of the original filed in this office.
P. DeWITT MON, CLERK OF COLUMN
By
Deputy Cast
Deputy Cast
Deputy Cast
Deputy Cast

inat:200812000282 Date:1/7/2008 Time:4:15 PM Doc Stemp-Deed:469.30 Doc,P.DeWitt Casen,Columbia County Page 1 of 2

WARRANTY DEED

This Werranty Deed, made this 7th day of January, 2008, BETWEEN RESIDENTIAL DEVELOPMENT GROUP, LLC, A Florida Limited Liability Company, whose post office address is Post Office Box 3659, Lake City, FL 32056-3659, of the County of Columbia, State of Florida, grantor*, and SIM-Q DEVELOPERS, LLC, a Florida Limited Liability Company, whose document number assigned by the Secretary of State of Florida is L07000103309 and whose Federal Tax I.D. Number is 26-1266365*, whose post office address is 122 SW Midtown Place, Suite 101, Lake City, Florida 32024, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lot 111, FRESERVE AT LAUREL LAKE, UNIT 1, a subdivision according to the plat thereof as recorded in Plat Book 9, Pages 18-25 of the public records of Columbia County, Florida.

*N.B.: THE PURPOSE OF INCLUDING THE DOCUMENT NUMBER AND THE FEDERAL TAX I.D. NUMBER OF THIS GRANTEE IS TO AVOID CONFUSION BETWEEN THIS GRANTEE AND ANY OTHER LIMITED LIABILITY COMPANY OF THE SAME OR SIMILAR NAME.

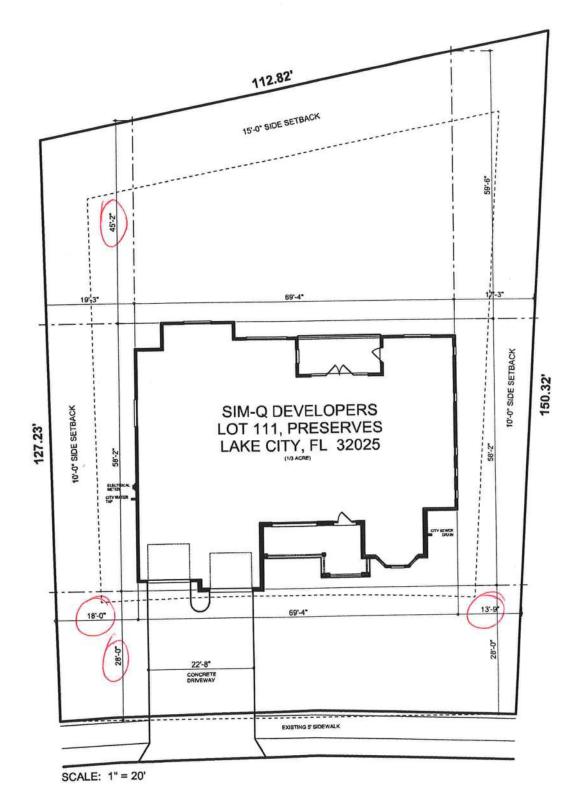
Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

Notice of Authorization

To be my representative and act on my behalf in all aspects of applying for a Building permit to be located in Columbia County. Contractor's signature Date Sworn and subscribed before me this8 day of, 2008 Linda R. Roder Commission #DD303275 Expires: Mar 24, 2008 Bonded Thou Atlantic Bonding Co., Inc. Personally known	I, Dave Simque, do hereby authorize Melanie Roder or Linda Roder,	
Contractor's signature Date Sworn and subscribed before me this	To be my representative and act on my behalf in all aspects of applying	for a
Sworn and subscribed before me this	Building permit to be located in Columbia County.	
Sworn and subscribed before me this		
Sworn and subscribed before me this	Contractor's signature	
Notary Public Linda R. Roder Commission #DD303275 Expires: Mar 24, 2008 Bonded Thru Atlantic Bonding Co., Inc.	Date	
Notary Public Linda R. Roder Commission #DD303275 Expires: Mar 24, 2008 Bonded Thru Atlantic Bonding Co., Inc.		
Notary Public Personally known Commission #DD303275 Expires: Mar 24, 2008 Bonded Thru Atlantic Bonding Co., Inc.	Sworn and subscribed before me this day of	, 2008
	Notary Public Commission # Expires: Mar Bonded T	DD303275 24, 2008 Thru



SW ROSEMARY PLACE

Residential System Sizing Calculation

Summary

Model Home

Project Title: Sim-Q Developers

Code Only Professional Version Climate: North

Lake City, FL 32025-

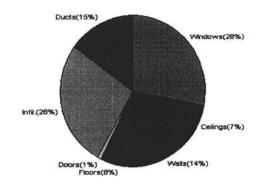
1/2/2008

				1/2/2000	
			ide(29) Altitude(152 ft.) Temp Ran	ge(M)	
Humidity data: Interior RH (50%) Outdoor	wet bulb (7	7F) Humidity difference(54gr.)		
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	51754	Btuh	Total cooling load calculation	74598	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	119.8	62000	Sensible (SHR = 0.75)	77.2	46500
Heat Pump + Auxiliary(0.0kW)	119.8	62000	Latent	107.9	15500
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			Total (Electric Heat Pump)	83.1	62000

WINTER CALCULATIONS

Winter Heating Load (for 2809 sqft)

