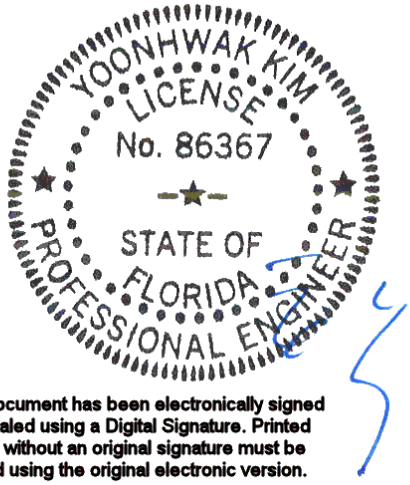

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ALPINE
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Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
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www.alpineitw.com

FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

04/29/2022

Site Information:	**Page 1:**
Customer: W. B. Howland Company, Inc.	*Job Number:* 22-7461
Job Description: Tre and Maria Johns Residence	
Address: LAKE CITY, FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	*IntelliVIEW Version:* 21.02.01 *JRef #:* 1Xf32150053
Wind Standard: ASCE 7-16 *Wind Speed (mph):* 130 *Building Type:* Closed	*Design Loading (psf):* 40.00

This package contains general notes pages, 17 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss
1	119.22.0913.42180	A01
3	119.22.0913.44830	A03
5	119.22.0913.47950	B02
7	119.22.0913.58853	B04
9	119.22.0914.02577	B06
11	119.22.0914.06867	B08
13	119.22.0914.10177	B10
15	119.22.0914.12830	B12
17	119.22.0914.17870	B14
19	GBLLETIN0118	

Item	Drawing Number	Truss
2	119.22.0913.43620	A02
4	119.22.0913.46577	B01
6	119.22.0913.49570	B03
8	119.22.0914.00830	B05
10	119.22.0914.05270	B07
12	119.22.0914.08630	B09
14	119.22.0914.11573	B11
16	119.22.0914.14047	B13
18	A14015ENC160118	
20	BRCLBSUB0119	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

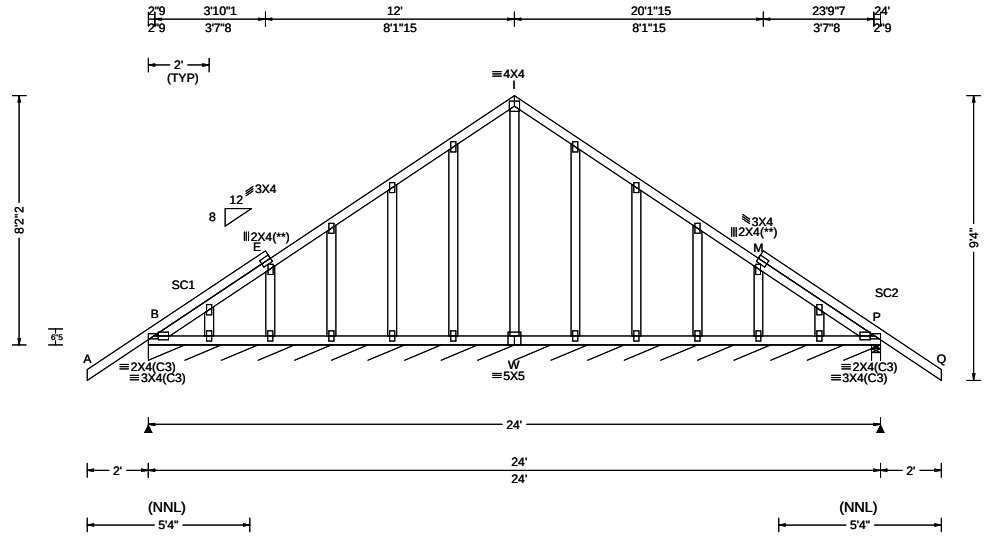
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 656254 GABL FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** A01	Cust: R 215 JRef:1Xf32150053 T1 DrwNo: 119.22.0913.42180 SSB / WHK 04/29/2022	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.002 B 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.004 B 839 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.002 M - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.004 H - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.361
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.104
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.141
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1214.12
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	81	/-	/-	/46	/14	/12
P	342	/-	/-	/201	/53	/-

Wind reactions based on MWFRS
B Brg Wid = 287 Min Req = -
P Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & P Fcperp = 565psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.

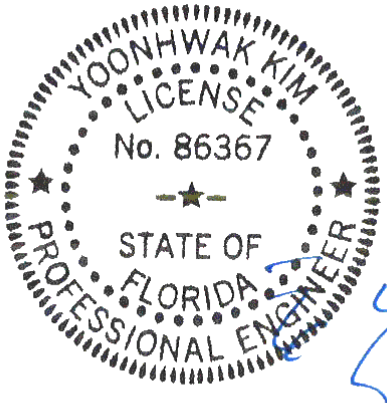
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 9-3-4.



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04/29/2022

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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155 Harlem Ave
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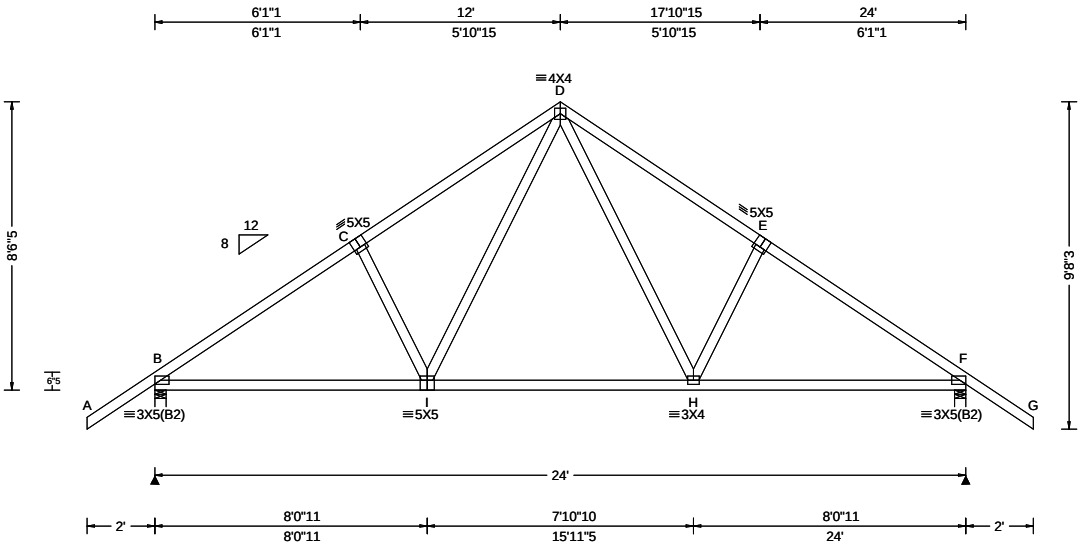
Glenview, IL 60025

SEQN: 656257 COMN FROM: CDM	Ply: 1 Qty: 4	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** A02	Cust: R 215 JRef:1Xf32150053 T2 DrwNo: 119.22.0913.43620 SSB / WHK 04/29/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.063	H	999	240
VERT(CL): 0.121	H	999	180
HORZ(LL): 0.033	F	-	-
HORZ(TL): 0.064	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.605
Max BC CSI: 0.735
Max Web CSI: 0.279

VIEW Ver: 21.02.01.1214.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1227	/-	/-	/714	/191	/292
F	1227	/-	/-	/714	/191	/-

Wind reactions based on MWFRS
B Brg Wid = 4.0 Min Req = 1.5 (Truss)
F Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings B & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	400	-1538	D - E	475	-1368
C - D	476	-1367	E - F	399	-1540

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - I	1180	-147	H - F	1181	-167
I - H	812	-31			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
I - D	555	-175	D - H	558	-173

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8-6-5.

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04/29/2022

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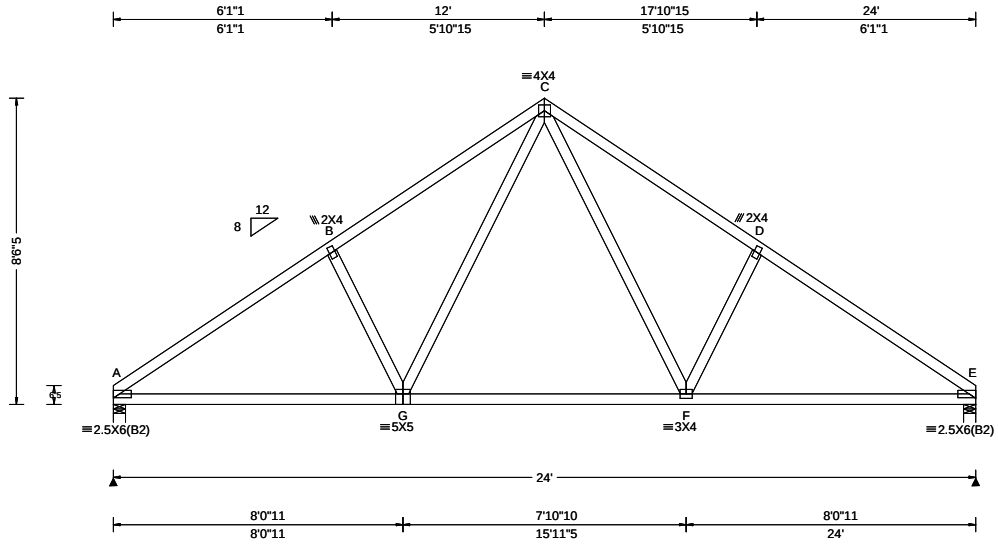
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656260 COMN FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** A03	Cust: R 215 JRef:1Xf32150053 T3 DrwNo: 119.22.0913.44830 SSB / WHK 04/29/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.044	F	999	240
VERT(CL): 0.092	F	999	180
HORZ(LL): 0.020	E	-	-
HORZ(TL): 0.042	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.368
Max BC CSI: 0.609
Max Web CSI: 0.202

