

### Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

**PERMIT** 000037167

APPLICANT JACOB KIRSCH	PHONE 344-4817
ADDRESS 484 NW TURNER AVE	LAKE CITY FL 32055
OWNER SOUTHEAST DEVELOPERS	PHONE 755-2082
ADDRESS 150 SW MORNING GLORY DR	LAKE CITY FL 32024
CONTRACTOR JACOB KIESCH	PHONE 344-4817
	AHAN, TR HOPE HENRY, TR MORNING GLORY
3RD LOT ON RIGHT	war, mior balbaci, more and obort
TYPE DEVELOPMENT SFD,UTILITY	ESTIMATED COST OF CONSTRUCTION 123200.00
HEATED FLOOR AREA 1914.00 TOTAL	AREA 2464.00 HEIGHT STORIES 1
FOUNDATION CONC WALLS FRAMED	ROOF PITCH 8/12 FLOOR SLAB
LAND USE & ZONING RSF-2	MAX. HEIGHT 20
Minimum Set Back Requirments: STREET-FRONT 25	5.00 REAR 15.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE X PP	DEVEL OBMENT DEDMIT NO
NO. EX.D.C. V PEOOD ZONE XFF	DEVELOPMENT PERMIT NO.
PARCEL ID 15-4S-16-03023-557 SUBDIV	ISION ROLLING MEADOWS
LOT 57 BLOCK PHASE UNIT	TOTAL ACRES 0.51
000001637 CBC1253775	
Culvert Permit No. Culvert Waiver Contractor's License CULVERT 08-0052 BK	
	Zoning checked by Approved for Issuance New Resident
COMMENTS: PLAT REQUIRES MFE AT 107 FT., ELEVATION	CONFIRMATION LETTER REQUIRED
AT CLAD	
AT SLAB	Charle # ar Carle 12110
AT SLAB	Check # or Cash 12110
	Check # or Cash 12110  NING DEPARTMENT ONLY (footer/Slab)
	NING DEPARTMENT ONLY  Monolithic  (footer/Slab)
FOR BUILDING & ZO	NING DEPARTMENT ONLY (footer/Slab)
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sla	NING DEPARTMENT ONLY    Monolithic   date/app. by   date/app. by   Sheathing/Nailing
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by	NING DEPARTMENT ONLY  Monolithic  date/app. by  (footer/Slab)  date/app. by
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbin	MING DEPARTMENT ONLY    Monolithic     date/app. by   date/app. by     ab   Sheathing/Nailing     date/app. by   date/app. by     ng above slab and below wood floor
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbin date/app. by	MING DEPARTMENT ONLY    Monolithic     date/app. by   date/app. by     ab
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbin date/app. by  Electrical rough-in Heat & Air Duct	NING DEPARTMENT ONLY    Monolithic     date/app. by   date/app. by     ab   Sheathing/Nailing     date/app. by   date/app. by     ng above slab and below wood floor     Peri, beam (Lintel)
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbing Rough-in plumbin date/app. by  Electrical rough-in Heat & Air Duct date/app. by	MING DEPARTMENT ONLY    Monolithic     date/app. by   date/app. by     ab
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbin date/app. by  Electrical rough-in Heat & Air Duct	MING DEPARTMENT ONLY    Monolithic     date/app. by   date/app. by     ab
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbing Rough-in plumbin date/app. by  Electrical rough-in Heat & Air Duct date/app. by  Permanent power C.O. Final date/app. by  M/H tie downs, blocking, electricity and plumbing	Ming DEPARTMENT ONLY    Monolithic     date/app. by   date/app. by     ab
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbing Rough-in plumbin date/app. by  Electrical rough-in Heat & Air Duct date/app. by  Permanent power C.O. Final date/app. by  M/H tie downs, blocking, electricity and plumbing date	Monolithic  date/app. by  Sheathing/Nailing  date/app. by  ab  Sheathing/Nailing  date/app. by  ng above slab and below wood floor  date/app. by  Peri. beam (Lintel)  date/app. by  Culvert  date/app. by  Pool  e/app. by  date/app. by
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbing Rough-in plumbing date/app. by  Electrical rough-in date/app. by  Permanent power C.O. Final date/app. by  M/H tie downs, blocking, electricity and plumbing date  Reconnection Pump pole	Monolithic  date/app. by  Sheathing/Nailing  date/app. by  date/app. by  ng above slab and below wood floor  Peri. beam (Lintel)  date/app. by  Culvert  date/app. by  Pool  Utility Pole  (footer/Slab)  Monolithic  date/app. by  date/app. by  date/app. by  date/app. by  Utility Pole
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbing Rough-in plumbing date/app. by  Electrical rough-in Heat & Air Duct date/app. by  Permanent power C.O. Final date/app. by  M/H tie downs, blocking, electricity and plumbing date/app. by  M/H Pole Travel Trailer	Monolithic  date/app. by  Sheathing/Nailing  date/app. by  date/app. by  ab Sheathing/Nailing  date/app. by  date/app. by  Peri. beam (Lintel)  date/app. by  Culvert  date/app. by  Pool  E/app. by  Utility Pole  date/app. by  Cate/app. by  Pool  Date/app. by  Cate/app. by  Date/app. by
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbing Rough-in plumbing date/app. by  Electrical rough-in Heat & Air Duct date/app. by  Permanent power C.O. Final date/app. by  M/H tie downs, blocking, electricity and plumbing date/app. by  Reconnection Pump pole date/app. by	NING DEPARTMENT ONLY    Monolithic     date/app. by   date/app. by     ab
Temporary Power Foundation date/app. by  Under slab rough-in plumbing Sladate/app. by  Framing Rough-in plumbing Rough-in plumbing date/app. by  Electrical rough-in Heat & Air Duct date/app. by  Permanent power C.O. Final date/app. by  M/H tie downs, blocking, electricity and plumbing date/app. by  M/H Pole Travel Trailer	NING DEPARTMENT ONLY    Monolithic     date/app. by   date/app. by     ab
FOR BUILDING & ZO  Temporary Power	NING DEPARTMENT ONLY    Monolithic     date/app. by   date/app. by     ab
FOR BUILDING & ZO  Temporary Power Foundation date/app. by  Under slab rough-in plumbing Slate/app. by  Framing Rough-in plumbing Rough-in plumbing Rough-in plumbing Adate/app. by  Electrical rough-in Heat & Air Duct date/app. by  Permanent power C.O. Final date/app. by  M/H tie downs, blocking, electricity and plumbing date/app. by  M/H Pole Pump pole	NING DEPARTMENT ONLY    Monolithic     date/app. by   date/app. by     ab
Temporary Power	NING DEPARTMENT ONLY    Monolithic   date/app. by   date/app. by     ab

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

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"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

CLERKS OFFICE

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

## **Columbia County Building Permit Application**

	7,5
For Office Use Only Application # 0801-50 Date R	Received 1-11-08 By Ut Permit # 1637/27167
Zonin	aksty L Landtha Branch and Alleria
Regules 1/1 L 10/ At. Elevation	on Confirmation Letter Paris /
	ite Road Info   Parent Parcel #   Development Permi
Name Authorized Person Signing Permit Jacob K.	irsch Phone 386-344-4817
Address 10 1000 1011/21 ave #101, 1	we City, FL 3205-5-
Owners Name Southeast Developers	Phone 386-755-2082
911 Address 484, NW Turner ave. #101	Late City, F6 37055
Contractors Name Sucob Kirsch - Compas	SS Builders Phone 386-344-4817
Address 150 5w Morning Glory Dr. La	ke City fe 32024
	Druelopeis Group, Lake City FL
Bonding Co. Name & Address N/H	The transfer of the party of th
Architect/Engineer Name & Address Nicholas Paul	Geisler 1758 NW Brown Id. l.C. F.
Mortgage Lenders Name & Address Columbia Bank	1 13 NUI hillshore st Lake, City
Circle the correct power company - FL Power & Light - Clay Property ID Number 15-45-16-03073-5	FL, 3202
Total Transits	10167
Driving Directions from Usgo/441 - go usgo wes	
Sw callahan ave, the follow to	N 11.
Follow sw Morning clary do to the 31	
Type of Construction S.F.D.	Mumber of Future
Total Acreage Lot Size 5 Do you need a Culv	Number of Existing Dwellings on Property
Actual Distance of Structure from Property Lines - Front 50	Side Side
Total Building Height 70'6" Number of Stories H	Ked. 7
Application is hereby made to obtain a permit to do work and in installation has commenced prior to the issuance of a permit an ill laws regulating construction in this jurisdiction.	stallations as indicated. I certify that no work or
Il laws regulating construction in this jurisdiction.	that all work be performed to meet the standards of
OWNERS AFFIDAVIT: I hereby certify that all the foregoing inforcempliance with all applicable laws and regulating construction	rmation is accurate and all work will be done in
	and zoning.
VARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE WICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INT. ENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE (	OF COMMENCMENT MAY RESULT IN YOU PAYING
ENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE (	OF COMMENCEMENT.
	1 1 2/1
wner Builder or Authorized Person by Notarized Letter	Contractor Signature
TATE OF FLORIDA  NOTARY PUBLIC-STATE OF FLORIDA  Michelle Fischer:	Contractors License Number (R( 17 (377)-
	Competency Card Number NOTARY STAMP/SEAL
worn to (or affirmed) and subscribed barries: SEP 24, 2010 BONDED THRO ALLANTIC BONDING CO., INC.	1
day of QUYC 2008.	Mechella Fornop
ersonally known or Produced Identification	Notary Signature (Revised See See
	(Revised Sept. 2006)

Ju novises; Jake

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

<u>YOU ARE HEREBY NOTIFIED</u> as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's License Number CBC 1253775—Columbia County Competency Card Number

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 3/ day of Jah

SEAL:

State of Florida Notary Signature (For the Contractor)

Personally known or Produced Identification



### Corporate Warranty Deed

This Indenture, made this 26th, June, 2006 A.D.

Between

Milestone HS, LLC, whose post office address is: P.O. Box 3001 Lake City, Florida 32056 a Florida Limited Liability Company existing under the laws of the State of Florida, Grantor and Southeast Developers Group, Inc whose post office address is: 197 SW Waterford Court Suite 105, Lake City, Florida, 32025,

(Whenever used herein the term "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth, that the grantor, for and in consideration of the sum of TEN AND NO/100 DOLLARS (\$10.00) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz:

Lots 57 & 58, ROLLING MEADOWS, according to the plat thereof, recorded in Plat Book 8, Page(s) 45 and 46 of the Public Records of Columbia County, Florida.

The above described property does not constitute the homestead property of the grantor described herein.

Subject to taxes for the current year, covenants, restrictions and easements of record, if any.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2005.

And the said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

In Witness Whereof, the said Grantor has caused this instrument to be executed in its name by its duly authorized officer and caused its corporate seal to be affixed the day and year first above written.

Milestone HS, LLC

Signed and Sealed in Our Presence:

Robert S. Stewart

Its: Manager

Matthew D. Rocco

State of Florida County of Columbia

The foregoing instrument was acknowledged before me this 26th day of June, 2006, by Robert S. Stewart, Manager of Milestone HS, LLC. a Corporation existing under the laws of the State of Florida, on behalf of the corporation. He/She is personally known to me or has produced DL as identification.

Notary Public

(Seal)

Matthew D. Rocco Notary Printed Name

My Commission Expires::

Prepared by: Matt Rocco, an employee of Sierra Title, LLC, 619 SW Baya Drive, Suite 102 Lake City, Florida 32025

My Commission DD150709 Expires September 17, 2006

Inst:2006015873 Date:07/03/2006 Time:14:28

Doc Stamp-Deed : 910.00

DC, P. DeWitt Cason, Columbia County B: 1088 P: 1929

Closer's Choice Florida Corporate Deed/Letter

## COLUMBIA COUNTY 9-1-1 ADDRESSING / GIS DEPARTMENT

P. O. Box 1787, Lake City, FL 32056-1787
Telephone: (386) 758-1125 \* Fax: (386) 758-1365 \* E-mail: ron\_croft@columbiacountyfla.com

### ADDRESS ASSIGNMENT DATA

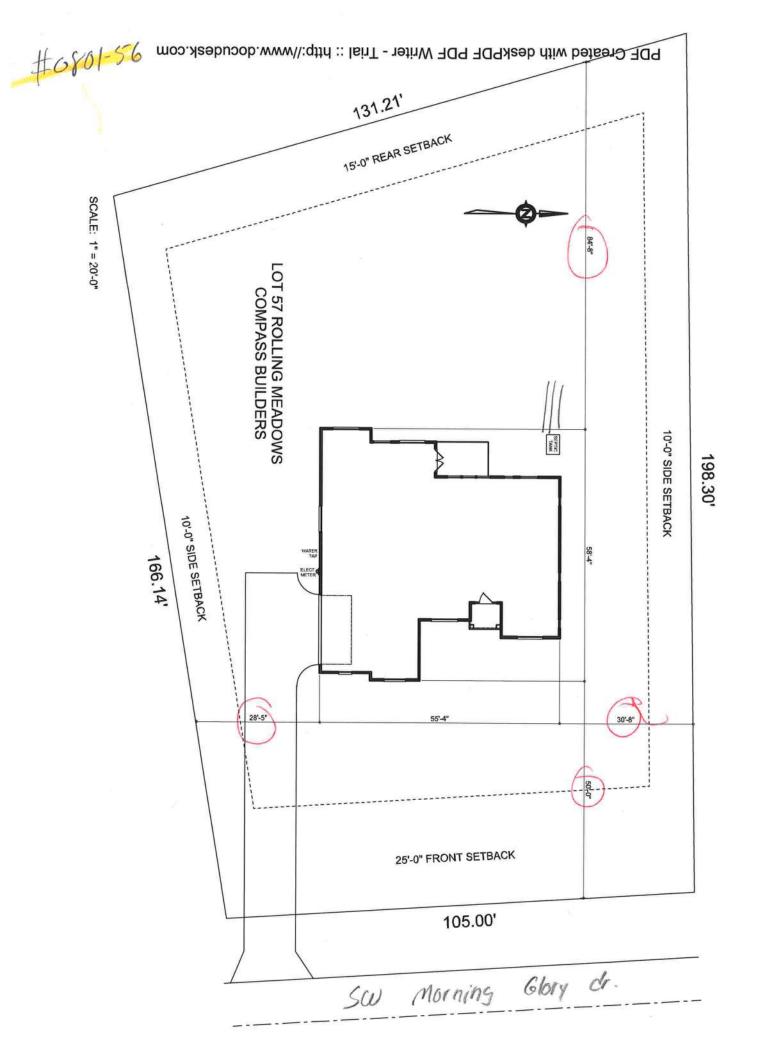
The Columbia County Board of County Commissioners has passed Ordinance 2001-9, which provides for a uniform numbering system. A copy of this ordinance is available in the Clerk of Court records, located in the courthouse. This new numbering system will increase the efficiency of POLICE, FIRE AND EMERGENCY MEDICAL vehicles responding to calls within Columbia County by immediately identifying the location of the caller.

<u>Residential or Other Structure on Parcel Number:</u> 15-4S-16-03023-557

Address Assignment: 150 SW MORNING GLORY DR, LAKE CITY, FL, 32024

Note: LOT 57 ROLLING MEADOWS S/D

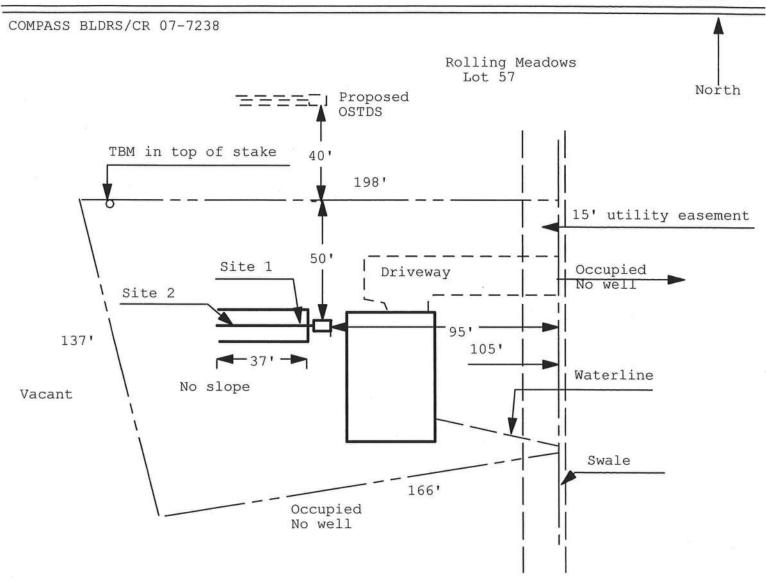
Any questions concerning this information should be referred to the Columbia County 9-1-1 Addressing / GIS Department at the address or telephone number above.



08-0052

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: 080 -56

### ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



				1 i	nch = 40 feet
	Plan Submitted Approved	By Ox Not Approved	ul Lay	Date ///	1/08
Ву	Man s	Zand		Columbia	СРНИ
Notes	s:				

# **Columbia County Building Department Culvert Permit**

### Culvert Permit No.

000001637

DATE 07/1	1/2008	PARCEL ID #	15-4S-16-03023-557	
APPLICANT	JACOB KIRSCH	tania de la companya	PHONE 34	14-4817
ADDRESS	484 NW TURN	ER AVE SUITE 101	LAKE CITY	FL 32055
OWNER SO	UTHEAST DEVE	OPERS	PHONE <u>75</u>	5-2082
ADDRESS 15	50 SW MORNI	NG GLORY DR	LAKE CITY	FL 32024
CONTRACTO	R JACOB KIESO	Н	PHONE 34	4-4817
LOCATION OF	FPROPERTY	90W, TL ON 247S, TL CAL	LAHAN, TR HOPE HENRY, T	R MORNING GLORY,
3RD LOT ON RIG	НТ			
			<u></u>	
SUBDIVISION	/LOT/BLOCK/I	PHASE/UNIT ROLLING	MEADOWS	57
SIGNATURE	1			
SIGNATURE -	5			
		ION REQUIREMENT		
X	driving surface	Il be 18 inches in diamete . Both ends will be mitere d concrete slab.	er with a total lenght of 32 fo ed 4 foot with a 4 : 1 slope a	eet, leaving 24 feet of and poured with a 4 inch
	<ul> <li>a) a majority</li> <li>b) the drivew</li> <li>Turnouts s</li> <li>concrete o</li> </ul>	ray to be served will be pa hall be concrete or paved	ng driveway turnouts are pa aved or formed with concre d a minimum of 12 feet wid ever is greater. The width sh	te. e or the width of the
	Culvert install	ation shall conform to the	e approved site plan standa	rds.
	Department of	Transportation Permit in	nstallation approved standa	rds.
	Other			
	D			
ALL PROPER SA	FETY REQUIRE	MENTS SHOULD BE FOLL	LOWED	SEI DO

DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



Project Name:

Compass Builders - Kezia

Compass Builders

## FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

Address: City, State: Owner: Climate Zone:	Lot:5,7Sub: Rollin Lake City, FL 320 Spec House North	ng Meadows, Plat: 025- Lot 57		2006 2006
1. New construct		New _	12. Cooling systems	C 42.0 l.Ph./h
	or multi-family its, if multi-family	Single family	a. Central Unit	Cap: 42.0 kBtu/hr SEER: 13.00
4. Number of Bed		1 —	b. N/A	SEEK. 13.00
5. Is this a worst		No —	U. IVA	_
6. Conditioned fl		1914 ft²	c. N/A	_
	d area: (Label reqd. by 13-10		0.1111	-
a. U-factor:		escription Area	13. Heating systems	
(or Single or I	Double DEFAULT) 7a.(Db	le Default) 294 3 ft <sup>2</sup>	a. Electric Heat Pump	Cap: 42.0 kBtu/hr
b. SHGC:	, , , , , , ,			HSPF: 7.70
(or Clear or T	Tint DEFAULT) 7b.	(Clear) 294.3 ft <sup>2</sup>	b. N/A	_
<ol><li>Floor types</li></ol>		***************************************		_
a. Slab-On-Grade	e Edge Insulation	R=5.0, 240.0(p) ft	c. N/A	_
b. N/A		_		
c. N/A		_	14. Hot water systems	
9. Wall types			a. Electric Resistance	Cap: 50.0 gallons
a. Frame, Wood,		R=13.0, 1487.7 ft <sup>2</sup>		EF: 0.90
b. Frame, Wood,	Adjacent	R=13.0, 220.0 ft <sup>2</sup>	b. N/A	_
c. N/A		_		_
d. N/A e. N/A		-	c. Conservation credits	_
10. Ceiling types		_	(HR-Heat recovery, Solar	
a. Under Attic		R=30.0, 2050.0 ft <sup>2</sup>	DHP-Dedicated heat pump) 15. HVAC credits	PT,
b. N/A		K-30.0, 2030.0 It	(CF-Ceiling fan, CV-Cross ventilation,	11, —
c. N/A		_	HF-Whole house fan,	
11. Ducts		_	PT-Programmable Thermostat,	
a. Sup: Unc. Ret	: Unc. AH: Garage	Sup. R=6.0, 50.0 ft	MZ-C-Multizone cooling,	
b. N/A			MZ-H-Multizone heating)	
		_		
GI	lass/Floor Area: 0.15	Total as-built		
		Total base	points: 25788	

I hereby certify that the plans and specifications covered by Review of the plans and this calculation are in compliance with the Florida Energy specifications covered by this Code. calculation indicates compliance with the Florida Energy Code. PREPARED BY: Before construction is completed DATE: this building will be inspected for compliance with Section 553.908 I hereby certify that this building, as designed, is in compliance Florida Statutes. with the Florida Energy Code. OWNER/AGENT: BUILDING OFFICIAL: DATE: DATE:

## **SUMMER CALCULATIONS**

## Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

	BASE					AS-	BUI	LT	AS-BUILT								
GLASS TYPES .18 X Condition Floor Are		SPM = F	Points	Type/SC		rhang Len	Hgt	Area X	SPI	их	SOF	= Points					
.18 1914.0	)	18.59	6405.0	1.Double, Clear	W	1.5	10.0	105.0	38	3.52	0.98	3959.0					
				2.Double, Clear	w	1.5	10.0		38	3.52	0.98	150.0					
4-				3.Double, Clear	N	1.5	10.0	40.0	19	9.20	0.98	754.0					
				4.Double, Clear	w	1.5	10.0	6.0	38	3.52	0.98	226.0					
				5.Double, Clear	W	1.5	10.0	16.0	38	3.52	0.98	603.0					
				6.Double, Clear	N	1.5	8.0	20.0	19	9.20	0.97	371.0					
				7.Double, Clear	E	1.5	8.0	30.0	42	2.06	0.96	1208.0					
				8.Double, Clear	E	7.5	10.0	13.3		2.06	0.59	332.0					
				9.Double, Clear	Е	1.5	10.0	30.0	42	2.06	0.98	1234.0					
				10.Double, Clear	S	1.5	8.0	30.0	35	5.87	0.92	993.0					
				As-Built Total:				294.3				9830.0					
WALL TYPES	Area X	BSPM	= Points	Туре		R-	Value	e Area	Х	SPI	M =	Points					
Adjacent	220.0	0.70	154.0	1. Frame, Wood, Exterior			13.0	1487.7		1.50		2231.5					
Exterior	1487.7	1.70	2529.1	2. Frame, Wood, Adjacent			13.0	220.0		0.60		132.0					
Base Total:	1707.7		2683.1	As-Built Total:				1707.7				2363.5					
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	X	SPI	M =	Points					
Adjacent	20.0	2.40	48.0	1.Exterior Insulated				20.0		4.10		82.0					
Exterior	20.0	6.10	122.0	2.Adjacent Insulated				20.0		1.60		32.0					
Base Total:	40.0		170.0	As-Built Total:				40.0				114.0					
CEILING TYPES	Area X	BSPM	= Points	Туре	F	R-Valu	ie /	Area X S	SPM	X S	CM =	Points					
Under Attic	1914.0	1.73	3311.2	1. Under Attic		;	30.0	2050.0	1.73 X	( 1.00		3546.5					
Base Total:	1914.0		3311.2	As-Built Total:				2050.0				3546.5					
FLOOR TYPES	Area X	BSPM	= Points	Туре		R-	Value	Area	Х	SPN	<b>/</b> =	Points					
Slab 24 Raised	40.0(p) 0.0	-37.0 0.00	-8880.0 0.0	1. Slab-On-Grade Edge Insul	lation		5.0	240.0(p	-	36.20		-8688.0					
Base Total:			-8880.0	As-Built Total:				240.0				-8688.0					
INFILTRATION	Area X	BSPM	= Points					Area	х	SPN	/I =	Points					
	1914.0	10.21	19541.9					1914.0	)	10.21		19541.9					