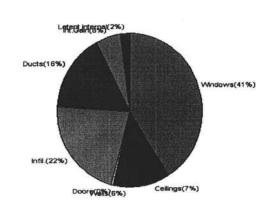
Load component			Load	
Window total	455	sqft	14657	Btuh
Wall total	2197	sqft	7214	Btuh
Door total	38	sqft	492	Btuh
Ceiling total	3100	sqft	3653	Btuh
Floor total	262	sqft	4285	Btuh
Infiltration	337	cfm	13654	Btuh
Duct loss			7799	Btuh
Subtotal			51754	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			51754	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 2809 sqft)

Load component			Load	
Window total	455	sqft	30581	Btuh
Wall total	2197	sqft	4390	Btuh
Door total	38	sqft	372	Btuh
Ceiling total	3100	sqft	5134	Btuh
Floor total			0	Btuh
Infiltration	295	cfm	5489	Btuh
Internal gain			4240	Btuh
Duct gain		- 1	10033	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain		- 1	60239	Btuh
Latent gain(ducts)		- 1	1981	Btuh
Latent gain(infiltration)			10779	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occu	pants/othe	r)	1600	Btuh
Total latent gain			14359	Btuh
TOTAL HEAT GAIN			74598	Btuh



Amerod by

Version 8 For Florida residences only

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Model Home

Project Title: Sim-Q Developers

Code Only Professional Version

Lake City, FL 32025-

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/2/2008

VAC I	D	0			
Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	30.0	32.2	966 Btu
2	2, Clear, Metal, 0.87	W	105.0	32.2	3380 Btu
3	2, Clear, Metal, 0.87	N	21.0	32.2	676 Btu
4	2, Clear, Metal, 0.87	W	80.0	32.2	2575 Btu
5	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btu
6	2, Clear, Metal, 0.87	N	42.0	32.2	1352 Btu
7	2, Clear, Metal, 0.87	N	4.0	32.2	129 Btu
8	2, Clear, Metal, 0.87	N	8.0	32.2	258 Btu
9	2, Clear, Metal, 0.87	NE	12.0	32.2	386 Btu
10	2, Clear, Metal, 0.87	E	18.0	32.2	579 Btu
11	2, Clear, Metal, 0.87	SE	12.0	32.2	386 Btu
12	2, Clear, Metal, 0.87	E	13.3	32.2	429 Btu
13	2, Clear, Metal, 0.87	E	60.0	32.2	1931 Btu
14	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btu
	Window Total		455(sqft)		14657 Btu
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1864	3.3	6120 Btu
2	Frame - Wood - Adj(0.09)	13.0	333	3.3	1094 Btu
	Wall Total		2197		7214 Btu
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		18	12.9	233 Btu
2	Insulated - Exterior		20	12.9	259 Btu
	Door Total		38	1	492Btu
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	3100	1.2	3653 Btu
	Ceiling Total		3100		3653Btu
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	262.0 ft(p)	16.4	4285 Btu
*	Floor Total	3	262	10.4	4285 Btu
	Tioor rotal		202		4200 Dtu
			Envelope Sub	total:	30301 Btu
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqft)	CFM=	
	Natural	0.80	25281 2197	337.1	13654 Btu
Ductload			(DLI	M of 0.177)	7799 Btu
All Zones		Sen	sible Subtotal All	Zones	51754 Btu

Manual J Winter Calculations

Residential Load - Component Details (continued)

Model Home

Lake City, FL 32025-

Sim-Q Developers

Code Only Professional Version

Climate: North

1/2/2008

	Subtotal Sensible Ventilation Sensible Total Btuh Loss	51754 Btuł 0 Btuł 51754 Btuł
EQUIPMENT		

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Model Home

Project Title: Sim-Q Developers

Professional Version

Lake City, FL 32025-

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/2/2008

Component Loads for Zone #1: Main

				gissessocation v	
Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	30.0	32.2	966 Btuh
2	2, Clear, Metal, 0.87	W	105.0	32.2	3380 Btuh
3	2, Clear, Metal, 0.87	N	21.0	32.2	676 Btuh
4 5 6	2, Clear, Metal, 0.87	W	80.0	32.2	2575 Btuh
5	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
6	2, Clear, Metal, 0.87	N	42.0	32.2	1352 Btuh
7	2, Clear, Metal, 0.87	N	4.0	32.2	129 Btuh
8	2, Clear, Metal, 0.87	N	8.0	32.2	258 Btuh
9	2, Clear, Metal, 0.87	NE	12.0	32.2	386 Btuh
10	2, Clear, Metal, 0.87	E	18.0	32.2	579 Btuh
11	2, Clear, Metal, 0.87	SE	12.0	32.2	386 Btuh
12	2, Clear, Metal, 0.87	E	13.3	32.2	429 Btuh
13	2, Clear, Metal, 0.87	E	60.0	32.2	1931 Btuh
14	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btuh
	Window Total	= X	455(sqft)		14657 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1864	3.3	6120 Btuh
2	Frame - Wood - Adj(0.09)	13.0	333	3.3	1094 Btuh
	Wall Total		2197	(MANUSATIV.	7214 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		18	12.9	233 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		38		492Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	3100	1.2	3653 Btuh
	Ceiling Total		3100	0.010-0.01	3653Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	262.0 ft(p)	16.4	4285 Btuh
	Floor Total	Ü	262	10.1	4285 Btuh
***************************************	11001 10101		202		4200 Btuil
		Z	one Envelope Sub	total:	30301 Btuh
Infiltration	Туре	ACH X Volu	ume(cuft) walls(sqft)	CFM=	
	Natural	0.80	25281 2197	337.1	13654 Btuh
Ductload	Average sealed, Supply(R6.	0-Attic), Retur	n(R6.0-Attic) (DL	M of 0.177)	7799 Btuh
Zone #1		Sens	sible Zone Subtot	al	51754 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Model Home

