

Width (ft) = 40	Eave Height (ft) = 22.67 H/S
Length (ft) = 100	Roof Slope (Rise/12) = 2.0:12

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 23 / 8TH EDITION

B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

BASIC SEISMIC FORCE RESISTING SYSTEM	(LATERAL DIRECTIONS) =	ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYSTEM	(ENDWALLS) =	ORDINARY STEEL CONCENTRICALLY BRACED FRAMES
BASIC SEISMIC FORCE RESISTING SYSTEM	(LONGITUDINAL DIRECTIONS) =	ORDINARY STEEL CONC. BRACED FRAMES
ANALYSIS PROCEDURE	=	EQUIVALENT LATERAL FORCE PROCEDURE

STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

MINIMUM DESIGN DEFLECTIONS			
Endwall Column	= 180	Roof Panel (Live)	= 60
Endwall Rafter (Live)	= 180	Roof Panel (Wind)	= 60
Endwall Rafter (Wind)	= 180	Rigid Frame (Horz)	= 60
Wall Girt	= 90	Rigid Frame (Vert)	= 180
Roof Purlin (Live)	= 150	Rigid Frame (Seismic)	= 50
Roof Purlin (Wind)	= 150		
Wall Panel	= 60		

A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.

B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.

3. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS – ALLOWABLE STRESS DESIGN" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D11.1. AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
- 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD-FORMED SECTIONS, CONFORM TO ASTM-A529 OR A572 . FLANGES AND WEB MATERIAL CONFORMS TO ASTM-A509 OR A572 GRADE 55 WITH A MINIMUM YIELD POINT OF 55,000 psi.
- 2) MATERIAL PROPERTIES OF HSS ROUND SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM YIELD POINT OF 42,000 psi.
- 3) MATERIAL PROPERTIES OF HSS RECT. OR SQUARE SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM YIELD POINT OF 46,000 psi.
- 4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992 WITH MINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992 WITH MINIMUM YIELD POINT OF 50,000 PSI.
- 5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.
- 6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY IN ACCORDANCE WITH AZ55 SPECIFICATIONS.
- 7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT CONDITION.
- 8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.
- 9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY FRAMING MUST BE "SNUG-TIGHT", EXCEPT AS FOLLOWS:
"FULLY-PRETENSION" A-325 BOLTS IF:
 - a) BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".
 - b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
 - c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS – REVERSALS ON THE CONNECTIONS.
 - d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 – SC".

- APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:

- A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:
 - 1) BE MADE IN CONTRASTING INK.
 - 2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
 - 3) BE LEGIBLE AND UNAMBIGUOUS.
- B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.
- C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.
- D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER.
- E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION. MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.

B) IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.

C) LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY.

D) MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.

E) DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.

B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS REQUIRED.

C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, LATEST ED.)

D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE LATEST ED.)

E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACTURER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.

F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.

G) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR IMMEDIATELY UPON ARRIVAL. CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATURE THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.

IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.

THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.

H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

I) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION. (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, LATEST ED.)

J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION. IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)

K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

COMPONENTS & CLADDING (unfactored)					
Wall Field Values	=	29.011	psf	/	-31.428 psf
Wall Edge Values	=	29.011	psf	/	-38.681 psf

REV.	PAGE	DESCRIPTION
	0	COVER PAGE
	1	ANCHOR BOLT LAYOUT
	1.1	ANCHOR BOLT DETAILS
	1.2	ANCHOR BOLT REACTIONS
	2	ROOF FRAMING LAYOUT
	2.1	RIGID FRAME CROSS SECTION
	3-3.1	ENDWALL FRAMING/SHEETING LAYOUT
	4-4.1	SIDEWALL FRAMING/SHEETING LAYOUT
	5-5.4	FRAMING DETAILS
	6	ROOF PANELS & TRIM
	6.1	ROOF PANEL DETAILS
	7	SIDEWALL PANEL DETAILS
	8	ENDWALL PANEL DETAILS
	9	SPECIAL DETAILS

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE.

FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMMODATE THE SEISMIC STORY DRIFTS.



☐ FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.

☒ FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFIED. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.

☐ FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION

COLORS:	
ROOF:	BLACK
WALLS:	BLACK
GABLE:	BLACK
EAVE:	BLACK
CORNER:	BLACK
FRAMED OPENINGS:	BLACK
BASE:	BLACK

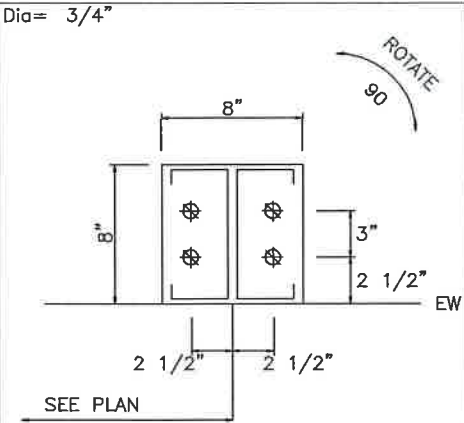
[illegible]

⌀ Dia = 3/4"

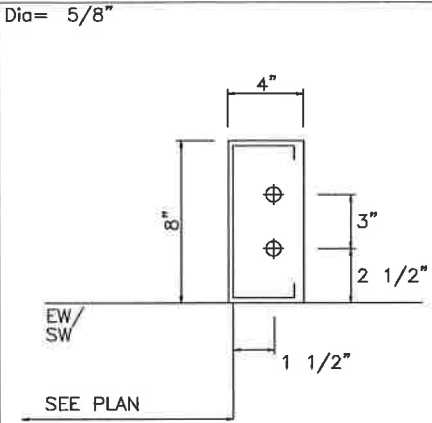


FIELD LOCATE:
(2) 3'-4" x 7'-2" FRAMED OPENING

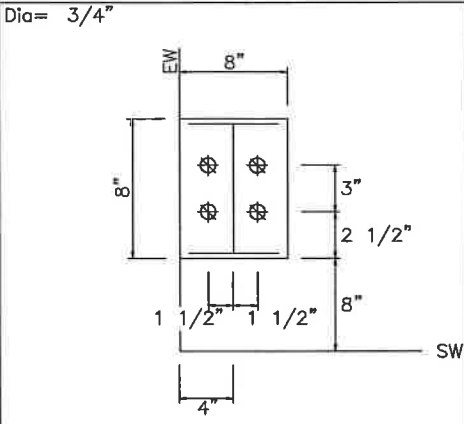
ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A		DATE: 1/2/25		
LOCATION: LAKE CITY, FL. 32024				
DRAWING NAME: ANCHOR BOLT LAYOUT				
DRAWING NO: PAGE 1		DRAWN BY: BJC		CHECKED BY: SPW
				SCALE: NONE



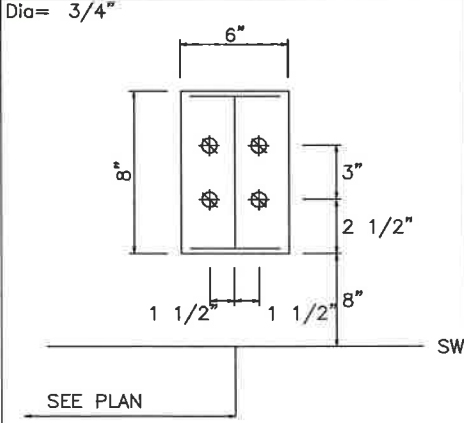
DETAIL A



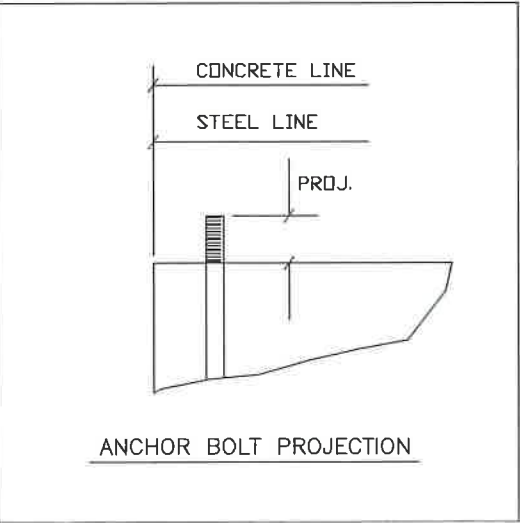
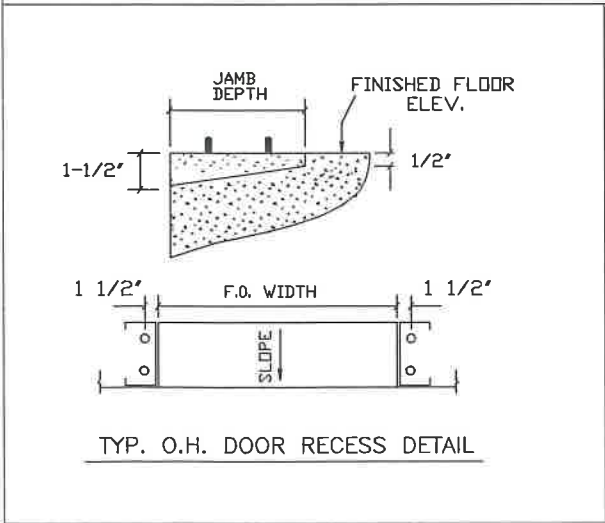
DETAIL D



DETAIL B

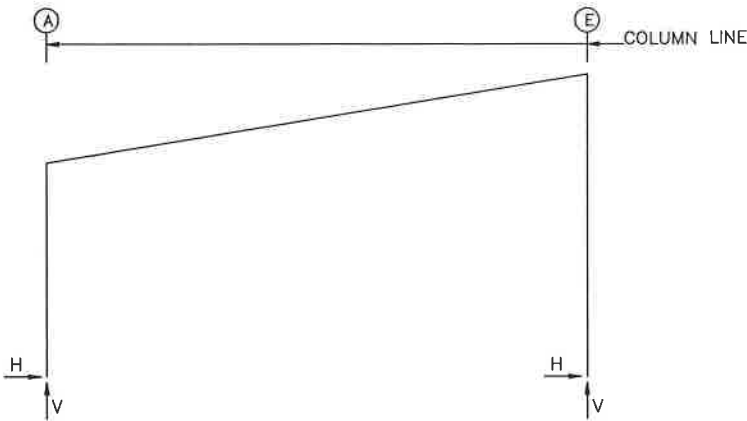


DETAIL C



ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER:				
THE WOODS CONTAINER PARK				
JOB NO:		DATE:		
8967A		1/2/25		
LOCATION:				
LAKE CITY, FL. 32024				
DRAWING NAME:				
ANCHOR BOLT DETAILS				
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:	
PAGE 1.1	BJC	SPW	NONE	

FRAME LINES: 2 3 4 5



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in)		Thick	Grout (in)
				Width	Length		
2*	A	4	0.750	6.000	8.000	0.375	0.0
2*	E	4	0.750	6.000	8.000	0.375	0.0

2* Frame lines: 2 3 4 5

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in)		Thick	Grout (in)
				Width	Length		
1	A	4	0.750	8.000	8.000	0.375	0.0
1	C	4	0.750	8.000	8.000	0.375	0.0
1	E	4	0.750	8.000	8.000	0.375	0.0
6	E	4	0.750	8.000	8.000	0.375	0.0
6	C	4	0.750	8.000	8.000	0.375	0.0
6	A	4	0.750	8.000	8.000	0.375	0.0

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width	(ft)	=	40.0
Length	(ft)	=	100.0
Eave Height	(ft)	=	16.0/ 22.7
Roof Slope	(rise/12)	=	2.00
Roof Dead Load	(psf)	=	3.0
Wall Dead Load			
Left Endwall	(psf)	=	2.0
Right Endwall	(psf)	=	2.0
Front Sidewall	(psf)	=	2.0
Back Sidewall	(psf)	=	2.0
Roof Live Load	(psf)	=	20.0
Frame Live Load	(psf)	=	12.0
Collateral Load	(psf)	=	5.0
Wind Speed	(mph)	=	120.0
Wind Code		=	FBC 23 (8th Edition)
Exposure		=	C
Closure		=	Enclosed
Internal Wind Coeff		=	-0.18, +0.18
Risk Category		=	II - Normal
Importance - Wind		=	1.00
Importance - Seismic		=	1.00
Seismic Design Category		=	B
Seismic Coeff	(Sms)	=	0.16

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Wind_Left1		Wind_Right1		Wind_Left2	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	A	0.5	1.7	0.7	2.1	1.6	4.9	-5.6	-10.7	3.7	-3.4	-6.2	-6.7
2*	E	-0.5	1.9	-0.7	2.1	-1.6	5.1	-0.8	-8.7	6.3	-9.5	-0.2	-4.7

Frame Line	Column Line	Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	A	3.3	0.6	0.4	-9.3	0.9	-6.3	-0.2	-0.2	0.2	0.2
2*	E	6.7	-5.5	0.5	-10.2	-1.2	-6.0	-0.1	0.2	0.1	-0.2

2* Frame lines: 2 3 4 5

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert	Seis Left Vert
1	A	0.5	0.5	1.9	-2.7	-1.4	-1.5	-0.3	-2.2	2.4	-2.9	-1.9	0.0
1	C	1.4	1.5	5.8	-9.4	-5.7	-6.4	-2.7	-4.1	4.5	-9.0	-5.4	0.0
1	E	0.6	0.5	1.9	-2.5	-2.2	-1.8	-1.4	-3.1	3.5	-2.6	-1.4	0.0

Frm Line	Col Line	Seis Right Vert	Seis Long Vert	E1PAT_LL_1 Horz	E1PAT_LL_2 Horz
1	A	0.0	0.0	0.0	2.2
1	C	0.0	0.0	0.0	2.9
1	E	0.0	0.0	0.0	-0.3

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert	Seis Left Vert
6	E	0.6	0.5	1.9	-2.2	-2.5	-1.4	-1.8	-3.1	3.5	-2.6	-1.4	0.0
6	C	1.4	1.5	5.8	-5.7	-9.4	-2.7	-6.4	-4.1	4.5	-9.0	-5.4	0.0
6	A	0.5	0.5	1.9	-1.4	-2.7	-0.3	-1.5	-2.2	2.4	-2.9	-1.9	0.0

Frm Line	Col Line	Seis Right Vert	Seis Long Vert	E2PAT_LL_1 Horz	E2PAT_LL_2 Horz
6	E	0.0	0.0	0.0	2.2
6	C	0.0	0.0	0.0	2.9
6	A	0.0	0.0	0.0	-0.3

ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Type	Proj (in)
⊕ 12		Jamb	5/8"	A307	1.50
⊕ 24		Endwall	3/4"	GR36	1.50
⊕ 32		Frame	3/4"	GR36	2.50

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- ALL REACTIONS ARE UNFACTORED.
- ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