VIEW Ver: 21.02.01.1214.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	1008	/-	/-	/591	/158	/221
E	1008	/-	/-	/592	/158	/-

Wind reactions based on MWFRS
A Brg Wid = 4.0 Min Req = 1.5 (Truss)
E Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings A & E are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	283	- 1422	C - D	352	- 1256
B - C	352	- 1255	D - E	283	- 1422

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - G	1096	- 151	F - E	1097	- 151
G - F	740	- 1			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
G - C	495	- 127	C - F	496	- 126

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

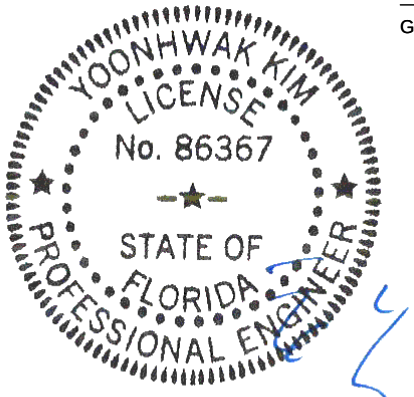
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8-6-5.



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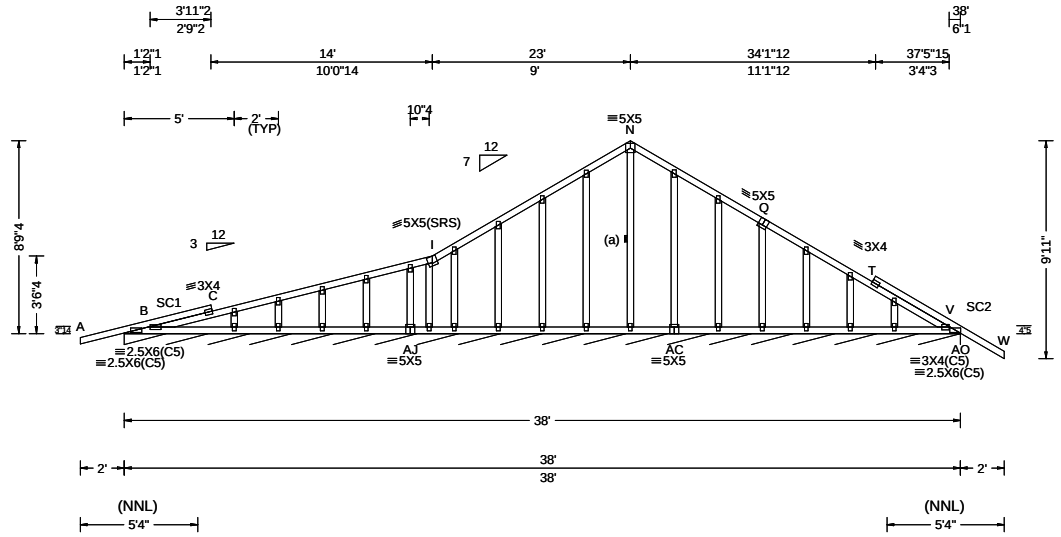
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656251 GABL FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B01		Cust: R 215 JRef:1Xf32150053 T6 DrwNo: 119.22.0913.46577 SSB / WHK 04/29/2022	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**	**▲Maximum Reactions (lbs), or *=PLF**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.80 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.014 B 999 180 HORZ(LL): 0.004 T - - HORZ(TL): 0.006 T - - Creep Factor: 2.0 Max TC CSI: 0.402 Max BC CSI: 0.121 Max Web CSI: 0.150 VIEW Ver: 21.02.01.1214.12	Gravity / Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AO*89 /- /- /45 /16 /7 Wind reactions based on MWFRS AO Brg Wid = 456 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

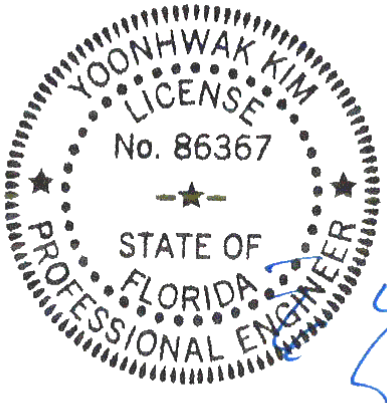
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 8-9-4.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999
04/29/2022

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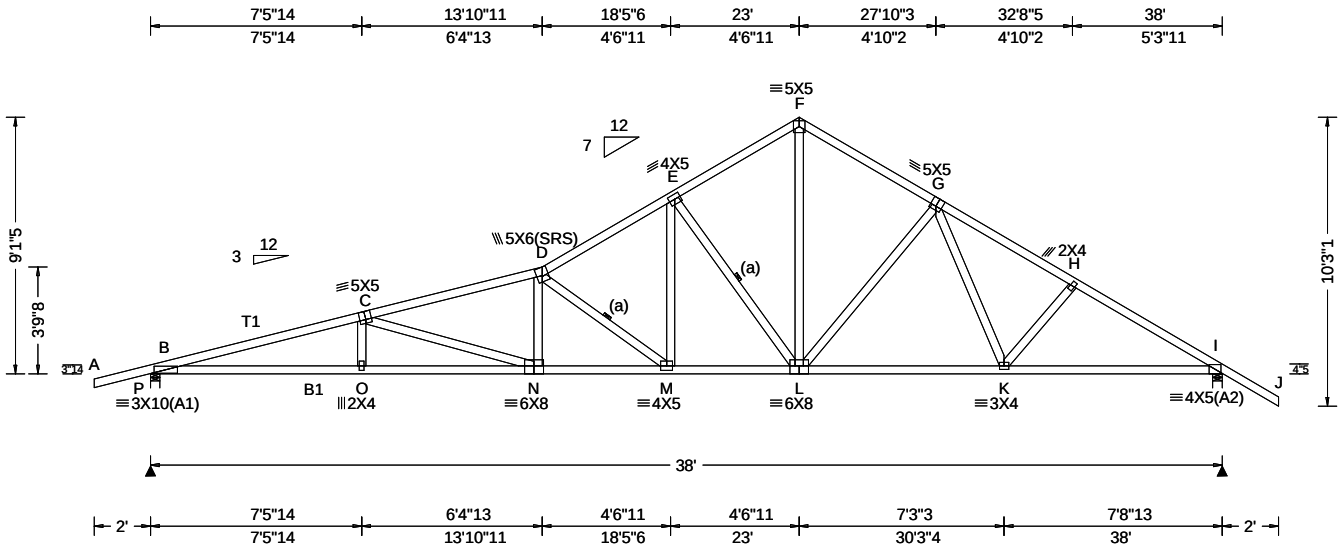
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656248 COMN FROM: CDM	Ply: 1 Qty: 2	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B02	Cust: R 215 JRef:1Xf32150053 T13 DrwNo: 119.22.0913.47950 SSB / WHK 04/29/2022



Loading Criteria (psf)

TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.80 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.375	N	999	240
VERT(CL): 0.736	N	614	180
HORZ(LL): 0.092	I	-	-
HORZ(TL): 0.180	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.888
Max BC CSI: 0.915
Max Web CSI: 0.862

VIEW Ver: 21.02.01.1214.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
P	1706	/-	/-	/892	/311	/266
I	1778	/-	/-	/1005	/297	/-

Wind reactions based on MWFRS
P Brg Wid = 4.0 Min Req = 1.5 (Truss)
I Brg Wid = 4.0 Min Req = 2.1 (Truss)
Bearings P & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1697	- 5097	F - G	742	- 2016
C - D	1421	- 4188	G - H	788	- 2570
D - E	1011	- 2947	H - I	786	- 2757
E - F	744	- 2005			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - O	4899	- 1544	M - L	2437	- 552
O - N	4893	- 1548	L - K	2012	- 381
N - M	3984	- 1189	K - I	2299	- 507

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - N	376	- 919	E - L	555	- 1302
N - D	440	- 67	F - L	1652	- 581
D - M	792	- 1916	L - G	249	- 532
M - E	1246	- 446	G - K	397	- 62

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

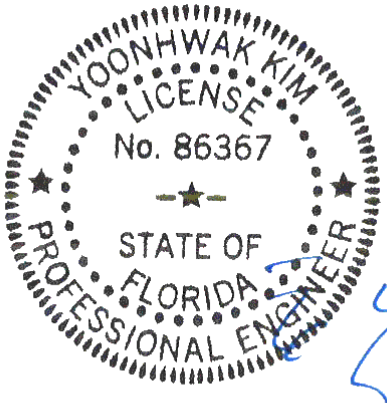
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-1-5.