## **SUMMER CALCULATIONS**

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

PERMIT #:

	BASE		AS-BUILT									
Summer Ba	se Points:	23231.2	Summer As-Built Points:									
Total Summer Points	X System Multiplier	= Cooling Points	Component Ratio	X Duct X System X Multiplier Multiplier DM x DSM x AHU)	Credit = Cooling Multiplier Points							
23231.2	0.3250	7550.2		EER/EFF(13.0) Ducts:Unc(S),Unc(R), 1.09 x 1.147 x 1.00) 0.260 1.250 0.260	Gar(AH),R6.0(INS) 0.950 8247.6 <b>0.950 8247.6</b>							

## WINTER CALCULATIONS

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

PERMIT #:

BASE		AS-	BUI	LT	100-24		
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	40	Overhang nt Len	Hgt	Area X	WPM X	WO	F = Points
.18 1914.0 20.17 6949.0	1.Double, Clear	W 1.5	10.0	105.0	20.73	1.01	2188.0
	2.Double, Clear	W 1.5	10.0	4.0	20.73	1.01	83.0
	3.Double, Clear	N 1.5	10.0	40.0	24.58	1.00	983.0
	4.Double, Clear	W 1.5	10.0	6.0	20.73	1.01	125.0
	5.Double, Clear	W 1.5	10.0	16.0	20.73	1.01	333.0
4	6.Double, Clear	N 1.5	8.0	20.0	24.58	1.00	491.0
	7.Double, Clear	E 1.5	8.0	30.0	18.79	1.02	574.0
	8.Double, Clear	E 7.5	10.0	13.3	18.79	1.21	303.0
	9.Double, Clear	E 1.5	10.0	30.0	18.79	1.01	570.0
	10.Double, Clear	S 1.5	8.0	30.0	13.30	1.04	415.0
	As-Built Total:			294.3			6065.0
WALL TYPES Area X BWPM = Points	Туре	R-	Value	Area	X WP	л =	Points
Adjacent 220.0 3.60 792.0	1. Frame, Wood, Exterior		13.0	1487.7	3.40		5058.2
Exterior 1487.7 3.70 5504.5	2. Frame, Wood, Adjacent		13.0	220.0	3.30		726.0
Base Total: 1707.7 6296.5	As-Built Total:			1707.7			5784.2
DOOR TYPES Area X BWPM = Points	Туре			Area	X WPN	/I =	Points
Adjacent 20.0 11.50 230.0	1.Exterior Insulated			20.0	8.40		168.0
Exterior 20.0 12.30 246.0	2.Adjacent Insulated			20.0	8.00		160.0
Base Total: 40.0 476.0	As-Built Total:			40.0			328.0
CEILING TYPES Area X BWPM = Points	Туре	R-Value	Ar	ea X W	PM X WO	CM =	Points
Under Attic 1914.0 2.05 3923.7	1. Under Attic		30.0	2050.0 2	2.05 X 1.00		4202.5
Base Total: 1914.0 3923.7	As-Built Total:			2050.0			4202.5
FLOOR TYPES Area X BWPM = Points	Туре	R-	Value	Area	X WPN	<b>/</b> =	Points
Slab         240.0(p)         8.9         2136.0           Raised         0.0         0.00         0.0	Slab-On-Grade Edge Insulation	n	5.0	240.0(p	7.60		1824.0
Base Total: 2136.0	As-Built Total:			240.0			1824.0
INFILTRATION Area X BWPM = Points				Area	X WPN	1 =	Points
1914.0 -0.59 -1129.3				1914.0	0.59		-1129.3

### WINTER CALCULATIONS

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

	BASE		AS-BUILT								
Winter Base	Points:	18651.9	Winter As-Built Points:	17074.4							
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit : Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	= Heating Points							
18651.9	0.5540	10333.2	(sys 1: Electric Heat Pump 42000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Gard 17074.4 1.000 (1.069 x 1.169 x 1.00) 0.443 0.950 17074.4 1.00 1.250 0.443 0.950	(AH),R6.0 8976.9 <b>8976.9</b>							

## **WATER HEATING & CODE COMPLIANCE STATUS**

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

BASE					AS-BUILT									
WATER HEA Number of Bedrooms	TING X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier	X Credit Multipli		Total	
3		2635.00		7905.0	50.0	0.90	3		1.00	2693.56	1.00		8080.7	
					As-Built To	otal:							8080.7	

	CODE COMPLIANCE STATUS												
BASE						AS-BUILT							
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
7550		10333		7905	- 1117-	25788	8248		8977		8081		25305

**PASS** 



## **Code Compliance Checklist**

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

#### 6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.  EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

### 6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

#### ESTIMATED ENERGY PERFORMANCE SCORE\* = 84.8

The higher the score, the more efficient the home.

Spec House, Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

1.	New construction or existing	New		12.	Cooling systems	
2.	Single family or multi-family	Single family			Central Unit	Cap: 42.0 kBtu/hr
3.	Number of units, if multi-family	1				SEER: 13.00
4.	Number of Bedrooms	3		b.	N/A	
5.	Is this a worst case?	No				
6.	Conditioned floor area (ft2)	1914 ft²		c.	N/A	
7.	Glass type 1 and area: (Label reqd. b	y 13-104.4.5 if not default)				_
a.	U-factor:	Description Area		13.	Heating systems	_
	(or Single or Double DEFAULT)	7a. (Dble Default) 294.3 ft²			Electric Heat Pump	Cap: 42.0 kBtu/hr
b.	SHGC:	(= 110 = 111111) = 1110 11	_		Company of the compan	HSPF: 7.70
	(or Clear or Tint DEFAULT)	7b. (Clear) 294.3 ft <sup>2</sup>		b.	N/A	-
8.	Floor types	(3344) 45 115 11				_
a.	Slab-On-Grade Edge Insulation	R=5.0, 240.0(p) ft		c.	N/A	-
b.	N/A					-
c.	N/A		_	14.	Hot water systems	_
9.	Wall types				Electric Resistance	Cap: 50.0 gallons
a.	Frame, Wood, Exterior	R=13.0, 1487.7 ft <sup>2</sup>				EF: 0.90
b.	Frame, Wood, Adjacent	R=13.0, 220.0 ft <sup>2</sup>		b.	N/A	
c.	N/A					
d.	N/A			c.	Conservation credits	76
e.	N/A				(HR-Heat recovery, Solar	, <del></del>
10.	Ceiling types				DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 2050.0 ft <sup>2</sup>		15.	HVAC credits	РТ, _
b.	N/A				(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A				HF-Whole house fan,	
11.	Ducts				PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 50.0 ft			MZ-C-Multizone cooling,	
	N/A		_		MZ-H-Multizone heating)	
			_		, , , , , , , , , , , , , , , , , , ,	
I cer	rtify that this home has complied	d with the Florida Energy	/ Efficie	ency	Code For Building	
Con	struction through the above ene	rgy saving features which	h will b	e ins	stalled (or exceeded)	OF THE STATE
in th	is home before final inspection.	Otherwise, a new EPL I	Display	Car	d will be completed	13 ( ) ( ) ( ) ( ) ( ) ( ) ( ) ( ) ( ) (
base	ed on installed Code compliant f	eatures.	- topiuj		a min oo completed	5
	der Signature:		Date:			
	an organia.		Date:			I I
Add	ress of New Home:		City/E	1 7	n.	1
			City/F	LL	p:	GOD WE TRU

\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCPB v4.5.2)

## **Residential System Sizing Calculation**

Summary Project Title:

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Kezia

Lot 57

Code Only Professional Version

Climate: North

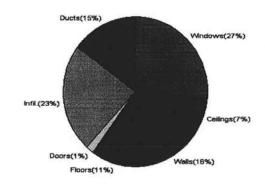
1/10/2008

				1/10/200	0
Location for weather data: Gaine	sville - Def	aults: Latite	ude(29) Altitude(152 ft.) Temp Ran	nge(M)	
Humidity data: Interior RH (50%	) Outdoor	wet bulb (7	7F) Humidity difference(54gr.)	J , ,	
Winter design temperature	33		Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	35547	Btuh	Total cooling load calculation	49767	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	118.2	42000	Sensible (SHR = 0.75)	77.4	31500
Heat Pump + Auxiliary(0.0kW)	118.2	42000	Latent		10500
			Total (Electric Heat Pump)	84.4	42000

### **WINTER CALCULATIONS**

Winter Heating Load (for 1914 sqft)

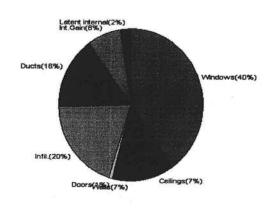
Load component			Load	
Window total	294	sqft	9474	Btuh
Wall total	1708	sqft	5608	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	2050	sqft	2416	Btuh
Floor total	240	sqft	3925	Btuh
Infiltration	204	cfm	8270	Btuh
Duct loss		Sacranach	5336	Btuh
Subtotal			35547	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			35547	Btuh



### **SUMMER CALCULATIONS**

Summer Cooling Load (for 1914 sqft)

Load component			Load	
Window total	294	sqft	19977	Btuh
Wall total	1708	sqft	3435	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	2050	sqft	3395	Btuh
Floor total			0	Btuh
Infiltration	179	cfm	3325	Btuh
Internal gain			3780	Btuh
Duct gain			6405	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain		1	40708	Btuh
Latent gain(ducts)		- 1	1330	Btuh
Latent gain(infiltration)		- 1	6528	Btuh
Latent gain(ventilation)		- 1	0	Btuh
Latent gain(internal/occup	pants/othe	r)	1200	Btuh
Total latent gain			9059	Btuh
TOTAL HEAT GAIN			49767	Btuh





Version 8 For Florida residences only PREPARED BY: 1-9-0 8

## **System Sizing Calculations - Winter**

## Residential Load - Whole House Component Details

Spec House

Project Title: Compass Builders - Kezia Code Only Professional Version

Lake City, FL 32025-

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

omponent L	oads for Whole House				
Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	105.0	32.2	3380 Btu
2	2, Clear, Metal, 0.87	W	4.0	32.2	129 Btu
3	2, Clear, Metal, 0.87	N	40.0	32.2	1288 Btu
4	2, Clear, Metal, 0.87	W	6.0	32.2	193 Btu
5	2, Clear, Metal, 0.87	W	16.0	32.2	515 Btu
6	2, Clear, Metal, 0.87	N	20.0	32.2	644 Btu
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btu
8	2, Clear, Metal, 0.87	E	13.3	32.2	429 Btu
9	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btu
10	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btu
	Window Total		294(sqft)	02.2	9474 Btu
Walls	Туре	R-Value	· Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1488	3.3	4886 Btu
2	Frame - Wood - Adj(0.09)	13.0	220	3.3	722 Btu
	Wall Total		1708	0.0	5608 Btu
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btu
2	Insulated - Adjacent		20	12.9	259 Btu
	Door Total		40		518Btu
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	2050	1.2	2416 Btu
	Ceiling Total		2050		2416Btu
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	240.0 ft(p)	16.4	3925 Btu
	Floor Total		240	50.00.000	3925 Btu
			Envelope Su	ıbtotal:	21941 Btu
Infiltration	Туре		ume(cuft) walls(sqf	t) CFM=	
	Natural	0.80	15312 1708	204.2	8270 Btu
Ductload			(D	LM of 0.177)	5336 Btu
All Zones		I Zones	35547 Btu		

### **Manual J Winter Calculations**

Residential Load - Component Details (continued)

Spec House

Project Title: Compass Builders - Kezia

Code Only Professional Version

Climate: North

1/10/2008

Lake City, FL 32025-

WHOLE HOUSE TO	TALS	
	Subtotal Sensible Ventilation Sensible	35547 Btuh 0 Btuh
	Total Btuh Loss	35547 Btuh

#### EQUIPMENT

1. Electric Heat Pump	#	42000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



Version 8 For Florida residences only

## **System Sizing Calculations - Winter**

## Residential Load - Room by Room Component Details

Spec House

Project Title: Compass Builders - Kezia

Professional Version

Lake City, FL 32025-

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

### Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	105.0	32.2	3380 Btuh
2	2, Clear, Metal, 0.87	W	4.0	32.2	129 Btuh
3	2, Clear, Metal, 0.87	N	40.0	32.2	1288 Btuh
4	2, Clear, Metal, 0.87	W	6.0	32.2	193 Btuh
5	2, Clear, Metal, 0.87	W	16.0	32.2	515 Btuh
6	2, Clear, Metal, 0.87	N	20.0	32.2	644 Btuh
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
8	2, Clear, Metal, 0.87	E	13.3	32.2	429 Btuh
9	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
10	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btuh
	Window Total	#E	294(sqft)		9474 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1488	3.3	4886 Btuh
2	Frame - Wood - Adj(0.09)	13.0	220	3.3	722 Btuh
	Wall Total		1708		5608 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	2050	1.2	2416 Btuh
	Ceiling Total		2050		2416Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	240.0 ft(p)	16.4	3925 Btuh
	Floor Total		240		3925 Btuh
		z	one Envelope Su	btotal:	21941 Btuh
Infiltration	Туре	ACH X Volu	ume(cuft) walls(sqf	) CFM=	
	Natural	0.80	15312 1708	204.2	8270 Btuh
Ductload	Average sealed, Supply(R6.	0-Attic), Retur	n(R6.0-Attic) (D	LM of 0.177)	5336 Btuh
Zone #1		Sens	sible Zone Subto	tal	35547 Btuh

## **Manual J Winter Calculations**

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Kezia

Code Only Professional Version Climate: North

1/10/2008

	Subtotal Sensible Ventilation Sensible Total Btuh Loss	35547 Btul 0 Btul 35547 Btul
EQUIPMENT		

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



Version 8 For Florida residences only

## **System Sizing Calculations - Summer**

## Residential Load - Whole House Component Details

Spec House

Project Title: Compass Builders - Kezia Code Only Professional Version

Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/10/2008

#### **Component Loads for Whole House**

	Type*		Over	hang	Win	dow Area	a(sqft)	H	HTM	Load	-	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross			Shaded	Unshaded			
1	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	105.0	0.0	105.0	29	80	8349	Btuh	
2	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	4.0	0.0	4.0	29	80	318	Btuh	
3	2, Clear, 0.87, None, N, N	N	1.5ft	10ft.	40.0	0.0	40.0	29	29	1158	Btuh	
4	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	6.0	0.0	6.0	29	80	477	Btuh	
5	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	16.0	0.0	16.0	29	80	1272	Btuh	
6	2, Clear, 0.87, None, N, N	N	1.5ft	8ft.	20.0	0.0	20.0	29	29	579	Btuh	
7	2, Clear, 0.87, None, N, N	E	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	Btuh	
8	2, Clear, 0.87, None, N, N	E	7.5ft	10ft.	13.3	5.8	7.5	29	80	768	Btuh	
9	2, Clear, 0.87, None, N, N	E	1.5ft	10ft.	30.0	0.0	30.0	29	80	2385	Btuh	
10	2, Clear, 0.87, None, N, N	S	1.5ft	8ft.	30.0	30.0	0.0	29	34	869	Btuh	
	Excursion		1000000000				*******		15500	1415	Btuh	
000000000000000000000000000000000000000	Window Total				294 (	sqft)				19977	Btuh	
Walls	Туре		R-Va	alue/U	l-Value	Area	(sqft)		нтм	Load		
1	Frame - Wood - Ext			13.0/	0.09	148	37.7		2.1	3103	Btuh	
2	Frame - Wood - Adj			13.0/	0.09 220.0 1.5					332	Btuh	
	Wall Total					1708 (sqft)				3435	Btuh	
Doors	Туре						(sqft)		нтм	Load		
1	Insulated - Exterior					20	0.0		9.8	196	Btuh	
2	Insulated - Adjacent					20	0.0		9.8	196	Btuh	
	Door Total					4	0 (sqft)			392	Btuh	
Ceilings	Type/Color/Surface	***********	R-Va	alue		Area	THE RESERVE AND ADDRESS OF THE PARTY OF THE		НТМ	Load		
1	Vented Attic/DarkShingle			30.0		205	50.0		1.7	3395	Btuh	
	Ceiling Total						0 (sqft)			3395		
Floors	Туре		R-Va	alue			ze		НТМ	Load		
1	Slab On Grade			5.0		24	40 (ft(p))		0.0	0	Btuh	
	Floor Total						0 (sqft)			0	Btuh	
						E	nvelope	Subtota	l:	27199	Btuh	
nfiltration	Typo			CLI	Value	- (a) - (4)	المند	(==ft)	CEM-	1		
iiiillialion	Type		A	CH	volum		wall area	(sqn)	CFM=	Load	<b>.</b>	
Internal	SensibleNatural		0	0.70		15312	1708		204.2		Btuh	
		(	Occup				cupant	/	Appliance	Load	n	
gain				6		X 23	0 +		2400	3780	Btur	
						S	ensible E	nvelope	e Load:	34303	Btuh	
Duct load							(DGI	M of 0.1	87)	6405	Btul	
						Ser	nsible Lo	oad All	Zones	40708	Btuh	

## **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Kezia Code Only Professional Version

Climate: North

1/10/2008

#### WHOLE HOUSE TOTALS

		-	
	Sensible Envelope Load All Zones	34303	Btul
	Sensible Duct Load	6405	Btuh
	Total Sensible Zone Loads	40708	Btu
	Sensible ventilation	0	Btul
	Blower	0	Btul
Whole House	Total sensible gain	40708	Btu
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6528	Btul
	Latent ventilation gain	0	Btu
	Latent duct gain	1330	Btu
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btul
	Latent other gain	0	Btul
	Latent total gain	9059	Btu
	TOTAL GAIN	49767	Btu

EQUIPMENT		
1. Central Unit	#	42000 Btuh

\*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

## **System Sizing Calculations - Summer**

### Residential Load - Room by Room Component Details

Spec House

Project Title:

**Professional Version** 

Lake City, FL 32025-

Compass Builders - Kezia

Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/10/2008

#### Component Loads for Zone #1: Main

HTM	Load	
ed Unshaded		
80	8349	Btuh
80	318	Btuh
29	1158	Btuh
80	477	Btul
80	1272	Btul
29	579	Btul
80	2385	
80		Btul
80	2385	Btul
34	869	Btuh
	18562	Btuh
НТМ	Load	
2.1		Btuh
1.5		Btuh
1.0	3435	
нтм	Load	Diui
200		<b>.</b>
9.8		Btul
9.8		Btuh
		Btuh
HTM	Load	
1.7	3395	Btuh
	3395	
нтм	Load	
0.0		Btuh
0.0		Btuh
	U	Blur
Subtotal:	25784	Btuh
CFM=	Load	
178.6	3325	Btuh
Appliance	Load	
2400	3780	Btu
pe Load:	32889	Btuh
1 of 0.187)	6141	Btu
	0.187) Load	

### The following window Excursion will be assigned to the system loads.

Windows	July excursion for System 1		1415 Btuh
		Excursion Subtotal:	1415 Btuh

## **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Project Title:
Compass Builders - Kezia

Project Title:
Compass Builders - Kezia

Spec House

Code Only Professional Version

Lake City, FL 32025-

Climate: North

1/10/2008

		Sensible Excursion Load	1679 Btul
--	--	-------------------------	-----------

## **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Kezia

Code Only Professional Version Climate: North

1/10/2008

#### WHOLE HOUSE TOTALS

		1	
	Sensible Envelope Load All Zones	34303	Btul
	Sensible Duct Load	6405	Btuh
	Total Sensible Zone Loads	40708	Btu
	Sensible ventilation	0	Btul
	Blower	0	Btu
Whole House	Total sensible gain	40708	Btu
<b>Totals for Cooling</b>	Latent infiltration gain (for 54 gr. humidity difference)	6528	Btu
	Latent ventilation gain	0	Btu
	Latent duct gain	1330	Btu
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btu
23	Latent other gain	0	Btu
	Latent total gain	9059	Btu
	TOTAL GAIN	49767	Btu

EQUIPMENT		
1. Central Unit	#	42000 Btuh

\*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)



Version 8 For Florida residences only

## **Residential Window Diversity**

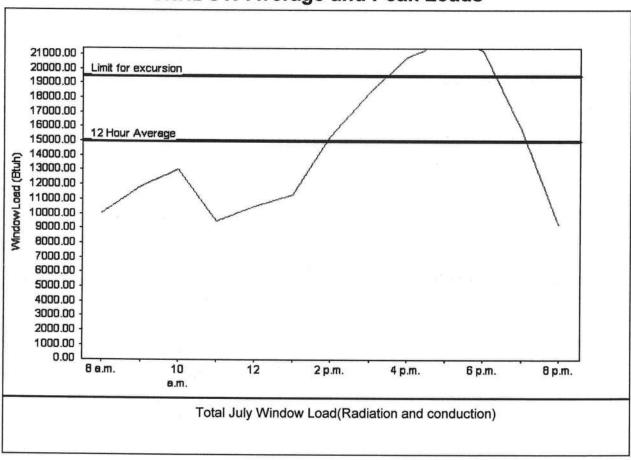
### MidSummer

Spec House Lake City, FL 32025Project Title: Compass Builders - Kezia Code Only Professional Version Climate: North

1/10/2008

Weather data for: Gainesville - Defaults					
Summer design temperature	92 F	Average window load for July	14983 Btu		
Summer setpoint	75 F	Peak window load for July	21892 Btu		
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	19478 Btu		
Latitude	29 North	Window excursion (July)	2414 Btuh		

### **WINDOW Average and Peak Loads**



Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida-residences only PREPARED BY:

DATE:

EnergyGauge® FLRCPB v4.5.2



## PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statutide approved products are listed online.

Category/Subcategory		approved products are listed online @ www. Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Jeld-Wen	Exterior Swinning door	FL- 49K-R1
B. SLIDING		11. 12. 11.14 000.	1- 170-21
C. SECTIONAL/ROLL UP	ROYNOF	Raynor	FL-4867
D. OTHER			1-2 -1/6/
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	MI- Products	Single Hung Window	FL-5108
B. HORIZONTAL SLIDER		7	1 2 3 3 3
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	James Hardie	Hardi Plant Siding	FL-889-RI
B. SOFFITS	Kaylan	Aluminum Soffit	FL - 4957
C. STOREFRONTS			- 1137
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	Elk Roofing	Asphalt shingles	FL-586-RZ
B. NON-STRUCT METAL		Jana San Maria	1-2 Jac-K2
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS	SIMPSON Strong to	Truss Straps	FL- 474-RI
B. WOOD ANCHORS		1	7/1-1/1
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
7 T			
6. NEW EXTERIOR			
ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobaite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE DATE

Prince Control

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

### ALL REQUIREMENTS ARE SUBJECT TO CHANGE **EFFECTIVE OCTOBER 1, 2005**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

## APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Applicant	Plans Ex	EMENTS: Two (2) complete sets of plans containing the following:
<b>Ø</b>	0	All drawings must be clear, concise and drawn to scale ("Optional"
Ì	0	Designers name and signature on descent of the control of the cont
1	0	Site Plan including:  a) Dimensions of lot
3		<ul> <li>b) Dimensions of building set backs</li> <li>c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.</li> </ul>
V		d) Provide a full legal description of property.  Wind-load Engineering Summary, calculations and any details required  Plans or specifications must state compliance with FBC Section 1609.  The following information must be shown as per section 1603.1.4 FBC  a. Basic wind speed (3-second must)
		<ul> <li>a. Basic wind speed (3-second gust), miles per hour (km/hr).</li> <li>b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.</li> </ul>
		<ul> <li>Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated.</li> <li>The applicable enclosure classifications and if their indicated.</li> </ul>
		e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the
	0	Elevations including: a) All sides
	0 0	b) Roof pitch     c) Overhang dimensions and detail with attic ventilation