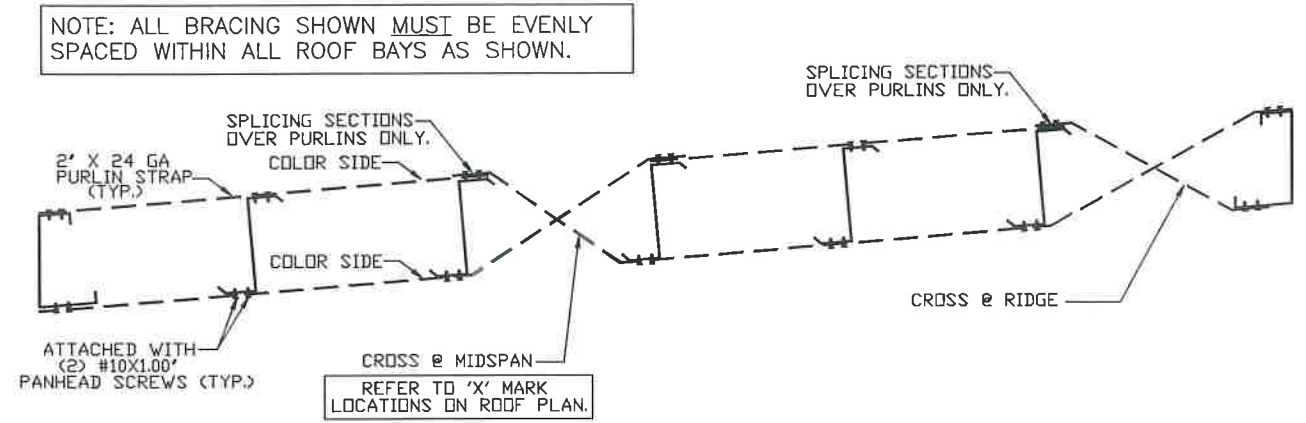
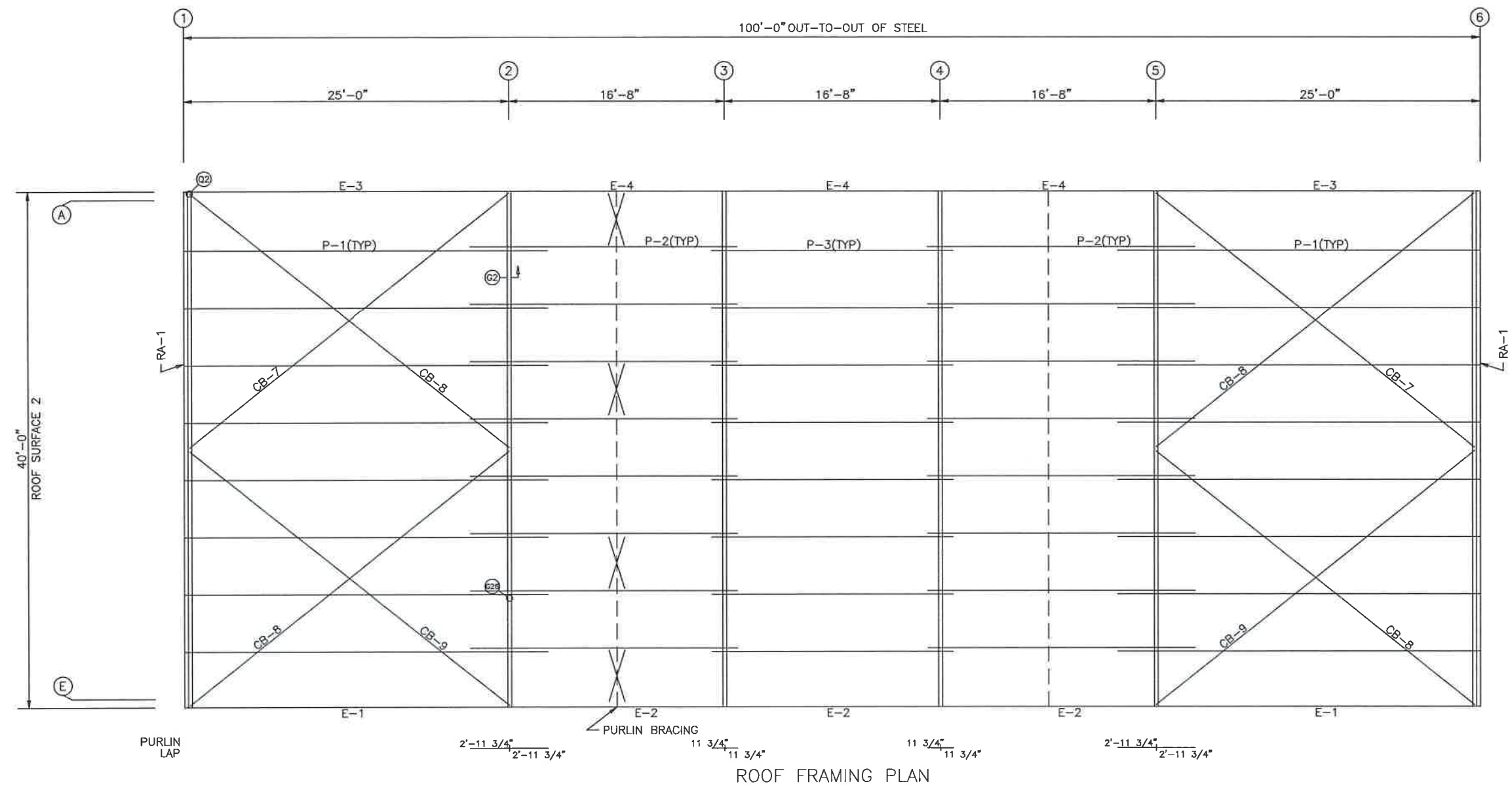
BUILDING BRACING REACTIONS

Wall Loc	Line	Col Line	± Reactions(k)				Panel_Shear (lb/ft)	
			Wind		Seismic		Wind	Seis
			Horz	Vert	Horz	Vert		
L_EW	1	A,C	4.6	4.3	0.2	0.2		
F_SW	E	1,2	2.1	1.8	0.4	0.3		
		5,6	2.1	1.8	0.4	0.3		
R_EW	6	C,A	4.6	4.3	0.2	0.2		
B_SW	A	6,5	1.8	1.0	0.4	0.2		
		2,1	1.8	1.0	0.4	0.2		

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A		DATE: 1/2/25		
LOCATION: LAKE CITY, FL. 32024				
DRAWING NAME: ANCHOR BOLT REACTIONS				
DRAWING NO: PAGE 1.2		DRAWN BY: BJC		CHECKED BY: SPW
				SCALE: NONE

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8x25Z12	27'-11 1/2"
P-2	8x25Z14	20'-7 1/2"
P-3	8x25Z16	18'-7 1/2"
E-1	8HE14@2	24'-11 1/2"
E-2	8HE14@2	16'-7 1/2"
E-3	8LE14@2	24'-11 1/2"
E-4	8LE14@2	16'-7 1/2"
CB-7	1/4 CBL	30'-10"
CB-8	1/4 CBL	31'-2"
CB-9	1/4 CBL	31'-3"



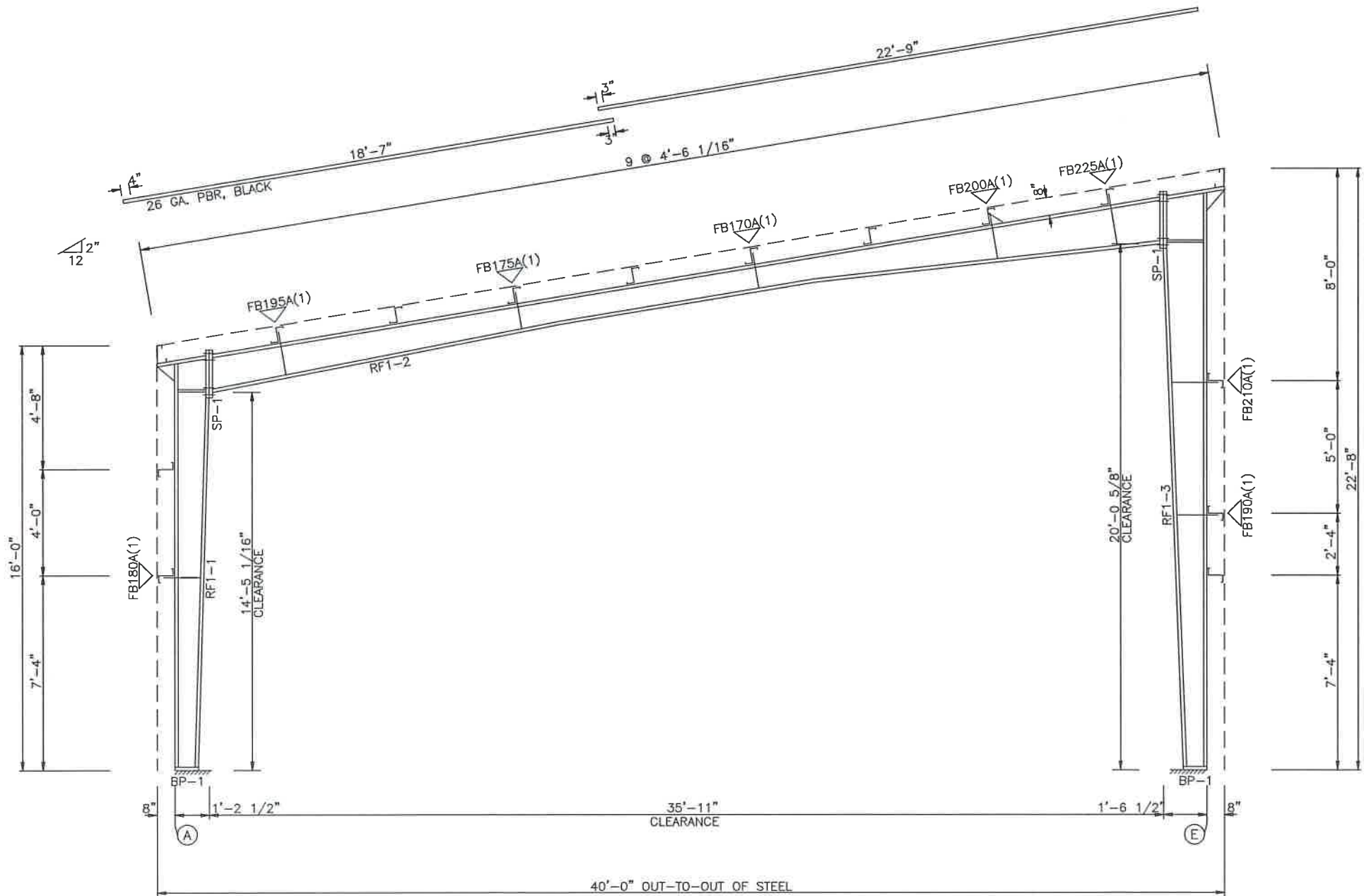
ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A		DATE: 1/2/25		
LOCATION: LAKE CITY, FL. 32024				
DRAWING NAME: ROOF FRAMING LAYOUT				
DRAWING NO: PAGE 2		DRAWN BY: BJC	CHECKED BY: SPW	SCALE: NONE

SPLICE BOLT TABLE						
MARK	Qty	Top	Bot	Int	TYPE	DIA Length
SP-1	4	4	0		A325	5/8" 2"

BASE PLATE TABLE			
COL	PLATE SIZE		
MARK	Width	THICK	Length
BP-1	6"	3/8"	8"

▽ FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A - L2x2x14

MEMBER TABLE						
MARK	Weight	Web Depth	Web THICK	PLATE Length	Outside Flange	Inside Flange
RF1-1	247	7.5/14.0	0.135	14'-0 15/16"	5 x 1/4" x 15'-4 5/8"	5 x 1/4" x 14'-1 1/16"
RF1-2	560	14.0/14.0	0.188	1'-6"	5 x 1/4" x 1'-10 1/2"	5 x 1/4" x 13'-4 3/16"
		14.0/ 9.5	0.135	13'-4 1/8"	5 x 1/4" x 20'-0"	5 x 1/4" x 9'-8 5/16"
		9.5/ 9.5	0.135	9'-8 5/16"	5 x 1/4" x 16'-3 5/8"	5 x 1/4" x 13'-2 3/4"
		9.5/18.0	0.135	13'-5 9/16"		5 x 1/4" x 19'-8 7/16"
RF1-3	353	18.0/11.2	0.135	14'-11"	5 x 1/4" x 2'-2 9/16"	
		11.2/ 7.5	0.135	6'-10 7/8"	5 x 1/4" x 2'-0"	
					5 x 1/4" x 19'-9 7/8"	



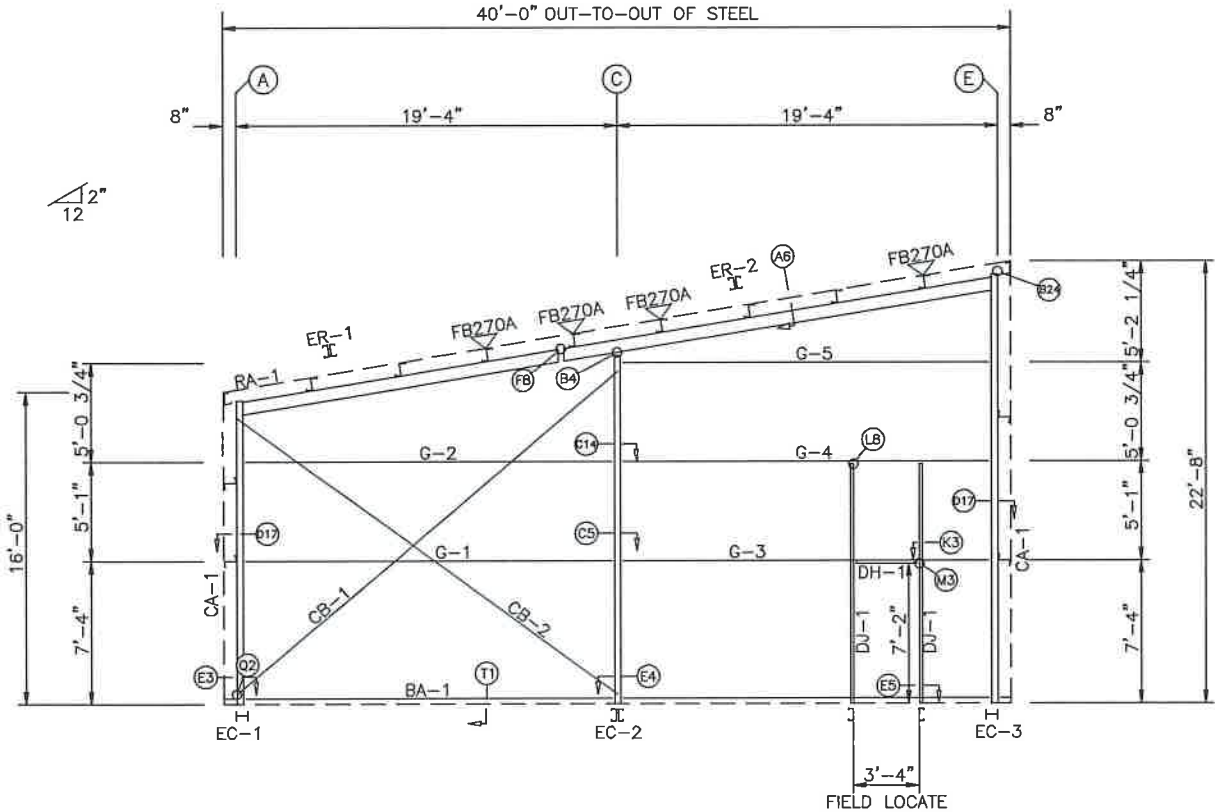
RIGID FRAME ELEVATION: FRAME LINE 2 3 4 5

ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER:				
THE WOODS CONTAINER PARK				
JOB NO:			DATE:	
8967A			1/2/25	
LOCATION:				
LAKE CITY, FL. 32024				
DRAWING NAME:				
RIGID FRAME CROSS SECTION				
DRAWING NO:		DRAWN BY:		CHECKED BY:
PAGE 2.1		BJC		SPW
				SCALE:
				NONE

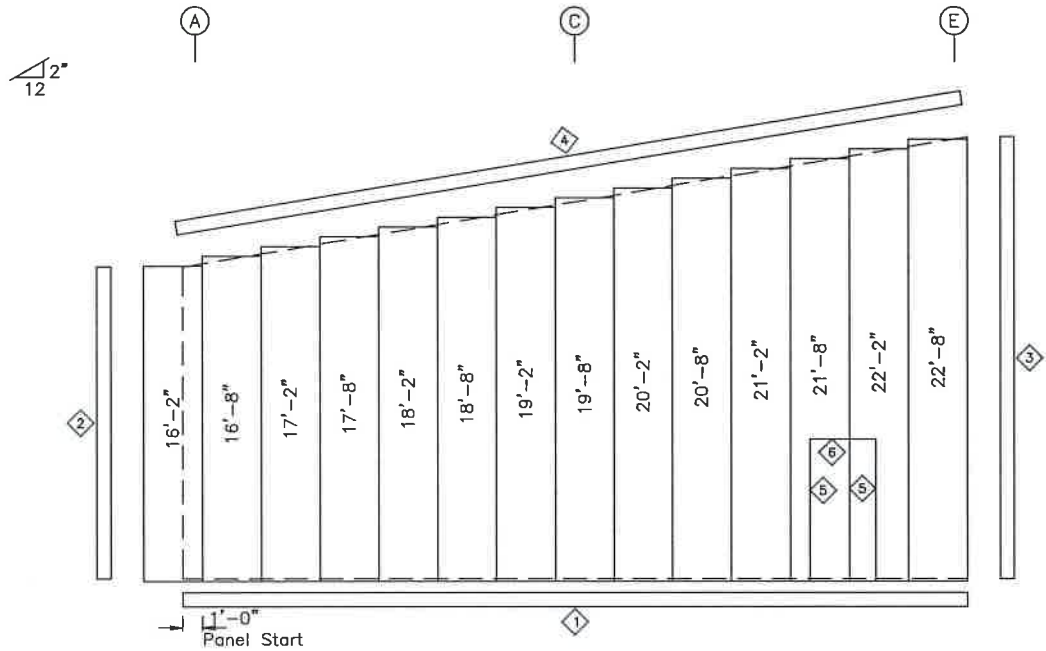
BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Cor_Column/Raf	4	A325	5/8"	2"
ER-1/ER-2	8	A325	5/8"	2"
EC-2/ER-2	2	A325	5/8"	2"