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04/29/2022

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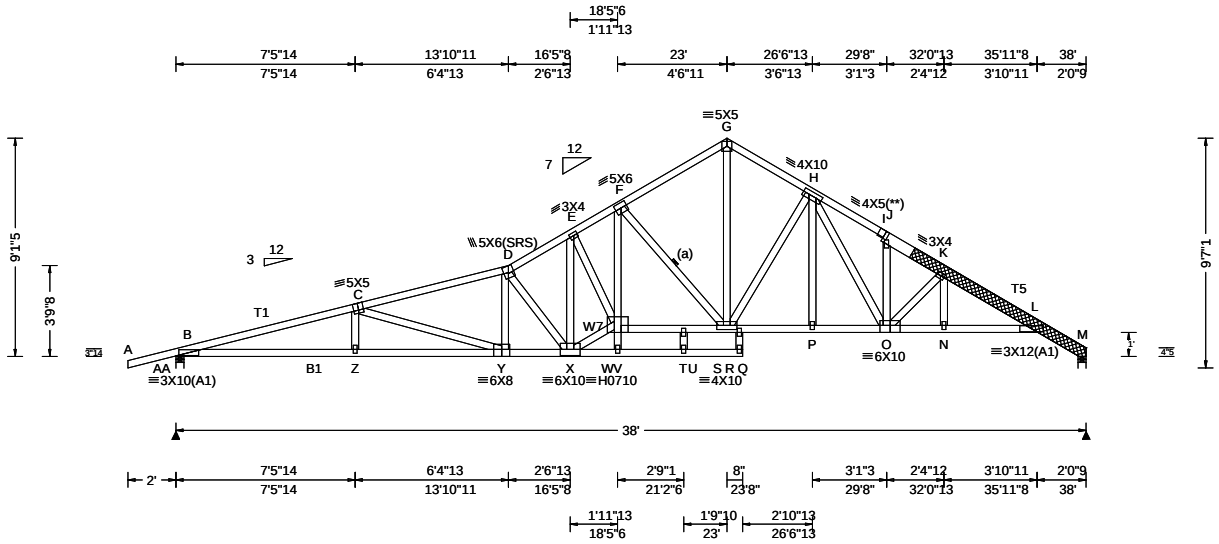
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656245 COMN FROM: CDM	Ply: 1 Qty: 5	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B03	Cust: R 215 JRef:1Xf32150053 T18 DrwNo: 119.22.0913.49570 SSB / WHK 04/29/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.419 E 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.842 E 535 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.094 L - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.189 L - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.826
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.843
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.852
	C&C Dist a: 3.80 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 4.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE, HS	VIEW Ver: 21.02.01.1214.12
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
AA	1676	/-	/-	/888	/311	/227
M	1535	/-	/-	/852	/282	/-

Wind reactions based on MWFRS
AA Brg Wid = 4.0 Min Req = 1.5 (Truss)
M Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings AA & M are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1020	- 4980	H - I	666	- 3038
C - D	794	- 4060	I - J	652	- 3045
D - E	680	- 3358	J - K	599	- 3011
E - F	682	- 3433	K - L	595	- 3258
F - G	492	- 2121	L - M	165	- 764
G - H	498	- 2105			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Z	4786	- 1011	S - R	2094	- 239
Z - Y	4779	- 1016	R - P	2140	- 248
Y - X	3862	- 688	P - O	2139	- 248
V - T	2910	- 383	O - N	3075	- 479
T - S	2895	- 382	N - L	3080	- 480

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - Y	358	- 954	F - S	379	- 1783
Y - D	407	- 63	S - H	194	- 677
D - X	450	- 1712	G - S	1789	- 381
X - V	3328	- 507	H - O	886	- 230
V - F	1801	- 337	O - K	172	- 733

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
T5 2x6 SP 2400f-2.0E;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W7 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Tray Scab(s)

(2) 2x6x8-5-7 x SP 2400f-2.0E scabs at right end. Attach one scab to each outer face of chord with: 0.131"x3", min. nails @ 8" oc, plus additional nail clusters at: BRG.: (4), heel: (6), 1st panel point: (2).

Wind

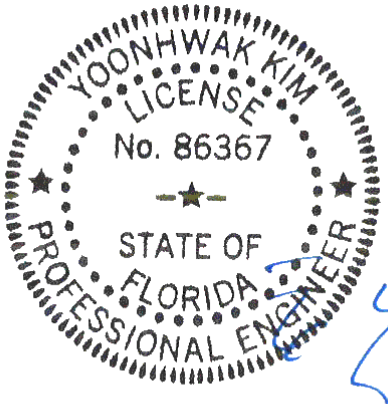
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-1-5.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).



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04/29/2022

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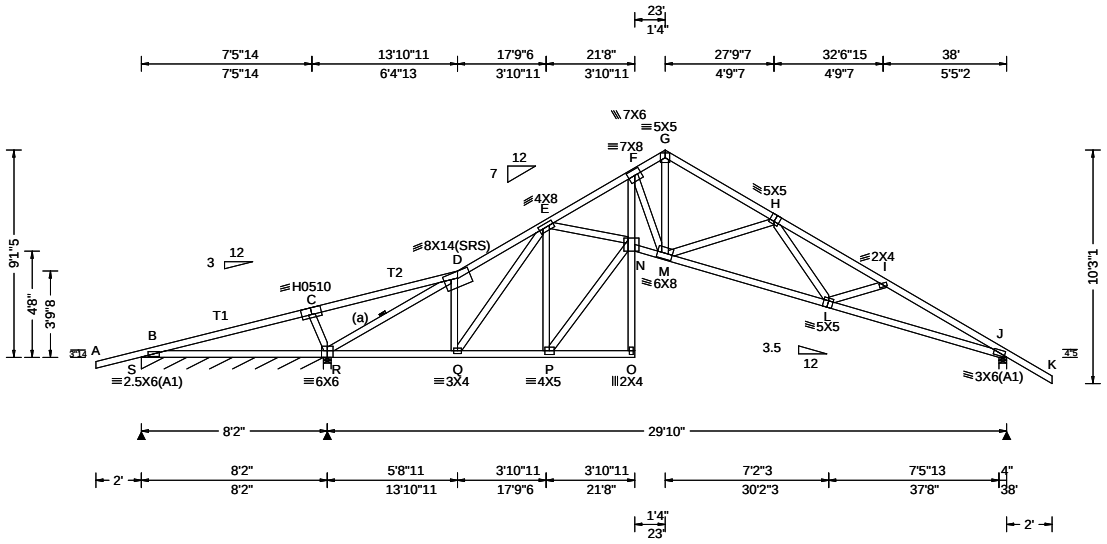
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656263 COMN FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B04	Cust: R 215 JRef:1Xf32150053 T4 DrwNo: 119.22.0913.58853 SSB / WHK 04/29/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.80 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.252	N	999	240
VERT(CL): 0.512	N	695	180
HORZ(LL): 0.193	J	-	-
HORZ(TL): 0.391	J	-	-

Creep Factor: 2.0
Max TC CSI: 0.630
Max BC CSI: 0.930
Max Web CSI: 0.885

VIEW Ver: 21.02.01.1214.12

▲Maximum Reactions (lbs), or *=PLF

		Gravity			Non-Gravity	
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
S*	63	/-44	/-	/15	/7	/33
R	2104	/-	/-	/1106	/101	/-
J	1257	/-	/-	/799	/33	/-
B		/-284				

Wind reactions based on MWFRS
S Brg Wid = 96.0 Min Req = -
R Brg Wid = 4.0 Min Req = 2.5
J Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings S, R, & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1807	- 166	F - G	278	- 1857
C - D	1934	- 164	G - H	254	- 1943
D - E	350	- 822	H - I	417	- 2832
E - F	265	- 2712	I - J	500	- 3136

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - R	523	- 3446	N - M	2506	- 10
R - Q	670	- 75	M - L	2314	- 143
Q - P	926	- 42	L - J	2741	- 331

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - R	291	- 507	P - N	1489	- 65
R - D	443	- 2941	N - F	2059	- 56
D - Q	448	0	F - M	121	- 1841
Q - E	13	- 473	M - G	1607	- 175
E - P	90	- 1109	M - H	246	- 622
E - N	1414	0	H - L	472	- 35

Lumber

Top chord: 2x4 SP #2; T1,T2 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.

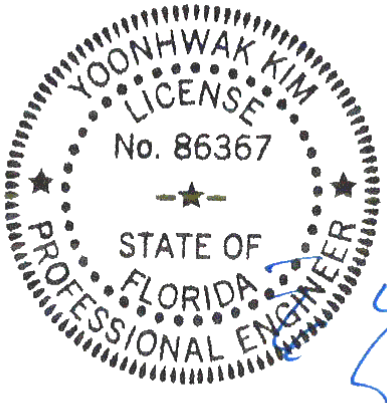
Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -354# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

Shim all supports to solid bearing.

The overall height of this truss excluding overhang is 9-1-5.