4		d) Location, size and height above roof of chimneys.
9		e) Location and size of skylights
<b>q</b>		f) Building height
Ø		e) Number of stories
(		Floor Plan including:
P		a) Rooms labeled and dimensioned.
•		b) Shear walls identified.
	0	c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
<b>b</b>		d) Show safety glazing of glass, where required by code.
ф	0	e) Identify egress windows in bedrooms, and size.
d)		f) Fireplace (gas vented), (gas non-vented) or wood burning with
		hearth, (Please circle applicable type).
<b>P</b>	0	g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
		h) Must show and identify accessibility requirements (accessible bathroom)
		Foundation Plan including:
þ		<ul> <li>a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.</li> </ul>
	0	b) All posts and/or column footing including size and reinforcing
		c) Any special support required by soil analysis such as piling
Çi 💮		d) Location of any vertical steel.
}		Roof System:
d	O	a) Truss package including:
(		<ol> <li>Truss layout and truss details signed and sealed by FI. Pro. Eng.</li> <li>Roof assembly (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)</li> </ol>
(D)	0	b) Conventional Framing Layout including:
٧		Rafter size, species and spacing
		2. Attachment to wall and uplift
		<ol><li>Ridge beam sized and valley framing and support details</li></ol>
		<ol> <li>Roof assembly (FBC 106.1.1.2)Roofing systems, materials.</li> </ol>
		manufacturer, fastening requirements and product evaluation with
		wind resistance rating)
_		Wall Sections including:
		a) Masonry wall
		All materials making up wall
		2. Block size and mortar type with size and spacing of reinforcement
		3. Lintel, tie-beam sizes and reinforcement
		Gable ends with rake beams showing reinforcement or gable truss and wall bracing details  All required expectation with a 1/2 and a
		<ol><li>All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall b</li></ol>
		designed by a Windload engineer using the engineered roof truss plans.
		6. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with resistance rating)
		7. Fire resistant construction (if required)
		8. Fireproofing requirements
		<ol><li>Shoe type of termite treatment (termiticide or alternative method)</li></ol>
		10. Slab on grade
*		a. Vapor retarder (6mil. Polyethylene with joints langed 6

inches and sealed)

b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports

11. Indicate where pressure treated wood will be placed

12. Provide insulation R value for the following:

2

1		
	0	b) Wood frame wall
G		
		<ol> <li>All materials making up wall</li> <li>Size and species of studs</li> </ol>
		3. Sheathing size, type and nailing schedule
		4. Headers sized
		5. Gable end showing balloon framing detail or gable truss and wall
		hinge bracing detail
		6. All required fasteners for continuous tie from roof to foundation
		(truss anchors, straps, anchor bolts and washers) shall be designed
		by a Windload engineer using the engineered roof truss plans.
		7. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with wind resistance rating)
		Fire resistant construction (if applicable)     Fireproofing requirements
		10. Show type of termite treatment (termiticide or alternative method)
		11. Slab on grade
		a. Vapor retarder (6Mil. Polyethylene with joints lapped 6
		inches and sealed
		b. Must show control joints, synthetic fiber reinforcement or
		weided wire fabric reinforcement and supports
		<ol> <li>Indicate where pressure treated wood will be placed</li> </ol>
		13. Provide insulation R value for the following:
		a. Attic space
		b. Exterior wall cavity c. Crawl space (if applicable)
		c) Metal frame wall and roof (designed, signed and sealed by Florida Prof.
		Engineer or Architect)
-1		Floor Framing System:
P		a) Floor truss package including layout and details, signed and sealed by Florida
7	-	registered Professional Engineer
Ц	0	b) Floor joist size and spacing
4		c) Girder size and spacing
#	0	d) Attachment of joist to girder
		e) Wind load requirements where applicable
٣		Plumbing Fixture layout
4	п	Electrical layout including:
H	0	a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
	0	b) Ceiling fans c) Smoke detectors
h	0	
4	Ö	d) Service panel and sub-panel size and location(s)
ď	ō	e) Meter location with type of service entrance (overhead or underground)  f) Appliances and HVAC equipment
7		g) Arc Fault Circuits (AFCI) in bedrooms
d		h) Exhaust fans in bathroom
T	=	HVAC information
d)		a) Energy Calculations (dimensions shall match plans)
		b) Manual J sizing equipment or equivalent computation
ф		c)Gas System Type (LP or Natural) Location and BTU demand of equipment
		Disclosure Statement for Owner Builders
ď		*** Notice Of Commencement Required Before Any Inspections Will Be Done
0		Private Potable Water

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)



Project Information for:

L265562

Builder:

Compass Builders

Lot:

57-1

Subdivision:

Rolling Meadows

County: Truss Count:

Baker 45

Design Program: MiTek 20/20 6.3 **Building Code:** 

FBC2004/TPI2002

Truss Design Load Information: Gravity:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for the Structural Engineering:

Jacob C. Kirsch Florida License No. CBC1253775

Address: 196 Southwest Huntsview Way Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Notes:

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Drwg. #

J1925226

J1925227

J1925228

J1925229

J1925230

J1925231

J1925232

J1925233

J1925234

J1925235

J1925236

J1925237

J1925238

J1925239

- 1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
- 2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
- 3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 section 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Truss ID

T22

T23

T24

T25

T26

T27

T28

T29

T30

T31

T32

T33

T34

T35

Date

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

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1/10/08

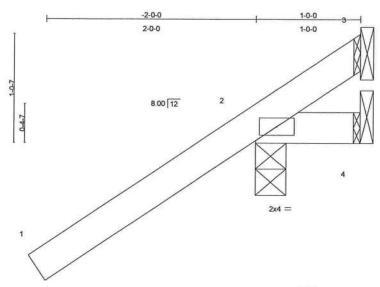
No.	Drwg. #	Truss ID	Date	No.
1	J1925195	CJ01	1/10/08	32
2	J1925196	CJ02	1/10/08	33
3	J1925197	CJ03	1/10/08	34
4	J1925198	EJ01	1/10/08	35
5	J1925199	EJ02	1/10/08	36
6	J1925200	EJ03	1/10/08	37
7	J1925201	HJ01	1/10/08	38
8	J1925202	HJ02	1/10/08	39
9	J1925203	P01	1/10/08	40
10	J1925204	P02	1/10/08	41
11	J1925205	T01	1/10/08	42
12	J1925206	T02	1/10/08	43
13	J1925207	T03	1/10/08	44
14	J1925208	T04	1/10/08	45
15	J1925209	T05	1/10/08	
16	J1925210	T06	1/10/08	
17	J1925211	T07	1/10/08	7
18	J1925212	T08	1/10/08	
19	J1925213	T09	1/10/08	
20	J1925214	T10	1/10/08	
21	J1925215	T11	1/10/08	
22	J1925216	T12	1/10/08	
23	J1925217	T13	1/10/08	
24	J1925218	T14	1/10/08	
25	J1925219	T15	1/10/08	
26	J1925220	T16	1/10/08	
27	J1925221	T17	1/10/08	
28	J1925222	T18	1/10/08	
29	J1925223	T19	1/10/08	
30	J1925224	T20	1/10/08	
31	J1925225	T21	1/10/08	



Ply Truss Truss Type Qty 0.0 J1925195 CJ01 JACK 6 1 Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:45 2008 Page 1



1	1-0-0	
	1-0-0	

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	OL 5.0 Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=270/0-3-8, 4=5/Mechanical, 3=-100/Mechanical

Max Horz 2=119(load case 6)

Max Uplift 2=-312(load case 6), 3=-100(load case 1)

Max Grav 2=270(load case 1), 4=14(load case 2), 3=161(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/61, 2-3=-90/111

**BOT CHORD** 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.32

#### **NOTES**

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II: Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 Ib uplift at joint 2 and 100 lb uplift at joint 3. Continued on page 2

January 10,2008

Scale = 1:10.4

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	5-11-12-2007 - 111-2007
	CJ01	JACK	6	1		J1925195
	1222.		37.4		Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:45 2008 Page 2

LOAD CASE(S) Standard

llus Lee use Design Engineer vida PE No. 34866 09 Coastel Bay Blvd, synton Beach, FL 33435

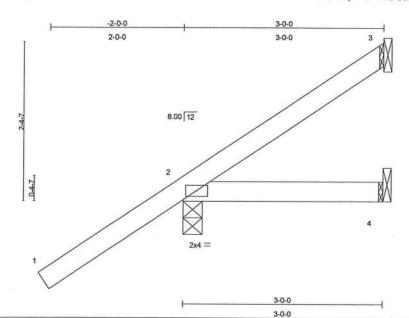
January 10,2008



Job Truss Truss Type Qty Ply 00 J1925196 CJ02 **JACK** 2 1 Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:46 2008 Page 1



LOADING (psf) SPACING 2-0-0 CSI DEFL L/d (loc) I/defl **PLATES** GRIP in TCLL 20.0 Plates Increase 1.25 TC 0.33 Vert(LL) -0.00 2-4 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.06 Vert(TL) -0.012-4 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.003 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 14 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=27/Mechanical, 2=258/0-3-8, 4=14/Mechanical

Max Horz 2=179(load case 6)

Max Uplift 3=-33(load case 7), 2=-207(load case 6)

Max Grav 3=34(load case 4), 2=258(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-72/19

**BOT CHORD** 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.27

#### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 207 lb uplift at joint 2. Continued on page 2

January 10,2008

Scale = 1:16.2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	CJ02	JACK	2	1		J1925196
		0.101			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:46 2008 Page 2

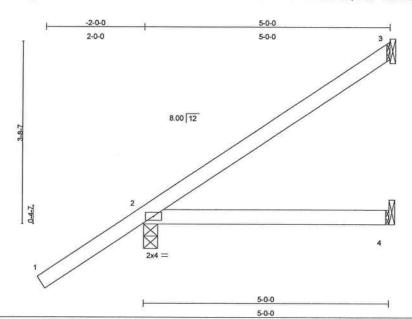
LOAD CASE(S) Standard

lius Lee 198 Design Engineer 196a PE No. 24868 00 Ceastel Bay Blvd 1910n Gesch, FL 33435



Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:46 2008 Page 1



LOADING (psf) **SPACING** 2-0-0 CSI DEFL (loc) L/d **PLATES** GRIP I/defl TCLL 20.0 Plates Increase 1.25 TC 0.33 Vert(LL) -0.032-4 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.16 Vert(TL) -0.052-4 >999 240 **BCLL** 10.0 WB Rep Stress Incr YES 0.00 Horz(TL) -0.003 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 20 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS

(lb/size) 3=101/Mechanical, 2=302/0-3-8, 4=24/Mechanical

Max Horz 2=240(load case 6)

Max Uplift 3=-103(load case 6), 2=-188(load case 6)

Max Grav 3=101(load case 1), 2=302(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-90/43

**BOT CHORD** 

2-4=0/0

# JOINT STRESS INDEX

2 = 0.30

#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 3 and 188 lb uplift at joint 2. Continued on page 2

Truss Design Engineer Florida PE No. 34868 1 199 Coastal Bay Blvd Boynton Beach, Ft 93435

January 10,2008

Scale = 1:22.1

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	6-856 NOVEMBER 18-10
	CJ03	JACK	2	1		J1925197
					Job Reference (optional)	

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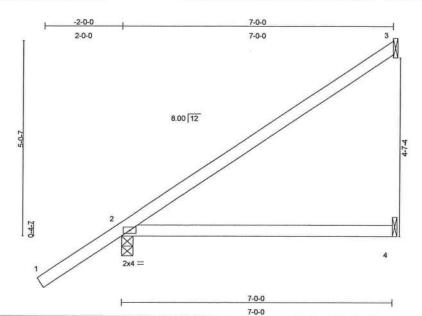
LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Plonda ME No. 34866 1100 Coestel Bay Blvd Bovotos Bases E.



Job	Truss	Truss Type	Qty	Ply	00	E-AVARA-GERGE
	EJ01	MONO TRUSS	13	1		J1925198
	Session .				Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.11	2-4	>730	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.20	2-4	>417	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)	1,1,5,1=1,5=4,					Weight: 27 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=164/Mechanical, 2=357/0-3-8, 4=34/Mechanical

Max Horz 2=218(load case 6)

Max Uplift 3=-110(load case 6), 2=-124(load case 6)

Max Grav 3=164(load case 1), 2=357(load case 1), 4=102(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-138/72

**BOT CHORD** 2-4=0/0

# JOINT STRESS INDEX

2 = 0.34

### **NOTES**

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 3 and 124 lb uplift at joint 2.

January 10,2008

Scale = 1:28.0

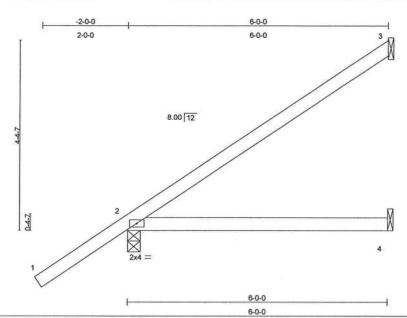
# LOAD CASE(S) Standard



Qty Job Ply 00 Truss Truss Type J1925199 **EJ02** MONO TRUSS Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.06	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.10	2-4	>673	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 24 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=133/Mechanical, 2=329/0-3-8, 4=29/Mechanical

Max Horz 2=271(load case 6)

Max Uplift 3=-139(load case 6), 2=-187(load case 6)

Max Grav 3=133(load case 1), 2=329(load case 1), 4=87(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-114/58

BOT CHORD 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.32

# NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 Ib uplift at joint 3 and 187 lb uplift at joint 2. Continued on page 2

January 10,2008

Scale = 1:25.0

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	EJ02	MONO TRUSS	4	1		J1925199
					Job Reference (optional)	

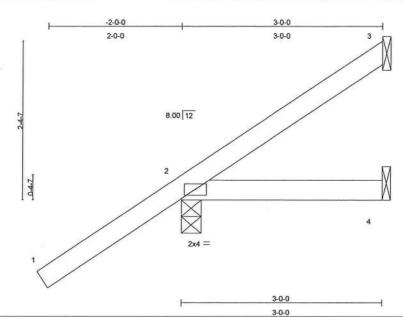
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:48 2008 Page 2

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
	EJ03	MONO TRUSS	ا			J1925200
	E303	MONO TROSS	4	'	Job Reference (optional)	

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LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) L/d **PLATES** in I/defl **GRIP** TCLL 20.0 Plates Increase 1.25 TC 0.33 Vert(LL) -0.00 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.06 Vert(TL) -0.012-4 >999 240 **BCLL** 0.00 -0.00 10.0 Rep Stress Incr YES WB Horz(TL) 3 n/a n/a BCDL Code FBC2004/TPI2002 (Matrix) Weight: 14 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

REACTIONS (lb/size) 3=27/Mechanical, 2=258/0-3-8, 4=14/Mechanical

Max Horz 2=179(load case 6)

Max Uplift 3=-33(load case 7), 2=-207(load case 6)

Max Grav 3=34(load case 4), 2=258(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-72/19

BOT CHORD 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.27

# NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 207 lb uplift at joint 2. Continued on page 2

January 10,2008

Scale = 1:16.2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	EJ03	MONO TRUSS	4	1		J1925200
	-3.7.2				Job Reference (optional)	

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LOAD CASE(S) Standard



Job Truss Truss Type Qty Ply 00 J1925201 **HJ01** MONO TRUSS 1 Job Reference (optional) Builders First Source, Jacksonville ,Florida 32244 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:49 2008 Page 1 8-5-13 2-9-15 4-0-0 4-5-13 Scale = 1:24.9 3x6 = 5.66 12 D-4-Z 7 2x4 || 3x6 = 4-0-0 8-5-13 4-0-0 4-5-13 LOADING (psf) SPACING 2-0-0 CSI DEFL L/d **PLATES** (loc) I/defl GRIP in TCLL 20.0 Plates Increase 1.25 TC 0.55 Vert(LL) -0.01 6-7 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.19 Vert(TL) -0.046-7 >999 240 BCLL 10.0 Rep Stress Incr NO WB 0.12 Horz(TL) 0.00 5 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 40 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins. **WEBS** 2 X 4 SYP No.3 Rigid ceiling directly applied or 10-0-0 oc **BOT CHORD** 

bracing.

REACTIONS (lb/size) 4=194/Mechanical, 2=392/0-4-15, 5=150/Mechanical

Max Horz 2=285(load case 5)

Max Uplift 4=-196(load case 5), 2=-231(load case 5), 5=-38(load case 5)

Max Grav 4=194(load case 1), 2=392(load case 1), 5=163(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/65, 2-3=-338/0, 3-4=-108/62

BOT CHORD

2-7=-132/265, 6-7=-132/265, 5-6=0/0

**WEBS** 

3-6=-292/146, 3-7=0/162

### JOINT STRESS INDEX

2 = 0.47, 3 = 0.12, 6 = 0.08 and 7 = 0.11

#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 4, 231 lb uplift at joint 2 and 38 lb uplift at joint 5.

Truse Design Engineer Florida PE No. 24888 1 100 Ceastal Bay Blvd Boynton Besch, FL 33435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	HJ01	MONO TRUSS	1	1		J1925201
	1				Job Reference (optional)	

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#### **NOTES**

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=26, B=26)-to-4=-115(F=-30, B=-30), 2=0(F=5, B=5)-to-5=-21(F=-6, B=-6)

Julius Les Truss Design Engineer Plonde PE No. 24868 1 109 Coestel Bay Blvd



Job Truss Truss Type Qty Ply 00 J1925202 HJ02 **JACK** 2 Job Reference (optional) Builders First Source, Jacksonville ,Florida 32244 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:49 2008 Page 1 4-2-15 2-9-15 4-2-15 3 Scale: 3/4"=1" 5.66 12 0.4-7 2x4 = 4-2-15 4-2-15 LOADING (psf) **SPACING** 2-0-0 CSI DEFL L/d in (loc) I/defl **PLATES GRIP** TCLL 20.0 Plates Increase 1.25 TC 0.55 Vert(LL) -0.01 2-4 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.10 Vert(TL) -0.022-4 >999 240 BCLL 10.0 Rep Stress Incr NO WB 0.00 Horz(TL) -0.003 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 18 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 4-2-15 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=13/Mechanical, 2=281/0-4-15, 4=14/Mechanical

Max Horz 2=127(load case 5)

Max Uplift 3=-4(load case 6), 2=-252(load case 5)

Max Grav 3=45(load case 3), 2=281(load case 1), 4=54(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/65, 2-3=-49/19

2-4=0/0 **BOT CHORD** 

JOINT STRESS INDEX

### 2 = 0.28

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.

2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 3 and 252 lb uplift at joint 2.

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). Continued on page 2

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters and NEAD MOTES OF THIS ARD INCLUDED MIT LET REPERENCE FACE MITTA'S BEFORE USE.

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. 

Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building occur. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	HJ02	JACK	2	1		J1925202
		FE 52 F 2	1000 pt		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:49 2008 Page 2

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54 Trapezoidal Loads (plf)

Vert: 2=-3(F=26, B=26)-to-3=-57(F=-2, B=-2), 2=0(F=5, B=5)-to-4=-11(F=-0, B=-0)

Julius Lee Truse Design Engineer Florida PE No. 24869 1109 Coestal Bay Blvd Boynton Beach, Ft. 93436



Job Truss Ply Truss Type Qty 0.0 J1925203 P01 HIP PIGGYBACK 1 1 Job Reference (optional) Builders First Source, Jacksonville ,Florida 32244 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:50 2008 Page 1 2-0-0 3-4-0 2-0-0 1-4-0 2-0-0 Scale = 1:11.4 3x6 = 4 2x4 || 8.00 12 2x4 = 2x4 || 2x4 = 5-4-0 LOADING (psf) SPACING 2-0-0 CSI DEFL L/d **PLATES** in (loc) I/defl GRIP 20.0 1.25 TCLL Plates Increase TC 0.04 Vert(LL) n/a 999 MT20 244/190 n/a TCDL 7.0 Lumber Increase 1.25 BC 0.03 Vert(TL) 999 n/a n/a **BCLL** 10.0 Rep Stress Incr YES WB 0.02 Horz(TL) 0.00 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 16 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 5-4-0 oc purlins. **WEBS** 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(lb/size) 1=-3/5-4-0, 7=-3/5-4-0, 2=111/5-4-0, 6=111/5-4-0, 8=95/5-4-0

Max Horz 1=-33(load case 4)

Max Uplift 1=-44(load case 4), 7=-28(load case 11), 2=-79(load case 5), 6=-63(load

case 4), 8=-16(load case 5)

Max Grav 1=51(load case 5), 7=31(load case 4), 2=138(load case 10), 6=138(load case 11), 8=95(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-53/54, 2-3=-69/52, 3-4=-36/59, 4-5=-36/59, 5-6=-69/52, 6-7=-15/30

BOT CHORD 2-8=-14/36, 6-8=-14/36

WEBS 4-8=-71/69

# JOINT STRESS INDEX

2 = 0.14, 3 = 0.02, 4 = 0.04, 5 = 0.02, 6 = 0.14 and 8 = 0.04

# NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

1 100 Caastal Bay Boynton Beach, Fi

Provide adequate drainage to prevent water ponding.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	0.0	
	P01	HIP PIGGYBACK	1	1	.4	J1925203
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:50 2008 Page 2

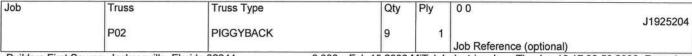
#### NOTES

- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 1, 28 lb uplift at joint 7, 79 lb uplift at joint 2, 63 lb uplift at joint 6 and 16 lb uplift at joint 8.
- 8) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

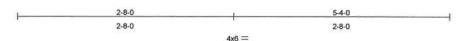
LOAD CASE(S) Standard

Julius Lee Truse Cesign Engineer Florida ME No. 34868 1169 Cesstal Bay Blvd Boynton Beach, Ft. 33436

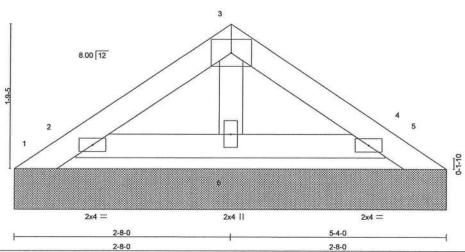




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Scale = 1:13.4



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.01	Horz(TL)	0.00	4	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TI	212002	(Mat	rix)	, ,					Weight: 17 lb	

# LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

# BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-4-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

#### REACTIONS

(lb/size) 1=-25/5-4-0, 5=-25/5-4-0, 2=133/5-4-0, 4=133/5-4-0, 6=92/5-4-0

Max Horz 1=-45(load case 4)

Max Uplift 1=-29(load case 4), 5=-25(load case 1), 2=-69(load case 6), 4=-62(load

case 7)

Max Grav 1=43(load case 5), 5=22(load case 7), 2=133(load case 1), 4=133(load case 1), 6=92(load case 1)

# FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-59/57, 2-3=-48/34, 3-4=-48/34, 4-5=-18/28

BOT CHORD 2-6=-8/29, 4-6=-8/29

WEBS

3-6=-68/29

### **JOINT STRESS INDEX**

2 = 0.13, 3 = 0.02, 4 = 0.13 and 6 = 0.02

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Truse Design Engineer Florida PE No. 34869 1100 Castel Bay Blyd Boynton Beach, FL 33431

 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	P02	PIGGYBACK	9	1		J1925204
		1.100.101.01			Job Reference (optional)	

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#### **NOTES**

4) Gable requires continuous bottom chord bearing.

