

RE: 6837 - G.C. CONST. / LOT # 6 R.R.

MiTek Industries, Inc.

14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017-5746

Site Information:

Project Customer: G.C. CONSTRUCTION Project Name: Lot # 6 River Rise
Lot/Block: Subdivision:
Address: 455 S.W. MARY NIK DR.
City: HIGH SPRING State: FL.

Name Address and License # of Structural Engineer of Record, if there is one, for the building.

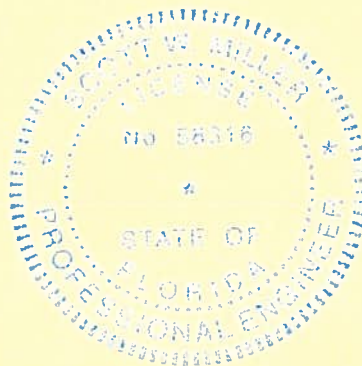
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 6.3
Wind Code: ASCE 7-02 Wind Speed: 110 mph Design Method: User defined
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 14 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date
1	I11010218	6837	T1	9/25/06
2	I11010219	6837	T1A	9/25/06
3	I11010220	6837	T1B	9/25/06
4	I11010221	6837	T1C	9/25/06
5	I11010222	6837	T1E	9/25/06
6	I11010223	6837	T2	9/25/06
7	I11010224	6837	T2A	9/25/06
8	I11010225	6837	T2B	9/25/06
9	I11010226	6837	T2D	9/25/06
10	I11010227	6837	T3	9/25/06
11	I11010228	6837	T3A	9/25/06
12	I11010229	6837	T3E	9/25/06
13	I11010230	6837	T4	9/25/06
14	I11010231	6837	T4E	9/25/06



The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by J & R Overhead.

Truss Design Engineer's Name: Miller, Scott

My license renewal date for the state of Florida is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

Scott W. Miller, FL Lic #58316
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert.#6634

September 25, 2006

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	111010218
0837	T1	PORCH	2	1		

J & R Overhead, LLC, Keystone Hgts

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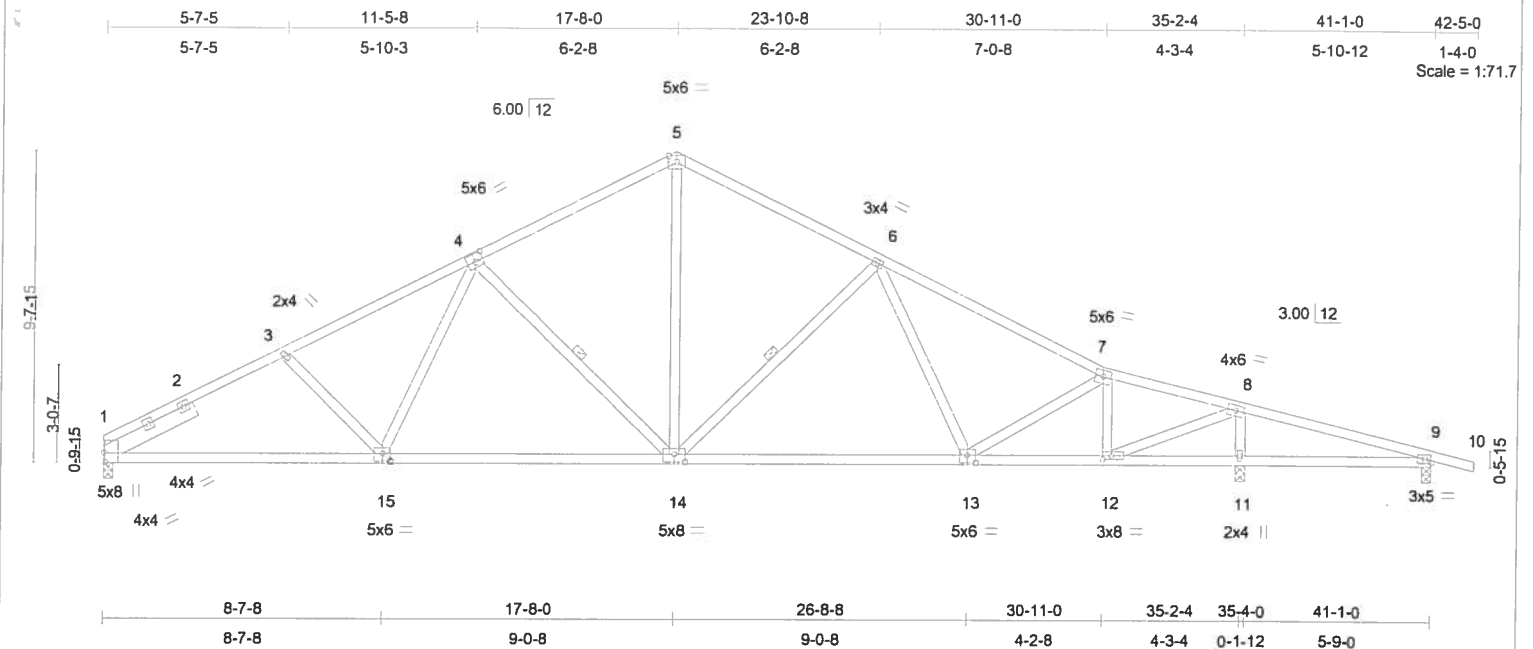


Plate Offsets (X,Y): [1:0-3-8, Edge], [4:0-3-0, 0-3-0], [12:0-3-8, 0-1-8], [13:0-3-0, 0-3-0], [14:0-4-0, 0-3-0], [15:0-3-0, 0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.48	Vert(LL)	-0.13 13-14	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.15	BC 0.55	Vert(TL)	-0.39 13-14	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.84	Horz(TL)	0.08 11	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)	Wind(LL)	0.10 14-15	>999	240	Weight: 222 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D
 WEBS 2 X 4 SYP No.3
 SLIDER Left 2 X 6 SYP SS 3-1-13

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-8-2 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 4-14, 6-14
 with 2 X 4 SYP No.3 with 2 - 10d (0.131"x3") nails and cross brace
 spacing of 20-0-0 oc.

REACTIONS (lb/size) 1=1359/0-3-8, 11=1973/0-3-8, 9=32/0-3-8
 Max Horz 1=-143(load case 3)
 Max Uplift 1=-369(load case 5), 11=-611(load case 6), 9=-216(load case 4)
 Max Grav 1=1359(load case 1), 11=1973(load case 1), 9=85(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-2291/625, 2-3=-2203/638, 3-4=-2066/604, 4-5=-1503/491, 7-8=-1452/435, 8-9=-214/936, 9-10=0/8, 5-6=-1504/496,
 6-7=-1870/527
 BOT CHORD 1-15=-585/1910, 14-15=-417/1680, 13-14=-278/1571, 12-13=-324/1426, 11-12=-850/244, 9-11=-850/244
 WEBS 3-15=-179/205, 4-15=-65/383, 4-14=-606/336, 5-14=-246/906, 6-14=-484/305, 6-13=0/197, 7-13=0/235, 7-12=-889/280,
 8-12=-597/2405, 8-11=-1808/560

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 369 lb uplift at joint 1, 611 lb uplift at joint 11 and 216 lb uplift at joint 9.