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04/29/2022

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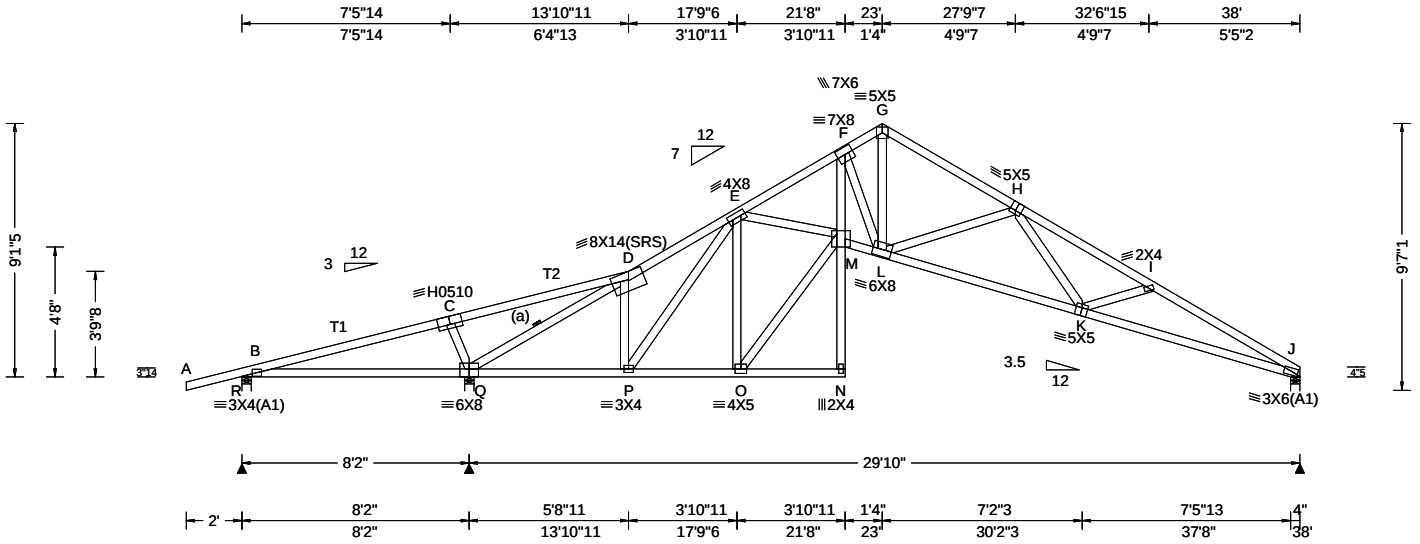
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656266 COMN FROM: CDM	Ply: 1 Qty: 2	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B05		Cust: R 215 JRef:1Xf32150053 T17 DrwNo: 119.22.0914.00830 SSB / WHK 04/29/2022	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.250 M 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.513 M 694 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.191 J - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.392 J - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.519
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.852
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.896
	C&C Dist a: 3.80 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 21.02.01.1214.12
	GCpi: 0.18	WAVE, HS	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

Loc	Gravity R+	Gravity / R-	Gravity / Rh	Non-Gravity / Rw	Non-Gravity / U	Non-Gravity / RL
R	211	/-287	/-	/-	/184	/227
Q	2189	/-	/-	/1199	/25	/-
J	1117	/-	/-	/677	/15	/-

Wind reactions based on MWFRS
R Brg Wid = 4.0 Min Req = 1.5 (Truss)
Q Brg Wid = 4.0 Min Req = 2.6
J Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings R, Q, & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1810	- 262	F - G	344	- 1875
C - D	1937	- 264	G - H	322	- 1961
D - E	306	- 832	H - I	504	- 2901
E - F	381	- 2740	I - J	595	- 3224

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	291	- 1726	M - L	2534	- 87
Q - P	679	- 97	L - K	2349	- 256
P - O	935	- 93	K - J	2832	- 466

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - Q	291	- 508	O - M	1503	- 147
Q - D	518	- 2955	M - F	2083	- 155
D - P	448	0	F - L	197	- 1860
P - E	35	- 476	L - G	1622	- 238
E - O	157	- 1121	L - H	253	- 641
E - M	1430	0	H - K	491	- 53

Lumber

Top chord: 2x4 SP #2; T1,T2 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.

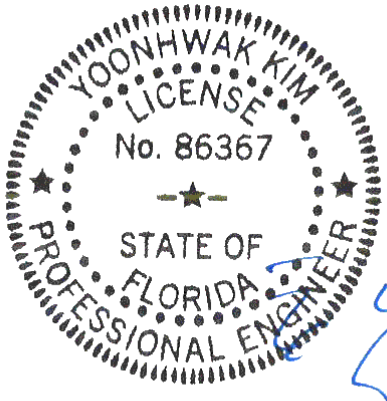
Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -287# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

Shim all supports to solid bearing.

The overall height of this truss excluding overhang is 9-1-5.



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Florida Certificate of Product Approval #FL 1999
04/29/2022

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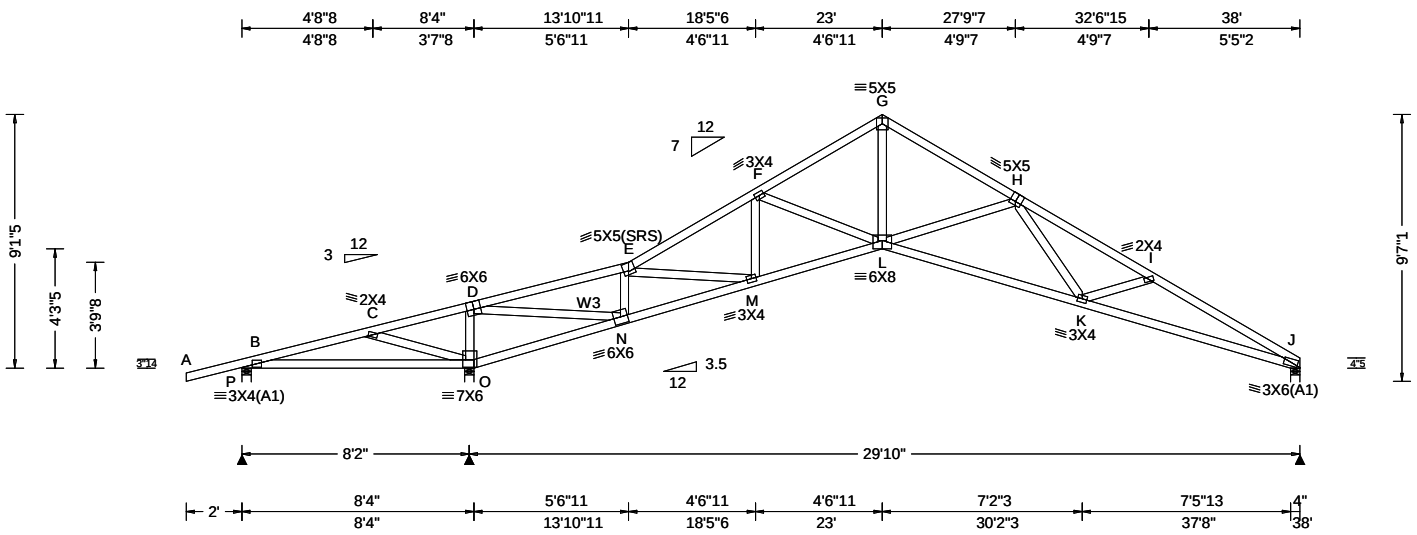
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656269 COMN FROM: CDM	Ply: 1 Qty: 4	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B06	Cust: R 215 JRef:1Xf32150053 T7 DrwNo: 119.22.0914.02577 SSB / WHK 04/29/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.80 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.209	H	999	240
VERT(CL): 0.435	H	814	180
HORZ(LL): 0.142	J	-	-
HORZ(TL): 0.294	J	-	-

Creep Factor: 2.0
Max TC CSI: 0.747
Max BC CSI: 0.833
Max Web CSI: 0.670

VIEW Ver: 21.02.01.1214.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
P	176	/-309	/-	/-	/199	/227
O	2257	/-	/-	/1241	/27	/-
J	1102	/-	/-	/669	/12	/-

Wind reactions based on MWFRS
P Brg Wid = 4.0 Min Req = 1.5 (Truss)
O Brg Wid = 4.0 Min Req = 2.7 (Truss)
J Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings P, O, & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1822	- 285	F - G	306	- 1886
C - D	2142	- 332	G - H	303	- 1895
D - E	226	- 1041	H - I	482	- 2852
E - F	353	- 2041	I - J	574	- 3176

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - O	312	- 1745	M - L	1796	- 132
O - N	403	- 2182	L - K	2294	- 235
N - M	1177	- 163	K - J	2790	- 447

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - O	182	- 519	E - M	638	- 19
D - O	334	- 1326	G - L	1539	- 179
D - N	2970	- 498	L - H	248	- 649
N - E	261	- 1142	H - K	507	- 53

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W3 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

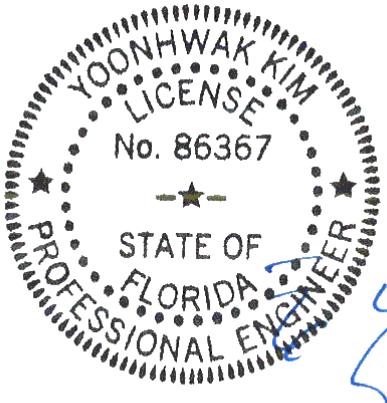
Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -309# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

Shim all supports to solid bearing.

The overall height of this truss excluding overhang is 9-1-5.