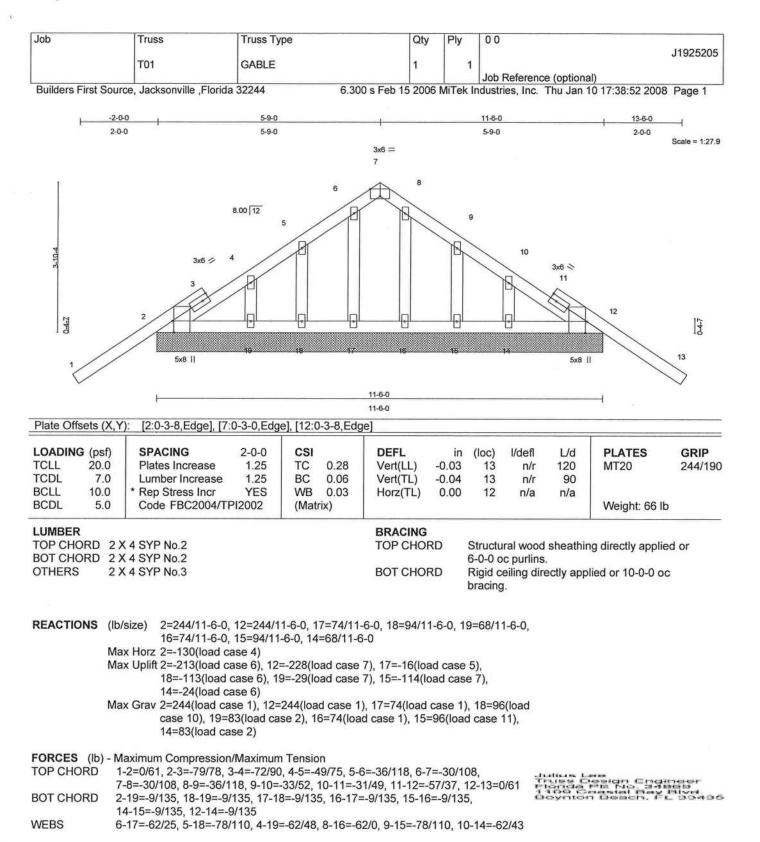
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1, 25 lb uplift at joint 5, 69 lb uplift at joint 2 and 62 lb uplift at joint 4.
- 7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Florida FE No. 24866 1109 Coestel Bay Blvd Boynton Beach, FL 33436





# JOINT STRESS INDEX

2 = 0.45, 3 = 0.00, 3 = 0.16, 4 = 0.03, 5 = 0.06, 6 = 0.03, 7 = 0.13, 8 = 0.03, 9 = 0.06, 10 = 0.03, 11 = 0.00, 11 = 0.16, 12 = 0.45, 14 = 0.03, 15 = 0.06, 16 = 0.02, 17 = 0.02, 18 = 0.06 and 19 = 0.03

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T01	GABLE	1	1		J1925205
	1.7.		100		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:52 2008 Page 2

#### NOTES

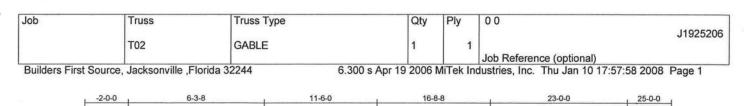
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 2, 228 lb uplift at joint 12, 16 lb uplift at joint 17, 113 lb uplift at joint 18, 29 lb uplift at joint 19, 114 lb uplift at joint 15 and 24 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Lee Fruse Cesian Engineer Flonds PE No. 34868 I 100 Cestiel Bay Blvd Bovnton Besch. FL 93495





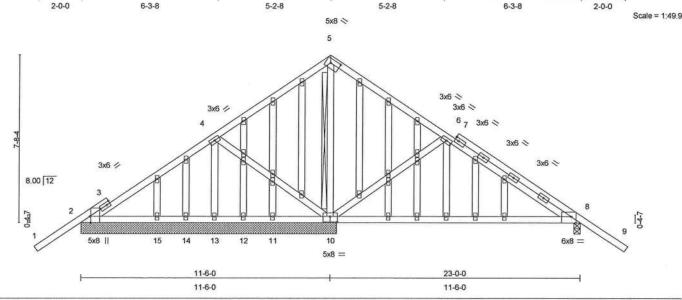


Plate Of	fsets (X,Y):	[2:0-3-8,Edge], [5:0	-5-4,0-2-4],	[8:0-2-9	,Edge], [1	0:0-4-0,0-3-0]						
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.24	8-10	>579	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.41	8-10	>331	240	(Cassinality	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.44	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		1 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 -					Weight: 191 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

# BRACING

TOP CHORD

**WEBS** 

**BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing. 2 X 4 SYP No.3 - 5-10 T-Brace:

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=656/11-9-8, 10=1822/11-9-8, 8=400/0-3-8, 11=-48/11-9-8, 12=38/11-9-8, 13=34/11-9-8, 14=-92/11-9-8, 15=195/11-9-8

Max Horz 2=263(load case 5)

Max Uplift 2=-405(load case 6), 10=-843(load case 6), 8=-276(load case 7), 11=-150(load case 2), 13=-12(load case 7), 14=-92(load case 1), 15=-81(load case 7)

Max Grav 2=675(load case 10), 10=1822(load case 1), 8=457(load case 11), 12=133(load case 2), 13=34(load case 2), 14=64(load case 7), 15=195(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-24/128, 2-3=-403/155, 3-4=-256/145, 4-5=-152/510, 5-6=-150/550, 6-7=-172/154,

7-8=-349/158, 8-9=0/62

**BOT CHORD** 2-15=-166/213, 14-15=-166/213, 13-14=-166/213, 12-13=-166/213, 11-12=-166/213,

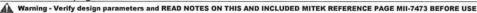
10-11=-166/213, 8-10=-36/257

4-10=-581/434, 5-10=-1061/484, 6-10=-563/405

January 10,2008

Continued on page 2

**WEBS** 



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connec Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erecti and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T02	GABLE	1	1		J1925206
	17.7.55				Job Reference (optional)	

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#### JOINT STRESS INDEX

 $2 = 0.81, \ 3 = 0.00, \ 3 = 0.52, \ 4 = 0.39, \ 5 = 0.99, \ 6 = 0.39, \ 7 = 0.00, \ 7 = 0.33, \ 7 = 0.33, \ 7 = 0.33, \ 7 = 0.33, \ 8 = 0.57, \ 10 = 0.84, \ 11 = 0.34, \ 12 = 0.34, \ 13 = 0.34, \ 14 = 0.34, \ 15 = 0.34, \ 16 = 0.34, \ 17 = 0.34, \ 18 = 0.34, \ 18 = 0.34, \ 19 = 0.34, \ 20 = 0.34, \ 20 = 0.34, \ 21 = 0.34, \ 22 = 0.34, \ 23 = 0.34, \ 24 = 0.34, \ 25 = 0.34, \ 26 = 0.34, \ 27 = 0.34, \ 28 = 0.34, \ 29 = 0.34, \ 30 = 0.34, \ 31 = 0.34, \ 31 = 0.34, \ 31 = 0.34, \ 32 = 0.34, \ 33 = 0.34, \ 34 = 0.00, \ 34 = 0.34, \ 35 = 0.34, \ 36 = 0.00 \ \text{and} \ 36 = 0.34$ 

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 405 lb uplift at joint 2, 843 lb uplift at joint 10, 276 lb uplift at joint 8, 150 lb uplift at joint 11, 12 lb uplift at joint 13, 92 lb uplift at joint 14 and 81 lb uplift at joint 15.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

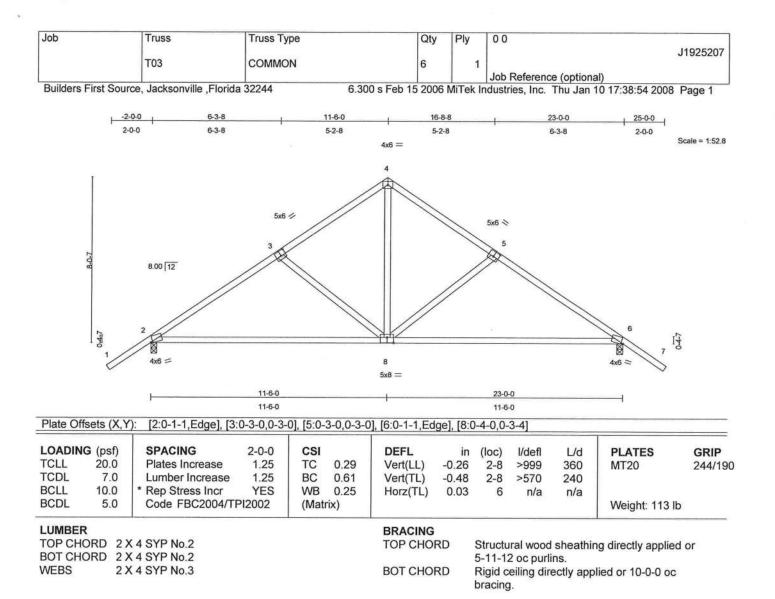
## LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-114(F=-60), 5-7=-141(F=-87), 7-9=-54, 2-8=-10

Julius Les Truss Design Engineer Florida FE No. 24865 1100 Coastal Bay Blvd





REACTIONS (lb/size) 2=847/0-3-8, 6=847/0-3-8

Max Horz 2=-212(load case 4)

Max Uplift 2=-251(load case 6), 6=-251(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1011/440, 3-4=-744/390, 4-5=-744/390, 5-6=-1011/440, 6-7=0/62

BOT CHORD 2-8=-162/761, 6-8=-162/761

WEBS 3-8=-286/252, 4-8=-237/511, 5-8=-286/252

#### JOINT STRESS INDEX

2 = 0.83, 3 = 0.41, 4 = 0.43, 5 = 0.41, 6 = 0.83 and 8 = 0.82

# NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

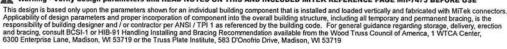
 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Les Truss Design Engineer Florida FE No. 24869 1109 Coastal Bay Blvd Boynton Beach, Ft. 20425

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





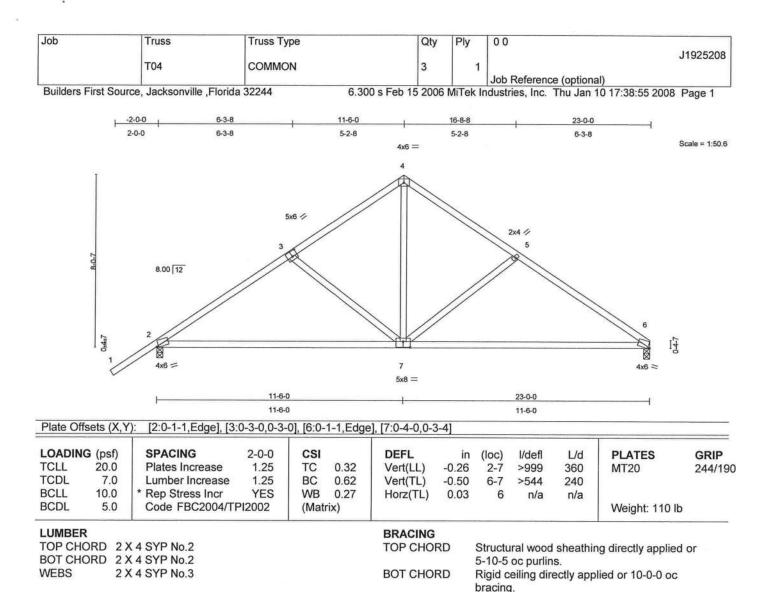
Job	Truss	Truss Type	Qty	Ply	00	
	тоз	COMMON	6	1		J1925207
					Job Reference (optional)	

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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 251 lb uplift at joint 6.

LOAD CASE(S) Standard





REACTIONS (lb/size) 6=721/0-3-8, 2=853/0-3-8

Max Horz 2=239(load case 5)

Max Uplift 6=-151(load case 7), 2=-252(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1023/458, 3-4=-756/409, 4-5=-760/411, 5-6=-1015/472

BOT CHORD 2-7=-265/770, 6-7=-291/788

WEBS 3-7=-286/251, 4-7=-263/515, 5-7=-306/281

# JOINT STRESS INDEX

2 = 0.83, 3 = 0.41, 4 = 0.42, 5 = 0.33, 6 = 0.83 and 7 = 0.83

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

Builders

Job	Truss	Truss Type	Qty	Ply	00	
	T04	COMMON	3	1		J1925208
	104	COMMON			Job Reference (optional)	

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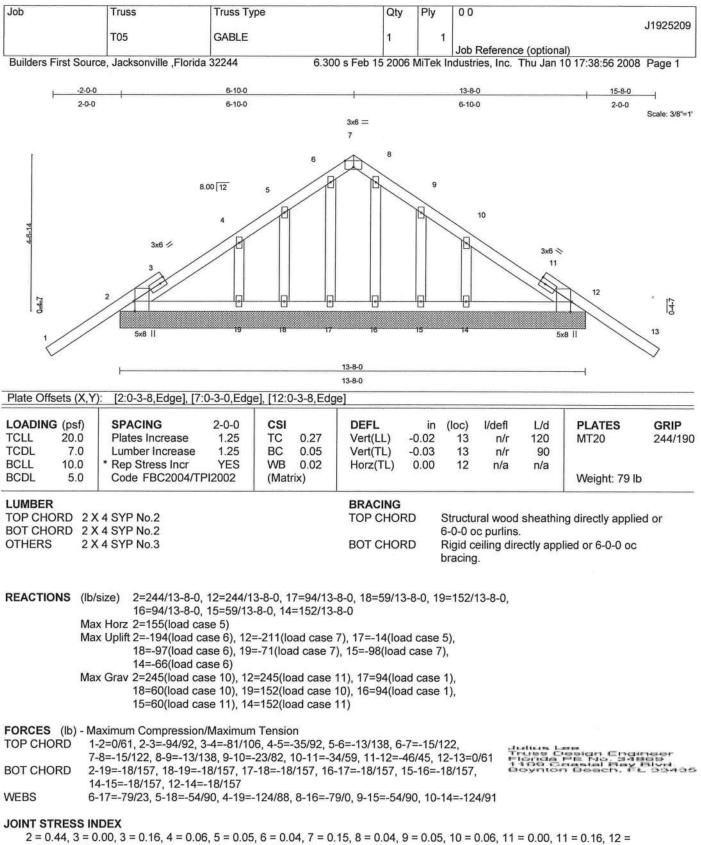
### NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 151 lb uplift at joint 6 and 252 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truss Cesign Engineer Flonda PE No. 24869 1189 Cesstel Bey Blvd





2 = 0.44, 3 = 0.00, 3 = 0.16, 4 = 0.06, 5 = 0.05, 6 = 0.04, 7 = 0.15, 8 = 0.04, 9 = 0.05, 10 = 0.06, 11 = 0.00, 11 = 0.16, 12 = 0.44, 14 = 0.05, 15 = 0.05, 16 = 0.03, 17 = 0.03, 18 = 0.05 and 19 = 0.05

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T05	GABLE	1	1		J1925209
					Job Reference (optional)	

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### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 194 lb uplift at joint 2, 211 lb uplift at joint 12, 14 lb uplift at joint 17, 97 lb uplift at joint 18, 71 lb uplift at joint 19, 98 lb uplift at joint 15 and 66 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonda PE No. 24869 1100 Coastal Bay Blyd Boynton Beach, Et. 33436



Job	Truss	Truss Type	Qty	Ply	00	#15/m #4.000 m 15/15/15/15/1
	T06	GABLE	1	1		J1925210
					Job Reference (optional)	

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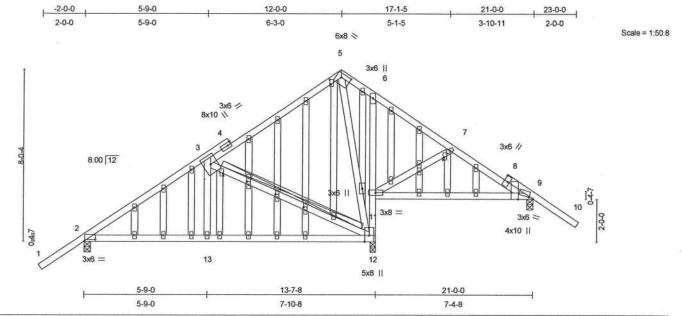


Plate Offsets (X,Y): [2:0-6-3,0-0-10], [5:0-5-4,0-2-4], [7:0-1-8,0-0-12], [8:Edge,0-2-0], [9:0-2-6,0-0-3], [9:0-0-4,Edge], [25:0-2-11,0-0-12], [26:0-1-12,0-1-0], [29:0-1-12,0-1-0], [32:0-1-12,0-1-0], [36:0-1-8,0-0-4]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	0.10	9-11	>857	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.14	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.71	Horz(TL)	0.02	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 194 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

6-12 2 X 4 SYP No.3

**WEBS** 

**OTHERS** 

2 X 4 SYP No.3

2 X 4 SYP No.3

**BOT CHORD** 

**WEBS** 

BRACING

TOP CHORD

Structural wood sheathing directly applied or 5-7-8

oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

T-Brace: 2 X 4 SYP No.3 - 3-12

Fasten T and I braces to narrow edge of web with

10d Common wire nails, 9in o.c., with 4in minimum

end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=687/0-3-8, 9=731/0-3-8, 12=1295/0-3-0

Max Horz 2=266(load case 5)

Max Uplift 2=-452(load case 6), 9=-757(load case 7), 12=-894(load case 6)

Max Grav 2=687(load case 1), 9=734(load case 11), 12=1295(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-836/628, 3-4=-301/371, 4-5=-148/314, 5-6=-181/226, 6-7=-132/278,

7-8=-322/490, 8-9=-502/597, 9-10=-80/131

**BOT CHORD** 2-13=-452/650, 12-13=-453/649, 11-12=-382/467, 6-11=-152/115, 9-11=-329/338

**WEBS** 3-13=0/217, 3-12=-585/506, 7-11=-383/483, 5-12=-701/590

#### JOINT STRESS INDEX

2 = 0.63, 3 = 0.62, 4 = 0.00, 4 = 0.52, 5 = 1.00, 6 = 0.25, 7 = 0.34, 7 = 0.66, 8 = 0.00, 8 = 0.68, 9 = 0.88, 9 = 0.57, 11 = 0.88, 12 = 0.30, 12 = 0.30, 13 = 0.68, 13 = 0.68, 14 = 0.88, 1513 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.16, 23 = 0.34, 24 = 0.34, 24 = 0.34, 25 = 0.340.34, 25 = 0.71, 26 = 0.40, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.40, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.40, 32 = 0.34, 33 = 0.34, 33 = 0.3434 = 0.34, 35 = 0.34, 36 = 0.46, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.34 and 42 = 0.34

January 10,2008

Continued on page 2

⚠ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connec Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	Т06	GABLE	1	1		J1925210
	10708			17	Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:58:55 2008 Page 2

#### **NOTES**

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 452 lb uplift at joint 2, 757 lb uplift at joint 9 and 894 lb uplift at joint 12.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

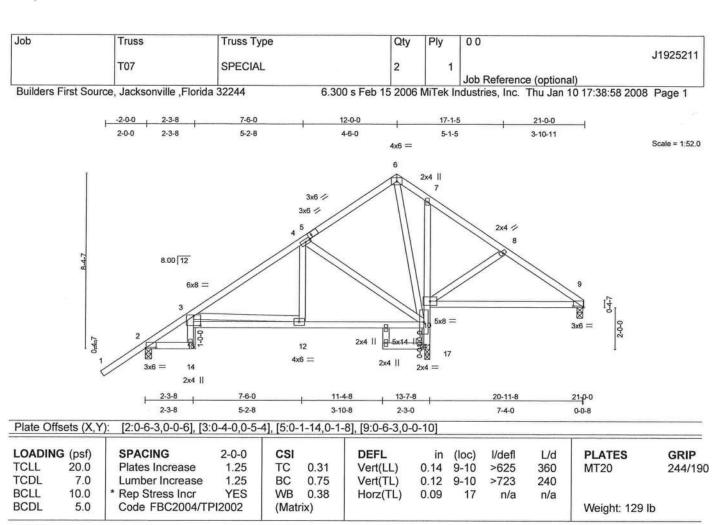
# LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-141(F=-87), 5-6=-141(F=-87), 6-10=-114(F=-60), 2-12=-10, 9-11=-10

Julius Les Truse Design Endineer Florida PE No. 34866 I 100 Geastal Bay Blvd





LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

3-14 2 X 4 SYP No.3, 15-16 2 X 4 SYP No.3

7-17 2 X 4 SYP No.3

2 X 4 SYP No.3

WEBS 2 X

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/siz

(lb/size) 9=79/0-3-8, 2=470/0-3-8, 17=896/0-3-0

Max Horz 2=246(load case 5)

Max Uplift 9=-180(load case 4), 2=-152(load case 6), 17=-343(load case 6) Max Grav 9=160(load case 11), 2=473(load case 10), 17=896(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-434/56, 3-4=-350/69, 4-5=-153/115, 5-6=-151/197, 6-7=-173/238,

7-8=-153/290, 8-9=-175/191

BOT CHORD 2-14=-106/276, 13-14=-4/66, 3-13=0/118, 12-13=-310/880, 11-12=-46/232,

11-17=-896/698, 10-11=-323/424, 7-10=-158/81, 9-10=-139/124

WEBS 4-12=-6/252, 3-12=-650/292, 8-10=-204/256, 4-11=-417/266, 6-11=-334/222

Julius Lee Truss Cesign Engineer Florida PE No. 24869 1189 Ceastal Bay Blvd

# JOINT STRESS INDEX

2 = 0.64, 3 = 0.62, 4 = 0.41, 5 = 0.63, 6 = 0.27, 7 = 0.37, 8 = 0.33, 9 = 0.61, 10 = 0.62, 11 = 0.24, 12 = 0.24, 13 = 0.00, 14 = 0.72, 15 = 0.33, 16 = 0.33 and 17 = 0.33

#### NOTES

1) Unbalanced roof live loads have been considered for this design. Continued on page 2

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ocd. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	Т07	SPECIAL	2	1		J1925211
	15.00,000,000				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:58 2008 Page 2

#### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 9, 152 lb uplift at joint 2 and 343 lb uplift at joint 17.

LOAD CASE(S) Standard

Julius Law Truss Design Engineer Flonda PE No. 34886 1109 Coasial Bay filvri Bovoton Beach, FL 33435

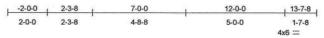


 Job
 Truss
 Truss Type
 Qty
 Ply
 0 0
 J1925212

 T08
 SPECIAL
 3
 1
 Job Reference (optional)

Builders First Source, Jacksonville .Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:59 2008 Page 1



Scale = 1:54.6

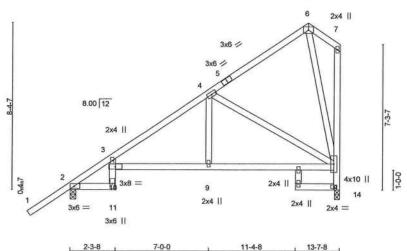


Plate Of	fsets (X,Y	'): [2:0-3-9,0-1-8]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	0.18	9-10	>897	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.23	9-10	>691	240	Line Control of Control	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.12	14	n/a	n/a		
<b>BCDL</b>	BCDL 5.0 Code FBC2004/TPI2002		(Mat	rix)						Weight: 90 lb		

4-8-8

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

2 A 4 STP NO.2 Except

3-11 2 X 4 SYP No.1D, 12-13 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 \*Except\*

7-14 2 X 4 SYP No.2

BRACING

TOP CHORD

**BOT CHORD** 

4-4-8

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 9-10-15 oc

bracing.

2-3-0

REACTIONS (lb/size) 2=557/0-3-8, 14=417/0-3-0

Max Horz 2=306(load case 6)

Max Uplift 2=-155(load case 6), 14=-157(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-569/22, 3-4=-555/160, 4-5=-131/12, 5-6=-104/33, 6-7=-27/44,

2-3-8

8-14=-417/273, 7-8=-41/30

BOT CHORD 2-11=-243/374, 10-11=-12/72, 3-10=-18/89, 9-10=-325/444, 8-9=-325/444

WEBS 4-9=-23/254, 4-8=-471/346, 6-8=-169/152

# JOINT STRESS INDEX

2 = 0.55, 3 = 0.66, 4 = 0.19, 5 = 0.23, 6 = 0.27, 7 = 0.17, 8 = 0.34, 9 = 0.18, 10 = 0.75, 11 = 0.28, 12 = 0.00, 13 = 0.00 and 14 = 0.00

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This Coffiling dasigned for C-C for members and forces, and for MWFRS for reactions specified.