LOAD CASE(S) Standard

Signature

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September 25, 2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII 7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

MiTek
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 14515 N. Outer Forty, Suite #300
 Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	111010219
6837	T1A	PORCH	6	1		

J & R Overhead, LLC, Keystone Hgts

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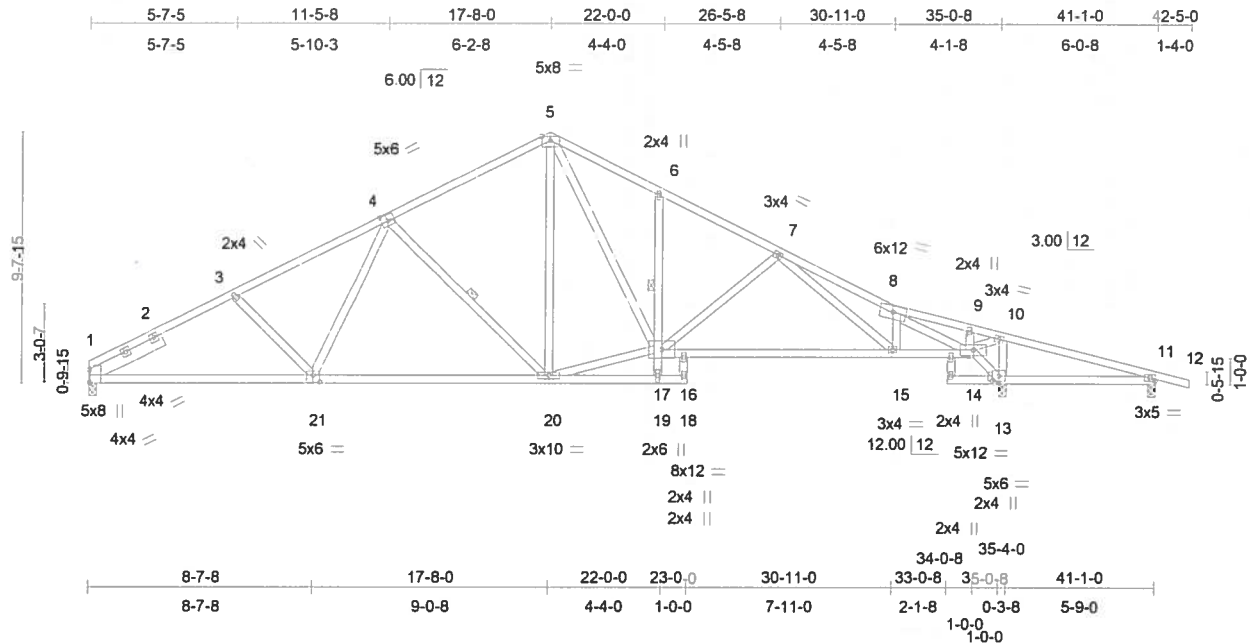


Plate Offsets (X,Y): [1:0-6-0,Edge], [4:0-3-0,0-3-0], [13:0-1-3,0-2-15], [21:0-3-0,0-3-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.46	Vert(LL)	-0.15 20-21	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.15	BC 0.53	Vert(TL)	-0.44 20-21	>961	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.64	Horz(TL)	0.12 13	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)	Wind(LL)	0.10 15-16	>999	240	Weight: 246 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2D	TOP CHORD Structural wood sheathing directly applied or 3-8-15 oc purlins.
BOT CHORD 2 X 4 SYP No.2D *Except*	BOT CHORD Rigid ceiling directly applied or 5-9-4 oc bracing. Except:
6-19 2 X 4 SYP No.3	1 Row at midpt 6-17
WEBS 2 X 4 SYP No.3	with 2 X 4 SYP No.3 with 2 - 10d (0.131"x3") nails nails and cross brace
SLIDER Left 2 X 6 SYP SS 3-1-13	spacing of 20-0-0 oc.
	WEBS 1 Row at midpt 4-20
	with 2 X 4 SYP No.3 with 2 - 10d (0.131"x3") nails nails and cross brace
	spacing of 20-0-0 oc.

REACTIONS (lb/size) 1=1334/0-3-8, 13=2093/0-3-8, 11=-64/0-3-8
Max Horz 1=-144(load case 3)
Max Uplift 1=-365(load case 5), 13=-643(load case 6), 11=-222(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2247/610, 2-3=-2158/630, 3-4=-2021/595, 4-5=-1447/480, 5-6=-1865/624, 6-7=-1904/520, 7-8=-2018/578,
8-9=-158/629, 9-10=-161/600, 10-11=-282/1305, 11-12=0/8
BOT CHORD 1-21=-577/1871, 20-21=-410/1637, 19-20=-231/0, 18-19=0/0, 17-19=0/234, 6-17=-250/196, 16-17=-340/1851,
15-16=-340/1851, 14-15=-319/1789, 13-14=-1719/462, 11-13=-1203/309
WEBS 3-21=-181/206, 4-21=-61/401, 4-20=-611/335, 5-20=-141/336, 17-20=-86/1314, 5-17=-301/925, 7-17=-296/224,
7-15=-119/89, 8-15=0/266, 8-14=-2761/678, 10-14=-76/773, 10-13=-871/280, 16-18=-150/0, 9-14=-70/5

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 365 lb uplift at joint 1, 643 lb uplift at joint 13 and 222 lb uplift at joint 11.

LOAD CASE(S) Standard

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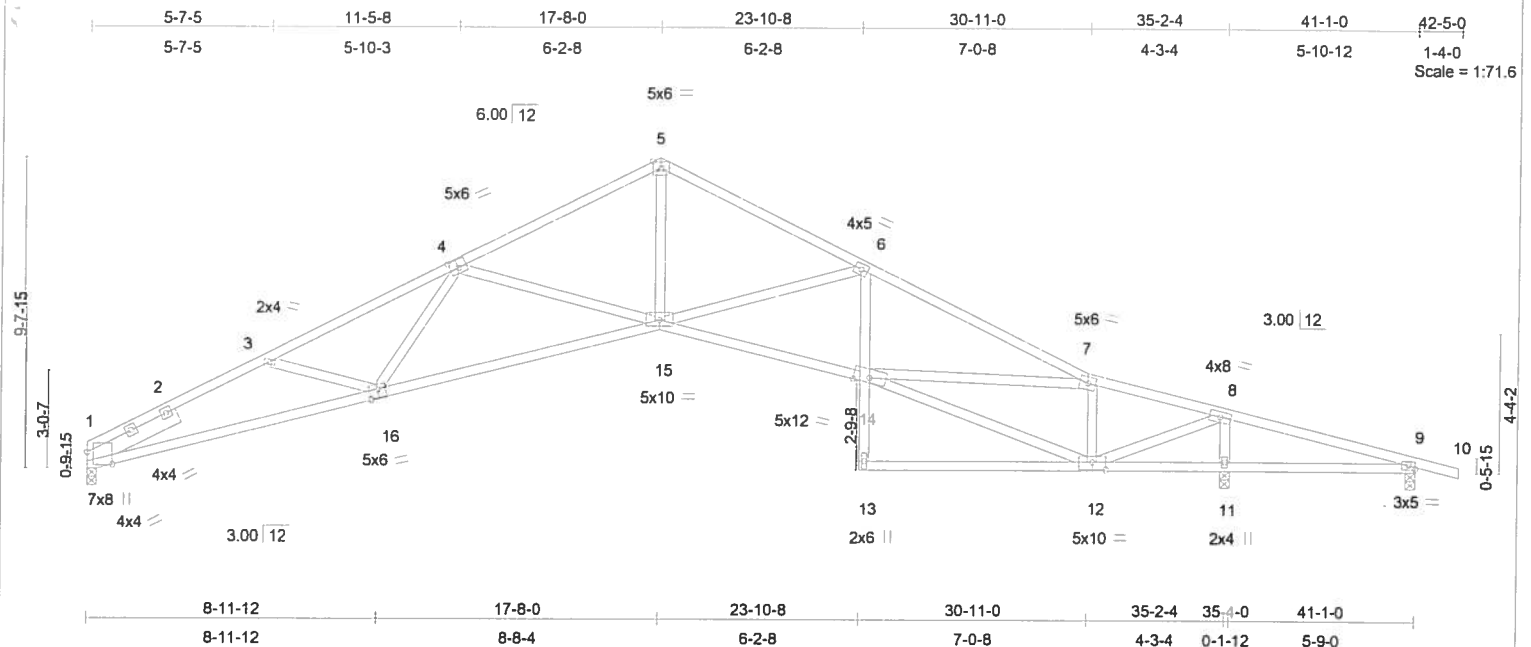
September 25,2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII 7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	I11010220
6837	T1B	PORCH	1	1	Job Reference (optional)	



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.15	TC 0.88	Vert(LL) -0.26 15-16 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.15	BC 0.90	Vert(TL) -0.75 15-16 >559 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.62	Horz(TL) 0.42 11 n/a n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)	Wind(LL) 0.26 15-16 >999 240	Weight: 218 lb	

REACTIONS (lb/size) 1=1295/0-3-8, 11=2404/0-3-8, 9=-340/0-3-8
 Max Horz 1=-143(load case 3)
 Max Uplift1=-354(load case 5), 11=-683(load case 6), 9=-340(load case 1)
 Max Gravl 1=1295(load case 1), 11=2404(load case 1), 9=26(load case 5)

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 354 lb uplift at joint 1, 683 lb uplift at joint 11 and 340 lb uplift at joint 9.