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04/29/2022

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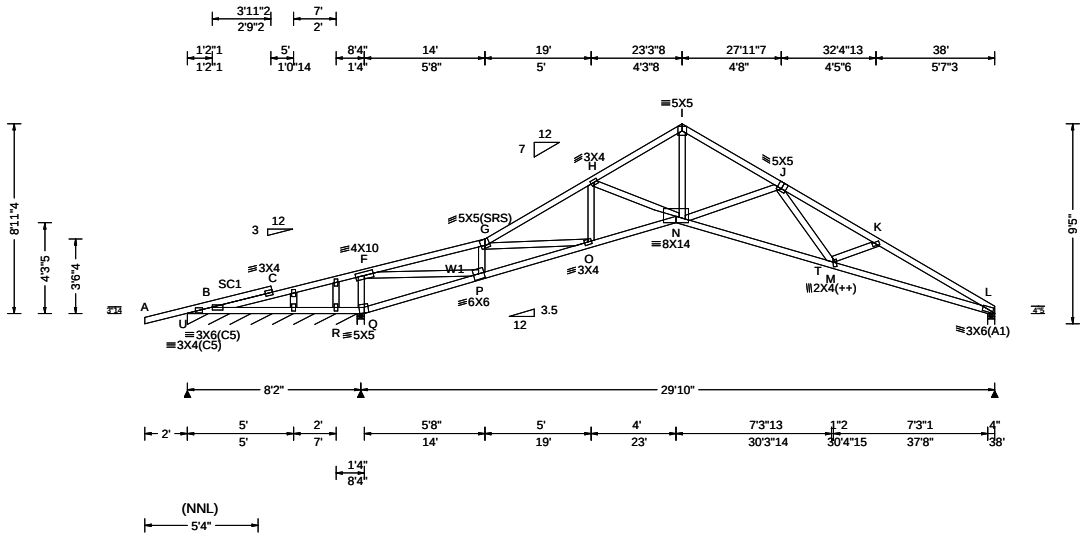
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656272 GABL	Ply: 1	**Job Number:** 22-7461	Cust: R 215 JRef:1Xf32150053 T8
FROM: CDM	Qty: 1	Tre and Maria Johns Residence	DrwNo: 119.22.0914.05270
Page 1 of 2		**Truss Label:** B07	SSB / WHK 04/29/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.80 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.235	N	999	240
VERT(CL): 0.482	N	735	180
HORZ(LL): 0.159	L	-	-
HORZ(TL): 0.326	L	-	-

Creep Factor: 2.0
Max TC CSI: 0.946
Max BC CSI: 0.824
Max Web CSI: 0.781

VIEW Ver: 21.02.01.1214.12

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
U*	41	/-56	/-	/-	/26	/29
Q	2235	/-	/-	/1244	/18	/-
L	1107	/-	/-	/671	/13	/-
B		/-258				
R		/-182				

Wind reactions based on MWFRS
U Brg Wid = 96.0 Min Req = -
Q Brg Wid = 4.0 Min Req = 2.6 (Truss)
L Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings U, Q, & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	2216	- 375	H - I	330	- 2043
C - F	2258	- 344	I - J	309	- 1924
F - G	294	- 1389	J - K	487	- 2875
G - H	393	- 2322	K - L	569	- 3186

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	367	- 2140	O - N	2039	- 171
Q - P	398	- 2279	N - M	2330	- 247
P - O	1550	- 251	M - L	2796	- 440

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
F - P	3461	- 564	J - T	503	- 49
G - O	473	0	M - T	515	- 74
N - J	247	- 651			

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.	Gables	Tens.	Comp.
F - Q	353	- 1417	N - I	1656	- 203
P - G	270	- 1128			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1 2x4 SP #2;
Stack Chord: SC1 2x4 SP #2;

Plating Notes

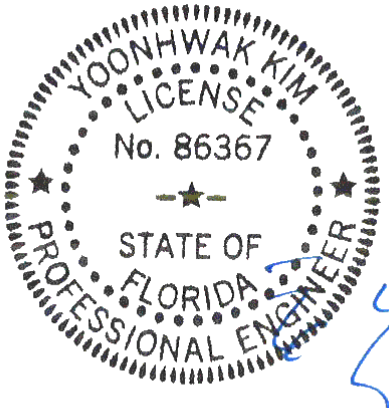
All plates are 2X4 except as noted.
(++) - This plate works for both joints covered.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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04/29/2022

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656272 GABL FROM: CDM Page 2 of 2	Ply: 1 Qty: 1	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B07	Cust: R 215 JRef:1Xf32150053 T8 DrwNo: 119.22.0914.05270 SSB / WHK 04/29/2022

Additional Notes

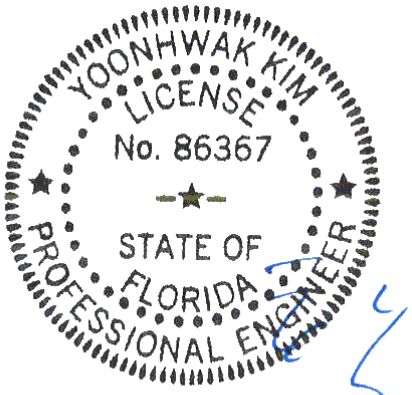
Negative reaction(s) of -450# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Shim all supports to solid bearing.

The overall height of this truss excluding overhang is 8-11-4.



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04/29/2022

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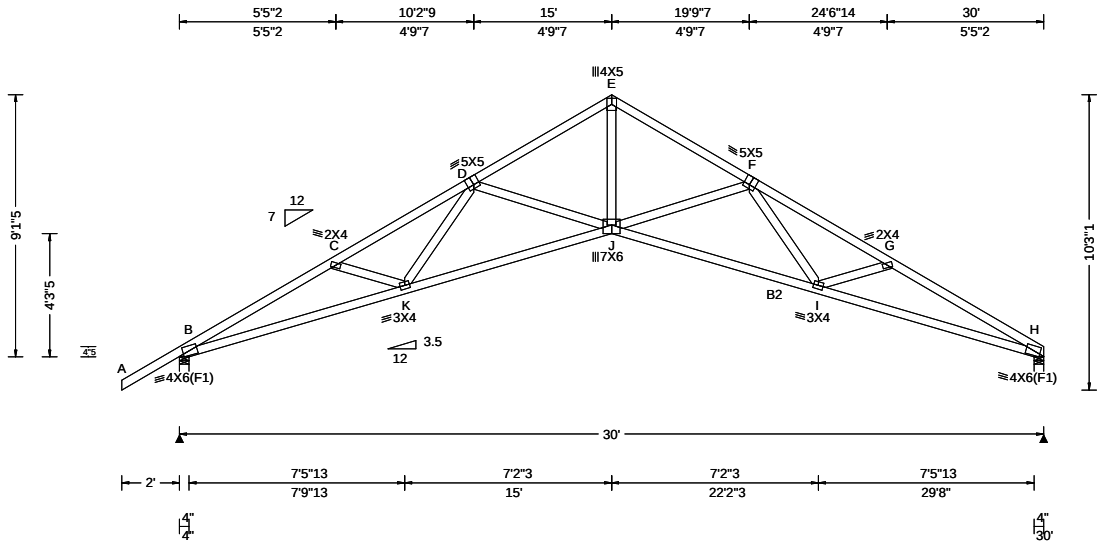
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656275 COMN FROM: CDM	Ply: 1 Qty: 4	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B08	Cust: R 215 JRef:1Xf32150053 T9 DrwNo: 119.22.0914.06867 SSB / WHK 04/29/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.292	J	999	240
VERT(CL): 0.606	J	587	180
HORZ(LL): 0.235	H	-	-
HORZ(TL): 0.487	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.694
Max BC CSI: 0.991
Max Web CSI: 0.812

VIEW Ver: 21.02.01.1214.12

▲Maximum Reactions (lbs)

		Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh		/ Rw	/ U	/ RL
B	1401	/-	/-		/838	/21	/262
H	1255	/-	/-		/723	/11	/-

Wind reactions based on MWFRS
B Brg Wid = 4.0 Min Req = 1.5 (Truss)
H Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings B & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	616	- 3641	E - F	383	- 2489
C - D	533	- 3341	F - G	558	- 3395
D - E	384	- 2490	G - H	648	- 3707

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - K	3190	- 479	J - I	2809	- 305
K - J	2788	- 295	I - H	3259	- 513

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
K - D	451	- 28	J - F	245	- 626
D - J	234	- 603	F - I	470	- 49
E - J	2131	- 254			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31; B2 2x4 SP #2;
Webs: 2x4 SP #3;

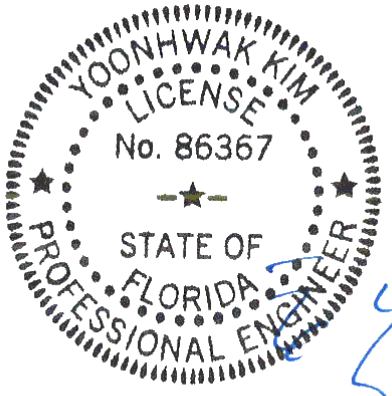
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-1-5.



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04/29/2022

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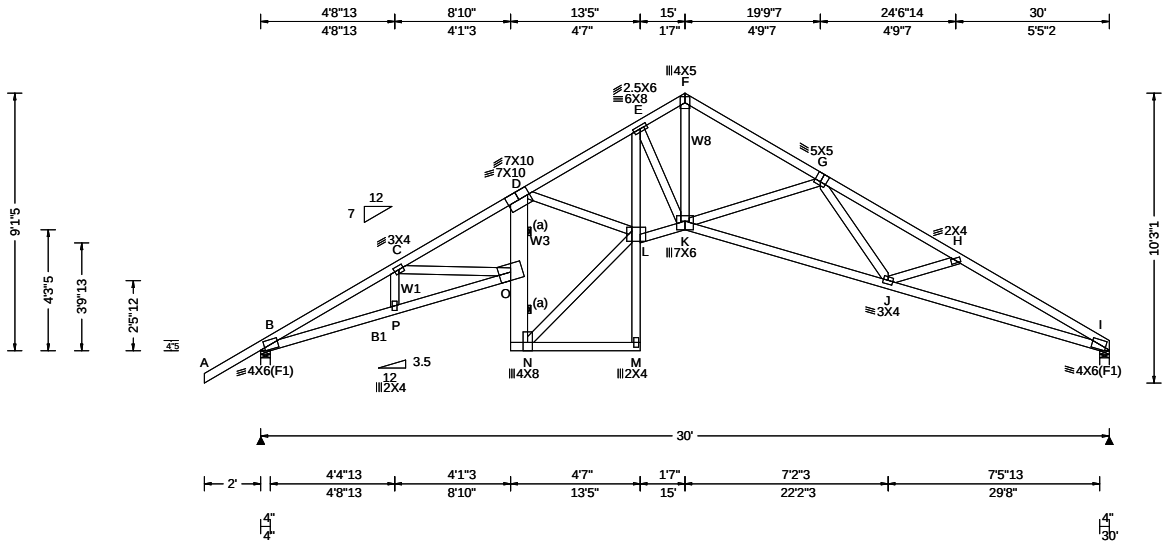
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656278 COMN FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B09	Cust: R 215 JRef:1Xf32150053 T15 DrwNo: 119.22.0914.08630 SSB / WHK 04/29/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.486	N	733	240
VERT(CL): 1.007	N	353	180
HORZ(LL): 0.433	I	-	-
HORZ(TL): 0.896	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.596
Max BC CSI: 0.987
Max Web CSI: 0.968