Lake City, FL 32025-

Sim-Q Developers

Code Only Professional Version Climate: North

1/2/2008

Subtotal Sensible Ventilation Sensible Total Btuh Loss	51754 Btu 0 Btu 51754 Btu

 Electric Heat Pump 62000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default) (HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Model Home

Project Title:

Code Only Professional Version

Lake City, FL 32025-

Sim-Q Developers

Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/2/2008

Component Loads for Whole House

	Type*		Over	hang	Win	dow Area	a(sqft)	H	ITM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		Unshaded		Unshaded	Loud	
1	2, Clear, 0.87, None, N, N	W	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	Btuh
2	2, Clear, 0.87, None, N, N	W	1.5ft	11ft.	105.0	0.0	105.0	29	80	8349	
3	2, Clear, 0.87, None, N, N	N	5.5ft	11ft.	21.0	0.0	21.0	29	29	608	
4	2, Clear, 0.87, None, N, N	W	9.5ft	11ft.	80.0	42.6	37.4	29	80	4207	
5	2, Clear, 0.87, None, N, N	S	5.5ft	11ft.	20.0	20.0	0.0	29	34	579	Btuh
6	2, Clear, 0.87, None,N,N	N	1.5ft	11ft.	42.0	0.0	42.0	29	29	1216	Btuh
7	2, Clear, 0.87, None,N,N	N	1.5ft	11ft.	4.0	0.0	4.0	29	29	116	Btuh
8	2, Clear, 0.87, None,N,N	N	1.5ft	11ft.	8.0	0.0	8.0	29	29	232	Btuh
9	2, Clear, 0.87, None,N,N	NE	1.5ft	10ft.	12.0	0.0	12.0	29	60	720	Btuh
10	2, Clear, 0.87, None,N,N	Е	1.5ft	10ft.	18.0	0.0	18.0	29	80	1431	Btuh
11	2, Clear, 0.87, None,N,N	SE	1.5ft	10ft.	12.0	0.0	12.0	29	63	750	Btuh
12	2, Clear, 0.87, None, N, N	E	13ft.	11ft.	13.3	12.9	0.4	29	80	407	Btuh
13	2, Clear, 0.87, None,N,N	E	8.5ft	11ft.	60.0	20.5	39.5	29	80	3732	Btuh
14	2, Clear, 0.87, None,N,N	S	1.5ft	9ft.	30.0	30.0	0.0	29	34	869	Btuh
	Excursion									4977	Btuh
	Window Total				455 (sqft)				30581	Btuh
Walls	Туре		R-Va	alue/L	I-Value	Area(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	186	3.7		2.1	3887	Btuh
2	Frame - Wood - Adj			13.0/	0.09	333	3.0		1.5		Btuh
	Wall Total		2197 (sqft)				4390				
Doors	Туре		Area (sqft) HTM			Load	Dian				
1	Insulated - Exterior					9.8	176	Btuh			
2	Insulated - Exterior				20.0 9.8				717.5	Btuh	
	Door Total					8 (sqft)		3.0	10,000	Btuh	
Ceilings	Type/Color/Surface		R-Va	lue		Area(the same of the sa		НТМ	Load	Diuii
1	Vented Attic/DarkShingle			30.0		,					D4 -
				30.0	3100.0 3100 (saft)			1.7	5134		
Floors	Ceiling Total		D 1/-	T	3100 (sqft) Size				5134	Btuh	
1	Туре		R-Va						НТМ	Load	
1	Slab On Grade			5.0		262 (ft(p))			0.0		Btuh
	Floor Total					262.0	0 (sqft)			0	Btuh
						En	velope S	Subtotal		40477	Btuh
nfiltration	Туре		Α	СН	Volume	e(cuft) w	all area	(sqft)	CFM=	Load	
	SensibleNatural			0.70		25281	2197		337.1	5489	Btuh
Internal		(Occup	ants		Btuh/oc	cupant	Α	ppliance	Load	
gain				8)	(230) +		2400	4240	Btuh
						Se	nsible E	nvelope	Load:	50206	Btuh
uct load							(DGN	of 0.20	00)	10033	Btuh
						Sen	sible Lo	ad All Z	ones.	60239 E	Stuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Model Home

Lake City, FL 32025-

Sim-Q Developers

Code Only **Professional Version** Climate: North

1/2/2008

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	50206	Btuh
	Sensible Duct Load	10033	Btuh
	Total Sensible Zone Loads	60239	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	60239	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	10779	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1981	200 200
	Latent occupant gain (8 people @ 200 Btuh per person)	1600	Btuh
	Latent other gain	0	Btuh
	Latent total gain	14359	ACCORDING TO
	TOTAL GAIN	74598	November - 5

EQUIPMENT		
1. Central Unit	#	62000 Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Model Home