TRIM TABLE FRAME LINE 1			
ID	PART	LENGTH	DETAIL
1	DRIP BASE	13'-7"	TRIM_16
2	O/S CORN	16'-2"	TRIM_5
3	O/S CORN	11'-7"	TRIM_5
4	RAKE TRM	13'-9"	TRIM_3
5	R JAMB	7'-5"	TRIM_8
6	R HEAD	3'-7"	TRIM_61

MEMBER TABLE FRAME LINE 1		
MARK	PART	LENGTH
EC-1	W8X10	15'-6 9/16"
EC-2	8X7DC12	17'-9 3/4"
EC-3	W8X10	21'-10 9/16"
ER-1	8X7DC12	16'-1 7/8"
ER-2	8X7DC12	21'-9 7/8"
DJ-1	8X2516	12'-5"
DH-1	8X25C16	3'-4"
G-1	8x25Z12	18'-3 5/8"
G-2	8x25C12	18'-3 5/8"
G-3	8x25Z16	18'-3 5/8"
G-4	8x35C12	18'-3 5/8"
G-5	8x25Z14	18'-3 5/8"
CB-1	5/16 CBL	25'-9"
CB-2	5/16 CBL	23'-11"



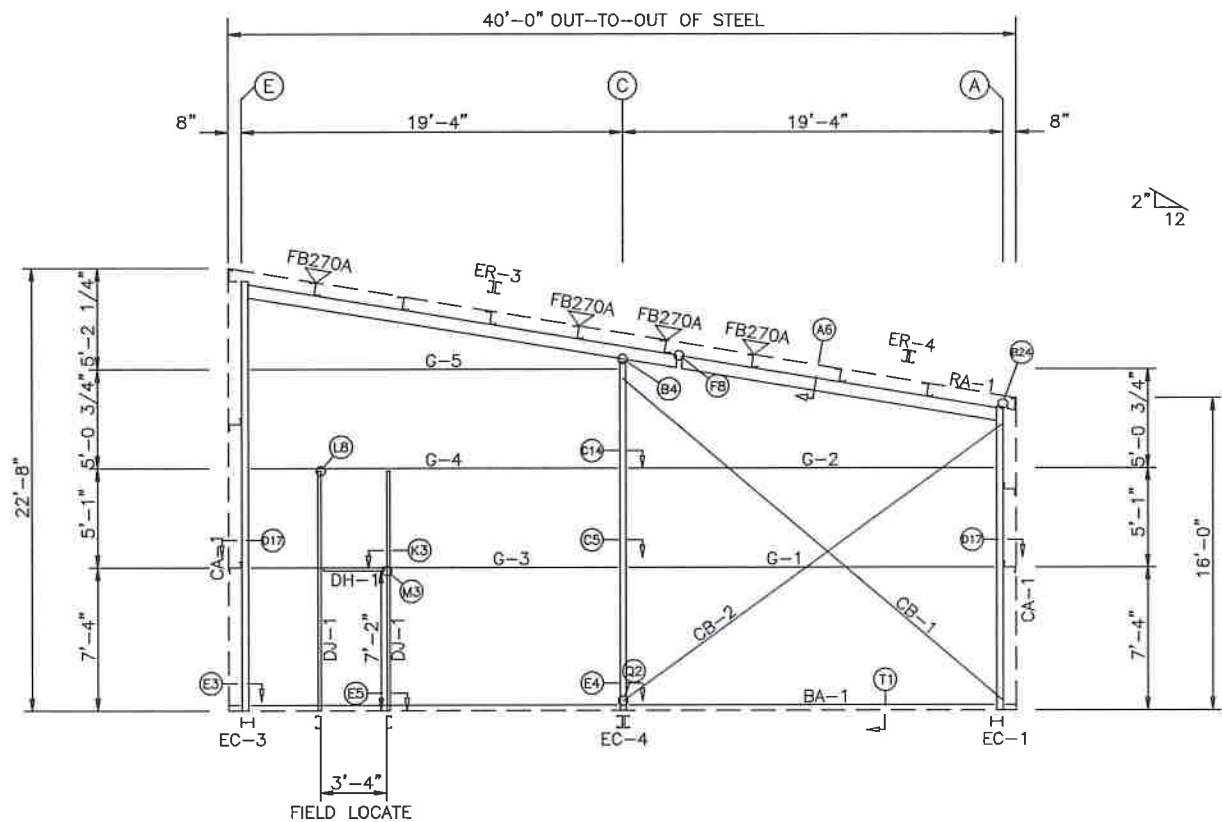
ENDWALL FRAMING: FRAME LINE 1



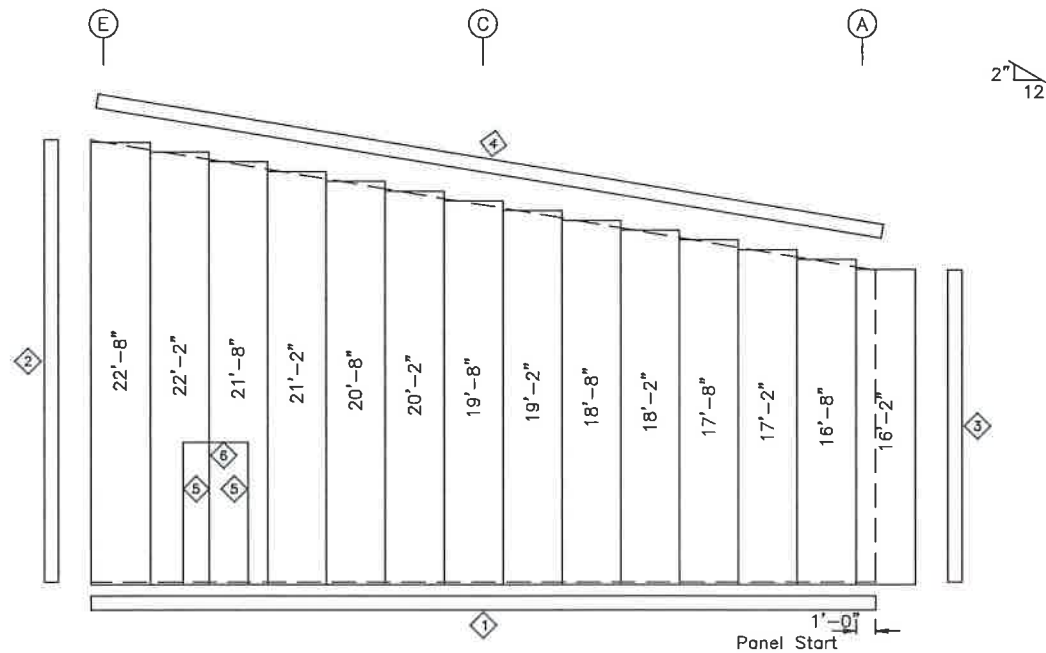
ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - BLACK

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

ISSUE	DET	CHK	DATE
BUILDINGS AND MORE			
CUSTOMER:			
THE WOODS CONTAINER PARK			
JOB NO:	8967A	DATE:	1/2/25
LOCATION:			
LAKE CITY, FL. 32024			
DRAWING NAME:			
ENDWALL FRAMING & SHEETING LAYOUT			
DRAWING NO:	PAGE 3	DRAWN BY:	BJC
CHECKED BY:	SPW	SCALE:	NONE



ENDWALL FRAMING: FRAME LINE 6



ENDWALL SHEETING & TRIM: FRAME LINE 6
PANELS: 26 GA. PBR - BLACK

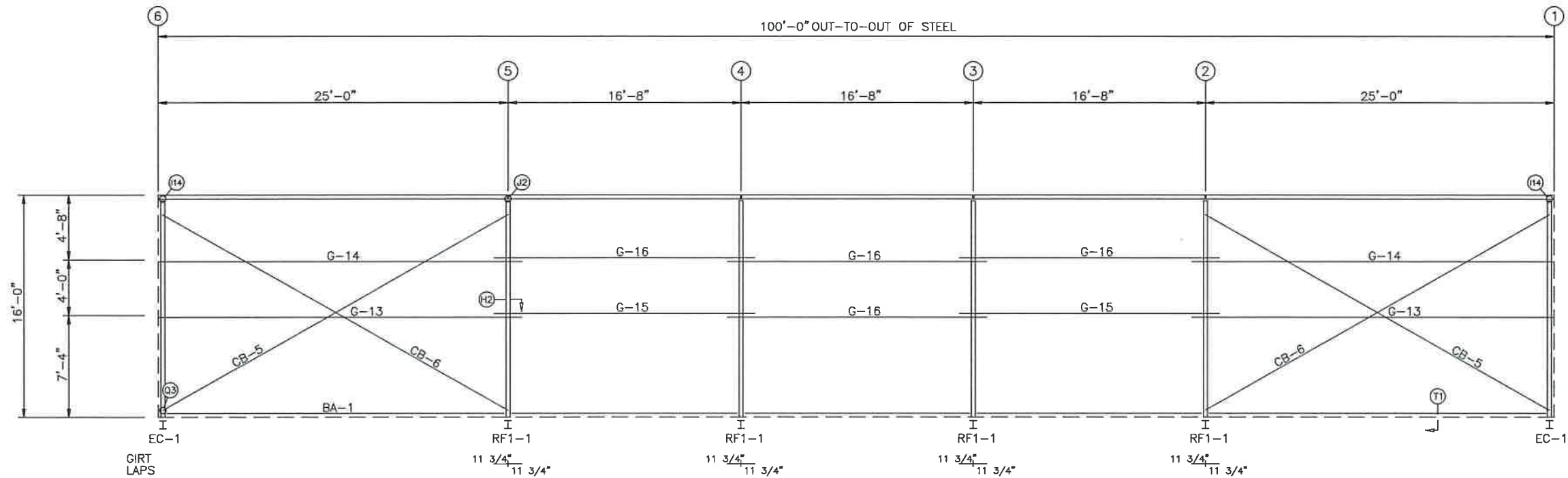
BOLT TABLE FRAME LINE 6				
LOCATION	QUAN	TYPE	DIA	LENGTH
Cor_Column/Raf	4	A325	5/8"	2"
ER-3/ER-4	8	A325	5/8"	2"
EC-4/ER-3	2	A325	5/8"	2"

TRIM TABLE FRAME LINE 6			
ID	PART	LENGTH	DETAIL
1	DRIP BASE	13'-7"	TRIM_16
2	O/S CORN	11'-7"	TRIM_5
3	O/S CORN	16'-2"	TRIM_5
4	RAKE TRM	13'-9"	TRIM_3
5	R JAMB	7'-5"	TRIM_8
6	R HEAD	3'-7"	TRIM_61

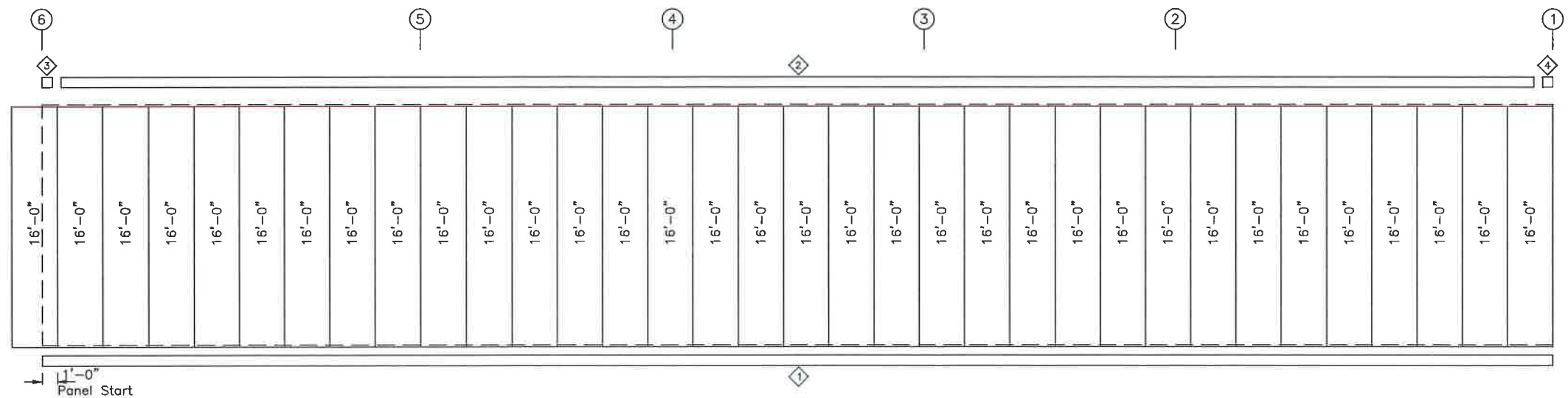
MEMBER TABLE FRAME LINE 6		
MARK	PART	LENGTH
EC-1	W8X10	15'-6 9/16"
EC-3	W8X10	21'-10 9/16"
EC-4	8X7DC12	17'-9 3/4"
ER-3	8X7DC12	21'-9 7/8"
ER-4	8X7DC12	16'-1 7/8"
DJ-1	8X25C16	12'-5"
DH-1	8X25C16	3'-4"
G-1	8x25Z12	18'-3 5/8"
G-2	8x25C12	18'-3 5/8"
G-3	8x25Z16	18'-3 5/8"
G-4	8x35C12	18'-3 5/8"
G-5	8x25Z14	18'-3 5/8"
CB-1	5/16 CBL	25'-9"
CB-2	5/16 CBL	23'-11"

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A			DATE: 1/2/25	
LOCATION: LAKE CITY, FL 32024				
DRAWING NAME: ENDWALL FRAMING & SHEETING LAYOUT				
DRAWING NO: PAGE 3.1		DRAWN BY: BJC	CHECKED BY: SPW	SCALE: NONE