LOAD CASE(S) Standard

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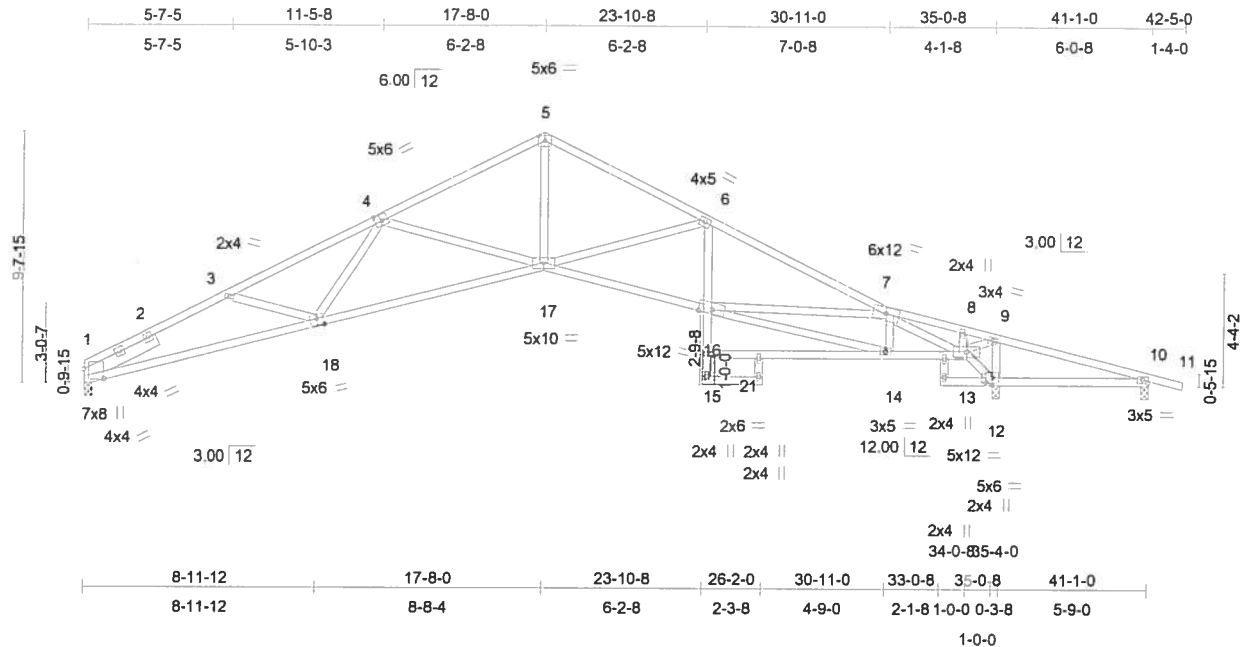
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14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	111010221
6837	T1C	PORCH	2	1	Job Reference (optional)	

J & R Overhead, LLC, Keystone Hgts

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Scale = 1:89.3

Plate Offsets (X,Y): [1:0-4-10,Edge], [4:0-3-0,0-3-0], [12:0-1-3,0-2-15], [18:0-3-0,0-3-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.89	Vert(LL)	-0.26 17-18	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.15	BC 0.91	Vert(TL)	-0.75 17-18	>560	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.75	Horz(TL)	0.42 12	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)	Wind(LL)	0.26 17-18	>999	240	Weight: 226 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2D	TOP CHORD Structural wood sheathing directly applied or 1-11-14 oc purlins.
BOT CHORD 2 X 4 SYP No.2D *Except*	BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
6-21 2 X 4 SYP No.3	
WEBS 2 X 4 SYP No.3	
SLIDER Left 2 X 6 SYP SS 3-1-12	

REACTIONS (lb/size) 1=1304/0-3-8, 12=2280/0-3-8, 10=-226/0-3-8
Max Horz 1=-143(load case 3)
Max Uplift1=-357(load case 5), 12=-667(load case 6), 10=-233(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3483/1021, 2-3=-3392/1034, 3-4=-3332/909, 4-5=-2516/612, 5-6=-2515/630, 6-7=-2947/690, 7-8=-377/1558,
8-9=-370/1512, 9-10=-373/1886, 10-11=0/8
BOT CHORD 1-18=-955/3015, 17-18=-747/2965, 16-17=-441/2660, 15-21=0/0, 15-16=0/150, 6-16=-108/97, 14-15=-1/117,
13-14=-312/1339, 12-13=-2393/548, 10-12=-1760/379
WEBS 3-18=-26/206, 4-18=-12/345, 4-17=-765/432, 5-17=-362/1806, 6-17=-536/357, 14-16=-336/1301, 7-16=-225/1197,
7-14=-285/169, 7-13=-3270/830, 9-13=0/375, 9-12=-652/250, 8-13=-59/18

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
 - 5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 357 lb uplift at joint 1, 667 lb uplift at joint 12 and 233 lb uplift at joint 10.

LOAD CASE(S) Standard

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Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	111010222
G837	T1E	PORCH	1	1	Job Reference (optional)	
J & R Overhead, LLC, Keystone Hgts						6 300 s Jul 11 2006 MiTek Industries, Inc. Mon Sep 25 14:33:47 2006 Page 1