Truss Design Engineer Flonds PE No. 34869 1109 Castel Bay Blvd. Boynton Beach, Ft. 35455

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	Т08	SPECIAL	3	1		J1925212
					Job Reference (optional)	

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### NOTES

- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 2 and 157 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonda PE No. 34888 1 100 Caestel Bay Blyd Boynton Beach, FL 33435



Job	Truss	Truss Type	Qty	Ply	00	
	Т09	SPECIAL	3	1		J1925213
	1000			12.	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:00 2008 Page 1

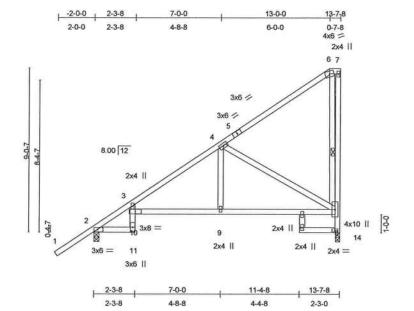


Plate Of	fsets (X,Y	(): [2:0-3-9,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.18	9-10	>898	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.23	9-10	>706	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.48	Horz(TL)	0.12	14	n/a	n/a		
BCDL	BCDL 5.0 Code FBC2004/TPI2002		(Mat	rix)						Weight: 93 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

3-11 2 X 4 SYP No.1D, 12-13 2 X 4 SYP No.3

**WEBS** 

2 X 4 SYP No.3 \*Except\*

7-14 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 9-7-10 oc

bracing. **WEBS** 1 Row at midpt

7-14

REACTIONS

(lb/size) 2=557/0-3-8, 14=417/0-3-0

Max Horz 2=348(load case 6)

Max Uplift 2=-131(load case 6), 14=-193(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-568/0, 3-4=-558/89, 4-5=-128/0, 5-6=-73/52, 8-14=-417/322,

7-8=-199/159, 6-7=-10/6

BOT CHORD 2-11=-257/373, 10-11=-9/72, 3-10=-15/84, 9-10=-346/450, 8-9=-346/450

**WEBS** 4-9=-20/255, 4-8=-491/373, 6-8=-314/362

# JOINT STRESS INDEX

2 = 0.53, 3 = 0.66, 4 = 0.19, 5 = 0.26, 6 = 0.52, 7 = 0.32, 8 = 0.32, 9 = 0.18, 10 = 0.73, 11 = 0.28, 12 = 0.00, 13 = 0.00 and 14 = 0.28, 12 = 0.00, 13 = 0.00 and 14 = 0.28, 12 = 0.00, 13 = 0.00, 13 = 0.00, 14 = 0.00, 15 == 0.00

#### NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

& hravide adeauste drainage to prevent water ponding.

January 10,2008

Scale = 1:59.9

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	Т09	SPECIAL	3	1		J1925213
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:00 2008 Page 2

#### NOTES

- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 2 and 193 lb uplift at joint 14.

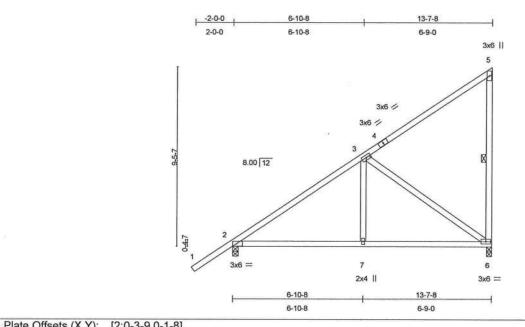
LOAD CASE(S) Standard

Julius Lee Trues Cesign Engineer Florida PE No. 34969 1189 Ceastel Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00	
	T10	MONO TRUSS	6	1		J1925214
					Job Reference (optional)	

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Plates Increase	4.05			DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
	1.25	TC	0.33	Vert(LL)	-0.04	2-7	>999	360	MT20	244/19
Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.08	2-7	>999	240	Constant And Rest Material	
* Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.01	6	n/a	n/a		
BCDL 5.0 Code FBC2004/TPI2002		(Mat	rix)						Weight: 80 lb	Ē
				BRACING						
	* Rep Stress Incr	* Rep Stress Incr YES Code FBC2004/TPI2002	* Rep Stress Incr YES WB Code FBC2004/TPI2002 (Mat	* Rep Stress Incr YES WB 0.47 Code FBC2004/TPI2002 (Matrix)	* Rep Stress Incr YES WB 0.47 Code FBC2004/TPI2002 (Matrix) Horz(TL)	* Rep Stress Incr YES WB 0.47 Code FBC2004/TPI2002 WB 0.47 (Matrix) Horz(TL) 0.01	* Rep Stress Incr YES WB 0.47 Horz(TL) 0.01 6 Code FBC2004/TPI2002 (Matrix)  BRACING	* Rep Stress Incr YES WB 0.47 Code FBC2004/TPI2002 WB 0.47 (Matrix) Horz(TL) 0.01 6 n/a	* Rep Stress Incr YES WB 0.47 Code FBC2004/TPI2002 WB 0.47 (Matrix)  **BRACING**  **Rep Stress Incr YES WB 0.47 (Matrix)  **BRACING**  **BRACING**	* Rep Stress Incr YES WB 0.47 Horz(TL) 0.01 6 n/a n/a Weight: 80 lb  BRACING

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

**BOT CHORD** 

6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

**WEBS** 

1 Row at midpt

5-6

REACTIONS (lb/size) 6=417/0-3-0, 2=557/0-3-8

Max Horz 2=358(load case 6)

Max Uplift 6=-205(load case 6), 2=-125(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-525/0, 3-4=-129/0, 4-5=-90/69, 5-6=-146/156

**BOT CHORD** 

2-7=-271/354, 6-7=-271/354

**WEBS** 

3-7=0/229, 3-6=-414/316

### JOINT STRESS INDEX

2 = 0.41, 3 = 0.17, 4 = 0.21, 5 = 0.30, 6 = 0.22 and 7 = 0.16

### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

Scale = 1:57.2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	T10	MONO TRUSS	6	1		J1925214
	10000				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:00 2008 Page 2

#### NOTES

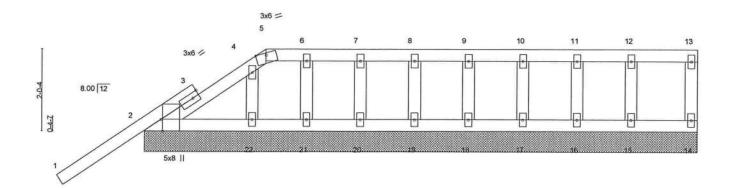
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 205 lb uplift at joint 6 and 125 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00
					J1925215
	T11	GABLE	1	1	
					Job Reference (optional)
<b>Builders First</b>	t Source, Jacksonville,	Florida 32244	6.300 s Feb 15 2006	MiTek II	ndustries, Inc. Thu Jan 10 17:39:01 2008 Page 1





						13-7-8	3					
Plate O	ffsets (X,Y	): [2:0-3-8,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.00	ìí	n/r	120	MT20	244/190
TCDI	7.0	Lumber Ingrases	1 25	DC.	0.04	Mod/TIN	0.01	4	1-	00	11.00	

13-7-8

LUMBER	0.00	V 4 0VP N 0				BRACING					w	
BCDL	5.0	Code FBC2004/TF	P12002	(Mat	rix)						Weight: 66 lb	6
BCLL	10.0	* Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.00	14	n/a	n/a		
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.01	1	n/r	90	100000000	
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.00	1	n/r	120	MT20	244/190

BOT CHORD WEBS	2 X 4 SYP No.2 2 X 4 SYP No.2 2 X 4 SYP No.3 2 X 4 SYP No.3	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing
OTHERS	2 X 4 SYP No.3	bracing.

**REACTIONS** (lb/size) 14=45/13-7-8, 2=241/13-7-8, 22=84/13-7-8, 21=86/13-7-8, 20=85/13-7-8, 19=85/13-7-8, 18=85/13-7-8, 17=86/13-7-8, 16=83/13-7-8, 15=94/13-7-8

Max Horz 2=166(load case 6)

Max Uplift 14=-30(load case 4), 2=-211(load case 6), 22=-25(load case 7), 21=-73(load case 4), 20=-49(load case 4), 19=-54(load case 4), 18=-53(load case 4), 17=-53(load case 4), 16=-53(load case 4), 15=-55(load case 4)

Max Grav 14=45(load case 1), 2=241(load case 1), 22=94(load case 2), 21=86(load case 1), 20=85(load case 1), 19=85(load case 1), 18=85(load case 1), 17=86(load case 1), 16=83(load case 1), 15=94(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-48/12, 3-4=-24/24, 4-5=-26/10, 5-6=-4/6, 6-7=-4/6, 7-8=-4/6,

8-9=-4/6, 9-10=-4/6, 10-11=-4/6, 11-12=-4/6, 12-13=-4/6, 13-14=-37/32

BOT CHORD 2-22=-6/4, 21-22=-6/4, 20-21=-6/4, 19-20=-6/4, 18-19=-6/4, 17-18=-6/4, 16-17=-6/4,

15-16=-6/4, 14-15=-6/4

4-22=-73/44, 6-21=-71/75, 7-20=-72/59, 8-19=-72/61, 9-18=-72/61, 10-17=-72/61,

11-16=-70/60, 12-15=-81/67

Julius Lee Truse Design Engineer Floride PE No. 24866 I 199 Ceastal Bay Blvd. Boynton Beach, FL 33436

Continued on page 2

**WEBS** 



Job	Truss	Truss Type	Qty	Ply	00	
	T11	GABLE	1	1		J1925215
	1000000		1000		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:01 2008 Page 2

### JOINT STRESS INDEX

2 = 0.44, 3 = 0.00, 3 = 0.15, 4 = 0.04, 5 = 0.04, 6 = 0.04, 7 = 0.03, 8 = 0.03, 9 = 0.03, 10 = 0.03, 11 = 0.03, 12 = 0.04, 13 = 0.07, 14 = 0.04, 15 = 0.04, 16 = 0.03, 17 = 0.03, 18 = 0.03, 19 = 0.03, 20 = 0.03, 21 = 0.04 and 22 = 0.03

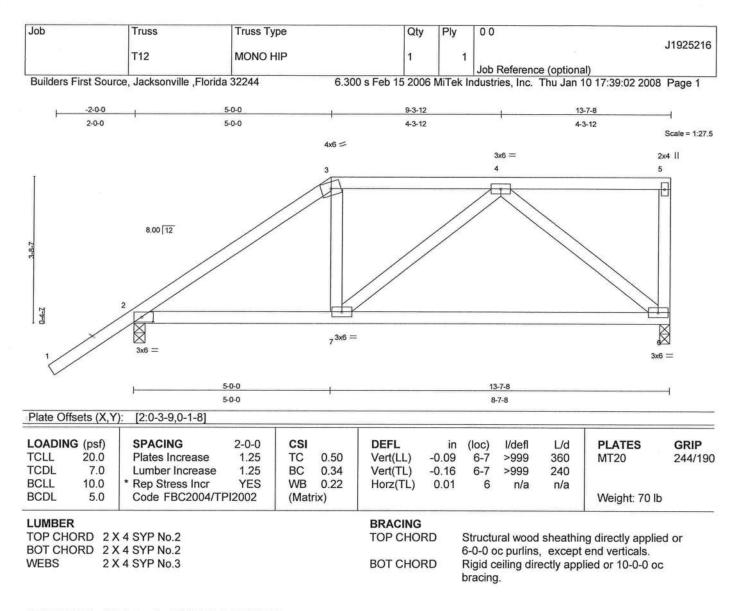
### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 14, 211 lb uplift at joint 2, 25 lb uplift at joint 22, 73 lb uplift at joint 21, 49 lb uplift at joint 20, 54 lb uplift at joint 19, 53 lb uplift at joint 18, 53 lb uplift at joint 17, 53 lb uplift at joint 16 and 55 lb uplift at joint 15.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda FE No. 24808 1 109 Coestel Bay filvi Boynton Besch, FL 23425





REACTIONS (lb/size) 6=417/0-3-0, 2=557/0-3-8

Max Horz 2=177(load case 6)

Max Uplift 6=-117(load case 4), 2=-177(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-567/216, 3-4=-402/233, 4-5=-53/4, 5-6=-106/83

**BOT CHORD** 2-7=-236/400, 6-7=-237/358

**WEBS** 3-7=-1/146, 4-7=-10/132, 4-6=-422/301

## JOINT STRESS INDEX

2 = 0.39, 3 = 0.34, 4 = 0.14, 5 = 0.54, 6 = 0.62 and 7 = 0.09

## NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	T12	MONO HIP	1	1		J1925216
					Job Reference (optional)	

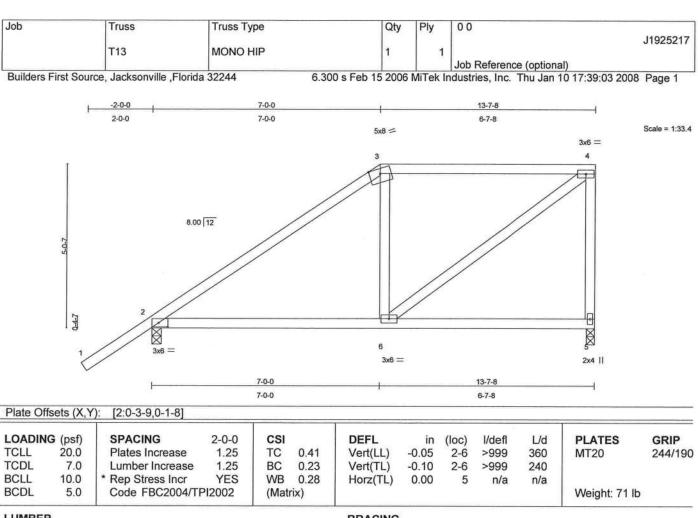
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:02 2008 Page 2

## NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 6 and 177 lb uplift at joint 2.

LOAD CASE(S) Standard





LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 WEBS

2 X 4 SYP No.3

BRACING

**BOT CHORD** 

Structural wood sheathing directly applied or TOP CHORD

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 5=417/0-3-0, 2=557/0-3-8

Max Horz 2=220(load case 6)

Max Uplift 5=-118(load case 5), 2=-177(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-517/167, 3-4=-340/224, 4-5=-386/276

**BOT CHORD** 

2-6=-230/345, 5-6=-21/30

**WEBS** 

3-6=-155/202, 4-6=-258/387

### JOINT STRESS INDEX

2 = 0.46, 3 = 0.68, 4 = 0.63, 5 = 0.53 and 6 = 0.22

## NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008



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Job	Truss	Truss Type	Qty	Ply	00	
-	T13	MONO HIP	1	1	J1	925217
	NAME OF THE PARTY				Job Reference (optional)	
Builders Firs	t Source, Jacksonville,	Florida 32244	6.300 s Feb 15 2006	MiTek Ir	ndustries, Inc. Thu Jan 10 17:39:03 2008 Pa	age 2

**NOTES** 

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 5 and 177 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida Pls No. 34866 1109 Coastal Bay Blyd.



Job	Truss	Truss Type	Qty	Ply	00	
	T14	MONO HIP	1	1		J1925218
	1.22				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:03 2008 Page 1

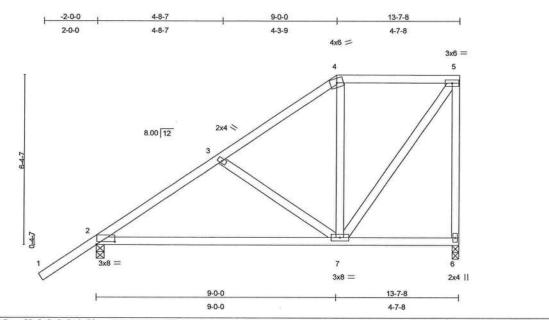


Plate Of	tsets (X, Y	'): [2:0-8-3,0-1-2]									_	
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.13	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.23	2-7	>706	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 83 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

**BOT CHORD** 

REACTIONS (lb/size) 6=417/0-3-0, 2=557/0-3-8

Max Horz 2=263(load case 6)

Max Uplift 6=-119(load case 5), 2=-170(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-544/153, 3-4=-332/104, 4-5=-220/145, 5-6=-402/276

**BOT CHORD** 2-7=-308/391, 6-7=-8/9

3-7=-204/192, 4-7=-109/150, 5-7=-237/363 WEBS

#### JOINT STRESS INDEX

2 = 0.70, 3 = 0.10, 4 = 0.42, 5 = 0.31, 6 = 0.24 and 7 = 0.36

# NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

Scale = 1:40.7

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	T14	MONO HIP	1	1		J1925218
	08100				Job Reference (optional)	

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#### **NOTES**

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 6 and 170 lb uplift at joint 2.

LOAD CASE(S) Standard



 Job
 Truss
 Truss Type
 Qty
 Ply
 0 0
 J1925219

 T15
 MONO HIP
 1
 1
 Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

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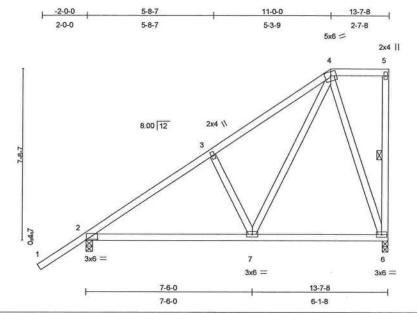


Plate Of	fsets (X,)	<u>(): [2:0-6-3,0-0-6]</u>										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.06	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.11	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.01	6	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	8 8					Weight: 87 lb	

### LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

# BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS

1 Row at midpt

5-6

REACTIONS (lb/size) 6=417/0-3-0, 2=557/0-3-8

Max Horz 2=305(load case 6)

Max Uplift 6=-146(load case 6), 2=-154(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-537/75, 3-4=-404/137, 4-5=-6/0, 5-6=-30/40

BOT CHORD 2-7=-294/375, 6-7=-100/124

WEBS 3-7=-250/255, 4-7=-193/330, 4-6=-382/316

#### JOINT STRESS INDEX

2 = 0.62, 3 = 0.14, 4 = 0.31, 5 = 0.14, 6 = 0.23 and 7 = 0.27

# **NOTES**

 Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Chambearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Trues Design Engineer Florida PE No. 34869 1109 Caustal Bay Blvd Boynton Beach, Ft. 33435

January 10,2008

Scale = 1:48.8

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	anne en totam even
	T15	MONO HIP	1	1		J1925219
			100		Job Reference (optional)	

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### **NOTES**

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 6 and 154 lb uplift at joint 2.

LOAD CASE(S) Standard

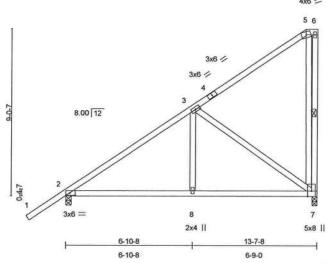


Job	Truss	Truss Type	Qty	Ply	00	
	T16	MONO HIP	1	1		J1925220
					Job Reference (optional)	

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Scale = 1:58.9



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.04	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.08	2-8	>999	240	0.0000000000000000000000000000000000000	2-738-00000000000000000000000000000000000
BCLL	10.0	* Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.01	7	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 91 lb	

2 X 4 SYP No.2
2 X 4 SYP No.2
2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS 1 Row at midpt

5-7

REACTIONS (lb/size) 7=417/0-3-0, 2=557/0-3-8

Max Horz 2=348(load case 6)

Max Uplift 7=-193(load case 6), 2=-131(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-522/0, 3-4=-129/0, 4-5=-76/55, 5-6=-11/8, 6-7=-195/162

BOT CHORD

2-8=-266/349, 7-8=-266/349

WEBS

3-8=0/230, 3-7=-398/297, 5-7=-322/362

## JOINT STRESS INDEX

2 = 0.40, 3 = 0.17, 4 = 0.19, 5 = 0.53, 6 = 0.39, 7 = 0.17 and 8 = 0.16

## NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Challe bearings age assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee Truse Design Engineer Flonda PE No. 34868 1109 Ceastal Bay Blvd Boynton Beach, FL 3343:

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	0 0	W244740457775524-152
	T16	MONO HIP	1	1		J1925220
					Job Reference (optional)	

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### **NOTES**

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 7 and 131 lb uplift at joint 2.

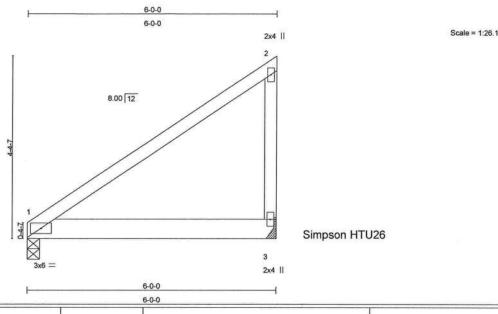
LOAD CASE(S) Standard

Julius Lee Pruss Design Engineer Florida FIS No. 24865 I 100 Coastal Bay Blvd Sovinton Beach, FL 23435



Job	Truss	Truss Type	Qty	Ply	00	200001 (12000) (12000)
	T17	MONO TRUSS	1	1		J1925221
		mene mee			Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.06	1-3	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.12	1-3	>554	240	000000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.00		n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 30 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D

**WEBS** 

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=613/0-3-8, 3=613/Mechanical

Max Horz 1=124(load case 5)

Max Uplift 1=-135(load case 5), 3=-219(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-86/69, 2-3=-154/110

**BOT CHORD** 1-3=0/0

## JOINT STRESS INDEX

1 = 0.07, 2 = 0.07 and 3 = 0.06

#### **NOTES**

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 135 Ib uplift at joint 1 and 219 lb uplift at joint 3.
- 5) Girder carries tie-in span(s): 11-8-0 from 0-0-0 to 6-0-0
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back

Continued on page 2

January 10,2008

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Job	Truss	Truss Type	Qty	Ply	00	
	T17	MONO TRUSS	1	1		J1925221
		mene mee			Job Reference (optional)	

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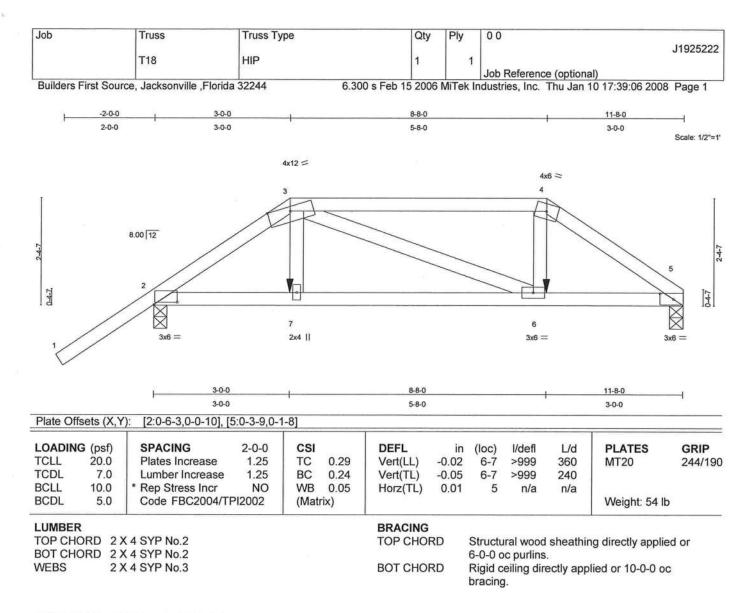
# LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-161(F=-151), 1-2=-54

Julius Lee Trues Design Engineer Flonds PE No. 34869 1109 Coestal Bay Blvd Boynton Beach, Ft. 23435





**REACTIONS** (lb/size) 5=432/0-3-8, 2=576/0-3-8

Max Horz 2=92(load case 5)

Max Uplift 5=-132(load case 3), 2=-218(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-648/214, 3-4=-551/218, 4-5=-679/226

**BOT CHORD** 

2-7=-209/499, 6-7=-208/504, 5-6=-173/545

**WEBS** 

3-7=0/159, 3-6=-70/63, 4-6=-1/164

#### JOINT STRESS INDEX

2 = 0.66, 3 = 0.63, 4 = 0.43, 5 = 0.34, 6 = 0.10 and 7 = 0.11

## NOTES

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Les Truse Design Engineer Flonds PE No. 24866 1100 Coastal Bay Blvd Boynton Besch, FL 99436

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	T18	HIP	1	1		J1925222
	INVEST				Job Reference (optional)	

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#### NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 5 and 218 lb uplift at joint 2.
- 7) Girder carries hip end with 3-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-64(F=-10), 4-5=-54, 2-7=-10, 6-7=-12(F=-2), 5-6=-10

Concentrated Loads (lb)

Vert: 7=-48(F) 6=-48(F)



lob	Truss	Truss Type		Qty	Ply	0.0				J1925223
	T19	MONO HIP	•	1		1 loh	Doforono	e (optional)		
Builders First S	Source, Jacksonville ,Florid	a 32244	6.300 s F	eb 15 2006	6 MiTek	Indust	ries, Inc.	Thu Jan 10	17:39:06 200	8 Page 1
										•
H	-2-0-0	5-0-0					1-8-0			
	2-0-0	5-0-0					6-8-0			Scale = 1:27.1
		0	4x12	=					3x6 = 6x8	
			3						4	
1			4	7						
				R						
1										
	8.00 12				//					Simpson HTU
72	0.00   12					//				Simpson HTC
3-8-7									1	
		//								
1				1				//		9
	2//		_					/	$\bigvee$	
2-4-7										
. 9										
	3x6 =		6	. 11					5	
1/			2x-	\$ II					3x6 =	
		5-0-0 5-0-0					1-8-0 5-8-0			
Plate Offsets	X,Y): [2:0-3-9,0-1-8]									
OADINO (-	0.000000	0.00			**	<i>u</i> ×	141.0			
OADING (ps CLL 20.		2-0-0 1.25		EFL (ort(LL)	in	(loc) 5-6	I/defl	L/d	PLATES	GRIP
CDL 7		1.25		/ert(LL) /ert(TL)	-0.03 -0.05	5-6	>999	360 240	MT20	244/190
CLL 10.		YES		forz(TL)	-0.03	7	n/a	n/a		
CDL 5			(Matrix)	( )	0.01	23 <b>6</b> 237	1110	11100	Weight: 61	lb
JMBER				RACING						
JIIIDLI	2 X 4 SYP No.2			CHOING						

**BOT CHORD** 

REACTIONS (lb/size) 2=487/0-3-8, 7=351/Mechanical

Max Horz 2=177(load case 6)

Max Uplift 2=-168(load case 6), 7=-100(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-446/166, 3-4=-59/42, 5-7=-77/195, 4-7=-171/151

**BOT CHORD** 

2-6=-203/308, 5-6=-200/310

**WEBS** 

WEBS

**OTHERS** 

3-6=0/182, 3-5=-286/180

## JOINT STRESS INDEX

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3

2 X 4 SYP No.3

2 = 0.48, 3 = 0.74, 4 = 0.22, 5 = 0.71, 6 = 0.13, 7 = 0.00 and 7 = 0.19

### NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Charles age assumed to be SYP No.2 crushing capacity of 565.00 psi

dista Lare Usa Clesica Oricla PE Nic

6-0-0 oc purlins, except end verticals.

bracing.