SIDEWALL FRAMING: FRAME LINE A

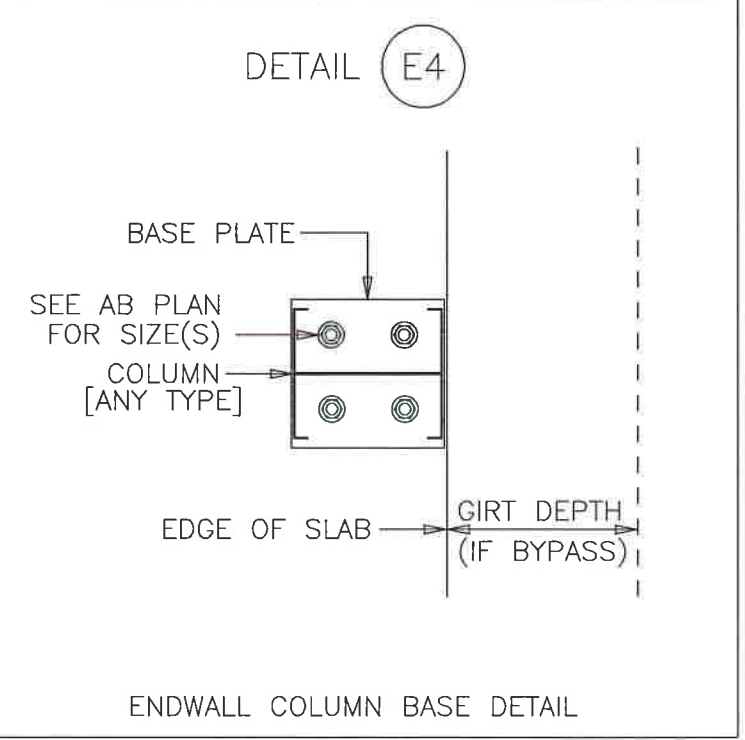
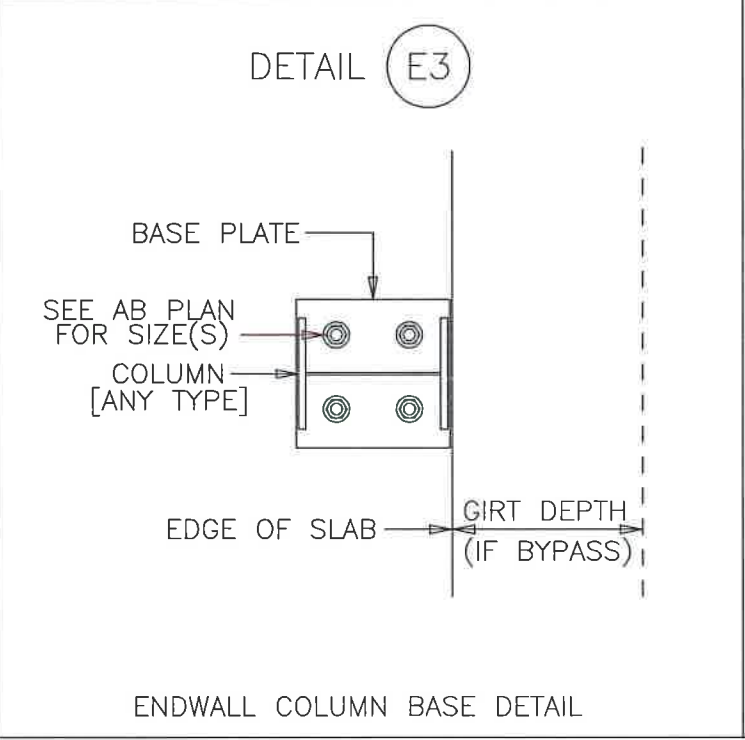
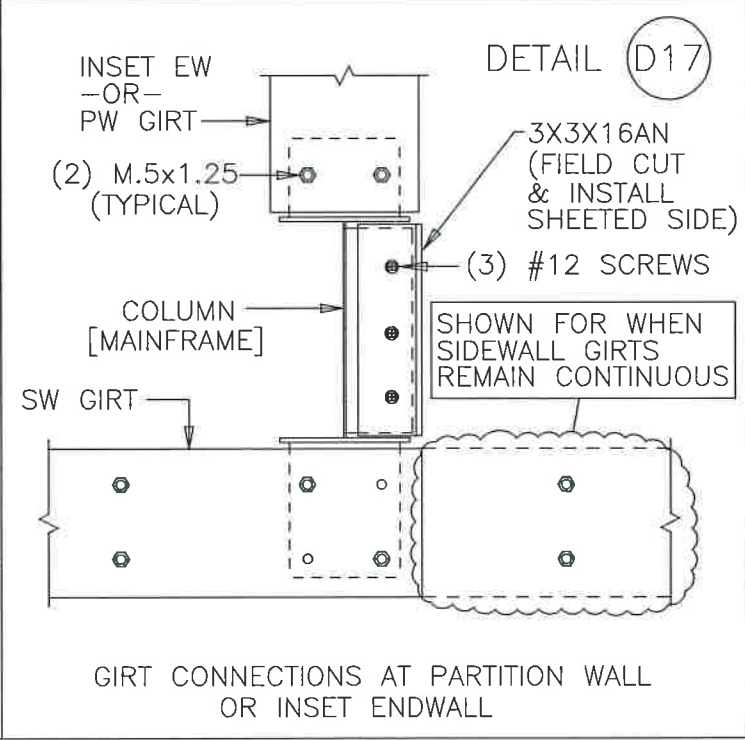
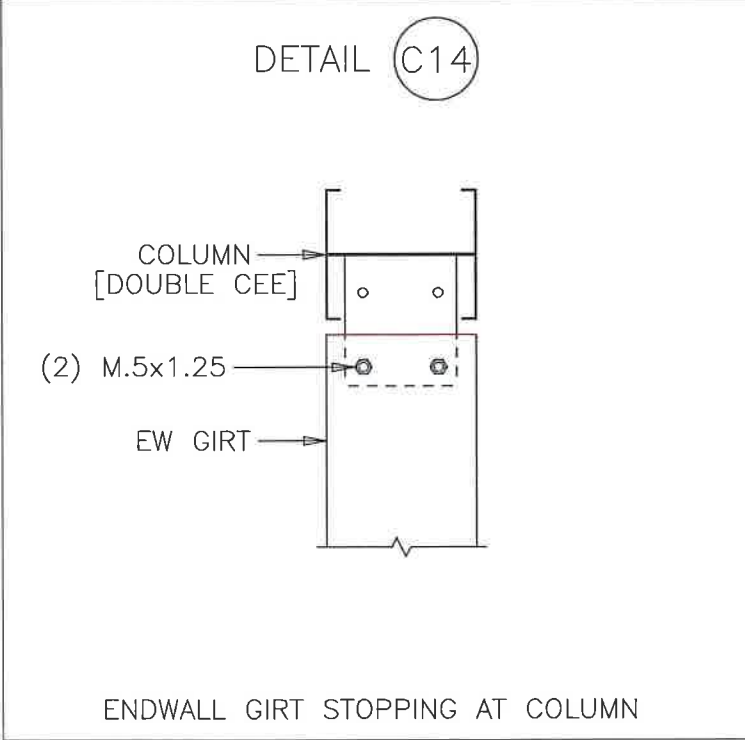
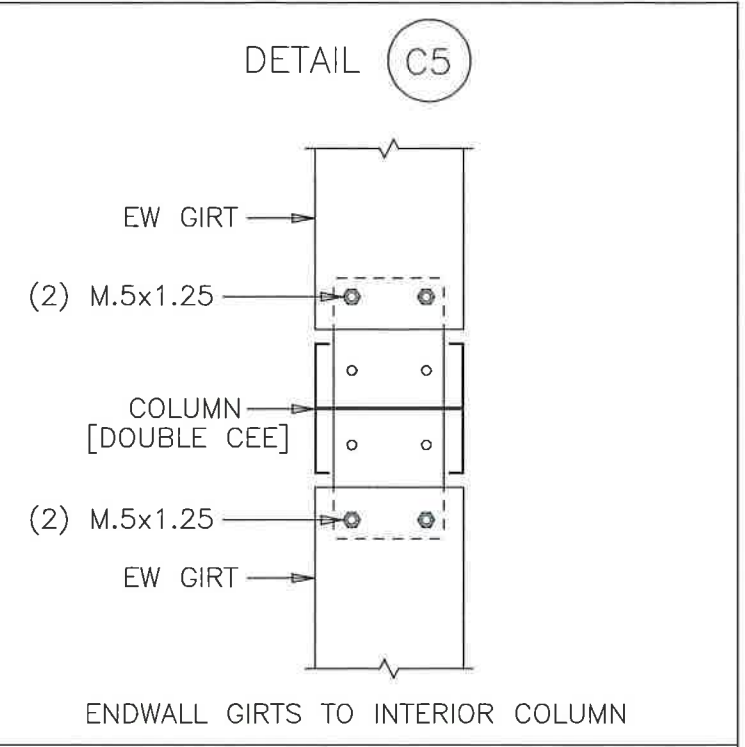
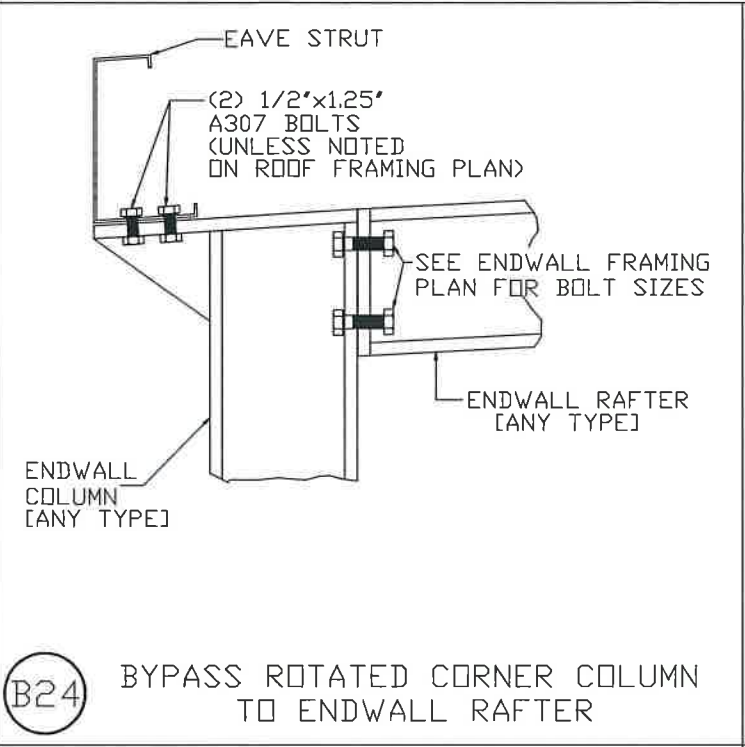
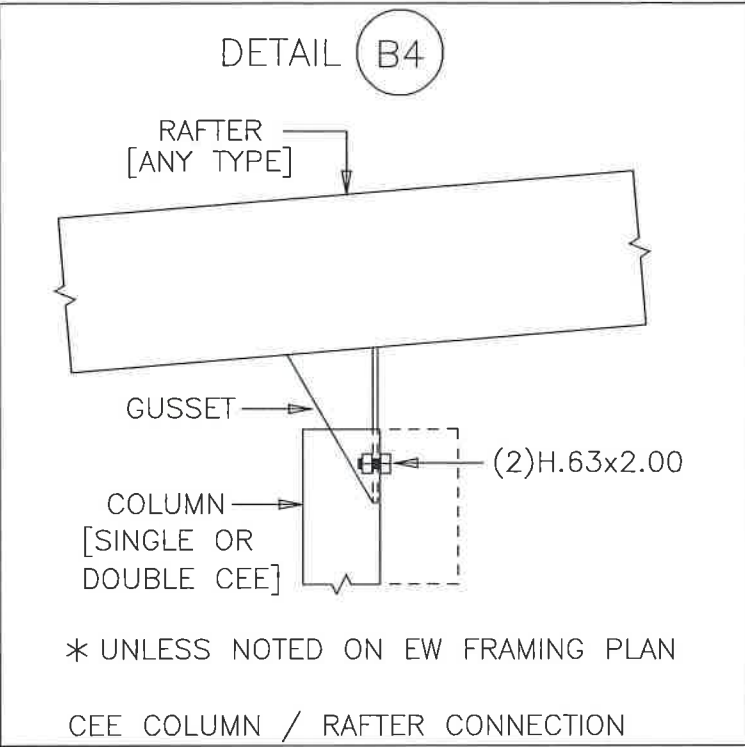
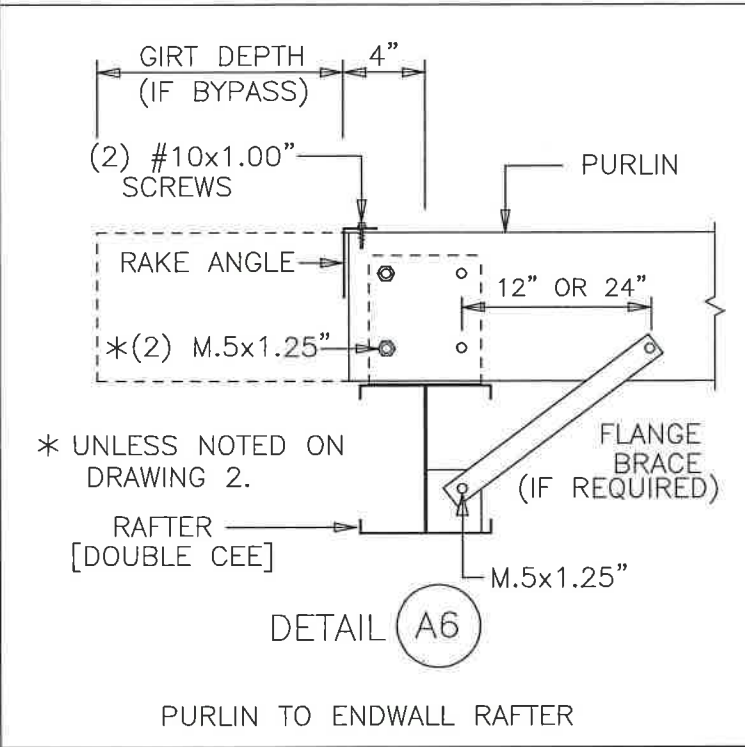


SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 GA. PBR - BLACK

TRIM TABLE FRAME LINE A			
ID	PART	LENGTH	DETAIL
1	DRIP BASE	16'-11"	TRIM_16
2	EAVE TRM	20'-3"	TRIM_12
3	R END LH	6"	TRIM_13
4	R END RH	6"	TRIM_13

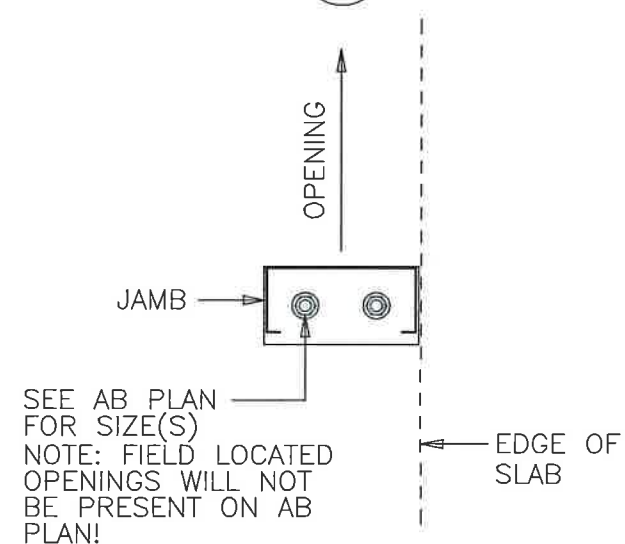
MEMBER TABLE FRAME LINE A		
MARK	PART	LENGTH
G-13	8x25Z12	25'-11 1/2"
G-14	8x25Z16	25'-11 1/2"
G-15	8x25Z14	18'-7 1/2"
G-16	8x25Z16	18'-7 1/2"
CB-5	0.50_ROD	29'-4"
CB-6	0.50_ROD	29'-3"

ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A		DATE: 1/2/25		
LOCATION: LAKE CITY, FL. 32024				
DRAWING NAME: SIDEWALL FRAMING & SHEETING LAYOUT				
DRAWING NO: PAGE 4	DRAWN BY: BJC	CHECKED BY: SPW	SCALE: NONE	

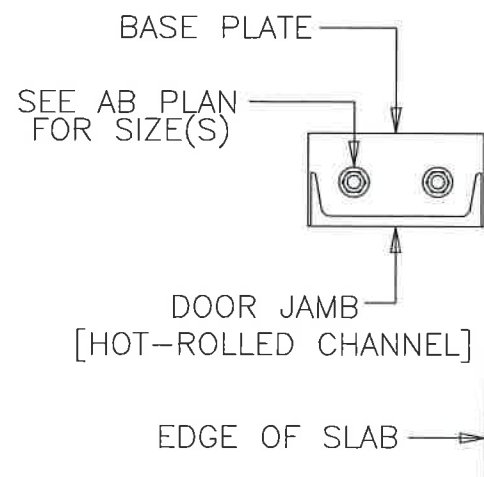


ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A		DATE: 1/2/25		
LOCATION: LAKE CITY, FL. 32024				
DRAWING NAME: FRAMING DETAILS				
DRAWING NO: PAGE 5		DRAWN BY: BJC	CHECKED BY: SPW	SCALE: NONE

DETAIL (E5)