Project Title: Sim-Q Developers

Code Only Professional Version

Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/2/2008

Component Loads for Zone #1: Main

	Type*		Over	hang	Win	dow Are	a(sqft)	ł	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt			Unshaded	Shaded	Unshaded	2000	
1	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	Btuh
2	2, Clear, 0.87, None, N, N	W	1.5ft	11ft.	105.0	0.0	105.0	29	80	8349	
3	2, Clear, 0.87, None, N, N	N	5.5ft	11ft.	21.0	0.0	21.0	29	29	608	
4	2, Clear, 0.87, None, N, N	W	9.5ft	11ft.	80.0	42.6	37.4	29	80	4207	
5	2, Clear, 0.87, None, N, N	S	5.5ft	11ft.	20.0	20.0	0.0	29	34	579	
6	2, Clear, 0.87, None, N, N	N	1.5ft	11ft.	42.0	0.0	42.0	29	29	1216	0.000
7	2, Clear, 0.87, None,N,N	N	1.5ft	11ft.	4.0	0.0	4.0	29	29	116	Btuh
8	2, Clear, 0.87, None, N, N	N	1.5ft	11ft.	8.0	0.0	8.0	29	29	232	Btuh
9	2, Clear, 0.87, None,N,N	NE	1.5ft	10ft.	12.0	0.0	12.0	29	60	720	Btuh
10	2, Clear, 0.87, None,N,N	E	1.5ft	10ft.	18.0	0.0	18.0	29	80	1431	Btuh
11	2, Clear, 0.87, None,N,N	SE	1.5ft	10ft.	12.0	0.0	12.0	29	63	750	Btuh
12	2, Clear, 0.87, None,N,N	E	13ft.	11ft.	13.3	12.9	0.4	29	80	407	Btuh
13 14	2, Clear, 0.87, None, N, N	E	8.5ft	11ft.	60.0	20.5	39.5	29	80	3732	Btuh
14	2, Clear, 0.87, None,N,N	S	1.5ft	9ft.	30.0	30.0	0.0	29	34	869	Btuh
	Window Total				455 (sqft)				25603	Btuh
Walls	Туре		R-Va	alue/U	-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	186	33.7		2.1	3887	Btuh
2	Frame - Wood - Adj			13.0/	0.09	33	3.0		1.5	(-1.75)	Btuh
	Wall Total					219	7 (sqft)		55500	4390	
Doors	Туре						(sqft)		НТМ	Load	Dian
1	Insulated - Exterior						3.0		9.8		Btuh
2	Insulated - Exterior						0.0		9.8		Btuh
	Door Total						8 (sqft)		0.0		Btuh
Ceilings	Type/Color/Surface		R-Va	lue			(sqft)		нтм	Load	Dian
1	Vented Attic/DarkShingle		E 157	30.0			0.0		1.7		Dt. L
~	Ceiling Total			30.0		37000	77.7		1.7	5134	
Floors	Type		R-Va	1			0 (sqft)			5134	Btuh
1			K-va			Si			HTM	Load	
1.	Slab On Grade			5.0			32 (ft(p))		0.0		Btuh
	Floor Total					262.	0 (sqft)			0	Btuh
						Z	one Enve	lope Su	ibtotal:	35499	Btuh
nfiltration			A	СН	Volume	e(cuft) v	wall area	sqft)	CFM=	Load	
	SensibleNatural			0.70		25281	2197	S S S	294.9	5489	Btuh
Internal			ccup	ants		Btuh/oc	cupant	A	ppliance	Load	
gain				8	>	X 23	0 +		2400	4240	Btuh
						Se	ensible E	nvelope	Load:	45228	Btuh
Ouct load	Average sealed, Supply	(R6.0-A	ttic), l	Retur	n(R6.0-	Attic)	(DGM o	f 0.200)	9038	Btuh
							Sensibl	e Zone	Load	54267 E	Stuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title:
Sim-Q Developers

Code Only
Professional Version

Model Home

Lake City, FL 32025-

Climate: North

The following window Excursion will be assigned to the system loads.				
Windows	July excursion for System 1 Excursion Subtotal:	4977 Btuh 4977 Btuh		
Duct load		995 Btuh		
	Sensible Excursion Load	5972 Btuh		

Manual J Summer Calculations

Residential Load - Component Details (continued)

Model Home

Lake City, FL 32025-

Sim-Q Developers

Code Only Professional Version Climate: North

1/2/2008

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	50206	Btuł
	Sensible Duct Load	10033	Btuh
	Total Sensible Zone Loads	60239	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	60239	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	10779	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1981	Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600	Btuh
	Latent other gain	0	Btuh
	Latent total gain	14359	
	TOTAL GAIN	74598	

EQUIPMENT		
1. Central Unit	#	62000 Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

Residential Window Diversity

MidSummer

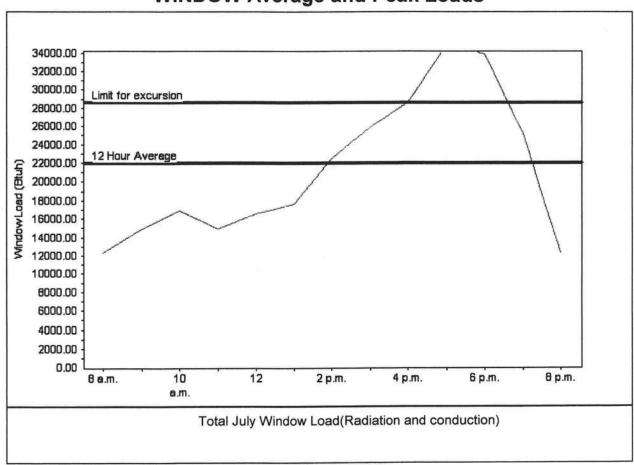
Model Home Lake City, FL 32025Project Title: Sim-Q Developers

Code Only Professional Version Climate: North

1/2/2008

Weather data for: Gainesville - Defaults					
Summer design temperature	92 F	Average window load for July	22025 Btu		
Summer setpoint	75 F	Peak window load for July	34939 Btu		
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	28633 Btu		
Latitude	29 North	Window excursion (July)	6307 Btuh		