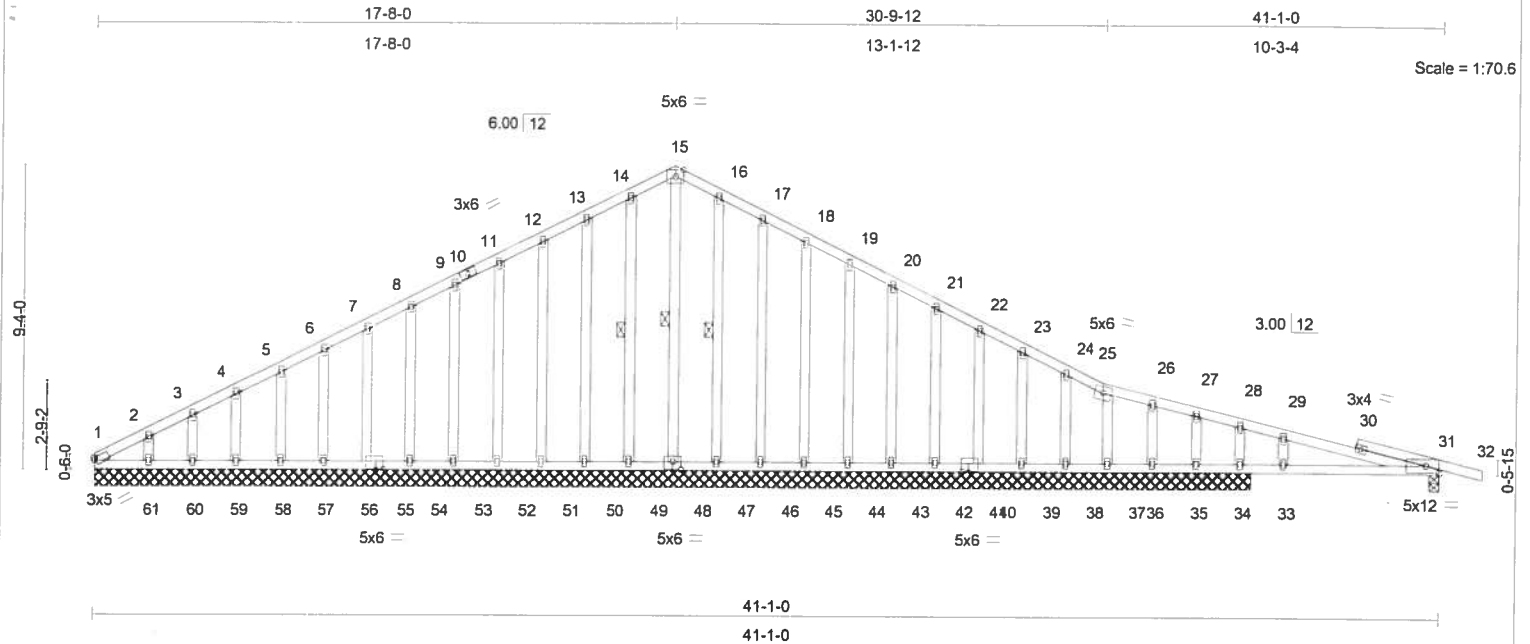


Plate Offsets (X,Y): [1:Edge,0-0-14], [31:Edge,0-2-9], [41:0-2-8,0-0-4], [48:0-3-0,0-3-0], [55:0-2-8,0-0-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.23	Vert(LL)	-0.02 31-33	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.15	BC 0.24	Vert(TL)	-0.06 31-33	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.09	Horz(TL)	0.01 31	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)	Wind(LL)	0.04 31-33	>999	240	Weight: 310 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2D	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2D	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 15-48, 14-49, 16-47
OTHERS 2 X 4 SYP No.3	with 2 X 4 SYP No.3 with 2 - 10d (0.131"x3") nails nails and cross brace
	spacing of 20-0-0 oc.

REACTIONS (lb/size) 1=49/35-4-0, 31=294/0-3-8, 48=106/35-4-0, 49=107/35-4-0, 50=107/35-4-0, 51=107/35-4-0, 52=107/35-4-0, 53=107/35-4-0, 54=107/35-4-0, 56=107/35-4-0, 57=107/35-4-0, 58=106/35-4-0, 59=108/35-4-0, 60=100/35-4-0, 61=132/35-4-0, 47=107/35-4-0, 46=107/35-4-0, 45=107/35-4-0, 44=107/35-4-0, 43=107/35-4-0, 42=107/35-4-0, 40=107/35-4-0, 39=107/35-4-0, 38=100/35-4-0, 36=179/35-4-0, 35=170/35-4-0, 34=544/35-4-0, 37=96/35-4-0

Max Horz 1=-138(load case 3)

Max Uplift 1=-23(load case 3), 31=-206(load case 4), 49=-17(load case 5), 50=-59(load case 5), 51=-51(load case 5), 52=-50(load case 5), 53=-50(load case 5), 54=-50(load case 5), 56=-50(load case 5), 57=-50(load case 5), 58=-50(load case 5), 59=-51(load case 5), 60=-46(load case 5), 61=-97(load case 5), 47=-9(load case 6), 46=-61(load case 6), 45=-51(load case 6), 44=-50(load case 6), 43=-50(load case 6), 42=-50(load case 6), 40=-50(load case 6), 39=-50(load case 6), 38=-49(load case 6), 36=-89(load case 4), 35=-170(load case 1), 34=-343(load case 4), 37=-23(load case 6)

Max Grav 1=106(load case 5), 31=294(load case 1), 48=165(load case 6), 49=107(load case 9), 50=108(load case 9), 51=107(load case 1), 52=107(load case 1), 53=107(load case 9), 54=107(load case 9), 56=107(load case 1), 57=107(load case 9), 58=106(load case 1), 59=108(load case 9), 60=100(load case 1), 61=132(load case 1), 47=108(load case 10), 46=107(load case 10), 45=107(load case 1), 44=107(load case 1), 43=107(load case 10), 42=107(load case 1), 40=107(load case 10), 39=107(load case 1), 38=100(load case 10), 36=179(load case 10), 35=145(load case 4), 34=544(load case 1), 37=96(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-219/54, 2-3=-158/62, 3-4=-125/72, 4-5=-90/82, 5-6=-56/97, 6-7=-22/121, 7-8=-20/145, 8-9=-20/169, 9-10=-20/187, 10-11=-2/193, 11-12=-20/216, 12-13=-20/241, 13-14=-20/268, 14-15=-21/274, 25-26=-51/37, 26-27=-71/23, 27-28=-68/19, 28-29=-113/37, 29-30=-130/33, 30-31=-136/8, 31-32=0/8, 15-16=-18/269, 16-17=-18/253, 17-18=-18/214, 18-19=-18/185, 19-20=-18/146, 20-21=-18/111, 21-22=-18/87, 22-23=-18/76, 23-24=-17/66, 24-25=-30/57

BOT CHORD 1-61=-3/159, 60-61=-3/159, 59-60=-3/159, 58-59=-3/159, 57-58=-3/159, 56-57=-3/159, 55-56=-3/159, 54-55=-3/159, 53-54=-3/159, 52-53=-3/159, 51-52=-3/159, 50-51=-3/159, 49-50=-3/159, 48-49=-3/159, 47-48=-3/159, 46-47=-3/159, 45-46=-3/159, 44-45=-3/159, 43-44=-3/159, 42-43=-3/159, 41-42=-3/159, 40-41=-3/159, 39-40=-3/159, 38-39=-3/159, 37-38=-3/159, 36-37=-8/162, 35-36=-8/162, 34-35=-8/162, 33-34=-8/162, 31-33=-8/162

WEBS 15-48=-152/0, 14-49=-81/30, 13-50=-81/72, 12-51=-80/64, 11-52=-80/63, 9-53=-80/64, 8-54=-80/64, 7-56=-80/64, 6-57=-80/64, 5-58=-80/64, 4-59=-81/64, 3-60=-77/62, 2-61=-92/103, 16-47=-81/22, 17-46=-81/74, 18-45=-80/64, 19-44=-80/63, 20-43=-80/64, 21-42=-80/64, 22-40=-80/64, 23-39=-81/64, 24-38=-71/60, 26-36=-110/75, 27-35=-27/73, 28-34=-259/173, 29-33=-89/64, 25-37=-80/44

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Chesterfield, MO, 63017
FL Cert.#6634

September 25, 2006

Continued on page 2



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII 7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	111010222
8837	T1E	PORCH	1	1	Job Reference (optional)	

J & R Overhead,LLC, Keystone Hgts

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NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02, 110mph (3-second gust); h=15ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed, MWFRS gable end zone; porch right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1, 206 lb uplift at joint 31, 17 lb uplift at joint 49, 59 lb uplift at joint 50, 51 lb uplift at joint 51, 50 lb uplift at joint 52, 50 lb uplift at joint 53, 50 lb uplift at joint 54, 50 lb uplift at joint 56, 50 lb uplift at joint 57, 50 lb uplift at joint 58, 51 lb uplift at joint 59, 46 lb uplift at joint 60, 97 lb uplift at joint 61, 9 lb uplift at joint 47, 61 lb uplift at joint 46, 51 lb uplift at joint 45, 50 lb uplift at joint 44, 50 lb uplift at joint 43, 50 lb uplift at joint 42, 50 lb uplift at joint 40, 50 lb uplift at joint 39, 49 lb uplift at joint 38, 89 lb uplift at joint 36, 170 lb uplift at joint 35, 343 lb uplift at joint 34 and 23 lb uplift at joint 37.