Rigid ceiling directly applied or 10-0-0 oc

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T19	MONO HIP	1	1	3	J1925223
	1.10	Morro Till	'	4.	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:06 2008 Page 2

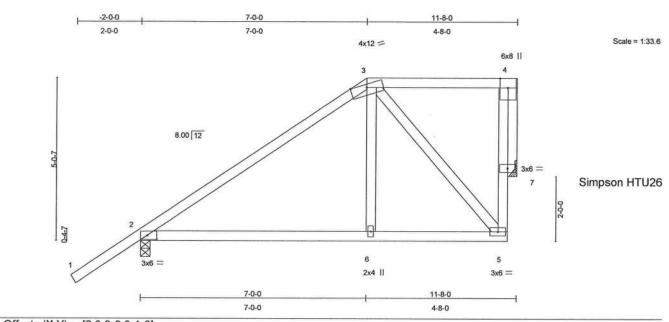
### NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 2 and 100 lb uplift at joint 7.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	0 0
		N 460 ( 4 Property Colors <b>4</b> 04772 From C			J1925224
	T20	MONO HIP	1	1	
				1	Job Reference (optional)



	fsets (X, Y										T	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.05	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.11	2-6	>999	240	Cartification.	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 66 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
<b>BOT CHORD</b>	2 X 4 SYP No.2		6-0-0 oc purlins, except end verticals.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc
OTHERS	2 X 4 SYP No.3		bracing.

REACTIONS (lb/size) 2=487/0-3-8, 7=351/Mechanical

Max Horz 2=220(load case 6)

Max Uplift 2=-163(load case 6), 7=-101(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-388/93, 3-4=-5/14, 5-7=-177/269, 4-7=-82/85

**BOT CHORD** 

2-6=-169/235, 5-6=-167/237

**WEBS** 

3-6=0/205, 3-5=-349/247

### JOINT STRESS INDEX

2 = 0.48, 3 = 0.97, 4 = 0.10, 5 = 0.17, 6 = 0.14, 7 = 0.00 and 7 = 0.19

## NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Chall hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Cincinson

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	00	
	T20	MONO HIP	1	1		J1925224
		2-19-19-19-19-19-19-19-19-19-19-19-19-19-			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:07 2008 Page 2

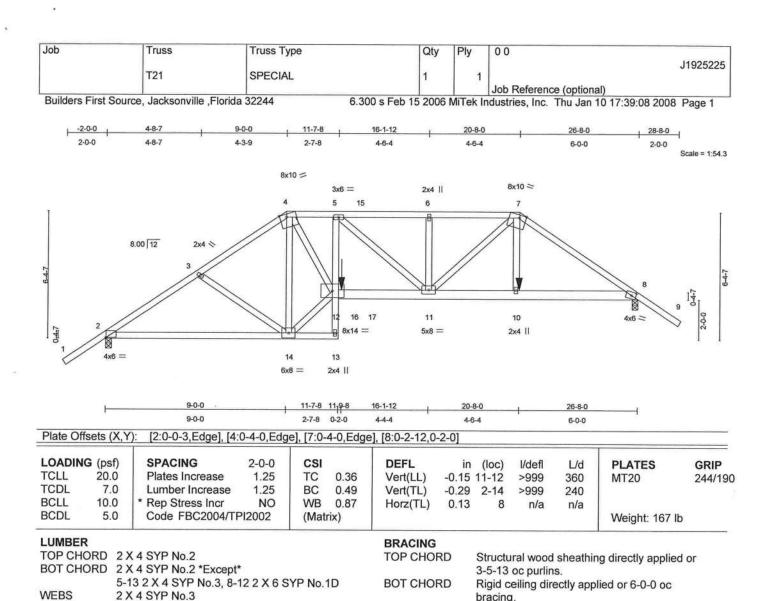
### **NOTES**

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 163 lb uplift at joint 2 and 101 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truse Design Endineer Florida PE No. 24869 1109 Coestal Bay Blvd. Boynton Beach, Ft. 23435





REACTIONS (lb/size) 2=1549/0-3-8, 8=1754/0-3-8

Max Horz 2=161(load case 5)

Max Uplift 2=-502(load case 5), 8=-628(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-2274/780, 3-4=-2075/765, 4-5=-3055/1226, 5-15=-2929/1212,

6-15=-2929/1212, 6-7=-2930/1211, 7-8=-2660/1018, 8-9=0/65

BOT CHORD 2-14=-721/1807, 13-14=-37/60, 12-13=-53/4, 5-12=-144/222, 12-16=-1226/3115,

16-17=-1221/3101, 11-17=-1222/3100, 10-11=-804/2141, 8-10=-800/2129

3-14=-169/116, 4-14=-1322/565, 4-12=-1062/2630, 5-11=-248/46, 6-11=-502/365,

7-11=-510/1114, 7-10=-137/350, 12-14=-811/2162

### JOINT STRESS INDEX

2 = 0.78, 3 = 0.33, 4 = 0.51, 5 = 0.45, 6 = 0.33, 7 = 0.66, 8 = 0.82, 10 = 0.33, 11 = 0.51, 12 = 0.64, 13 = 0.43 and 14 = 0.78

# NOTES

**WEBS** 

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

& Rinkide adequate drainage to prevent water ponding.

nion Beach. FL 93435

Engineer.

January 10,2008

A Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	T21	SPECIAL	1	1		J1925225
		20 = 3 0 = 3	120		Job Reference (optional)	

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#### NOTES

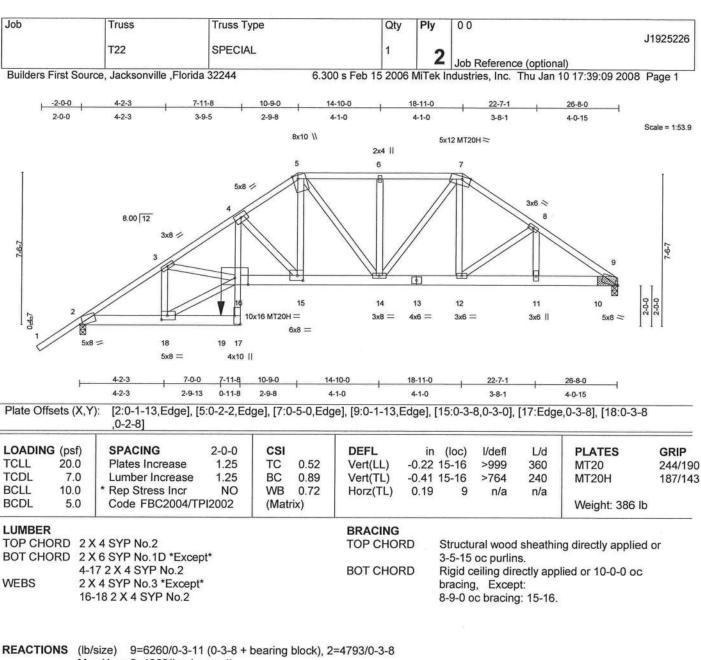
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 502 lb uplift at joint 2 and 628 lb uplift at joint 8.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
  - Uniform Loads (plf)
    - Vert: 1-4=-54, 4-15=-54, 7-15=-104(F=-50), 7-9=-54, 2-13=-10, 12-17=-10, 10-17=-19(F=-9), 8-10=-10
  - Concentrated Loads (lb)
    - Vert: 10=-288(F) 16=-613(F)

Julius Lee Trues Cosign Engineer Plonda Pls No. 34869 1109 Coastel Bay Blvd





Max Horz 2=1362(load case 4)

Max Uplift 9=-2215(load case 3), 2=-1451(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-8022/2462, 3-4=-12060/4575, 4-5=-8821/3495, 5-6=-7555/2893,

6-7=-7555/2893, 7-8=-7639/2790, 8-9=-9294/3303

BOT CHORD 2-18=-3323/6567, 18-19=-470/686, 17-19=-470/686, 16-17=-596/1608,

4-16=-1853/4399, 15-16=-4194/9985, 14-15=-2987/7450, 13-14=-2346/6391,

12-13=-2346/6391, 11-12=-2688/7646, 10-11=-2688/7646, 9-10=-2688/7646

3-18=-3085/1584, 3-16=-1733/3497, 16-18=-3261/6722, 4-15=-3820/1813, 5-15=-1949/4463, 5-14=-132/279, 6-14=-134/24, 7-14=-800/1037, 7-12=-803/2486

5-15=-1949/4463, 5-14=-123/279, 6-14=-124/94, 7-14=-890/1937, 7-12=-893/2486,

8-12=-1627/636, 8-11=-595/1719

## JOINT STRESS INDEX

**WEBS** 

2 = 0.72, 3 = 0.86, 4 = 0.72, 5 = 0.93, 6 = 0.33, 7 = 0.95, 8 = 0.64, 9 = 0.82, 9 = 0.00, 9 = 0.00, 10 = 0.00, 10 = 0.00, 10 = 0.00, 11 = 0.27, 12 = 0.79, 13 = 0.90, 14 = 0.96, 15 = 0.60, 16 = 0.68, 17 = 0.78 and 18 = 0.74

Continued on page 2

January 10,2008

Chamber



Job	Truss	Truss Type	Qty	Ply	00	
	T22	SPECIAL	1			J1925226
				2	Job Reference (optional)	

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#### NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc, 2 X 4 - 1 row at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) 2 X 6 SYP No.1D bearing block 12" long at jt. 9 attached to back face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners per block. Bearing is assumed to be SYP.
- 4) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 6) Provide adequate drainage to prevent water ponding.
- 7) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) All plates are MT20 plates unless otherwise indicated.
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2215 lb uplift at joint 9 and 1451 lb uplift at joint 2.
- 11) Girder carries tie-in span(s): 33-4-0 from 22-6-8 to 26-8-0; 25-8-0 from 8-0-0 to 22-6-8

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

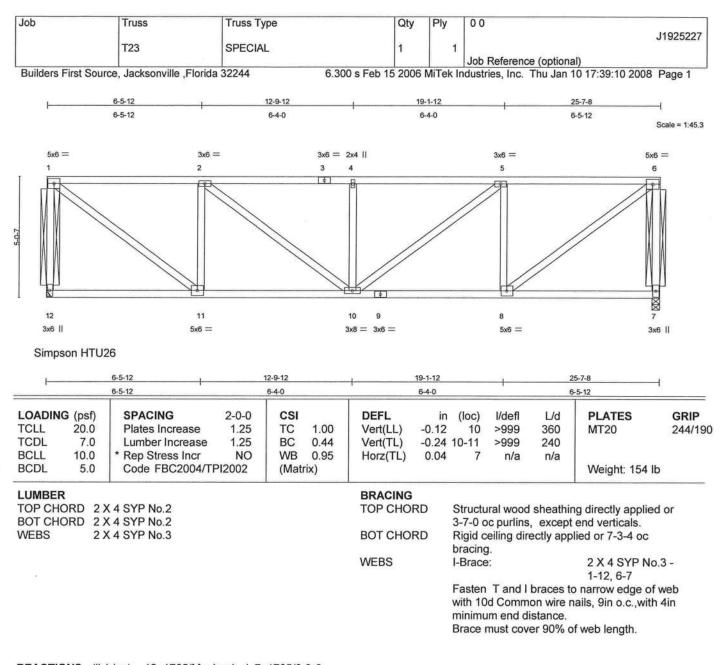
Vert: 1-5=-54, 5-7=-54, 7-9=-54, 2-17=-10, 11-16=-384(F=-374), 9-11=-507(F=-497)

Concentrated Loads (lb)

Vert: 19=-1765(F)

dullus Les Truss Design Engineer Plonda PE No. 34869 1100 Coestal Bay Blyd Boynon Basch El 10448





REACTIONS (lb/size) 12=1765/Mechanical, 7=1765/0-3-8

Max Uplift 12=-701(load case 3), 7=-701(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-12=-1686/729, 1-2=-1867/741, 2-3=-2445/970, 3-4=-2445/970, 4-5=-2445/970,

5-6=-1867/741, 6-7=-1686/729

**BOT CHORD** 11-12=-30/75, 10-11=-741/1867, 9-10=-741/1867, 8-9=-741/1867, 7-8=-30/75

1-11=-889/2239, 2-11=-1217/632, 2-10=-286/722, 4-10=-724/419, 5-10=-286/722,

5-8=-1217/632, 6-8=-889/2239

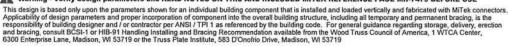
# JOINT STRESS INDEX

**WEBS** 

1 = 0.70, 2 = 0.42, 3 = 0.32, 4 = 0.33, 5 = 0.42, 6 = 0.70, 7 = 0.39, 8 = 0.70, 9 = 0.60, 10 = 0.68, 11 = 0.70 and 12 = 0.39

January 10,2008

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	00	
	T23	SPECIAL	1	1		J1925227
				1 .	Job Reference (optional)	= =

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### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 701 lb uplift at joint 12 and 701 lb uplift at joint 7.
- 6) Girder carries hip end with 0-0-0 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

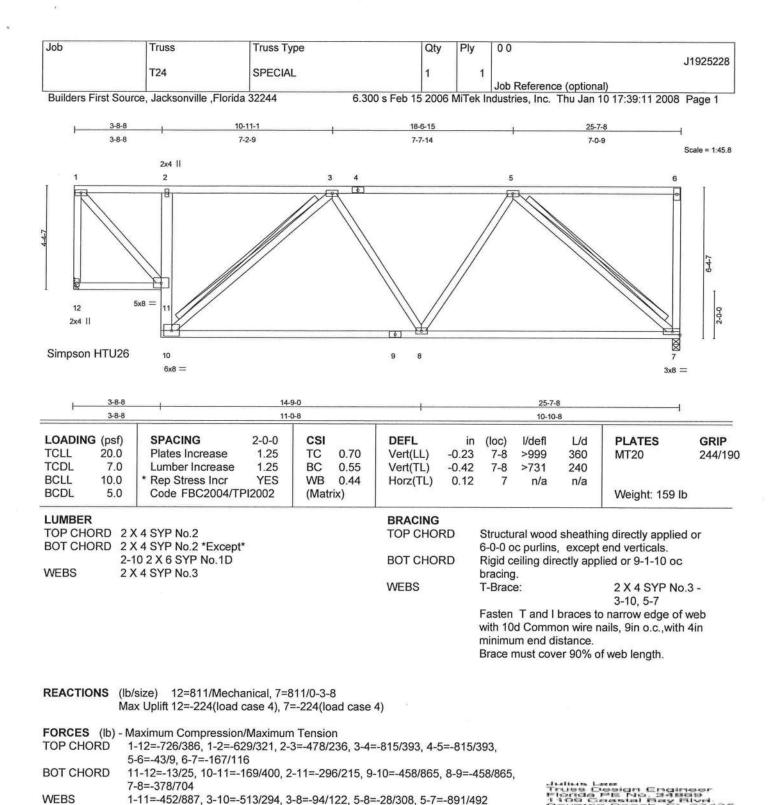
### LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-6=-118(F=-64), 7-12=-22(F=-12)

Julius Les Truse Design Engineer Flonda PE No. 34868 1109 Coestel Bay Blvd





## JOINT STRESS INDEX

1 = 0.57, 2 = 0.33, 3 = 0.42, 4 = 0.25, 5 = 0.42, 6 = 0.34, 7 = 0.57, 8 = 0.42, 9 = 0.52, 10 = 0.68, 11 = 0.56 and 12 = 0.70

January 10,2008

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	58786075-75
	T24	SPECIAL	1	1		J1925228
	35 - 35 - 35 - 35 - 35 - 35 - 35 - 35 -				Job Reference (optional)	

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### NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All plates are 3x6 MT20 unless otherwise indicated.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 12 and 224 lb uplift at joint 7.

LOAD CASE(S) Standard



Job Truss Truss Type Qty Ply 00 J1925229 T25 SPECIAL 1 Job Reference (optional) Builders First Source, Jacksonville ,Florida 32244 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:12 2008 Page 1 10-11-0 18-7-0 25-7-8 10-11-0 7-8-0 7-0-8 Scale: 1/4"=1" 2x4 || 2 5x8 = 12 2x4 || Simpson HTU26 10 9 4x10 = 3-8-8 14-9-0 25-7-8 3-8-8 11-0-8 10-10-8 LOADING (psf) **SPACING** 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 0.65 Plates Increase 1.25 TC 7-8 360 Vert(LL) -0.24>999 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.51 Vert(TL) -0.427-8 >720 240 **BCLL** 10.0 Rep Stress Incr YES 0.48 WB Horz(TL) 0.11 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 173 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 \*Except\* 6-0-0 oc purlins, except end verticals. 2-10 2 X 6 SYP No.1D **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc WEBS 2 X 4 SYP No.3 bracing. WEBS T-Brace: 2 X 4 SYP No.3 - 6-7, 3-10, 3-8, 5-7 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 12=811/Mechanical, 7=811/0-3-8

Max Uplift 12=-224(load case 4), 7=-224(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-12=-731/387, 1-2=-476/243, 2-3=-393/194, 3-4=-669/323, 4-5=-669/323,

5-6=-33/6, 6-7=-164/114

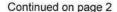
**BOT CHORD** 11-12=-9/17, 10-11=-168/398, 2-11=-300/218, 9-10=-376/709, 8-9=-376/709,

7-8=-311/577

**WEBS** 1-11=-408/801, 3-10=-461/264, 3-8=-88/115, 5-8=-26/295, 5-7=-809/447

# JOINT STRESS INDEX

1 = 0.59, 2 = 0.33, 3 = 0.44, 4 = 0.24, 5 = 0.44, 6 = 0.31, 7 = 0.53, 8 = 0.44, 9 = 0.54, 10 = 0.95, 11 = 0.54 and 12 = 0.61





Job	Truss	Truss Type	Qty	Ply	0.0	
	T25	SPECIAL	1	1		J1925229
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:12 2008 Page 2

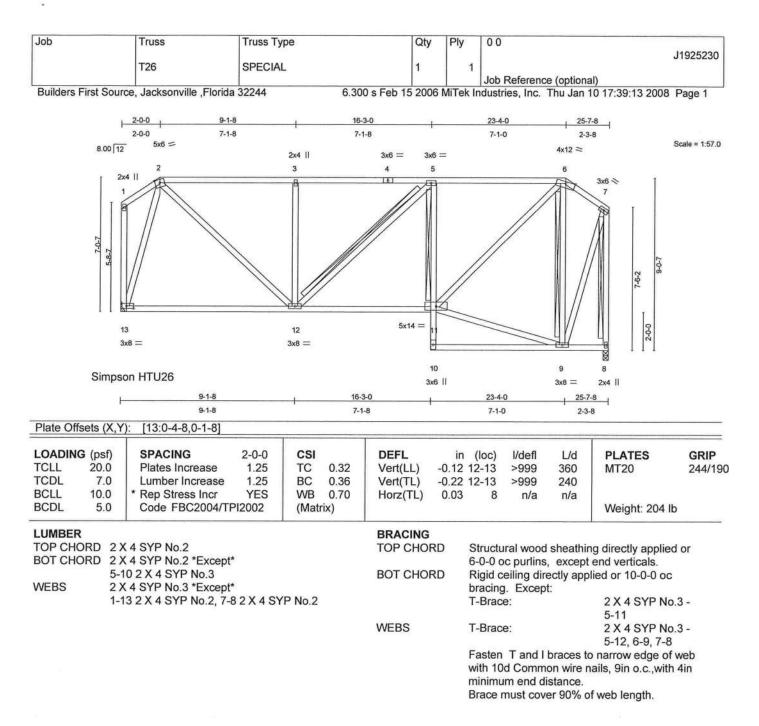
#### **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 12 and 224 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Plonds PE No. 34888 1100 Coestal Bay Blvd Boynton Beach, FL 35495





REACTIONS (lb/size) 8=811/0-3-8, 13=811/Mechanical

Max Horz 13=-40(load case 4)

Max Uplift 8=-261(load case 4), 13=-264(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-52/61, 2-3=-742/428, 3-4=-742/428, 4-5=-742/428, 5-6=-740/438,

6-7=-245/139, 1-13=-73/65, 7-8=-804/398

BOT CHORD 12-13=-117/230, 11-12=-374/744, 10-11=0/101, 5-11=-397/274, 9-10=-2/30,

8-9=-2/3

WEBS 2-12=-346/709, 3-12=-399/286, 5-12=-7/25, 6-11=-387/749, 6-9=-677/393,

9-11=-98/191, 7-9=-335/702, 2-13=-823/481

Truse Design Engineer Florida PE No. 34869 1108 Geastal Bay Blyd Boynton Beach, FL 93435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T26	SPECIAL	1	1		J1925230
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:13 2008 Page 2

#### JOINT STRESS INDEX

1 = 0.48, 2 = 0.59, 3 = 0.33, 4 = 0.27, 5 = 0.35, 6 = 0.78, 7 = 0.54, 8 = 0.33, 9 = 0.87, 10 = 0.31, 11 = 0.50, 12 = 0.68 and 13 = 0.61

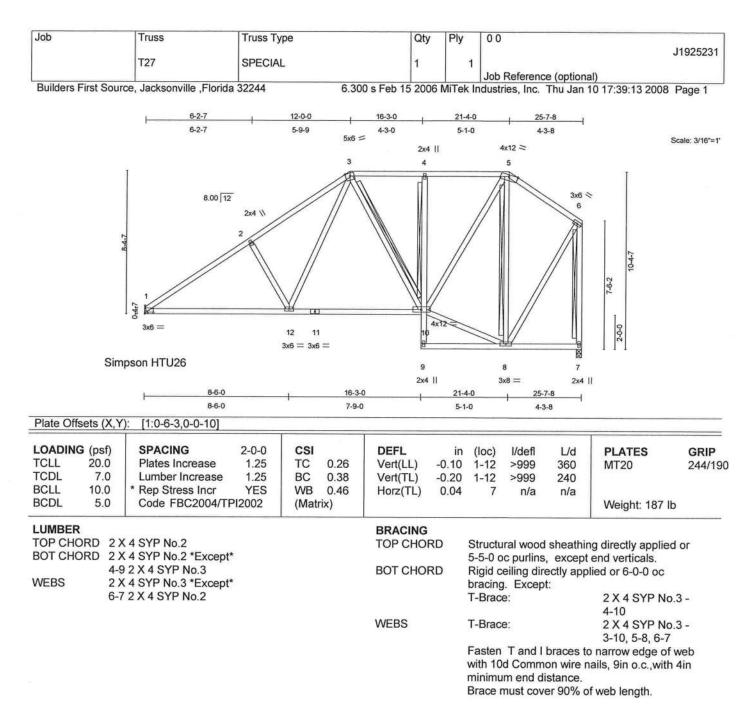
#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 8 and 264 lb uplift at joint 13.