VIEW Ver: 21.02.01.1214.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1398	/-	/-	/835	/22	/262
I	1253	/-	/-	/721	/12	/-

Wind reactions based on MWFRS
B Brg Wid = 4.0 Min Req = 1.5 (Truss)
I Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings B & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	576	- 3591	F - G	386	- 2475
C - D	531	- 3200	G - H	560	- 3394
D - E	450	- 2871	H - I	650	- 3704
E - F	407	- 2403			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - P	3129	- 446	K - J	2803	- 309
P - O	3162	- 456	J - I	3257	- 514
L - K	2449	- 134			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
O - D	72	- 480	E - K	197	- 844
O - N	179	- 1399	K - G	249	- 637
D - L	1049	0	F - K	2181	- 304
N - L	2082	- 226	G - J	481	- 46
L - E	827	- 139			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W1,W8 2x4 SP #2;
W3 2x8 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

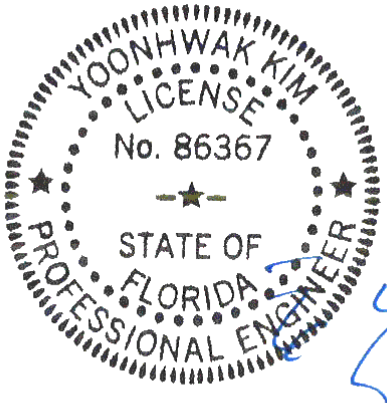
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-1-5.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999
04/29/2022

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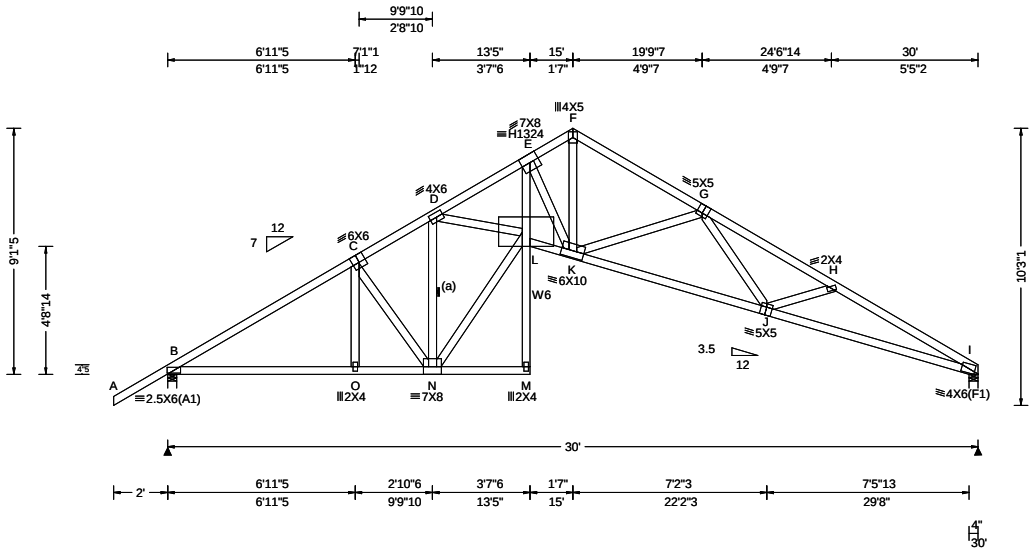
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656282 COMN FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B10	Cust: R 215 JRef:1Xf32150053 T16 DrwNo: 119.22.0914.10177 SSB / WHK 04/29/2022



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.327 L 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.676 L 526 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.276 I - -
Des Ld:	40.00	EXP: C Kzt: NA		HORZ(TL): 0.570 I - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.589
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.989
Spacing:	24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.941
		C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 9.00 ft	Plate Type(s):	
		GCpi: 0.18	WAVE, HS	VIEW Ver: 21.02.01.1214.12
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1392	/-	/-	/836	/22	/262
I	1251	/-	/-	/720	/12	/-

Wind reactions based on MWFRS
B Brg Wid = 4.0 Min Req = 1.6 (Truss)
I Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings B & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	361	- 1970	F - G	385	- 2478
C - D	391	- 1679	G - H	563	- 3383
D - E	510	- 3887	H - I	652	- 3695
E - F	407	- 2401			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - O	1608	- 218	K - J	2801	- 311
O - N	1609	- 217	J - I	3249	- 516
L - K	3641	- 216			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
N - D	131	- 1677	E - K	313	- 2866
N - L	2470	- 251	K - F	2134	- 300
D - L	1927	- 42	K - G	251	- 626
L - E	3264	- 283	G - J	467	- 48

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W6 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

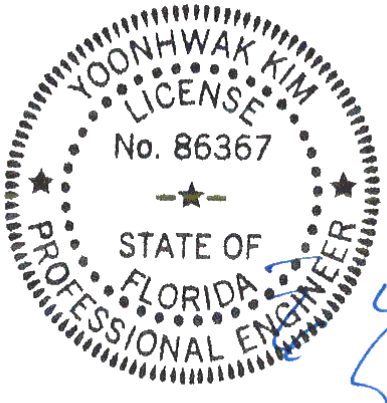
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-1-5.



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Florida Certificate of Product Approval #FL 1999
04/29/2022

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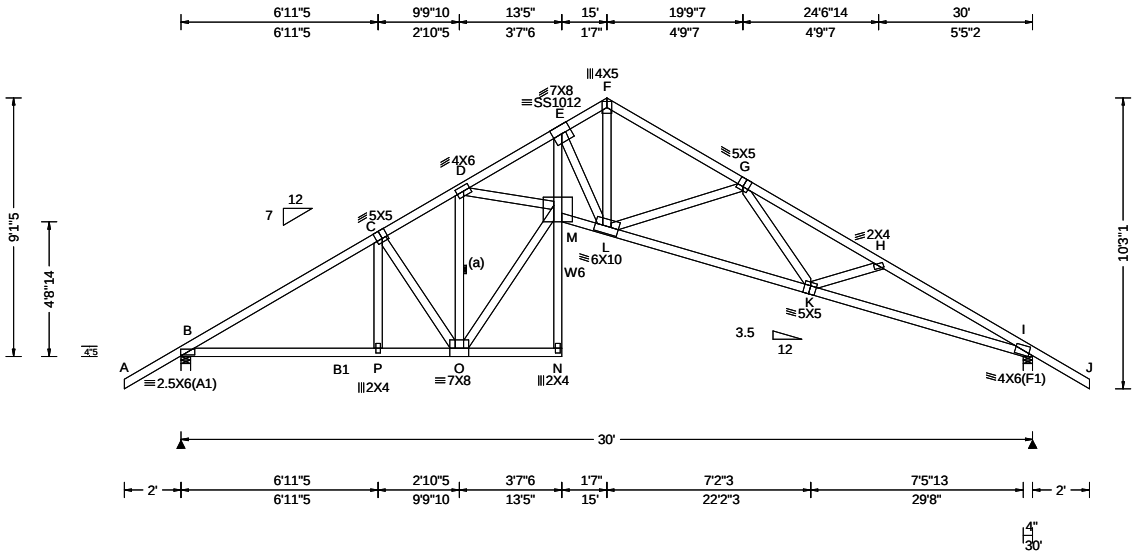
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656285 COMN FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B11	Cust: R 215 JRef:1Xf32150053 T10 DrwNo: 119.22.0914.11573 SSB / WHK 04/29/2022	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.320 M 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.654 M 544 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.262 I - -
Des Ld:	40.00	EXP: C Kzt: NA		HORZ(TL): 0.536 I - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.685
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.534
Spacing:	24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.950
		C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 9.00 ft	Plate Type(s):	
		GCpi: 0.18	WAVE, 18SS	VIEW Ver: 21.02.01.1214.12
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1387	/-	/-	/836	/21	/285
I	1393	/-	/-	/835	/22	/-

Wind reactions based on MWFRS
B Brg Wid = 4.0 Min Req = 1.6 (Truss)
I Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings B & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	352	- 1957	F - G	321	- 2465
C - D	386	- 1669	G - H	478	- 3311
D - E	390	- 3939	H - I	562	- 3612
E - F	344	- 2387			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - P	1595	- 152	L - K	2766	- 201
P - O	1594	- 153	K - I	3164	- 386
M - L	3733	- 54			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - O	135	- 380	E - L	201	- 2998
O - D	56	- 1672	L - F	2121	- 239
O - M	2470	- 156	L - G	243	- 601
D - M	1976	0	G - K	448	- 30
M - E	3417	- 149			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;
Webs: 2x4 SP #3; W6 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