LOAD CASE(S) Standard



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Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	I11010223
6837	T2	TRIPLE FINK	1	1	Job Reference (optional)	

J & R Overhead, LLC, Keystone Hgts

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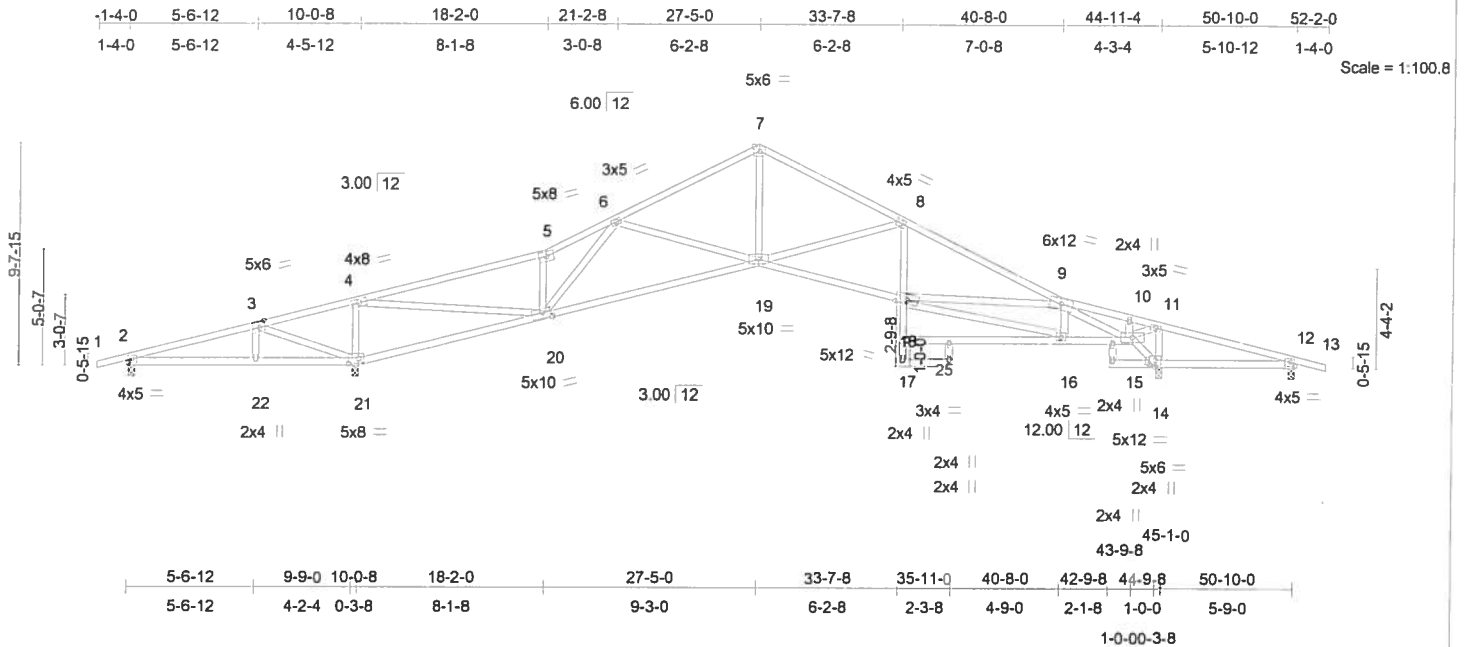


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [14:0-1-3,0-2-15], [20:0-4-8,0-3-0], [21:0-5-4,0-2-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.89	Vert(LL)	-0.19 19-20	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.15	BC 0.68	Vert(TL)	-0.62 19-20	>677	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.81	Horz(TL)	0.24 14	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)	Wind(LL)	0.17 19-20	>999	240		
								Weight: 272 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D *Except*
 8-25 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3 *Except*
 4-20 2 X 4 SYP No.2D

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-9-8 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 4-7-2 oc bracing.

REACTIONS

(lb/size) 2=-41/0-3-8, 21=2386/0-3-8, 14=1996/0-3-8, 12=-120/0-3-8
 Max Horz 21=-137(load case 6)
 Max Uplift2=-271(load case 3), 21=-824(load case 5), 14=-614(load case 6), 12=-232(load case 4)
 Max Grav2=49(load case 9), 21=2386(load case 1), 14=1996(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/8, 2-3=-149/1221, 3-4=-535/1874, 4-5=-1666/400, 5-6=-1834/547, 6-7=-1962/433, 7-8=-1971/456, 8-9=-2440/596,
 9-10=-255/1166, 10-11=-248/1116, 11-12=-286/1506, 12-13=0/8
 BOT CHORD 2-22=-1127/183, 21-22=-1135/190, 20-21=-1938/560, 19-20=-389/1846, 18-19=-313/2195, 17-25=0/0, 17-18=0/150,
 8-18=-79/82, 16-17=0/119, 15-16=-299/1283, 14-15=-1926/463, 12-14=-1395/312
 WEBS 3-22=-103/191, 3-21=-684/453, 4-21=-1550/552, 4-20=-845/3428, 5-20=-774/365, 6-20=-303/57, 6-19=-233/230,
 7-19=-189/1288, 8-19=-541/366, 16-18=-325/1241, 9-18=-110/803, 9-16=-245/161, 9-15=-2763/737, 11-15=0/396,
 11-14=-659/250, 10-15=-63/30

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02, 110mph (3-second gust); h=15ft; TCDL=5.0psf, BCDL=5.0psf, Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 271 lb uplift at joint 2, 824 lb uplift at joint 21, 614 lb uplift at joint 14 and 232 lb uplift at joint 12.

LOAD CASE(S) Standard

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 Suite 300
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September 25,2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

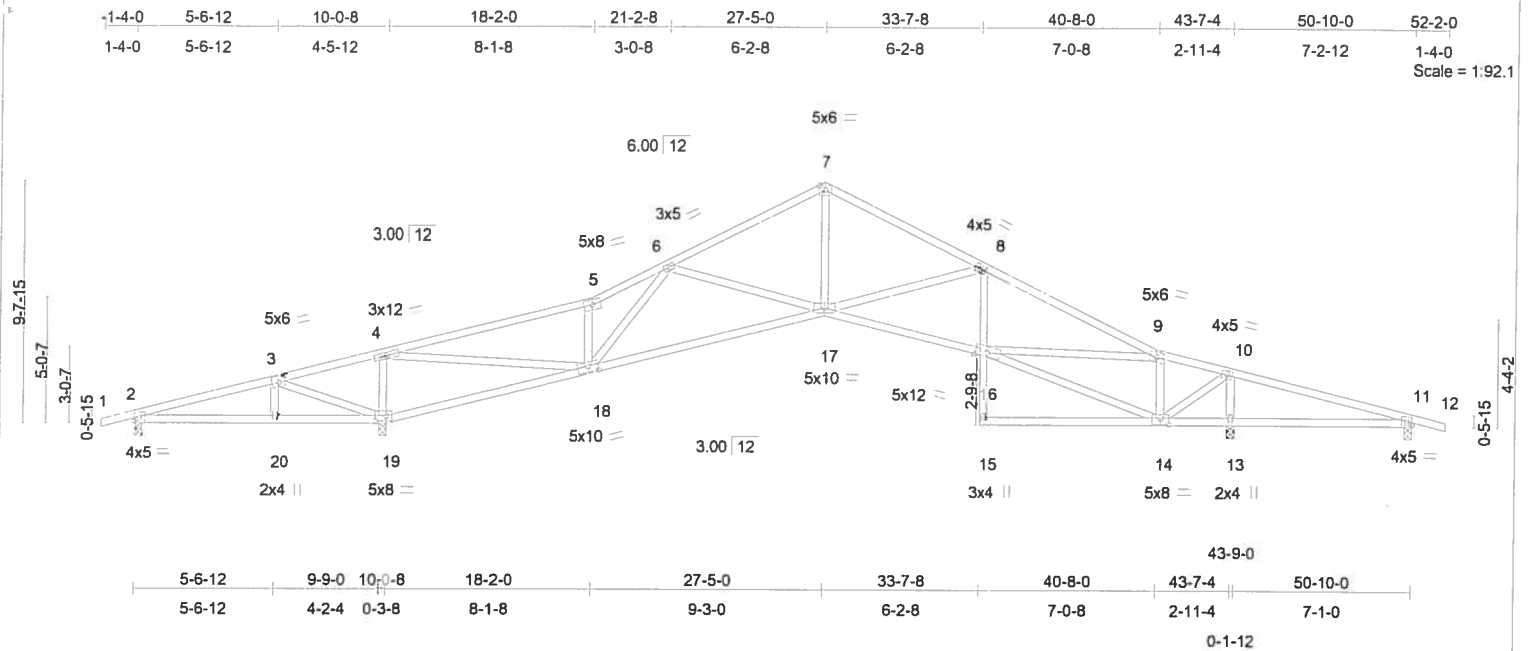
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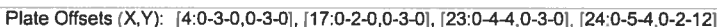
Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	111010225
6837	T2B	TRIPLE FINK	3	1	Job Reference (optional)	