WINDOW Average and Peak Loads



This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY: _____
DATE: _____



COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001

ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

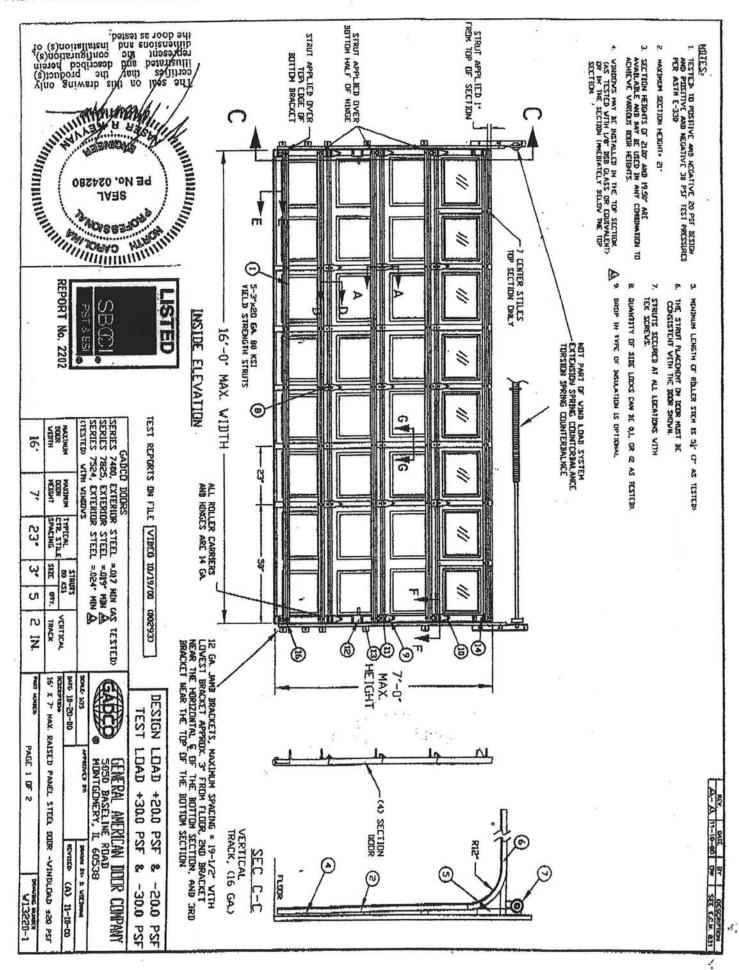
- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

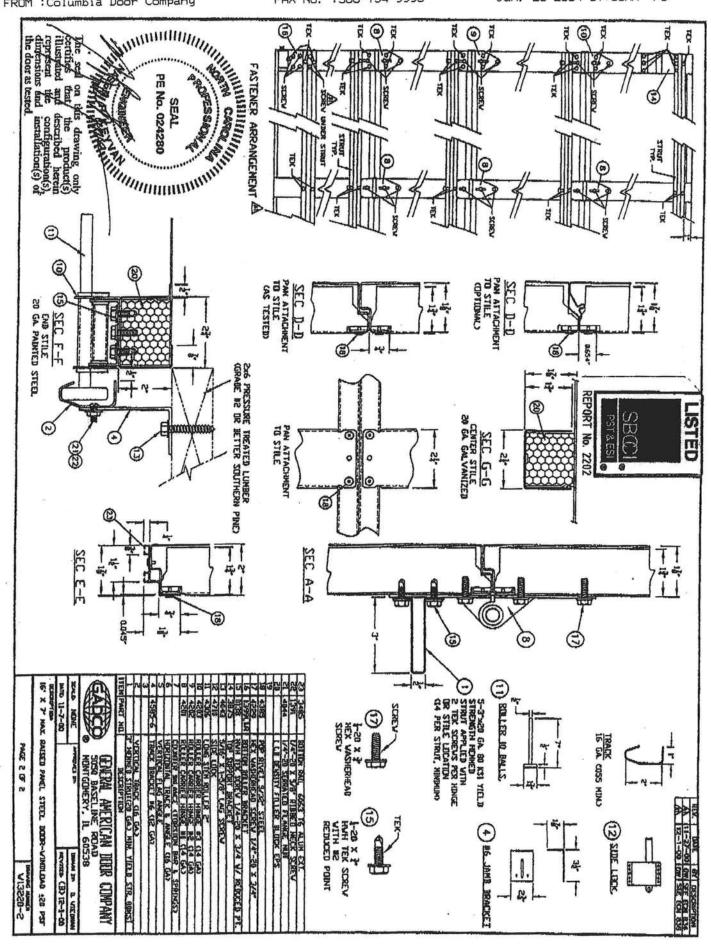
APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL	REQUIREME	NTS: Two (2) complete sets of plans containing the following:
Applicant	Plans Examiner	
ø.		All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
Ø		Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
d	0	Site Plan including: a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
,≰		Wind-load Engineering Summary, calculations and any details required a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional
ń,		Elevations including:
ø		a) All sides
Ø		b) Roof pitch
ø		c) Overhang dimensions and detail with attic ventilation
0 0 0 0 0 0 0 0 0		d) Location, size and height above roof of chimneys
Ø		e) Location and size of skylights
Ø		f) Building height
D/	П	e) Number of stories