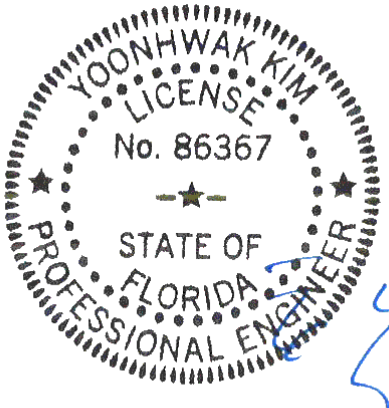
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-1-5.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999
04/29/2022

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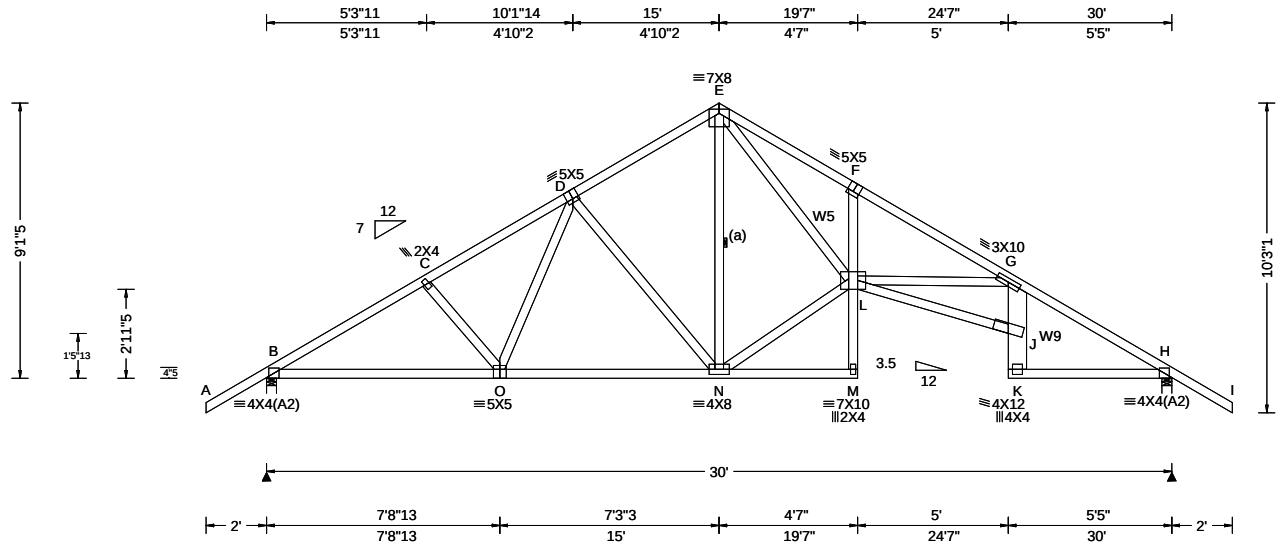
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656308 COMN FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B12	Cust: R 215 JRef:1Xf32150053 T24 DrwNo: 119.22.0914.12830 SSB / WHK 04/29/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.259	F	999	240
VERT(CL): 0.528	F	674	180
HORZ(LL): 0.179	H	-	-
HORZ(TL): 0.364	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.584
Max BC CSI: 0.879
Max Web CSI: 0.599

VIEW Ver: 21.02.01.1214.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1384	/-	/-	/833	/23	/285
H	1386	/-	/-	/835	/22	/-

Wind reactions based on MWFRS
B Brg Wid = 4.0 Min Req = 1.6 (Truss)
H Brg Wid = 4.0 Min Req = 1.6 (Truss)
Bearings B & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	376	-1996	E - F	575	-3190
C - D	382	-1805	F - G	460	-3223
D - E	351	-1295	G - H	359	-2020

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - O	1648	-191	L - J	3333	-409
O - N	1368	-96	K - H	1662	-193

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
O - D	405	-37	N - L	1253	0
D - N	190	-500	L - G	210	-451
E - L	2655	-309	G - J	187	-992

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W5 2x4 SP #2;
W9 2x8 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

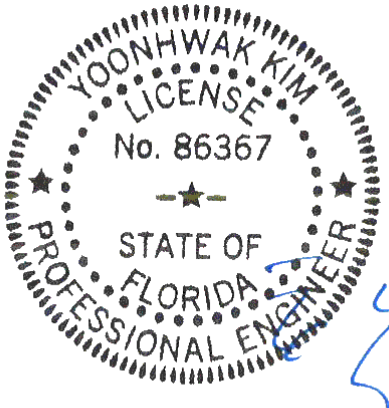
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-1-5.



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Florida Certificate of Product Approval #FL 1999
04/29/2022

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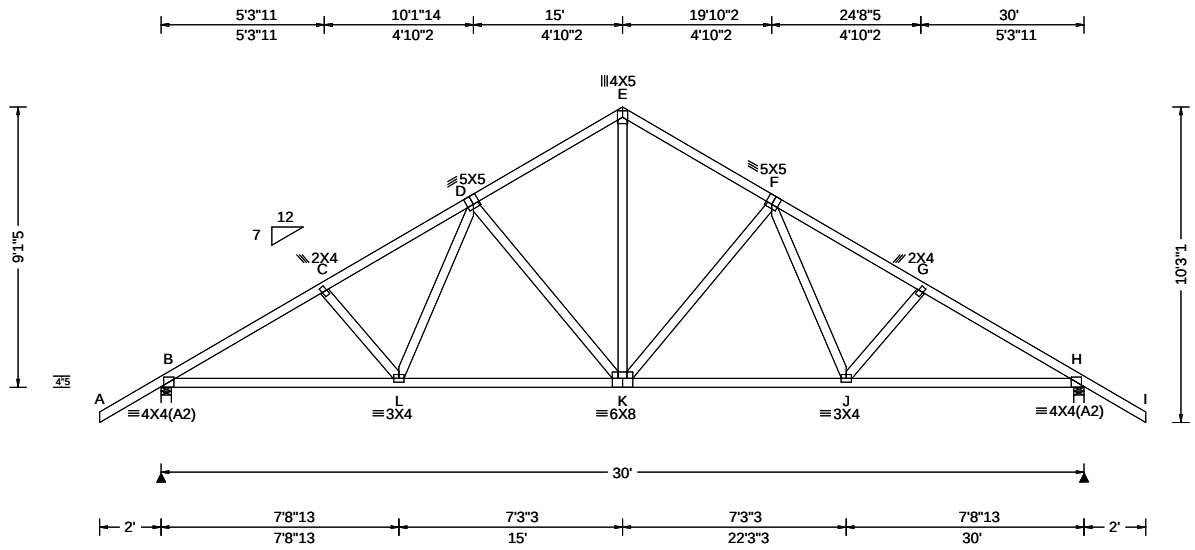
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656291 COMN FROM: CDM	Ply: 1 Qty: 6	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B13	Cust: R 215 JRef:1Xf32150053 T11 DrwNo: 119.22.0914.14047 SSB / WHK 04/29/2022



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.099	K	999	240
VERT(CL): 0.189	K	999	180
HORZ(LL): 0.044	H	-	-
HORZ(TL): 0.084	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.435
Max BC CSI: 0.692
Max Web CSI: 0.602

VIEW Ver: 21.02.01.1214.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1477	/-	/-	/832	/239	/285
H	1477	/-	/-	/832	/239	/-

Wind reactions based on MWFRS
B Brg Wid = 4.0 Min Req = 1.7 (Truss)
H Brg Wid = 4.0 Min Req = 1.7 (Truss)
Bearings B & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	377	- 2183	E - F	352	- 1435
C - D	383	- 1994	F - G	382	- 1994
D - E	351	- 1435	G - H	377	- 2183

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - L	1808	- 203	K - J	1511	- 100
L - K	1511	- 99	J - H	1808	- 208

Maximum Web Forces Per Ply (lbs)

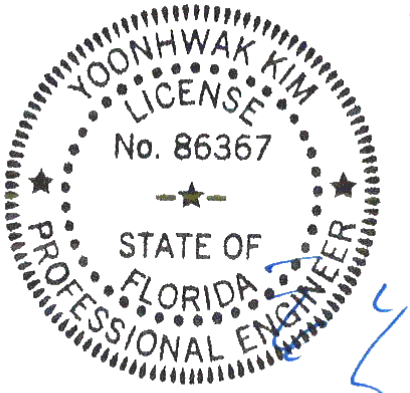
Webs	Tens.	Comp.	Webs	Tens.	Comp.
L - D	403	- 37	K - F	188	- 532
D - K	188	- 532	F - J	403	- 37
E - K	1060	- 219			

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 9-1-5.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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04/29/2022