J & R Overhead, LLC, Keystone Hgts

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LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
 5-23 2 X 4 SYP No.2D
LBR SCAB 3-6 2 X 4 SYP No.2D one side

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-6-9 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 4-5-10 oc bracing.
JOINTS	1 Brace at Jt(s): 20

REACTIONS (lb/size) 2=-74/0-3-8, 24=2459/0-3-8, 16=2092/0-3-8, 14=-188/0-3-8
 Max Horz 24=136(load case 5)
 Max Uplift2=-282(load case 3), 24=-813(load case 5), 16=-599(load case 6), 14=-232(load case 4)
 Max Grav 2=27(load case 9), 24=2459(load case 1), 16=2092(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/8, 2-3=-104/1221, 3-26=-92/1224, 4-26=-88/1256, 4-5=-493/1969, 5-27=-1656/390, 6-27=-1650/392, 6-7=-1591/410, 7-8=-1815/554, 8-9=-2052/433, 9-10=-1904/442, 10-11=-2748/534, 11-12=-1435/443, 12-13=-842/341, 13-14=-306/1715, 14-15=0/8
BOT CHORD	2-25=-1163/141, 24-25=-1171/148, 23-24=-2037/530, 21-23=-396/1885, 20-21=-236/2426, 19-20=0/190, 10-20=-237/1802, 19-22=0/0, 18-19=0/14, 17-18=-234/833, 16-17=-1596/314, 14-16=-1596/314
WEBS	4-25=-103/195, 4-24=-746/453, 5-24=-1578/551, 5-23=-812/3512, 7-23=-751/355, 8-23=-393/66, 8-21=-183/236, 9-21=-183/1296, 10-21=-1518/250, 18-20=-252/1760, 11-20=-66/1223, 11-18=-1327/254, 12-18=-45/397, 12-17=-878/296, 13-17=-563/2571, 13-16=-1944/539

NOTES

- 1) Attached 8-0-8 scab 3 to 6, front face(s) 2 X 4 SYP No.2D with 1 row(s) of 10d (0.131"x3") nails spaced 9" o.c. except : starting at 5-7-4 from end at joint 3, nail 1 row(s) at 7 o.c. for 2-5-4.
- 2) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 2, 813 lb uplift at joint 24, 599 lb uplift at joint 16 and 232 lb uplift at joint 14.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

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FL Cert. #6634

September 25, 2006



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MTI-7473 BEFORE USE

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



MiTek

14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	I11010226
6837	T2D	TRIPLE FINK	8	1	Job Reference (optional)	

J & R Overhead, LLC, Keystone Hgts

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-7=-60, 7-9=-60, 9-12=-60, 12-15=-60, 2-24=-20, 20-24=-20, 19-22=-20, 14-19=-20

Concentrated Loads (lb)

Vert: 22=-25(F)



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Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	111010227
6837	T3	FINK	5	1		

J & R Overhead, LLC, Keystone Hgts

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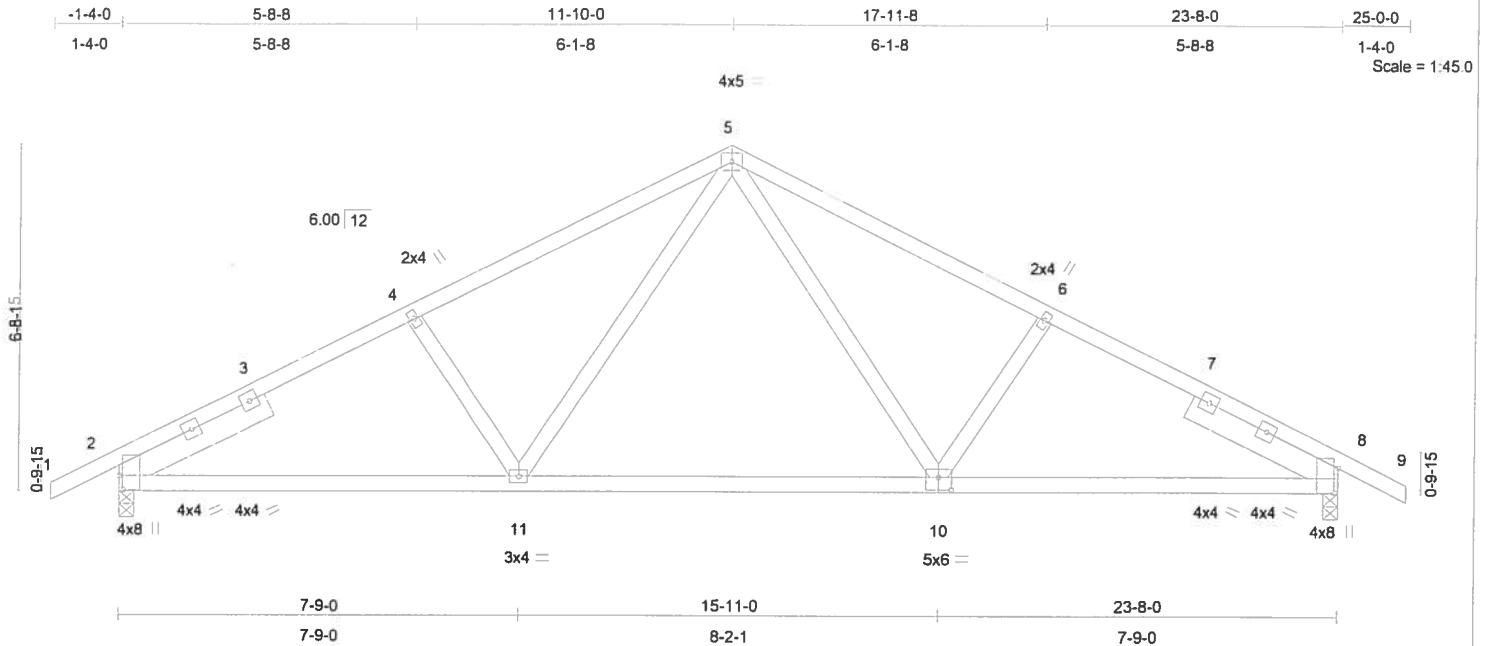


Plate Offsets (X,Y): [2:0-3-4,0-0-14], [8:0-5-12,0-0-14], [10:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.32	Vert(LL)	-0.07 10-11	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.15	BC 0.39	Vert(TL)	-0.22 10-11	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.15	Horz(TL)	0.04 8	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)	Wind(LL)	0.05 10-11	>999	240		
								Weight: 127 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP SS 3-2-8, Right 2 X 6 SYP SS 3-2-8

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1027/0-3-8, 8=1027/0-3-8
Max Horz 2=-96(load case 6)
Max Uplift 2=-330(load case 5), 8=-330(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/9, 2-3=-1486/381, 3-4=-1408/407, 4-5=-1308/411, 5-6=-1308/411, 6-7=-1408/407, 7-8=-1486/381, 8-9=0/9
BOT CHORD 2-11=-342/1223, 10-11=-145/889, 8-10=-246/1223
WEBS 4-11=-256/232, 5-11=-135/441, 5-10=-135/441, 6-10=-256/232

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 330 lb uplift at joint 2 and 330 lb uplift at joint 8.