LOAD CASE(S) Standard

Julius Lass Trues Elesian Engineer Planda PE No. 34868 1109 Coastal Bay Blvd Boynton Beach, FL 23435





REACTIONS (lb/size) 1=811/Mechanical, 7=811/0-3-8

Max Horz 1=217(load case 5)

Max Uplift 1=-156(load case 6), 7=-151(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1218/536, 2-3=-1055/585, 3-4=-599/415, 4-5=-598/417, 5-6=-413/244,

6-7=-789/422

**BOT CHORD** 1-12=-577/938, 11-12=-343/647, 10-11=-343/647, 9-10=0/64, 4-10=-259/155,

8-9=-21/4, 7-8=-6/8

**WEBS** 2-12=-282/286, 3-12=-229/380, 3-10=-115/108, 5-10=-305/599, 5-8=-569/318,

8-10=-132/311, 6-8=-265/573

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T27	SPECIAL	1	1		J1925231
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:14 2008 Page 2

#### JOINT STRESS INDEX

1 = 0.61, 2 = 0.33, 3 = 0.44, 4 = 0.33, 5 = 0.71, 6 = 0.50, 7 = 0.33, 8 = 0.61, 9 = 0.35, 10 = 0.54, 11 = 0.25 and 12 = 0.46

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

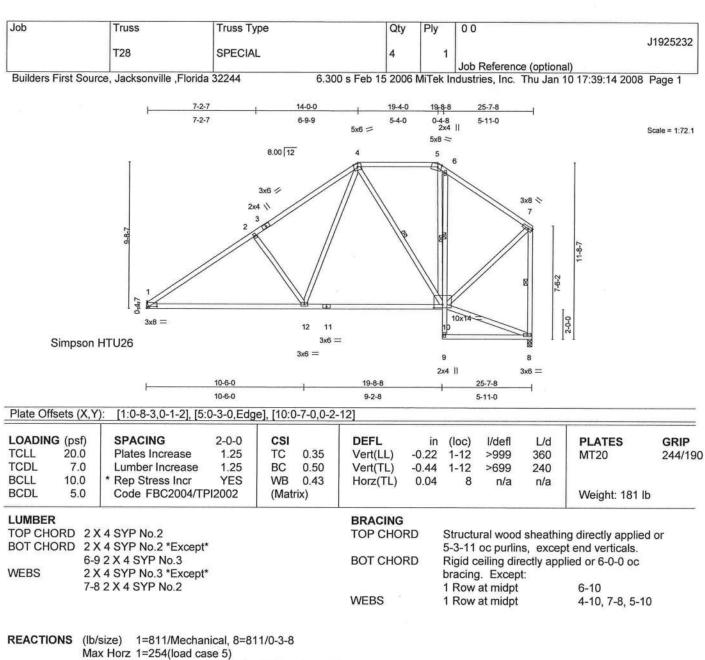
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 156 lb uplift at joint 1 and 151 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Les Trues Design Engineer Flonda PE No. 34869 1109 Coastal Bay Blyd 1000 Coastal Bay Blyd





Max Uplift 1=-162(load case 6), 8=-151(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1182/515, 2-3=-962/512, 3-4=-938/541, 4-5=-402/343, 5-6=-497/440,

6-7=-574/322, 7-8=-788/421

**BOT CHORD** 1-12=-545/901, 11-12=-266/559, 10-11=-266/559, 9-10=0/84, 6-10=-171/217,

WEBS 2-12=-336/337, 4-12=-243/445, 4-10=-336/193, 7-10=-230/541, 8-10=-13/43,

5-10=-176/128

## JOINT STRESS INDEX

1 = 0.65, 2 = 0.33, 3 = 0.44, 4 = 0.59, 5 = 0.56, 6 = 0.45, 7 = 0.71, 8 = 0.35, 9 = 0.63, 10 = 0.29, 11 = 0.35 and 12 = 0.47

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T28	SPECIAL	4	1		J1925232
	1000000				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:15 2008 Page 2

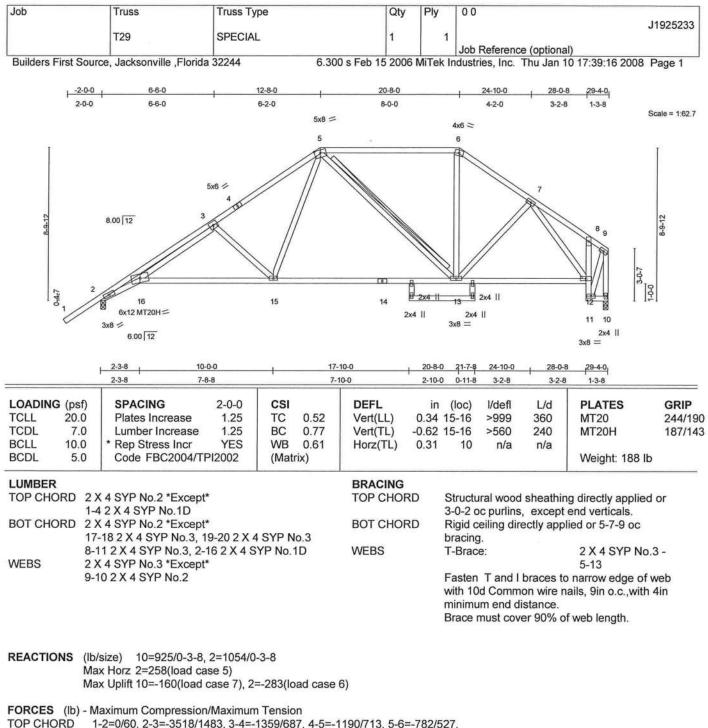
#### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 1 and 151 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida FE No. 24866 1109 Coasial Bay Blyd Boynton Beach, FL 23436





1-2=0/60, 2-3=-3518/1483, 3-4=-1359/687, 4-5=-1190/713, 5-6=-782/527,

6-7=-973/553, 7-8=-513/308, 8-9=-401/192, 9-10=-1005/473

**BOT CHORD** 

15-16=-673/1355, 14-15=-359/908, 13-14=-359/908, 12-13=-318/720,

11-12=-598/302, 8-12=-103/117, 10-11=-12/25, 2-16=-1335/3076

**WEBS** 3-15=-403/335, 5-13=-251/166, 6-13=-59/269, 7-13=-111/179, 9-11=-351/731,

3-16=-726/1900, 5-15=-202/492, 7-12=-585/251

# JOINT STRESS INDEX

2 = 0.77, 3 = 0.74, 4 = 0.25, 5 = 0.58, 6 = 0.75, 7 = 0.41, 8 = 0.41, 9 = 0.56, 10 = 0.43, 11 = 0.69, 12 = 0.52, 13 = 0.56, 14 = 0.560.50, 15 = 0.48, 16 = 0.91, 17 = 0.33, 18 = 0.33, 19 = 0.33 and 20 = 0.33

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	0.0	- Carrier and Carr
	T29	SPECIAL	1	1		J1925233
					Job Reference (optional)	

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## NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) All plates are 3x6 MT20 unless otherwise indicated.

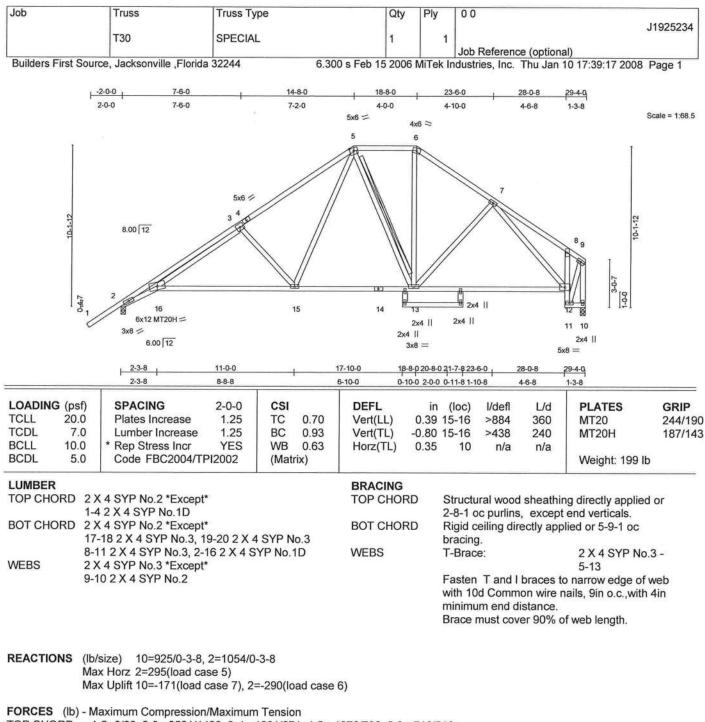
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 160 lb uplift at joint 10 and 283 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 34868 1 100 Caastal Bay Blvd Boynton Beach, FL 33435





TOP CHORD 1-2=0/60, 2-3=-3521/1409, 3-4=-1291/671, 4-5=-1276/706, 5-6=-716/518,

6-7=-933/553, 7-8=-562/329, 8-9=-406/178, 9-10=-1022/459

15-16=-626/1284, 14-15=-257/778, 13-14=-257/778, 12-13=-322/762,

11-12=-595/324, 8-12=-143/174, 10-11=-11/25, 2-16=-1258/3081

3-15=-458/389, 5-13=-251/183, 6-13=-175/282, 7-13=-108/171, 9-11=-367/745,

3-16=-685/1969, 5-15=-285/519, 7-12=-544/234

# JOINT STRESS INDEX

**BOT CHORD** 

**WEBS** 

2 = 0.77, 3 = 0.76, 4 = 0.61, 5 = 0.63, 6 = 0.31, 7 = 0.41, 8 = 0.63, 9 = 0.57, 10 = 0.42, 11 = 0.70, 12 = 0.69, 13 = 0.62, 14 = 0.620.28, 15 = 0.46, 16 = 0.91, 17 = 0.33, 18 = 0.33, 19 = 0.33 and 20 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T30	SPECIAL	1	1		J1925234
	288			100	Job Reference (optional)	

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## NOTES

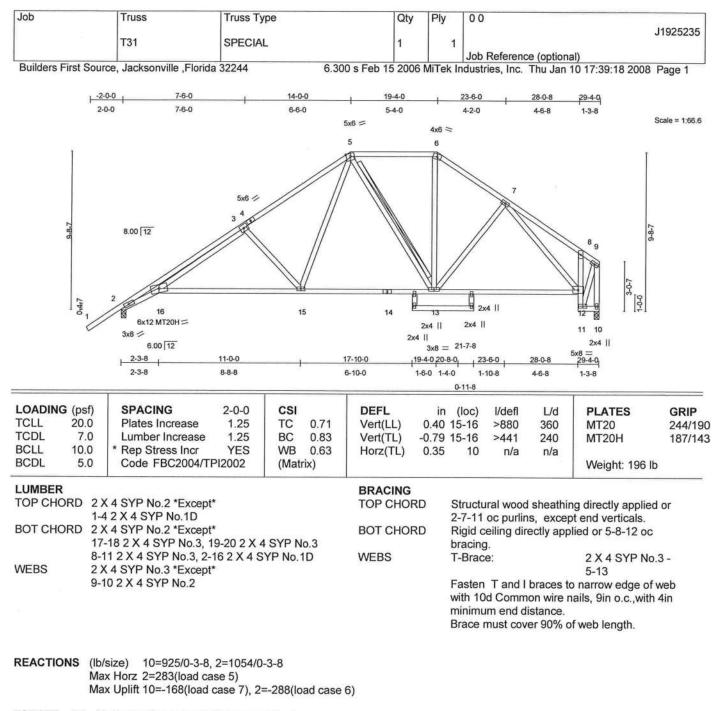
1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 3x6 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 10 and 290 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Flonds PE No. 24ses 1109 Coestel Bay Blvd





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-3520/1425, 3-4=-1289/672, 4-5=-1274/703, 5-6=-735/519,

6-7=-940/561, 7-8=-566/343, 8-9=-401/180, 9-10=-1018/464

**BOT CHORD** 15-16=-622/1277, 14-15=-286/816, 13-14=-286/816, 12-13=-320/758,

11-12=-607/329, 8-12=-161/190, 10-11=-11/25, 2-16=-1272/3080

3-15=-438/370, 5-13=-239/166, 6-13=-149/280, 7-13=-85/148, 9-11=-375/754,

3-16=-704/1975, 5-15=-261/496, 7-12=-540/217

# JOINT STRESS INDEX

**WEBS** 

2 = 0.77, 3 = 0.76, 4 = 0.54, 5 = 0.56, 6 = 0.37, 7 = 0.41, 8 = 0.58, 9 = 0.58, 10 = 0.42, 11 = 0.71, 12 = 0.57, 13 = 0.57, 14 = 0.570.27, 15 = 0.48, 16 = 0.91, 17 = 0.33, 18 = 0.33, 19 = 0.33 and 20 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	LEGAL DE CONTROL DE CANTON
	T31	SPECIAL	1	1		J1925235
				55.0	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:18 2008 Page 2

### NOTES

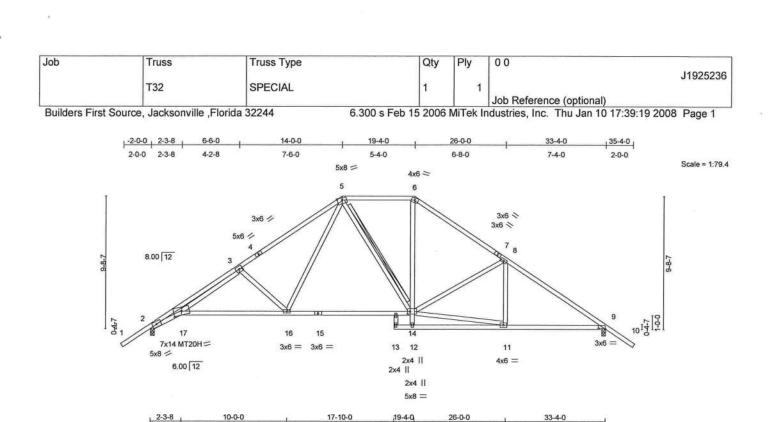
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 3x6 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 10 and 288 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Honda PE No. 24868 1100 Caastal Bay Blvd





7-8-8

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.38	16-17	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.78	16-17	>506	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.35	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 202 lb	

1-6-0

6-8-0

7-10-0

|--|

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

13-18 2 X 4 SYP No.3, 2-17 2 X 4 SYP No.1D

**WEBS** 

**WEBS** 

2 X 4 SYP No.3

2-3-8

# BRACING

**BOT CHORD** 

TOP CHORD

Structural wood sheathing directly applied or

2-2-0 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc

7-4-0

bracing.

**WEBS** 

T-Brace:

2 X 4 SYP No.3 -

5-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

**REACTIONS** (lb/size) 9=1184/0-3-8, 2=1183/0-3-8

Max Horz 2=-260(load case 4)

Max Uplift 9=-304(load case 7), 2=-305(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-4051/1270, 3-4=-1636/752, 4-5=-1521/784, 5-6=-1028/631,

6-7=-1308/659, 7-8=-1325/627, 8-9=-1617/693, 9-10=0/62

**BOT CHORD** 16-17=-516/1616, 15-16=-180/1030, 14-15=-180/1030, 12-13=0/0, 11-12=-3/33,

9-11=-361/1246, 2-17=-914/3549

3-17=-465/2140, 3-16=-479/366, 5-16=-229/551, 5-14=-157/181, 12-14=0/127,

6-14=-162/407, 8-14=-289/270, 8-11=-85/121, 11-14=-361/1231

## JOINT STRESS INDEX

2 = 0.83, 3 = 0.83, 4 = 0.27, 5 = 0.65, 6 = 0.56, 7 = 0.48, 8 = 0.41, 9 = 0.74, 11 = 0.51, 12 = 0.33, 13 = 0.33, 14 = 0.60, 15 = January 10,2008 Continued 66 - 2062 = 17 = 0.78 and 18 = 0.33

🔬 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	00	
	T32	SPECIAL	1	1		J1925236
			1.30		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:19 2008 Page 2

### NOTES

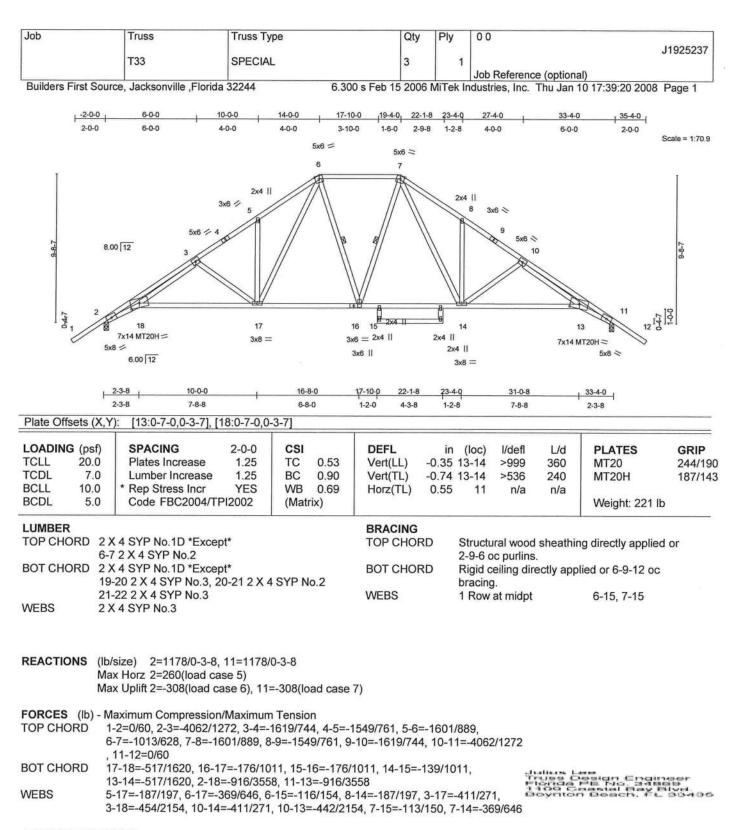
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 304 lb uplift at joint 9 and 305 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truss Design Enginser Flonda PE No. 24868 i 199 Coestel Bay Blvd Boynton Beach, FL 99435





# JOINT STRESS INDEX

2 = 0.84, 3 = 0.84, 4 = 0.29, 5 = 0.33, 6 = 0.38, 7 = 0.38, 8 = 0.33, 9 = 0.29, 10 = 0.84, 11 = 0.84, 13 = 0.76, 14 = 0.72, 15 = 0.34, 16 = 0.36, 17 = 0.72, 18 = 0.76, 19 = 0.33, 20 = 0.33, 21 = 0.33 and 22 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	0.0	
	T33	SPECIAL	3	1		J1925237
		3.5 = 3.0.55			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:20 2008 Page 2

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2 and 308 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Les Truse Design Endineer Florida PE No. 34866 1109 Caastal Bay Blvd Boynton Beach, FL 33436



Job Truss Truss Type Qty Ply 00 J1925238 T34 SPECIAL Job Reference (optional) Builders First Source, Jacksonville ,Florida 32244 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 18:07:07 2008 Page 1 6-0-0 14-0-0 35-4-0 19-4-0 22-1-8 23-4-0 27-4-0 33-4-0 6-0-0 4-0-0 4-0-0 3-10-0 1-6-0 2-9-8 1-2-8 4-0-0 6-0-0 2-0-0 Scale = 1:67.7 5x6 = 5x6 = 8.00 12 5 6 2x4 II 3x6 4 3x6 > 5x8 < 16 15 13 12 7x14 MT20H= 3x6 = 2x4 | 12x4 II 7x14 MT20H = 5x8 = 2x4 II 3x6 || 5x8 > 6.00 12 3x8 = Simpson HTU26 10-0-0 16-8-0 1,7-10-P 33-4-0 31-0-8 2-3-8 7-8-8 6-8-0 1-2-0 4-3-8 1-2-8 7-8-8 2-3-8 Plate Offsets (X,Y): [12:0-7-0,0-3-7], [17:0-7-0,0-3-7] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES** GRIP in (loc) 20.0 TCLL 1.25 0.83 Plates Increase TC Vert(LL) -0.41 16-17 >977 360 MT20 244/190 TCDL 1.25 Vert(TL) 7.0 Lumber Increase BC 0.94 -0.84 16-17 >470 240 MT20H 187/143 BCLL 10.0 Rep Stress Incr YES WB 0.77 Horz(TL) 0.61 n/a n/a Code FBC2004/TPI2002 BCDL 5.0 (Matrix) Weight: 217 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 2-1-4 **BOT CHORD** 2 X 4 SYP No.2 \*Except\* oc purlins. 18-19 2 X 4 SYP No.3, 20-21 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 2-2-0 oc bracing. **WEBS** 

1-17 2 X 4 SYP No.1D, 10-12 2 X 4 SYP No.1D

**WEBS** 2 X 4 SYP No.3

1 Row at midpt

5-14, 6-14

REACTIONS

(lb/size) 1=1053/Mechanical, 10=1182/0-3-8

Max Horz 1=-287(load case 4)

Max Uplift 1=-210(load case 6), 10=-309(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4328/1622, 2-3=-1641/776, 3-4=-1568/793, 4-5=-1620/918, 5-6=-1021/639,

6-7=-1609/902, 7-8=-1557/774, 8-9=-1628/757, 9-10=-4113/1303, 10-11=0/60

**BOT CHORD** 16-17=-575/1661, 15-16=-179/1020, 14-15=-179/1020, 13-14=-148/1017, 12-13=-528/1629, 1-17=-1265/3824, 10-12=-944/3606

4-16=-184/192, 5-16=-394/663, 5-14=-118/156, 7-13=-188/198, 2-16=-440/313,

2-17=-746/2389, 9-13=-413/272, 9-12=-460/2196, 6-14=-114/149, 6-13=-371/648

# JOINT STRESS INDEX

1 = 0.90, 2 = 0.63, 3 = 0.30, 4 = 0.34, 5 = 0.40, 6 = 0.39, 7 = 0.34, 8 = 0.31, 9 = 0.65, 10 = 0.87, 12 = 0.77, 13 = 0.74, 14 = 0.35, 15 = 0.40, 12 = 0.40, 13 = 0.40, 14 = 0.40, 15 = 0.0.37, 16 = 0.76, 17 = 0.81, 18 = 0.34, 19 = 0.34, 20 = 0.34 and 21 = 0.34

# NOTES

WFBS

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is Coffinite of the General control of the control of

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building design and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	-0.00 A MODE -0.00
	T34	SPECIAL	1	1		J1925238
	1070				Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 18:07:07 2008 Page 2

## **NOTES**

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

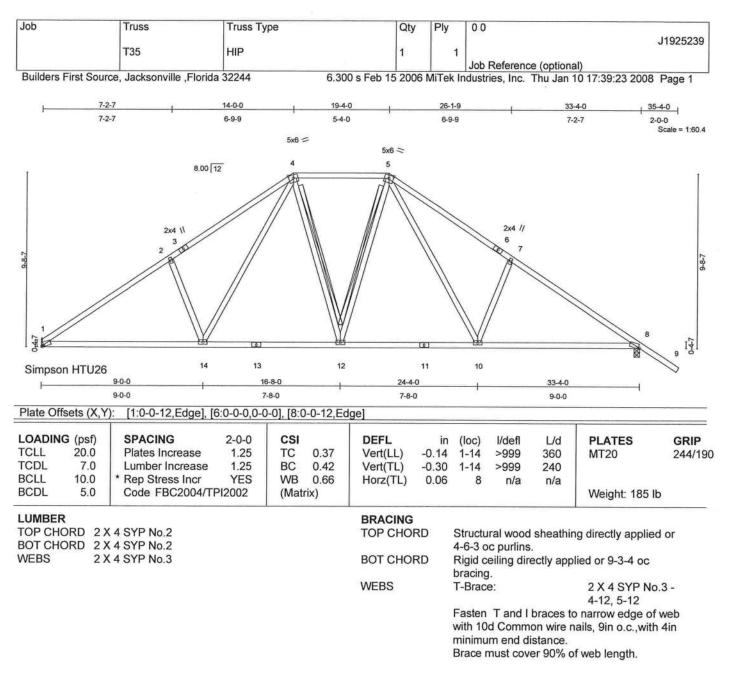
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 7) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 1 and 309 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonds PE No. 34869 I 100 Cessiel Bey Blyd Boynton Besch, FL 35436





REACTIONS (lb/size) 1=1054/Mechanical, 8=1182/0-3-8

Max Horz 1=-286(load case 4)

Max Uplift 1=-210(load case 6), 8=-309(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1621/749, 2-3=-1482/830, 3-4=-1460/859, 4-5=-924/631, 5-6=-1352/827,

6-7=-1460/798, 7-8=-1602/722, 8-9=0/62

**BOT CHORD** 1-14=-418/1261, 13-14=-162/916, 12-13=-162/916, 11-12=-138/913,

10-11=-138/913, 8-10=-387/1238

2-14=-336/345, 4-14=-326/477, 4-12=-120/173, 5-12=-123/178, 5-10=-282/447,

7-10=-320/323

# JOINT STRESS INDEX

**WEBS** 

1 = 0.80, 2 = 0.33, 3 = 0.50, 4 = 0.59, 5 = 0.59, 6 = 0.50, 7 = 0.33, 8 = 0.80, 10 = 0.47, 11 = 0.32, 12 = 0.50, 13 = 0.32 and 14 January 10,2008 Continued on page 2

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building does. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation availed from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T35	HIP	1	1		J1925239
		S230-50 E :			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:23 2008 Page 2

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 3x6 MT20 unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 1 and 309 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda FE No. 34869 1109 Coastal Bay Blvd

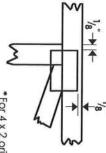


# Symbols

# PLATE LOCATION AND ORIENTATION



\*Center plate on joint unless dimensions indicate otherwise.
Dimensions are in inches. Apply
plates to both sides of truss and securely seat.