1	2.5	Floor Plan including:
d d	D	a) Rooms labeled and dimensioned
′Ø		b) Shear walls
Ď,		c) Windows and doors (including garage doors) showing size, mfg., approval
		listing and attachment specs. (FBC 1707) and safety glazing where needed
2000		(egress windows in bedrooms to be shown)
		 d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
D		 e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
Ø		f) Must show and identify accessibility requirements (accesssable bathroom) Foundation Plan including:
Ò,		a) Location of all load-bearing wall with required footings indicated as standard
	_	Or monolithic and dimensions and reinforcing
Γ /		b) All posts and/or column footing including size and reinforcing
Ď Ž	0	c) Any special support required by soil analysis such as piling
Ø		d) Location of any vertical steel
1		Roof System:
Ø		 a) Truss package including: 1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
		2. Roof assembly (FBC 104.2.1 Roofing system, materials,
		manufacturer, fastening requirements and product evaluation with
		wind resistance rating)
4	-	b) Conventional Framing Layout including:
ø		1. Rafter size, species and spacing
		2. Attachment to wall and uplift
		3. Ridge beam sized and valley framing and support details
		4. Roof assembly (FBC 104.2.1 Roofing systems, materials,
		manufacturer, fastening requirements and product evaluation with
		wind resistance rating)
		Wall Sections including:
0	0	a) Masonry wall
	_	1. All materials making up wall
		2. Block size and mortar type with size and spacing of reinforcement
		3. Lintel, tie-beam sizes and reinforcement
		4. Gable ends with rake beams showing reinforcement or gable truss
		and wall bracing details
		All required connectors with uplift rating and required number and
		size of fasteners for continuous tie from roof to foundation
		Roof assembly shown here or on roof system detail (FBC 104.2.1
		Roofing system, materials, manufacturer, fastening requirements
		and product evaluation with resistance rating)
		7. Fire resistant construction (if required)
		8. Fireproofing requirements
		9. Shoe type of termite treatment (termiticide or alternative method)
		10. Slab on grade
		 Vapor retarder (6mil, Polyethylene with joints lapped 6 inches and sealed)
		 Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
		11. Indicate where pressure treated wood will be placed
		12. Provide insulation R value for the following:
		a. Attic space
		b. Exterior wall cavity
		c. Crawl space (if applicable)

LI	b) wood frame wall
	 All materials making up wall
	Size and species of studs
	Sheathing size, type and nailing schedule
	4. Headers sized
18	 Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
	6. All required fasteners for continuous tie from roof to foundation
	(truss anchors, straps, anchor bolts and washers)
	7. Roof assembly shown here or on roof system detail (FBC104.2.1
	Roofing system, materials, manufacturer, fastening requirements
	and product evaluation with wind resistance rating)
	8. Fire resistant construction (if applicable)
	9. Fireproofing requirements
	10. Show type of termite treatment (termiticide or alternative method)
	11. Slab on grade
	a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
	b. Must show control joints, synthetic fiber reinforcement or
	welded wire fabric reinforcement and supports
	 Indicate where pressure treated wood will be placed Provide insulation R value for the following:
	a. Attic space
	b. Exterior wall cavity
	c. Crawl space (if applicable)
	c) Metal frame wall and roof (designed, signed and sealed by Florida Prof.
	Engineer or Architect)
	Floor Framing System:
	 a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
	b) Floor joist size and spacing
	c) Girder size and spacing
D	d) Attachment of joist to girder
	e) Wind load requirements where applicable
	Plumbing Fixture layout
	Electrical layout including:
	a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
	b) Ceiling fans
	c) Smoke detectors
	d) Service panel and sub-panel size and location(s)
	e) Meter location with type of service entrance (overhead or underground)
П	f) Appliances and HVAC equipment
	HVAC information
	a) Manual J sizing equipment or equivalent computation b) Exhaust fans in bathroom
	Energy Calculations (dimensions shall match plans)
	Gas System Type (LP or Natural) Location and BTU demand of equipment
u	Disclosure Statement for Owner Builders
	Notice Of Commencement
	Private Potable Water
	a) Size of pump motor
127	b) Size of pressure tank
	c) Cycle stop valve if used

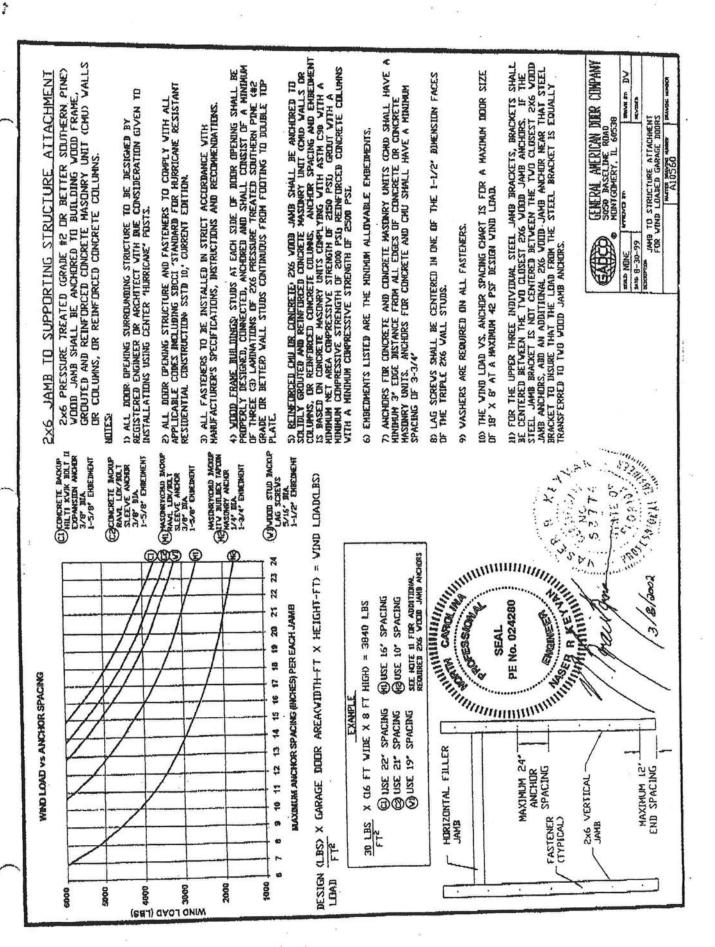




j

٠.