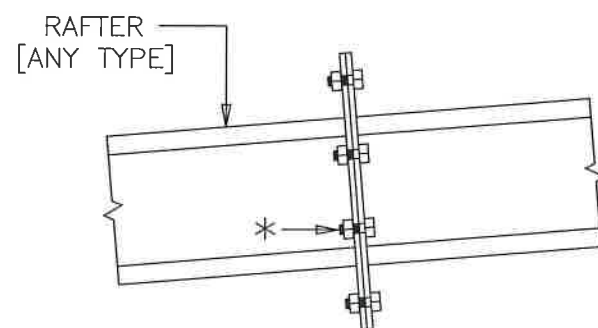


FRAMED OPENING JAMB BASE DETAIL



BASE PLATE DETAIL FOR
E11 HOT-ROLLED CHANNEL USED
AS JAMBS

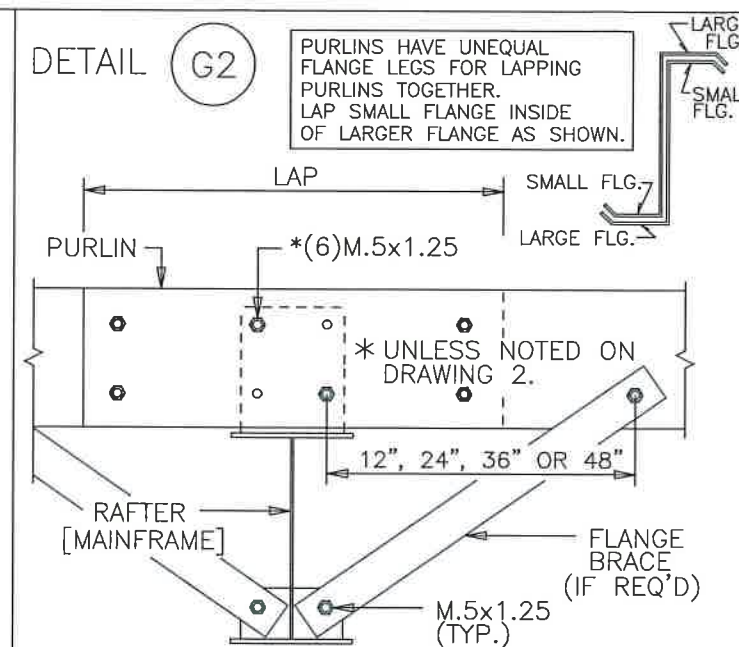
DETAIL (F8)



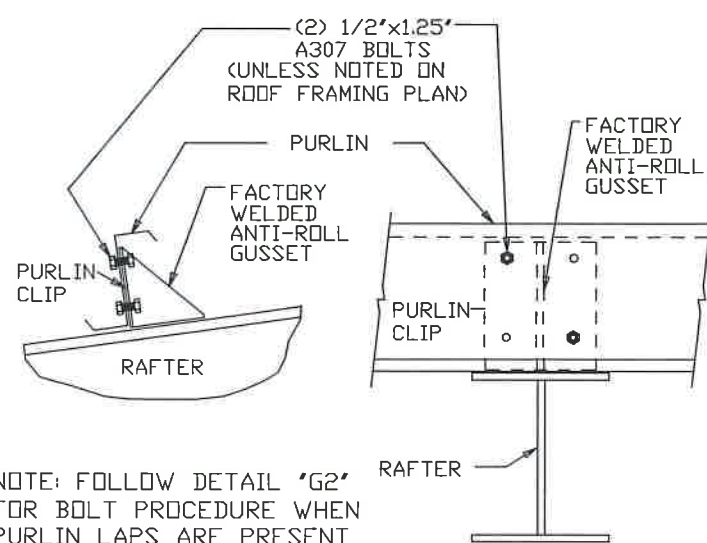
* SEE EW FRAMING PLAN FOR SIZE
AND QUANTITY OF BOLTS.

RAFTER DETAIL AT SPLICE

DETAIL (G2)

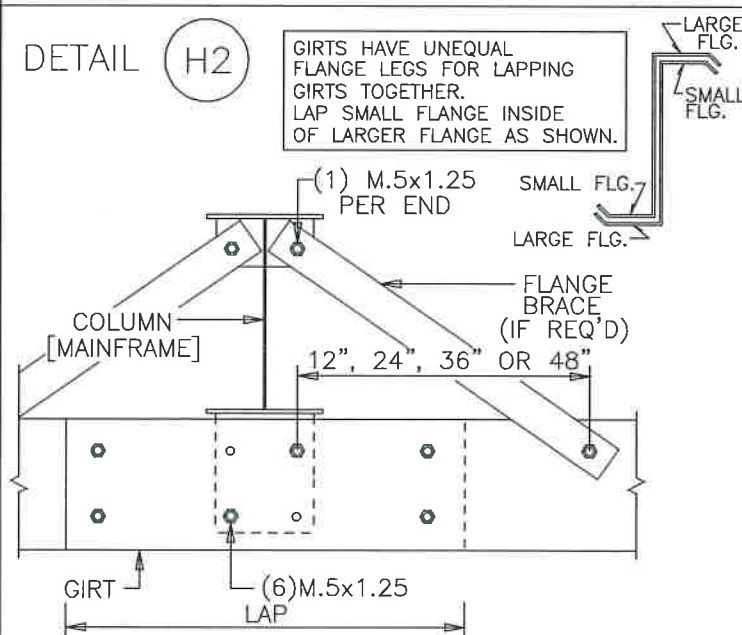


PURLIN TO MAINFRAME RAFTER

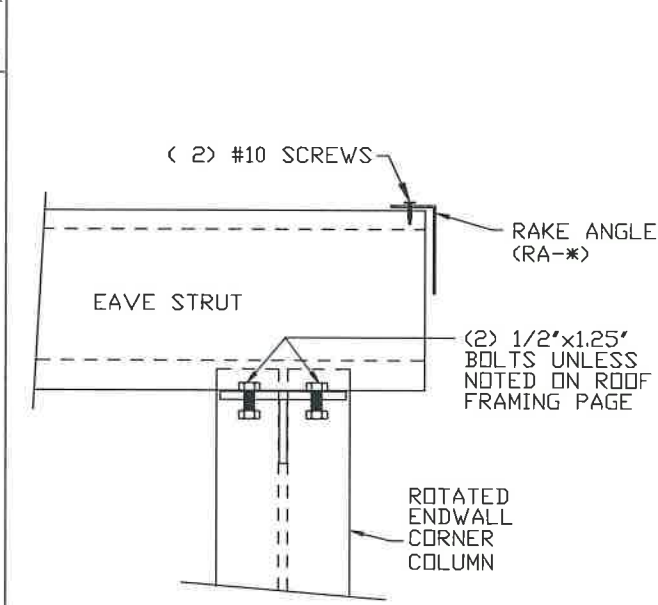


G26 WELDED ANTI-ROLL CLIP

DETAIL (H2)

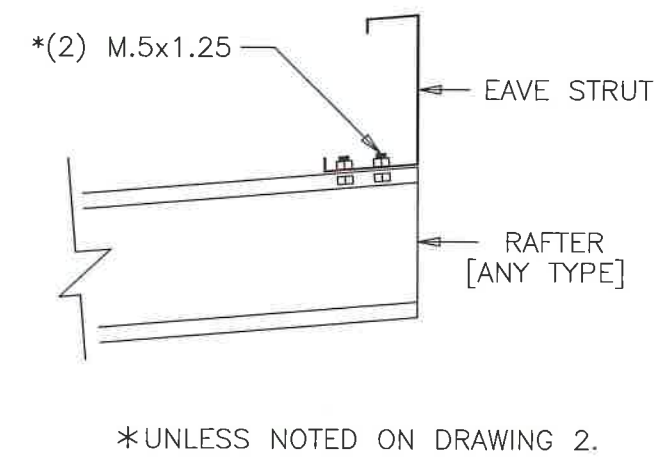


GIRT TO MAINFRAME COLUMN



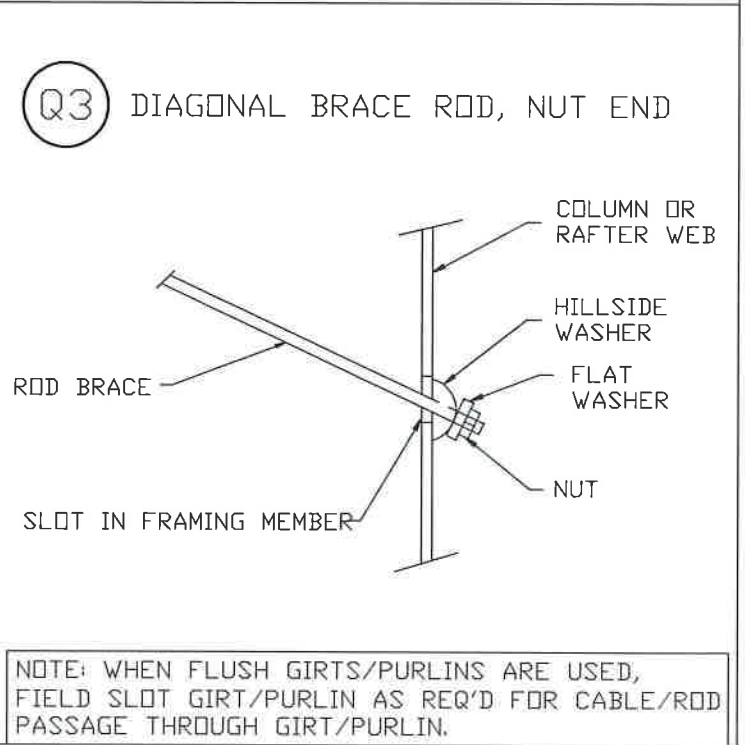
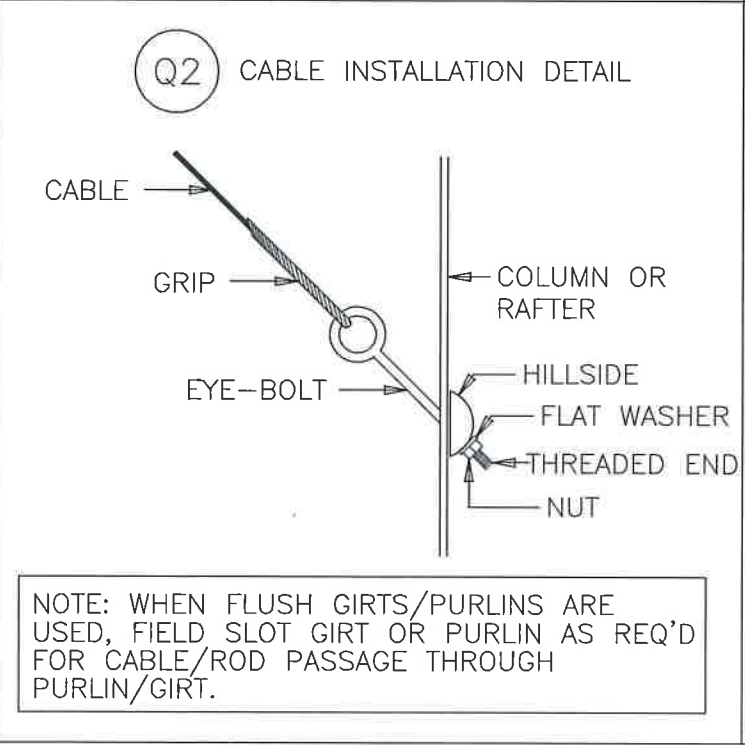
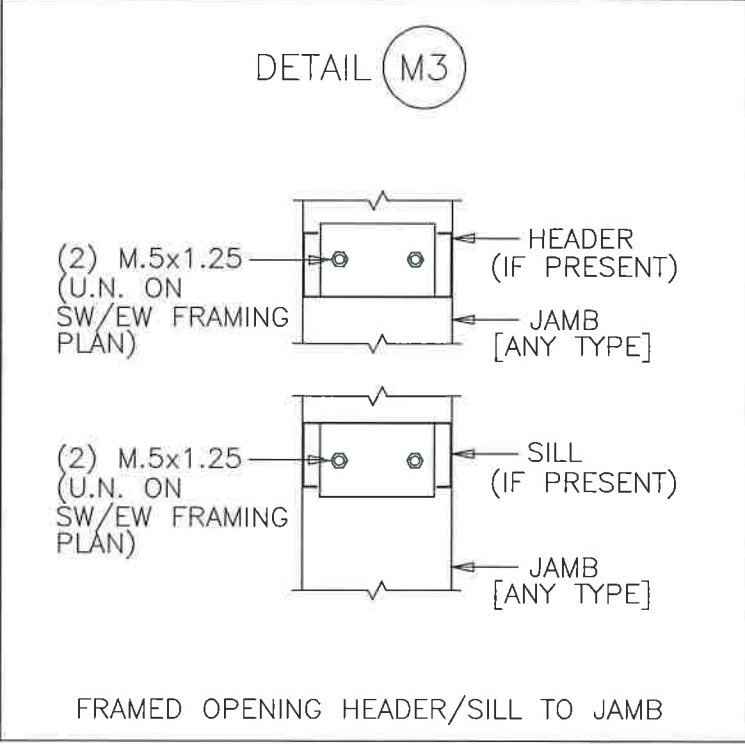
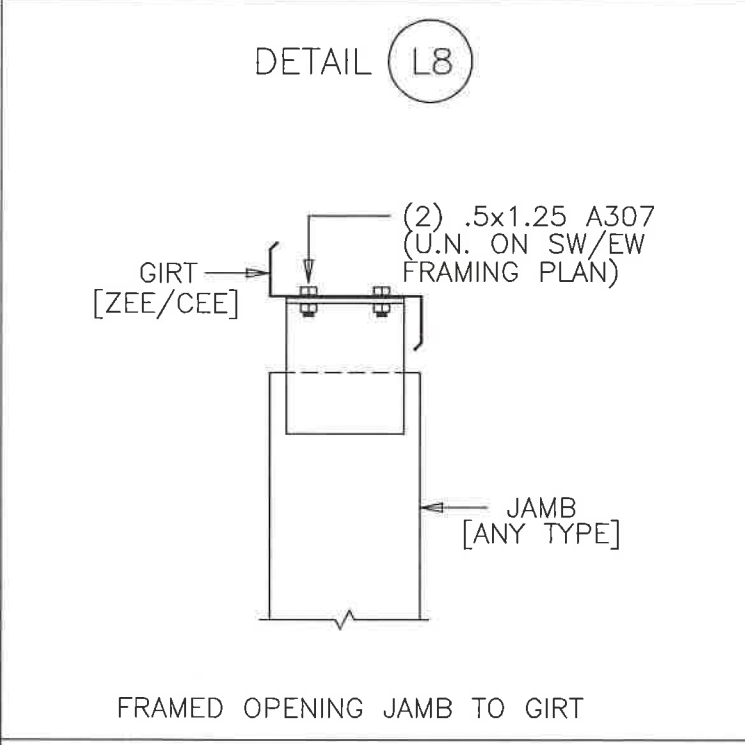
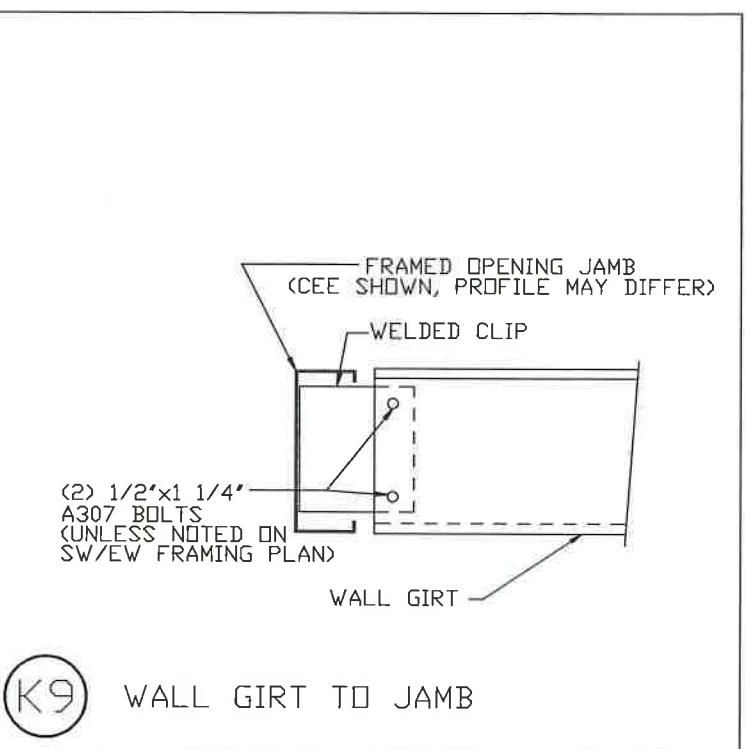
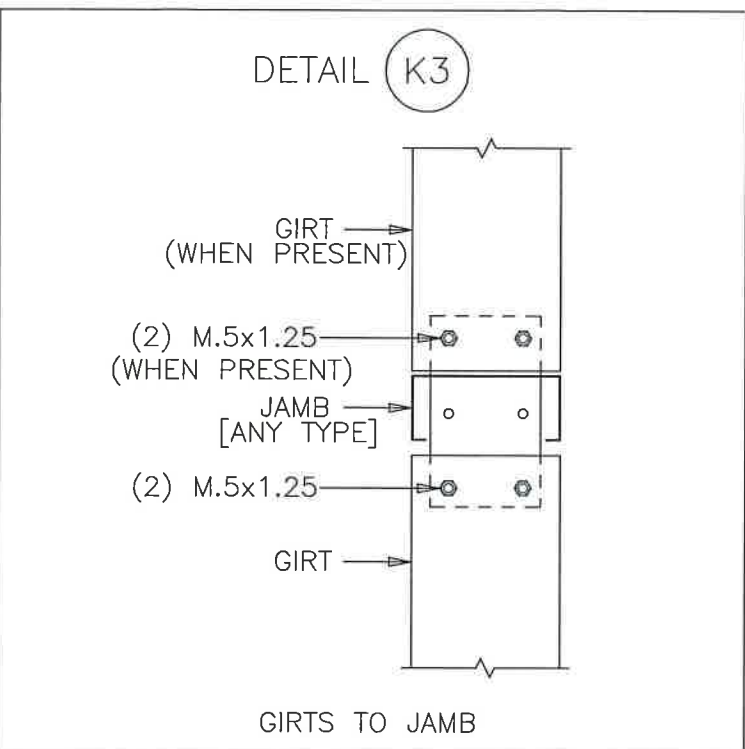
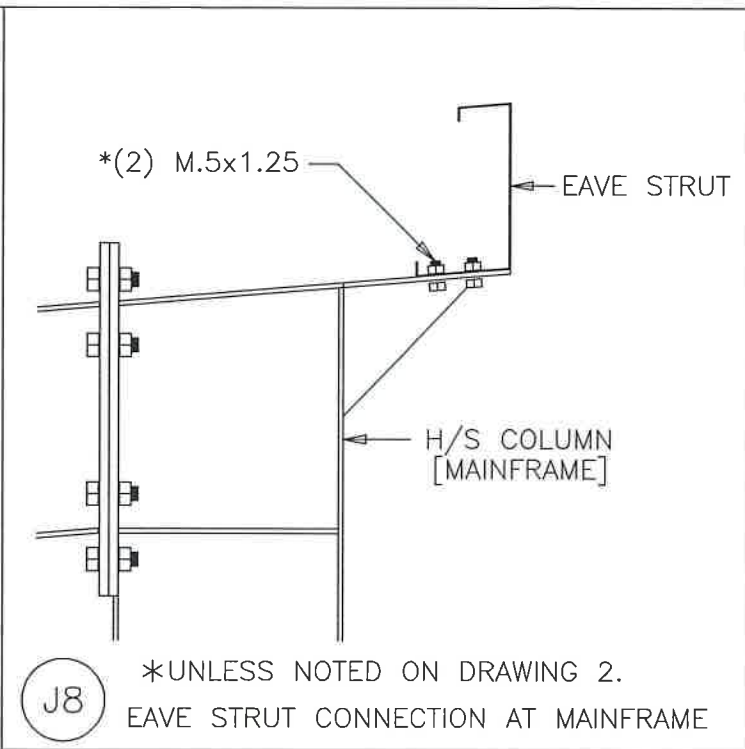
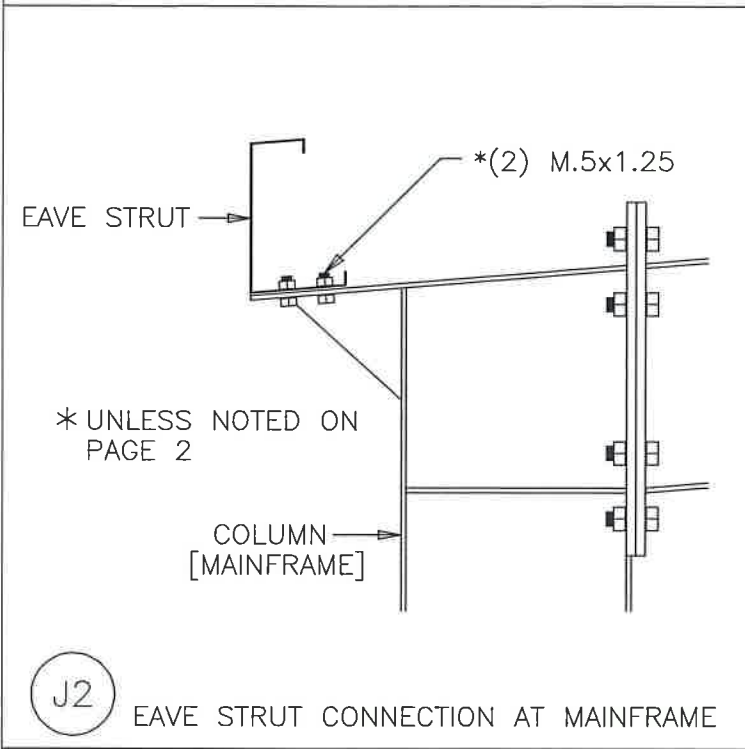
I14 EAVE STRUT TO ENDWALL
CORNER COLUMN