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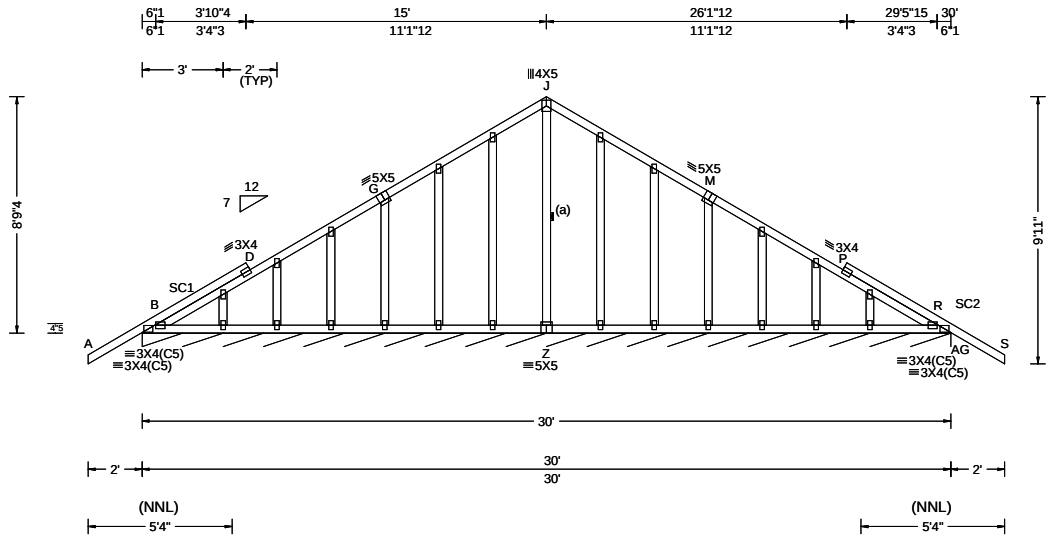
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 656311 GABL FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 22-7461 Tre and Maria Johns Residence **Truss Label:** B14	Cust: R 215 JRef:1Xf32150053 T5 DrwNo: 119.22.0914.17870 SSB / WHK 04/29/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.002 D 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.004 D 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.003 D - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.004 H - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.403
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.044
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.149
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1214.12
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AG	*92	/-	/-	/47	/16	/9

Wind reactions based on MWFRS
AG Brg Wid = 360 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

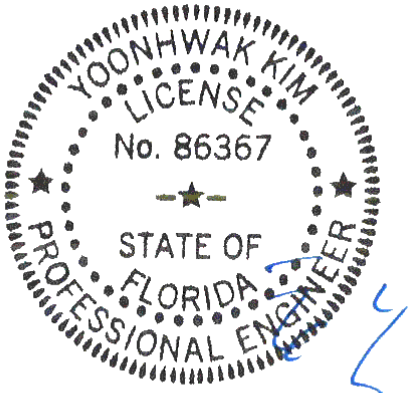
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 8-9-4.



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04/29/2022

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Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) 'L' braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

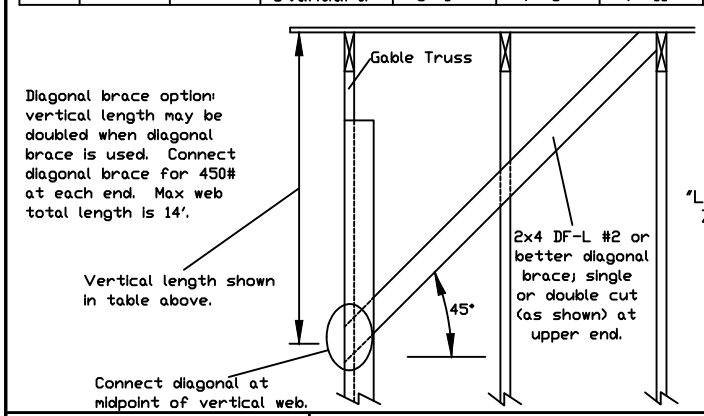
'L' bracing must be a minimum of 80% of web member length.

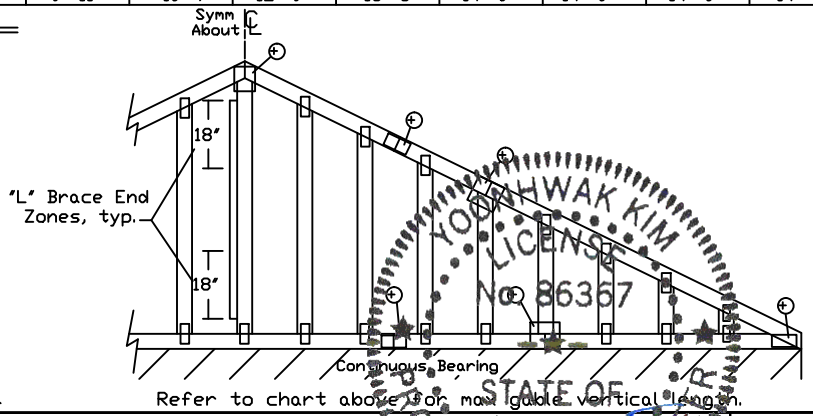
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





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ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

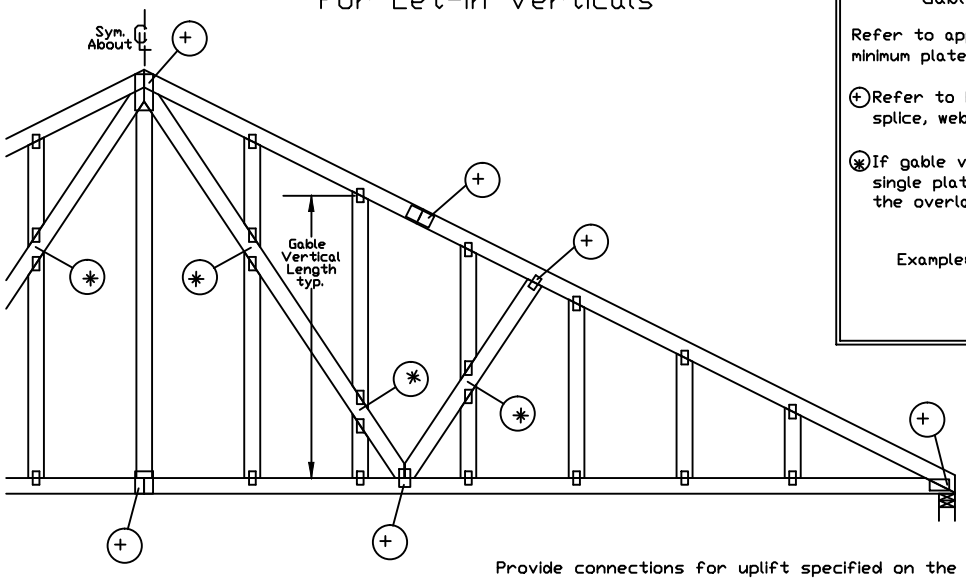
MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-16-GAB14015
DATE	01/26/2018
DRWG	A14015ENC160118

01/26/2018
PE REG# 278, Yoonhwak Kim, FL PE #86367

Gable Detail
For Let-in Verticals



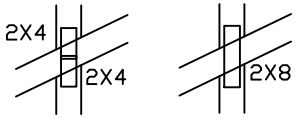
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

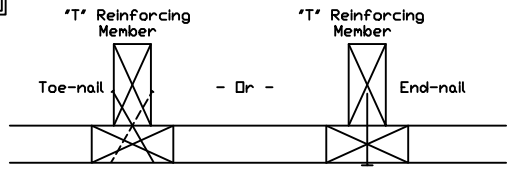
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊛ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3.",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

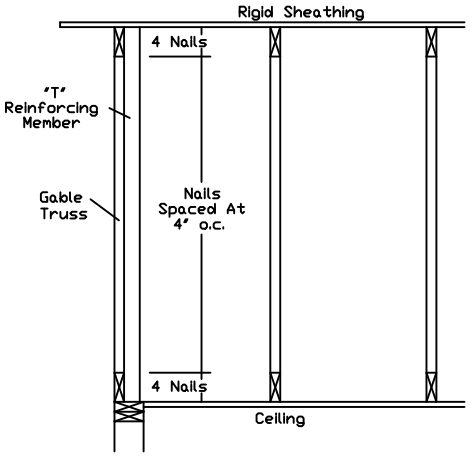
Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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Glenview, IL 60025

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MAX. TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX. SPACING	24.0"

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

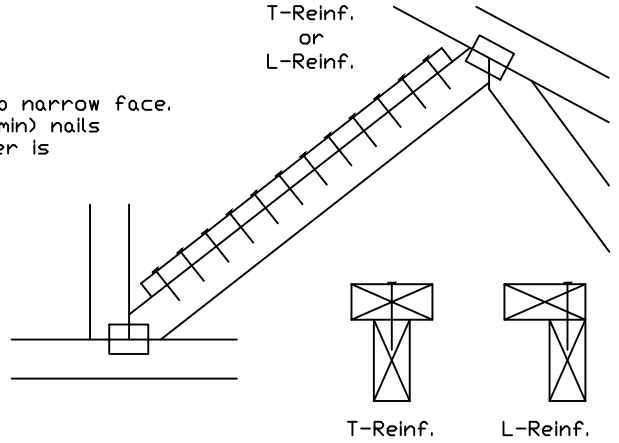
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Alternative Reinforecement Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

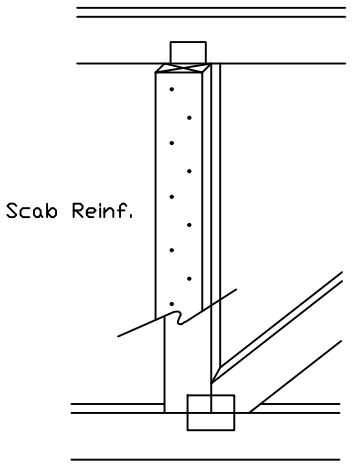
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



YOONHWAK KIM LICENSE No. 86367 STATE OF FLORIDA PROFESSIONAL ENGINEER

ALPINE AN ITW COMPANY

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

TC LL		PSF
TC DL		PSF
BC DL		PSF
BC LL		PSF
TOT. LD.		PSF
DUR. FAC.		
SPACING		

REF	CLR Subst.
DATE	01/02/19
DRWG	BRCLBSUB0119

4/29/2022
FL Reg.# 78, Yoonhwak Kim, FL PE #86367