LOAD CASE(S) Standard

Signature

Scott W. Miller, FL Lic #58316
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14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
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September 25, 2006

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Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	I11010228
6837	T3A	FINK	6	1	Job Reference (optional)	

J & R Overhead, LLC, Keystone Hgts

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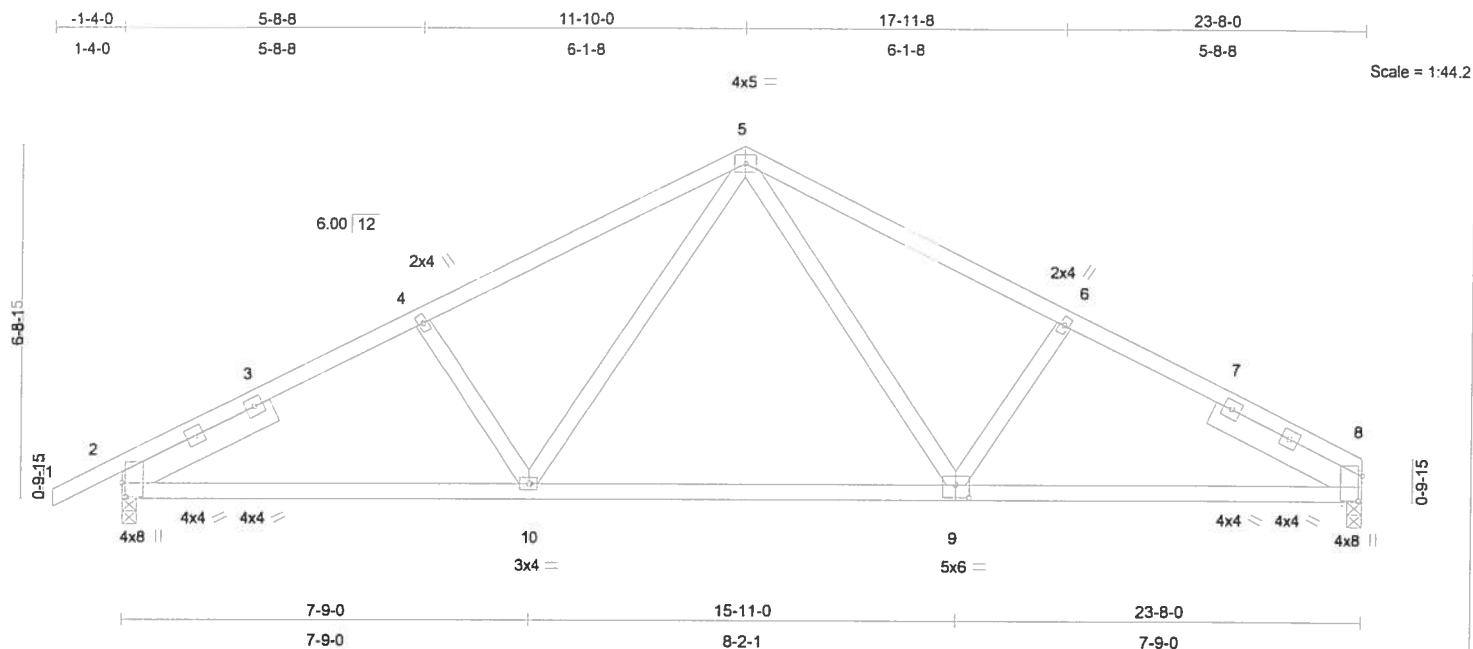


Plate Offsets (X,Y): [2:0-3-4,0-0-14], [8:0-5-12,0-0-14], [9:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.31	Vert(LL)	-0.07 9-10	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.15	BC 0.39	Vert(TL)	-0.22 9-10	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.16	Horz(TL)	0.04 8	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)	Wind(LL)	0.05 9-10	>999	240		
								Weight: 125 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D
 WEBS 2 X 4 SYP No.3
 SLIDER Left 2 X 6 SYP SS 3-2-8, Right 2 X 6 SYP SS 3-2-8

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-2-3 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=1029/0-3-8, 8=944/0-3-8
 Max Horz 2=100(load case 5)
 Max Uplift 2=-330(load case 5), 8=-255(load case 6)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/9, 2-3=-1491/382, 3-4=-1413/408, 4-5=-1312/412, 5-6=-1319/421, 6-7=-1420/418, 7-8=-1503/398
 BOT CHORD 2-10=-347/1226, 9-10=-150/893, 8-9=-269/1237
 WEBS 4-10=-256/232, 5-10=-135/441, 5-9=-145/451, 6-9=-264/239

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 330 lb uplift at joint 2 and 255 lb uplift at joint 8.

LOAD CASE(S) Standard

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September 25, 2006

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 14515 N. Outer Forty, Suite #300
 Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	111010229
6837	T3E	FINK	1	1	Job Reference (optional)	
J & R Overhead, LLC, Keystone Hgts						6.300 s Jul 11 2006 MiTek Industries, Inc. Mon Sep 25 14:33:56 2006 Page 1