\*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



\*This symbol indicates the required direction of slots in connector plates

# PLATE SIZE

4 × 4

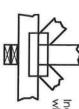
The first dimension is the width perpendicular to slots. Second dimension is the length parallel

# LATERAL BRACING



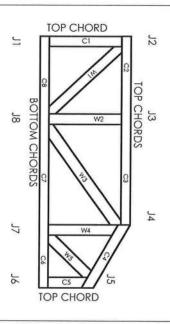
Indicates location of required continuous lateral bracing.

# BEARING



which bearings (supports) occur. Indicates location of joints at

# Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

# CONNECTOR PLATE CODE APPROVALS

ICBO 3907, 4922

BOCA

SBCCI

9667, 9432A

WISC/DILHR 960022-W, 970036-N

561

NER NER



MiTek Engineering Reference Sheet: MII-7473

# Damage or Personal Injury Failure to Follow Could Cause Property General Safety Notes

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 12 Cut members to bear tightly against each other.
- ω Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- 4 Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)

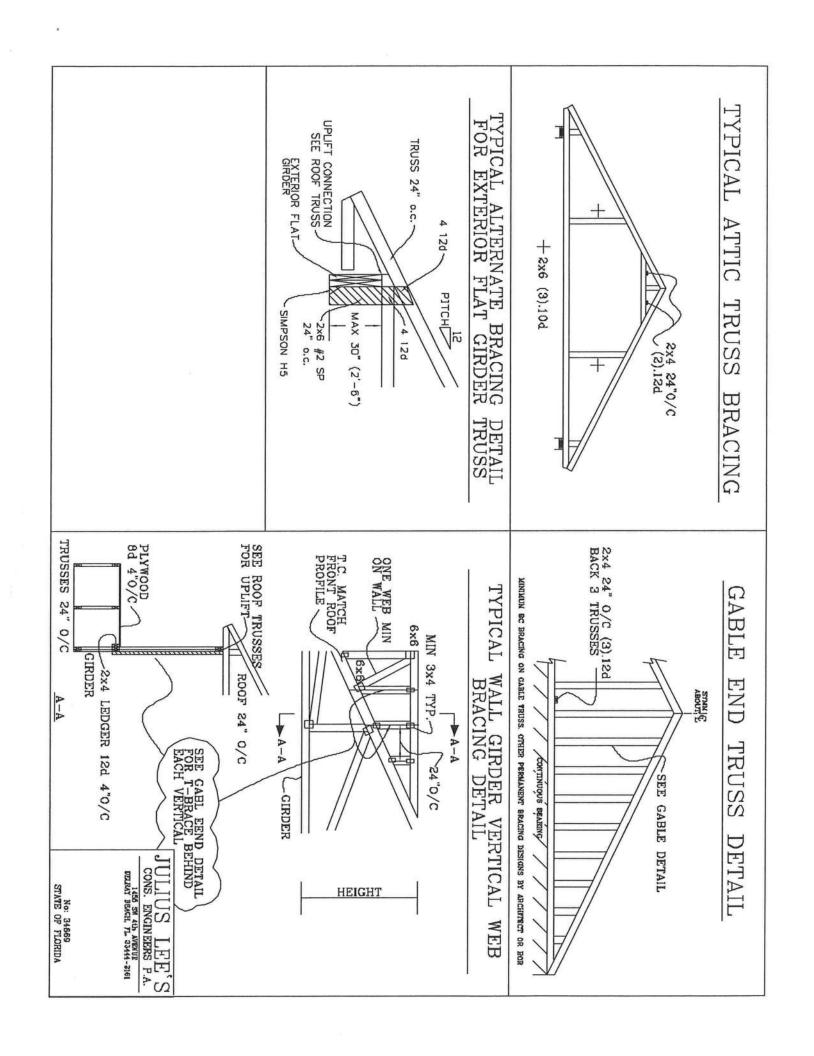
5

- 0 Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication. Unless expressly noted, this design is not applicable for use with fire retardant or
- 7 is the responsibility of truss fabricator. General Camber is a non-structural consideration and preservative treated lumber
- 00 Plate type, size and location dimensions practice is to camber for dead load deflection shown indicate minimum plating requirements
- 9 Lumber shall be of the species and size, and grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- 12. Anchorage and / or load transferring others unless shown connections to trusses are the responsibility of
- Do not overload roof or floor trusses with stacks of construction materials
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- 15. Care should be exercised in handling erection and installation of trusses.

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TRUSTER TO BEST REQUIRE EXTREME CARE IN FABRICATING, MANDLING, SUPPING, INSTALLING AND PARCING, EXTERNITOR TO BEST 1-921 BUILDING COPEOPENI SAFETY INFORMATION, PUBLISSES BY 1-91 (TRUST ENGINEERS P.A.)  PARCING, EST DUNCTION OF COPEOPENI SAFETY PRACTICES PRICE TO PERFORM THE FORMAL PROPERTY ATTACHED REGID COLUNG.  NO: 34869  MAX. TOT. LD. 60 PSF  NO: 34869  MAX. SPACING 24.0"	A CROUP B  13 3° 12 111° 12 111° 12 111° 12 111° 12 11° 13 11° 13 11° 13 11° 14 0° 16 0° 16 0° 16 0° 16 0° 16 0° 16 0° 16 0° 16 0° 16 0° 16 0° 1
ASC E 11/ G MITEK	SPECIES AND GRADES: ROUP A: HEM-PIR HEM-PIR SOUTHERN PINE #3 STANDARD  ROUP B: SOUTHERN PINE #3 STANDARD  ROUP B: LAZ STANDARD  ROUTES: COMP B: LAZ STANDARD  DOUGLAS FIR-LARCH #3 TANDARD  DOUGLAS FIR-LARCH #4 TANDARD  DOUGLAS FIR-LARCH #4 TO EFFERA IS L/240. RECTIONS FOR 136 FLF OVER RO (6 PSP TC DEAD LOAD). LOAD FROM 4 O.C. BFTREN ZONES. SPACE MAILS AT 5" O.C. AND 4" O.C. BFTREN ZONES. SPACE MAILS AT 5" O.C. AND 4" O.C. BFTREN ZONES. SPACE MAILS AT 5" O.C. AND 6" O.C. BFTREN ZONES. SE SPACE MAILS AT 5" O.C. AND 6" O.C. BFTREN ZONES

### DIAGONAL BRACE OPTION: VENTICAL LENGTH MAY BE DOUBLED WINN DIAGONAL BRACE IS USED. CONNECT BRACE IS USED. WAY WEB AT EACH END. MAY WEB TOTAL LENGTH IS 14\*. **VERTICAL** LENGTH MAX **GABLE** VERTICAL LENGTH SHOWN IN TABLE ABOVE. SPACING SPECIES CRADE 24" 16 O.C. O.C. O.C. CONNECT DIAGONAL AT GABLE VERTICAL SPF SPF DFL SPF DFL DFL SP SP SP H H ASCE STANDARD #1 #2 #3 STUD \$100 STANDARD \$11 \$1 #2 \$1 \$100 \$1 / #2 \$3 \$TUD \$TANDARD STANDARD STANDARD STANDARD BEEM BRACE 7-02: MEMORANCEM TRASSIS REPURE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-93 GRULLING COMPOSET SAFETY RETEXTIONS, PUBLISHED BY FPI CIRCUSS PLATE INSTITUTE, 589 D'ENFORTO EX. SUITE 200, MINISON, VI. SAFETY PARCITIES PRIBE TO PERSONNING OF ANCEICA, 5800 ENTERPRISE, LN. MODISON, VI SATIS) FOR SAFETY PARCITIES PRIBE TO PERSONNING THESE TWICTIONS, UNICESS OTHER ENTER ORIGINAL TO CALDY SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BUTTOM COORD SHALL HAVE A PROPERTY ATTACHED REGISTRANCE. GABLE TRUSS 130 ZX1 SP OR DT-L #2 OH BETTER DIAGONAL BRACE, SINGLE OR DOUBLE CUT (AS SHOWN) GROUP A (1) 1X4 "L" BRACE • (1) 2X4 "L" BRACE • AT UPPER END MPH GROUP H WIND GROUP A SPEED, GROUP B REFER 30 18 H (2) 2X4 "L" BRACE \*\* GROUP A 9, 10, CHART MEAN 6" ABOVE FOR MAX GABLE VERTICAL LENGTH EX4 MEN OR BETTER CONTINUOUS GROUP B 9' 10" 9' 10" 9' 6" 10' 7" 10' 7" 10' 4" 10' 4" 9' 9" 10, 1, HEIGHT, **(** Ð CONS. SNIBVÉR (1) 2X6 "L" CROUP A CROUP B 12 10 2 DELRAY BEACH, PL 33444-2161 No: 34869 STATE OF FLORIDA IUS LEE 0 ENCLOSED, 12 13 BRACE . P.T. (2) ZXB GROUP A 10, 10, 10 S \_ ď MAX. MAX. GROUP BRACE 11 12 23 13 23 14 23 10 3 3 7 12. 11 10' 10" 13 0 12' 6" (D TOT. 1.00, SPACING E. ATTACH EACH "L" BRACE WITH 10d NAILS AT 8" O.C. # FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. # TUR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. CABLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLAT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). 'L' BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH. LIVE LOAD DEPLECTION CRITERIA IS L/240. DOUGLAS FIR-LARCH #3 STUD STANDARD SPRUCE-PINE-FIR #1 / #2 STANDARD #3 STUD BRACING GROUP SPECIES PLYWOOD OVERHANG. GABLE TRUSS DETAIL EXPOSURE SOUTHERN PINE 60 GREATER THAN 1. 6". BUT LESS THAN 11' 6". GREATER THAN 11' 6". 24.0" PEAK, SPLICE, AND HEEL PLATES. PSS THAN 4' 0" CABLE VERTICAL PLATE SIZES PSF DATE REF -ENG DWG MITEK STD CABLE 30' E HT GROUP HEM-FIR HI & BIR GROUP B: 0 DOUGLAS FIR-LARCH 11/26/03 STANDARD ASCE7-02-GAB13030 A: 13 2 NO SPLICE AND HEM-PIR 2.5X4 NOTES 77.7 STANDARD GRADES:



BOT CHORD 2X4 2X4 300 222 BETTER BETTER

# PIGGYBACK DETAIL

TYPE

SPANS

UP

30

3

88 5

52

Ħ >

**4**X8 2X4

5X6

5X8

586 3%5

2.5X4

2.6X4

C

1.5X3

1.5X4

1.5X4

1.5X4 **5X6** 

5X6

5X5

TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

IP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTON CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGCYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS. REFER TO BUGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:
110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED ELDG,
LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL-5 PSF, WIND BC DL-5 PSF
110 MPH WIND, 30' MEAN HGT, FBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL-5 PSF, WIND BC DL-5 PSF

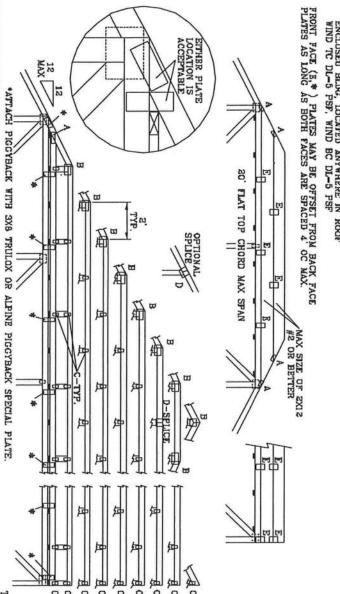
130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C. WIND TC DL=6 PSF, WIND BC DL=6 PSF

H U

> 4XB 584

OR 3X6 TRULOX AT 4'

DC,



WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T MEMBE MEMBE
10' TO 14'	2x4 "t" brace. Same grade. Species as wer Member. Or better. and 80% length of wer Member. Attach with 16d nails at 4° oc.

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.575" EQUAL, PER FACE PER PLY. (4) NAILS IN EACH ) BE CONNECTED. REFER TO DRAWING 160 TL FOR INFORMATION.

NAILS, CONTROL NEMBER

70R

TO SUPPORTING TRUSS WILLS PER FACE PER PLY. ATE TO EACH TRUSS FACE		
ATION. ATTACH TO SUPPORTING TRUSS WITH 20° X 1.375" NAILS PER FACE PER PLY. APPLY ACK SPECIAL PLATE TO EACH TRUSS FACE AND 4' OC OR LESS.	-	
TO SUPPORTING TRUSS WITH ILS PER FACE PER PLY. APPLY ATE TO EACH TRUSS FACE AND	9.5	
TO SUPPORTING TRUSS WILLS PER FACE PER PLY. ATE TO EACH TRUSS FACE		(
TO SUPPORTING TRUSS WILLS PER FACE PER PLY. ATE TO EACH TRUSS FACE	-	10
		TO SUPPORTING TRUSS WILLS PER FACE PER PLY. ATE TO EACH TRUSS FACE

DELRAY BEACH, TL. 33444-2161	1400 SW 4th AVENUE	CONS. ENGINEERS P.A.	N. H.	THIS DRAWIN
TO DOT AT	1.33 DUR. FAC.	55 PSF AT	MAX LOADING	THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045
	DRWC	DATE	REF	634,016
	DRWG MITEK STD PIGG	DATE 09/12/07	PIGGYBACK	834,017 & 847,045

MAYARHIMGAN TRUSTESS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BACING. REFER TO ESSI 1-13 SUILING CID-PONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 383 GTOOFRED 28, SUITE 200, MANISON, VI. 33759 AND AFEA COORD TRUSS COUNCIL OF AMERICA, 6300 CHEEPRISE LN, MANISON, VI. 33759 FOR SAFETY PARCTICES PRIOR TO PERFORMING THE SEFECTY AND AFEA OF THE PRIOR OF THE SAFETY AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BIGIO CEILING.

1.15	1.23	50		A. 55	NAX MAX
47 PSF 1.15 DUR.	טטא.	PSF	DUR.	PSF /	X LOA
AT FAC.	FAC.	AT	FAC.	AT	LOADING
		-ENG	DRWGMITEK	DATE	REF
		IL	MITEK	09/12/07	PIGG
			STD	2/07	PIGGYBACK
			PIGG		×

No: 34869 STATE OF FLORIDA

SPACING

24.0"

# VALLEYTRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 2X4 SP #3 OR BETTER. OR BETTER.

ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).

\*

ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCI BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. OR (3) 16d ENCLOSED FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

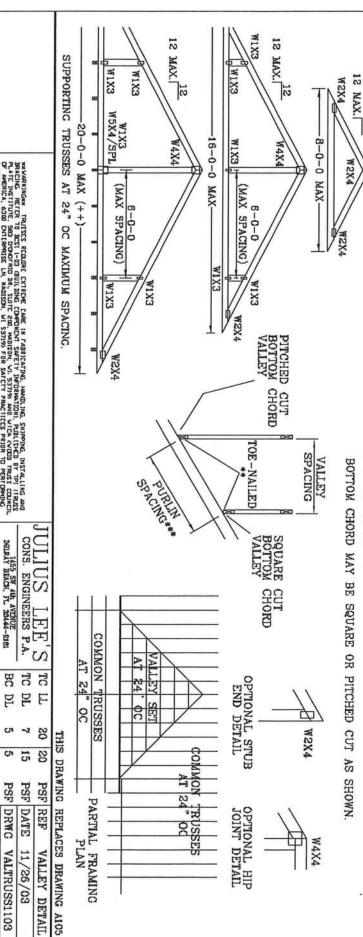
\*\*\* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD. ++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES

NOT EXCEED 12'0".

CUT FROM 2X6 OR LARGER AS REQ'D

12

4-0-0 MAX



MEMORINISME TRUSSES REQUIRE EXTROME CAME (H FABRICATING, MANDLING, SKIPPING, INSTALLING AND BACING REFER TO BEST 1-00 SOULDING COMPICKY SAFETY INFORMATION, PUBLICACID BY TPE (TRUSS PLANT INSTITUTE, SEO DOORTROD BY, SUITE 200, MANISON, V. 1857199 AND VICEA VOOD TRUSS COUNCIL OF AMERICA, GADD CONTRACKE M, MADISON, VI 1877199 FOR SAFETY PARCIFECE PRIOR TO PERFORMO THESE FUNCTIONS UNLESS OFFICIALISMS. VI 1877199 FOR SAFETY PARCIFECE PRIOR TO PERFORMO THE CONTRACKED AND STRUCTURAL PARCIFEC PRIOR OF THACKED STRUCTURAL PARCIES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACKED RIGHD CELLING.

CONS.

DELRAY BEACH, I'L 38444-2161

BC LL

0

-ENG

Ç

PSF DRWG

VALTRUSS1103 11/26/03

15

PSF DATE

32 0

40

PSF PSF

No: 34868 STATE OF FLORIDA

SPACING DUR.FAC. 1.25 TOT. LD.

24" 1.25

# TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE. END DISTANCE. SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

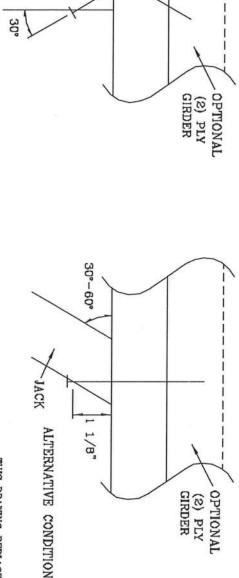
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

# MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF		SOUTHERN PINE	DOUGLAS	DOUGLAS FIR-LARCH		HEM-FIR	SPR	SPRUCE PINE FIR
TOE-NAILS	1 PLY	2 PLIES 1 PLY	1 PLY	2 PLIES	1 PLY	2 PLIES	1	PLY
ผ	197#	256#	181#	234#	156#	203#	154#	4#
မ	296#	383#	271#	351#	234#	304#	#052	<b>)</b> #
4	394#	511#	361#	468#	312#	406#	307#	7#
ch	493#	639#	452#	585#	390#	507#	384#	#
ALL VALUE	TO WAY BE	MILITIAN I	DO AR OF	ישתאומשחם:	NOTEN	ALL VALUES MAY BE WILLLIGHTED BY ADDRODRIATE DIBATION OF LOAD EACTION	A N	

5000



1/8"

THIS DRAWING REPLACES DRAWING 784040

			L PANELS AND BOTTON CHORD SHALL HAVE		HINVARDING H. TRUSSES REDUIRE EXTREME CARE IN FABRICATING, HANDLING, SUPPING, INSTALLING AND BRACING. REFER TO BOSI 1-03 COULDING COMPONENT SAFETY (MYDNATION), PUBLISHED BY FRY CRUSS	
STATE OF FLORIDA	No: 34869			DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	S, TET SOITINE
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	C. 1.00	PSF	PSF	PSF	PSF	PSF
				DRWG	DATE	REF
11			JL	CNTONAIL1103	09/12/07	TOE-NAIL

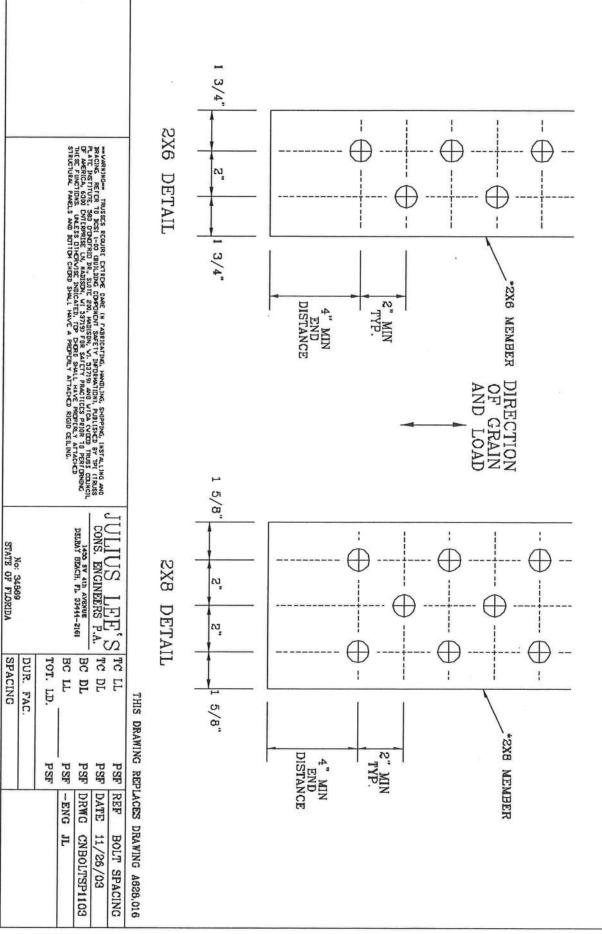
# DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

\* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



# TRULOX CONNECTION DETAIL

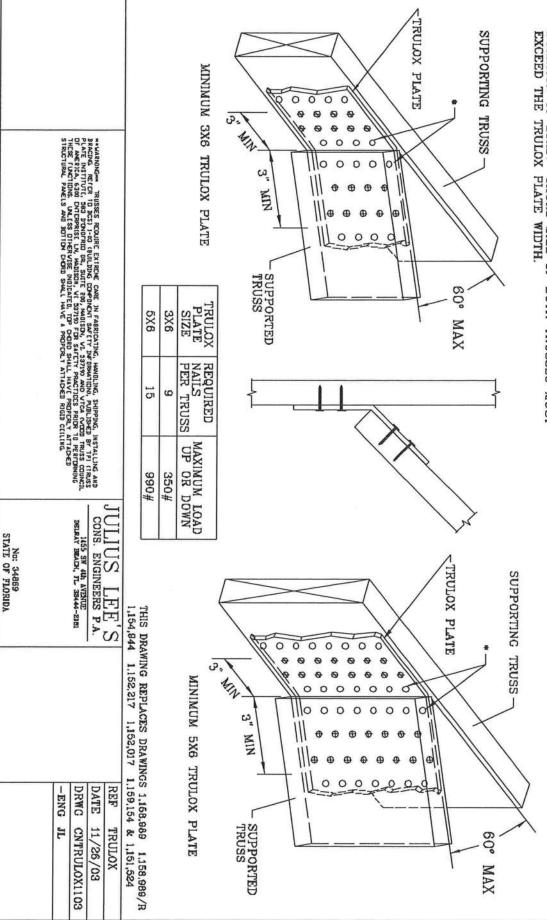
II GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\( \phi \)).

\* NAILS MAY BE OMITTED FROM THESE ROWS.

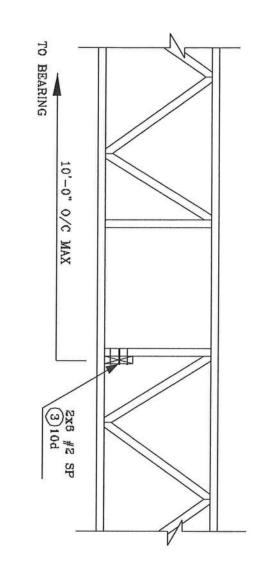
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

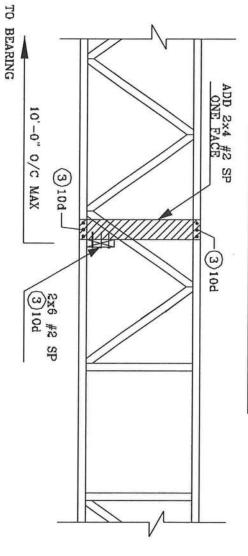
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



# STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



# ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



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