2







Community Affairs

BCIS Home Log In Hot Topics Submit Surcharge Stats & Facts Publications FBC Staff BCIS Site Map Links Search

Product Approval USER: Public User

Product Approval Menu > Product or Application Search > Application List > Application Detail

 COMMUNITY PLANNING NANAGENENT OFFICE OF THE HOUSING & COMMUNITY DEVELOPMENT

Application Type

Archived

の日の東田である

Application Status Comments Code Version

> New FL5108

2004

Approved

Product Manufacturer

Address/Phone/Email MI Windows and Doors

Gratz, PA 17030 650 W Market St surich@miwd.com (717) 365-3300 ext 2101

Authorized Signature

Window

surich@miwd.com Steven Urich

Technical Representative Address/Phone/Email

Address/Phone/Email Quality Assurance Representative

1 of 9

FROM-AAMA



AAMA CERTIFICATION PROGRAM



AUTHORIZATION FOR PRODUCT CERTIFICATION

MI Windows & Doors, Inc. P.O. Box 370 Gratz, PA 17030-0370

Attn: Biff Emley

The product described below is hereby approved for listing in the next issue of the AAMA Certified Products Directory. The approval is based on successful completion of tests, and the reporting to the Administrator of the results of tests, accompanied by related drawings, by an AAMA Accredited Laboratory.

The listing below will be added to the next published AAMA Certified Products Directory.

SPECIFICATION				3	
AAMA/NWWOA 101/LS. 2-97 H-R55*-36x62	RECORD OF PRODUCT TESTED				
COMPANY AND PLANT LOCATION	CODE NO.	SERIES MODEL & PRODUCT DESCRIPTION	MAXIMUM	SIZE TESTED	NO.
MI Windows & Doors, Inc. (Oldsmar, FL) MI Windows & Doors, Inc. (Smyrna, TN)	MTL-8 MTL-9	185/3185 SH (Fin) (AL)(O/O)(OG) (ASTM)	FRAME 3'0' x 5'2"	<u>SASH</u> 210° x 27°	By Request

- This Certification will expire May 14, 2008 and requires validation until then by continued listing in the current AAMA Certified Products Directory.
- Product Tested and Reported by: Architectural Testing, Inc. 3.

Report No.: 01-50360.02

Date of Report: June 14, 2004

NOTE: PLEASE REVIEW, AND ADVISE ALI IMMEDIATELY IF DATA, AS SHOWN, NEEDS CORRECTION.

Date:

August 1, 2005

CC: AAMA

JGS/df

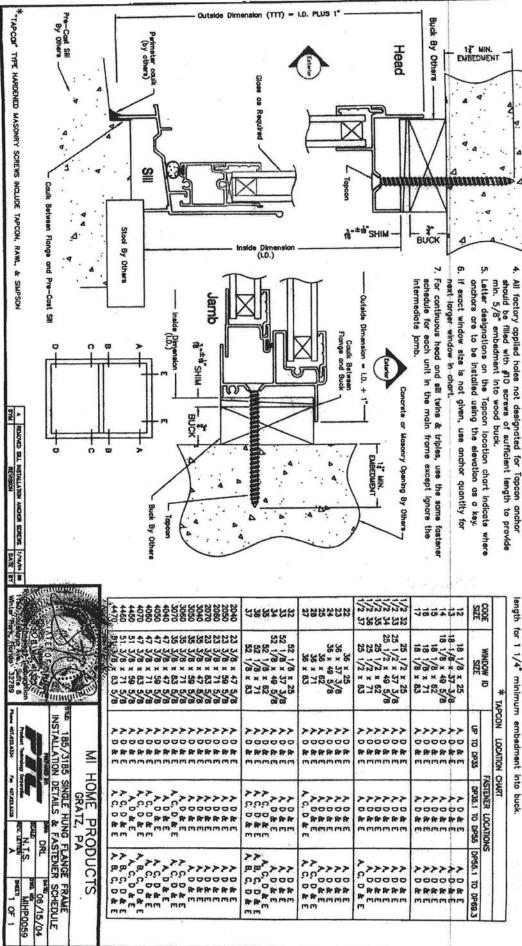
ACP-04 (Rev. 5/03)

Validated for Certification:

Associated Laboratories, Inc.

Authorized for Cartification:

American Architectural Manufacturers Association



"TWO BY" (1 1/2") BUCKS

"TWO BY" bucks are engineered and fastened to masonry opening BY OTHERS.

1. Before Installation, caulk back of flange, or face of buck

"ONE BY" (3/4") BUCKS (SHOWN)

Shim as required with load bearing shims at each installation anchor as shown. 3/16" dia. masonry Tapcon must be of a length to have 1 1/4" embedment into masonry or concrete.