DETAIL (I18)

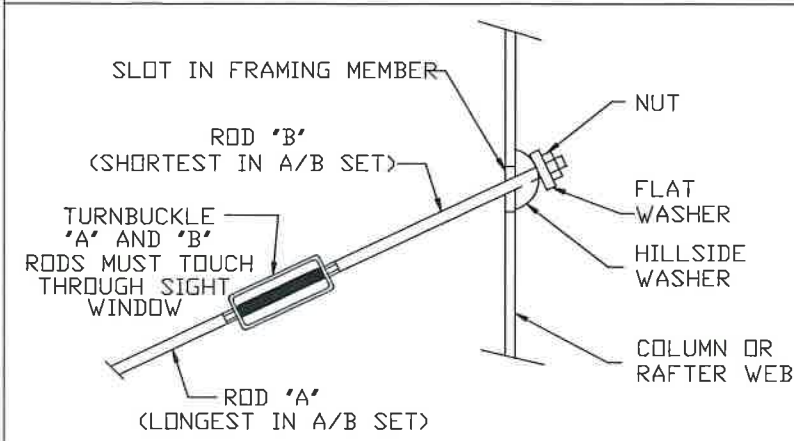


EAVE STRUT CONNECTION AT ENDWALL

ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A		DATE: 1/2/25		
LOCATION: LAKE CITY, FL. 32024				
DRAWING NAME: FRAMING DETAILS				
DRAWING NO: PAGE 5 1		DRAWN BY: BJC	CHECKED BY: SPW	SCALE: NON

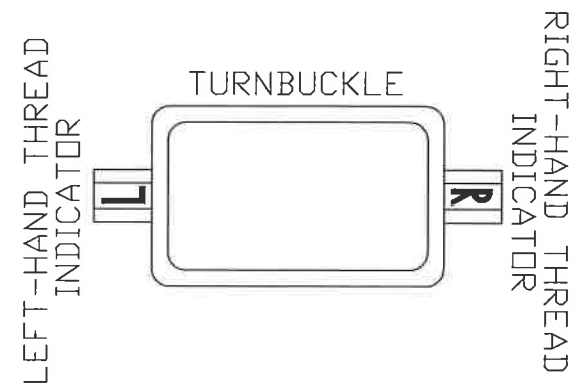


ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A		DATE: 1/2/25		
LOCATION: LAKE CITY, FL. 32024				
DRAWING NAME: FRAMING DETAILS				
DRAWING NO: PAGE 5.2		DRAWN BY: BJC	CHECKED BY: SPW	SCALE: NONE



TURNBUCKLE LOCATIONS:
CLOSEST TO EAVE OF BUILDING @ SIDEWALLS
CLOSEST TO RAKE OF BUILDING @ ENDWALLS
CLOSEST TO BUILDING PEAK/HIGH EAVE @ ROOF

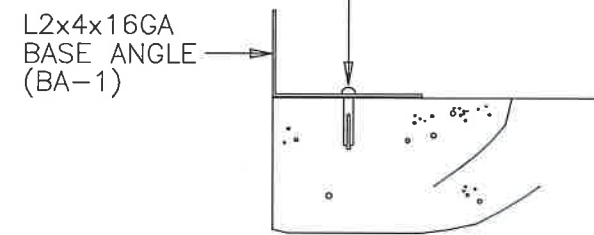
NOTE: WHEN FLUSH GIRTS/PURLINS ARE USED,
FIELD SLOT GIRT/PURLIN AS REQ'D FOR CABLE/ROD
PASSAGE THROUGH GIRT/PURLIN.



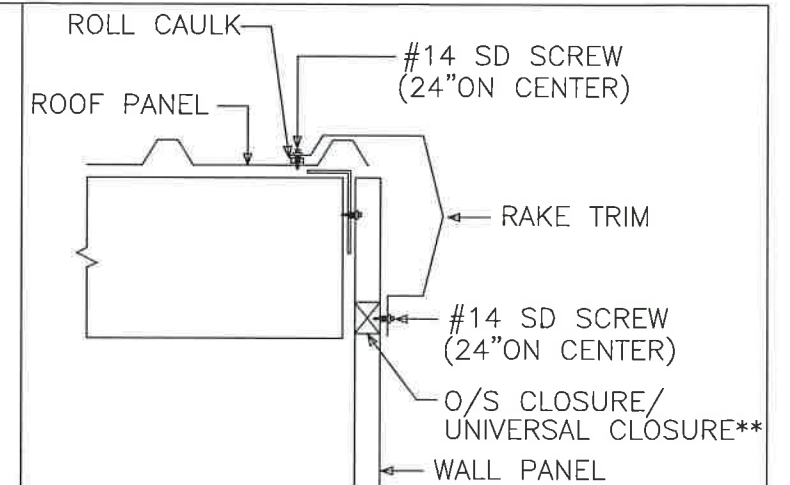
"A" RODS SENT WITH RH/RH THREADS
"B" RODS SENT WITH LH/RH THREADS
NOTE: IT IS THE ERECTOR'S RESPONSIBILITY TO
DETERMINE LEFT/RIGHT THREADS IN THE FIELD.
THREADED ENDS OF RODS ARE NOT MARKED!

DETAIL T1

1/4" x 1 1/4" ZINC HAMMER DRIVES
ZAMAK ALLOY (ASTM B633, SC1, TYPE III)
(24" ON CENTER)



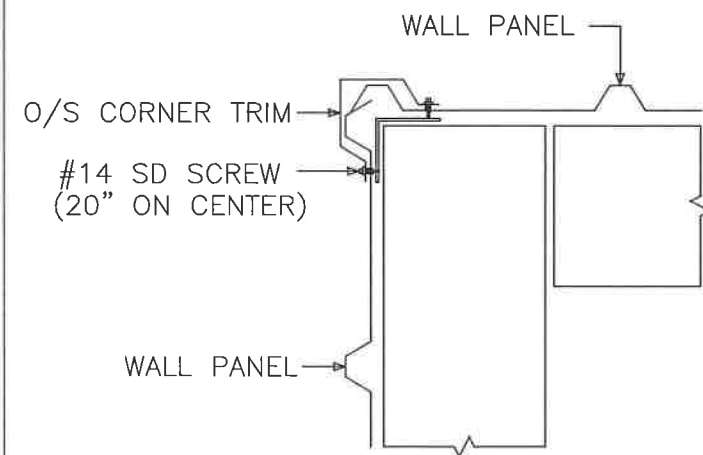
BASE ANGLE DETAIL



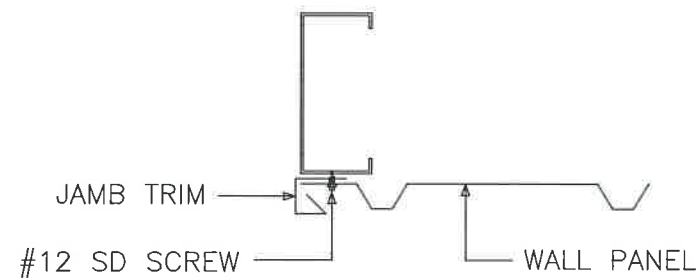
** - O/S CLOSURE FOR ROOF PITCHES UP TO 2:12"
UNIVERSAL CLOSURE FOR ROOF PITCHES ABOVE 2:12"

NOTE: FIELD CUT UNIVERSAL CLOSURES TO MATCH WALL
PANEL PROFILE (ON PITCH).

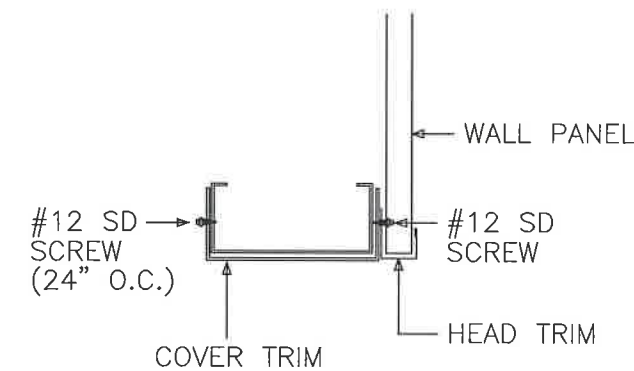
TRIM_3
RAKE TRIM DETAIL



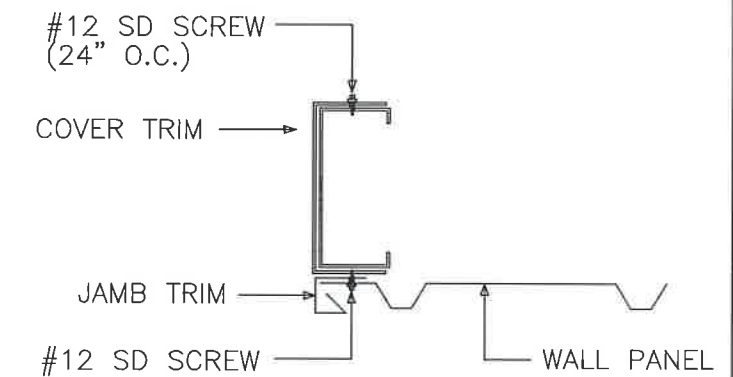
TRIM_5
O/S CORNER DETAIL



TRIM_8
JAMB TRIM DETAIL AT JAMB

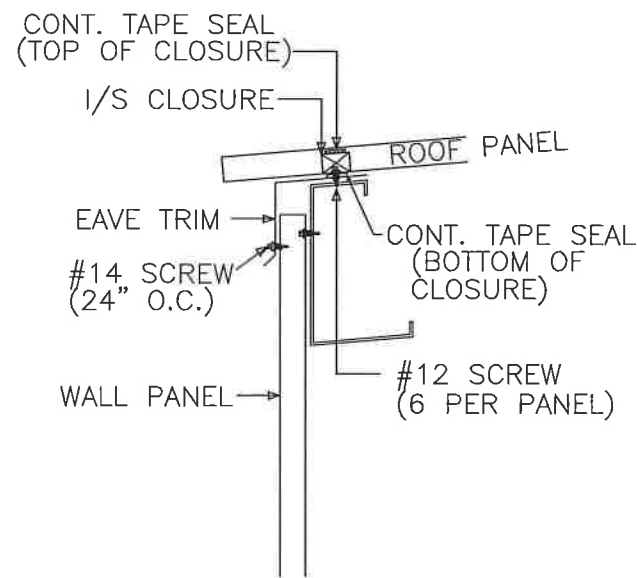


TRIM_10
COVER TRIM DETAIL AT HEADER



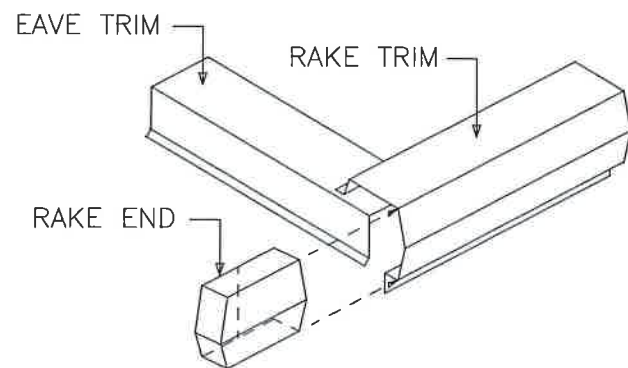
TRIM_11
COVER TRIM DETAIL AT JAMB

ISSUE	DET	CHK	DATE
BUILDINGS AND MORE			
CUSTOMER:			
THE WOODS CONTAINER PARK			
JOB NO:	8967A	DATE:	1/2/25
LOCATION:			
LAKE CITY, FL 32024			
DRAWING NAME:			
FRAMING DETAILS			
DRAWING NO:	PAGE 5.3	DRAWN BY:	BJC
CHECKED BY:	SPW	SCALE:	NONE