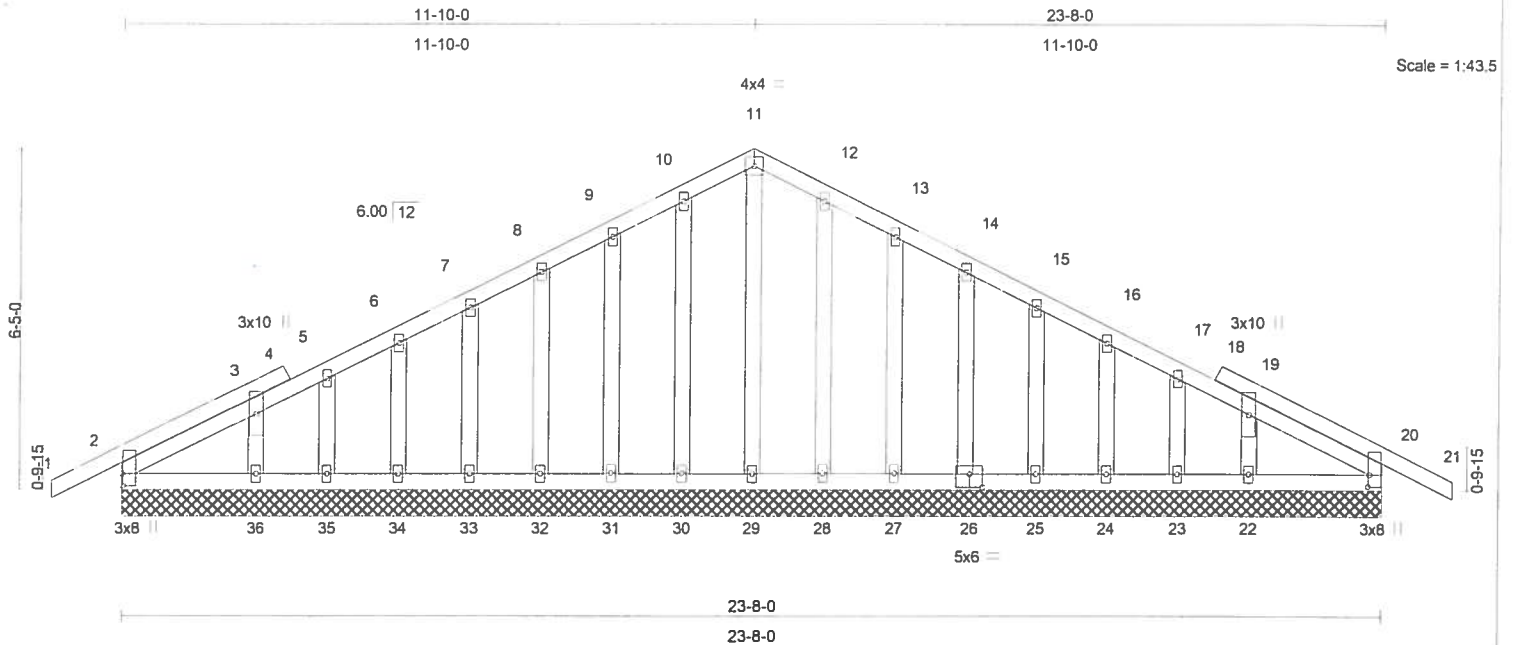


Plate Offsets (X,Y): [2:0-2-12,0-0-3], [20:0-2-12,0-0-5], [26:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.09	Vert(LL)	-0.00	21	n/r	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.15	BC 0.03	Vert(TL)	-0.01	21	n/r	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.06	Horz(TL)	0.00	20	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)	Wind(LL)	0.00	21	n/r	120		
										Weight: 165 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS (lb/size) 2=198/23-8-0, 20=197/23-8-0, 29=96/23-8-0, 30=106/23-8-0, 31=107/23-8-0, 32=107/23-8-0, 33=106/23-8-0, 34=109/23-8-0, 35=94/23-8-0, 36=152/23-8-0, 28=105/23-8-0, 27=108/23-8-0, 26=107/23-8-0, 25=105/23-8-0, 24=109/23-8-0, 23=93/23-8-0, 22=152/23-8-0
Max Horz 2=-91(load case 6)
Max Uplift 2=-56(load case 5), 20=-79(load case 6), 30=-33(load case 5), 31=-56(load case 5), 32=-50(load case 5), 33=-50(load case 5), 34=-50(load case 5), 35=-42(load case 5), 36=-77(load case 5), 28=-28(load case 6), 27=-57(load case 6), 26=-50(load case 6), 25=-52(load case 6), 24=-50(load case 6), 23=-46(load case 6), 22=-69(load case 6)
Max Grav 2=198(load case 1), 20=197(load case 1), 29=105(load case 6), 30=107(load case 9), 31=107(load case 9), 32=107(load case 1), 33=106(load case 9), 34=109(load case 9), 35=94(load case 1), 36=153(load case 9), 28=107(load case 10), 27=109(load case 10), 26=107(load case 1), 25=105(load case 10), 24=109(load case 10), 23=93(load case 1), 22=153(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/9, 2-3=-122/46, 3-4=-64/45, 4-5=-60/48, 5-6=-45/69, 6-7=-31/93, 7-8=-31/117, 8-9=-31/141, 9-10=-31/167, 10-11=-30/181, 11-12=-30/176, 12-13=-31/151, 13-14=-31/114, 14-15=-29/79, 15-16=-29/51, 16-17=-28/27, 17-18=-21/10, 18-19=-26/7, 19-20=-80/28, 20-21=0/9
BOT CHORD 2-36=0/115, 35-36=0/115, 34-35=0/115, 33-34=0/115, 32-33=0/115, 31-32=0/115, 30-31=0/115, 29-30=0/115, 28-29=0/115, 27-28=0/115, 26-27=0/115, 25-26=0/116, 24-25=0/116, 23-24=0/116, 22-23=0/116, 20-22=0/116
WEBS 11-29=92/0, 10-30=-81/46, 9-31=-81/69, 8-32=-80/63, 7-33=-80/63, 6-34=-81/65, 5-35=-72/52, 3-36=-112/100, 12-28=-81/41, 13-27=-81/71, 14-26=-81/63, 15-25=-80/64, 16-24=-81/64, 17-23=-72/55, 19-22=-112/92

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end-zone; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 1'-4" oc.

Continued on page 2

Scott W. Miller, FL Lic #58316
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Chesterfield, MO, 63017
FL Cert.#6634

September 25, 2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII 7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	I11010229
.6637	T3E	FINK	1	1	Job Reference (optional)	

J & R Overhead,LLC, Keystone Hgts

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NOTES

- *9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint 2, 79 lb uplift at joint 20, 33 lb uplift at joint 30, 56 lb uplift at joint 31, 50 lb uplift at joint 32, 50 lb uplift at joint 33, 50 lb uplift at joint 34, 42 lb uplift at joint 35, 77 lb uplift at joint 36, 28 lb uplift at joint 28, 57 lb uplift at joint 27, 50 lb uplift at joint 26, 52 lb uplift at joint 25, 50 lb uplift at joint 24, 46 lb uplift at joint 23 and 69 lb uplift at joint 22.

LOAD CASE(S) Standard



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J & R Overhead, LLC, Keystone Hgts



LUMBER TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 4-3-12 oc purlins. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. WEBS 1 Row at midpt 6-16, 8-16 with 2 X 4 SYP No.3 with 2 - 10d (0.131"x3") nails nails and cross brace spacing of 20-0-0 oc.
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FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/8, 2-3=-46/211, 3-4=-283/713, 4-5=-1605/390, 5-6=-1758/532, 6-7=-1362/451, 7-8=-1371/437, 8-9=-1754/495, 9-10=-1375/412, 10-11=-206/886, 11-12=0/8
BOT CHORD 2-20=-152/0, 19-20=-158/0, 18-19=-650/366, 17-18=-650/366, 16-17=-295/1406, 15-16=-229/1457, 14-15=-301/1350, 13-14=-802/236, 11-13=-802/236
WEBS 3-20=-102/179, 3-19=-577/426, 4-19=-1677/602, 4-17=-587/2259, 5-17=-758/360, 6-17=-137/304, 6-16=-415/240, 7-16=-203/764, 8-16=-488/307, 8-15=0/209, 9-15=0/218, 9-14=-844/265, 10-14=-565/2273, 10-13=-1724/540

LOAD CASE(S) Standard

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September 25, 2006

Jgb	Truss	Truss Type	Qty	Ply	G.C. CONST. / LOT # 6 R.R.	111010231
F837	T4E	TRIPLE FINK	1	1	Job Reference (optional)	

J & R Overhead, LLC, Keystone Hgts

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NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind ASCE 7-02; 110mph (3-second gust); h=15ft; TC DL=5.0psf, BC DL=5.0psf, Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 207 lb uplift at joint 38, 269 lb uplift at joint 2, 833 lb uplift at joint 66, 802 lb uplift at joint 65, 172 lb uplift at joint 64, 11 lb uplift at joint 63, 47 lb uplift at joint 62, 39 lb uplift at joint 61, 42 lb uplift at joint 60, 49 lb uplift at joint 59, 50 lb uplift at joint 58, 50 lb uplift at joint 57, 51 lb uplift at joint 56, 58 lb uplift at joint 55, 19 lb uplift at joint 54, 13 lb uplift at joint 52, 60 lb uplift at joint 51, 51 lb uplift at joint 50, 50 lb uplift at joint 49, 50 lb uplift at joint 48, 50 lb uplift at joint 47, 51 lb uplift at joint 46, 49 lb uplift at joint 45, 49 lb uplift at joint 44, 23 lb uplift at joint 43, 76 lb uplift at joint 42, 132 lb uplift at joint 41 and 325 lb uplift at joint 40.

LOAD CASE(S) Standard



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