'n

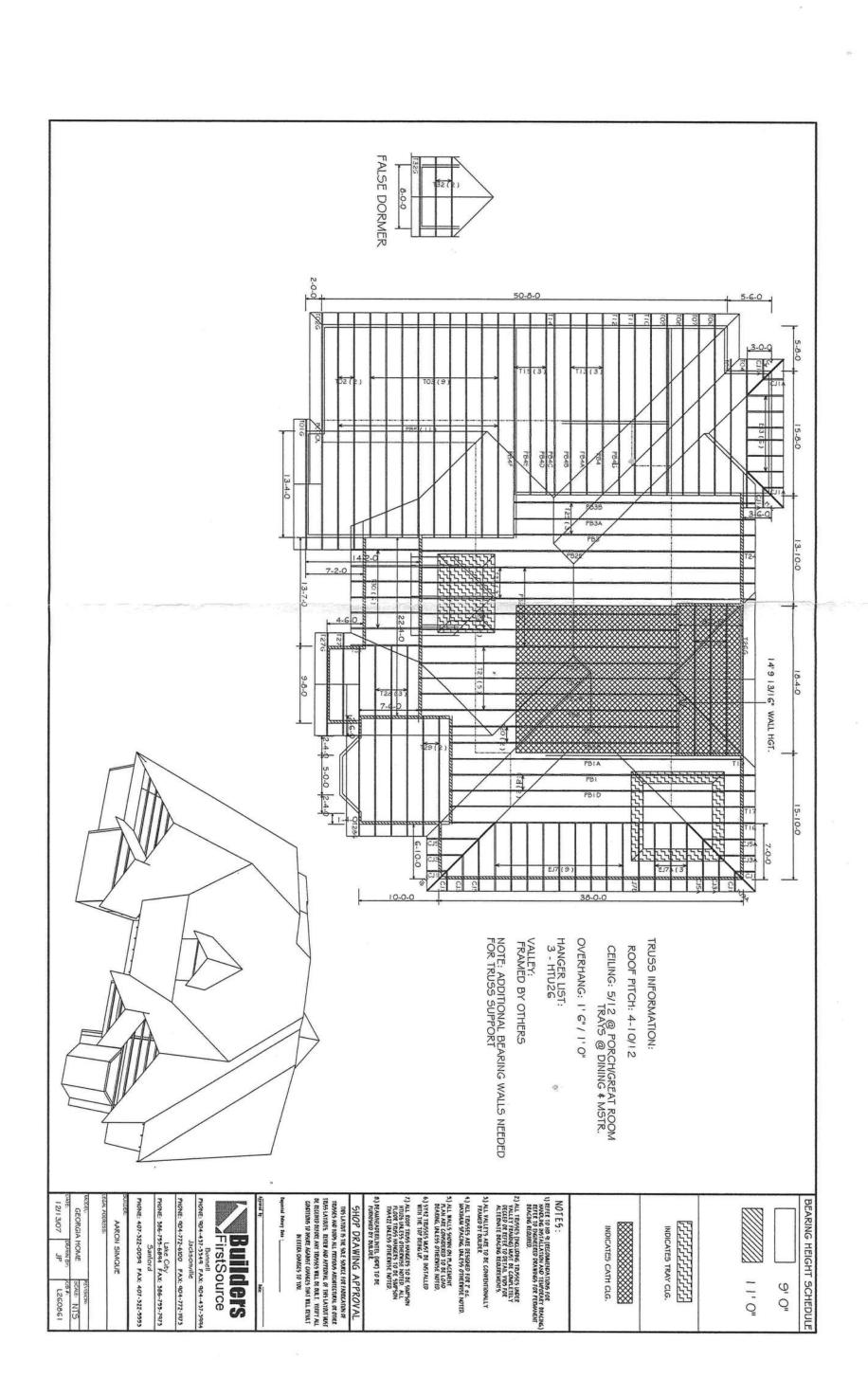
Concrete header (shown) or steel lintel By Others

length for 1 1/4" minimum embedment into buck. for "one by" bucks except use #10 screws of sufficient Follow the same instructions and fastener requirements



the

2050 2050 2050 2050 2050 2050 2050 2050	ឧឧ	222222	252555 252555 252555	388288	SIZE	
9 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	52 1/8 × 25 52 1/8 × 37 3/8 52 1/8 × 49 5/8 52 1/8 × 62 52 1/8 × 62 52 1/8 × 71 52 1/8 × 83	36 × 25 36 × 49 5/8 36 × 65/8 36 × 67 36 × 71 36 × 83	25 1/2 × 25 25 1/2 × 49 5/8 25 1/2 × 49 5/8 25 1/2 × 62 25 1/2 × 71 25 1/2 × 83	18 1/8 × 25 18 1/8 × 37 3/8 18 1/8 × 49 5/8 18 1/8 × 62 18 1/8 × 71 18 1/8 × 83	SIZE OI WOUNIN	1
人人人人人人人人人人人人人人人人 〇〇〇〇〇〇〇〇〇〇〇〇〇〇〇〇〇〇〇〇〇	人人人人人人のひりりりのようなののののののののののののののののののののののののののののののののの	>>>>> 000000 944444 mmmmmm	>>>>> 000000 \$4444 mmmmm	>>>>> 000000 000000 000000000000000000	UP 10 DP35	IAPCON LOCATION
	>>> 000 000 000 000 000 000 000 000 000	ト ト ト ト ト ト ト ト ト ト ト ト ト ト ト ト ト ト ト	**************************************	**************************************	DP35.1 TO DPS5	
	↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑	>> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	>>>>> 00000 00000 00000 000000 00000000	>>>>> 000000 888888 mmmmmm	DP55.1 TO DP69.3	



COLUMBIA COUNTY ENVIRONMENTAL HEALTH
PERMIT'X 00-009 P
OWNER: SIM- Q-DEVElopers ADDRESS: 453 S.W. ROSEMARY Dr.
FOR: A/C BARN CITY UTIL LIGHTS POOL/ENC ROOF SCREEN RM SIGN WELL OTHER City Sewer + water