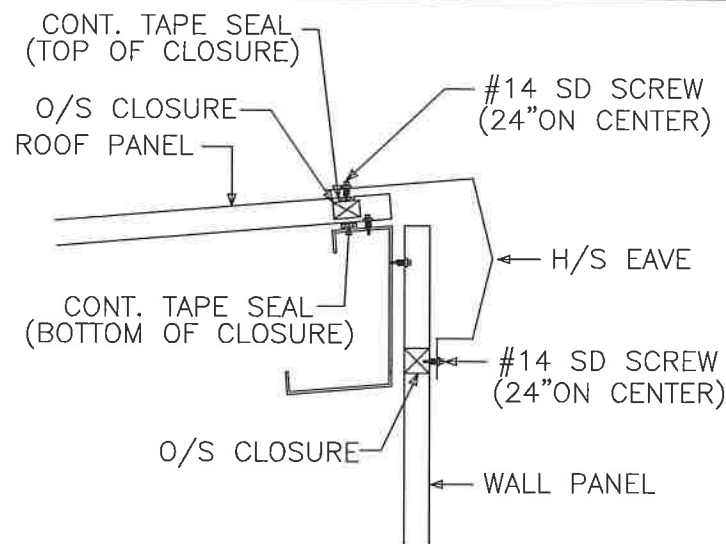
TRIM_12

EAVE TRIM DETAIL



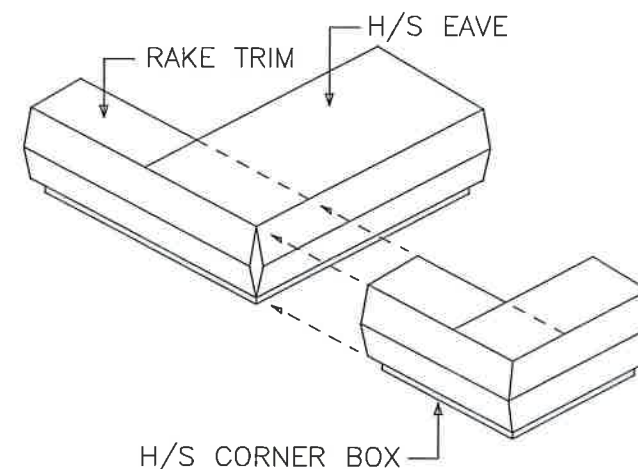
TRIM_13

RAKE END DETAIL



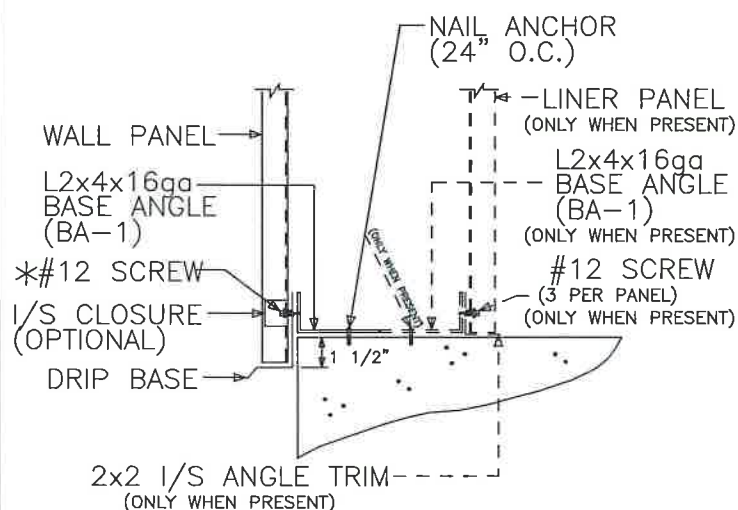
TRIM_14

H/S EAVE DETAIL



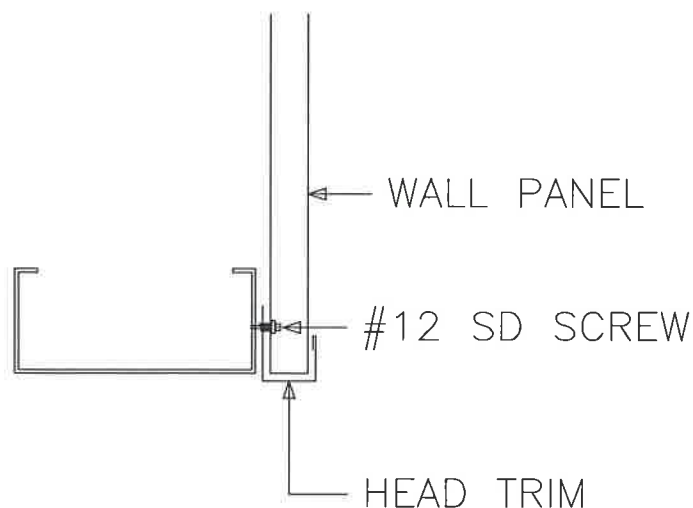
TRIM_15

H/S CORNER BOX DETAIL



*= 6 PER PANEL FOR STANDARD PBR
3 PER PANEL FOR REV. ROLLED PBR

TRIM_16 BASE TRIM DETAIL



TRIM_61

HEAD TRIM DETAIL AT HEADER

STRUCTURAL BOLTED CONNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK.
- [2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- [3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- [4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
- [5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

MORTISE PREPPED PERSONNEL DOORS

ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

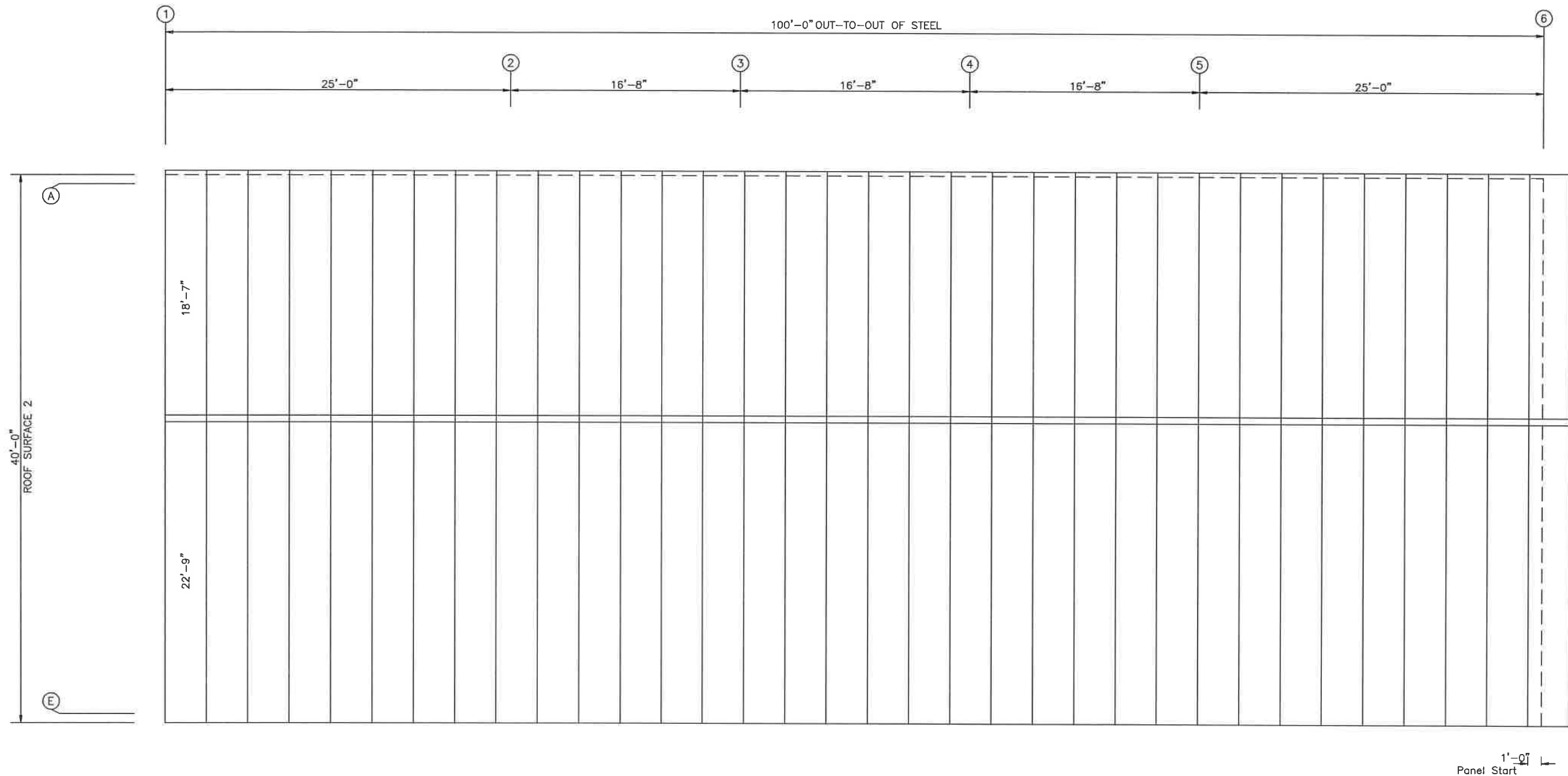
(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RESPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.

BUILT-UP MEMBER LEGEND

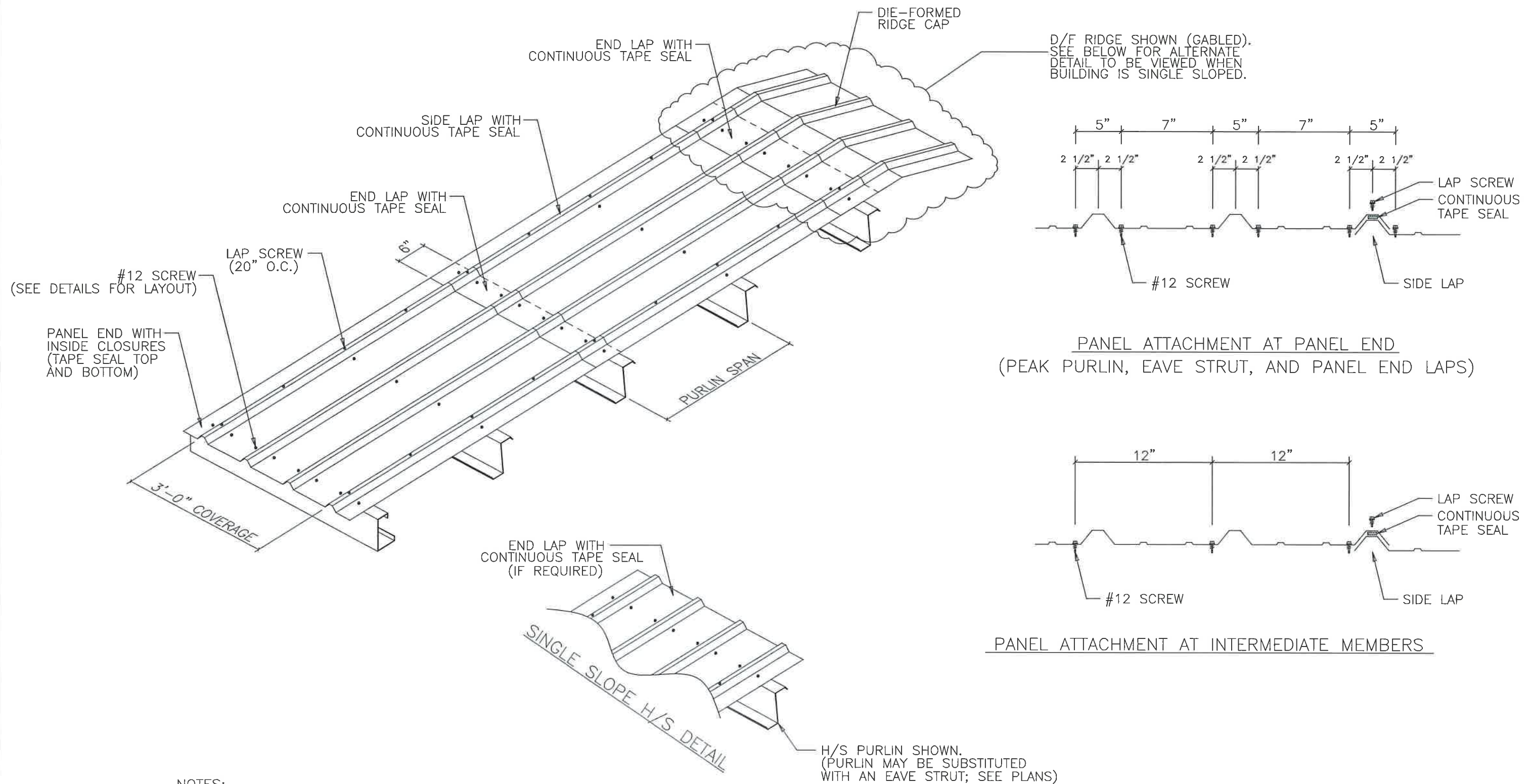
BEAM TYPE	BEAM DEPTH	FLANGE WIDTH	FLANGE THK.	WEB THK.
B08541				
B= BUILT-UP	08= 8" 10= 10" 12= 12" 14= 14" ETC.	5, 6, 8, 10 OR 12 (INCHES)	MEASURED IN 16ths. (4= 1/4" 5= 5/16" ETC.)	1= 10ga 3= 3/16" ETC.

ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A		DATE: 1/2/25		
LOCATION: LAKE CITY, FL. 32024				
DRAWING NAME: FRAMING DETAILS				
DRAWING NO: PAGE 5.4	DRAWN BY: BJC	CHECKED BY: SPW	SCALE: NONE	



ROOF SHEETING PLAN
PANELS: 26 GA. PBR - BLACK

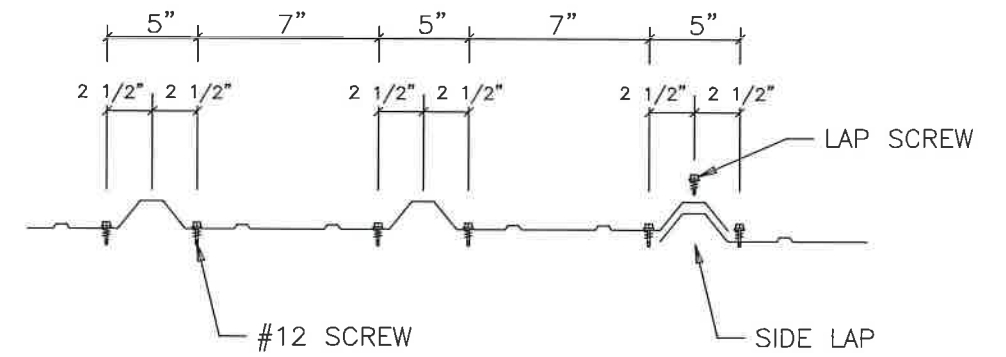
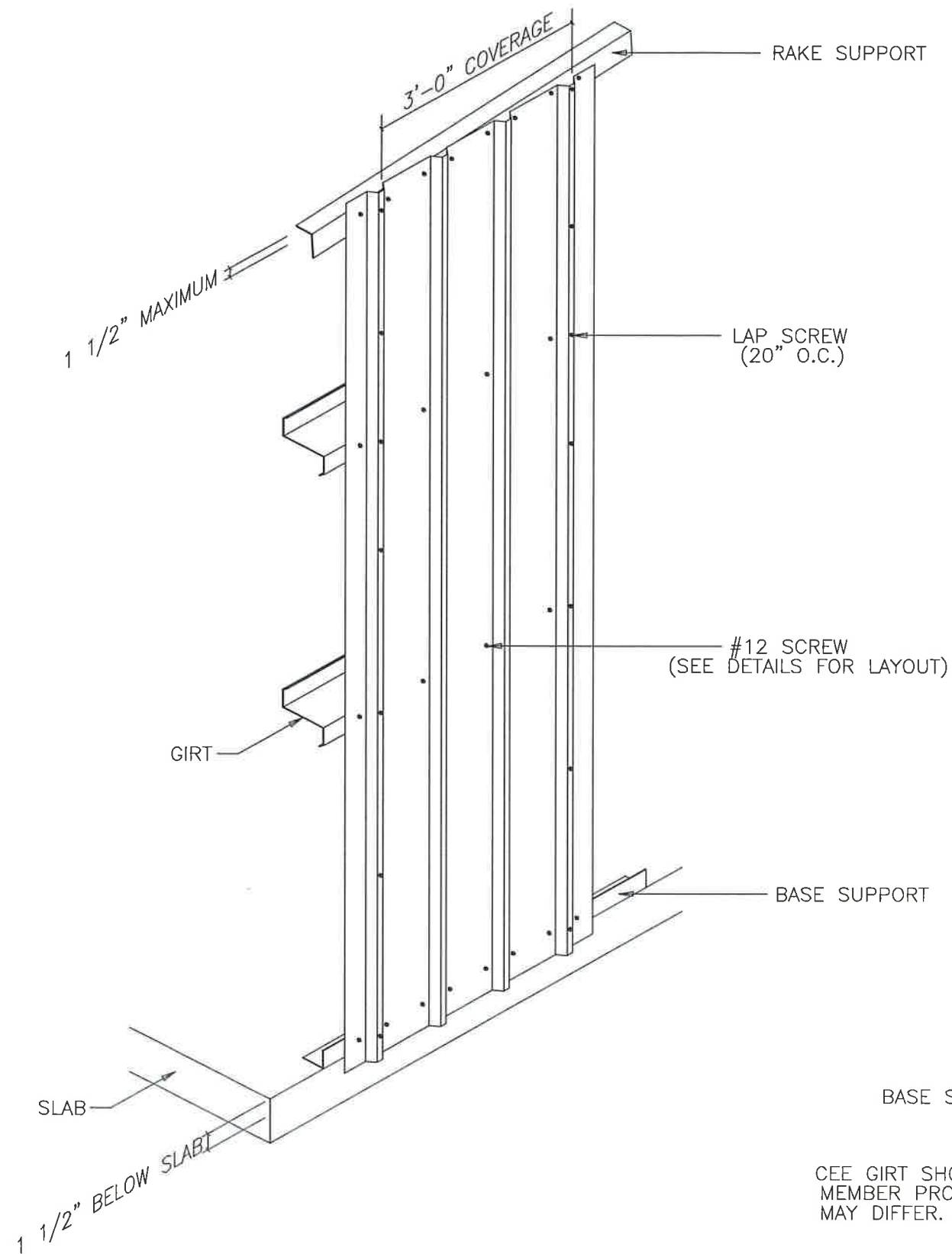
ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A		DATE: 1/2/25		
LOCATION: LAKE CITY, FL. 32024				
DRAWING NAME: ROOF PANELS & TRIM				
DRAWING NO: PAGE 6		DRAWN BY: BJC		CHECKED BY: SPW
				SCALE: NONE



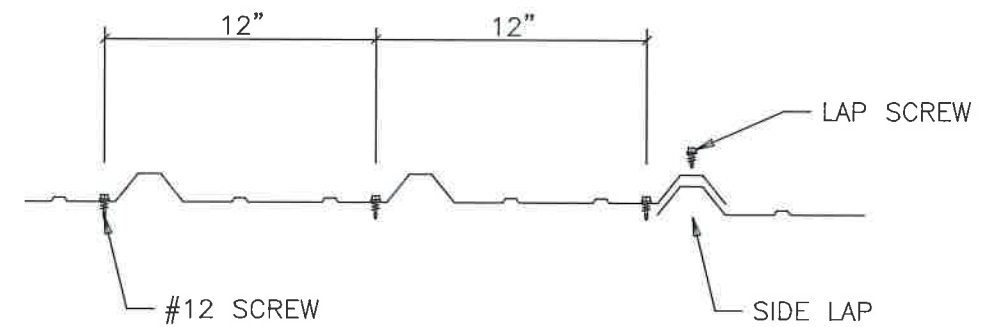
NOTES:

- [1] ALL END LAPS MUST BE A MINIMUM OF 6".
- [2] METAL SHAVINGS MUST BE SWEEPED FROM THE ROOF EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.
- [4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

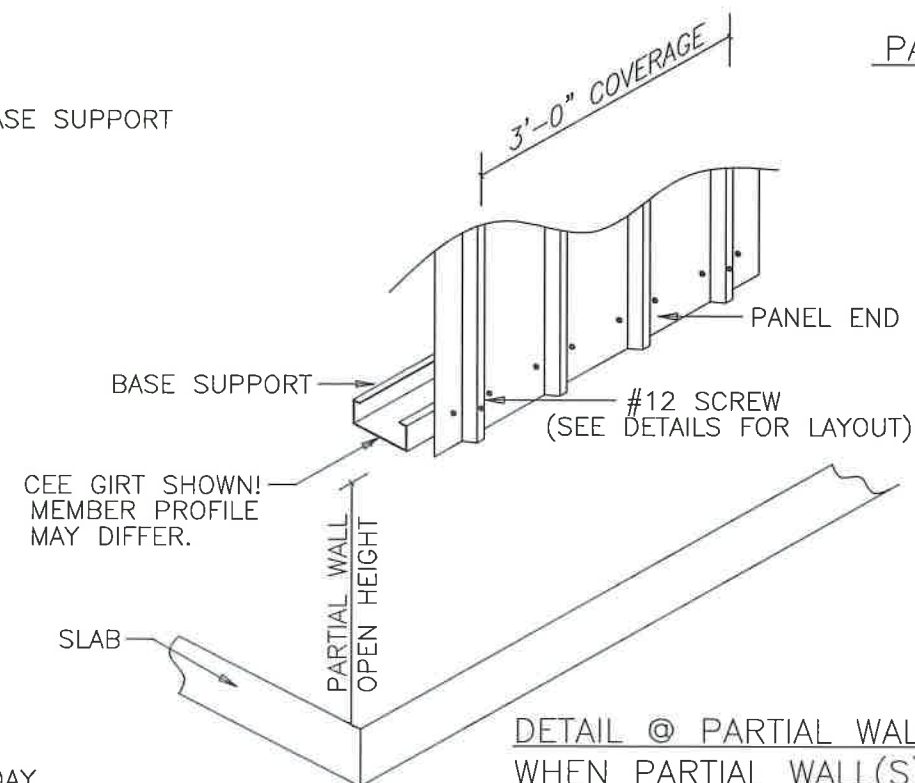
ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER:				
THE WOODS CONTAINER PARK				
JOB NO:		DATE:		
8967A		1/2/25		
LOCATION:				
LAKE CITY, FL. 32024				
DRAWING NAME:				
ROOF PANEL DETAILS				
DRAWING NO:		DRAWN BY:		CHECKED BY:
PAGE 6.1		BJC		SPW
				SCALE:
				NONE



PANEL ATTACHMENT AT PANEL END
(BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS

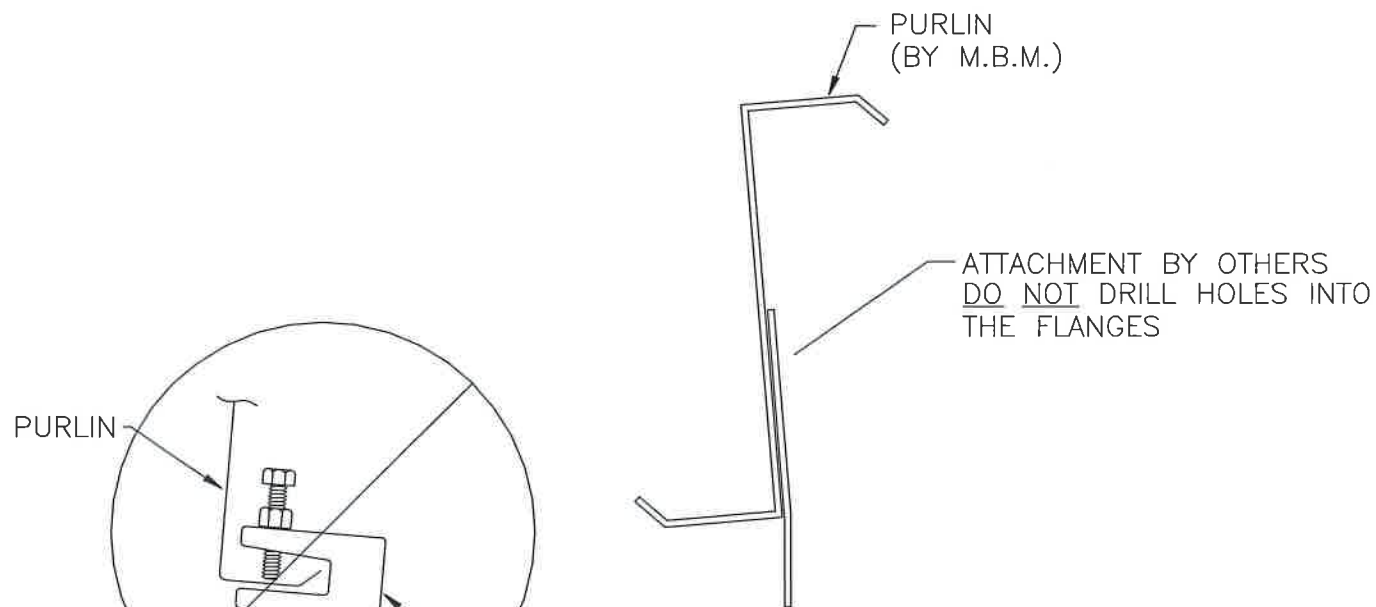


DETAIL @ PARTIAL WALL
WHEN PARTIAL WALL(S)
ARE PRESENT

NOTES:

- [1] METAL SHAVINGS MUST BE SWEEPED FROM THE WALL EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

ISSUE		DET	CHK	DATE
BUILDINGS AND MORE				
CUSTOMER: THE WOODS CONTAINER PARK				
JOB NO: 8967A		DATE: 1/2/25		
LOCATION: LAKE CITY, FL. 32024				
DRAWING NAME: ENDWALL PANEL DETAILS				
DRAWING NO: PAGE 8		DRAWN BY: BJC	CHECKED BY: SPW	SCALE: NONE



NOTE: M.B.M. only provides the roof purlin. All other material and hardware is by others.

Recommended Connection Detail

NOTE

MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

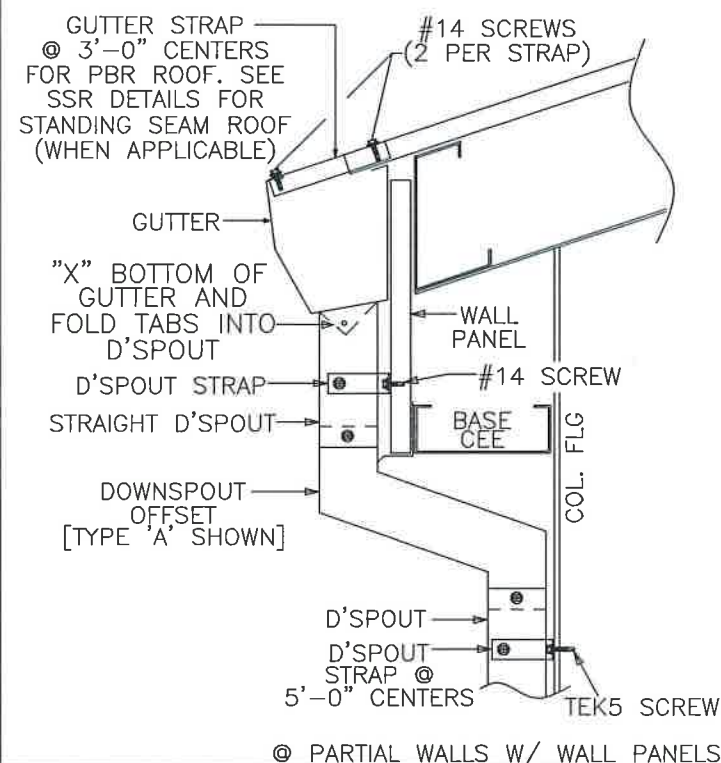
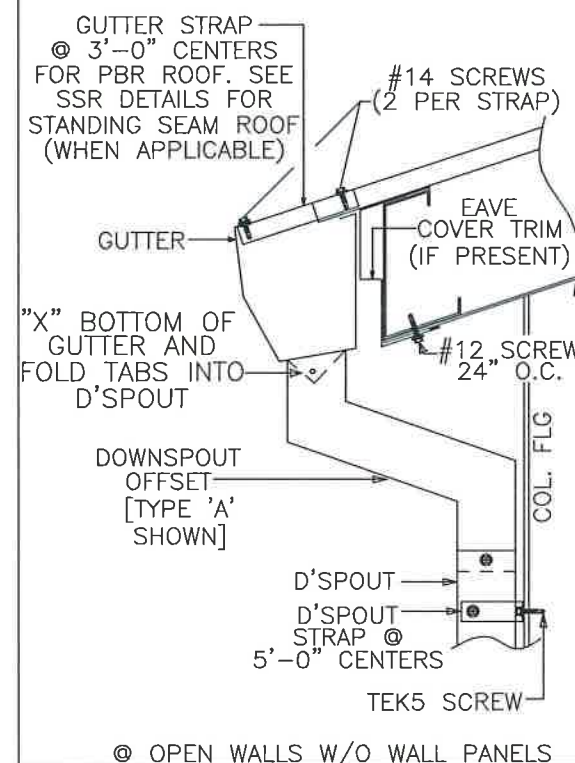
HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF-DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.K.A. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS

DO NOT INSTALL GUTTER WITH OUTSIDE FACE PERPENDICULAR TO THE GROUND.

INSTALL GUTTER WITH OUTSIDE FACE PERPENDICULAR TO THE ROOF.

GUTTER INSTALLATION DETAIL
(ONLY IF PROVIDED)



NOTE: REGARDLESS OF DOWNSPOUT OFFSET SCENARIO, TEK5 SCREWS MUST BE USED TO ATTACH DOWNSPOUT STRAPS TO PEMB FRAMING. WHEN WALL PANELS SPAN FROM GROUND TO EAVE (FULL SPAN), #14 SCREWS WILL BE USED TO ATTACH DOWNSPOUT STRAPS TO WALL PANELS.

ISSUE	DET	CHK	DATE
BUILDINGS AND MORE			
CUSTOMER:			
THE WOODS CONTAINER PARK			
JOB NO:	8967A	DATE:	1/2/25
LOCATION:			
LAKE CITY, FL. 32024			
DRAWING NAME:			
SPECIAL DETAILS			
DRAWING NO:	PAGE 9	DRAWN BY:	BJC
CHECKED BY:	SPW	SCALE:	NONE